DATE 06/15/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024635

APPLICANT ROGER WHIDDON PHONE 386.754.7367

ADDRESS 582 NW BROOK LOOP LAKE CITY FL 32055

OWNER H&M CONSTRUCTION PHONE 813.209.0363

ADDRESS 161 SW LIGHTER GLEN LAKE CITY FL 32024

CONTRACTOR ROGER WHIDDON PHONE 386.754.7367

LOCATION OF PROPERTY 47-S TOC-242,TR TO CANNON CREEK DR,TR TO GERALD CONNER TO LIGHTER GLEN,TL & TOWARDS THE L END OF CUL-DE-SAC.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 96600.00

HEATED FLOOR AREA 1932.00 TOTAL AREA 2661.00 HEIGHT 20.60 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03114-105 SUBDIVISION CANNON CREEK PLACE

LOT 5 BLOCK PHASE UNIT TOTAL ACRES 0.50

000001115		CRC1328025	R Whidden	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
18"X32'MITERED	05-1114-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: PLAT REQUIRES 1ST. FLOOR ELEVATION TO BE 106.0 FT. ELEVATION LETTER REQUIRED. SEE ATTACHED LETTER FROM PREVIOUS CONTRACTOR TO WARRANT NEW PERMIT ISSUANCE. Check # or Cash 2122

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 485.00 CERTIFICATION FEE $ 13.30 SURCHARGE FEE $ 13.30

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 611.60

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-

For Office Use Only Application # 0606-50 Date Received 6/15 By JW Permit # 1115/24635

Application Approved by - Zoning Official BLK Date 15.06.06 Plans Examiner OK JTH Date 6-15-06

Flood Zone X per plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Dev

Comments SEE ATTACHED 2ND PAGE (1ST APPLICATION SUBMITTED)

Plat Requires 1ST Floor Elevation to be 106.0 St. Elevation Letter Required * NOC*

Applicants Name Roger Whiddon. Fax: SAME Phone 386-754-7367

Address 582 NW Brook Loop Cell 386-867-0812

Owners Name H & M Construction John Poulos Phone 813-209-0363

911 Address 161 S.W. Lighter Glen, Lake City FL 32024

Contractors Name Whiddon Construction Co. Inc. Phone 386-754-7367

Address 582 NW Brook Loop, Lake City FL 32055

Fee Simple Owner Name & Address None

Bonding Co. Name & Address None

Architect/Engineer Name & Address Daniel Shaheen, Lake City 365-1892

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Ener

Property ID Number 24-4S-16-03114-105 Estimated Cost of Construction: 135000

Subdivision Name Cannon Creek Place: Lot 5 Block ____ Unit ____ Phase ____

Driving Directions 47 S to C-242 TR to SW Cannon Creek Dr. TR. To Gerald Court Dr TR then TL on SW Lighter Glen towards the end to the Cul-de-sac

Type of Construction New SFR Number of Existing Dwellings on Property 0

Total Acreage .5 Acre Lot Size ½ Acre Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Dri

Actual Distance of Structure from Property Lines - Front 60' Side 60' Side 60' Rear 80'

Total Building Height 20'-6" Number of Stories 1 Heated Floor Area 1932 SF Roof Pitch 6-12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards o all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

R Whidden

Owner Builder or Agent (Including Contractor)

R Whidden

Contractor Signature

Contractors License Number CRC1328025

Competency Card Number

NOTARY STAMP/SEAL

Notary Public State of Florida Tracy L Duckett My Commission DD Expires 08/31/2009

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 15 day of June 20 06.

Personally known ____ or Produced Identification ✓

FL DL

Tracy L Duckett

Notary Signature

OR # 2121 -

Columbia County Building Permit Application

left message 12/2/05

Revised 9-23-04

For Office Use Only Application # 0511-103 Date Received 11/29/05 By G Permit # 9131 23947

Application Approved by - Zoning Official BLK Date 01.12.05 Plans Examiner OK JTH Date 12-7-05

Flood Zone X Per PLAT Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES Low Dev.

Comments Plat Requires 1st Floor Elevation to be 106.0 ft. Elevation Letter Required

Applicants Name Hugo Escalante Phone 386-288-8666

Address 6210 S.W. CR 18, Ford White, FL 32038

Owners Name H&M Construction John Poulos Jr Phone 813-209-0363

911 Address 161 S.W. Lighter Glen, Lake City, FL 32025

Contractors Name Hugo Escalante, EWPL INC Phone 386-288-8666

Address 6210 S.W. CR 18 Ft White, FL 32038

Fee Simple Owner Name & Address None

Bonding Co. Name & Address None

Architect/Engineer Name & Address Daniel Shaheen, Lake City, FL 365-1892

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 34-4S-16-03114-105 Estimated Cost of Construction 135,000 -

Subdivision Name Cannon Creek Place Lot 5 Block ___ Unit ___ Phase ___

Driving Directions 47S TO C-252 TR TO SW CANNON CREEK DR. TR TO Gerald Cowen Dr. TR. SO LEFT to SW Lighter Glen Towards the left. END Cul-de-Sac.

Type of Construction New SFR Number of Existing Dwellings on Property 0

Total Acreage .5 Acre Lot Size 1/2 Acre Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 60'/ Side 60'/ Side 60'/ Rear 80'/

Total Building Height 20'-6" Number of Stories 1 Heated Floor Area 1932 SF Roof Pitch 6-12

Porches 227 GARAGE 502 Living 1932 TOTAL 2661

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Hugo Escalante

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

CARRIE L. REVELLE
MY COMMISSION # DD 181697
EXPIRES: February 11, 2007
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me this 29th day of November 2005.

Personally known X or Produced Identification ___

Hugo Escalante

Contractor Signature

Contractors License Number CRC1326967

Competency Card Number ___

NOTARY STAMP/SEAL

Carrie L. Revelle

Notary Signature

H & M Construction Corp.

10155 COLLINS AVE.
SUITE# 1004
MIAMI BEACH, FL. 33154

Phone 305-866-7031
Fax 305-865-8460

June 1, 2006

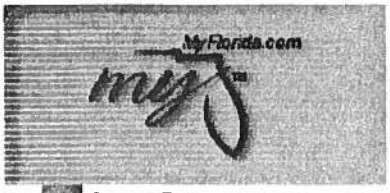
Columbia County Building Department

To whom it may concern,

This letter is to inform the Columbia County Building Department of our intention to replace our current contractor for our project on 161 SW Lighter Glenn located in Cannon Creek Place subdivision, permit number 23942. Our current contractor is Hugo Escalante of EWPL telephone # 386 288-8666 and he is being relaced with Roger Whiddon of Whiddon Construction located at 582 NW Brook Loop Lake City, Fl 32055 telephone # 386 984-5588. If you require any further information please contact us at 305 866-7031.

Sincerely,

Raymond Morel Slate



Log On

DBPR Home | Online Services Home | Help | Site Map

11:52:12 AM

Public Services
Search for a Licensee
Apply for a License
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Apply to Retake Exam
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File a Complaint
AB&T Delinquent Invoice & Activity List Search

User Services
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Change License Status
Maintain Account
Change My Address
View Messages
Change My PIN
View Continuing Ed

Term Glossary
Online Help

Licensee Details

Licensee Information

Name:	**WHIDDON, ROGER DOUGLAS (Primary Name)** **(DBA Name)**
Main Address:	**582 NW BROOK LOOP** **LAKE CITY Florida 32055-8965**
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1328025**
Status:	**Current,Active**
Licensure Date:	**06/24/2005**
Expires:	**08/31/2006**
Special Qualifications	**Qualification Effective**
Qualified Business License Required	**06/24/2005**

View Related License Information

View License Complaint

| Terms of Use | | Privacy Statement |

EWPL INC

P.O. Box 280
Fort White Fl 32038
386-288-8666

June 6, 2006

Columbia County Building
and Zoning Department.

Dear Sir or Madam:

Please be advised that I'm canceling permit # 23942. I will be releasing all related information to the new contractor: Whiddon Construction Company. If there any further question please contact me at 386288-8666. Please refund the balance of the permit fee.

Sincerely,

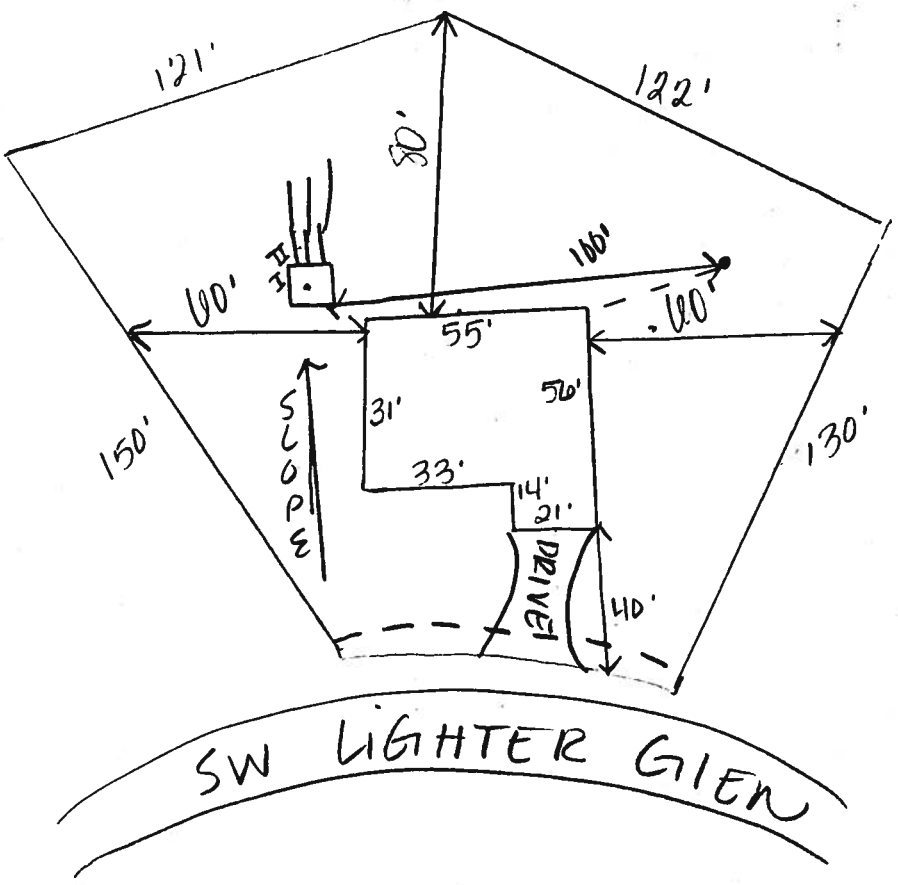
Hugo Escalante

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1114N

---------- PART II - SITEPLAN ----------

e: 1 inch = 50 feet.



ks: _____

Plan submitted by: _____

Approved ✓ Not Approved ____

MASTER CONTRACTOR

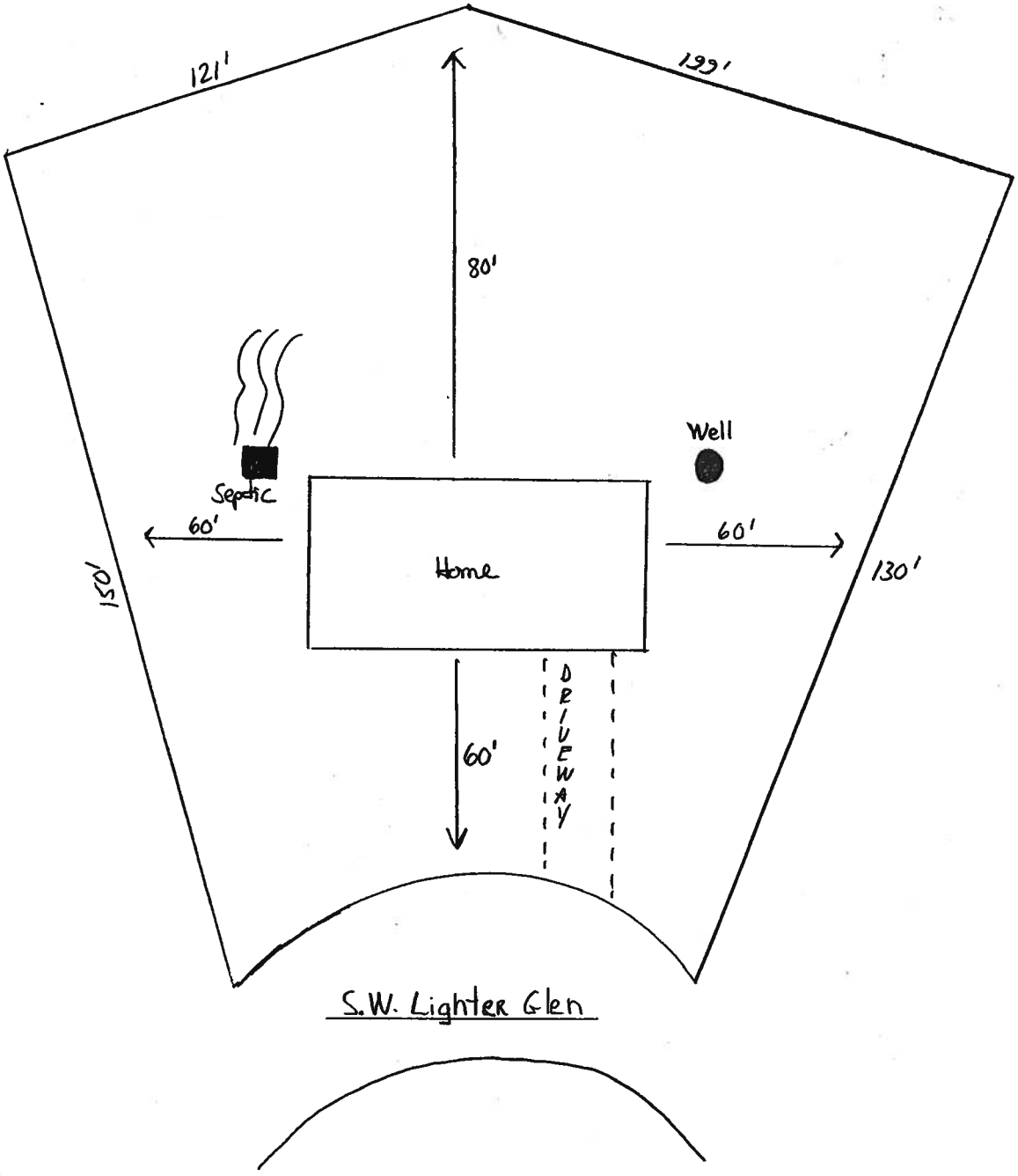
Date OCT 26 2005

Sallie Braddy · ESI · COLUMBIA

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Lot 5 Canoe Creek S/D
WD 1056-3031
Parcel # 34-45-16-03114-105



Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001115

DATE 06/15/2006 PARCEL ID # 24-4S-16-03114-105

APPLICANT ROGER WHIDDON PHONE 386.754.7367

ADDRESS 582 NW BROOK LOOP LAKE CITY FL 32055

OWNER H&M CONSTRUCTION PHONE 813.209.0363

ADDRESS 161 SW LIGHTER GLEN LAKE CITY FL 32024

CONTRACTOR ROGER WHIDDON PHONE 386.754.7367

LOCATION OF PROPERTY 47-C-242,TR TO CANNON CREEK DR TO GERALD CONNER,TR TO LIGHTER GLN,TL & TOWARDS THE END OF CUL-DE-SAC ON THE L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT CANNON CREEK PLACE 5

SIGNATURE R Whidden

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

RETURN TO: ELAINE GONZALEZ - COLUMBIA BANK - P O BOX 1609 LAKE CITY FL 32056

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 24 4S 16 03114 105

Permit #24635

1. Description of property: (legal description of the property and street address or 911 address)
LOT #5 Cannon Creek Place a Subdivision according to Plat Book 8 Pages 21 - 24 of the Public Records of Columbia Co FL.

2. General description of improvement: Single Family Residence

3. Owner Name & Address Hdm Construction Corp. 10155 Collins Ave #1001 Bal Harbour FL Interest in Property 33154 - Fee Simple

4. Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name Whiddon Construction & Ins. Phone Number
Address 582 NW Brook Loop Lake City FL 32055

6. Surety Holders Name Phone Number
Address
Amount of Bond

7. Lender Name Columbia Bank Phone Number 754-8801 x 102
Address 173 NW Hillsboro Street Lake City FL 32055

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:
Name Phone Number
Address

9. In addition to himself/herself the owner designates of
to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –
(a) 7. Phone Number of the designee

10. Expiration date of the Notice of Commencement (th
(Unless a different date is specified)

Inst:2006017701 Date:07/27/2006 Time:10:29
DC,P.DeWitt Cason,Columbia County B:1090 P:241

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Sworn to (or affirmed) and subscribed before day of June 26, 2006

NOTARY STAMP/SEAL

DESIDER KELLERMANN
MY COMMISSION # DD 383097
EXPIRES: January 3, 2009
Bonded Thru Notary Public Underwriters

Signature of Owner

Signature of Notary

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Shaw Teague
Deputy Clerk
Date 07-27-2006

CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

24635

Donald F. Lee & Associates, Inc.

Surveyors & Engineers

140 NW Ridgewood Avenue
Lake City, Florida 32055
(386) 755-6166
Fax (386) 755-6167
donald@dlfa.com

Wednesday, December 27, 2006

FROM: Tim Delbene, P.L.S.

TO: Columbia County Building & Zoning Dept.

CC: H&M Construction

RE: Foundation Elevation Check – Lot 5, Cannon Creek Place

We have obtained elevations on a foundation under construction on the above referenced lot (Date of Survey: 8/4/2006). The elevations are based on Local Benchmark Datum. The results are as follows:

Floor Elevation (at Stemwall):	**108.91'**
Garage Floor Elevation:	**108.22'**

The record subdivision plat for Cannon Creek Place indicates a minimum floor elevation of 106.00' for the subject Lot 5.

SIGNED: Timothy A. Delbene

Timothy A. Delbene, P.L.S.
Florida Reg. Cert. No. 5594

DATE: 12/27/2006.

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise. # 24635 # 24864

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB109476 Company Phone No. 386-755-3611
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Roger Whidden Const Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 1615 SW Zig Zag Terr Blvd Lake City FL

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside 0 Inside 0 Type of Fill 0

Section 4: Treatment Information

Date(s) of Treatment(s) 9-21-06
Brand Name of Product(s) Used Bora-Care
EPA Registration No. 64405-1
Approximate Final Mix Solution % 23%
Approximate Size of Treatment Area: Sq. ft. 2661 Linear ft. 230 Linear ft. of Masonry Voids 230
Approximate Total Gallons of Solution Applied 5
Was treatment completed on exterior? ☒ Yes ☐ No
Service Agreement Available? ☒ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) Steve Brannen Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Steve Brannen Date 9-21-06

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03114-105 **Building permit No.** 000024635

Use Classification SFD/UTILITY **Fire:** 50.22

Permit Holder ROGER WHIDDON **Waste:** 150.75

Owner of Building H&M CONSTRUCTION **Total:** 200.97

Location: 161 SW LIGHTER GLEN(CANNON CREEK PL., LOT 5)

Date: 01/10/2007

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 1787 * Lake City, FL 32056-1787
Telephone: (386) 758-1125 * FAX (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

To: Mr. John Kerce, Building and Zoning Coordinator

Fr: Ronal Croft, 9-1-1 Addressing

Dt: August 25, 2005

Re: 9-1-1 Addressing of "Cannon Creek Place" Subdivision.

Please find attached 9-1-1 Addressing data for Cannon Creek Place Subdivision in Section 24, Township 4 South, Range 16 East.

Please contact us at Telephone Number 758-1125 if there are any questions concerning the addressing of this subdivision.

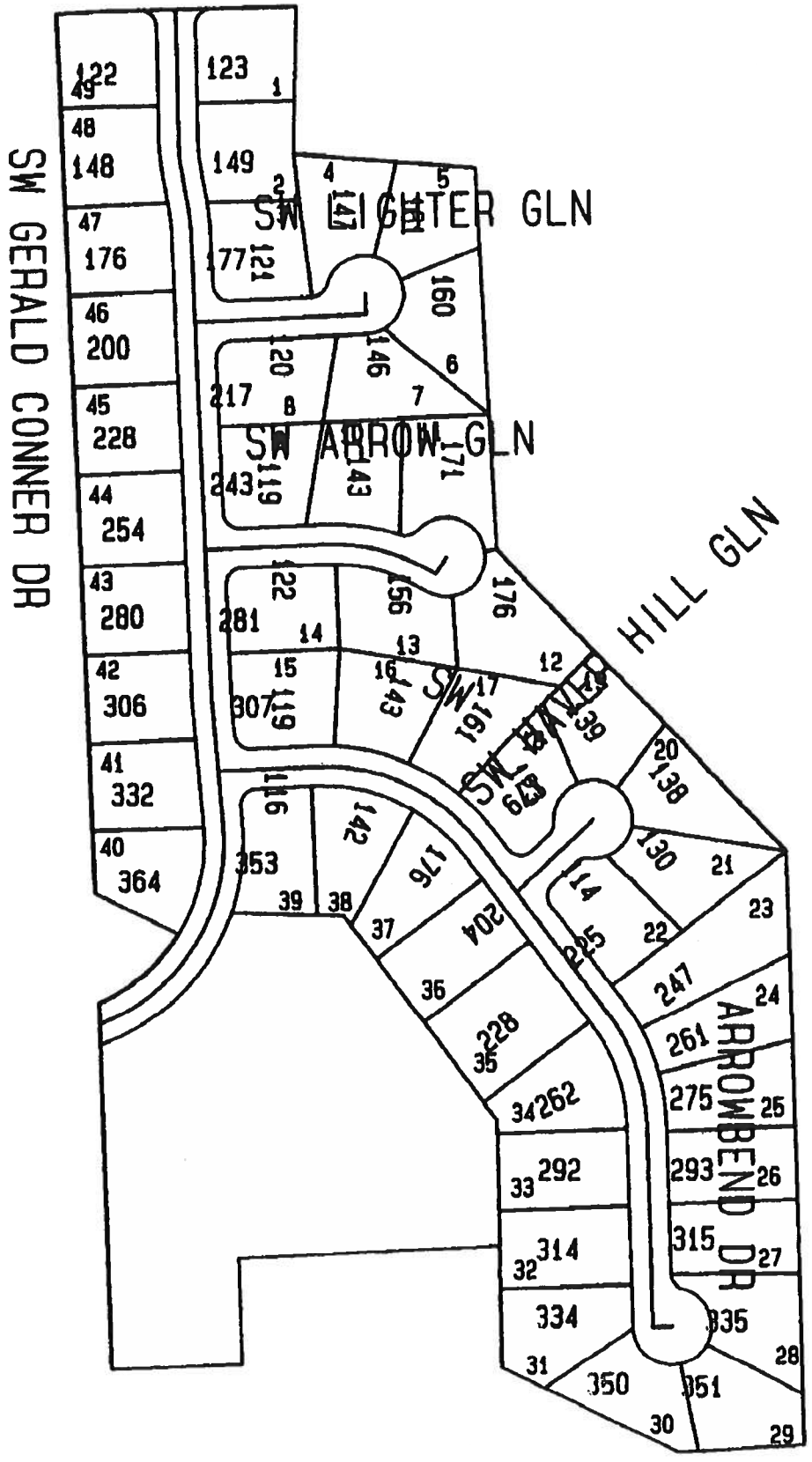
XC: Environmental Health Department
Lake City Post Office
George Johnson, Bell South
Larry Cook, Property Appraiser's Office
File

Columbia County 9-1-1 Addressing / GIS Department
Address Assignment Data
Cannon Creek Place Subdivision, Section 24, Township 4 South, Range 16 East
Columbia County, Florida

LOT#:	ADDRESS ASSIGNED
1	123 SW GERALD CONNER DR
2	149 SW GERALD CONNER DR
3*	177 SW GERALD CONNER DR
3*	121 SW LIGHTER GLN
4	147 SW LIGHTER GLN
5	161 SW LIGHTER GLN
6	160 SW LIGHTER GLN
7	146 SW LIGHTER GLN
8*	120 SW LIGHTER GLN
8*	217 SW GERALD CONNER DR
9*	243 SW GERALD CONNER DR
9*	119 SW ARROW GLN
10	143 SW ARROW GLN
11	171 SW ARROW GLN
12	176 SW ARROW GLN
13	156 SW ARROW GLN
14*	122 SW ARROW GLN
14*	281 SW GERALD CONNER DR
15*	307 SW GERALD CONNER DR
15*	119 SW ARROWBEND DR
16	143 SW ARROWBEND DR
17	161 SW ARROWBEND DR
18*	179 SW ARROWBEND DR
18*	123 SW HAVER HILL GLN
19	139 SW HAVER HILL GLN
20	138 SW HAVER HILL GLN
21	130 SW HAVER HILL GLN
22*	114 SW HAVER HILL GLN
22*	225 SW ARROWBEND DR
23	247 SW ARROWBEND DR
24	261 SW ARROWBEND DR
25	275 SW ARROWBEND DR
26	293 SW ARROWBEND DR
27	315 SW ARROWBEND DR
28	335 SW ARROWBEND DR
29	351 SW ARROWBEND DR
30	350 SW ARROWBEND DR
31	334 SW ARROWBEND DR
32	314 SW ARROWBEND DR
33	292 SW ARROWBEND DR
34	262 SW ARROWBEND DR
35	228 SW ARROWBEND DR
36	204 SW ARROWBEND DR
37	176 SW ARROWBEND DR
38	142 SW ARROWBEND DR
39*	116 SW ARROWBEND DR
39*	353 SW GERALD CONNER DR
40	364 SW GERALD CONNER DR
41	332 SW GERALD CONNER DR
42	306 SW GERALD CONNER DR
43	280 SW GERALD CONNER DR
44	254 SW GERALD CONNER DR
45	228 SW GERALD CONNER DR
46	200 SW GERALD CONNER DR
47	176 SW GERALD CONNER DR
48	148 SW GERALD CONNER DR
49	122 SW GERALD CONNER DR

(NOTE: * IDENTIFIES CORNER LOTS. CONTACT THE 9-1-1 ADDRESSING DEPARTMENT FOR CORRECT ADDRESS.)

Columbia County 9-1-1 Addressing / GIS Department
August 25, 2005
Cannon Creek Place Subdivision Address Assignments
Scale: 1 inch = 250 feet



Lot 5 Cannon Creek Place

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

Roof System:

- a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment
g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
   (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

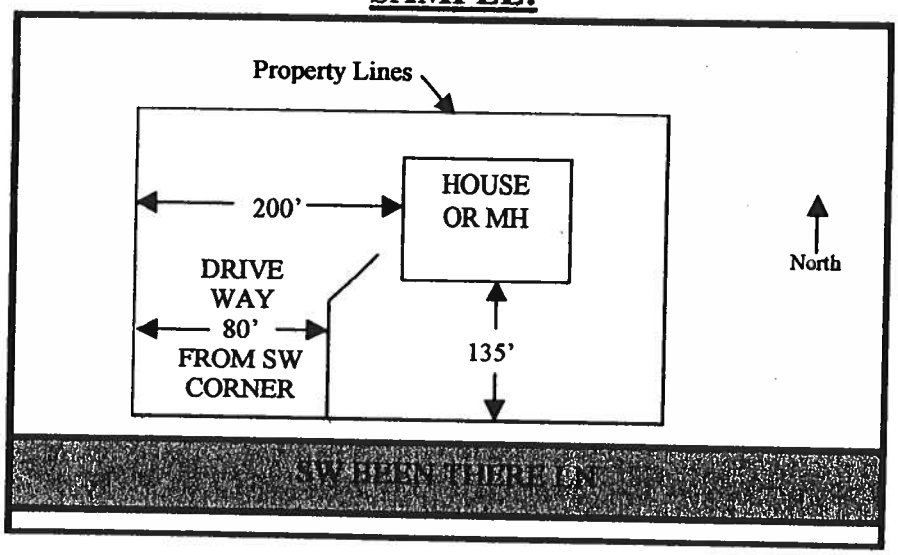
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

Residential System Sizing Calculation

Summary

EWPL INC
Lot 5 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

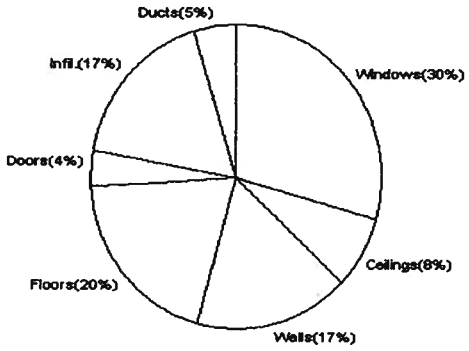
10/11/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**32409**	**Btuh**	**Total cooling load calculation**	**31653**	**Btuh**
Submitted heating capacity	36000	Btuh	Submitted cooling capacity	36000	Btuh
Submitted as % of calculated	111.1	%	Submitted as % of calculated	113.7	%

WINTER CALCULATIONS

Winter Heating Load (for 1932 sqft)

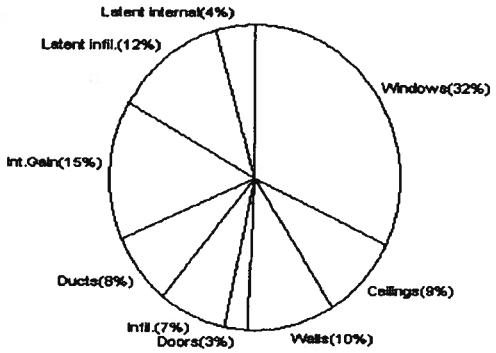
Load component			Load	
Window total	339	sqft	9594	Btuh
Wall total	1892	sqft	5517	Btuh
Door total	80	sqft	1260	Btuh
Ceiling total	1932	sqft	2512	Btuh
Floor total	204	ft	6446	Btuh
Infiltration	129	cfm	5537	Btuh
Subtotal			**30866**	**Btuh**
Duct loss			1543	Btuh
TOTAL HEAT LOSS			**32409**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1932 sqft)

Load component			Load	
Window total	339	sqft	10254	Btuh
Wall total	1892	sqft	3130	Btuh
Door total	80	sqft	798	Btuh
Ceiling total	1932	sqft	2743	Btuh
Floor total			0	Btuh
Infiltration	113	cfm	2236	Btuh
Internal gain			4800	Btuh
Subtotal(sensible)			**23961**	**Btuh**
Duct gain			2396	Btuh
Total sensible gain			**26357**	**Btuh**
Latent gain(infiltration)			3916	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**5296**	**Btuh**
TOTAL HEAT GAIN			**31653**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: _____
DATE: 10-11-05

Manual J Winter Calculations

Residential Load - Component Details (continued)

EWPL INC
Lot 5 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

10/11/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Winter

Residential Load - Component Details

EWPL INC
Lot 5 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

10/11/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	84.0	28.3	2377 Btuh
2	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
3	2, Clear, Metal, DEF	E	30.0	28.3	849 Btuh
4	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	SW	21.0	28.3	594 Btuh
6	2, Clear, Metal, DEF	S	70.0	28.3	1981 Btuh
7	2, Clear, Metal, DEF	N	16.0	28.3	453 Btuh
8	2, Clear, Metal, DEF	W	21.0	28.3	594 Btuh
9	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
10	2, Clear, Metal, DEF	S	42.0	28.3	1189 Btuh
	Window Total		339		9594 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	232	1.6	371 Btuh
2	Frame - Exterior	13.0	1660	3.1	5146 Btuh
	Wall Total		1892		5517 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		20	17.9	359 Btuh
2	Wood - Adjac		20	9.2	184 Btuh
3	Wood - Exter		40	17.9	718 Btuh
	Door Total		80		1260Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1932	1.3	2512 Btuh
	Ceiling Total		1932		2512Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	204.0 ft(p)	31.6	6446 Btuh
	Floor Total		204		6446 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	19320(sqft)	129	5537 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			129	5537 Btuh

Totals for Heating	**Subtotal**	**30866 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1543 Btuh**
	Total Btuh Loss	**32409 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

EWPL INC
Lot 5 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

10/11/2005

Totals for Cooling	**Subtotal**	**23961**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**2396**	**Btuh**
	Total sensible gain	**26357**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**3916**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**31653**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC
Lot 5 Cannon Creek
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 10/11/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, N, N	N	1.5	7.5	84.0	0.0	84.0	22	22	1848 Btuh
2	2, Clear, DEF, N, N	N	6	3	12.5	0.0	12.5	22	22	275 Btuh
3	2, Clear, DEF, N, N	E	1.5	5.5	30.0	4.5	25.5	22	72	1936 Btuh
4	2, Clear, DEF, N, N	S	1.5	7	30.0	30.0	0.0	22	37	660 Btuh
5	2, Clear, DEF, N, N	SW	8	7.5	21.0	21.0	0.0	22	62	462 Btuh
6	2, Clear, DEF, N, N	S	8	8	70.0	35.0	35.0	22	37	2065 Btuh
7	2, Clear, DEF, N, N	N	1.5	6	16.0	0.0	16.0	22	22	352 Btuh
8	2, Clear, DEF, N, N	W	1.5	7.5	21.0	1.1	19.9	22	72	1456 Btuh
9	2, Clear, DEF, N, N	N	1.5	3	12.5	0.0	12.5	22	22	275 Btuh
10	2, Clear, DEF, N, N	S	1.5	8	42.0	42.0	0.0	22	37	924 Btuh
	Window Total				339					10254 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Adjacent	13.0	232.0	1.0	241 Btuh
2	Frame - Exterior	13.0	1660.0	1.7	2888 Btuh
	Wall Total		1892.0		3130 Btuh

Doors	Type	Area	HTM	Load
1	Wood - Exter	20.0	10.0	200 Btuh
2	Wood - Adjac	20.0	10.0	200 Btuh
3	Wood - Exter	40.0	10.0	399 Btuh
	Door Total	80.0		798 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1932.0	1.4	2743 Btuh
	Ceiling Total		1932.0		2743 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	204.0 ft(p)	0.0	0 Btuh
	Floor Total		204.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.35	19320	112.9	2236 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			113	2236 Btuh

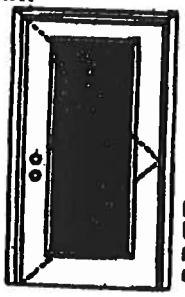
Internal gain		Occupants		Btuh/occupant		Appliance	Load
		6	X	300	+	3000	4800 Btuh

X
Glazed Inswing Unit

COP-WL EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water (unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-standard, state or local building codes specify the ratings required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:
1/4 GLASS:

1/2 GLASS:

*This glass kit may also be used in the following door styles: 8-panel; 8-panel with oval; Sydney 8-panel; Sydney 8-panel with oval.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

FIREDOOR Corporation System Quality Doors

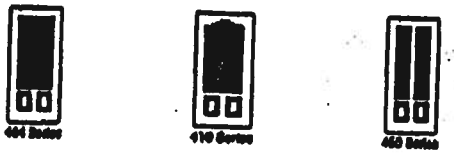
Exclusively from Masonite
Masonite International Corporation

X
Glazed Inswing Unit

COP WL FD4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
3/4 GLASS:



FULL GLASS:

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 28-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip fits surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Entergy
Entry Systems

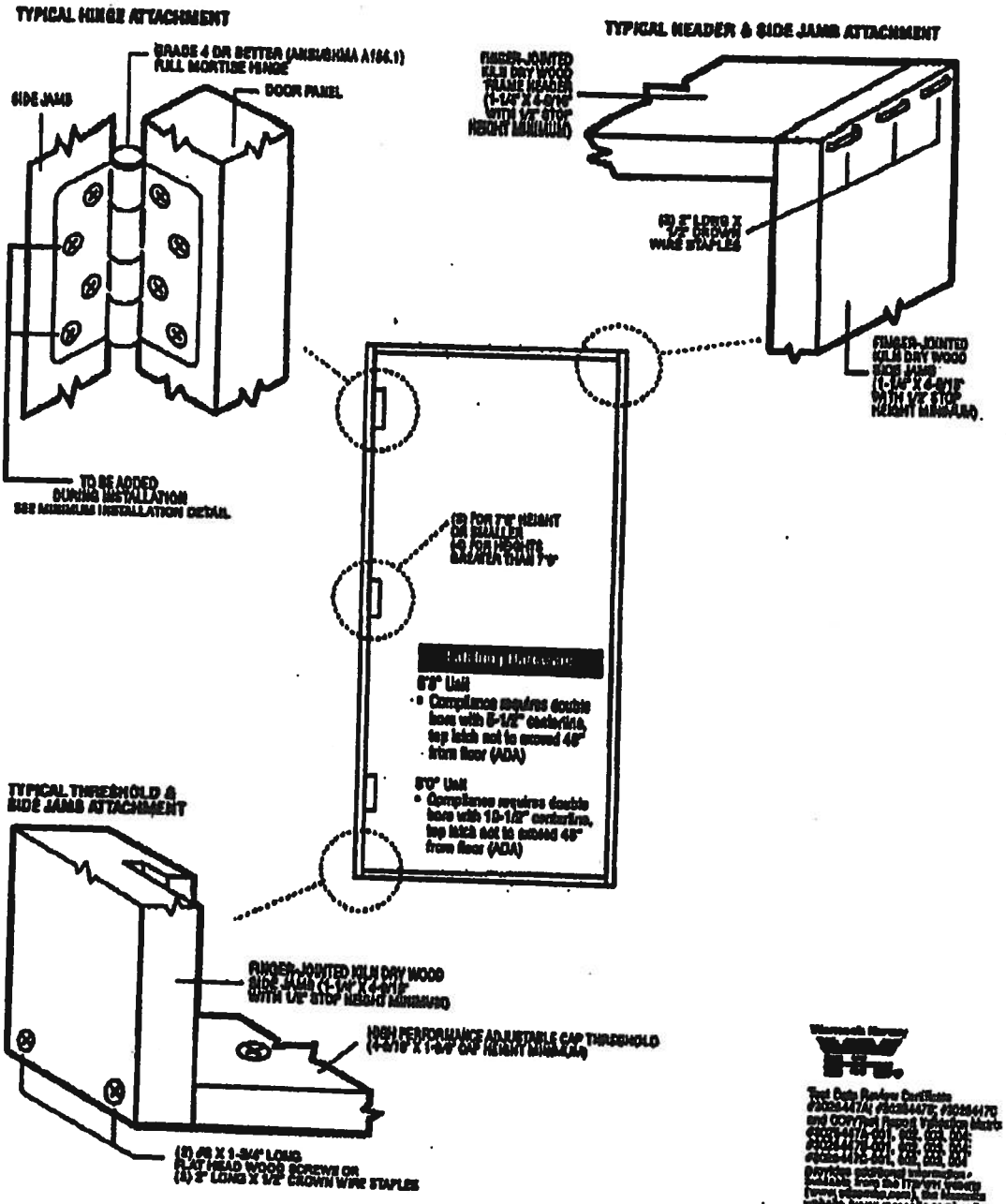
June 17, 2002

Masonite International Corporation

X
Unit

MAD-WI-MA0001-02

INSWING UNIT WITH SINGLE DOOR



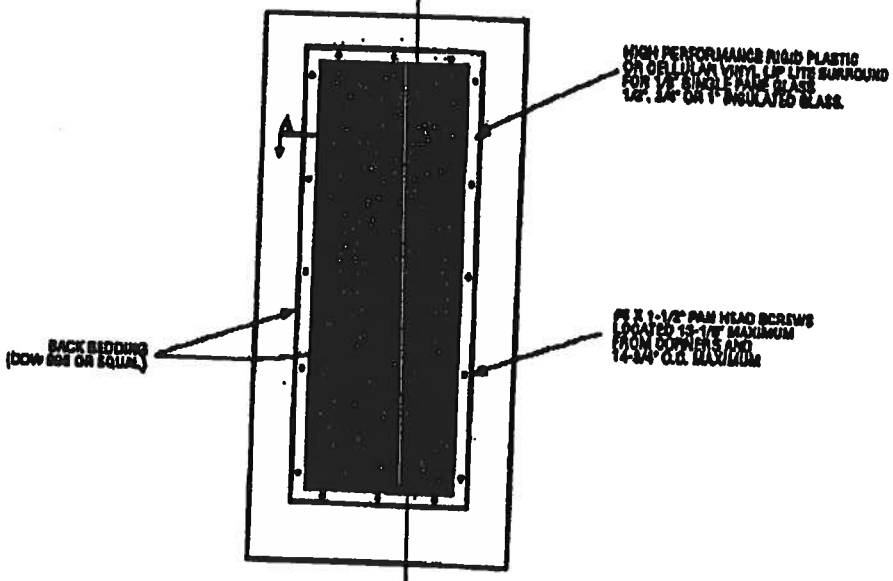
Test Data Review Certificate [illegible] and CCMTest Report Validation Matrix [illegible]

For additional information, contact your Masonite representative, visit the Masonite website (www.masonite.com) or the Masonite technical group.

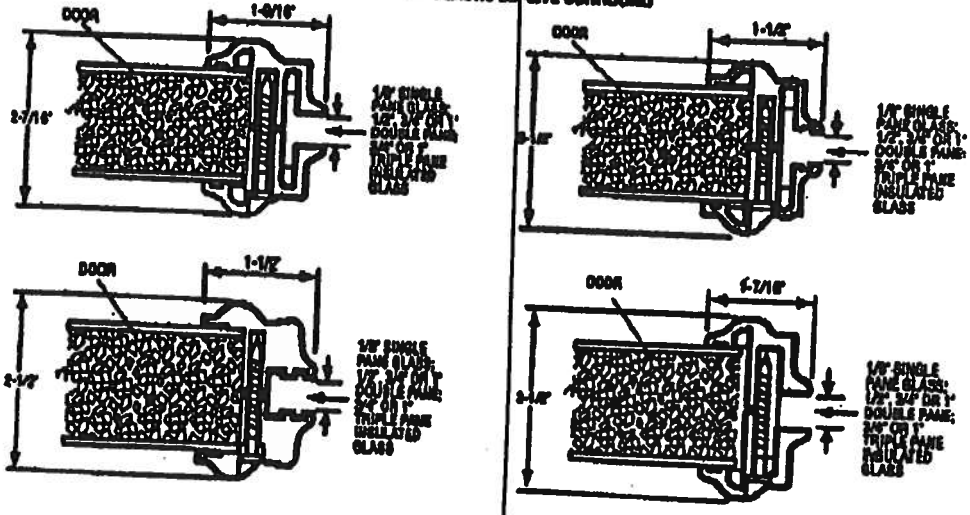
Masonite

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

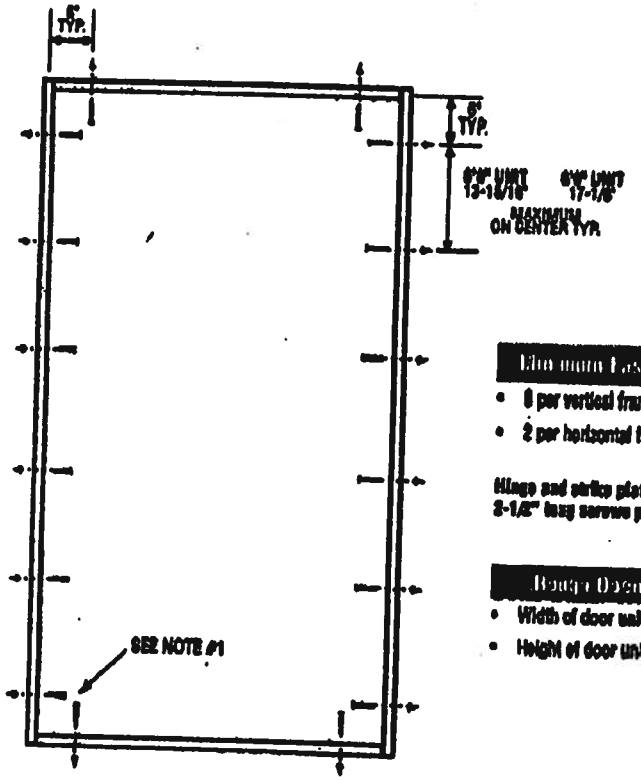
Test Data, Review Certificate #3029447A; 3029447B; 3029447C and Control Panel Inspection Report #3029447A-001, 002, 003; #3029447B-001, 002, 003; #3029447C-001, 002, 003 provide additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical group.

Exclusively from Masonite
Masonite International Corporation

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 8 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 3-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #3033647A; #3033647B; #3033647C and CCR/Test Report Validation Matrix #3033647A-001, 002, 003, 004; #3033647B-001, 002, 003, 004; #3033647C-001, 002, 003, 004 provides additional information - available from the ITW/ROH website (www.itwrohs.com) or the Masonite internal system. (www.masonite.com), the Masonite website

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8248*, 8256*, 8241*, 8248, 8261* or 8256 Compliance requires that 6" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

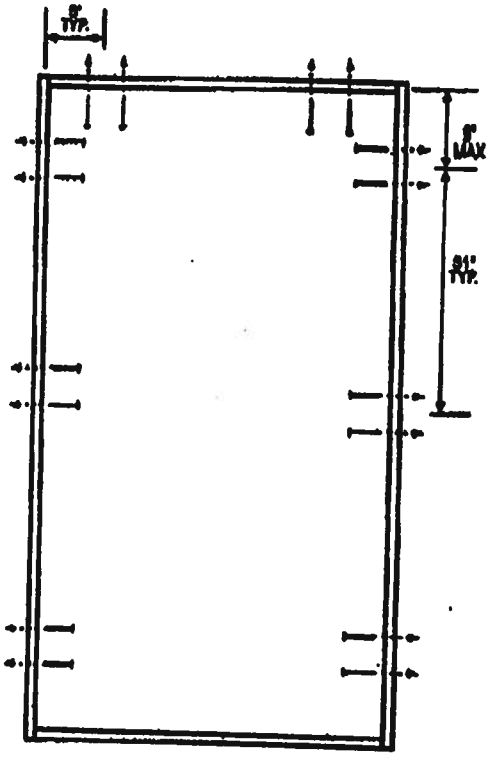
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 400 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.2A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 8 per vertical framing member for 70" height and smaller
- 8 per vertical framing member for heights greater than 70"
- 4 per horizontal framing member

Hinge and strike plates require two 3-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3039447X; #3039447B, #3039470 and COP Test Report Validation Memo #3039447-001, 002, 003, 004; #3039447B-001, 002, 003, 004; #3039470-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etsnorth.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 3240*, 3280*, 3241*, 3248, 3281* or 3286**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener ratings from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

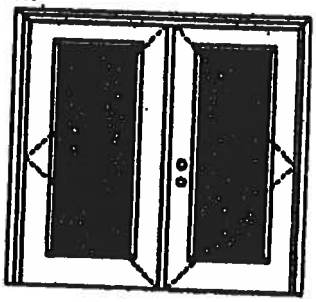
Masonite

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

THE Data Review Contains #WH4470 and SOPTest Report Validation Matrix #WH4470-SOP provides additional information - available from the ITS/WH website (www.warnockhersey.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size - 6'0" x 8'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed -- see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:

1/2 GLASS:

*This glass lite may also be used in the following door styles: 6-panel, 6-panel with scroll, 6-panel Sydney, 6-panel Sydney with scroll.

Entergy
Entry Systems

June 17, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

FIBERDOR Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

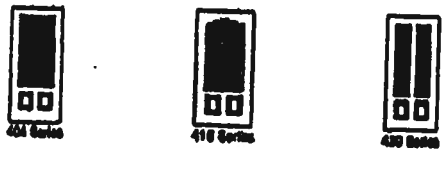
XX
Glazed Outswing Unit

COP-WI-FM4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



FULL GLASS:

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom and rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Baltz

State of Florida, Professional Engineer
Kurt Baltzazor, P.E. – License Number 56533

Entergy
Entry Systems

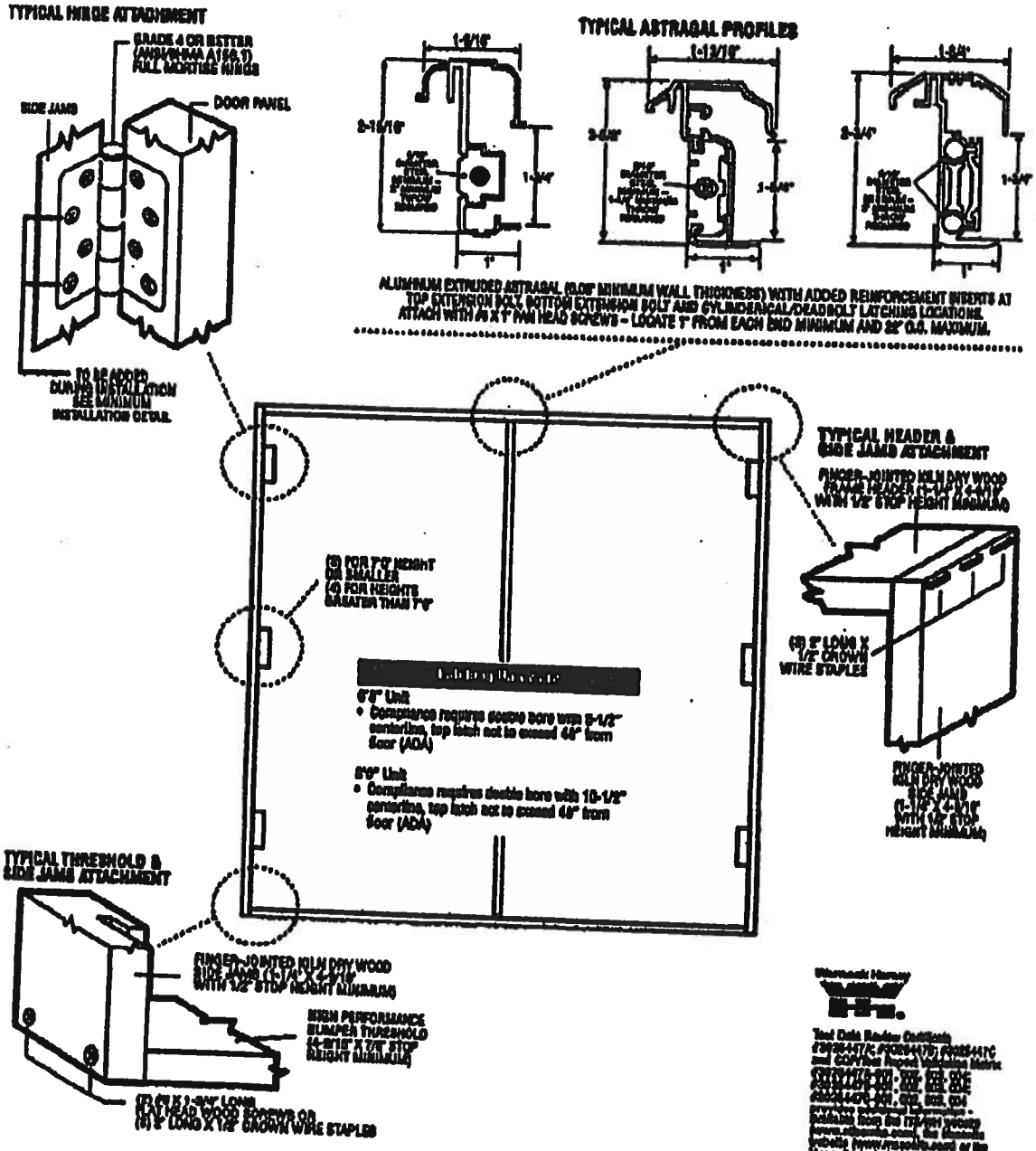
June 17, 2006

Masonite International Corporation

XX
Unit

MAD WL-MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR



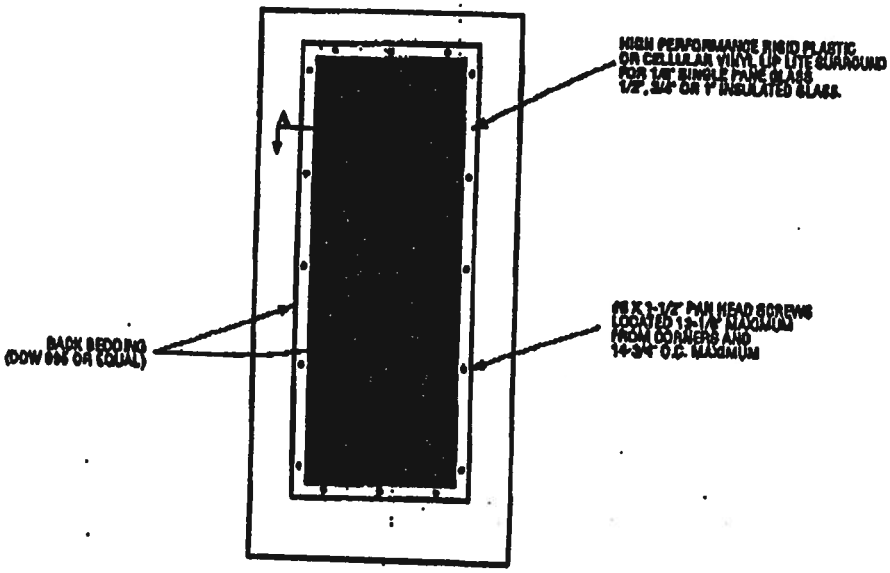
Warnock Hersey

Test Data Review Certificate

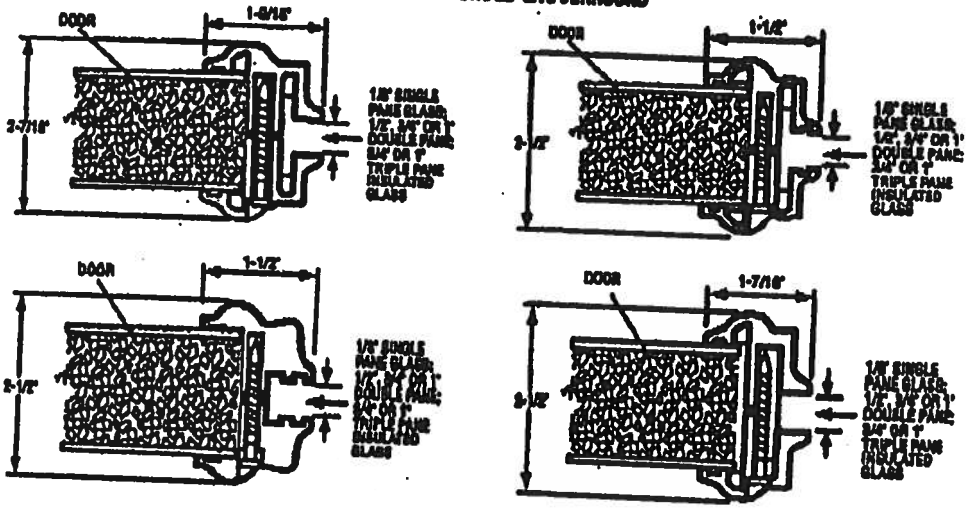
Masonite

MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Masonite Warranty

Test Data Review Certificate #30284478; #30284479; #30284480 and CORTest Report Validation Letter #3028447A-001, CCD 0707, #3028447B-001, CCL 0707, #3028447C-001, CCL 0707 provide additional information - available from the ITS/ETL website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

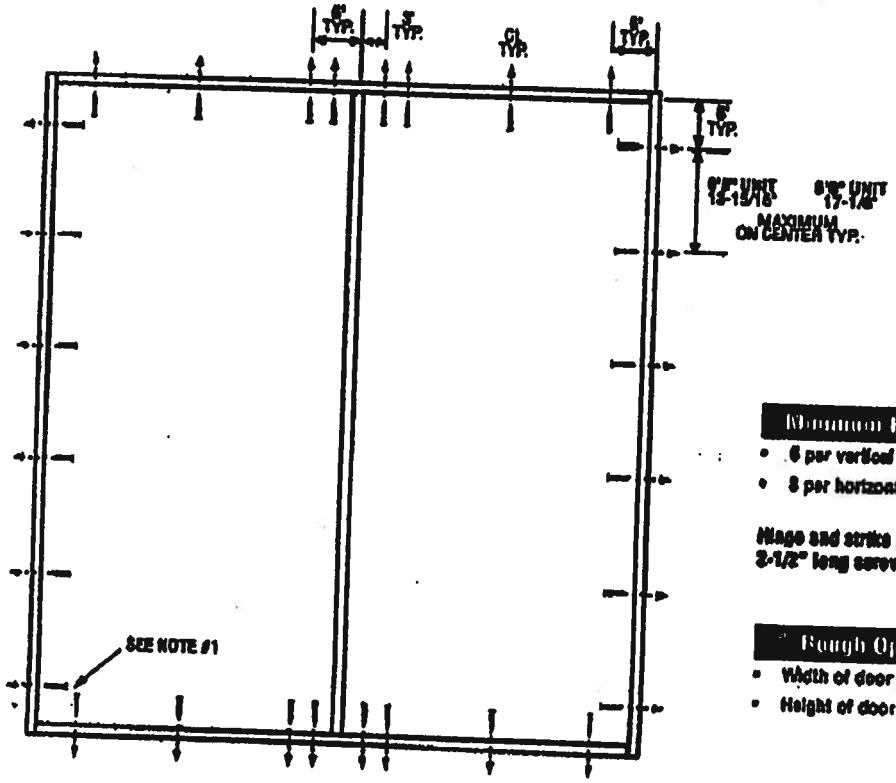
FRONDORA Masonite Premium Quality Doors

Exclusively from Masonite® Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Common Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4" .

The Data Review Database #30284478; #30284479; #30284480 and #30[illegible] Smart Validation tests #30284474-001, 002, 003, and #30284475-001, 002, 003, 004 #30284476-001, 002, 003, 004 provide additional information - available from the ITW/WI website (www.itwwi.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8247*, 8287*, 8242*, 8247, 8282* or 8287
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

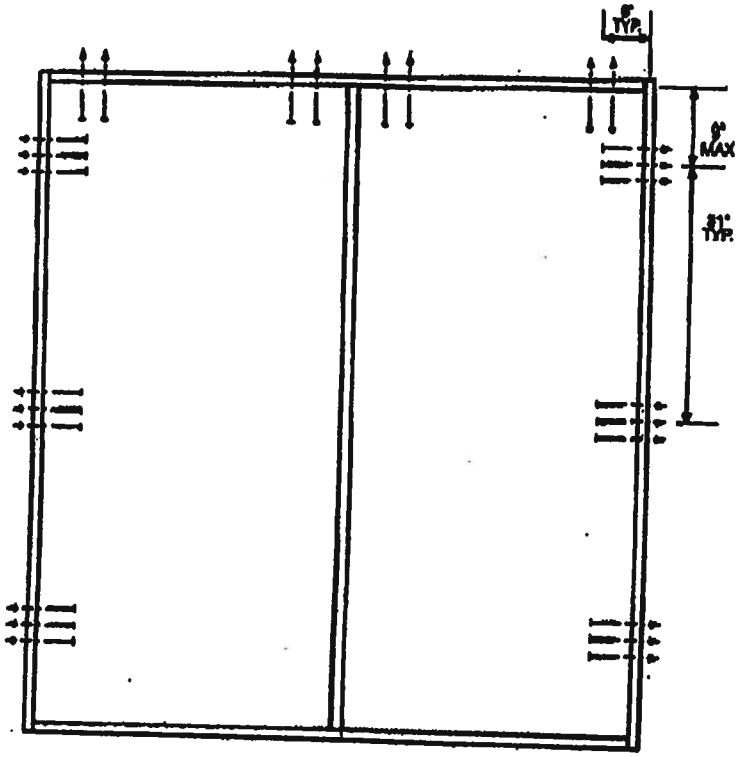
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

XX
Unit

MID WL MA0002 02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #30254473; #30254478; #30254470 and CCRTest Report Validation Match #30254474-001, 002, 003, 004; #30254479-001, 002, 003, 004; #30254480-001, 002, 003, 004 provides additional information - available from the ITS/Intertek website (www.intertek.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 0247*, 0257*, 8242*, 8247, 8282* or 8287
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:baw

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
11[illegible]
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

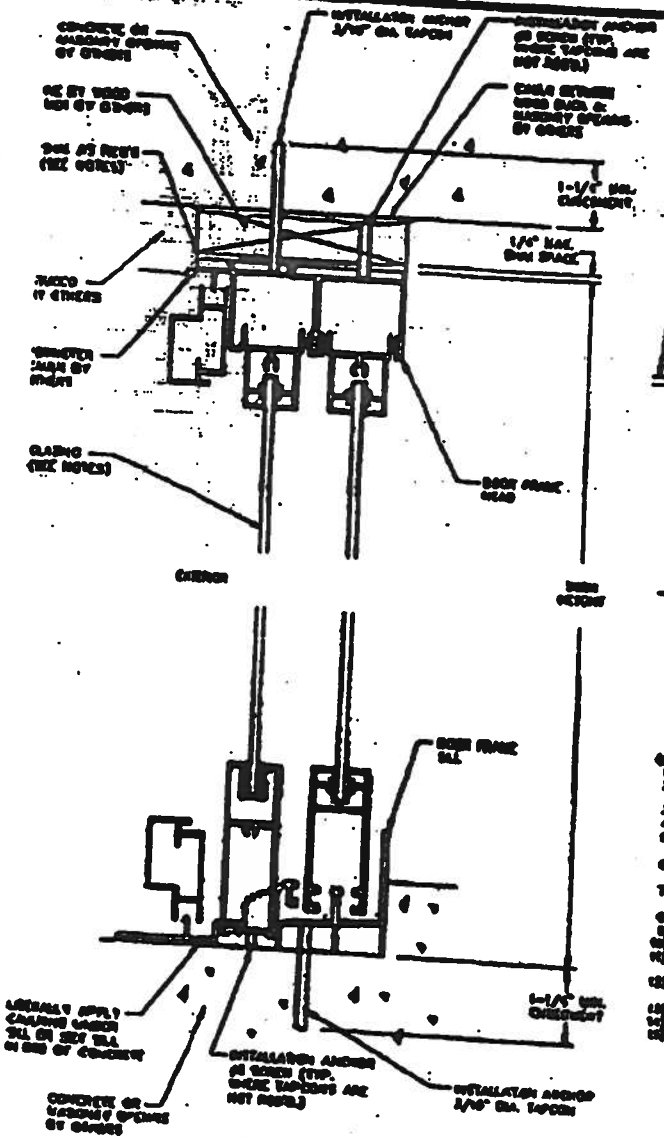
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- 2'- 6" WIDE DP + 40.0 / - 55.4
- 3'- 0" WIDE DP + 40.0 / - 48.5
- 4'- 0" WIDE DP + 40.0 / - 40.3

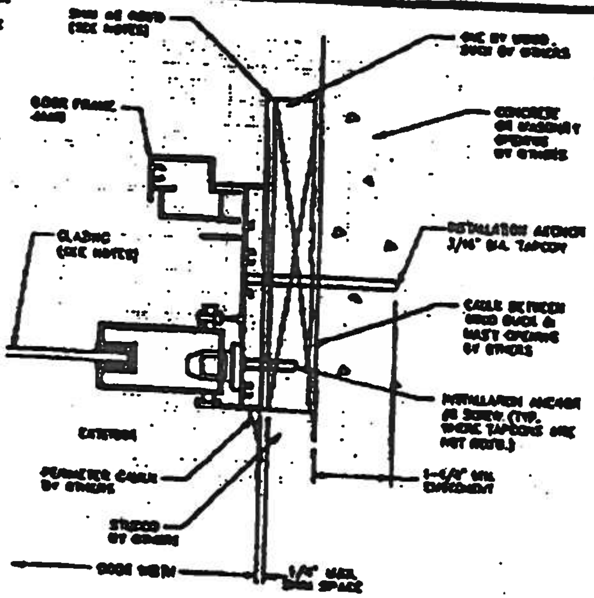
THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686



SECTION A-A



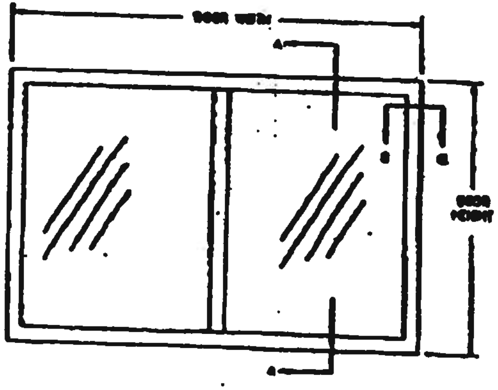
SECTION B-B

NOTES:

[illegible]

TAPCON INSTALLATION CHART

CALL SIZE	DOOR SIZE	[illegible]	[illegible]	[illegible]	[illegible]
[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]



EXTERIOR ELEVATION

MI HOME PRODUCTS
GRATZ. PA.

SERIES 470 SLIDING GLASS DOOR
INSTALLATION WITH TAPCONS

MANUAL MARTINEZ | BB | 1/29/02
CIVIL | N.T.S. | MHP0015
47182 | | 1 OF 1.

NO. 47182
STATE OF FLORIDA

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

ALLEN N. REEVES
CERTIFICATE
NO. 13354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
STATE OF FLORIDA
PROFESSIONAL ENGINEERS

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES CERTIFICATE NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19356
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

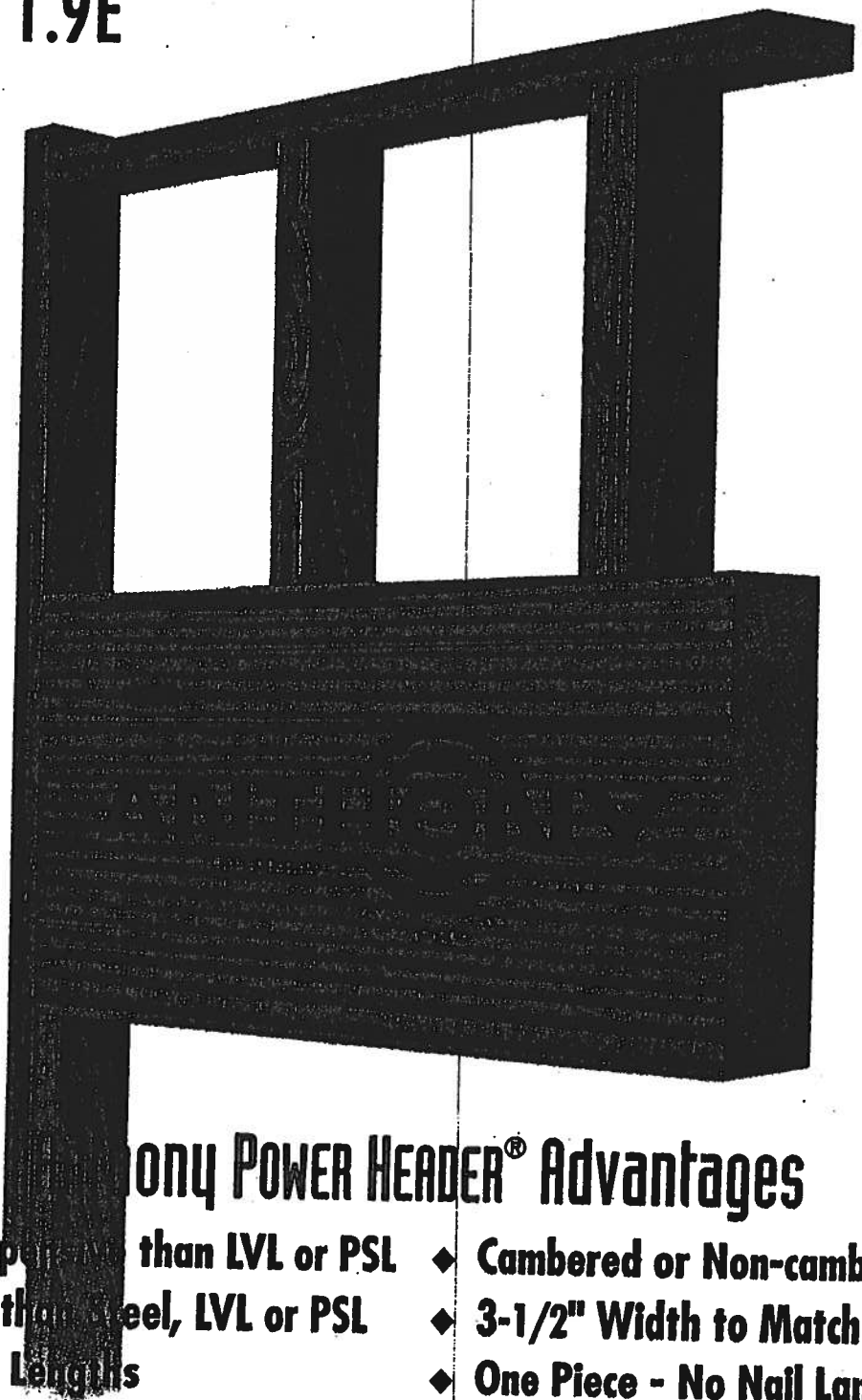
ALLEN M. REEVES ★ CERTIFICATE NO. 19354 ★ STATE OF FLORIDA ★ PROFESSIONAL ENGINEER ★

Allen M. Reeves

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- Less Expensive than LVL or PSL
- Lighter than Steel, LVL or PSL
- Pre-Cut Lengths
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

Anthony Power Header®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	9-3/4	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8		9-3/4		
8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
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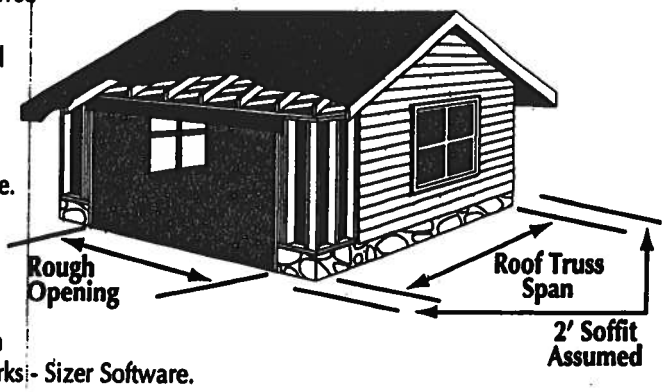
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP $30F_b$ POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony Power Header®

26F$_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

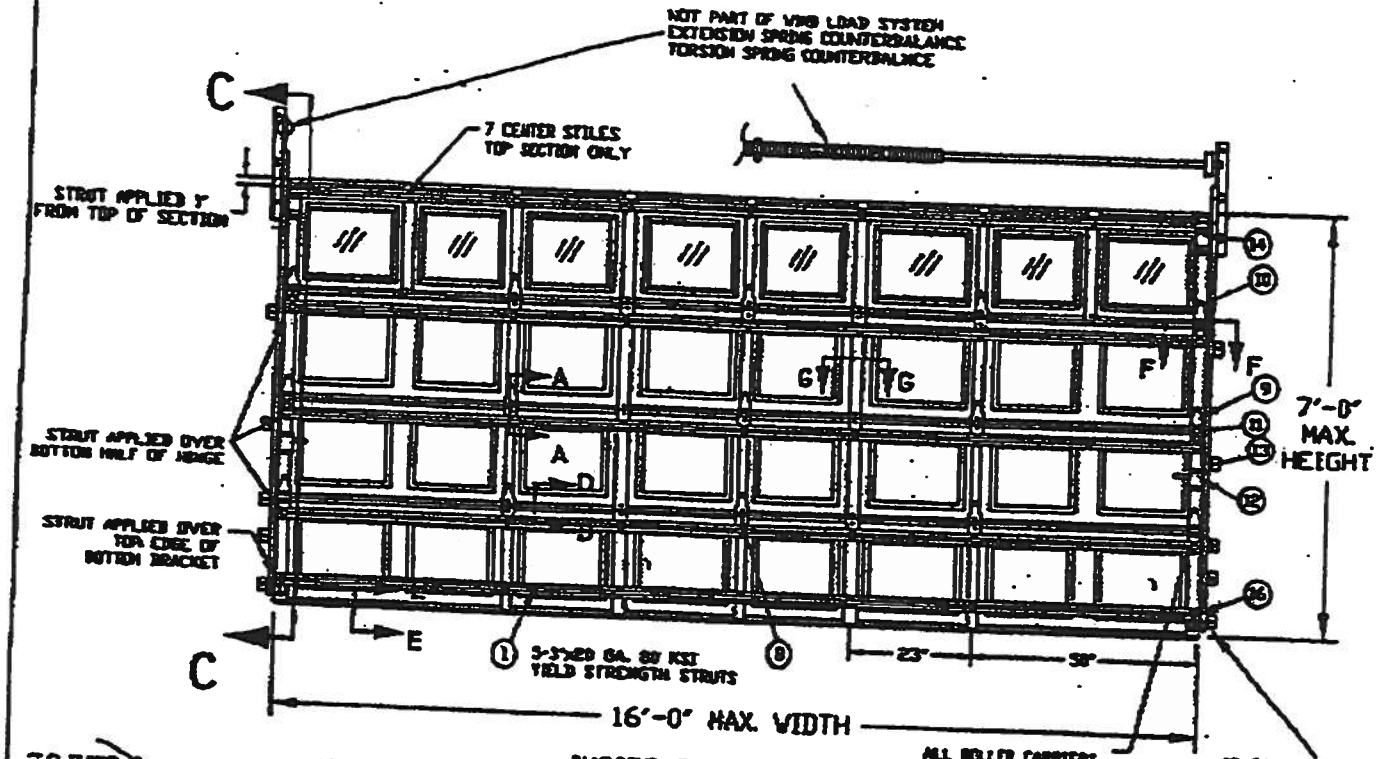
For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

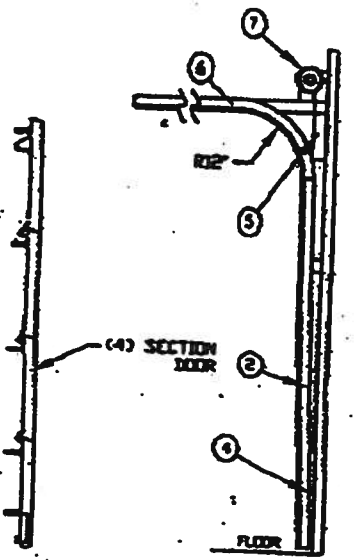
NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION, GAS TESTED WITH 3/16" ROD GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5" OR AS TESTED
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 4,3, OR 2 AS TESTED
9. DROP IN TYPE OF INSULATION IS OPTIONAL.

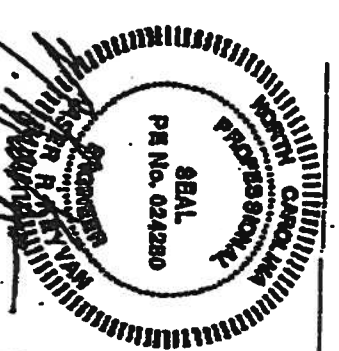
REV.	DATE	BY	DESCRIPTION
A-A	01-10-00	DW	SEE E.C.N. 631



INSIDE ELEVATION



SEC C-C
VERTICAL TRACK, (16 GA.)



The seal on this drawing only certifies that the described product(s) illustrated and the dimensions and configuration(s) of the door as tested.

LISTED SPDC PRIMED
REPORT No. 2202

TEST REPORTS ON FILE: VIDEO 10/19/00 4008930

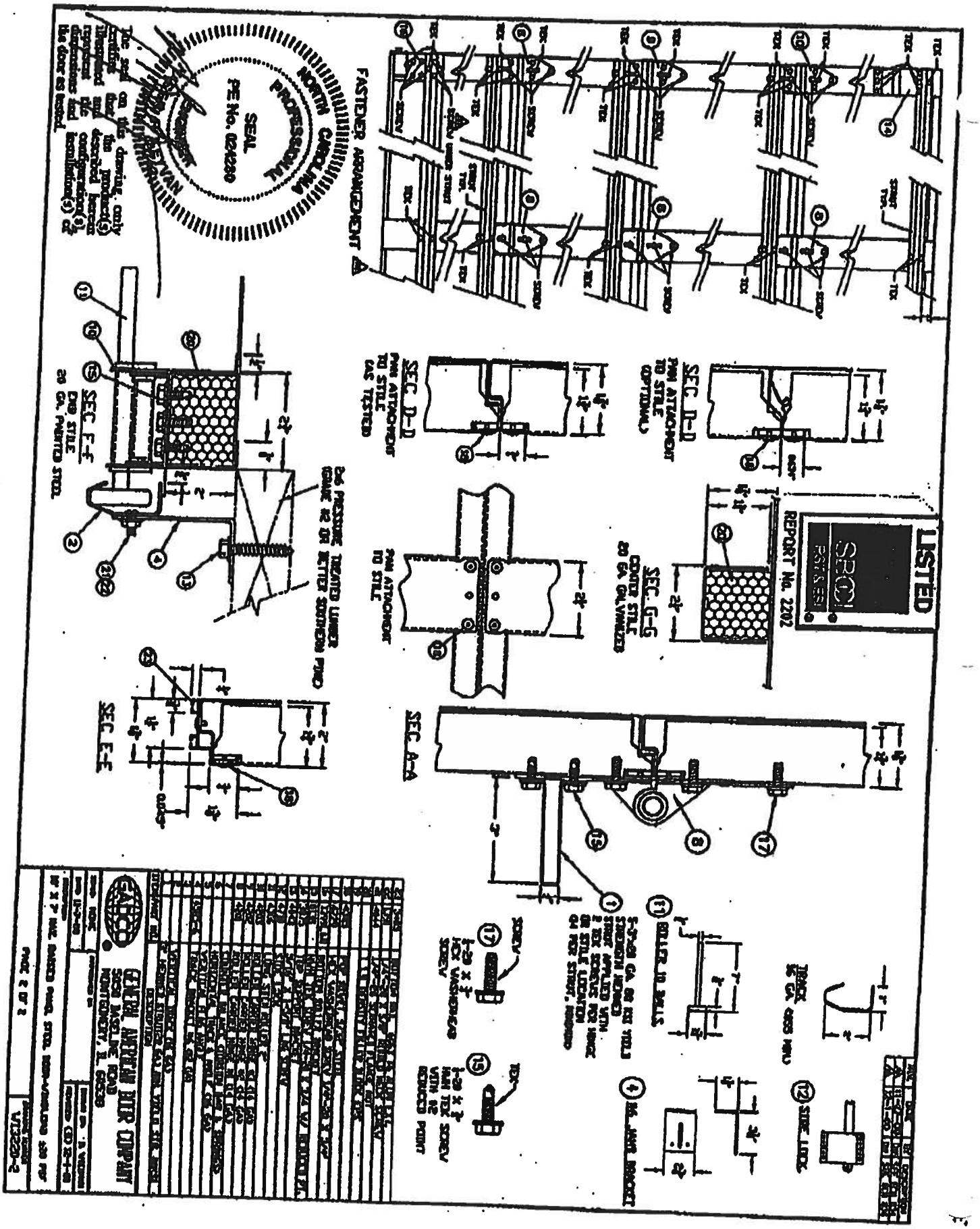
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

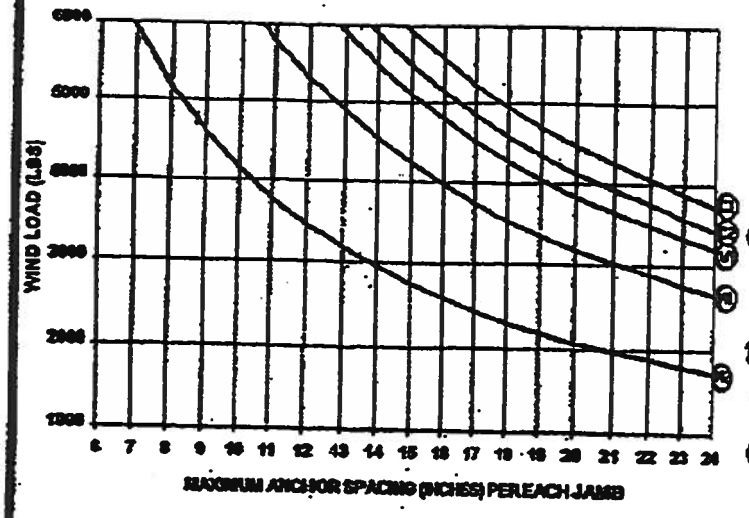
GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN GA TESTED
SERIES 7823, EXTERIOR STEEL =.019" MIN ⚠
SERIES 7324, EXTERIOR STEEL =.024" MIN ⚠
(TESTED WITH WINDOWS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

GADCO
GENERAL AMERICAN DOOR COMPANY
5058 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NTS	APPROVED BY:
DWG: 10-20-00	DRAWN BY: J. VEDDERS
DESCRIPTION: 16' x 7' MAX RAISED PANEL STEEL DOOR -/WIND LOAD +20 PSF	REVISED: (A) 11-30-00
PART NUMBER: PAGE 1 OF 2	DRAWING NUMBER: V13220-1


FASTENER ARRANGEMENT
SEC D-D
SEC H-H
SEC F-F
SEC G-G
SEC A-A
SEC E-E
LISTED
REPORT No. 2202
CENTRAL AIRFLOW DOOR COMPANY



(3) CONCRETE 3400P HILTI KWIK BOLT II EXPANSION ANCHOR 3/8" DIA. 1-5/8" EMBEDMENT

(4) CONCRETE 3400P RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-3/8" EMBEDMENT

(5) MASONRY/CMU 3400P RAWL LOK/BOLT SLEEVE ANCHOR 3/8" DIA. 1-3/8" EMBEDMENT

(6) MASONRY/CMU 3400P ITW BUILDEX TAPCON MASONRY ANCHOR 1/4" DIA. 1-3/4" EMBEDMENT

(7) WOOD STUD 3400P LAG SCREWS 5/16" DIA. 1-1/2" EMBEDMENT

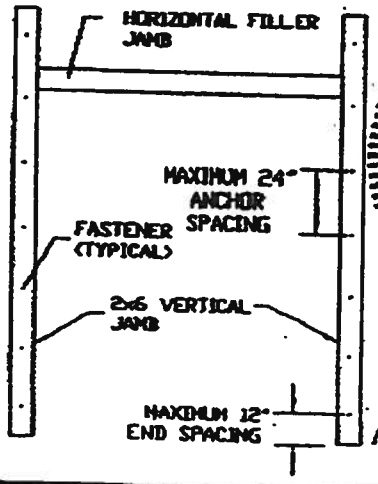
DESIGN LOAD (LBS)/FT² X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

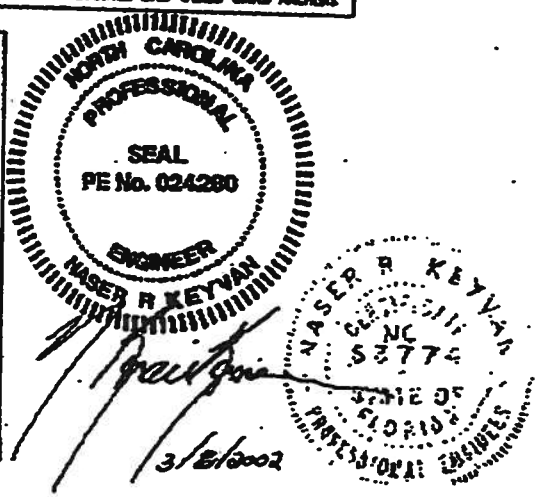
EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

(2) USE 22" SPACING
(3) USE 21" SPACING
(4) USE 19" SPACING
(5) USE 16" SPACING
(6) USE 10" SPACING

SEE NOTE 10 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS





2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SDDCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2850 PSI. GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI, REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

ISSUED NONE	APPROVED BY	DRAWN BY BV
DATE 8-30-99		CHECKED
REVISIONS	JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS	
	DRAWING NUMBER A10560	DRAWING NUMBER

ELK

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39¾"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size	13¼"x 39¾"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅓"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

***See actual limited warranty for conditions and limitations.**
****Check for product availability.**

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. [illegible]

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

Project Information for:	L135896		
Builder:	HUGO ESCALANTE	Date:	11/15/2005
Lot:	LOT 5 CANNON CREEK	Start Number:	1595
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY	Refer to Master:	
Truss Page Count:	40		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: 29

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1115051595	11/15/2005				
2	CJ3	1115051596	11/15/2005				
3	CJ5	1115051597	11/15/2005				
4	EJ7	1115051598	11/15/2005				
5	EJ7A	1115051599	11/15/2005				
6	EJ7B	1115051600	11/15/2005				
7	EJ7G	1115051601	11/15/2005				
8	EJ7GA	1115051602	11/15/2005				
9	HJ9	1115051603	11/15/2005				
10	T01	1115051604	11/15/2005				
11	T01G	1115051605	11/15/2005				
12	T02	1115051606	11/15/2005				
13	T03	1115051607	11/15/2005				
14	T04	1115051608	11/15/2005				
15	T05	1115051609	11/15/2005				
16	T06	1115051610	11/15/2005				
17	T07	1115051611	11/15/2005				
18	T08	1115051612	11/15/2005				
19	T09	1115051613	11/15/2005				
20	T10	1115051614	11/15/2005				
21	T11	1115051615	11/15/2005				
22	T12	1115051616	11/15/2005				
23	T13	1115051617	11/15/2005				
24	T14	1115051618	11/15/2005				
25	T15	1115051619	11/15/2005				
26	T16	1115051620	11/15/2005				
27	T17	1115051621	11/15/2005				
28	T18	1115051622	11/15/2005				
29	T19	1115051623	11/15/2005				
30	T20	1115051624	11/15/2005				
31	T21	1115051625	11/15/2005				
32	T22	1115051626	11/15/2005				
33	T23	1115051627	11/15/2005				
34	T24	1115051628	11/15/2005				
35	T25	1115051629	11/15/2005				
36	T26	1115051630	11/15/2005				
37	T27	1115051631	11/15/2005				
38	T28	1115051632	11/15/2005				
39	T29	1115051633	11/15/2005				
40	T29G	1115051634	11/15/2005				

NOV 15 2005

Dwg.#1115051594

02:00:39 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**ESCALANTE, HUGO (Primary Name)**
	EWPL INC (DBA Name)
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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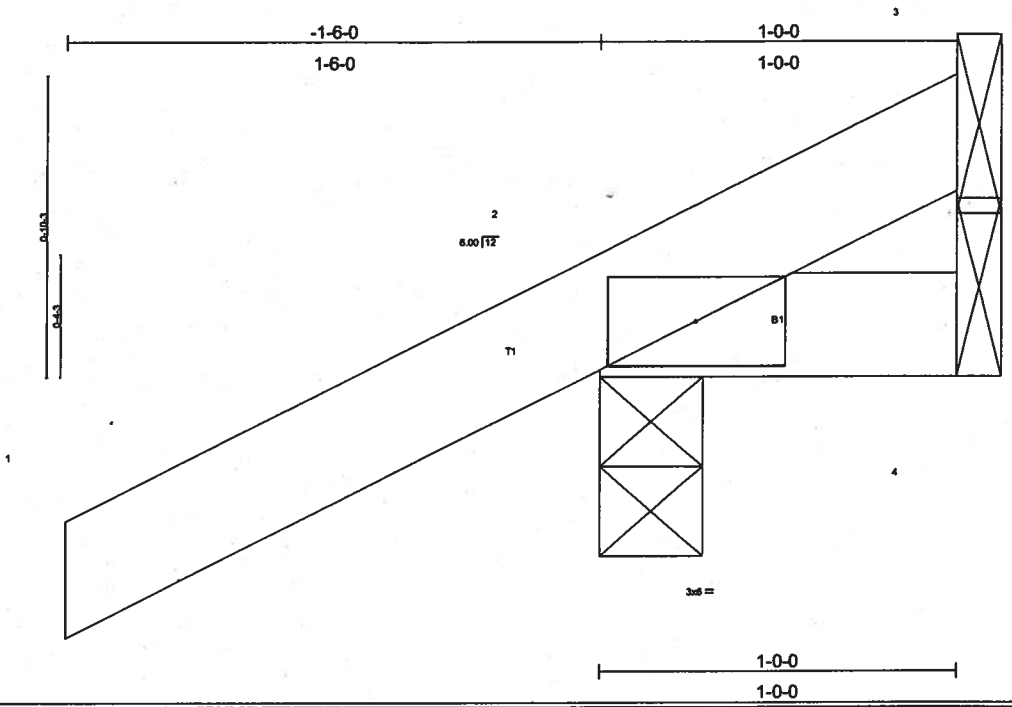
New Search Back

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, LLC EB 8196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	CJ1	MONO TRUSS	8	1	Dwg.#115051595

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:24 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-220(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2 and 40 lb uplift at joint 3.

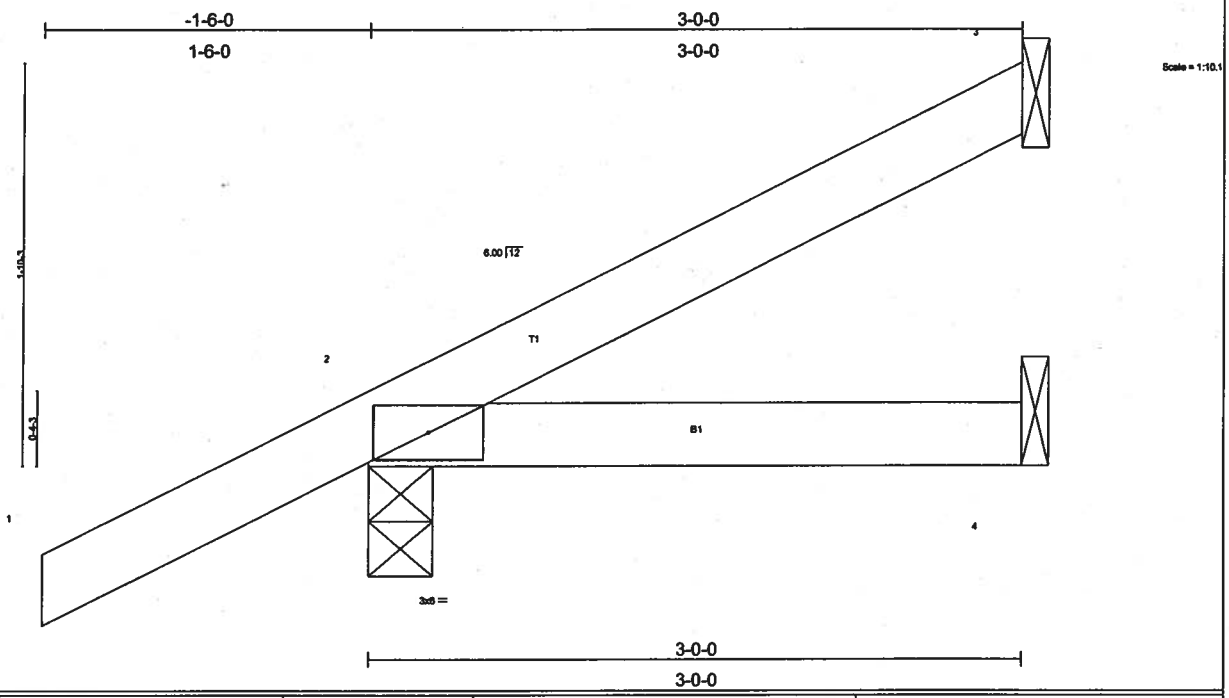
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	CJ3	MONO TRUSS	8	1	Job Reference (optional)

Dwg.#115051596

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:25 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-47(load case 5), 2=-187(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-52/16
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 187 lb uplift at joint 2.

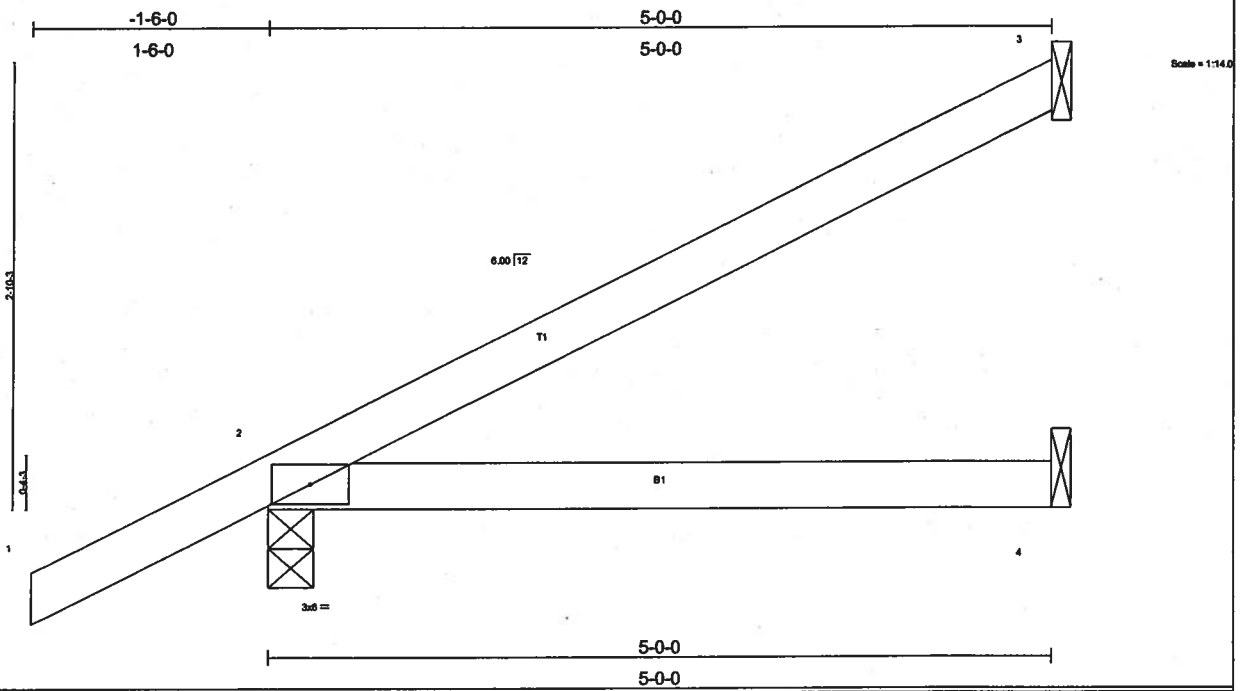
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	CJ5	MONO TRUSS	8	1	Job Reference (optional)

Dwg.#115051597

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:26 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-124(load case 5), 2=-197(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-112/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 197 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	EJ7	MONO TRUSS	29	1	Dwg.#115051598

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:26 2005 Page 1

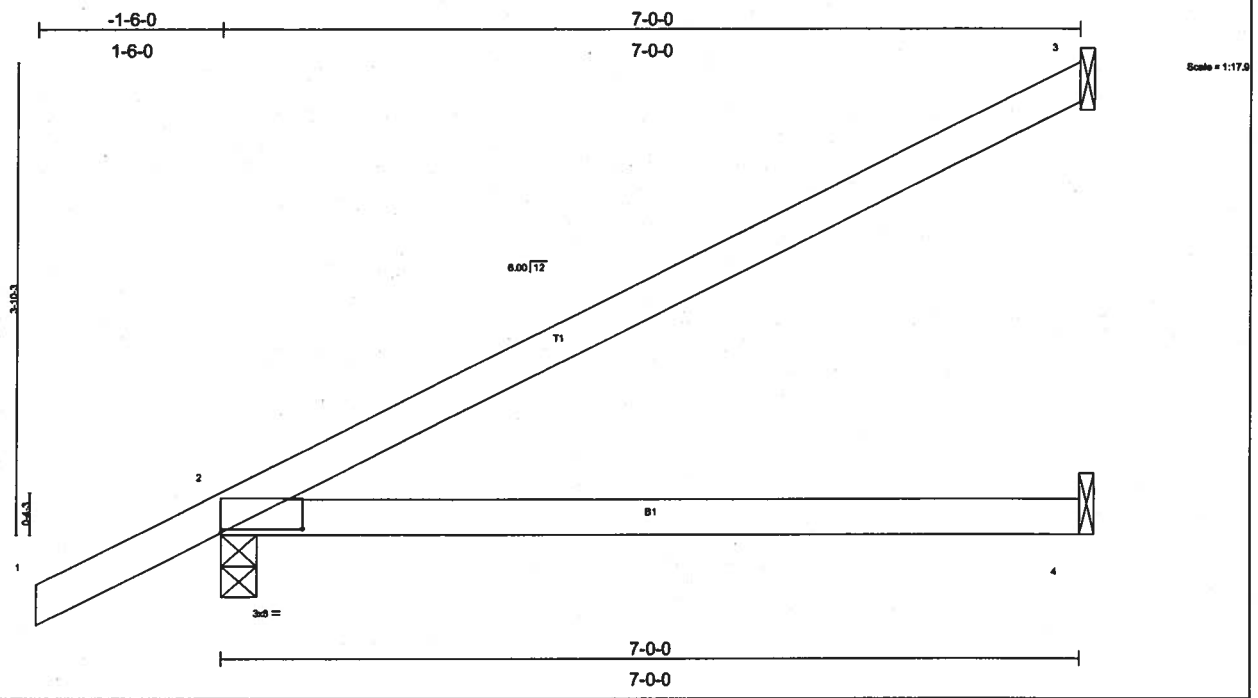


Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=247(load case 5)
Max Uplift3=-170(load case 5), 2=-217(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-142/59
BOT CHORD 2-4=0/0

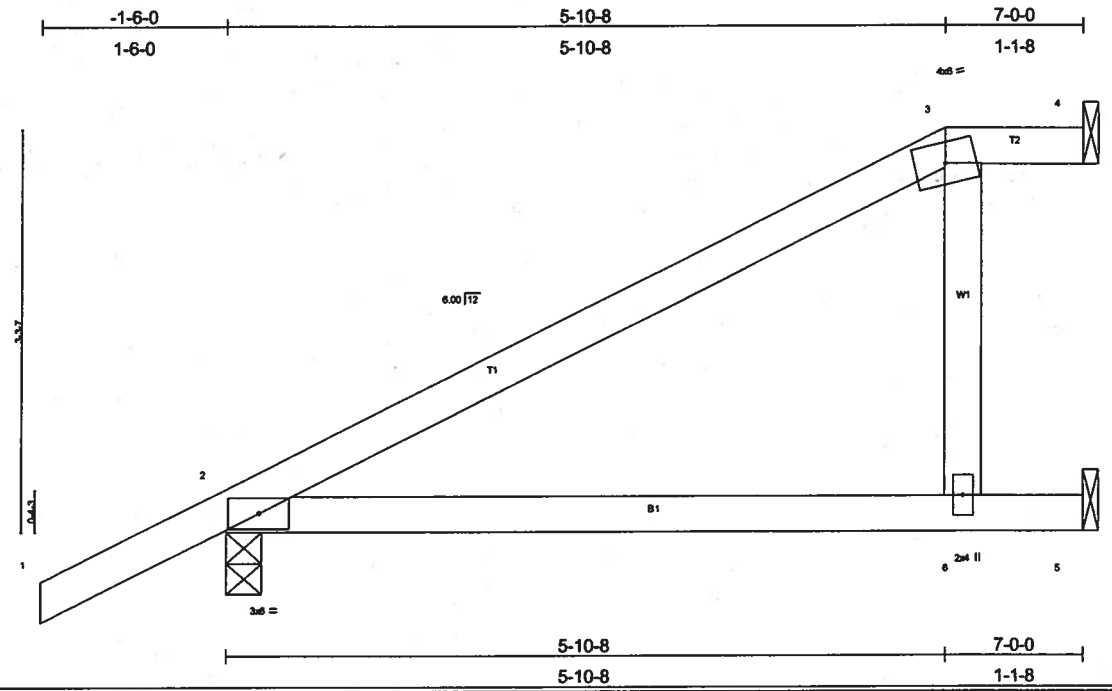
NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 3, 217 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051599
L139896	EJ7A	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:27 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.20	2-6	>404	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.25	2-6	>323	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.06	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=77/Mechanical, 2=385/0-3-8, 5=197/Mechanical
Max Horz 2=219(load case 5)
Max Uplift4=-13(load case 3), 2=-229(load case 5), 5=-160(load case 5)
Max Grav 4=94(load case 10), 2=385(load case 1), 5=197(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-113/42, 3-4=-0/1
BOT CHORD 2-6=-15/5, 5-6=0/0
WEBS 3-6=-112/302

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 4, 229 lb uplift at joint 2 and 160 lb uplift at joint 5.

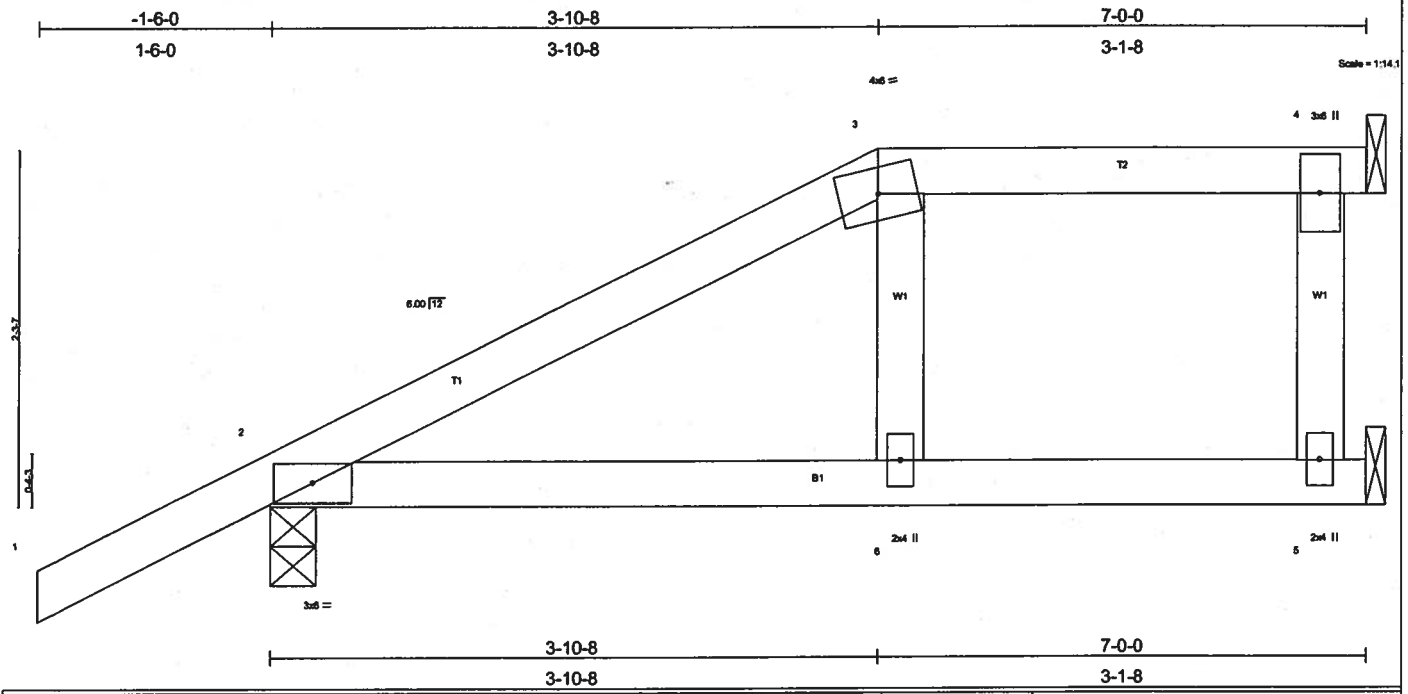
LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	EJ7B	MONO HIP	1	1	Dwg.#1115051600

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:28 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.19	2-6	>422	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.22	2-6	>355	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.11	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=376/0-3-8, 4=138/Mechanical, 5=127/Mechanical
Max Horz 2=164(load case 5)
Max Uplift2=-236(load case 5), 4=-83(load case 4), 5=-36(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-80/6, 3-4=-0/0
BOT CHORD 2-6=-12/4, 5-6=0/0
WEBS 3-6=-55/167, 4-5=0/0

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2, 83 lb uplift at joint 4 and 36 lb uplift at joint 5.
6) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#115051601
L139896	EJ7G	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:28 2005 Page 1

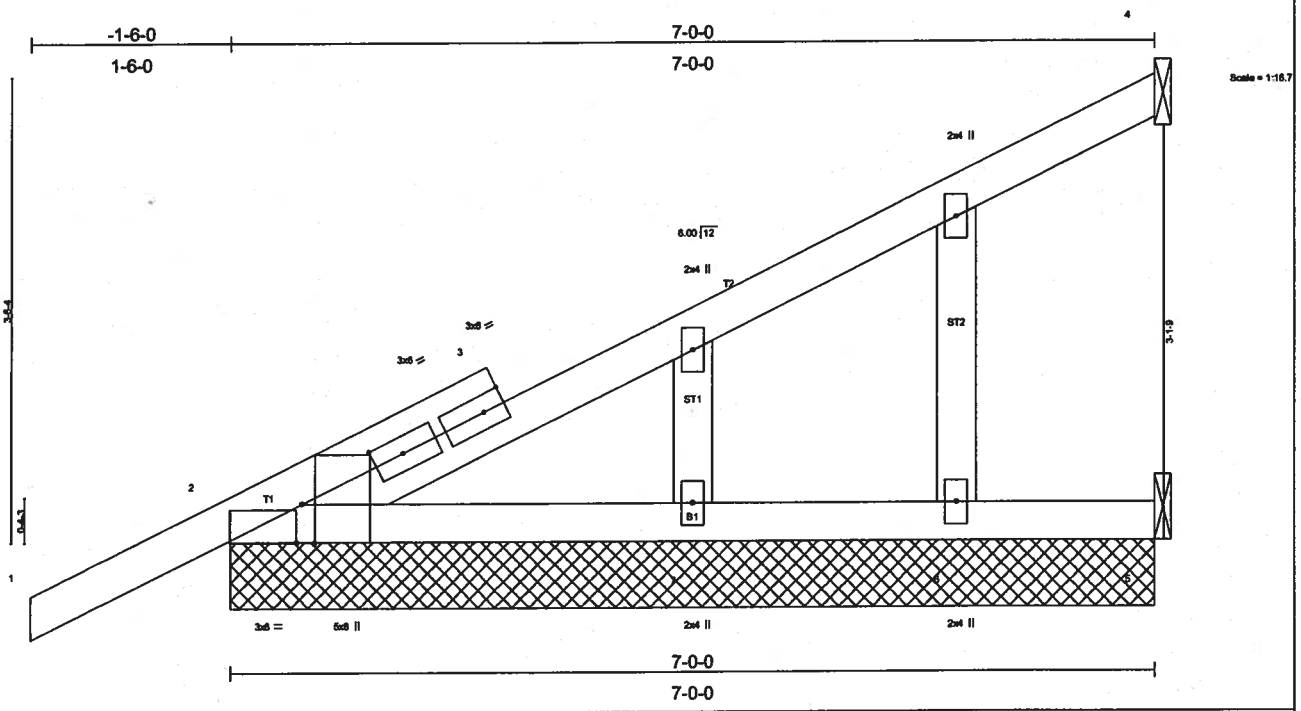


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	0.03	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/7-0-0, 4=146/Mechanical, 5=35/Mechanical, 7=243/7-0-0, 6=-27/7-0-0
Max Horz 2=229(load case 5)
Max Uplift2=-175(load case 5), 4=-154(load case 5), 5=-12(load case 5), 7=-116(load case 5), 6=-27(load case 1)
Max Grav 2=266(load case 1), 4=146(load case 1), 5=35(load case 1), 7=243(load case 1), 6=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-146/0, 3-4=-112/49
BOT CHORD 2-7=0/0, 6-7=0/0, 5-6=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable studs spaced at 2-0-0 oc.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 2, 154 lb uplift at joint 4, 12 lb uplift at joint 5, 116 lb uplift at joint 7 and 27 lb uplift at joint 6.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	EJ7GA	MONO HIP	1	1	Dwg.#115051602

Job Reference (optional)

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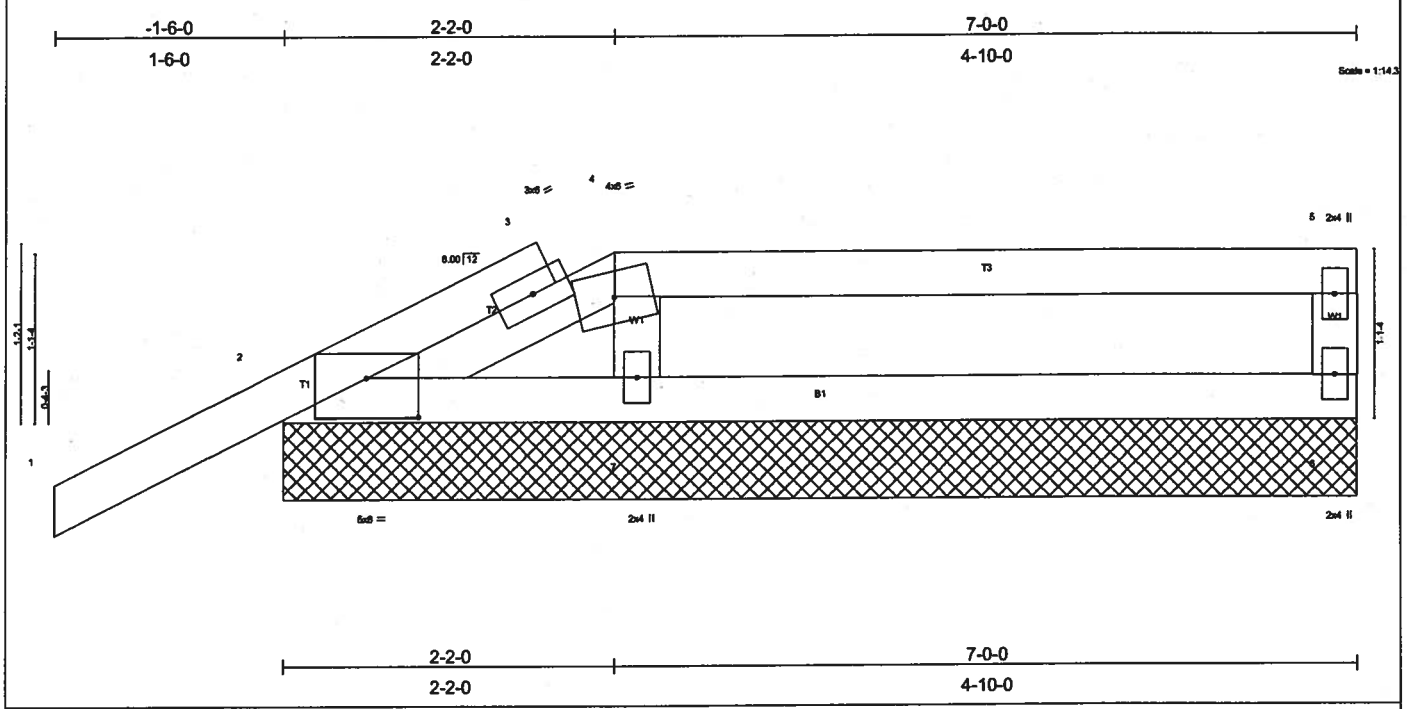

Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=198/7-0-0, 6=204/7-0-0, 7=464/7-0-0
Max Horz 2=99(load case 5)
Max Uplift2=-196(load case 5), 6=-100(load case 3), 7=-206(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/51, 2-3=-77/28, 3-4=-10/10, 4-5=-0/0
BOT CHORD 2-7=-70/60, 6-7=0/0
WEBS 4-7=-338/393, 5-6=-148/173

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 100 lb uplift at joint 6 and 206 lb uplift at joint 7.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-79(F=-25), 4-5=-79(F=-25), 2-6=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#115051603
L139896	HJ9	MONO TRUSS	4	1	Job Reference (optional)

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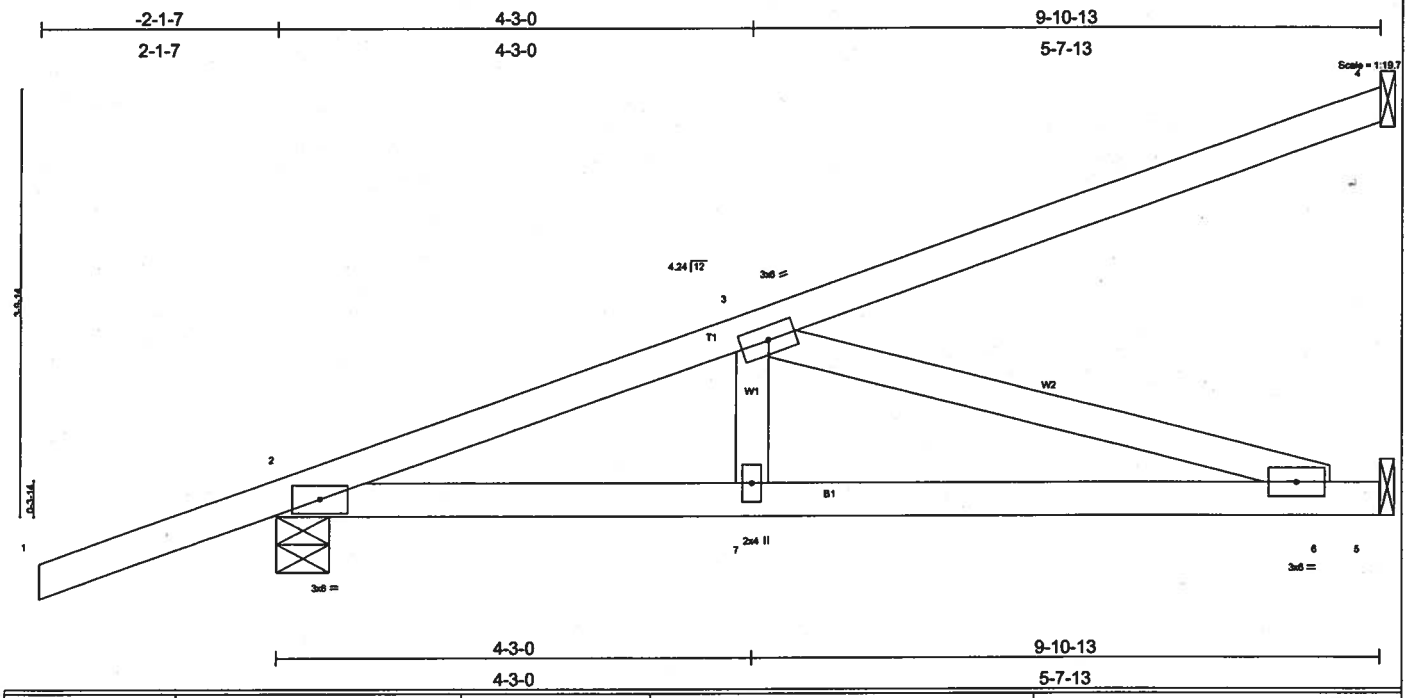

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.50	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=386/Mechanical
Max Horz 2=303(load case 2)
Max Uplift4=-289(load case 2), 2=-285(load case 2), 5=-107(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-934/246, 3-4=-124/65
BOT CHORD 2-7=-475/869, 6-7=-475/869, 5-6=0/0
WEBS 3-7=0/208, 3-6=-906/495

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 4, 285 lb uplift at joint 2 and 107 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T01	COMMON	2	1	Job Reference (optional)

Dwg.#1115051604

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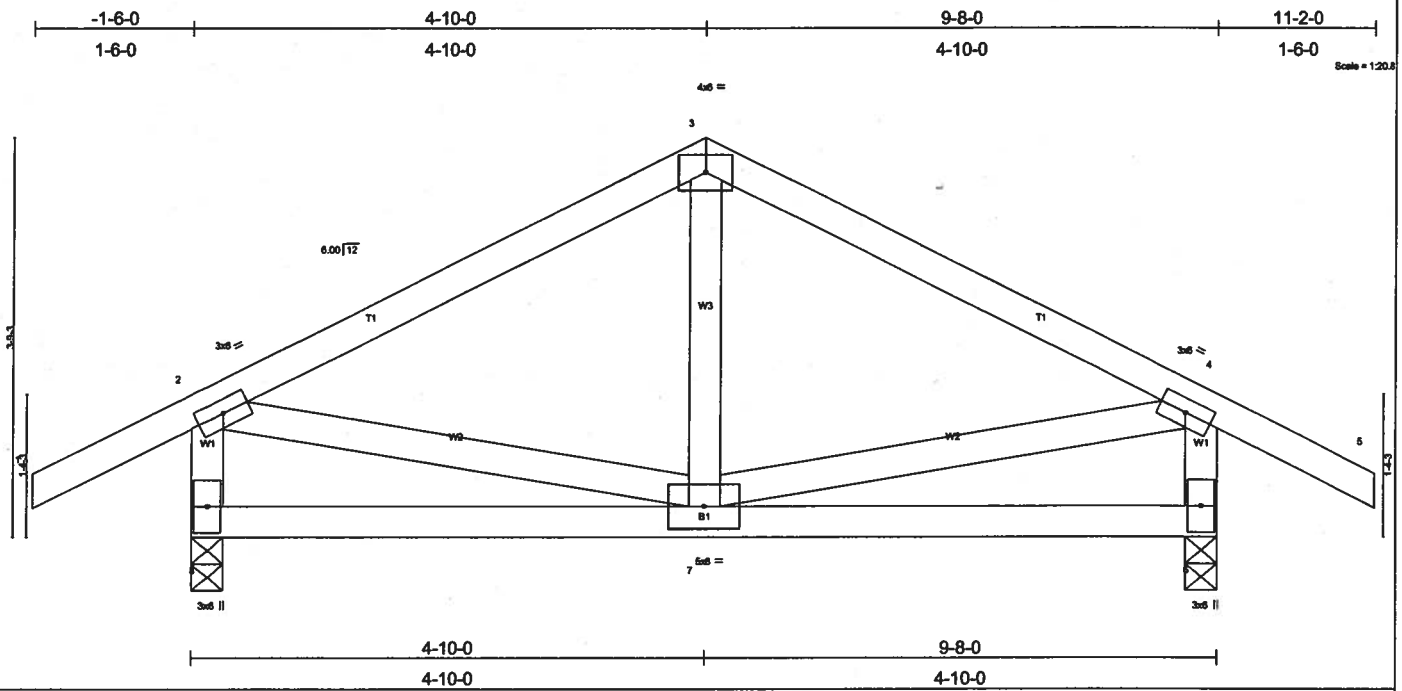

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.03	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	0.03	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=483/0-3-8, 6=483/0-3-8
Max Horz 8=87(load case 4)
Max Uplift8=-417(load case 5), 6=-417(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-380/578, 3-4=-380/578, 4-5=0/40, 2-8=-413/604, 4-6=-413/604
BOT CHORD 7-8=-134/83, 6-7=-71/83
WEBS 3-7=-223/67, 2-7=-219/222, 4-7=-219/222

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 417 lb uplift at joint 8 and 417 lb uplift at joint 6.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051605
L139896	T01G	COMMON	1	1	Job Reference (optional)

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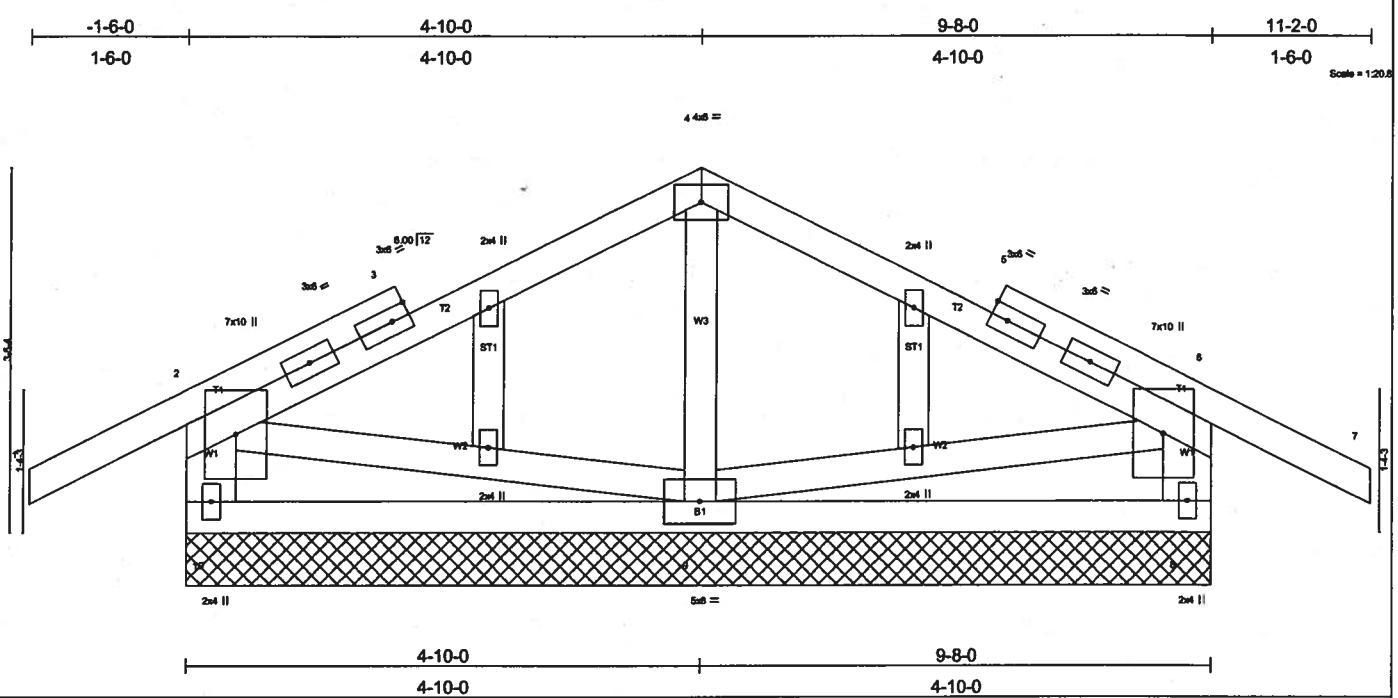

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.11	Vert(TL)	-0.02	7	n/r	90		
BCLL 10.0	Rep Stress Incr NO	WB 0.07	Horz(TL)	0.00	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 6 SYP No.1D, W1 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=401/9-8-0, 8=401/9-8-0, 9=474/9-8-0
Max Horz 10=79(load case 4)
Max Uplift10=-266(load case 5), 8=-269(load case 6), 9=-186(load case 5)
Max Grav 10=405(load case 9), 8=405(load case 10), 9=474(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/51, 2-3=-118/80, 3-4=-41/76, 4-5=-41/76, 5-6=-118/80, 6-7=-6/51, 2-10=-338/386, 6-8=-338/386
BOT CHORD 9-10=-109/151, 8-9=-36/151
WEBS 4-9=-292/263, 2-9=-120/163, 6-9=-120/163

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 10, 269 lb uplift at joint 8 and 186 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-79(F=-25), 2-4=-79(F=-25), 4-6=-79(F=-25), 6-7=-79(F=-25), 8-10=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T02	HIP	1	1	Dwg.#I15051606

Job Reference (optional)

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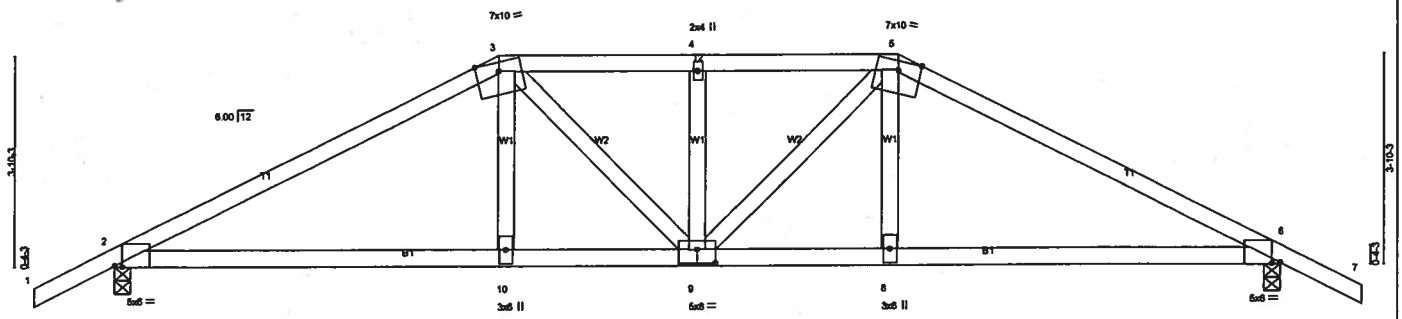
-1-6-0	3-9-4	7-0-0	10-8-0	14-4-0	17-6-12	21-4-0	22-10-0
1-6-0	3-9-4	3-2-12	3-8-0	3-8-0	3-2-12	3-9-4	1-6-0

Scale = 1:40.4



3-9-4	7-0-0	10-8-0	14-4-0	17-6-12	21-4-0
3-9-4	3-2-12	3-8-0	3-8-0	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.16	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.26	6-8	>955	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-8 oc bracing.

REACTIONS (lb/size) 2=1841/0-3-8, 6=1841/0-3-8
Max Horz 2=-92(load case 5)
Max Uplift2=-978(load case 4), 6=-978(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3339/1619, 3-4=-3176/1615, 4-5=-3176/1615, 5-6=-3339/1619, 6-7=0/35
BOT CHORD 2-10=-1368/2897, 9-10=-1380/2930, 8-9=-1327/2930, 6-8=-1316/2897
WEBS 3-10=-291/803, 3-9=-315/472, 4-9=-365/397, 5-9=-316/472, 5-8=-291/803

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 978 lb uplift at joint 2 and 978 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 321 lb up at 14-4-0, and 539 lb down and 321 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-113(F=-58), 5-7=-54, 2-10=-30, 8-10=-62(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#115051607
L139896	T03	HIP	1	1	Job Reference (optional)

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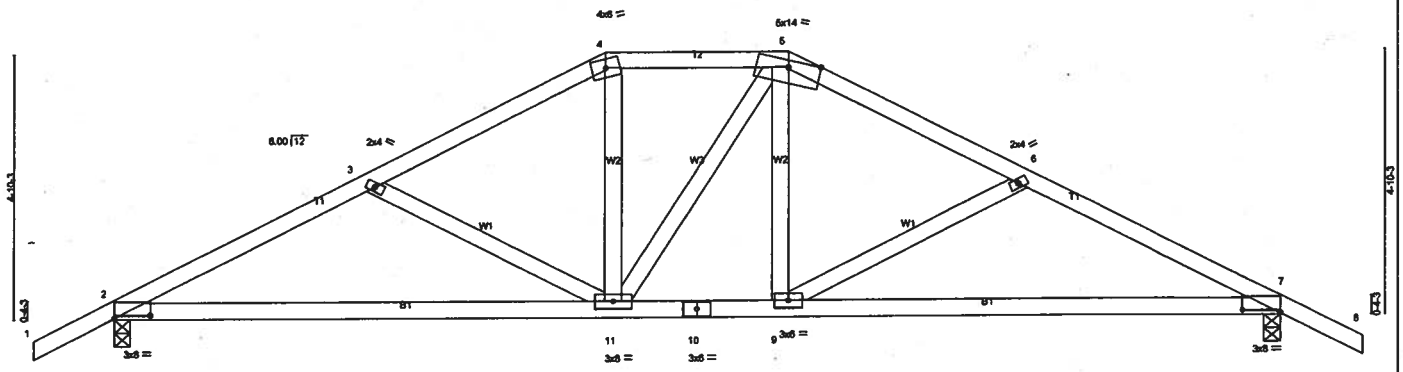
-1-6-0	4-9-4	9-0-0	12-4-0	16-6-12	21-4-0	22-10-0
1-6-0	4-9-4	4-2-12	3-4-0	4-2-12	4-9-4	1-6-0

Scale = 1:40.4



9-0-0	12-4-0	21-4-0
9-0-0	3-4-0	9-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.30	7-9	>829	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-7 oc bracing.

REACTIONS (lb/size) 2=973/0-3-8, 7=973/0-3-8
Max Horz 2=-109(load case 6)
Max Uplift2=-475(load case 5), 7=-475(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1468/815, 3-4=-1199/857, 4-5=-1030/648, 5-6=-1198/657, 6-7=-1468/815, 7-8=0/35
BOT CHORD 2-11=-568/1277, 10-11=-304/1028, 9-10=-304/1028, 7-9=-568/1277
WEBS 3-11=-291/300, 4-11=-100/314, 5-11=-102/106, 5-9=-100/315, 6-9=-293/300

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 475 lb uplift at joint 2 and 475 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051608
L139896	T04	COMMON	2	1	Job Reference (optional)

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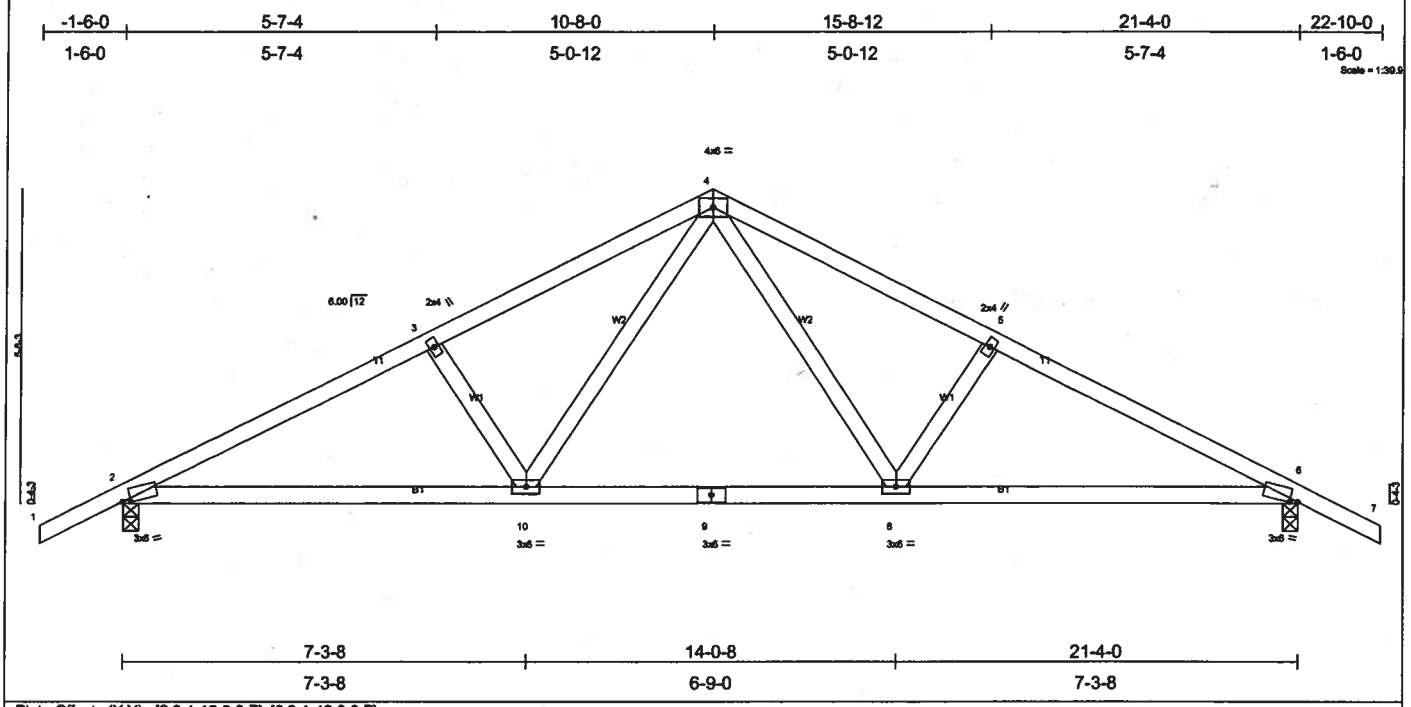

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.19	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.31	8-10	>821	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-10 oc bracing.

REACTIONS (lb/size) 2=1141/0-3-8, 6=1141/0-3-8
Max Horz 2=-123(load case 6)
Max Uplift2=-568(load case 5), 6=-568(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1888/1014, 3-4=-1732/1007, 4-5=-1732/1007, 5-6=-1888/1014, 6-7=0/35
BOT CHORD 2-10=-732/1619, 9-10=-383/1112, 8-9=-383/1112, 6-8=-732/1619
WEBS 3-10=-250/291, 4-10=-365/734, 4-8=-364/734, 5-8=-250/291

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 568 lb uplift at joint 2 and 568 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANYON CREEK Dwg.#I115051609
L139896	T05	COMMON	5	1	Job Reference (optional)

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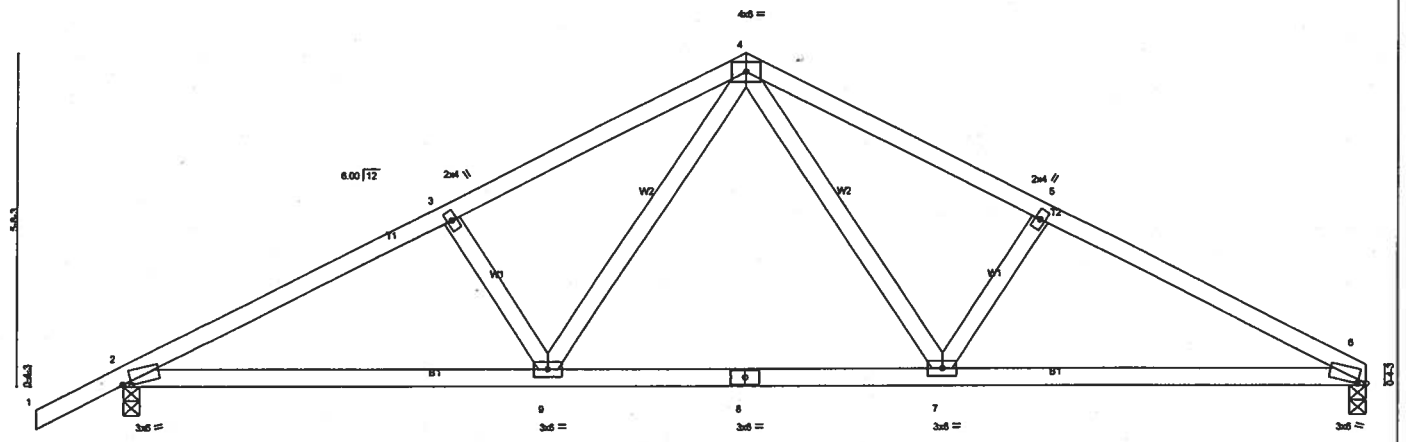
-1-6-0 | 5-7-4 | 10-8-0 | 15-8-12 | 21-4-0

1-6-0 | 5-7-4 | 5-0-12 | 5-0-12 | 5-7-4

Scale = 1:37.7



7-3-8 | 14-0-8 | 21-4-0

7-3-8 | 6-9-0 | 7-3-8

Plate Offsets (X,Y): [2:0-1-12,0-0-7], [6:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.19	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.30	7-9	>830	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 97 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-14 oc bracing.

REACTIONS (lb/size) 6=1049/0-3-8, 2=1145/0-3-8
Max Horz 2=144(load case 5)
Max Uplift6=-450(load case 6), 2=-570(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1894/1027, 3-4=-1738/1020, 4-5=-1752/1046, 5-6=-1894/1055
BOT CHORD 2-9=-813/1624, 8-9=-465/1119, 7-8=-465/1119, 6-7=-845/1640
WEBS 3-9=-250/292, 4-9=-361/733, 4-7=-402/753, 5-7=-260/311

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 6 and 570 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=-50), 6-7=-30

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051610
L139896	T06	SPECIAL	1	1	Job Reference (optional)

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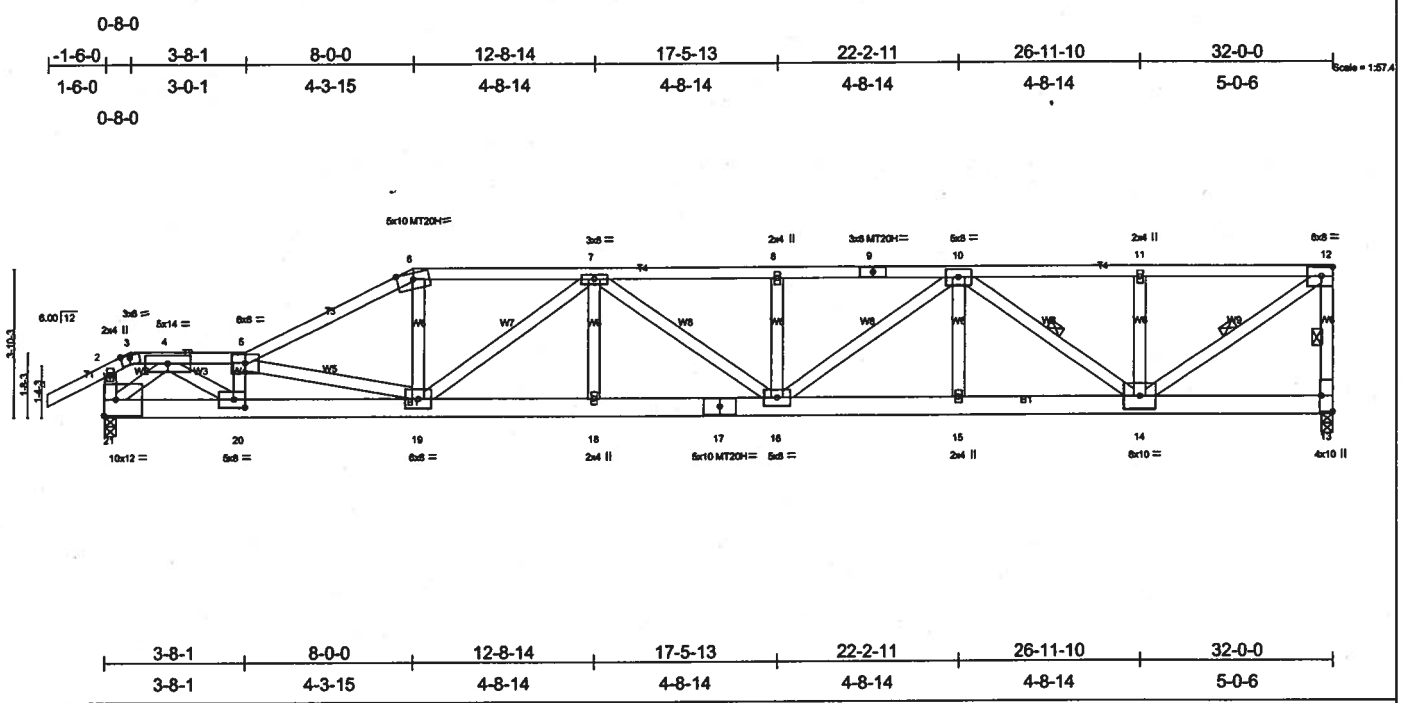

Plate Offsets (X,Y): [13:Edge,0-3-8], [20:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	0.47	16-18	>814	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.74	16-18	>515	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.84	Horz(TL)	0.14	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 208 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2, W9 2 X 4 SYP No.2, W1 2 X 4 SYP No.1D, W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-4-11 oc bracing.
WEBS 1 Row at midpt 12-13, 10-14, 12-14

REACTIONS (lb/size) 13=2838/0-3-8, 21=2696/0-3-8
Max Horz 21=226(load case 4)
Max Uplift13=-1601(load case 3), 21=-1421(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-258/154, 3-4=-214/142, 4-5=-5976/3114, 5-6=-5829/3177, 6-7=-5292/2935, 7-8=-6576/3693, 8-9=-6576/3693, 9-10=-6576/3693, 10-11=-3449/1939, 11-12=-3449/1939, 12-13=-2651/1587, 2-21=-237/227
BOT CHORD 20-21=-1554/2812, 19-20=-3335/6244, 18-19=-3636/6480, 17-18=-3636/6480, 16-17=-3636/6480, 15-16=-3162/5610, 14-15=-3162/5610, 13-14=-72/100
WEBS 4-20=-1998/3868, 5-20=-2401/1319, 5-19=-1086/592, 6-19=-1072/2149, 7-19=-1489/909, 7-18=0/295, 7-16=-91/119, 8-16=-521/519, 10-16=-659/1198, 10-15=0/335, 10-14=-2678/1516, 11-14=-545/554, 12-14=-2290/4108, 4-21=-3427/1790

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1601 lb uplift at joint 13 and 1421 lb uplift at joint 21.
5) Girder carries hip end with 0-0-0 right side setback, 8-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 616 lb down and 397 lb up at 8-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-5=-54, 5-6=-54, 6-12=-113(F=-59), 19-21=-30, 13-19=-62(F=-33)
Concentrated Loads (lb)
Vert: 19=-616(F)

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#115051611
L139896	T07	SPECIAL	1	1	Job Reference (optional)

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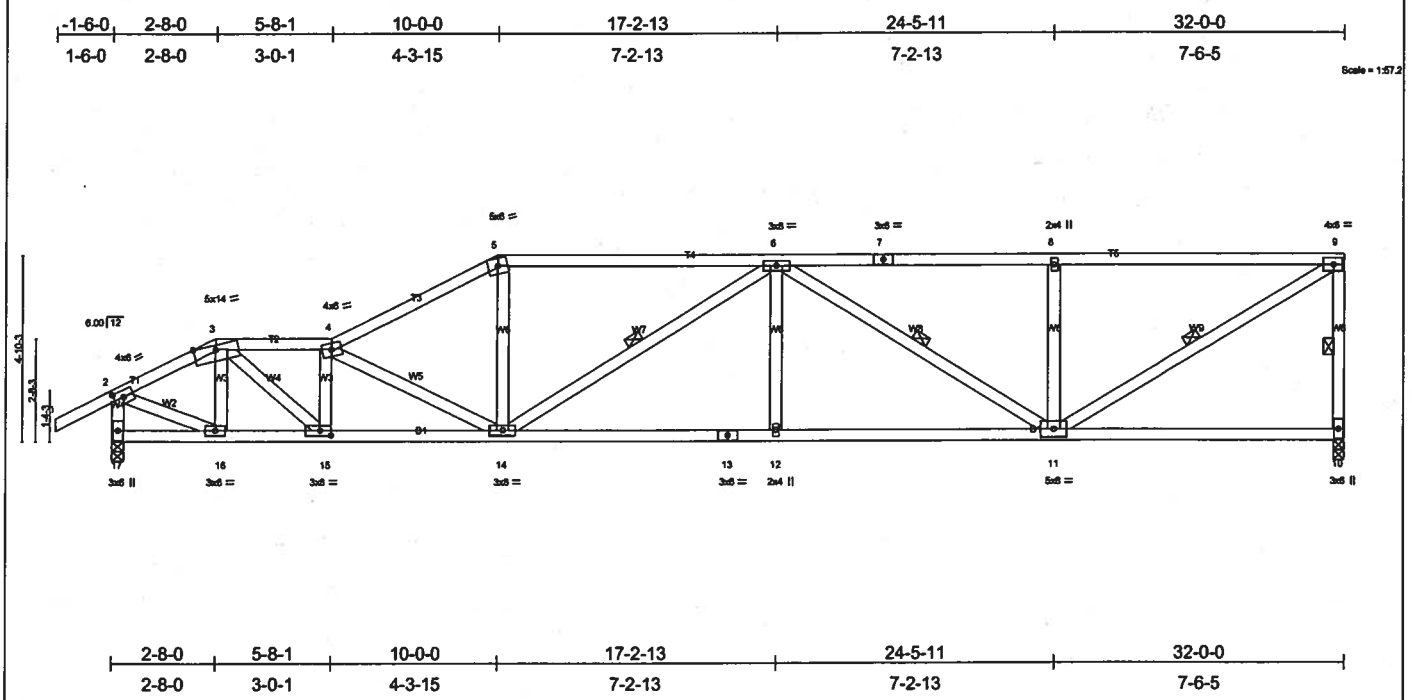

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [15:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.34	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-7 oc bracing.
WEBS 1 Row at midpt 9-10, 6-14, 6-11, 9-11

REACTIONS (lb/size) 10=1329/0-3-8, 17=1423/0-3-8
Max Horz 17=283(load case 5)
Max Uplift10=-619(load case 4), 17=-613(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1515/771, 3-4=-2461/1281, 4-5=-2293/1181, 5-6=-2053/1135, 6-7=-1733/901, 7-8=-1733/901, 8-9=-1733/901, 9-10=-1218/698, 2-17=-1373/825
BOT CHORD 16-17=-248/33, 15-16=-829/1292, 14-15=-1445/2516, 13-14=-1257/2373, 12-13=-1257/2373, 11-12=-1257/2373, 10-11=-34/68
WEBS 3-16=-458/255, 3-15=-760/1522, 4-15=-900/520, 4-14=-548/358, 5-14=-214/661, 6-14=-380/269, 6-12=0/217, 6-11=-756/421, 8-11=-406/366, 9-11=-1018/1957, 2-16=-647/1394

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 619 lb uplift at joint 10 and 613 lb uplift at joint 17.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T08	SPECIAL	1	1	Dwg.#115051612

Job Reference (optional)

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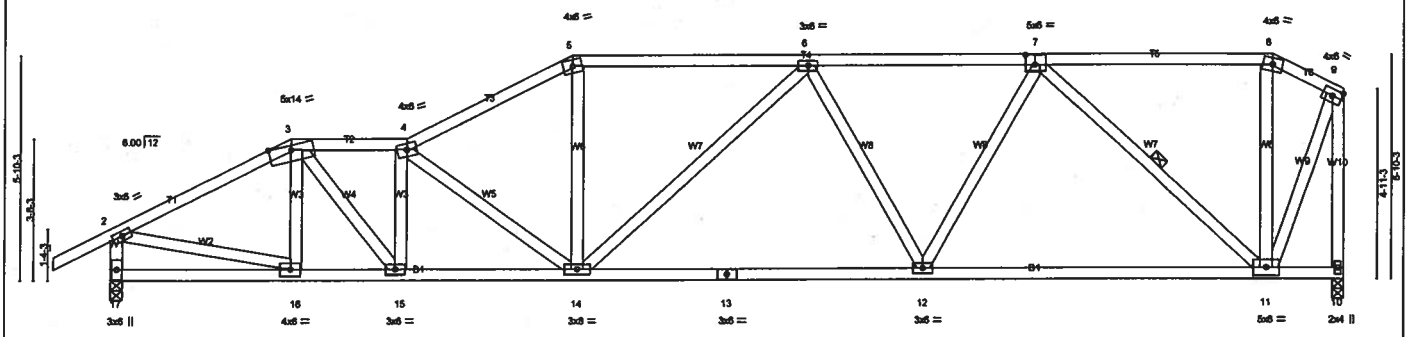
-1-6-0 | 4-8-0 | 7-8-1 | 12-0-0 | 18-0-11 | 24-1-5 | 30-2-0 | 32-0-0

1-6-0 | 4-8-0 | 3-0-1 | 4-3-15 | 6-0-11 | 6-0-11 | 6-0-11 | 1-10-0

Scale = 1:57.2



4-8-0 | 7-8-1 | 12-0-0 | 21-1-0 | 30-2-0 | 32-0-0

4-8-0 | 3-0-1 | 4-3-15 | 9-1-0 | 9-1-0 | 1-10-0

Plate Offsets (X,Y): [7:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.36	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.07	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-14 oc bracing.
WEBS 1 Row at midpt 7-11

REACTIONS (lb/size) 17=1423/0-3-8, 10=1329/0-3-8
Max Horz 17=305(load case 5)
Max Uplift17=-631(load case 5), 10=-545(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1791/910, 3-4=-2210/1195, 4-5=-2058/1090, 5-6=-1831/1048, 6-7=-1724/926, 7-8=-459/274, 8-9=-500/256, 2-17=-1340/832, 9-10=-1376/671
BOT CHORD 16-17=-316/130, 15-16=-940/1535, 14-15=-1292/2242, 13-14=-1018/1878, 12-13=-1018/1878, 11-12=-760/1416, 10-11=-5/4
WEBS 3-16=-225/177, 3-15=-519/1046, 4-15=-739/414, 4-14=-535/379, 5-14=-206/597, 6-14=-202/170, 6-12=-322/302, 7-12=-237/645, 7-11=-1317/739, 8-11=-56/119, 2-16=-649/1455, 9-11=-604/1254

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 631 lb uplift at joint 17 and 545 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T09	SPECIAL	1	1	Dwg.#115051613

Job Reference (optional)

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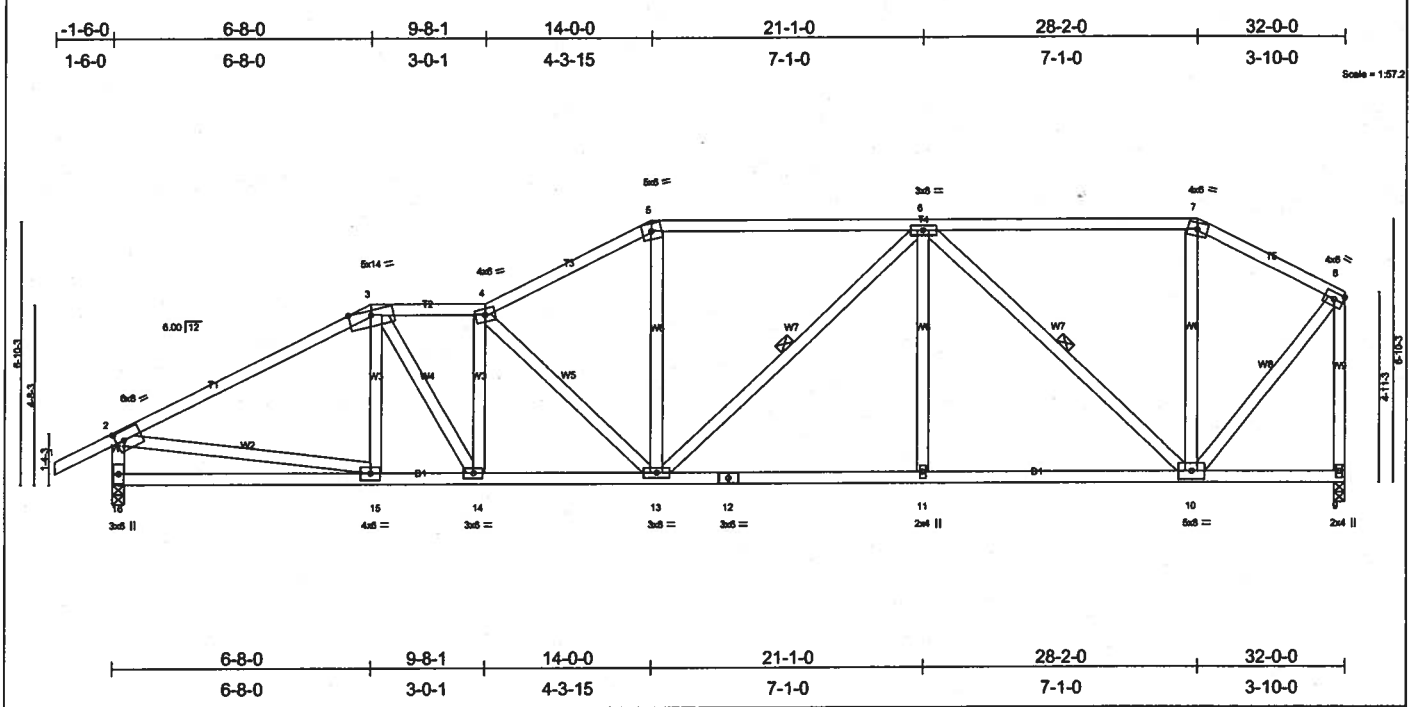

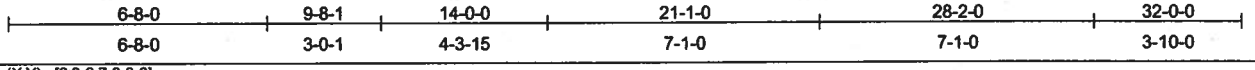

Plate Offsets (X,Y): [2:0-2-7,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	-0.14	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.22	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-4 oc bracing.
WEBS 1 Row at midpt 6-13, 6-10

REACTIONS (lb/size) 16=1423/0-3-8, 9=1329/0-3-8
Max Horz 16=321(load case 5)
Max Uplift16=-648(load case 5), 9=-479(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1887/964, 3-4=-1994/1119, 4-5=-1825/1019, 5-6=-1623/987, 6-7=-719/476, 7-8=-830/462, 2-16=-1310/844, 8-9=-1290/709
BOT CHORD 15-16=-412/271, 14-15=-959/1608, 13-14=-1147/2011, 12-13=-781/1497, 11-12=-781/1497, 10-11=-781/1497, 9-10=-8/7
WEBS 3-15=-73/130, 3-14=-327/706, 4-14=-525/287, 4-13=-559/385, 5-13=-145/463, 6-13=-161/183, 6-11=0/218, 6-10=-1088/575, 7-10=0/93, 2-15=-559/1358, 8-10=-566/1125

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 648 lb uplift at joint 16 and 479 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#115051614
L139896	T10	SPECIAL	1	1	Job Reference (optional)

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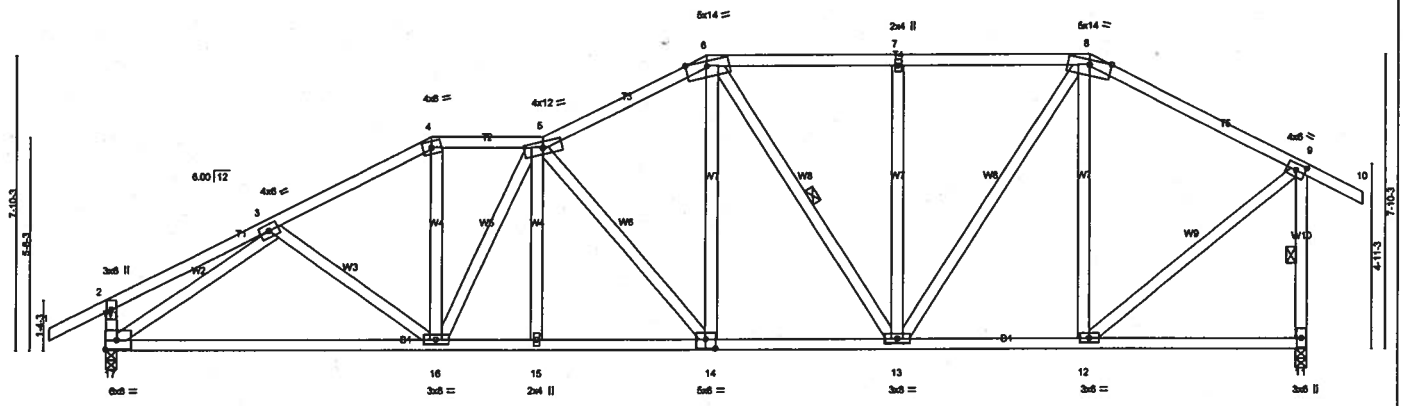
-1-6-0	4-5-12	8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0	33-6-0
1-6-0	4-5-12	4-2-4	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0	1-6-0

Scale = 1:58.7



8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0
8-8-0	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0

Plate Offsets (X,Y): [9:0-2-15,0-2-0], [14:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.13	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.23	16-17	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.06	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 229 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-2 oc bracing.
WEBS 1 Row at midpt 6-13, 9-11

REACTIONS (lb/size) 17=1421/0-3-8, 11=1421/0-3-8
Max Horz 17=297(load case 4)
Max Uplift17=-674(load case 5), 11=-565(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-335/151, 3-4=-1802/1023, 4-5=-1583/979, 5-6=-1611/1004, 6-7=-1264/887, 7-8=-1264/887, 8-9=-1024/684, 9-10=0/40, 2-17=-348/327, 9-11=-1335/865
BOT CHORD 16-17=-715/1467, 15-16=-786/1806, 14-15=-786/1805, 13-14=-558/1415, 12-13=-333/844, 11-12=-46/98
WEBS 3-16=-58/162, 4-16=-262/561, 5-16=-476/290, 5-15=0/46, 5-14=-629/443, 6-14=-312/645, 6-13=-288/181, 7-13=-281/247, 8-13=-376/793, 8-12=-495/280, 3-17=-1538/913, 9-12=-410/1060

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 674 lb uplift at joint 17 and 565 lb uplift at joint 11.

LOAD CASE(S) Standard

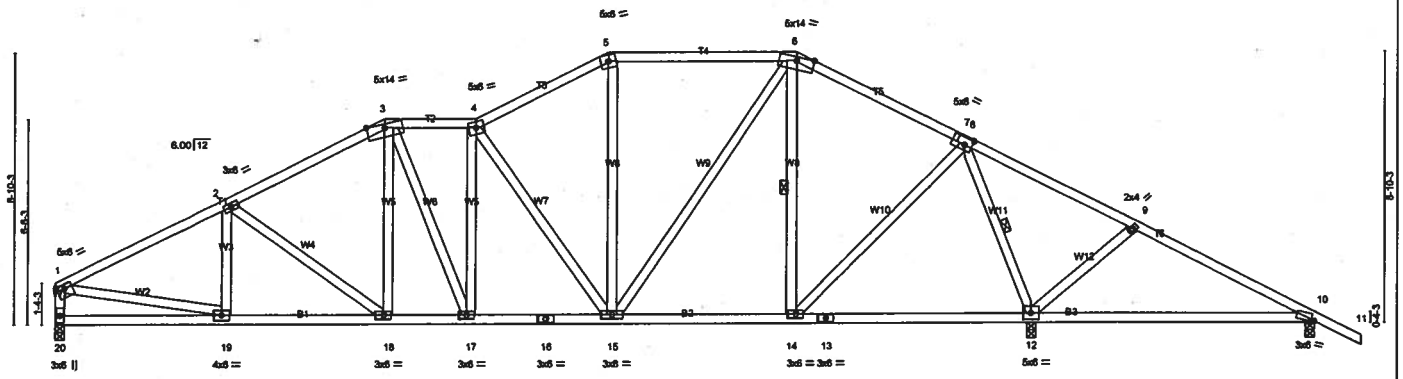
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051615
L139896	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:40 2005 Page 1

5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	29-7-13	35-1-11	41-2-0	42-8-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	5-5-13	5-5-13	6-0-5	1-6-0

Scale = 1:72.0



5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	31-10-4	41-2-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	7-8-4	9-3-12

Plate Offsets (X,Y): [1:Edge,0-1-12], [7:0-2-12,0-3-0], [10:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	0.41	10-12	>266	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	0.35	10-12	>312	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-14, 8-12

REACTIONS (lb/size) 20=1256/0-3-8, 12=2045/0-3-8, 10=221/0-3-8
Max Horz 20=-209(load case 3)
Max Uplift20=-539(load case 5), 12=-913(load case 6), 10=-345(load case 6)
Max Grav 20=1256(load case 1), 12=2045(load case 1), 10=282(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1744/943, 2-3=-1589/943, 3-4=-1461/954, 4-5=-1238/836, 5-6=-1076/816, 6-7=-882/612, 7-8=-886/592, 8-9=-177/587, 9-10=-79/357, 10-11=0/35, 1-20=-1167/676
BOT CHORD 19-20=-195/213, 18-19=-652/1501, 17-18=-494/1368, 16-17=-511/1464, 15-16=-511/1464, 14-15=-114/728, 13-14=0/174, 12-13=0/174, 10-12=-287/160
WEBS 2-19=-118/170, 2-18=-177/204, 3-18=-104/235, 3-17=-111/228, 4-17=-121/128, 4-15=-681/476, 5-15=-97/225, 6-15=-346/648, 6-14=-449/309, 8-14=-301/898, 8-12=-1633/892, 9-12=-351/439, 1-19=-600/1315

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 20, 913 lb uplift at joint 12 and 345 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

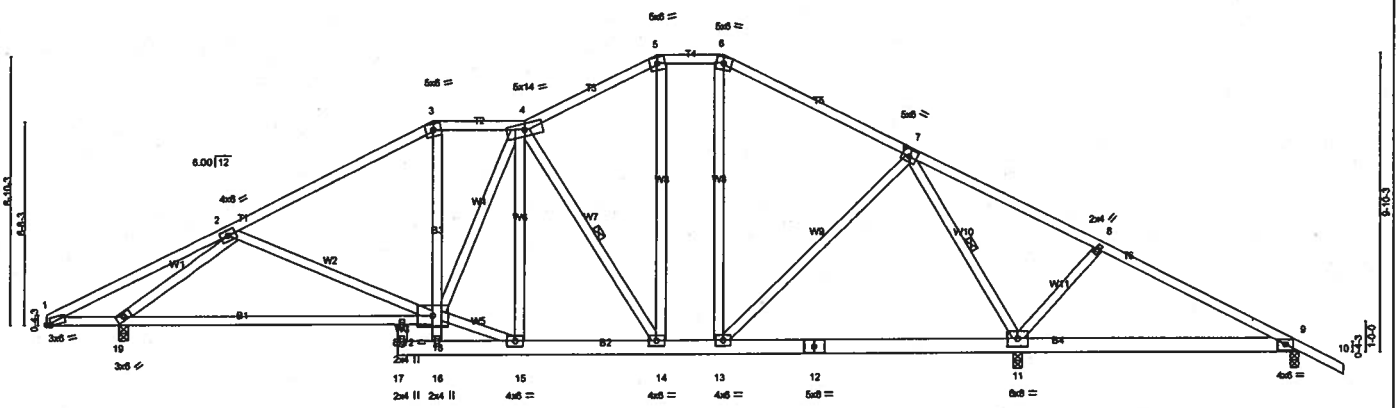
Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T12	SPECIAL	1	1	Dwg.#1115051616

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:41 2005 Page 1

6-0-4	11-6-0	12-8-0	15-8-1	20-0-0	22-2-0	28-3-13	34-5-11	41-2-0	42-8-0
6-0-4	5-5-12	1-2-0	3-0-1	4-3-15	2-2-0	6-1-13	6-1-13	6-8-5	1-6-0

Scale = 1:72.4



2-5-12	11-6-0	12-8-0	15-8-1	20-0-0	22-2-0	31-10-4	41-2-0
2-5-12	9-0-4	1-2-0	3-0-1	4-3-15	2-2-0	9-8-4	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.14	9-11	>768	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	0.12	9-11	>888	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 280 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 7-11

REACTIONS (lb/size) 11=1806/0-3-8, 9=343/0-3-8, 19=1424/0-3-8
Max Horz 19=-254(load case 6)
Max Uplift11=-872(load case 6), 9=-373(load case 6), 19=-686(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-303/85, 2-3=-1566/836, 3-4=-1310/825, 4-5=-977/724, 5-6=-827/705, 6-7=-998/697, 7-8=-14/329, 8-9=-56/103, 9-10=0/39
BOT CHORD 1-19=-10/341, 18-19=-575/1252, 16-18=0/59, 3-18=-99/383, 16-17=0/0, 15-16=-231/0, 14-15=-380/1204, 13-14=-135/827, 12-13=-69/454, 11-12=-69/454, 9-11=-57/93
WEBS 2-18=-16/179, 15-18=-256/1446, 4-18=-46/282, 4-15=-230/28, 4-14=-758/515, 5-14=-238/318, 6-13=-56/215, 7-13=-92/529, 7-11=-1409/760, 8-11=-335/419, 2-19=-1640/1232

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 872 lb uplift at joint 11, 373 lb uplift at joint 9 and 686 lb uplift at joint 19.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

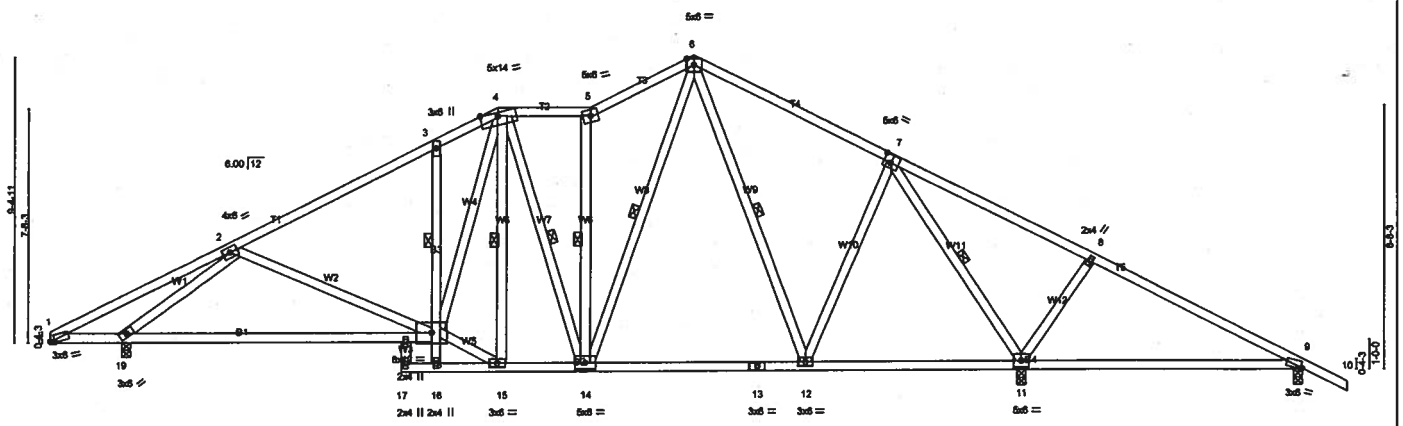
Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T13	SPECIAL	1	1	Job Reference (optional)

Dwg.#115051617

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:42 2005 Page 1

6-0-4	11-6-0	14-8-0	17-8-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
6-0-4	5-5-12	3-2-0	3-0-1	3-4-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.4



14-8-0						
2-5-12	11-6-0	12-6-0	17-8-1	24-9-2	31-10-4	41-2-0
2-5-12	9-0-4	1-0-0 2-2-0	3-0-1	7-1-2	7-1-2	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.45	9-11	>243	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	0.38	9-11	>290	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 273 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 3-18
WEBS 1 Row at midpt 4-15, 4-14, 5-14, 6-14, 6-12, 7-11

REACTIONS (lb/size) 11=1892/0-3-8, 9=277/0-3-8, 19=1400/0-3-8
Max Horz 19=-261(load case 6)
Max Uplift11=-918(load case 6), 9=-345(load case 6), 19=-669(load case 5)
Max Grav 11=1892(load case 1), 9=331(load case 10), 19=1400(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-301/91, 2-3=-1523/808, 3-4=-1435/955, 4-5=-994/747, 5-6=-1131/880, 6-7=-798/622, 7-8=-106/477, 8-9=-98/274, 9-10=0/35
BOT CHORD 1-19=-14/339, 18-19=-562/1225, 16-18=0/64, 3-18=-239/341, 16-17=0/0, 15-16=-184/0, 14-15=-267/991, 13-14=-97/737, 12-13=-97/737, 11-12=-5/453, 9-11=-211/208
WEBS 2-18=0/186, 15-18=-167/1241, 4-18=-420/976, 4-15=-472/84, 4-14=-46/71, 5-14=-593/487, 6-14=-511/850, 6-12=-286/169, 7-12=-88/513, 7-11=-1535/789, 8-11=-373/466, 2-19=-1609/1204

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 918 lb uplift at joint 11, 345 lb uplift at joint 9 and 669 lb uplift at joint 19.

LOAD CASE(S) Standard

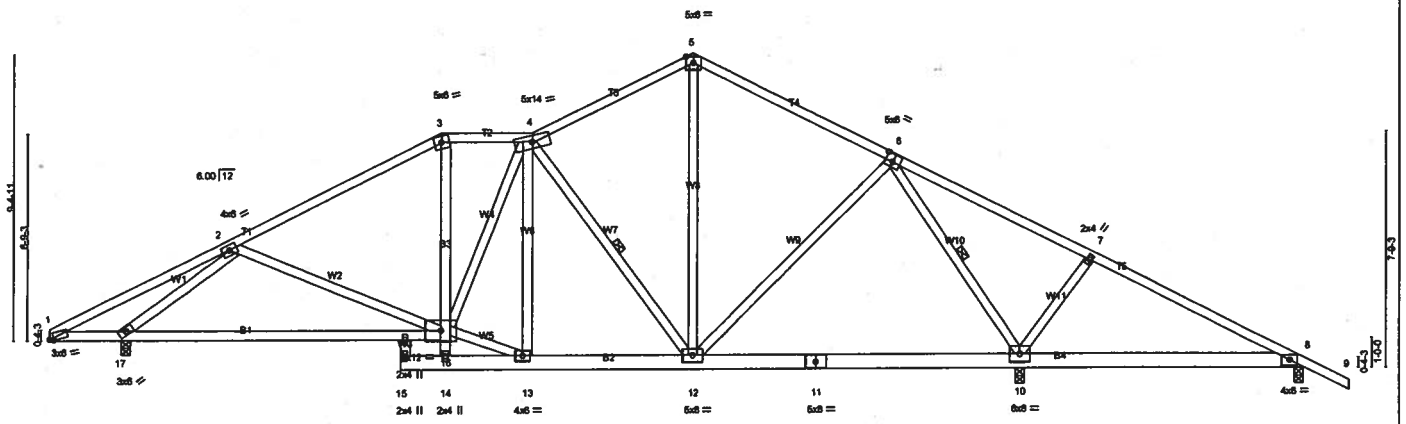
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051618
L139896	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:43 2005 Page 1

6-0-4	11-6-0	12-10-0	15-10-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
6-0-4	5-5-12	1-4-0	3-0-1	5-2-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.4



2-5-12	11-6-0	12-10-0	15-10-1	23-10-2	31-10-4	41-2-0
2-5-12	9-0-4	1-4-0	3-0-1	8-0-2	8-0-2	9-3-12

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.14	8-10	>778	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	0.13	8-10	>877	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 270 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 6-10

REACTIONS (lb/size) 10=1903/0-3-8, 8=271/0-3-8, 17=1405/0-3-8
Max Horz 17=-263(load case 6)
Max Uplift10=-914(load case 6), 8=-348(load case 6), 17=-668(load case 5)
Max Grav 10=1903(load case 1), 8=329(load case 10), 17=1405(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-306/78, 2-3=-1527/801, 3-4=-1272/796, 4-5=-916/666, 5-6=-943/661, 6-7=-116/469, 7-8=-107/287, 8-9=0/39
BOT CHORD 1-17=-3/343, 16-17=-565/1231, 14-16=0/98, 3-16=-71/354, 14-15=0/0, 13-14=-239/0, 12-13=-344/1140, 11-12=-18/441, 10-11=-18/441, 8-10=-233/234
WEBS 2-16=-4/195, 13-16=-205/1367, 4-16=-73/364, 4-13=-360/77, 4-12=-657/484, 5-12=-253/433, 6-12=-105/510, 6-10=-1482/831, 7-10=-345/433, 2-17=-1602/1211

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 914 lb uplift at joint 10, 348 lb uplift at joint 8 and 668 lb uplift at joint 17.

LOAD CASE(S) Standard

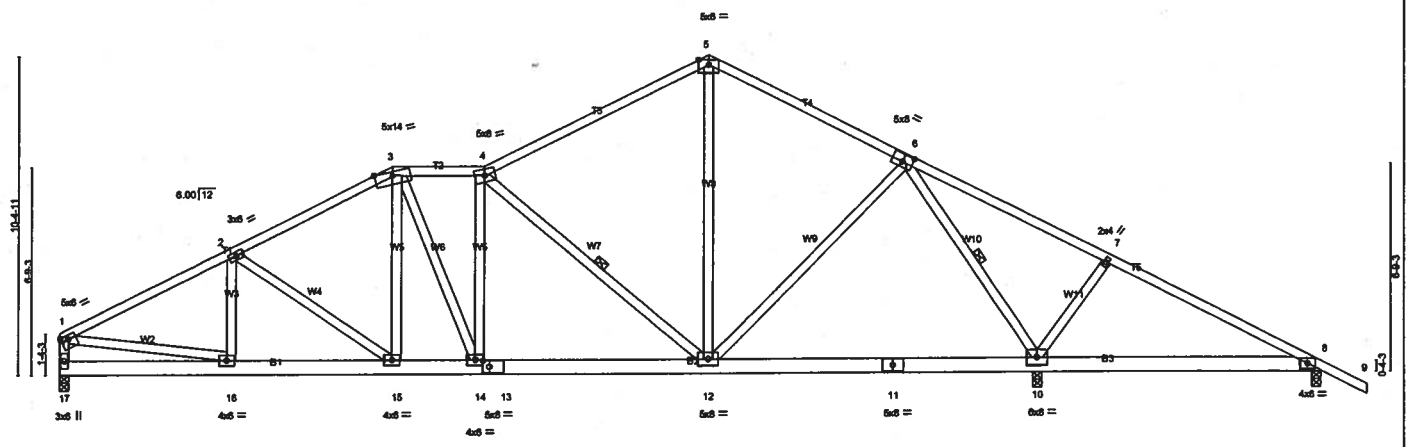
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051619
L139896	T15	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:44 2005 Page 1

5-6-12	10-10-0	13-10-1	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
5-6-12	5-3-4	3-0-1	7-2-15	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:72.0



5-6-12	10-10-0	13-10-1	21-1-0	31-10-4	41-2-0
5-6-12	5-3-4	3-0-1	7-2-15	10-9-4	9-3-12

Plate Offsets (X,Y): [1:Edge,0-1-12], [6:0-4-0,0-3-0], [13:0-2-7,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.14	8-10	>781	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.12	8-10	>882	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 276 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 6-10

REACTIONS (lb/size) 17=1264/0-3-8, 10=2012/0-3-8, 8=247/0-3-8
Max Horz 17=-239(load case 6)
Max Uplift17=-549(load case 5), 10=-964(load case 6), 8=-328(load case 6)
Max Grav 17=1264(load case 1), 10=2012(load case 1), 8=315(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1782/963, 2-3=-1613/956, 3-4=-1504/993, 4-5=-1033/702, 5-6=-1024/712, 6-7=-166/524, 7-8=-140/334, 8-9=0/39, 1-17=-1153/666
BOT CHORD 16-17=-274/293, 15-16=-700/1534, 14-15=-533/1387, 13-14=-580/1507, 12-13=-580/1507, 11-12=-28/460, 10-11=-28/460, 8-10=-275/280
WEBS 2-16=-115/173, 2-15=-207/216, 3-15=-111/258, 3-14=-176/287, 4-14=-141/173, 4-12=-885/648, 5-12=-241/456, 6-12=-164/591, 6-10=-1612/929, 7-10=-349/438, 1-16=-549/1263

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 17, 964 lb uplift at joint 10 and 328 lb uplift at joint 8.

LOAD CASE(S) Standard

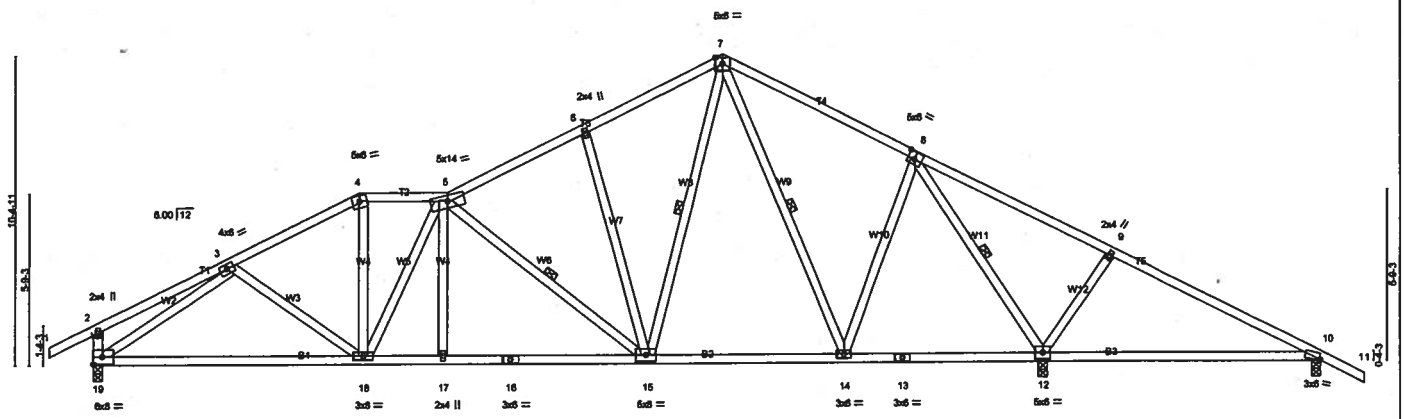
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T16	SPECIAL	1	1	Dwg.#1115051620
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:45 2005 Page 1

1-6-0	4-6-12	8-10-0	11-10-1	16-5-8	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	4-6-12	4-3-4	3-0-1	4-7-8	4-7-8	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



8-10-0	11-10-1	18-6-2	25-2-3	31-10-4	41-2-0
8-10-0	3-0-1	6-8-1	6-8-1	6-8-1	9-3-12

Plate Offsets (X,Y): [8:0-3-0,0-3-0], [10:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.45	10-12	>246	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	0.37	10-12	>294	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.06	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 258 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-15, 7-15, 7-14, 8-12

REACTIONS (lb/size) 12=2020/0-3-8, 19=1353/0-3-8, 10=239/0-3-8
Max Horz 19=-210(load case 6)
Max Uplift12=-964(load case 6), 19=-661(load case 5), 10=-327(load case 6)
Max Grav 12=2020(load case 1), 19=1353(load case 1), 10=309(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-342/148, 3-4=-1676/956, 4-5=-1466/919, 5-6=-1267/817, 6-7=-1199/887, 7-8=-807/640, 8-9=-155/559, 9-10=-142/356, 10-11=0/35, 2-19=-351/326
BOT CHORD 18-19=-679/1384, 17-18=-659/1659, 16-17=-659/1657, 15-16=-659/1657, 14-15=-135/802, 13-14=-9/458, 12-13=-9/458, 10-12=-275/257
WEBS 3-18=-11/119, 4-18=-240/514, 5-18=-416/241, 5-17=0/103, 5-15=-734/483, 6-15=-247/273, 7-15=-558/946, 7-14=-414/217, 8-14=-133/589, 8-12=-1684/885, 9-12=-374/467, 3-19=-1423/860

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 964 lb uplift at joint 12, 661 lb uplift at joint 19 and 327 lb uplift at joint 10.

LOAD CASE(S) Standard

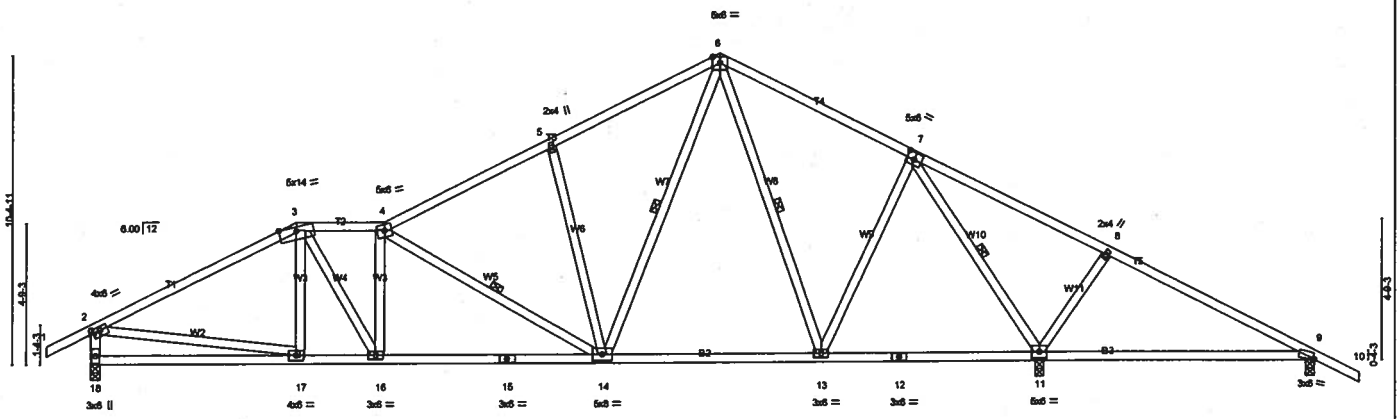
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051621
L139896	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:46 2005 Page 1

1-6-0	6-10-0	9-10-1	15-5-8	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	6-10-0	3-0-1	5-7-8	5-7-8	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



6-10-0	9-10-1	17-2-2	24-6-3	31-10-4	41-2-0
6-10-0	3-0-1	7-4-1	7-4-1	7-4-1	9-3-12

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.45	9-11	>242	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	0.38	9-11	>287	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 248 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1344/0-3-8, 11=2058/0-3-8, 9=209/0-3-8
Max Horz 18=-210(load case 6)
Max Uplift18=-659(load case 5), 11=-974(load case 6), 9=-319(load case 6)
Max Grav 18=1344(load case 1), 11=2058(load case 1), 9=290(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1753/944, 3-4=-1843/1098, 4-5=-1405/866, 5-6=-1334/965, 6-7=-823/642, 7-8=-181/622, 8-9=-158/419, 9-10=0/35, 2-18=-1227/833
BOT CHORD 17-18=-268/290, 16-17=-654/1486, 15-16=-782/1856, 14-15=-782/1856, 13-14=-133/783, 12-13=-3/421, 11-12=-3/421, 9-11=-328/280
WEBS 3-17=-77/113, 3-16=-324/663, 4-16=-404/308, 4-14=-764/492, 5-14=-307/357, 6-14=-610/992, 6-13=-375/219, 7-13=-144/607, 7-11=-1721/911, 8-11=-376/469, 2-17=-513/1214

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint 18, 974 lb uplift at joint 11 and 319 lb uplift at joint 9.

LOAD CASE(S) Standard

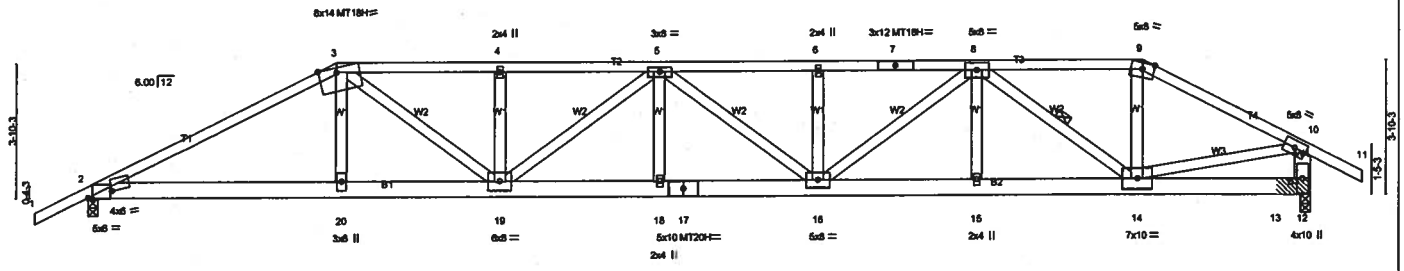
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#115051622
L139896	T18	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:48 2005 Page 1

-1-6-0	3-9-4	7-0-0	11-8-4	16-2-12	20-9-4	25-3-12	30-0-0	34-10-0	36-4-0
1-6-0	3-9-4	3-2-12	4-8-4	4-6-8	4-6-8	4-6-8	4-8-4	4-10-0	1-6-0

Scale = 1:62.7



3-9-4	7-0-0	11-8-4	16-2-12	20-9-4	25-3-12	30-0-0	34-10-0
3-9-4	3-2-12	4-8-4	4-6-8	4-6-8	4-6-8	4-8-4	4-10-0

Plate Offsets (X,Y): [2:0-8-10,0-0-7], [2:0-1-10,Edge], [3:0-6-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	0.56	16-18	>736	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.89	16-18	>464	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.74	Horz(TL)	0.17	12	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 218 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 6 SYP No.1D, W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-1-3 oc bracing.
WEBS 1 Row at midpt 8-14

REACTIONS (lb/size) 2=2968/0-3-8, 12=3155/0-3-12 (0-3-8 + bearing block)
Max Horz 2=-241(load case 5)
Max Uplift2=-1564(load case 4), 12=-1636(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-5883/3187, 3-4=-6969/3936, 4-5=-6968/3937, 5-6=-7366/4173, 6-7=-7366/4173, 7-8=-7366/4173, 8-9=-3634/2132, 9-10=-4128/2241, 10-11=0/42, 10-12=-2918/1614
BOT CHORD 2-20=-2678/5179, 19-20=-2691/5214, 18-19=-4160/7733, 17-18=-4160/7733, 16-17=-4160/7733, 15-16=-3233/6063, 14-15=-3233/6063, 13-14=-268/477, 12-13=-268/477
WEBS 3-20=-321/838, 3-19=-1356/2280, 4-19=-518/529, 5-19=-991/587, 5-18=0/338, 5-16=-480/258, 6-16=-494/497, 8-16=-909/1655, 8-15=0/337, 8-14=-3088/1740, 9-14=-546/1325, 10-14=-1636/3193

NOTES
1) 2 X 6 SYP No.1D bearing block 12" long at jt. 12 attached to front face with 3 rows of 0.131"x3" Nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1564 lb uplift at joint 2 and 1636 lb uplift at joint 12.
7) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-9=-113(F=-58), 9-10=-112(F=-58), 10-11=-54, 2-20=-30, 12-20=-62(F=-33)
Concentrated Loads (lb)
Vert: 20=-539(F)

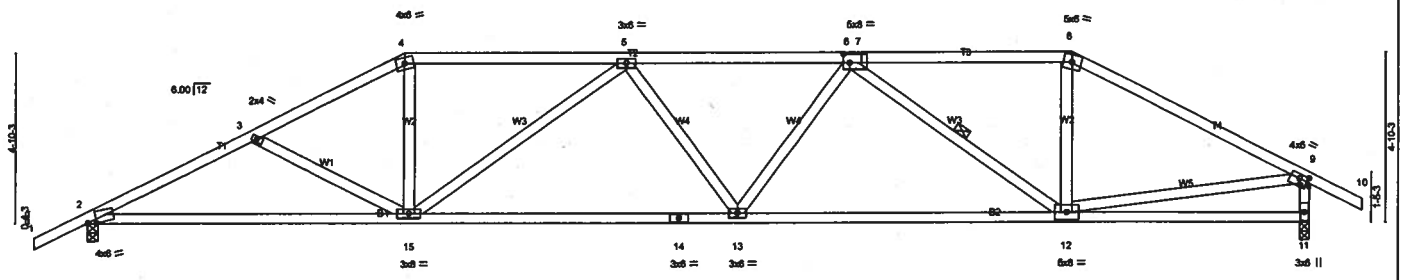
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051623
L139896	T19	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:48 2005 Page 1

-1-6-0	4-9-4	9-0-0	15-4-0	21-8-0	28-0-0	34-10-0	36-4-0
1-6-0	4-9-4	4-2-12	6-4-0	6-4-0	6-4-0	6-10-0	1-6-0

Scale = 1:62.7



9-0-0	18-6-0	28-0-0	34-10-0
9-0-0	9-6-0	9-6-0	6-10-0

Plate Offsets (X,Y): [2:0-3-0,0-0-11], [7:0-2-0,0-3-0], [9:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.30	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.50	12-13	>836	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.12	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-8 oc bracing.
WEBS 1 Row at midpt 6-12

REACTIONS (lb/size) 2=1540/0-3-8, 11=1540/0-3-8
Max Horz 2=118(load case 5)
Max Uplift2=-650(load case 5), 11=-620(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2681/1409, 3-4=-2457/1276, 8-9=-2081/1066, 9-10=0/40, 9-11=-1444/909, 4-5=-2180/1216, 5-6=-2748/1477, 6-7=-1803/1044, 7-8=-1803/1044
BOT CHORD 2-15=-1100/2343, 14-15=-1194/2715, 13-14=-1194/2715, 12-13=-1127/2582, 11-12=-85/217
WEBS 3-15=-212/258, 4-15=-290/779, 5-15=-749/451, 5-13=0/109, 6-13=-25/315, 6-12=-1035/549, 8-12=-161/598, 9-12=-646/1592

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 650 lb uplift at joint 2 and 620 lb uplift at joint 11.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

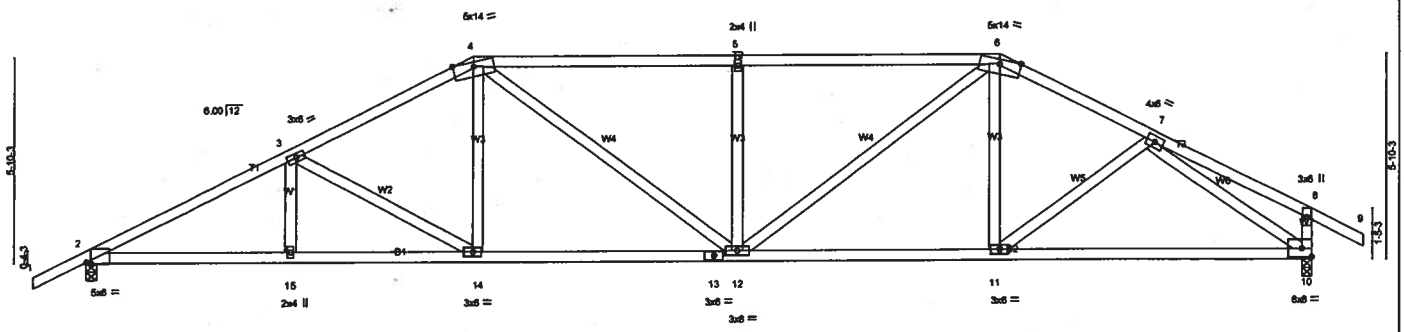
Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T20	HIP	1	1	Dwg.#1115051624

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:49 2005 Page 1

-1-6-0	5-9-4	11-0-0	18-6-0	26-0-0	30-3-4	34-10-0	36-4-0
1-6-0	5-9-4	5-2-12	7-6-0	7-6-0	4-3-4	4-6-12	1-6-0

Scale = 1:62.7



5-9-4	11-0-0	18-6-0	26-0-0	34-10-0
5-9-4	5-2-12	7-6-0	7-6-0	8-10-0

Plate Offsets (X,Y): [2:0-1-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.35	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.11	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 192 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-5 oc bracing.

REACTIONS (lb/size) 2=1540/0-3-8, 10=1540/0-3-8
Max Horz 2=135(load case 5)
Max Uplift2=-672(load case 5), 10=-644(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2733/1390, 3-4=-2270/1241, 4-5=-2303/1341, 5-6=-2303/1341, 6-7=-1994/1106, 7-8=-336/172, 8-9=0/40, 8-10=-358/345
BOT CHORD 2-15=-1073/2365, 14-15=-1073/2365, 13-14=-791/1990, 12-13=-791/1990, 11-12=-662/1754, 10-11=-695/1582
WEBS 3-15=0/165, 3-14=-442/325, 4-14=-119/410, 4-12=-291/513, 5-12=-429/375, 6-12=-377/762, 6-11=-10/135, 7-11=-133/317, 7-10=-1700/954

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 672 lb uplift at joint 2 and 644 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

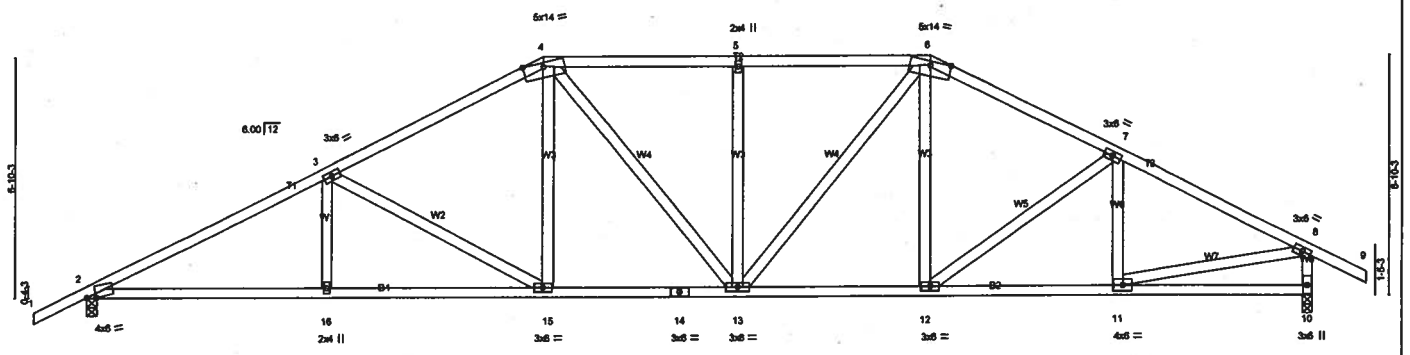
Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T21	HIP	1	1	Dwg.#1115051625

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:50 2005 Page 1

-1-6-0	6-9-4	13-0-0	18-6-0	24-0-0	29-3-4	34-10-0	36-4-0
1-6-0	6-9-4	6-2-12	5-6-0	5-6-0	5-3-4	5-6-12	1-6-0

Scale = 1:62.7



6-9-4	13-0-0	18-6-0	24-0-0	29-3-4	34-10-0
6-9-4	6-2-12	5-6-0	5-6-0	5-3-4	5-6-12

Plate Offsets (X,Y): [2:0-3-4,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.16	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.27	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.10	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 204 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-8 oc bracing.

REACTIONS (lb/size) 2=1540/0-3-8, 10=1540/0-3-8
Max Horz 2=151(load case 5)
Max Uplift2=-691(load case 5), 10=-665(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2705/1394, 3-4=-2097/1185, 4-5=-1920/1187, 5-6=-1920/1187, 6-7=-1911/1107, 7-8=-1990/1068, 8-9=0/40, 8-10=-1448/924
BOT CHORD 2-16=-1066/2337, 15-16=-1066/2337, 14-15=-687/1814, 13-14=-687/1814, 12-13=-606/1659, 11-12=-730/1717, 10-11=-34/162
WEBS 3-16=0/220, 3-15=-608/435, 4-15=-172/462, 4-13=-189/307, 5-13=-303/269, 6-13=-244/505, 6-12=-78/233, 7-12=-109/183, 7-11=-196/203, 8-11=-742/1592

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 691 lb uplift at joint 2 and 665 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#115051626
L139896	T22	HIP	1	1	Job Reference (optional)

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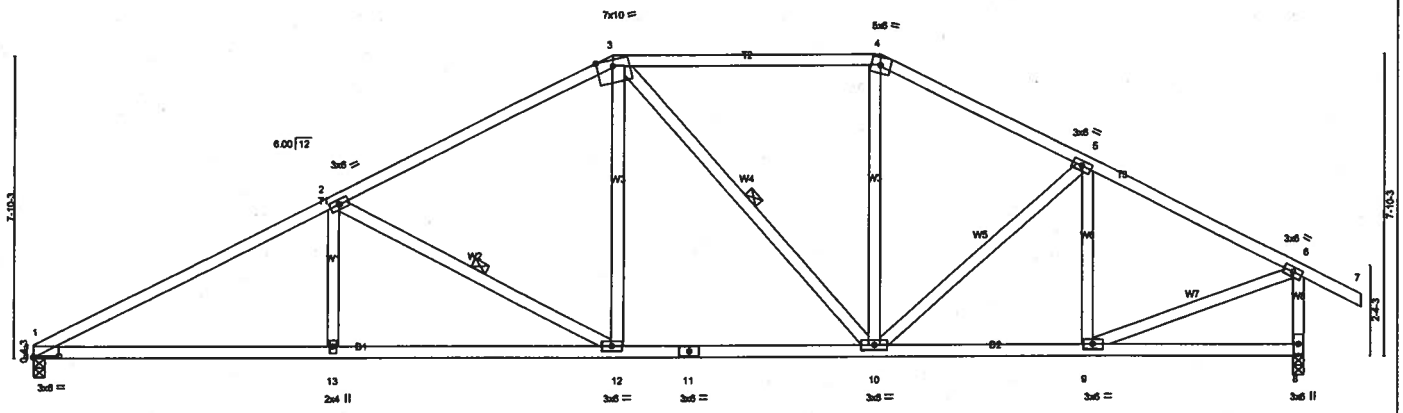
7-9-4	15-0-0	22-0-0	27-4-4	33-0-0	34-6-0
7-9-4	7-2-12	7-0-0	5-4-4	5-7-12	1-6-0

Scale = 1:57.2



7-9-4	15-0-0	22-0-0	27-4-4	33-0-0
7-9-4	7-2-12	7-0-0	5-4-4	5-7-12

Plate Offsets (X,Y): [1:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.32	1-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 187 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-5 oc bracing.
WEBS 1 Row at midpt 2-12, 3-10

REACTIONS (lb/size) 1=1372/0-3-8, 8=1465/0-3-8
Max Horz 1=181(load case 4)
Max Uplift1=-566(load case 5), 8=-640(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2511/1342, 2-3=-1777/1055, 3-4=-1356/950, 4-5=-1568/977, 5-6=-1539/883, 6-7=0/40, 6-8=-1378/897
BOT CHORD 1-13=-1025/2160, 12-13=-1025/2160, 11-12=-537/1518, 10-11=-537/1518, 9-10=-523/1314, 8-9=-18/77
WEBS 2-13=0/267, 2-12=-740/558, 3-12=-211/552, 3-10=-346/175, 4-10=-135/335, 5-10=-98/168, 5-9=-322/252, 6-9=-597/1325

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 1 and 640 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051627
L139896	T23	HIP	1	1	Job Reference (optional)

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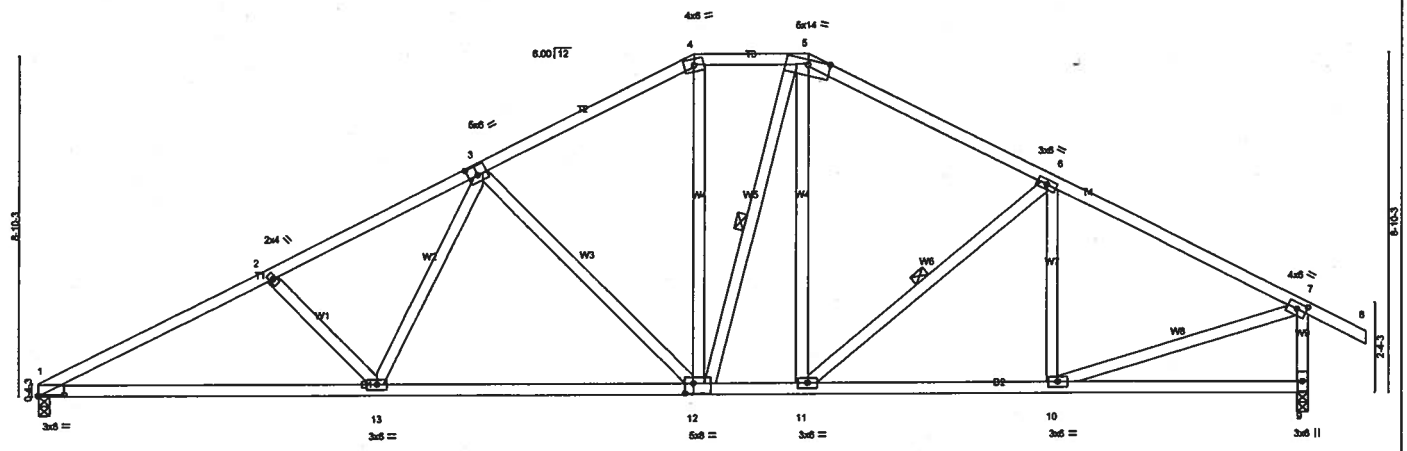
6-0-5	11-6-3	17-0-0	20-0-0	26-4-4	33-0-0	34-6-0
6-0-5	5-5-13	5-5-13	3-0-0	6-4-4	6-7-12	1-6-0

Scale = 1:57.2



8-9-4	17-0-0	20-0-0	26-4-4	33-0-0
8-9-4	8-2-12	3-0-0	6-4-4	6-7-12

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [3:0-3-0,0-3-0], [7:0-2-15,0-2-0], [12:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.34	1-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-9-10 oc bracing.
WEBS 1 Row at midpt 5-12, 6-11

REACTIONS (lb/size) 1=1372/0-3-8, 9=1465/0-3-8
Max Horz 1=198(load case 4)
Max Uplift1=-601(load case 5), 9=-657(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2520/1409, 2-3=-2309/1330, 3-4=-1560/1016, 4-5=-1332/978, 5-6=-1499/974, 6-7=-1620/934, 7-8=0/40, 7-9=-1368/908
BOT CHORD 1-13=-1111/2200, 12-13=-768/1768, 11-12=-389/1270, 10-11=-555/1379, 9-10=-13/102
WEBS 2-13=-291/352, 3-13=-236/551, 3-12=-634/513, 4-12=-259/452, 5-12=-151/328, 5-11=-130/267, 6-11=-241/236, 6-10=-213/233, 7-10=-598/1343

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 1 and 657 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T24	COMMON	2	1	Job Reference (optional)

Dwg.#115051628

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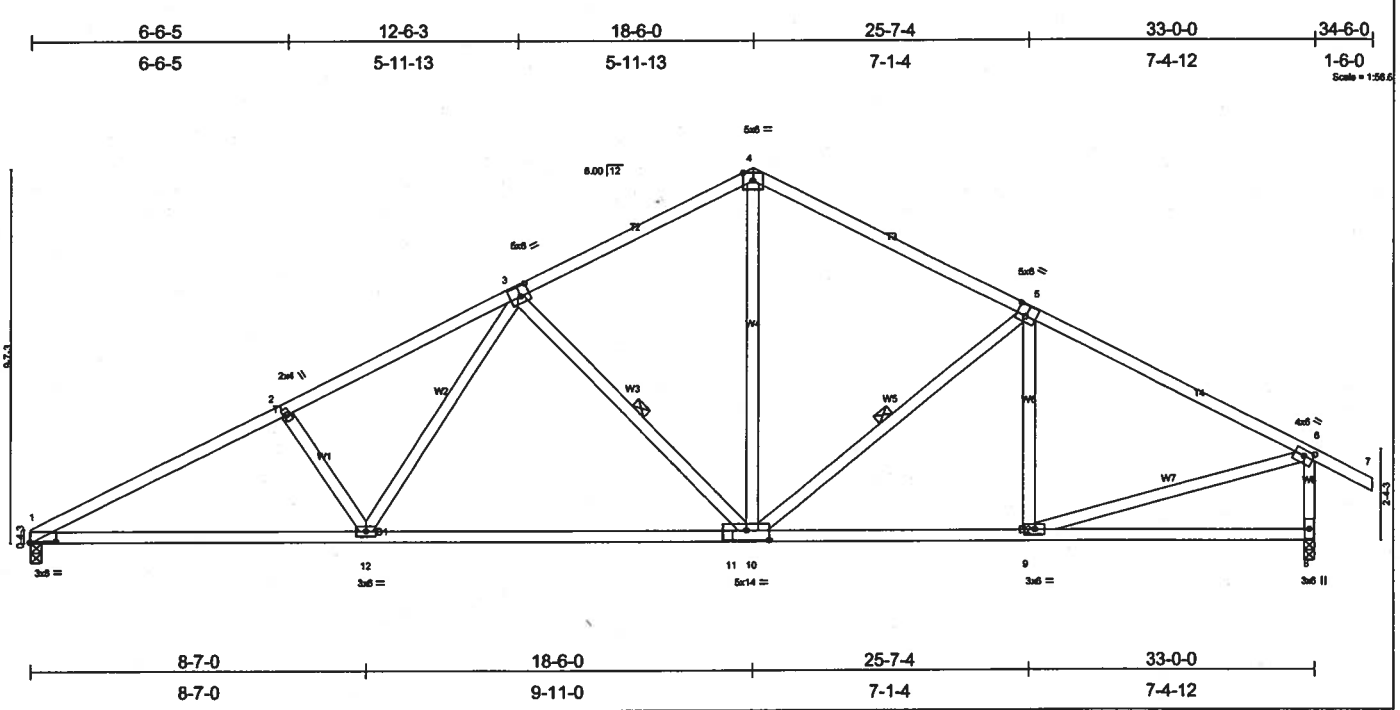

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [3:0-2-12,0-3-0], [5:0-3-0,0-3-4], [6:0-3-0,0-1-12], [11:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.26	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.44	10-12	>886	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-13 oc bracing.
WEBS 1 Row at midpt 3-10, 5-10

REACTIONS (lb/size) 1=1372/0-3-8, 8=1465/0-3-8
Max Horz 1=211(load case 5)
Max Uplift1=-585(load case 5), 8=-670(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2520/1398, 2-3=-2346/1379, 3-4=-1442/968, 4-5=-1462/960, 5-6=-1651/947, 6-7=0/40, 6-8=-1350/921
BOT CHORD 1-12=-1188/2195, 11-12=-807/1676, 10-11=-807/1676, 9-10=-648/1399, 8-9=-56/141
WEBS 2-12=-304/381, 3-12=-324/666, 3-10=-669/537, 4-10=-507/869, 5-10=-303/288, 5-9=-171/219, 6-9=-616/1309

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 585 lb uplift at joint 1 and 670 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T25	SPECIAL	1	1	Dwg.#1115051629

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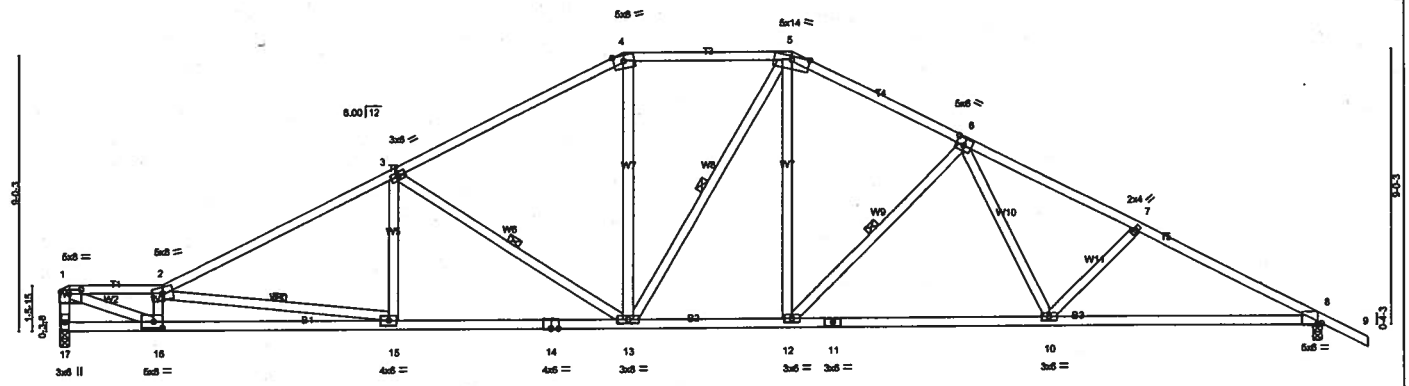
Job Reference (optional)

0-3-8	3-3-9	10-9-12	18-4-0	23-10-0	29-5-3	35-0-5	41-2-0	42-8-0
0-3-8	3-0-1	7-6-4	7-6-4	5-6-0	5-7-3	5-7-3	6-1-11	1-6-0

Scale = 1:72.0



3-3-9	10-9-12	18-4-0	23-10-0	32-2-12	41-2-0
3-3-9	7-6-4	7-6-4	5-6-0	8-4-12	8-11-4

Plate Offsets (X,Y): [1:0-4-8,0-1-12], [6:0-3-0,0-3-0], [8:0-1-11,Edge], [16:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.35	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.57	15-16	>866	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-5-11 oc bracing.
WEBS 1 Row at midpt 2-15, 3-13, 5-13, 6-12

REACTIONS (lb/size) 17=1715/0-3-8, 8=1807/0-3-8
Max Horz 17=-246(load case 6)
Max Uplift17=-560(load case 6), 8=-812(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4004/2182, 2-3=-3243/1755, 3-4=-2343/1393, 4-5=-2018/1343, 5-6=-2278/1388, 6-7=-3030/1682, 7-8=-3239/1756, 8-9=0/35, 1-17=-1616/867
BOT CHORD 16-17=-109/191, 15-16=-2155/4182, 14-15=-1314/2833, 13-14=-1314/2833, 12-13=-720/1986, 11-12=-1068/2417, 10-11=-1068/2417, 8-10=-1386/2833
WEBS 1-16=-2263/4124, 2-16=-1468/950, 2-15=-1366/851, 3-15=-99/488, 3-13=-983/682, 4-13=-288/656, 5-13=-156/242, 5-12=-325/654, 6-12=-633/506, 6-10=-206/544, 7-10=-278/334

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 560 lb uplift at joint 17 and 812 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051630
L139896	T26	SPECIAL	1	1	Job Reference (optional)

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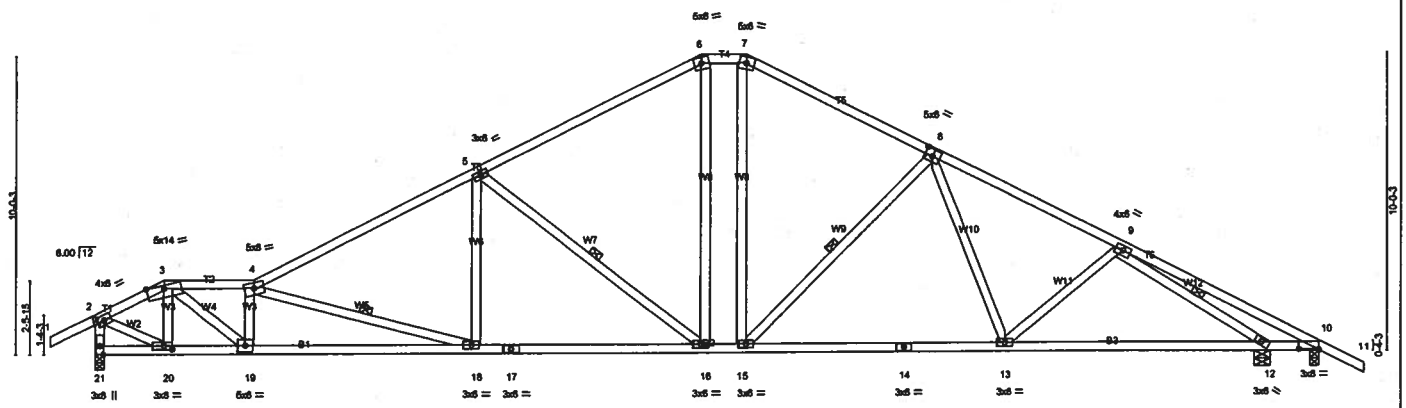
1-6-0	2-3-8	5-3-9	12-9-12	20-4-0	21-10-0	28-1-3	34-4-5	41-2-0	42-8-0
1-6-0	2-3-8	3-0-1	7-6-4	7-6-4	1-6-0	6-3-3	6-3-3	6-9-11	1-6-0

Scale = 1:74.0



2-3-8	5-3-9	12-9-12	20-4-0	21-10-0	30-6-6	39-2-12	41-2-0
2-3-8	3-0-1	7-6-4	7-6-4	1-6-0	8-8-6	8-8-6	1-11-4

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [8:0-3-0,0-3-0], [10:0-8-0,0-0-6], [20:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.31	16-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.49	16-18	>959	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.14	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-3 oc bracing.
WEBS 1 Row at midpt 4-18, 5-16, 8-15, 9-12

REACTIONS (lb/size) 21=1739/0-3-8, 12=1760/0-6-7, 10=113/0-3-8
Max Horz 21=-204(load case 6)
Max Uplift21=-814(load case 5), 12=-627(load case 6), 10=-269(load case 6)
Max Grav 21=1739(load case 1), 12=1760(load case 1), 10=118(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1763/976, 3-4=-3327/1878, 4-5=-2818/1590, 5-6=-1970/1251, 6-7=-1682/1217, 7-8=-1948/1262, 8-9=-2450/1437, 9-10=-242/163, 10-11=0/35, 2-21=-1682/1052
BOT CHORD 20-21=-139/209, 19-20=-688/1488, 18-19=-1733/3410, 17-18=-1103/2449, 16-17=-1103/2449, 15-16=-560/1682, 14-15=-877/2032, 13-14=-877/2032, 12-13=-1076/2109, 10-12=-1/138
WEBS 3-20=-687/349, 3-19=-1294/2318, 4-19=-1256/851, 4-18=-1001/656, 5-18=-149/547, 5-16=-1000/694, 6-16=-328/609, 7-15=-376/635, 8-15=-574/463, 8-13=-96/315, 9-13=-27/230, 9-12=-2382/1347, 2-20=-829/1643

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 814 lb uplift at joint 21, 627 lb uplift at joint 12 and 269 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK
L139896	T27	SPECIAL	1	1	Dwg.#115051631

Job Reference (optional)

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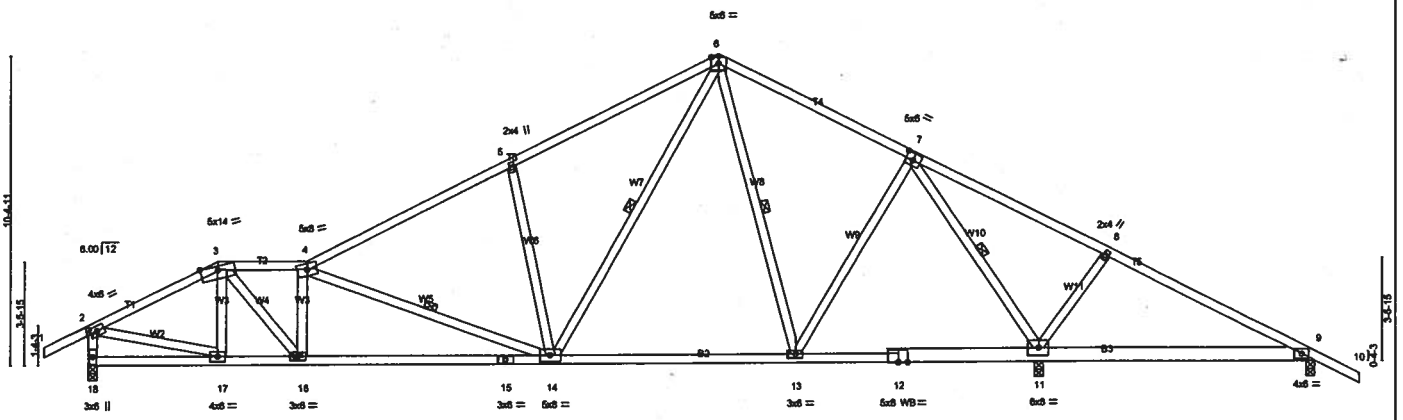
1-6-0	4-3-8	7-3-9	14-2-4	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	4-3-8	3-0-1	6-10-12	6-10-12	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



4-3-8	7-3-9	15-5-13	23-8-0	31-10-4	41-2-0
4-3-8	3-0-1	8-2-4	8-2-4	8-2-4	9-3-12

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.14	9-11	>789	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	0.12	9-11	>918	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1332/0-3-8, 11=2113/0-3-8, 9=166/0-3-8
Max Horz 18=-212(load case 6)
Max Uplift18=-651(load case 5), 11=-995(load case 6), 9=-303(load case 6)
Max Grav 18=1332(load case 1), 11=2113(load case 1), 9=268(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1610/869, 3-4=-2127/1211, 4-5=-1585/920, 5-6=-1513/1062, 6-7=-836/636, 7-8=-266/715, 8-9=-234/510, 9-10=0/39, 2-18=-1247/817
BOT CHORD 17-18=-166/215, 16-17=-637/1371, 15-16=-968/2157, 14-15=-968/2157, 13-14=-121/761, 12-13=0/374, 11-12=0/376, 9-11=-426/370
WEBS 3-17=-283/164, 3-16=-596/1133, 4-16=-848/493, 4-14=-865/557, 5-14=-377/449, 6-14=-684/1077, 6-13=-342/233, 7-13=-174/633, 7-11=-1771/987, 8-11=-349/436, 2-17=-613/1305

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 18, 995 lb uplift at joint 11 and 303 lb uplift at joint 9.

LOAD CASE(S) Standard

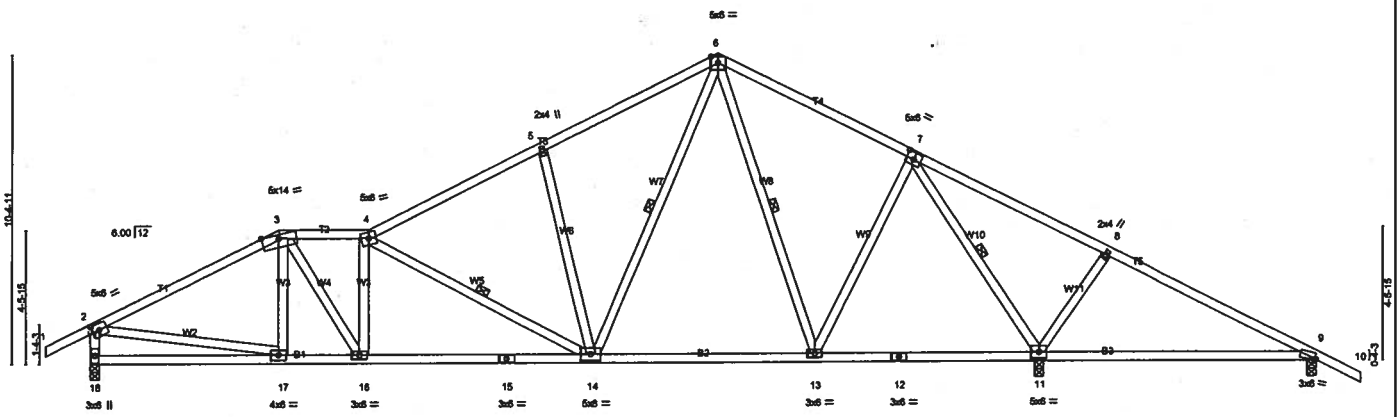
NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#115051632
L139896	T28	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Nov 14 13:22:56 2005 Page 1

1-6-0	6-3-8	9-3-9	15-2-4	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	6-3-8	3-0-1	5-10-12	5-10-12	6-6-3	6-6-3	7-0-11	1-6-0

Scale = 1:74.0



6-3-8	9-3-9	16-9-13	24-4-0	31-10-4	41-2-0
6-3-8	3-0-1	7-6-4	7-6-4	7-6-4	9-3-12

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.46	9-11	>241	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	0.39	9-11	>286	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 246 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1342/0-3-8, 11=2068/0-3-8, 9=202/0-3-8
Max Horz 18=-210(load case 6)
Max Uplift18=-658(load case 5), 11=-976(load case 6), 9=-318(load case 6)
Max Grav 18=1342(load case 1), 11=2068(load case 1), 9=286(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1737/937, 3-4=-1900/1121, 4-5=-1444/880, 5-6=-1372/987, 6-7=-827/642, 7-8=-189/637, 8-9=-165/435, 9-10=0/35, 2-18=-1232/830
BOT CHORD 17-18=-242/248, 16-17=-657/1475, 15-16=-818/1916, 14-15=-818/1916, 13-14=-131/779, 12-13=0/411, 11-12=0/411, 9-11=-342/287
WEBS 3-17=-117/121, 3-16=-378/754, 4-16=-458/351, 4-14=-779/502, 5-14=-322/376, 6-14=-625/1007, 6-13=-365/221, 7-13=-149/613, 7-11=-1730/919, 8-11=-376/470, 2-17=-546/1251

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 658 lb uplift at joint 18, 976 lb uplift at joint 11 and 318 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051633
L139896	T29	MONO TRUSS	1	1	Job Reference (optional)

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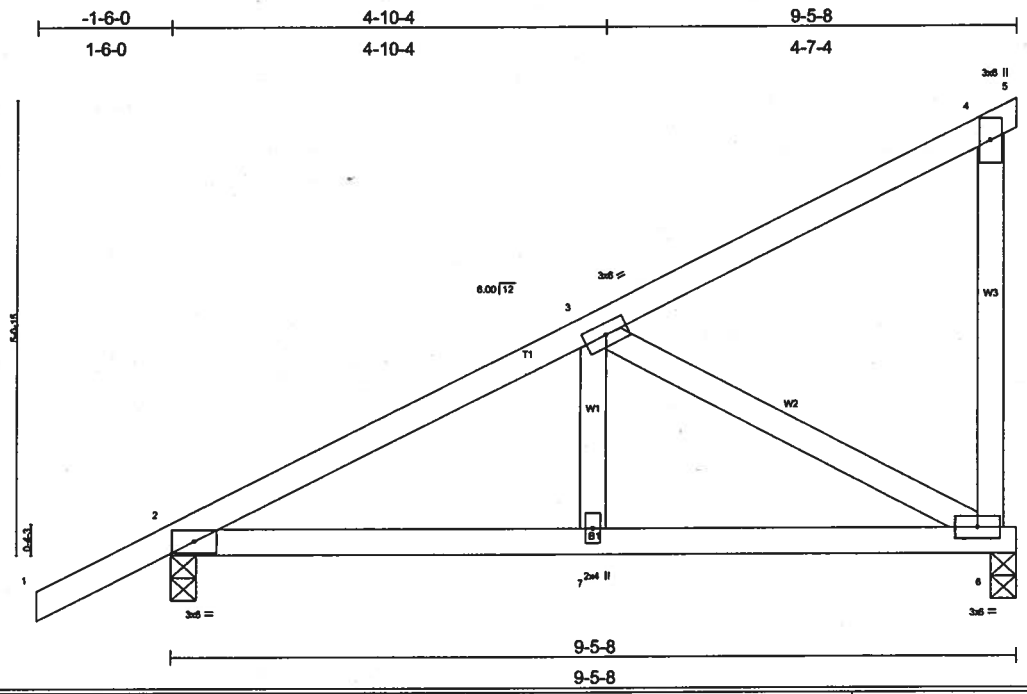

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	0.04	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.

REACTIONS (lb/size) 6=375/0-3-8, 2=476/0-3-8
Max Horz 2=315(load case 5)
Max Uplift6=-385(load case 5), 2=-374(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-512/540, 3-4=-81/27, 4-5=-2/0, 4-6=-104/147
BOT CHORD 2-7=-723/407, 6-7=-723/407
WEBS 3-6=-439/781, 3-7=-324/147

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 6 and 374 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 15,2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO-LOT 5 CANNON CREEK Dwg.#1115051634
L139896	T29G	MONO TRUSS	1	1	Job Reference (optional)

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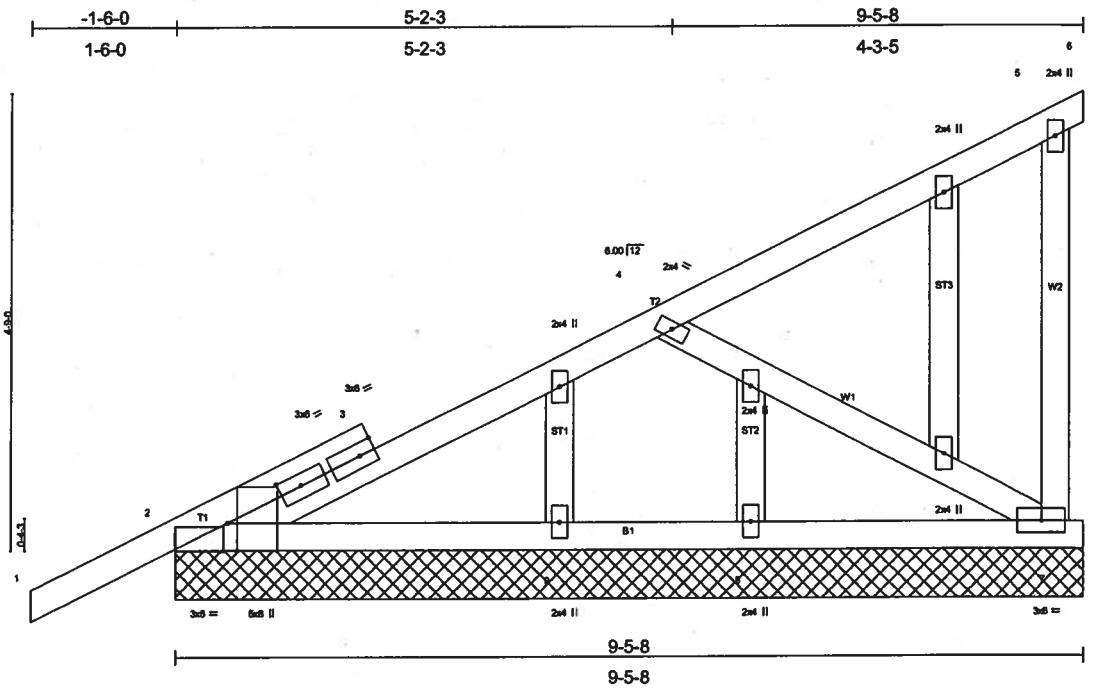

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	0.03	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.18	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-2 oc bracing.

REACTIONS (lb/size) 2=522/9-5-8, 6=-161/9-5-8, 7=582/9-5-8, 9=147/9-5-8, 8=52/9-5-8
Max Horz 2=298(load case 5)
Max Uplift2=-313(load case 5), 6=-161(load case 1), 7=-486(load case 5), 9=-12(load case 5)
Max Grav 2=522(load case 1), 6=161(load case 5), 7=582(load case 1), 9=147(load case 1), 8=52(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6/51, 2-3=-506/240, 3-4=-450/238, 4-5=-92/54, 5-6=-82/100, 5-7=-323/433
BOT CHORD 2-9=-462/402, 8-9=-462/402, 7-8=-462/402
WEBS 4-7=-447/515

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable requires continuous bottom chord bearing.
4) Gable studs spaced at 2-0-0 oc.
5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 161 lb uplift at joint 6, 486 lb uplift at joint 7 and 12 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-79(F=-25), 5-6=-79(F=-25), 2-7=-30

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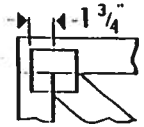
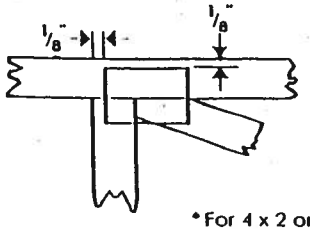
Symbols

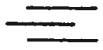
PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

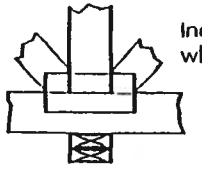
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

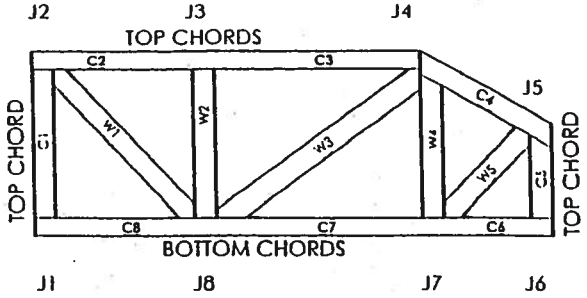
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

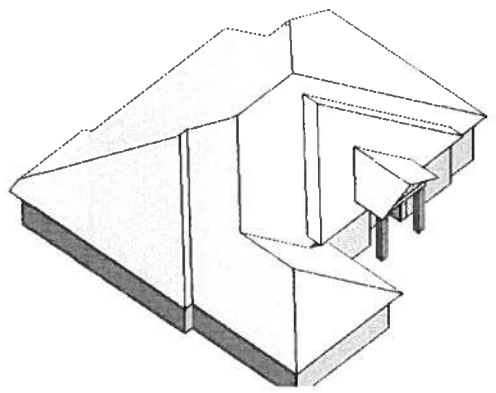
BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

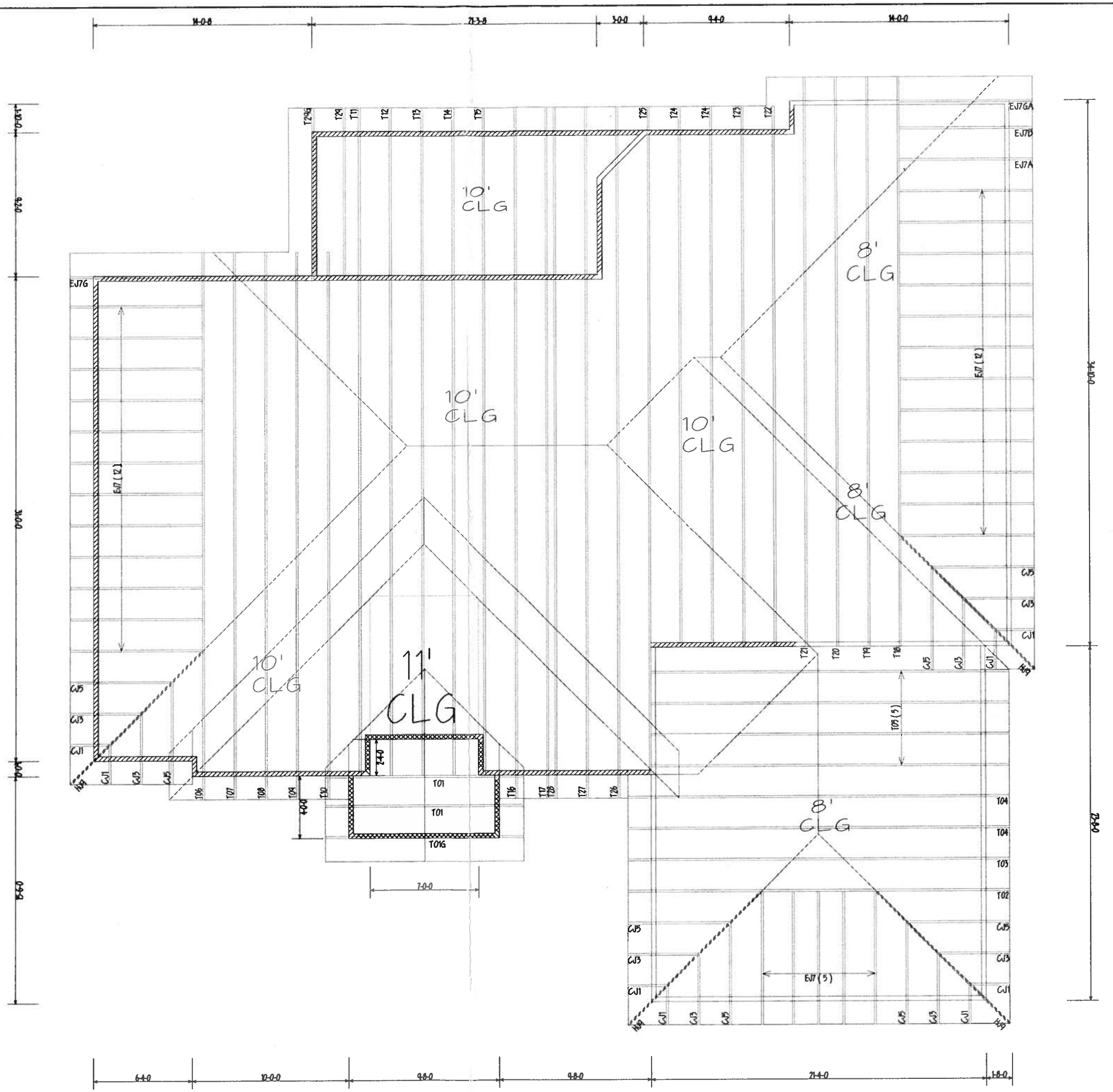
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.





BEARING HEIGHT SCHEDUL

8'-0"

10'-0"

11'-0"

1'4" OH
6/12 PITCH

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY B REFER TO ENGINEERED DRAWINGS FOR PERM BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTE

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROV

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATI TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL G TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAY BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VE CONDITIONS TO INSURE AGAINST CHANGES THAT WILL IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____

Builder FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-43

Jacksonville
PHONE: 904-772-6100 FAX: 904-77

Lake City
PHONE: 904-755-6894 FAX: 904-75

Sanford
PHONE: 407-322-0059 FAX: 407-32

BUILDER: HUGO ESCALANTE		
LEGAL ADDRESS: LOT 5 CANNON CREEK		
MODEL: NATHAN		REVISION: SCALE:
DATE: 11-14-05	DRAWN BY: JRD	JOB #: L13