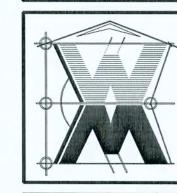


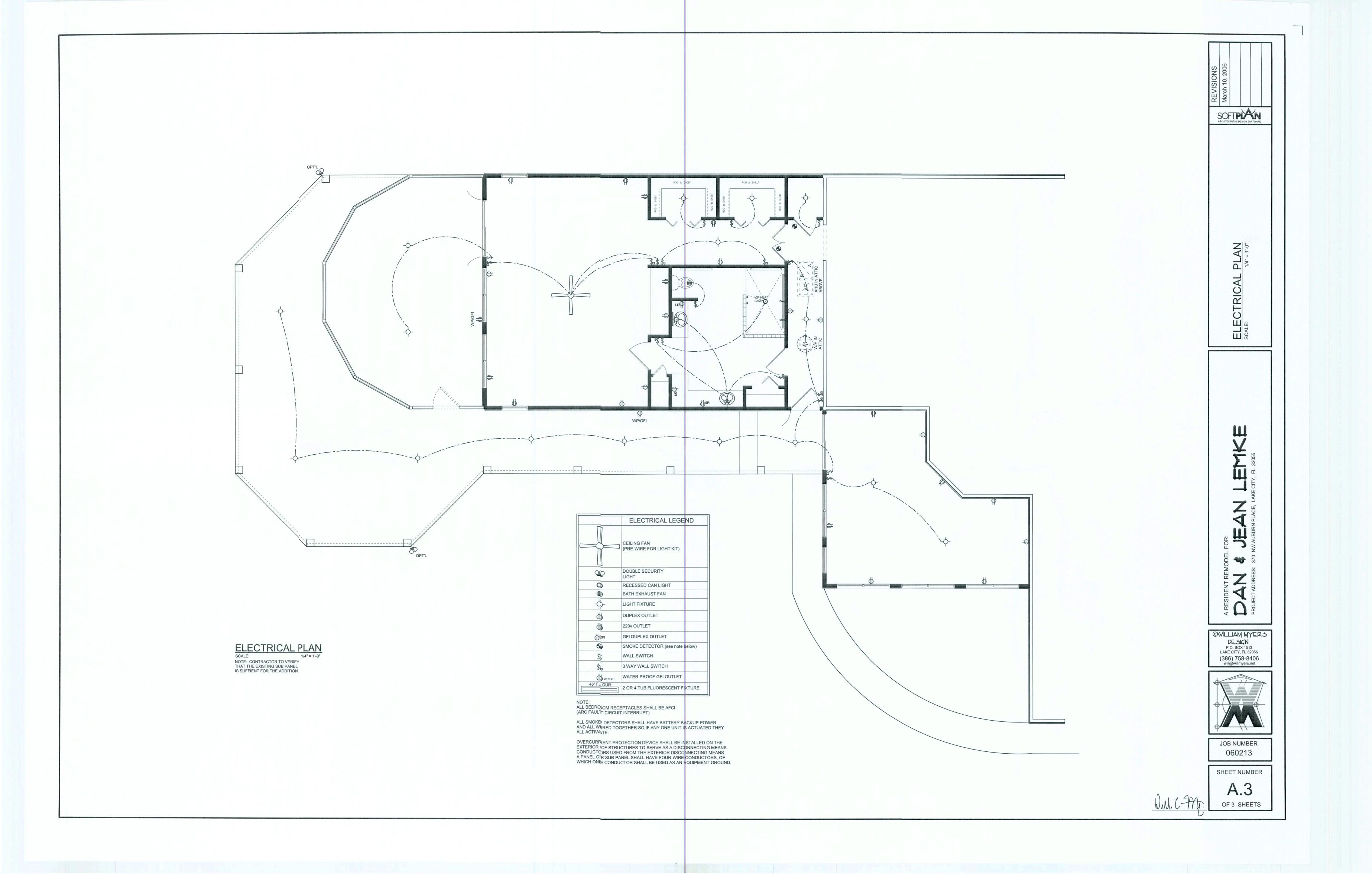
OWILLIAM MYERS **DE. SIGN**P.O. BOX 1513
LAKE CITY, FL 32056 (386) 758-8406 will@willmyers.net

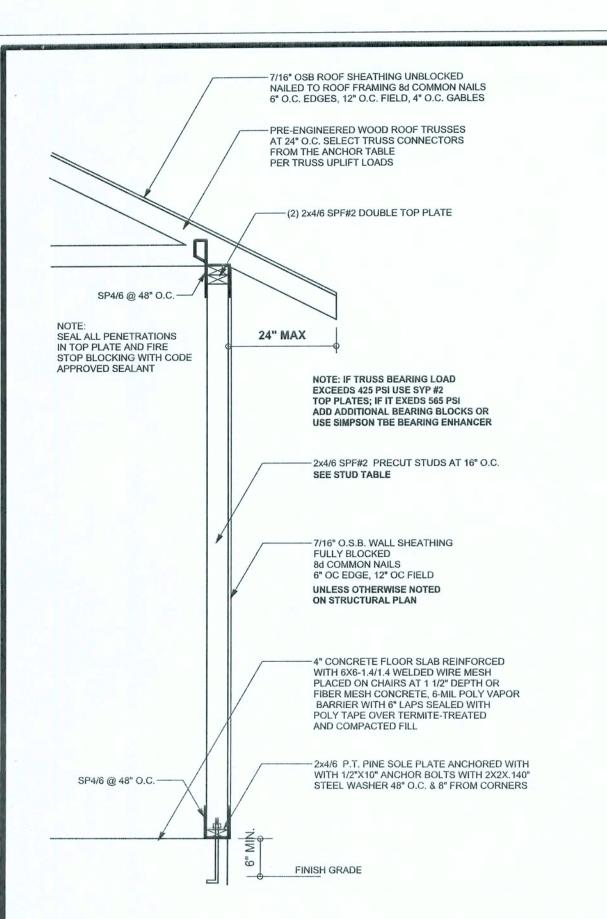


JOB NUMBER 060213

SHEET NUMBER

OF 3 SHEETS





ROOF SHEATHING -\ /- 8d @ 6" O.C. ROOF SHEATHING -ROOF TRUSS -ROOF TRUSS -7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 4" O.C. TOP & BOTTOM 6" O.C. EDGE, 12" O.C. FIELD 2 x 4/6 SPF #2 STUDS AT 16" O.C. - 1/2" X 7" WEDGE ANCHORS AT 48" OC UNO -7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 4" O.C. TOP & BOTTOM 6" O.C. EDGE, 12" O.C. FIELD SEE FOUNDATION DETAILS

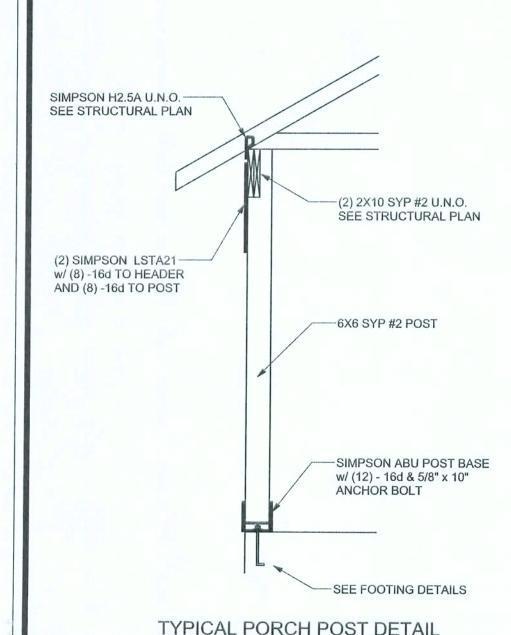
INTERIOR SHEAR WALL DETAIL

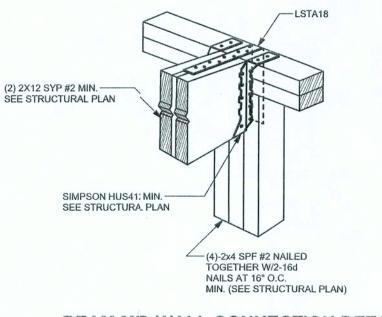
ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

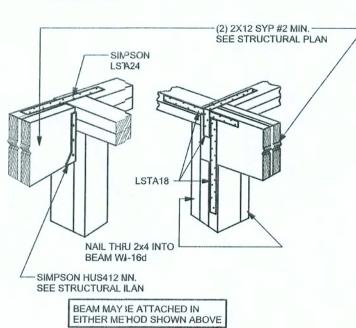
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

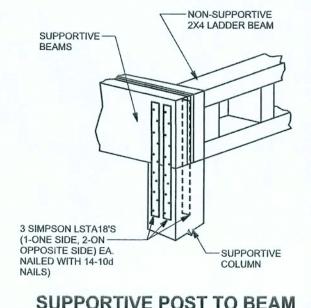




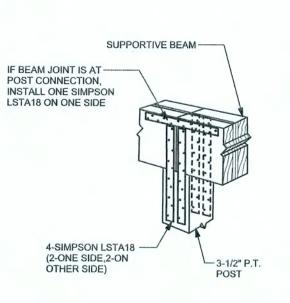
BEAM MID-WALL CONNECTION DETAIL SCALE: N.T.S.



BEAM CORNER CONNECTION. DETAIL SCALE: N.T.S.



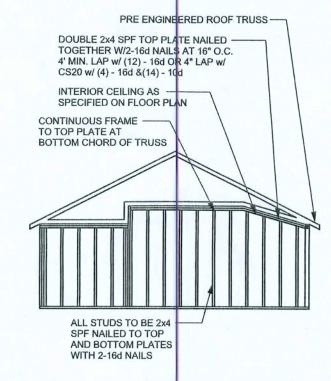
SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

GRADE & SPECIES TABLE

			Fb (psi)	E (10 ⁶ psi)
2x8	SY	#2	1200	1.6
2x10	SYF	#2	1050	1.6
2x12	SYF	#2	975	1.6
GLB	24F-\	/3 SP	2400	1.8
LSL	TIMBER	STRAND	1700	1.7
LVL	MICR	DLAM	1600	1.9
PSL	PARA	LAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

-NAIL SHEATHING TO HEADER AND TOP

SCALE: N.T.S.

(6) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER INTO KING STUD CRIPPLES IF REQUIRED (4) .131 x 3 1/4" GUN NAILS CRIPPLES IF REQUIRED (4) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER INTO KING STUD (4) .131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL INTO JACK STUD U.N.O. (SEE STRUCTURAL PLAN) SP4 OR (2) H2.5A OR (2) SSP

ALL OPENINGS (U.N.O.

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

TYPICAL HEADER STRAPING DETAIL

MASONRY NOTES:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION

R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN

TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE

COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS

MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED

LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO

SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL

RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE

REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF

BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF

DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT

TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES

RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements	
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi	
2.1	Mortar	ASTM C 270, Type N, UNO	
2.2	Grout	ASTM C 476, admixtures require approva	
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" runnin bond and 12"x12" or 16"x16" column block	
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"	
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)	
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS	
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS	
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.	
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.	

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: $6" \times 6" \times 1.4 \times 1.4 \times 1.4$, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

ROOF SYSTEM DESIGN

TRUSS SHEETS.

	ILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE ICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 MENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE	A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL DOAD ENGINEER IMMEDIATELY.
DESIGN, I TRUSS-TO	HE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, O-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL B LOCATIONS.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

DESIGN DATA

	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED F 12" EMBEDMEN
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED I 12" EMBEDMEN
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED F 12" EMBEDMEN
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED F 12" EMBEDMEN
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone Effective Wind Area (ft2)

19.9 -25.5 | 18.1 | -21.8

4 21.8 -23.6 18.5 -20.4

5 21.8 -29.1 18.5 -22.6

Doors & Windows | 21.8 | -29.1

16x7 Garage Door | 18.5 | -21.0

Worst Case

(Zone 5, 10 ft2)

8x7 Garage Door

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

.) WIND IMPORTANCE FACTOR = 1.0

ROOF ANGLE = 10-45 DEGREES

.) MEAN ROOF HEIGHT = <30 FT

) WIND EXPOSURE = B

BUILDING CATEGORY = II

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT

ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%

SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

REVISIONS

SOFTPLAN

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering

comply with section R301.2.1, florida building

ode residential 2004, to the best of my

Mark Disosway, P.E. hereby expressly reser

LIMITATION: This design is valid for one

building, at specified location.

MARK DISOSVVAY
P.E. 53915

Isaac Construction

Dan & Jean Lemke Addition

ADDRESS: 370 NW Auburn Place, Lake City, Florida 32055

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

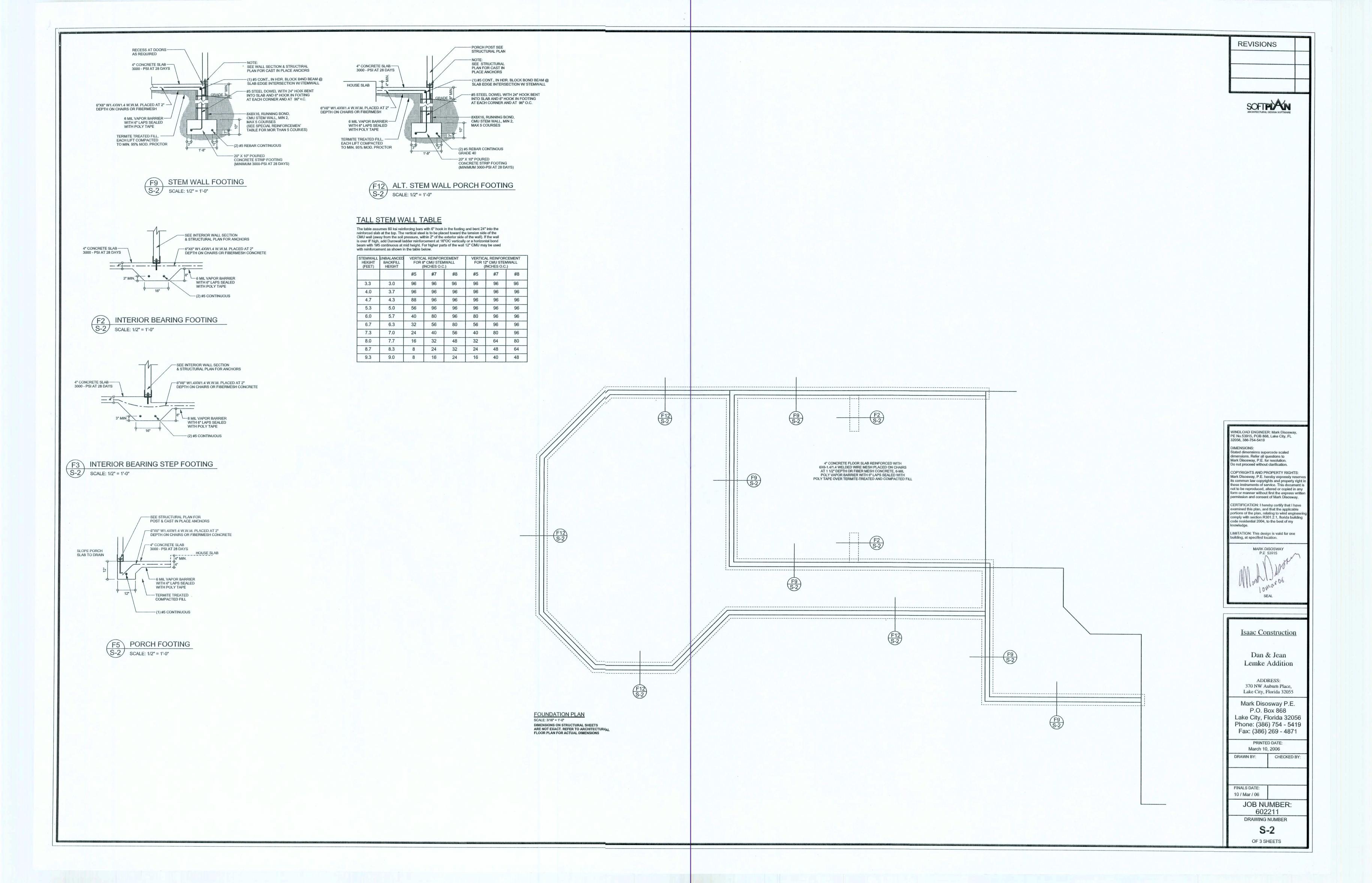
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March 10, 2006

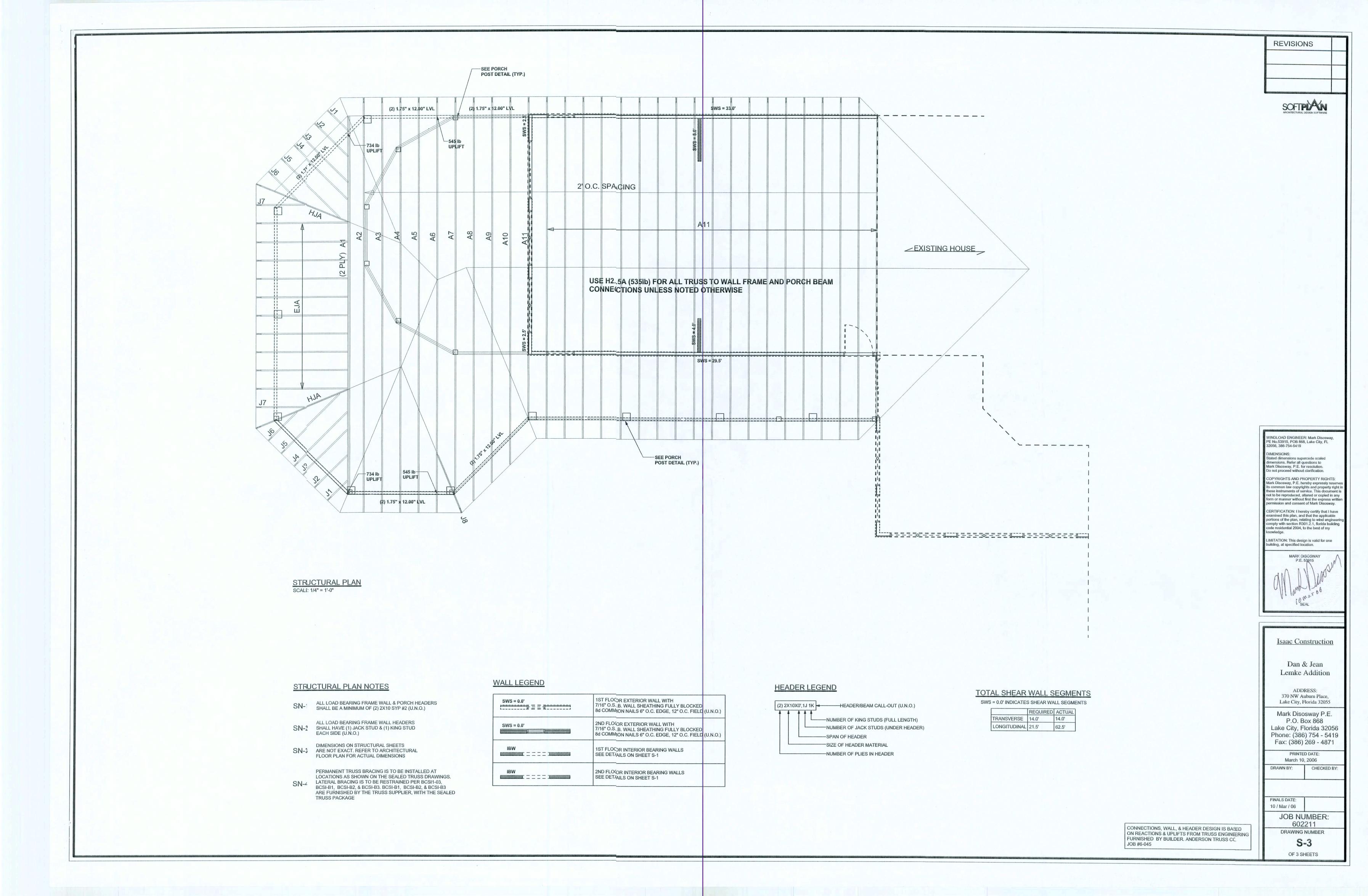
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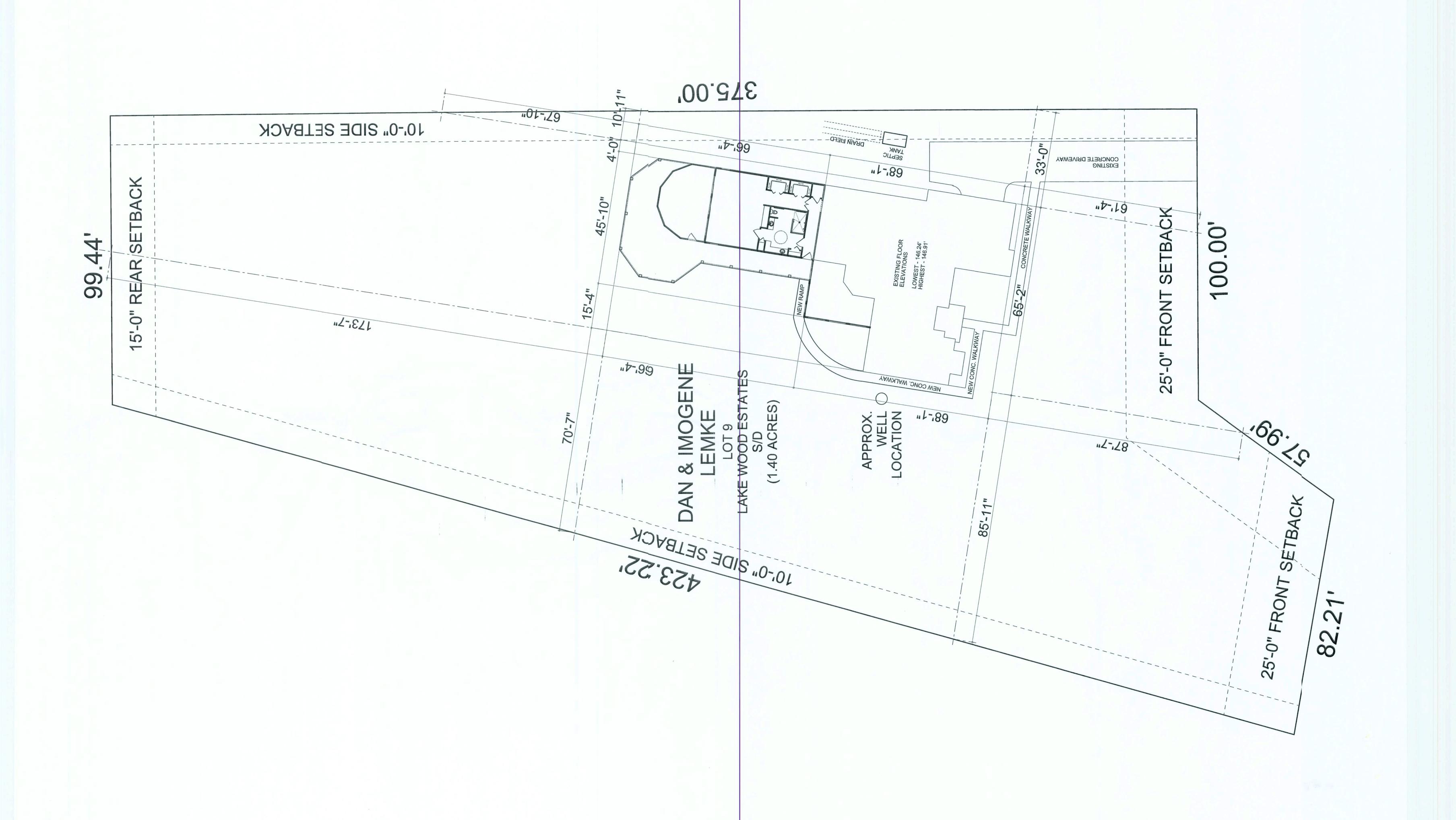
FINALS DATE: 10 / Mar / 06

> JOB NUMBER: 602211 DRAWING NUMBER

> > OF 3 SHEETS







DESCRIPTION:

COMMENCE AT THE SOUTHEAST CORNER OF SECTION 23, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN S. 88 44 04" W. ALONG THE SOUTH LINE OF SAID SECTION 23 A DISTANCE OF 1188.16 FEET TO THE WESTERLY RIGHT- OF-WAY LINE OF OLD MILL ROAD, SAID POINT BEING ON THE ARC OF A CURVE CONCAVE TO THE LEFT HAVING A RADIUS OF 1436.94 FEET AND A CENTRAL ANGLE OF 00°-20'-42"; THENCE NORTHWESTERLY ALONG THE ARC OF SAID CURVE, ALSO SAID WESTERLY RIGHT-OF- WAY LINE 8.65 FEET TO THE POINT OF BEGINNING; THENCE S. 68°-38'-49" W. 792.40 FEET; THENCE N. 02°-43'-12" W. 206. 58 FEET; THENCE N. 30°- 24'- 17" W. 290.00 FEET; THENCE N. 18°- 20'- 32" W. 395.00 FEET; THENCE N. 30°- 24'-17" W. 420.00 FEET; THENCE N. 59° 35'- 43" E. 210.00 FEET; THENCE S. 30°-24'-17" E. 268.53 FEET; THENCE N. 82° 38'-49" E. 333.98 FEET; THENCE N. 10° 38'-49" E. 71. 21 FEET; THENCE N. 50°21'-11" W. 168.26 FEET; THENCE N. 67°-01'-21" W. 436.12 FEET; THENCE S. 89- 38'-49" W. 100.00 FEET; THENCE N 00°-21'-11" W. 425.00 FEET TO A POINT ON THE SOUTHERLY RIGHT- OF WAY LINE OF LAKE JEFFERY DRIVE; THENCE N. 58°- 15'- 07" E. 149.33 FEET TO A POINT ON THE NORTHERLY RIGHT- OF- WAY LINE OF SAID LAKE JEFFERY DRIVE; THENCE N. 07°-09'-28" E. 542.12 FEET TO A POINT ON THE SOUTHWESTERLY RIGHT- OF- WAY LINE OF STATE ROAD NO. 250; THENCE S. 50°- 21'-11" E. ALONG SAID SOUTHWESTERLY RIGHT- OF- WAY LINE 1786.00 FEET TO A POINT ON THE EASTERLY RIGHT- OF- WAY LINE OF SAID OLD MILL ROAD, SAID POINT BEING ON THE POINT OF CURVE OF A CURVE CONCAVE TO THE LEFT HAVING A RADIUS OF 50.00 FEET AND A TOTAL CENTRAL ANGLE OF 90°; THENCE SOUTHWESTERLY ALONG THE ARC OF SAID CURVE, ALSO SAID EASTERLY RIGHT-OF-WAY LINE 78.54 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE S. 39° 38'- 49" W. STILL ALONG SAID EASTERLY RIGHT OF - WAY LINE 255.00 FEET TO THE POINT OF CURVE OF A CURVE CONCINE TO THE LEFT HAVING A RADIUS OF 560.00 FEET AND A TOTAL CENTRAL ANGLE OF 25°- 30'-00"; THENCE SOUTHWESTERLY ALONG THE ARC OF SAID CURVE; ALSO SAID EASTERLY RIGHT-OF-WAY LINE 249.23 FEET TO THE POINT OF COMPOUND CURVE; THENCE N. 75°-51'-11" W. 80 00 FEET TO A POINT ON THE WESTERLY RIGHT- OF- WAY LINE OF SAID OLD MILL ROAD, SAID POINT BEING ALSO ON THE POINT OF CURVE OF A CURVE CONCAVE TO THE LEFT HAVING A RADIUS OF 1020.00 FEET AND A TOTAL CENTRAL ANGLE OF 35° 30'-00"; THENCE SOUTHEASTERLY ALONG THE ARC OF SAID CURVE, ALSO SAID WESTERLY RIGHT- OF- WAY LINE 631.98 FEET TO THE POINT OF REVERSE CURVE OF A CURVE CONCAVE TO THE RIGHT HAVING A RADIUS OF 1436.94 FEET AND A TOTAL CENTRAL ANGLE OF 23° 12'- 53"; THENCE SOUTHEASTERLY ALONG THE ARC OF SAID CURVE, ALSO SAID WESTERLY RIGHT - OF - WAY LINE 50. 01 FEET TO THE POINT OF BEGINNING. CONTAINING 46.27 ACRES, MORE OR LESS.

COUNTY ATTORNEY'S CERTIFICATE :

I HEREBY CERTIFY THAT I HAVE EXAMINED THE FOREGOING PLAT AND THAT IT COMPLIES IN FORM WITH COLUMBIA COUNTY SUBDIVISION ORDINANCE NO. 78-7 AND CHAPTER 177 OF THE FLORIDA STATUTES.

DATE Chow. 1, 1983

COUNTY ACCEPTANCE FOR MAINTENANCE :

I HEREBY CERTIFY THAT THE IMPROVEMENTS HAVE BEEN CONSTRUCTED IN AN ACCEPTABLE MANNER AND IN ACCORDANCE WITH COUNTY SPECIFICATIONS OR THAT A PERFORMANCE BOND OR INSTRUMENT IN THE AMOUNT OF \$__ HAS BEEN POSTED TO ASSURE COMPLETION OF ALL REQUIRED IMPROVEMENTS AND MAINTAINENCE IN CASE OF DEFAULT.

DATE: 11/1/83 Mike Koherto

"LAKE JEFFERY"

(PHASE I)

IN

SECTION 23 & 26, TOWNSHIP 3 SOUTH, RANGE 16 EAST COUNTY, FLORIDA COLUMBIA

APPROVED	BY BOARD	OF	COUNTY	COMMISSIONERS
COLUMBIA	COUINTY,	FLOR	IDA :	

SIGNELD: ()	Ine fight	CHAIRMAN CLERK
DATE :	33	

CERTIFICATE OF CLERK:

THIS PLAT HAVING BEEN APPROVED BY THE COLUMBIA COUNTY SEC BOARD OF COUNTY COMMISSIONERS, IS ACCEPTED FOR FILES AND RECORDED THIS 3 NO DAY OF November 1983 A.D. IN PLAT BOOK 5 PAGE 39-39-A

CLERK OF THE CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

DEDICATION:

KNOW ALL MEN BY THESE PRESENTS THAT FAIRWAY VIEW CORPORATION, A FLORIDA CORPORATION, AS OWNER AND NONB NATIONAL BANK OF FLORIDA, A NATIONAL BANKING CORPORATION, AS MORTGAGEE HAVE CAUSED THE LANDS HEREON DESCRIBLED TO BE SURVEYED, SUBDINDED, AND PLATTED TO BE KNOWN AS " LAKE JEFFERY" AND THAT ALL STREETS, ROADS, AND EASEMENTS, AS SHOWN AND DEPICTED HEREON ARE HEREBY DEDICATED TO THE PERPETUIAL USE OF THE PUBLIC FOR PROPER USES AND PURPOSES THEREION STATED. IN WITNESS WHERIEOF THE PARTIES HERETO HAVE CAUSED THESE PRESENTS

FAIRWAY VIEW COURP. OWNER M.B. RAVNDAL, PARSIDENT CHRISTINA J. ELLIINGTON, SECRETARY

NCNB NAT'L BANK OF FLORIDA, MORTGAGEE CLARENCE B. CANNON, III, VICE - PRESIDENT GILBERT W. MILLER, VICE - PRESIDENT

AS TO ALL PARTIES:

TO BE EXECUTED WITH THEIR SEALS AFFIXED HERETO.

ACKNOWLEDGEMENT STATE OF FLORIDA, COUNTY OF COLUMBIA:

I HEREBY CERTIFY THAT ON THIS 19 DAY OF 1983 A.D., BEFORE ME PERSONALLY APPEARED M.B. RAVNDAL, PRESIDENT AND CHRISTINA J. ELLINGTON, SECRETARY OF FAIRWAY VIEW CORPORATION, A FLORIDA CORPORATION, AS OWNER AND CLARENCE B. CANNON III, WICE-PRESIDENT AND GILBERT W. MILLER, VICE-PRESIDENT OF NCNB NATIONAL BANK OF FLORIDA, A NATIONAL BANKING CORPORATION, AS MORTGAGEE, TO ME KNOWN TO BE THE INDIVIDUALS DESCRIBED IN AND WHO EXECUTED THE HOREGOING DEDICATION, AND THEY ACKNOWLEDGE EXECUTION THEREOF FOR AND ON BEHALF OF SAID INDIVIDUALS AND SAID CORPORATIONS WITH THEIR SEALS DULY AFFIXED THERETO. WITNESS MY HAIND AND SEAL, STATE OF FLORIDA THIS ___ DAY OF DOUBLE 19 ___ A.D.

MY COMMISSION EXPIRES:

NOTARY PUBLIC, STATE OF FLORIDA

Time & Dutheres

- 100' R/W

"LAKE JEFFERY"

(PHASE I)

IN

SECTION 23 8 26, TOWNSHIP 3 SOUTH, RANGE 16 EAST

COLUNIBIA COUNTY, FLORIDA

UNPLATTE	D
LANDS	

	ACRE	LAGES	
LOT	AREA	LOT	AREA
1. 2.3.4.5.6.7.8.9.0.11.12.13.14.	1.42 Ac. 1.33 1.36 1.36 1.45 1.45 1.60 2.30 1.54 1.71 1.50 1.30 1.33 1.30	15. 16. 17. 18. 19. 20. 21. 22. 23. 24. 25. 26. 27. 28.	1.07 Ac. 1.21 1.61 1.53 1.46 1.09 0.97 1.36 1.74 1.00 1.00 1.03 1.04 0.99

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3) \	7		4.	7 1
and the same of th		ROAD & CUR	VE DATA	
eliality out the specific prices when	Δ	RADIUS	TANGENT	ARC LENGT
①	25°- 30'- 00"	600.00	/35.77'	267.04
2	35° 30' 00"	980.00'	3/3.70	607.20
3	23°- 12'-53"	1,476.94	303.37	598.42'
4	25° 00' 00"	1,260.00'	279.34	549.78
5	210-00-00"	880.00	163.10	322.54
6	05°- 15'- 00"	2,840.00'	130.21	260.23
7	40°-00'-00"	600.00'	218.38	4/8.88

LEGEND :

- (1.) \(\overline{\overlin
- (2.) [] = 4"x4" CONCRETE MONUMENT, P.C.P. (PERMANENT CONTROL POINT) WITH BRASS CAP IN TOP STAMPED L.L. LEE & ASSOCIATES, INC. WITH SURVEYOR'S NUMBER, DATE, & MCNUMENT NUMBER.
- (3.) O = IRON PIPE SET.
- (4) 4" × 4" CONCRETE MONUMENT FOUND IN PLACE.
- (5) BOUNDARY & BEARINGS BASED ON PRIOR WORK IN AREA BY L.L. LEE & ASSOCIATES, INC. .
- (6) BOUNDARY HAS A CLOSURE PRECISION OF I FOOT IN 947,000 FEET.
- (7) PRELIMINARY PLANS APPROVED BY COLUMBIA COUNTY BOARD OF COUNTY COMMISSIONERS: 10 4 -83

SURVEYOR'S CERTIFICATION :

I HEREBY' CERTIFY THAT THIS PLAT IS A TRUE AND CORRECT REPRESENTATION OF THE LAND SURVEYED AND SHOWN HEREON, THAT THE SURVEY WAS MADE UINDER MY RESPONSIBLE DIRECTION AND SUPERVISION, AND THAT PERMANENT CONTROL POINTS HAVE BEEN SET AND THAT SURVEY DATA AND MONUMENTATION COMPLIES WITH THE COLUMBIA COUNTY SUBDIVISION ORDINANCE NO. 78-7 AND CHAPTER 177 OF THE FLORIDA STATUTES.

RUFUS C. OGDEN, P.L.S.
FLA. REG. CERT, # 1095
DATE: October 19,193