ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V3P487-Z0207171903

Truss Fabricator	Anderson Truss Company
Job Identification	14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL
Truss Count	21
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

02/07/2014

Notes

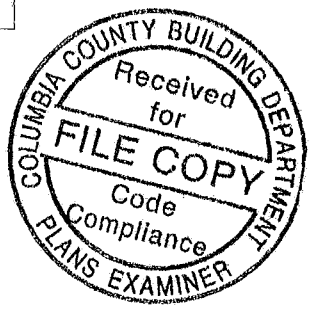
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	73501	A 27'3"8 Common	14038001	02/07/14
2	73502	A1 27'3"8 Common	14038002	02/07/14
3	73503	A2 27'3"8 Common	14038003	02/07/14
4	73504	AGE 27'3"8 Gable	14038004	02/07/14
5	73505	B1 30' Common	14038005	02/07/14
6	73506	B2 30' Common	14038006	02/07/14
7	73507	B3 37' Common	14038007	02/07/14
8	73508	B4 36'10"8 Comm	14038008	02/07/14
9	73509	B5 36'10"8 Comm	14038009	02/07/14
10	73510	BGE 30' Gable	14038010	02/07/14
11	73511	BGE1 37' Common	14038011	02/07/14
12	73512	BGE2 36'10"8 Gab	14038012	02/07/14
13	73513	C 14'4" Mono	14038013	02/07/14
14	73514	CJ3 3' Jack	14038014	02/07/14
15	73515	CJ5 5' Jack	14038015	02/07/14
16	73516	EJ7 7' End Jack	14038016	02/07/14
17	73517	H11 14'4" Mono H	14038017	02/07/14
18	73518	H13 14'4" Mono H	14038018	02/07/14
19	73519	H7 14'4" Mono Hi	14038019	02/07/14
20	73520	H9 14'4" Mono Hi	14038020	02/07/14
21	73521	HJ7 9'10"13 Hip	14038021	02/07/14



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - A 27 3 8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

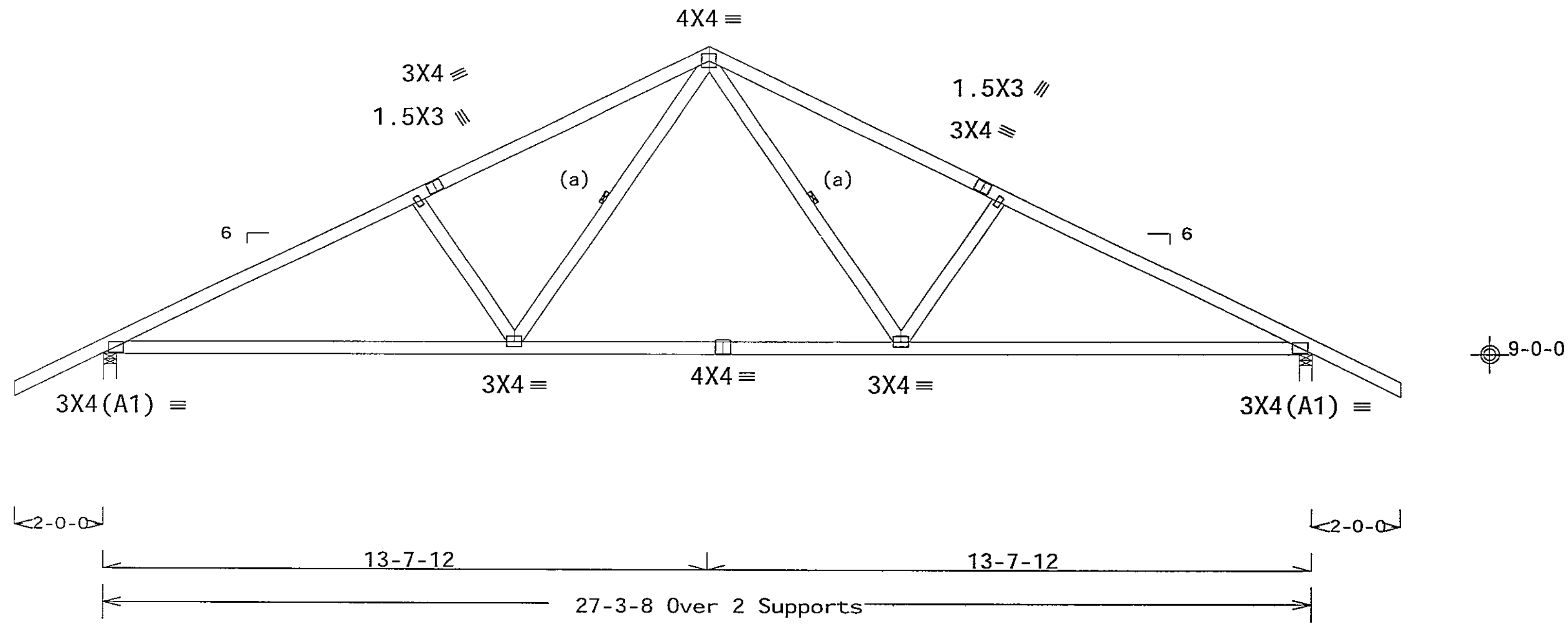
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=1227 U=51 W=3 5'
RL=129/-129

R=1228 U=51 W=3 5"

PLT TYP Wave

Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

12 03 04 0326 14 QTY:13 FL/-/3/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating handling shipping installing and bracing Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTCA) for safety practices prior to performing these functions Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping installation & bracing of trusses Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job's general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbcindustry com ICC www iccsafe org

WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73501
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038001
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37 0 PSF	SEQN-	330983
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - A1 27 3 8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

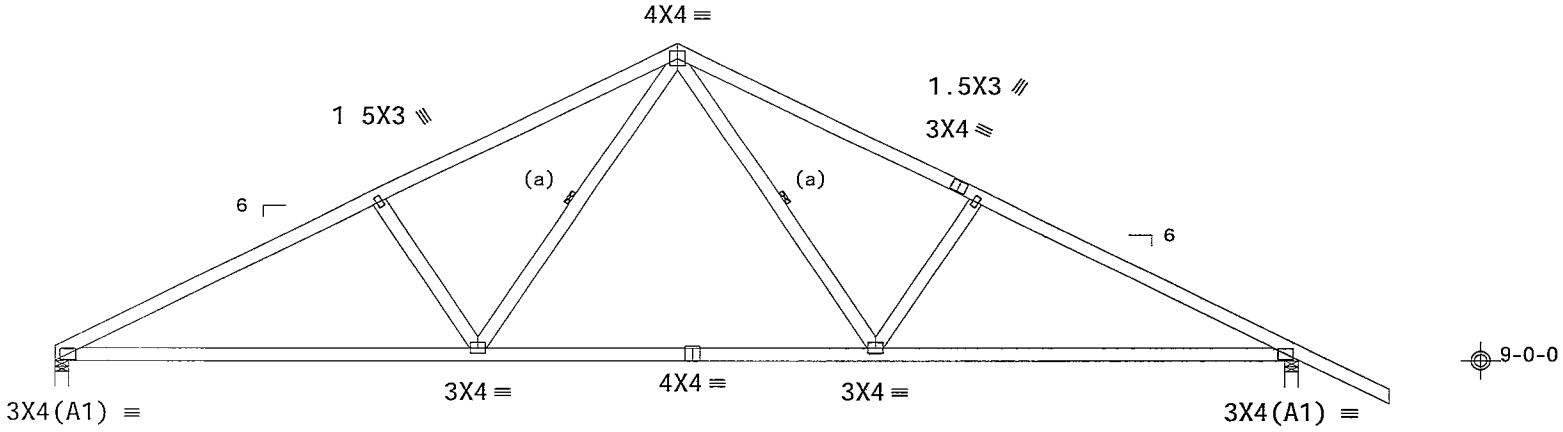
MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.14 | QTY:8 | FL/-/3/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handling sh pp ng nstal ng and brac ng Refer to follow the latest edit on of BCSI (Building Component Safety Informat on by TPI and WTCA) for pract ces prior to performing these funct ons Installers shall prov de temporary brac ng per Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ceiling Locations shown for permanent lateral restraint of webs shall have bracing nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev at on fr any fa lure to build the truss n conformance w th ANSI/TPI 1 or for handl ng sh pp ng nstallation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard p ate pos t ons A seal on th s drawing or cover page l sting this drawing nd cates acceptance of profess ona engineering respons bility solely for the design shown The suitabil ty and use of th s des gn for any s the responsibility of the Build ng Des gner per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF R9114- 73502
TC DL	7.0 PSF	DATE 02/07/14
BC DL	10.0 PSF	DRW HCUSR9114 14038002
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 331009
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1V3P487_Z02

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - A2 27 3'8 Common Girder)

Top chord 2x4 SP M-30
Bot chord 2x6 SP M-26
Webs 2x4 SP #3-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 56 plf at 0 00 to 56 plf at 13 65
TC- From 56 plf at 13 65 to 56 plf at 19 91
TC- From 56 plf at 19 91 to 56 plf at 29 29
BC- From 10 plf at 0 00 to 10 plf at 14 00
BC- From 10 plf at 14 00 to 10 plf at 27 29
BC- From 4 plf at 27 29 to 4 plf at 29 29
BC- 1149 85 lb Conc Load at 1 23
BC- 1017 49 lb Conc Load at 3 23, 5 23, 7 23 9 23
11 23 13 23,15 23,17 23,19 23,21 23,23 23,25 23

MWFRS loads based on trusses located at least 30 00 ft from roof edge

2 COMPLETE TRUSSES REQUIRED

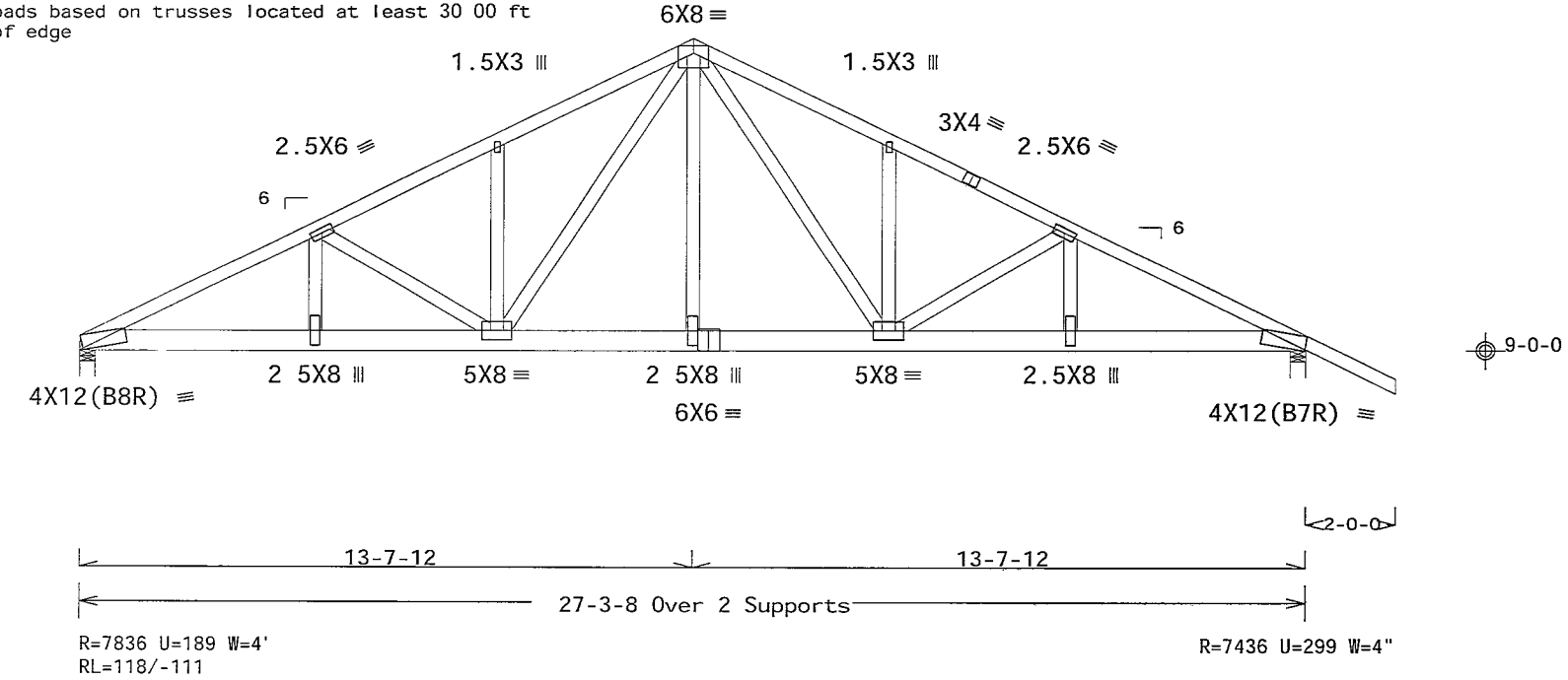
Nail Schedule 0 131"x3 , min nails
Top Chord 1 Row @12 00' o c
Bot Chord 1 Row @ 3 25' o c
Webs 1 Row @ 4" o c
Use equal spacing between rows and stagger nails in each row to avoid splitting

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)
PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03-04-0326.13 QTY:1 FL/-/3/-/-/R/- Scale =.25"/Ft.

ALPINE
TTW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handling sh pp ng nstall ng and brac ng Ref r to BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for safety practices pr or to performing these funct ons Installers shall provide temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ce l ng Locations shown for permanent lateral restraint of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73503
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038003
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349529
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3P487_Z02

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - AGE 27 3 8 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0 OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24' o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

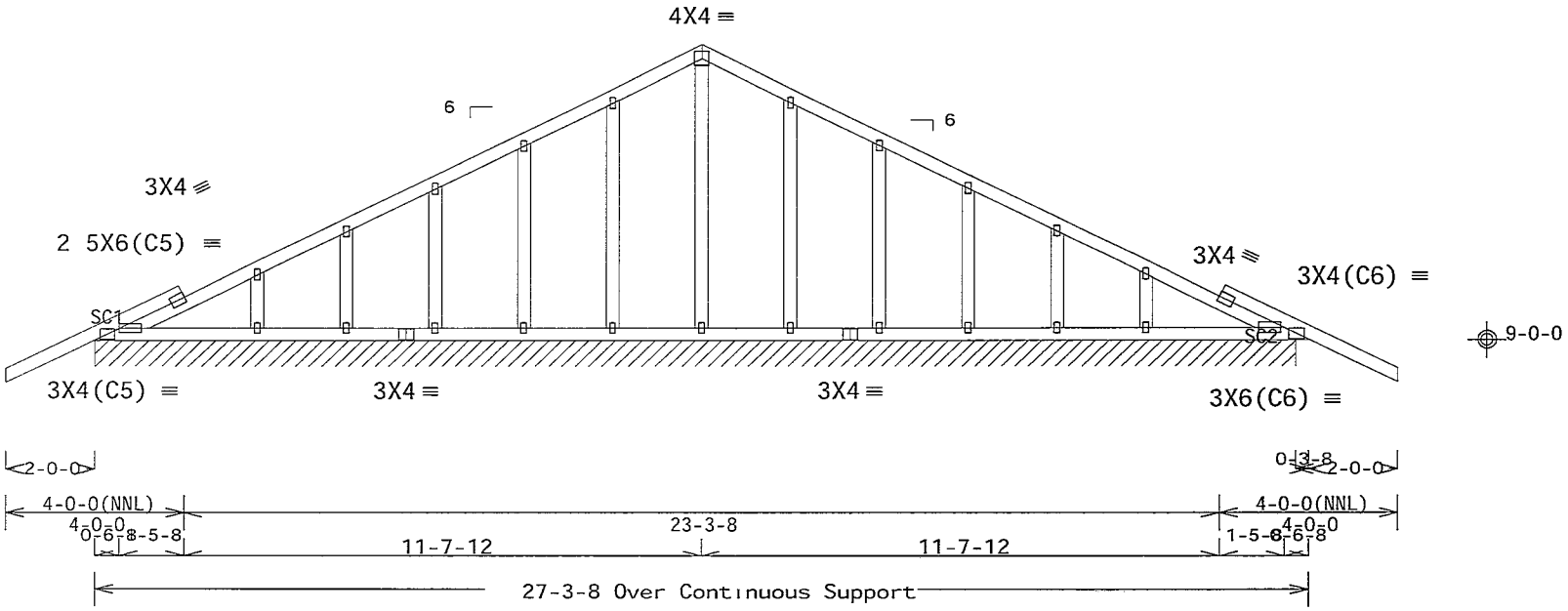
Right cantilever is exposed to wind

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=178 PLF U=19 PLF W=27-0-0
RL=10/-10 PLF

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12.03.04.0826.14 QTY:1 FL/-/3/-/-/R/- Scale =.25"/Ft.

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Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20 0 PSF	REF	R9114- 73504
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038004
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330978
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - B1 30 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

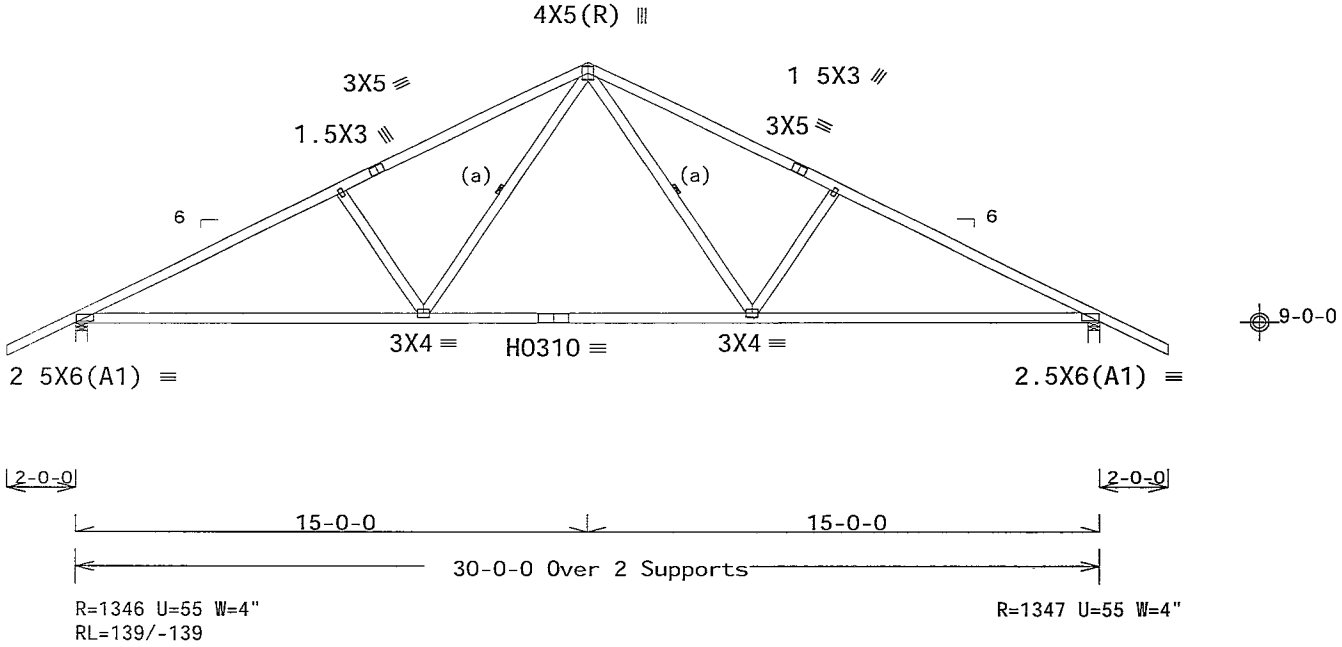
Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.14 | QTY:7 | FL/-/3/-/-/R/- | Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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02/07/2014

TC LL	20.0 PSF	REF	R9114- 73505
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038005
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330985
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - B2 30 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP M-30
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

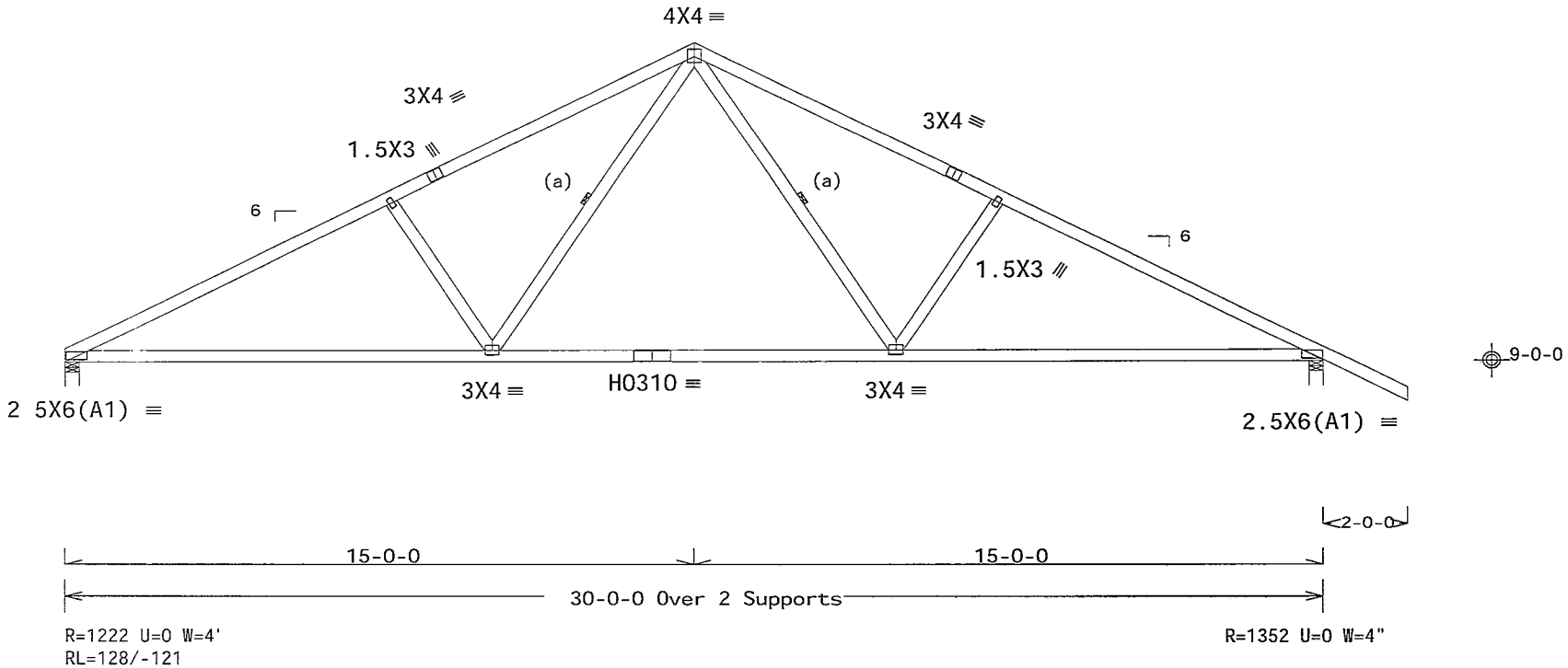
MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 12.03.04.0326.14 QTY:2 FL/-/3/-/-/R/- Scale =.25"/Ft.

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73506
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37 0 PSF	SEQN-	331011
DUR.FAC	1.25		
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - B3 37 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

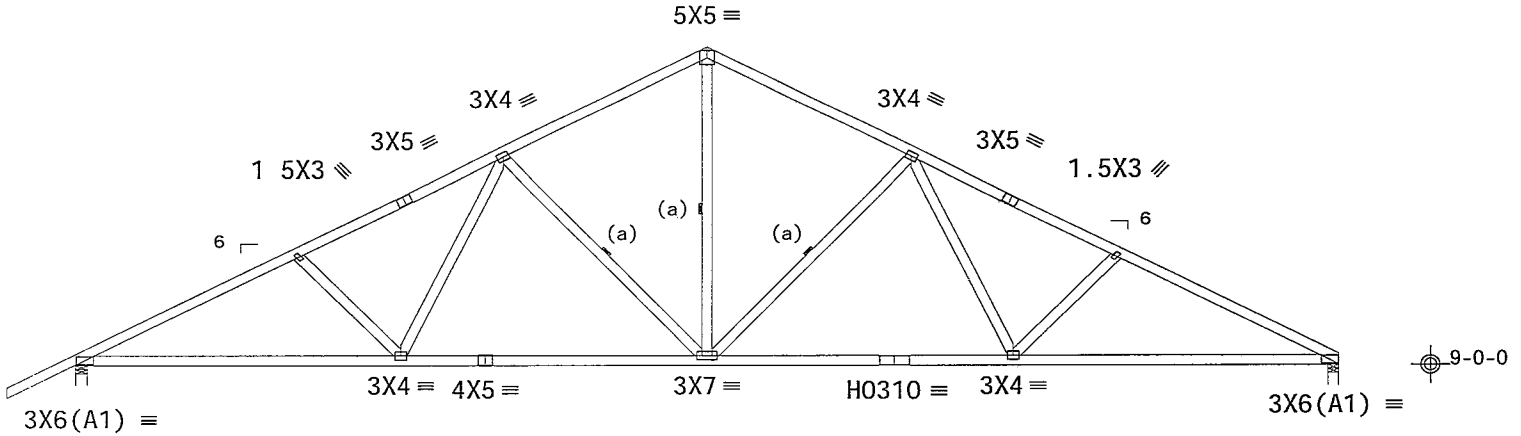
MWFRS loads based on trusses located at least 15 00 ft from roof edge

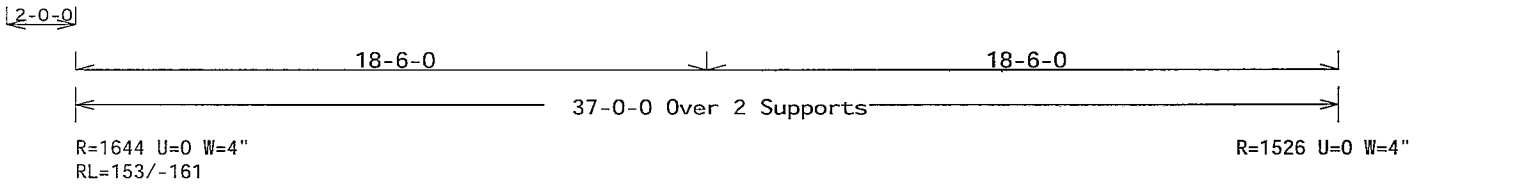
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP 20 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 12.03-04-0326.13 | QTY:15 FL/-/3/-/-/R/- | Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabr cat ng handl ng shipp ng nstall ng and brac ng Refer to and follow the latest edit on of BCSI (Bu ld ng Component Safety Informat on by TP and WTCA) for safety pract ces pr or to perform ng these funct ons Installers shall prov de temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chords shall have a properly attached r g d co l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from the des gn any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for hand ng sh pp ng nstall ng & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard p ate pos t ons A seal on this draw ng or cover page l sting th s draw ng nd cates acceptance of profess onal eng neer ng respons b l ty solely for the des gn shown The su tab l ty and use of th s des gn for any s ng e the respons b l ty of the Bu ld ng Des gner per ANSI/TPI 1 Sec 2 For more informat on see general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73507
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038007
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	349481
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - B4 36 10 8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP M-26 supporting member
(14) 0 148 x3 nails into supporting member,
(4) 0 148 x3 nails into supported member

MWFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf. GCpi(+/-)=0 18

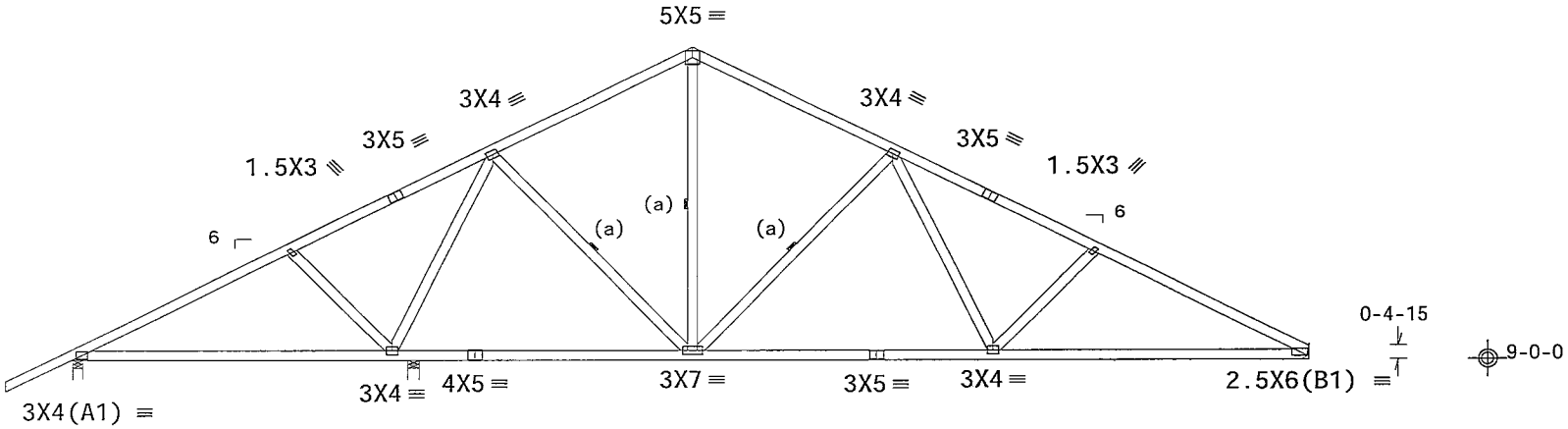
Wind loads and reactions based on MWFRS with additional C&C member design

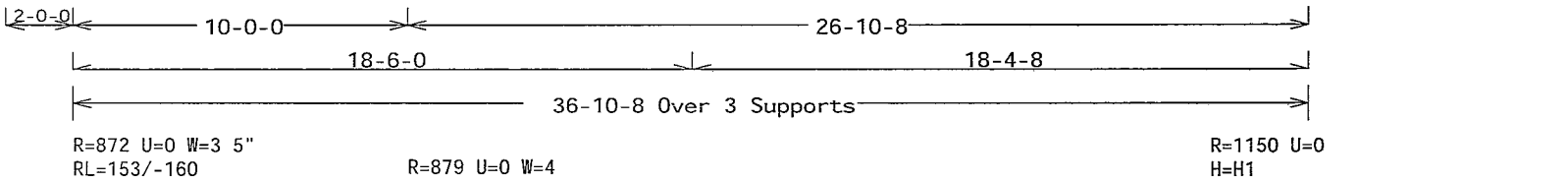
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=872 U=0 W=3 5"
RL=153/-160

R=879 U=0 W=4

R=1150 U=0
H=H1

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 03 04 0326.13 | QTY:1 | FL/-/3/-/-/R/- | Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons b e for any dev at on from th s design any fa ure to build the truss n conformance w th ANSI/TPI 1 or for hand ng sh pp ng nstallation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to draw ngs 160A Z for standard p ate pos t ons A local off c al draw ng or cover page list ng this draw ng nd cates acceptance of profess ona eng neer ng respons bility solely for the design shown The su tab l ty and use of th s des gn for any s ructure is the responsibility of the Building Des gner per ANSI/TPI 1 Sec 2 For more nformat on see general notes page ITW-BCG www twbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www ccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20 0 PSF	REF R9114- 73508
TC DL	7.0 PSF	DATE 02/07/14
BC DL	10.0 PSF	DRW HCUSR9114 14038008
BC LL	0 0 PSF	HC-ENG WHK/WHK
TOT LD	37.0 PSF	SEQN- 349484
DUR.FAC.	1.25	FROM JMW
SPACING	24.0"	JREF- 1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - B5 36'10 8 Common)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B B1 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

H = recommended connection based on manufacturer tested capacities and calculations Conditions may exist that require different connections than indicated Refer to manufacturer publication for additional information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (2)2x6 SP M-26 supporting member
(14) 0 148 x3 nails into supporting member,
(4) 0 148 x3 nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

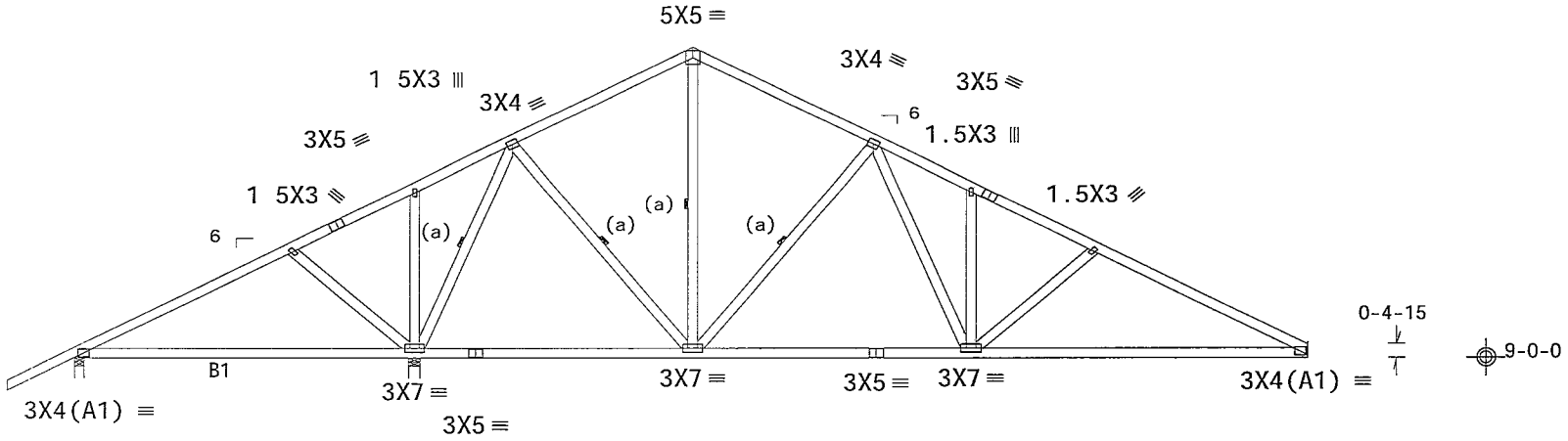
Wind loads and reactions based on MWFRS with additional C&C member design

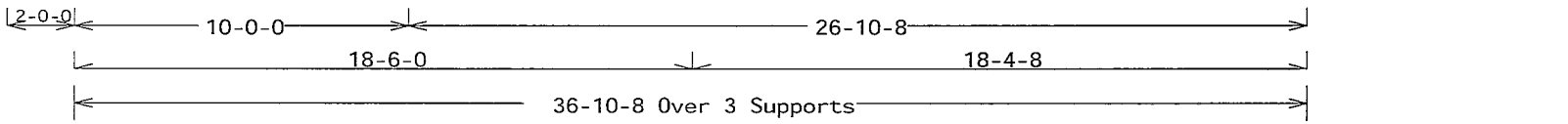
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage

(a) Continuous lateral restraint equally spaced on member

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50





R=388 U=11 W=3 5'
RL=153/-160

R=1857 U=74 W=4

R=1017 U=25
H=H1

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 03 04 0326.13 | QTY:12 FL/-/3/-/-/R/- | Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handl ng sh pp ng nstal ng and brac ng Ref r to and follow the latest ed t on of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for pract ces pr or to performing these funct ons Installers shall provide temporary bracing per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs sha have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from th s des gn any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for hand ng sh pp ng nstallat on & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard p ate pos t ons Appeal of th draw ng or cover page list ng th s drawing nd cates acceptance of profess ona eng neer ng respons b l ty solely for the des gn shown The suitabil ty and use of th s des gn for any s ng e bu ld ng the respons b l ity of the Build ng Des gner per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW-BCG www twbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73509
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	349137
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - BGE 30 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with '13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212 GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

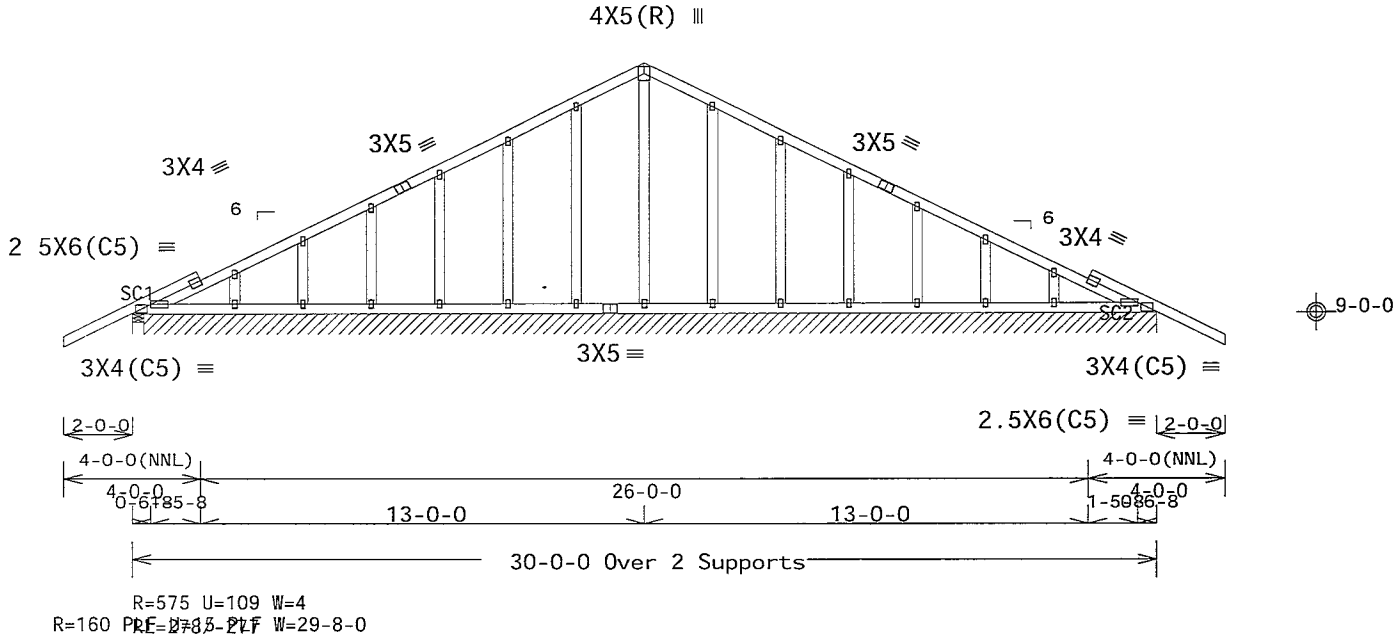
Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0' OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=575 U=109 W=4
R=160 PRF=278.5-PLF W=29-8-0

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0326.14 QTY:1 FL/-/3/-/-/R/- Scale = 1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73510
TC DL	7 0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	330970
DUR.FAC.	1 25		
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - BGE1 37 Common)

Top chord 2x4 SP 2850f-2 3E T2 2x4 SP M-30
Bot chord 2x4 SP 2850f-2 3E
Webs 2x4 SP #3-13B W6 2x4 SP #2-13B
Stack Chord SC1 2x4 SP M-30 Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 344 Rt Wedge 2x4 SP #3-13B

Lumber grades designated with '13B use design values approved 1/30/2013 by ALSC

Truss spaced at 24 0 OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24 o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS

(**) 1 plate(s) require special positioning Refer to scaled plate plot details for special positioning requirements

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

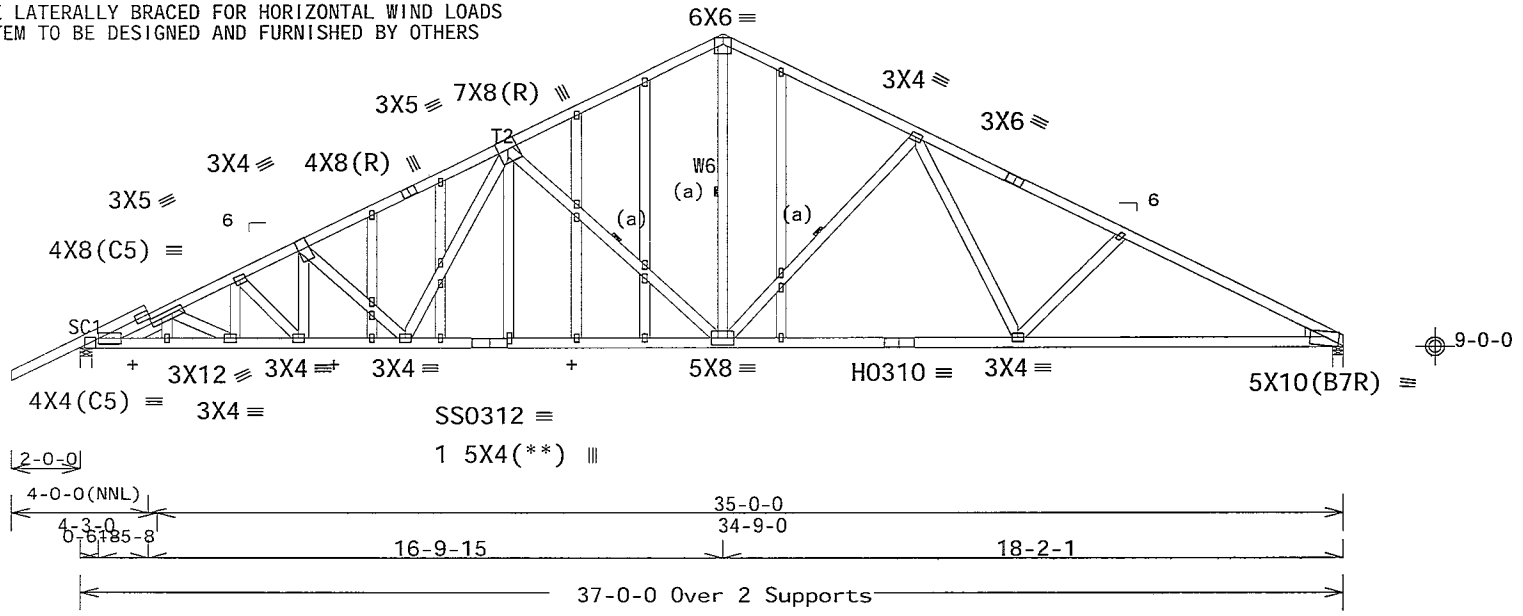
Wind loads and reactions based on MWFRS with additional C&C member design

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

(a) Continuous lateral restraint equally spaced on member

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note All Plates Are 1 5X3 Except As Shown
PLT TYP 20 Gauge HS 18 Gauge HS, Design Crit FBC2010Res/TPI-2007(STD)
Wave FT/RT=10%(0%)/0(0) 12 03 04 08 26.13 QTY.1 FL/-/3/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20 0 PSF	REF	R9114- 73511
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038011
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37 0 PSF	SEQN-	349476
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - BGE2 36 10 8 Gable)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24 o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

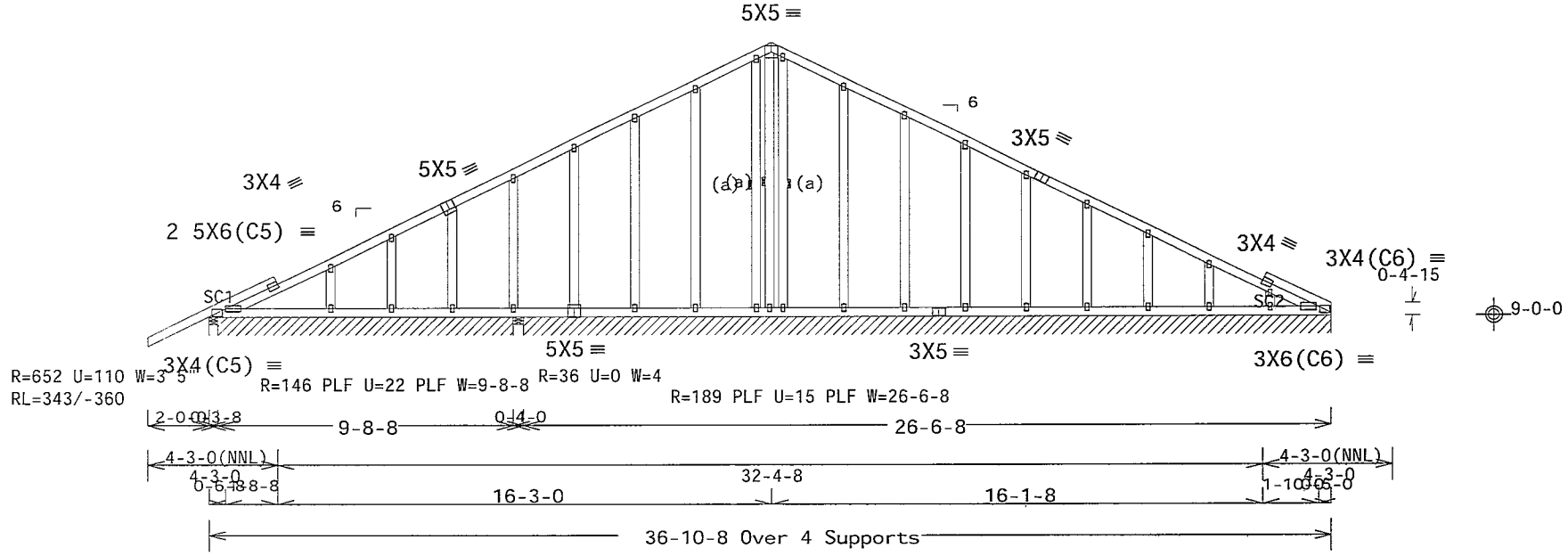
Truss spaced at 24 0" OC designed to support 2-3-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

(a) Continuous lateral restraint equally spaced on member

In lieu of structural panels use purlins to brace TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03-04-0026.13 QTY:1 FL/-/3/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73512
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349142
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - C 14 4 Mono)

Top chord 2x4 SP 2850f-2 3E T2 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

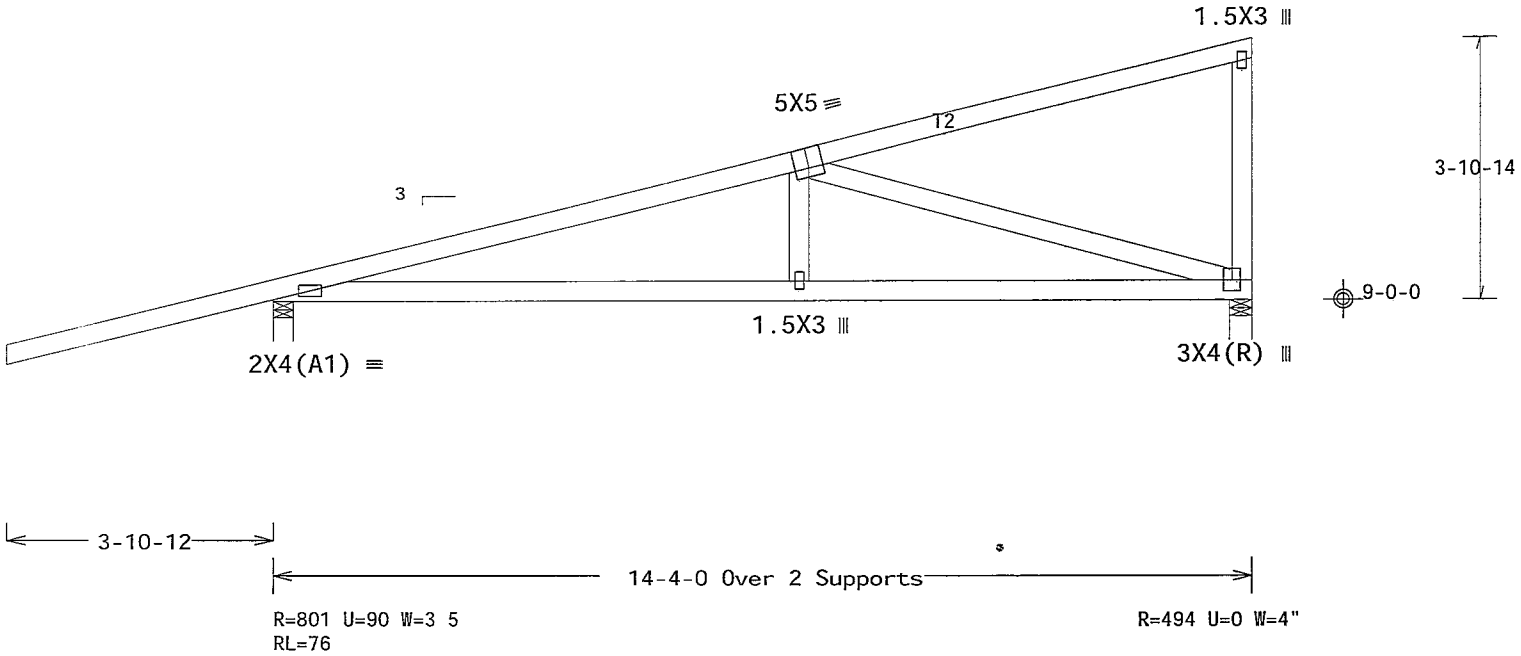
Top chord overhangs have been checked only for loads as indicated
Overhangs not checked for man loads or long-term deflection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12.03.04.0326.13 | QTY:8 | FL/-/3/-/-/R/- | Scale =.375"/Ft

ALPINE
ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
* IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care in fabr cat ng handling shipp ng install ng and brac ng Re to follow the latest edition of BCSI (Build ng Component Safety Informat on by TPI and WTCA) fo practices pr or to perform ng these funct ons Installers shall prov de temporary brac ng Unless noted otherwise top chord shall have properly attached structural sheathing and bot shall have a properly attached rigid ce l ng Locations shown for permanent lateral restra shall have brac ng installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be respons ble for any dev at on any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for hand ng sh pp ng brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on Deta ls unless noted otherw se Refer to draw ngs 160A-Z for standard plate pos t ons draw ng or cover page l sting th s draw ng nd cates acceptance of profess ona engineer r respons b l ty solely for the des gn shown The su tab l ty and use of th s des gn for any the respons b l ty of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more nformat on see general notes page ITW-BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www ccsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73513
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038013
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37 0 PSF	SEQN-	349480
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - CJ3 3 Jack)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

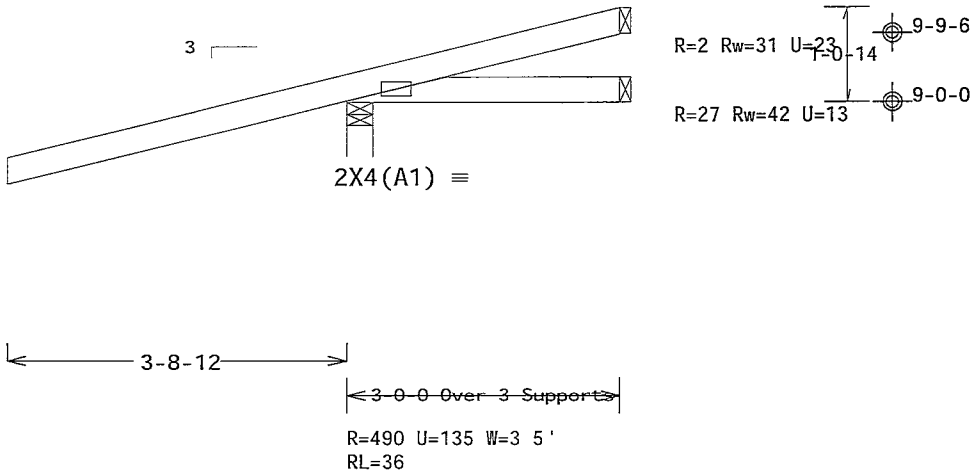
Bottom chord checked for 10 00 psf non-concurrent live load

Top chord overhangs have been checked only for loads as indicated
Overhangs not checked for man loads or long-term deflection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 03 04 0326.13 | QTY:2 | FL/-/3/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73514
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038014
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37 0 PSF	SEQN-	349505
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - CJ5 5 Jack)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

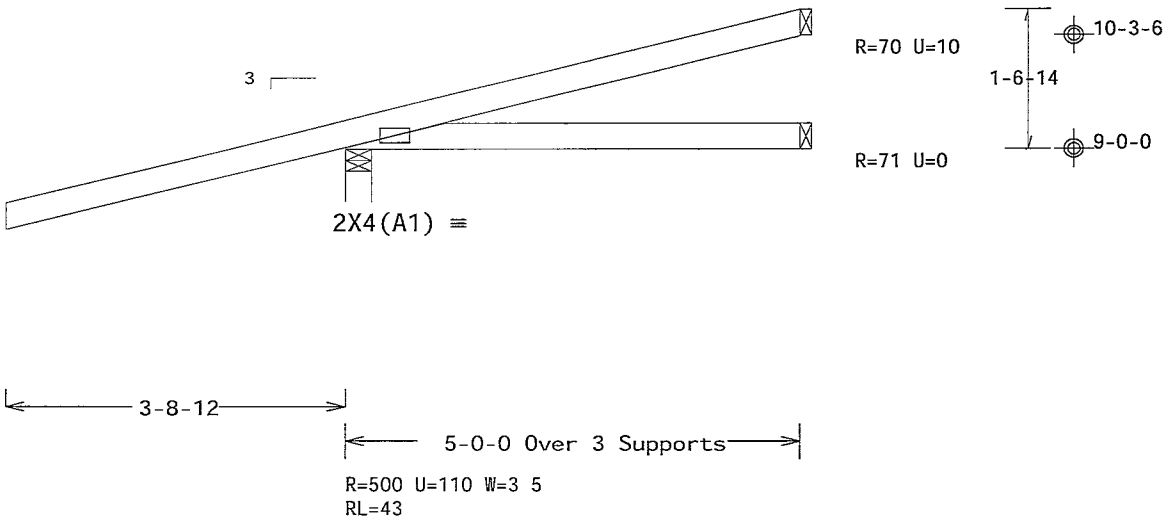
Top chord overhangs have been checked only for loads as indicated
Overhangs not checked for man loads or long-term deflection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 03 04 0326.13 | QTY:2 | FL/-/3/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng hand ng sh pp ng nstall ng and brac ng Re fo ow the latest ed t on of BCSI (Bu d ng Component Safety Information by TPI and WTCA) fo practices pr or to perform ng these funct ons Installers shall provide temporary bracing Unless noted otherwise top chord shall have properly attached structural sheathing and bot shall have a properly attached rigid ceil ng Locat ons shown for permanent lateral restraint of webs shall have bracing installed per BCSI sect ons B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from this des gn any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for hand ng sh pp ng nstall at on & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Deta ls unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos tions Any fa on this draw ng or cover page l st ng th s draw ng nd cates acceptance of profess onal engineering respons b l ty solely for the des gn shown The su tab l ity and use of th s des gn for any the respons b l ty of the Bu ld ng Dos gner per ANSI/TPI 1 Sec 2 For more nformation see general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20 0 PSF	REF	R9114- 73515
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038015
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349509
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - EJ7 7 End Jack)

Top chord 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B

Lumber grades designated with 13B' use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

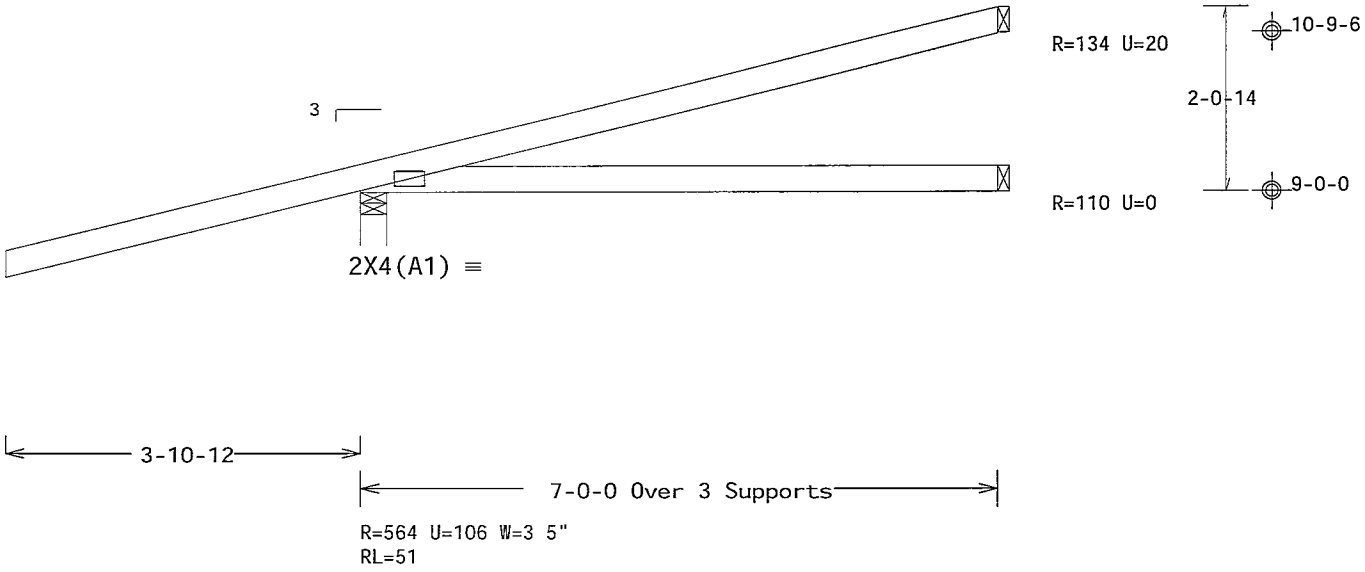
Top chord overhangs have been checked only for loads as indicated
Overhangs not checked for man loads or long-term deflection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 03 04 0326.13 | QTY:4 | FL/-/3/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handling sh pp ng nstall ng and brac ng Re f to follow the latest ed t on of BCSI (Bu ld ng Component Safety Information by TPI and WTCA) for safety pract ces pr or to performing these functions Installers shall prov de temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chords shall have a properly attached r g d ce l ng Locations shown for permanent lateral restraint of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from this design any fa lure to build the truss n conformance w th ANSI/TPI 1 or for hand l ng sh pp ng nstallation & bracing of trusses Apply plates to each face of truss and pos t on as shown above and on the Joint Detai s unless noted otherwise Refer to draw ngs 160A-Z for standard p ate pos t ons Appeal to this draw ng or cover page listing this draw ng nd cates acceptance of profess onal eng neer r s respons bility solely for the design shown The suitab l ty and use of th s design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more nformat on see general notes page ITW-BCG www twbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF	R9114- 73516
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038016
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD	37.0 PSF	SEQN-	349478
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - H11 14 4 Mono Hip)

Top chord 2x4 SP 2850f-2 3E T2 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B" use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 54 plf at -3 90 to 54 plf at 11 00
TC- From 54 plf at 11 00 to 54 plf at 14 33
BC- From 4 plf at -3 90 to 4 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 0 29
BC- From 50 plf at 0 29 to 50 plf at 14 00
BC- From 20 plf at 14 00 to 20 plf at 14 33

Wind loads and reactions based on MWFRS with additional C&C member design

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

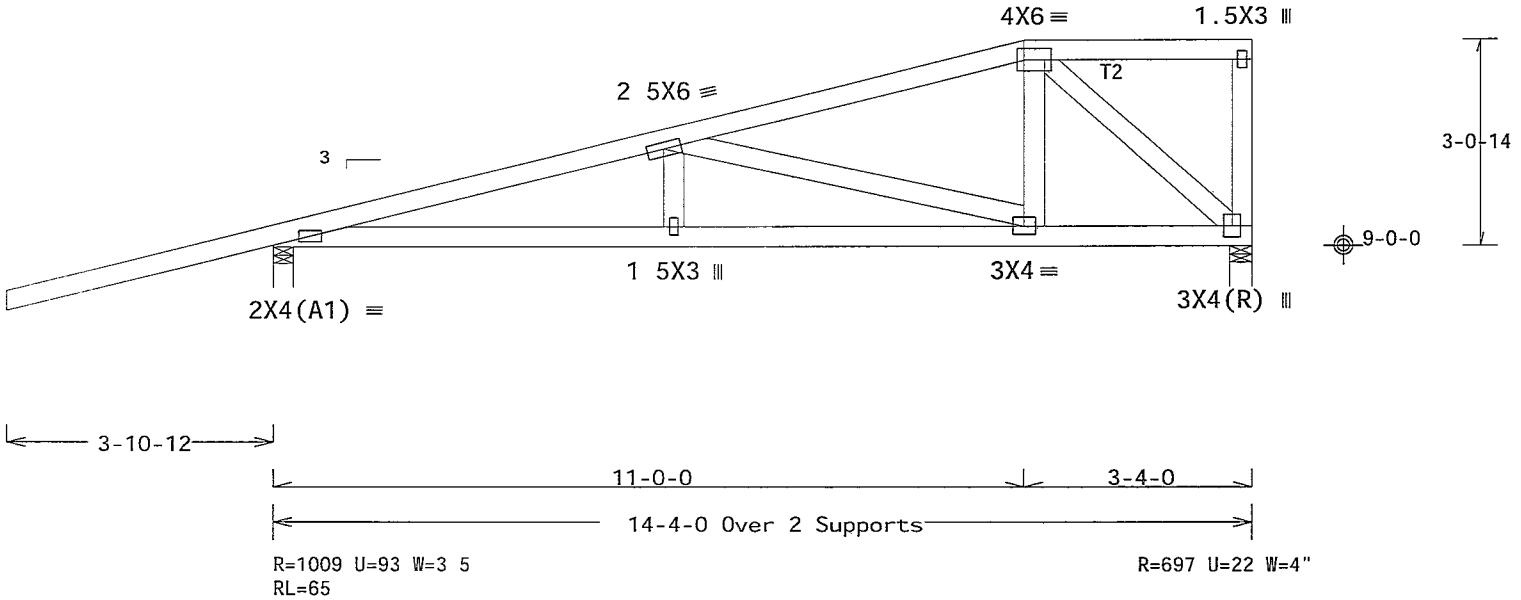
Right end vertical not exposed to wind pressure

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 03 04 0326 13 QTY:1 FL/-/3/-/-/R/- Scale = 375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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Trusses requ re extreme care n fabr cat ng handling sh pp ng nstal ng and brac ng Refer to follow the latest ed tion of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for pract ces pr or to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherw se top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ce l ng Locat ons shown for permanent lateral restraint of webs shall have brac ng nstalled per BCSI sect ons B3 B7 or B10 as appl cable

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20 0 PSF	REF	R9114- 73517
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038017
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349538
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - H13 14 4 Mono Hip)

Top chord 2x4 SP #1-13B T1 2x4 SP 2850f-2 3E
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Top chord overhangs have been checked only for loads as indicated Overhangs not checked for man loads or long-term deflection

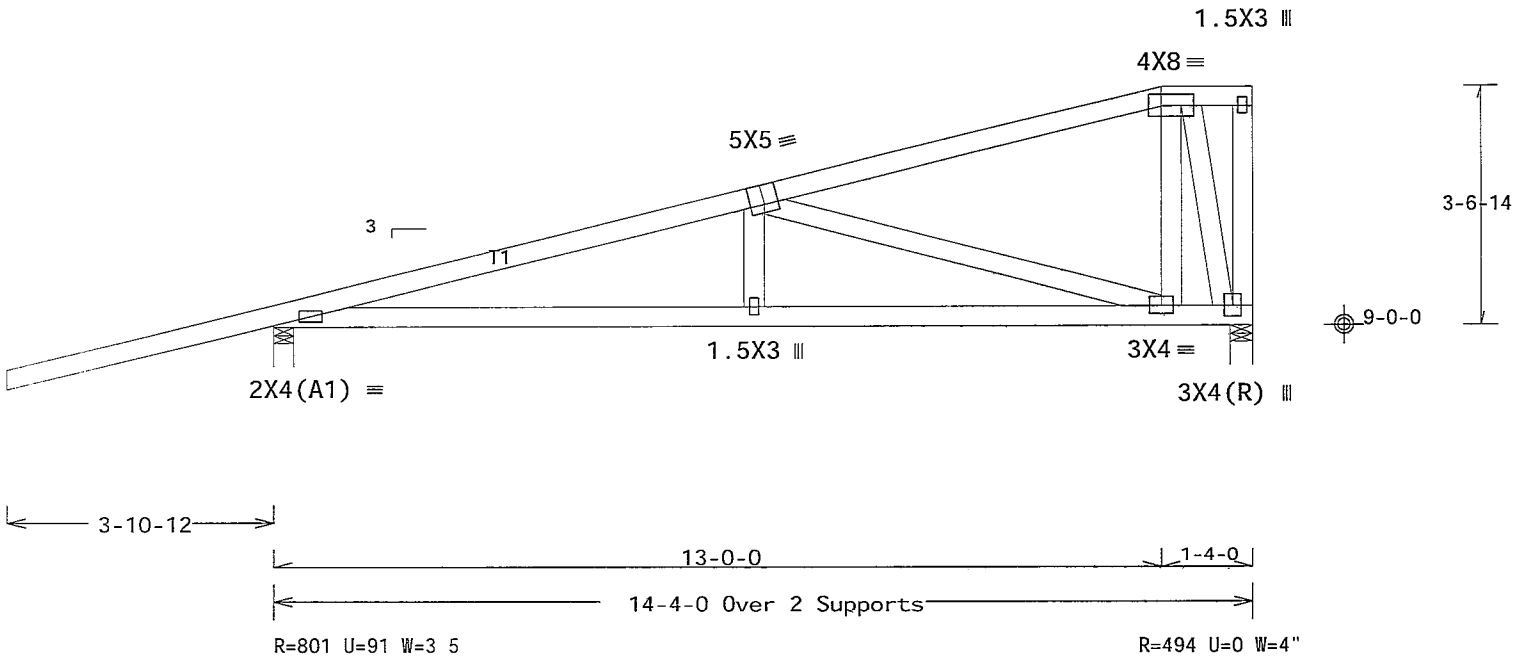
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

MWFRS loads based on trusses located at least 15 00 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 00 24 D326 13 QTY:1 FL/-/3/-/-/R/- Scale = 375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20 0 PSF	REF	R9114- 73518
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038018
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349490
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City, FL - H7 14'4 Mono Hip Girder)

Top chord 2x4 SP 2850f-2 3E T2 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B' use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 54 plf at -3 90 to 54 plf at 7 00
TC- From 27 plf at 7 00 to 27 plf at 14 33
BC- From 4 plf at -3 90 to 4 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 0 29
BC- From 50 plf at 0 29 to 50 plf at 7 03
BC- From 40 plf at 7 03 to 40 plf at 14 00
BC- From 10 plf at 14 00 to 10 plf at 14 33
TC- 171 99 lb Conc Load at 7 03
TC- 134 13 lb Conc Load at 9 06,11 06,13 06
BC- 330 27 lb Conc Load at 7 03
BC- 109 71 lb Conc Load at 9 06,11 06,13 06

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

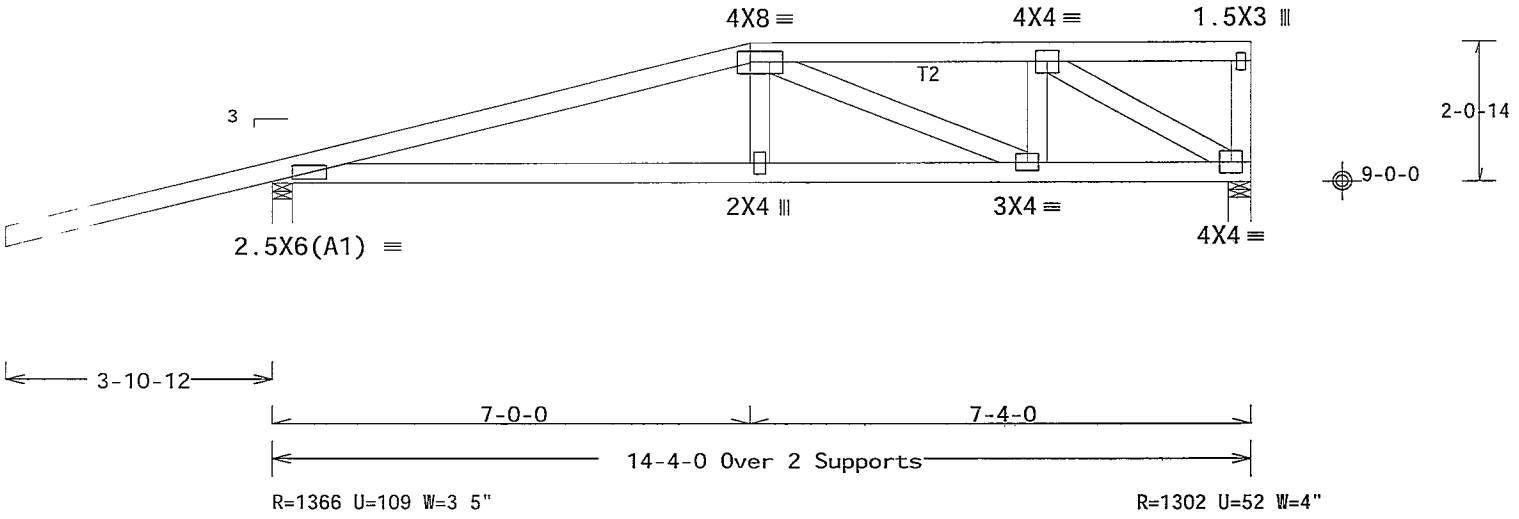
Wind loads and reactions based on MWFRS

Right end vertical not exposed to wind pressure

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 06 04 03 26 13 | QTY:1 | FL/-/3/-/-/R/- | Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20.0 PSF	REF R9114- 73519
TC DL	7.0 PSF	DATE 02/07/14
BC DL	10.0 PSF	DRW HCUSR9114 14038019
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT LD.	37.0 PSF	SEQN- 349528
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - H9 14 4" Mono Hip)

Top chord 2x4 SP 2850f-2 3E T2 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B' use design values approved 1/30/2013 by ALSC

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 54 plf at -3 90 to 54 plf at 9 00
TC- From 54 plf at 9 00 to 54 plf at 14 33
BC- From 4 plf at -3 90 to 4 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 0 29
BC- From 50 plf at 0 29 to 50 plf at 14 00
BC- From 20 plf at 14 00 to 20 plf at 14 33

Wind loads and reactions based on MWFRS

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

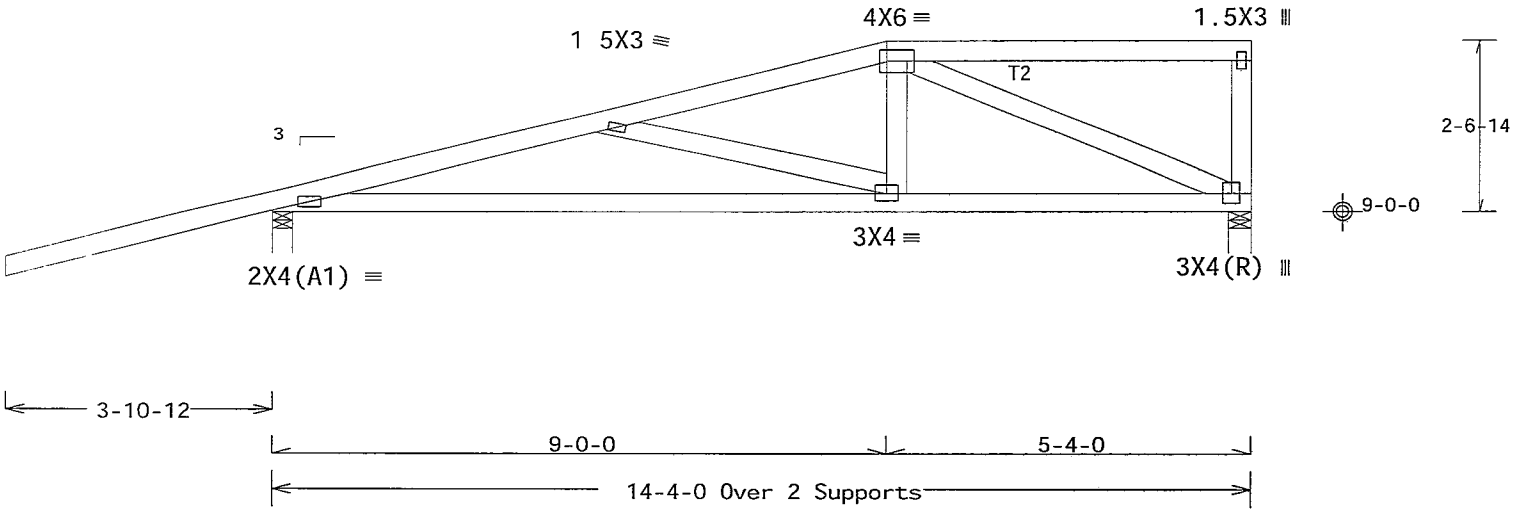
Right end vertical not exposed to wind pressure

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

MWFRS loads based on trusses located at least 7 50 ft from roof edge



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 12 00 04 0326 13 QTY:1 FL/-/3/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0 278

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ITW Bu ld ng Components Group Inc (ITWBCG) shall not be respons ble for any dev at on from th s design any failure to bu ld the truss in conformance w th ANSI/TPI 1 or for hand l ng sh pp ng installation & brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Data is unless noted otherwise Refer to draw ngs 160A-Z for standard plate pos t ons A sl draw ng or cover page l sting th s draw ng nd cates acceptance of profess ona engineer ng respons b l ty solely for the des gn shown The su tab l ty and use of th s des gn for any str the respons b l ty of the Building Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th general notes page ITW BCG www itwbcg com TPI www tpinst org WTCA www sbc ndustry com ICC www iccsafe org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER 02/07/2014

TC LL	20.0 PSF	REF	R9114- 73520
TC DL	7.0 PSF	DATE	02/07/14
BC DL	10.0 PSF	DRW	HCUSR9114 14038020
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	349532
DUR.FAC.	1.25	FROM	JMW
SPACING	24.0"	JREF-	1V3P487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-022--BRYAN ZECHER /FARNELL RESIDENCE -- Lake City FL - HJ7 9 10'13 Hip Jack Girder)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

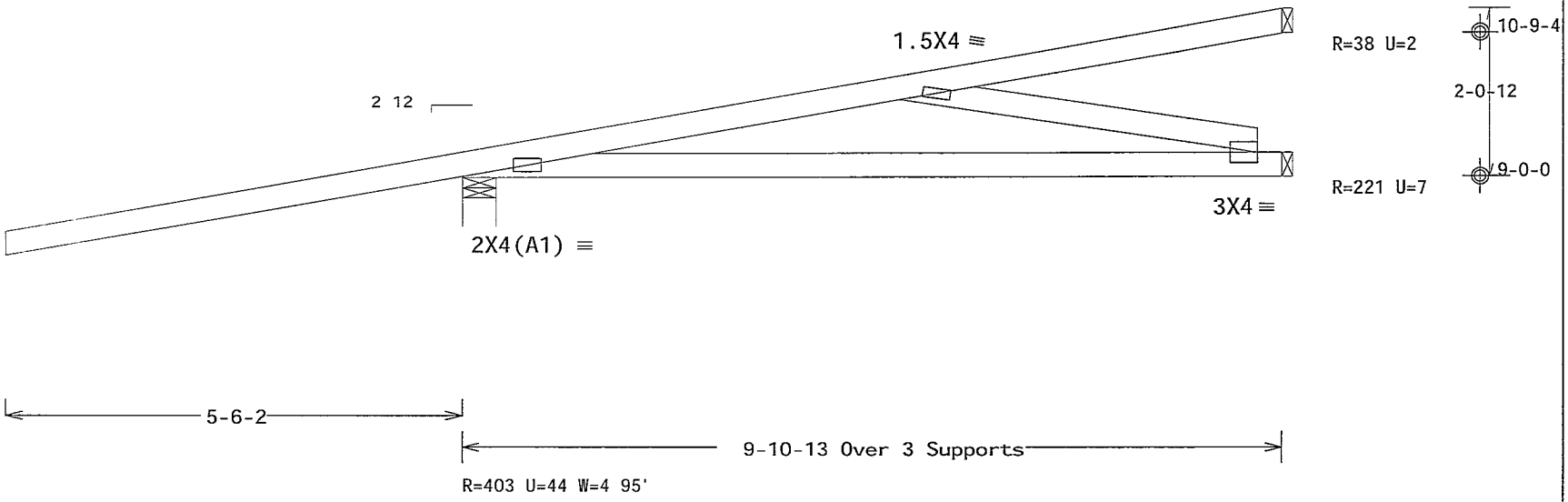
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

Special loads
------(Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC- From 0 plf at -5 51 to 54 plf at 0 00
TC- From 2 plf at 0 00 to 2 plf at 9 90
BC- From 0 plf at -5 51 to 4 plf at 0 00
BC- From 20 plf at 0 00 to 20 plf at 4 31
BC- From 2 plf at 4 31 to 2 plf at 9 90
TC- 4 05 lb Conc Load at 4 31
TC- 140 45 lb Conc Load at 7 13
BC- 53 44 lb Conc Load at 4 31
BC- 142 25 lb Conc Load at 7 13

Bottom chord checked for 10 00 psf non-concurrent live load



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave | FT/RT=10%(0%)/0(0) | 12 06 04 0926 13 | QTY:1 | FL/-/3/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
02/07/2014

TC LL	20 0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R9114- 73521
DATE	02/07/14
DRW	HCUSR9114 14038021
HC-ENG	WHK/WHK
SEQN-	349512
FROM	JMW
JREF-	1V3P487_Z02

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement

Alternative reinforcement specified in chart below may be conservative For minimum allowable reinforcement, re-run design with appropriate reinforcement type

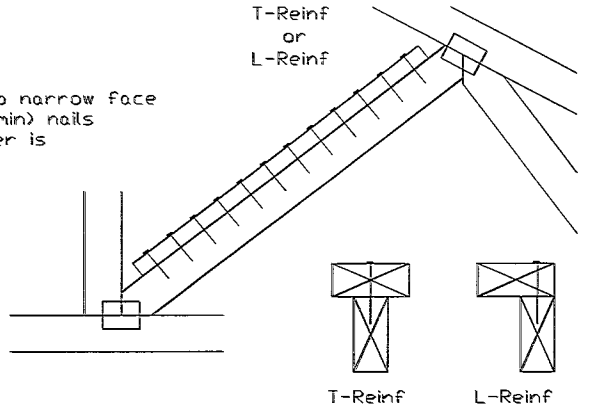
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T reinforcement L reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design

(*) Center scab on wide face of web Apply (1) scab to each face of web

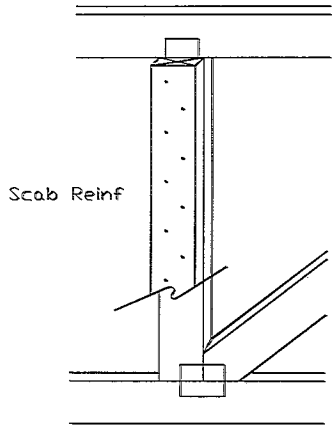
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length



Building Components Group Inc

Earth City MO 63045

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WILLIAM H. KRICK LICENSE No 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

02/07/2014

TC LL	PSF	REF CLR Subst
TC DL	PSF	DATE 8/15/13
BC DL	PSF	DRWG BRCLBSUB0813
BC LL	PSF	
TOT LD	PSF	
DUR FAC		
SPACING		

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 15' Mean Height Partially Enclosed Exposure C Kzt = 1.00
Or 100 mph Wind Speed 15' Mean Height Enclosed Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" o.c.	SPF / HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP / DF	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" o.c.	SPF / HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP / DF	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF / HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP / DF	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem-Fir
#1 & Btr
#1

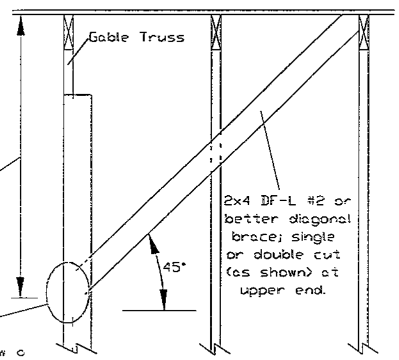
Douglas Fir-Larch
#1
#2

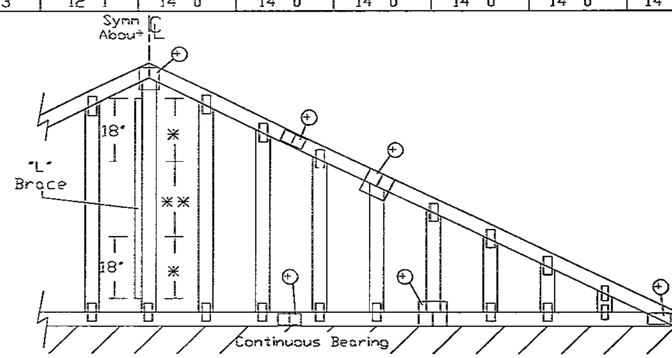
Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 33# at each end. Max web total length is 14'.





Refer to chart above for max gable vertical length.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

※ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

※※For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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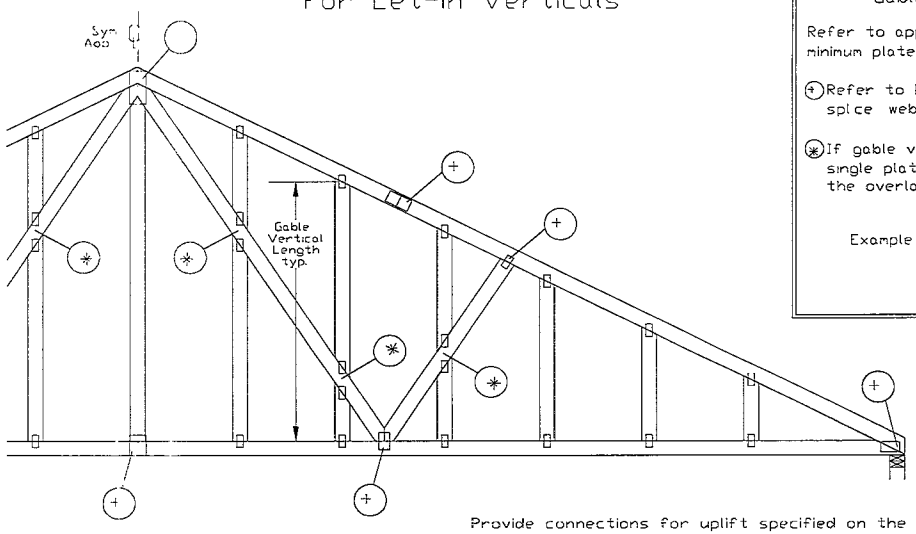
ITW Building Components Group Inc.
Earth City MO 63045

WILLIAM H. KRICK
LICENSE
No 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
'14
02/07/2014

MAX TOT LD 60 PSF	
MAX SPACING 24.0"	
REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

Gable Detail
For Let-in Verticals



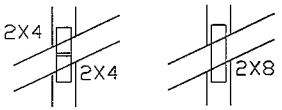
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

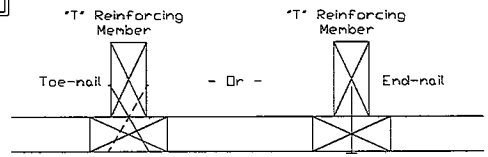
(+) Refer to Engineered truss design for peak splice web and heel plates

(*) If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web.

Example



"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members multiply "T" increase by length (based on appropriate ITW gable detail)

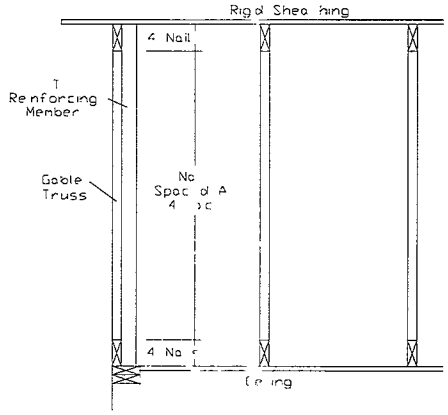
Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1.00
- Gable Vertical = 24"o.c SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8' 7" = 11' 2"



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148"x 3" min) Nails at 4" o.c plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" o.c plus
- (4) toenails in the top and bottom chords

This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A13015980109 A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109 A10015020109 A14015020109,
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212 A14015ENC100212 A16015ENC100212,
A18015ENC100212 A20015ENC100212 A20015END100212 A20015PED100212,
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212 A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

TW
Building Components Group Inc.

Earth City MO 63045

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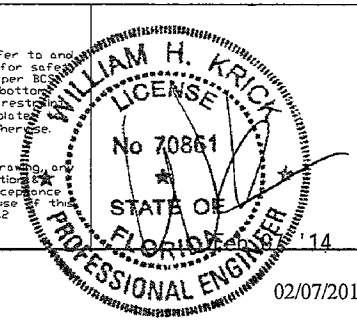


02/07/2014

MAX TOT LD	60 PSF
DUR FAC	ANY
MAX SPACING	24 0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30' Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph 30ft Mean Hgt, ASCE 7-10, Part Enclosed, Exp C
Kzt = 1.00 Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber plates and other information not shown on this detail

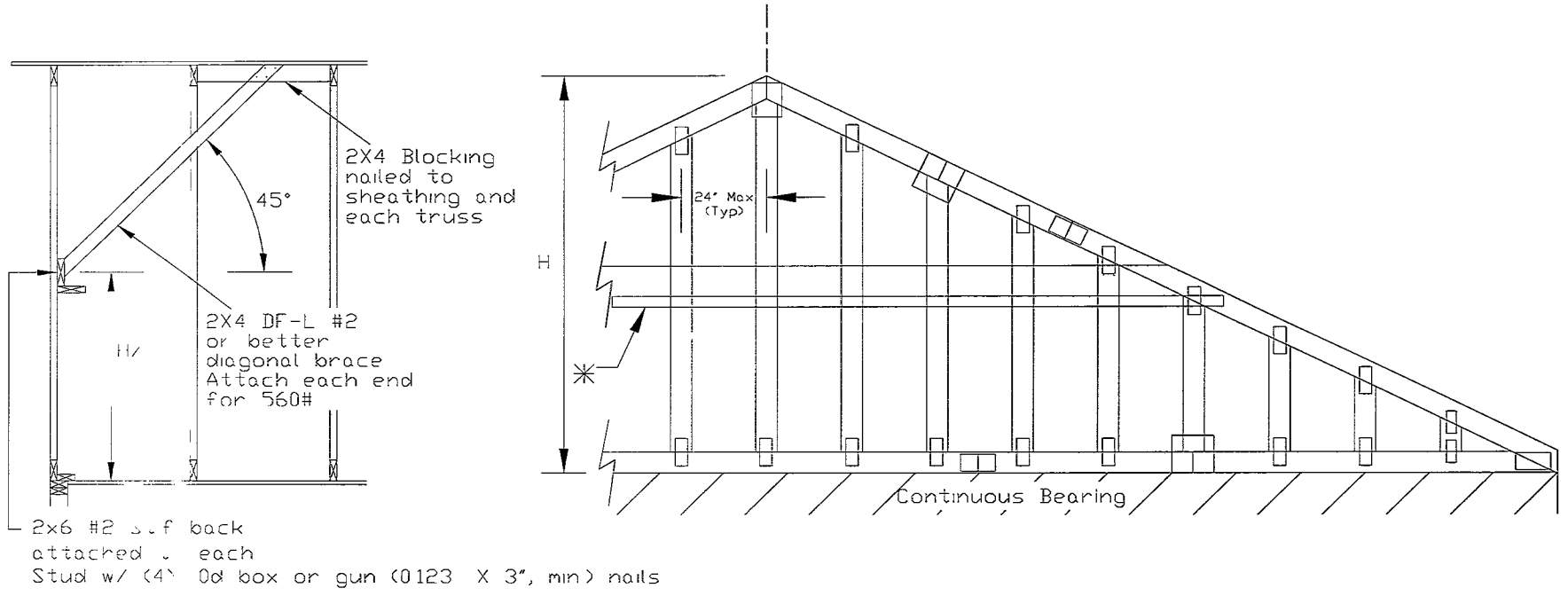
Nails 10d box or gun (0.128 x3",min) nails

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" o.c.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
** MPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) or safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise: top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plate to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, or failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites.
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org, WTCA: www.sbcindustry.org ICC: www.iccsa e.org

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LICENSE
No 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER

02/07/2014

MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212