ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

SIMPSON H2.5A U.N.O. -

SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' WALL HEIGHT

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
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		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
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GLB 24F-V3 SP		2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL MICROLAM		1600	1.9
PSL	PARALAM	2900	2.0

GENERAL NOTES:

PRE ENGINEERED ROOF TRUSS -

DOUBLE 2x4 SPF TOP PLATE NAILED -

TOGETHER W/2-16d NAILS AT 16" O.C

4' MIN. LAP w/ (12) - 16d OR 4" LAP w/

CS20 w/ (4) - 16d &(14) - 10d

SPECIFIED ON FLOOR PLAN

ALL STUDS TO BE 2x4 -

SPF NAILED TO TOP

WITH 2-16d NAILS

AND BOTTOM PLATES

CONTINUOUS FRAME TO

CEILING DIAPHRAGM DETAIL

INTERIOR CEILING AS -

CONTINUOUS FRAME -

TO TOP PLATE AT BOTTOM CHORD OF TRUSS

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS. AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET AVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: $6" \times 6" \text{ W}1.4 \times \text{W}1.4$, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO DWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

ROOF SYSTEM DESIGN

TRUSS SHEETS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 200, SECTION

1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN

TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS

THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE

COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS

MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED

LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO

SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL

RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE

REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF

BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT

TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES

RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

	R ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE FOR THE WIND LOAD ENGINEER'S SCOPE OF WORK.
그리고 하다 하고 있는데 없는 이 없는 사람들이 되었다면 하는데 하는데 하는데 하는데 바다를 하는데 없다.	OUNDATION BEARING CAPACITY, GRADE AND D AND DEBRIS ZONE, AND FLOOD ZONE.
	NSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 TED WIND VELOCITY AND DESIGN PRESSURES.
	D PATH FROM TRUSSES TO FOUNDATION. IF YOU DISTINUOUS LOAD PATH CONNECTION, CALL MEDIATELY.
DESIGN, PLACEMENT PLANS, T	TURER'S SEALED ENGINEERING INCLUDES TRUSS 'EMPORARY AND PERMANENT BRACING DETAILS, NS, AND UPLIFT AND REACTION LOADS FOR ALL

ANCHOR TABLE

MANUFACTURER'S ENGINEERING

< 420

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR*

< 245

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 6485

< 9035

< 9250

< 435

< 420

< 825

< 600

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

3-10d

1 -10d

6-10d

2-10d

14-10d

16-10d

18-8d

28-8d

TO STUDS

8-16d

18-10d, 1 1/2"

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

10-10d, 1 1/2" 2-10d, 1 1/2"

10-10d, 1 1/2" 2-10d, 1 1/2"

H2.5

H2.5A

H14-1

H14-2

H10-1

H10-2

MTS24C

HTS24

2 - HTS24

LGT2

HEAVY GIRDER TIEDOWNS

MGT

HGT-3

HGT-4

STUD STRAP CONNECTOR

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SPH4

SPH6

LSTA18

LSTA21

CS16

STUD ANCHORS

LTTI31

HD2A

PAHD42

HPAHD22

ABU66

3-8d

4-8d

4-8d

4-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2"

8-8d, 1 1/2°

6-10d

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

22 -10d

16 -10d

16 -10d

16 -10d

TO STUDS

TO FOUNDATION

-5/8" THREADED ROI

12" EMBEDMENT

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

12" EMBEDMENT

TO STUDS

4 -10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

-5/8" THREADED ROD

WIND LOADS PER	R FLORIDA BUILDING CODE 2004, SECTION 1609
ON UPPER HALF	PLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; BHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% BSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.
BUILDING IS NOT	IN THE HIGH VELOCITY HURRICANE ZONE
DI III DINIG 10 110-	

3.) WIND IMPORTANCE FACTOR = 1.0

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING, 1609)

Zone	Effective Wind Area (ft2)			
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
	& Wind st Cas 5, 10	е	21.8	-29.1
8x7 Garage Door			19.5	-22.9
16x7 Garage Door		18.5	-21.0	

DESIGN LOADS FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

DESIGN DATA

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (FBC TABLE 1609 B&C)

1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Wors (Zone	st Cas	е	21.8	-29.1
8x7 Gara	age D	oor	19.5	-22.9
16x7 Garage Door			18.5	-21.0

Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

October 19, 2005 DRAWN BY: CHECKED BY:

510062 DRAWING NUMBER

Butler Builders

Mockingbird Model ADDRESS: Calvery Place Mason City, FL

NDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

Do not proceed without clarification COPYRIGHTS AND PROPERTY RIGHTS:

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its common law copyrights and property right in these instruments of service. This document is

not to be reproduced, altered or copied in any form or manner without first the express writte

ermission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have

amined this plan, and that the applicable

ortions of the plan, relating to wind enginee

comply with section 1609, florida building coo 2004, to the best of my knowledge.

.IMITATION: This design is valid for one

P.E. 53915

uilding, at specified location.

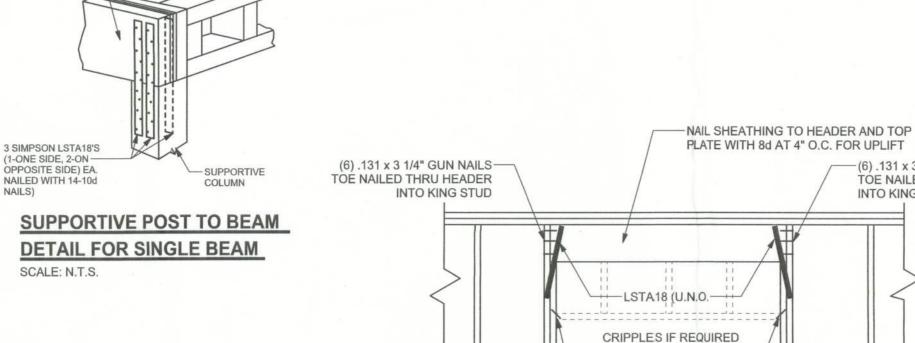
32056, 386-754-5419

(Columbia County) Mark Disosway P.E. P.O. Box 868

PRINTED DATE:

FINALS DATE: 19 / Oct / 05 JOB NUMBER:

> S-1 OF 3 SHEETS



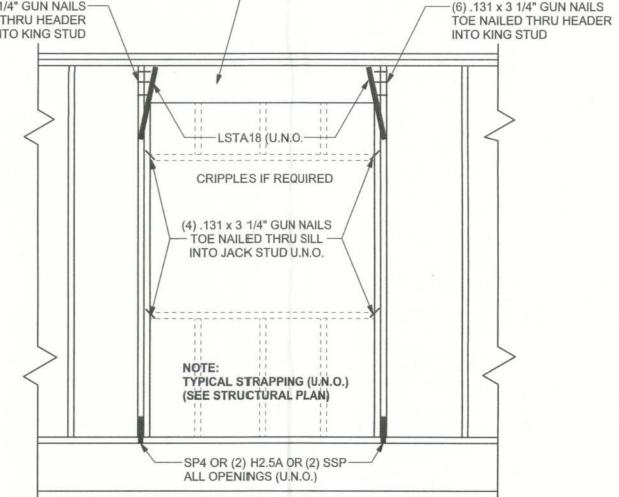
SUPPORTIVE BEAM ----IF BEAM JOINT IS AT-INSTALL ONE SIMPSON LSTA18 ON ONE SIDE 4-SIMPSON LSTA18 ---(2-ONE SIDE, 2-ON OTHER SIDE)

SUPPORTIVE -

BEAMS

SUPPORTIVE CENTER POST TO BEAM DETAIL

2X4 LADDER BEAM



(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10"-0" WALL HIGHT U.N.O.) TYPICAL HEADER STRAPING DETAIL

MASONRY NOTES:

ACI530.1-02 Section

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

Specific Requirements

1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

TYPICAL PORCH POST DETAIL

(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

ANCHOR BOLT

(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN

SIMPSON HUS412 MIN. -

SEE STRUCTURAL PLAN

(4)-2x4 SPF #2 NAILED

BEAM MID-WALL CONNECTION DETAIL

LSTA18

EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

SEE STRUCTURAL PLAN

SCALE: N.T.S.

MIN. (SEE STRUCTURAL PLAN)

SEE STRUCTURAL PLAN