

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

MARK DISOSWAY P.E. 53915

SPEC HOUSE

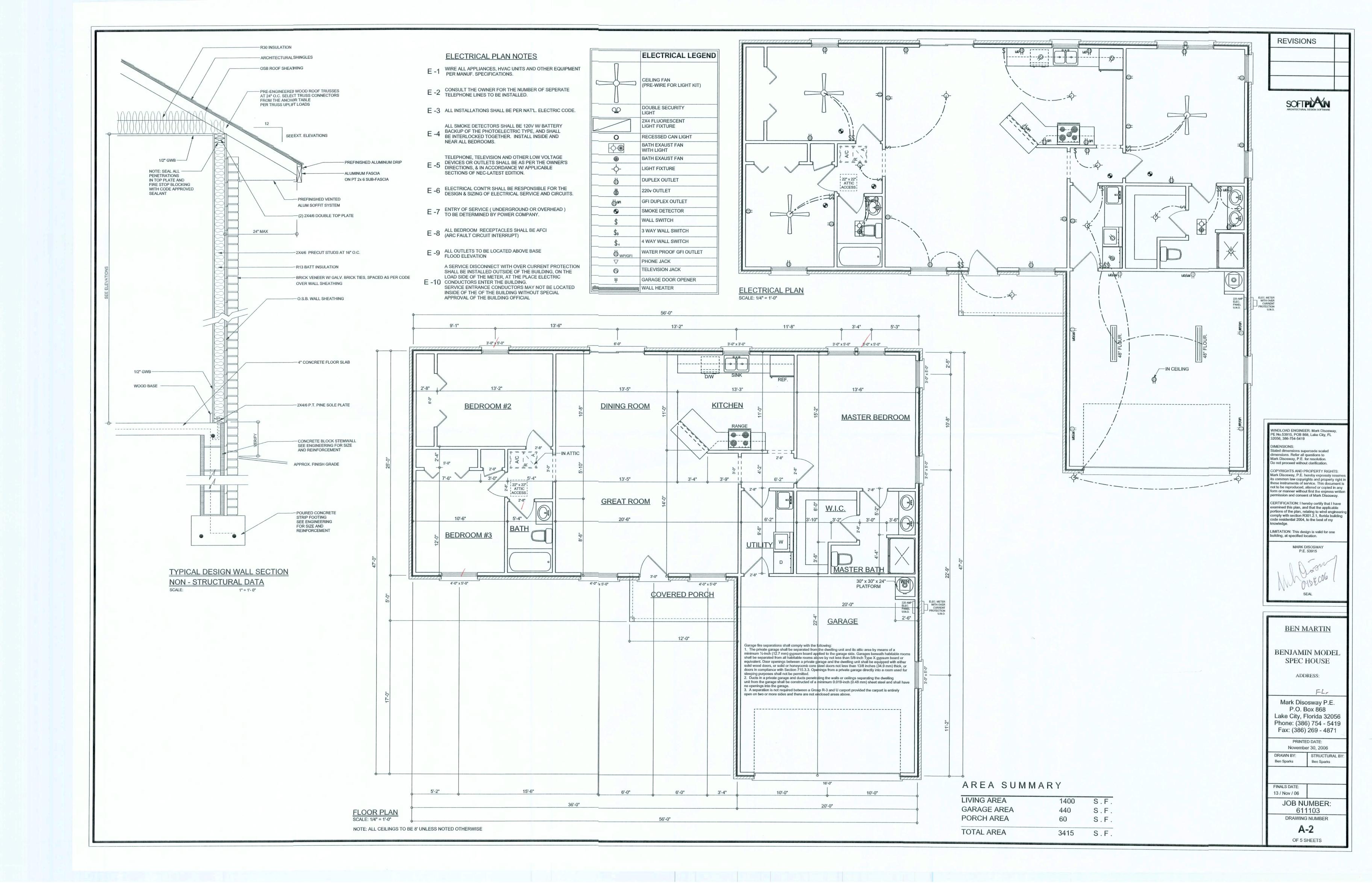
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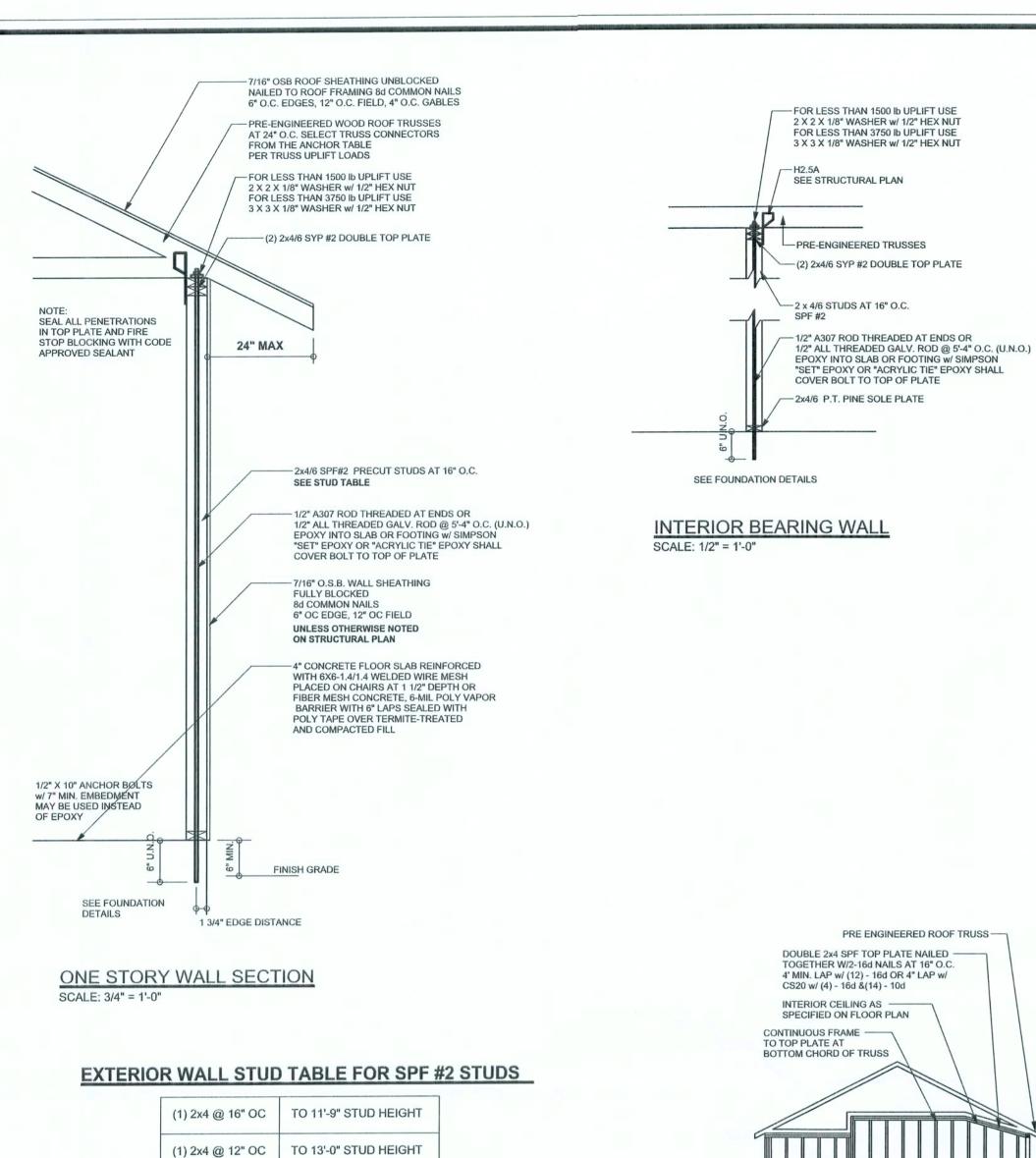
> PRINTED DATE: November 30, 2006

Ben Sparks

JOB NUMBER: 611103

OF 5 SHEETS





PRE ENGINEERED ROOF TRUSS — ALL STUDS TO BE 2x4 ----AND BOTTOM PLATES WITH 2-16d NAILS

CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL** SCALE: N.T.S.

FOR LESS THAN 1500 Ib UPLIFT USE 2 X 2 X 1/8" WASHER IF TRUSS TO WALL STRAPS ARE NAILED FOR LESS THAN 3750 Ib UPLIFT USE TO THE HEADER THE SP4/6 @ 48" O.C. 3 X 3 X 1/8" WASHER ARE NOT REQUIRED -NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT (7) .131 x 3 1/4" GUN NAILS--(7) .131 x 3 1/4" GUN NAILS —SP4/6 @ 48" O.C. (U.N.O.) / TOE NAILED THRU HEADER TOE NAILED THRU HEADER INTO KING STUD INTO KING STUD CRIPPLES IF REQUIRED (5) .131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL-INTO JACK STUD U.N.O. TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN)

TYPICAL 1 STORY HEADER STRAPING DETAIL

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(FOR: 120) MPH, 10'-0" WALL HEIGHT U.N.O.)

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

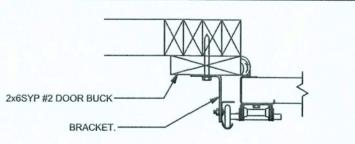
IPLIFT LBS. SYP UPLIFT LBS. SPF		TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED 12" EMBEDME	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED 12" EMBEDME	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED 12" EMBEDMEN	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED 12" EMBEDME	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		TO FOUNDATIO	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB	
< 4175	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d			
< 3335	< 3335	HPAHD22	16-16d			
< 2200	< 2200	ABU44	12-16d		1/2" AB	
< 2300	< 2300	ABU66	12-16d		1/2" AB	
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB	

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK, HORIZONTAL JAMBS DO NOT RANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	WNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK
	NS, FOUNDATION BEARING CAPACITY, GRADE AND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND REQUIREMENTS FOR THE	CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 STATED WIND VELOCITY AND DESIGN PRESSURES.
	LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU S A CONTINUOUS LOAD PATH CONNECTION, CALL OR IMMEDIATELY.
DESIGN, PLACEMENT PLA	FACTURER'S SEALED ENGINEERING INCLUDES TRUSS NS, TEMPORARY AND PERMANENT BRACING DETAILS, ECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS)

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

/EAL	D LOADS PER FLORIDA BUILDING CODE 2004					
ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WI' IN ROOF HEIGHT NOT EXCEEDING LEAST HO UPPER HALF OF HILL OR ESCARPMENT 60FT PE AND UNOBSTRUCTED UPWIND FOR 50x I	DRIZONTAL D	IMEN OFT IN	SION	OR 6	0 FT;
BUII	DING IS NOT IN THE HIGH VELOCITY HURRIC	CANE ZONE				
BUII	DING IS NOT IN THE WIND-BORNE DEBRIS R	REGION				
1.)	BASIC WIND SPEED = 110 MPH					
2.)	WIND EXPOSURE = B					
3.)	WIND IMPORTANCE FACTOR = 1.0					
4.)	BUILDING CATEGORY = II					
5.)	ROOF ANGLE = 10-45 DEGREES					
6.)	MEAN ROOF HEIGHT = <30 FT					
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (I	ENCLOSED B	UILDI	NG)		
8.)	COMPONENTS AND CLADDING DESIGN WIN	ID PRESSUR	ES (T	ABLE	R301	.2(2)
		Zone	Effec	tive W	ind Ar	ea (ft2
			-	0		100
		1 2	-	-21.8 -25.5	18.1	-18. -21.
		2 O'hg	19.9	-40.6	10.1	-40.
	2 2 2 1	3	19.9	-25.5	18.1	-21.
3	4 2 5	3 O'hg	1	-68.3	10.1	-42.
	13 4	4	21.8	-23.6	18.5	-20.
	55	5	21.8	-29.1	18.5	-22.
	*					
	THE THE PARTY OF T	Doors	& Wind	dows	21.8	-29.
	**************************************	Wor	st Cas	е	21.8	-29.
	**************************************	Wor (Zone	st Cas 5, 10	e ft2)		-29.
	**************************************	Wor	st Cas 5, 10 age D	e ft2) oor	21.8 19.5 18.5	-29. -22.

WIND LOADS BED ELOPIDA PLILI DING CODE 2004 DECIDENTIAL OFFICIAL DOCADO

REVISIONS

INDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

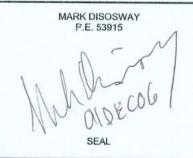
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portions of the plan, relating to wind engineering comply with section R301.2.1, florida building

ode residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.



BEN MARTIN

BENJAMIN MODEL SPEC HOUSE

ADDRESS:

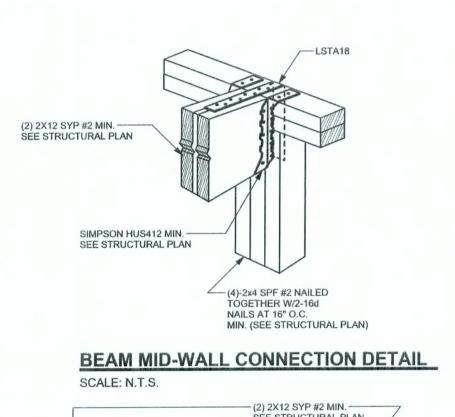
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PRINTED DATE: November 30, 2006 DRAWN BY: STRUCTURAL B Ben Sparks Ben Sparks

FINALS DATE: 13 / Nov / 06 JOB NUMBER:

611103 DRAWING NUMBER

OF 5 SHEETS



(1) 2x6 @ 16" OC

(1) 2x6 @ 12" OC

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

TO 18'-10' STUD HEIGHT

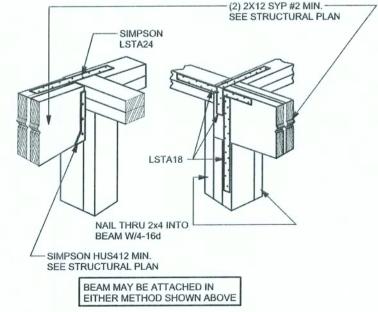
TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,

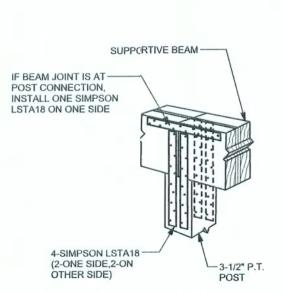
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.



BEAM CORNER CONNECTION. DETAIL SCALE: N.T.S.



SUPPORTIVE . BEAMS

3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON-OPPOSITE SIDE) EA.

NAILED WITH 14-10d

SCALE: N.T.S.

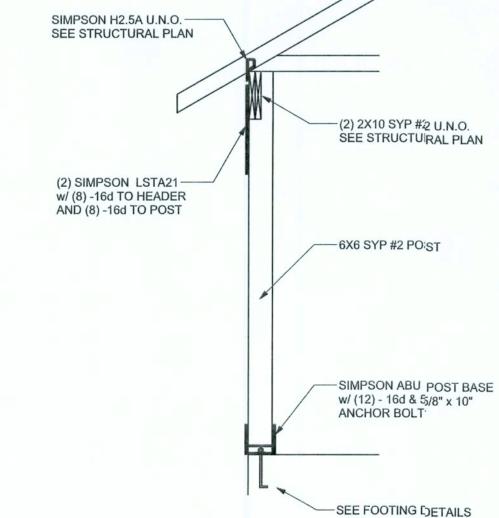
SUPPORTIVE CENTER POST TO BEAM DETAIL SCALE: N.T.S.

SUPPORTIVE POST TO BEAM

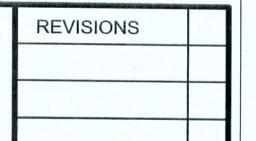
DETAIL FOR SINGLE BEAM

2X4 LADDER BEAM

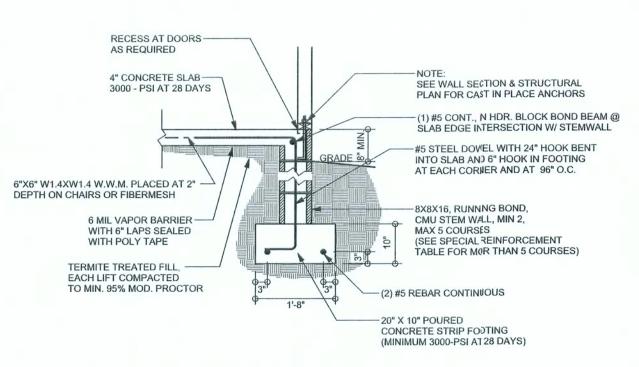
-SUPPORTIVE



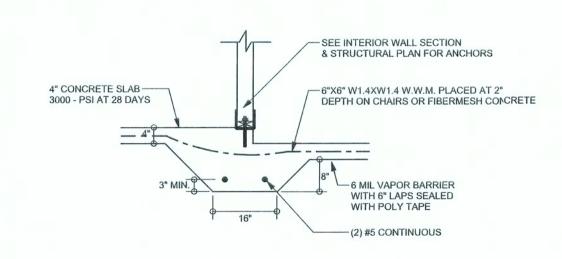
TYPICAL PORCH POST DETAIL SCALE: 1/2" = 1'-0"



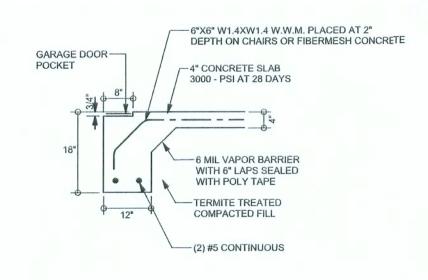




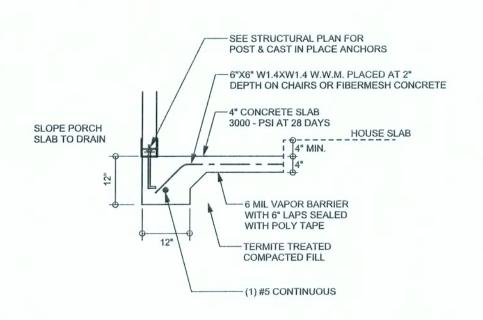




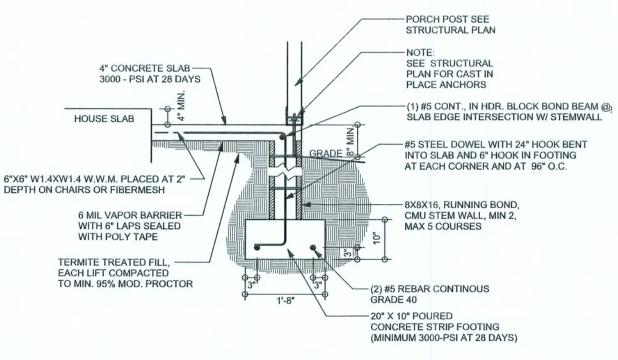
F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"





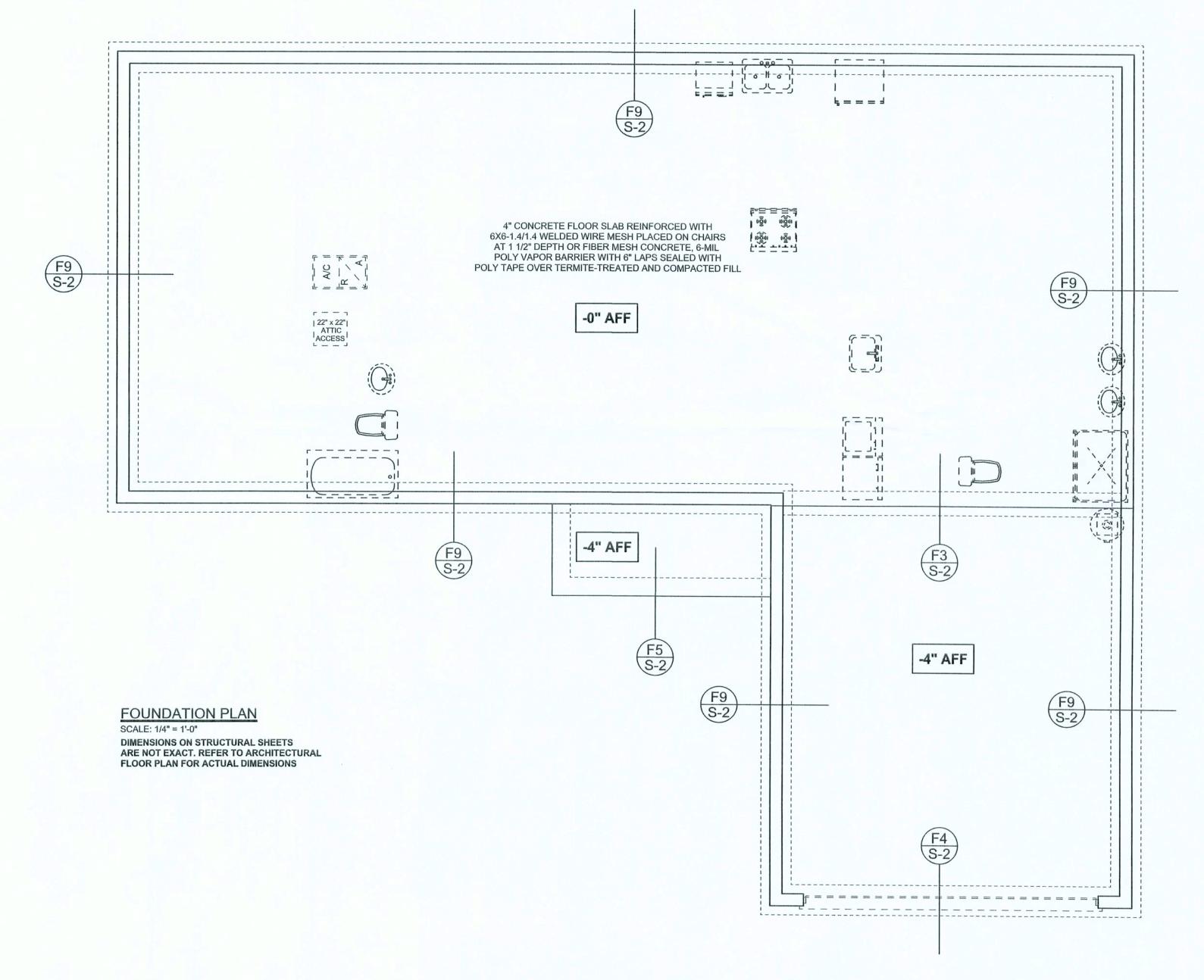


F12 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	EIGHT BACKFILL		VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	96	
5.3	5.0	56	96	96	96	96	96	
6.0	5.7	40	80	96	80	96	96	
6.7	6.3	32	56	80	56	96	96	
7.3	7.0	24	40	56	40	80	96	
8.0	7.7	16	32	48	32	64	80	
8.7	8.3	8	24	32	24	48	64	
9.3	9.0	8	16	24	16	40	48	



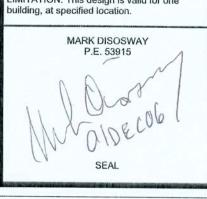
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419



BEN MARTIN

BENJAMIN MODEL SPEC HOUSE

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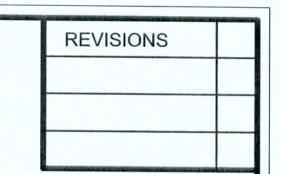
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Ben Sparks Ben Sparks

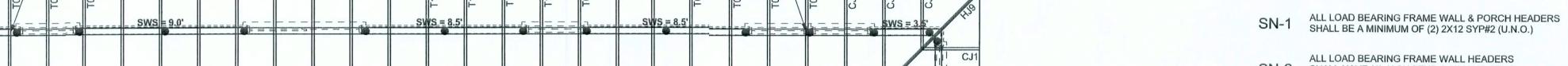
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JOB NUMBER: 611103

S-2OF 5 SHEETS

DRAWING NUMBER





UPLIFT

(2) 1.75 k9.5 X16 ,1J 2K

UPLIFT

943 LB — UPLIFT

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

STRUCTURAL PLAN NOTES

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD - INDICATES LOCATION OF:

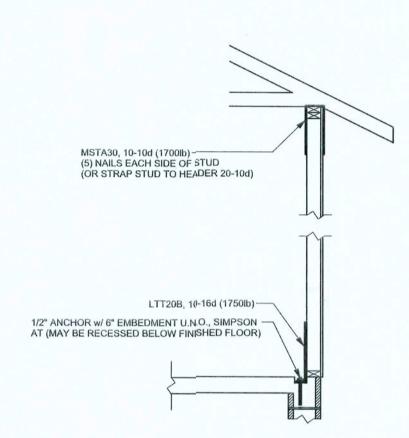
2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) ----NUMBER OF KING STUDS (FULL LENGTH) NUMBER OF JACK STUDS (UNDER HEADER) -----SPAN OF HEADER SIZE OF HEADER MATERIAL ----NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS REQUIRED ACTUAL TRANSVERSE 31.5' 83.5' LONGITUDINAL 16.5.1' 51.5'



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL SCALE: 1/2" = 1'-0"

> WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS:

Mark Disosway, P.E. hereby expressly reserve its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. MARK DISOSWAY P.E. 53915

BEN MARTIN

BENJAMIN MODEL SPEC HOUSE

ADDRESS:

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: November 30, 2006 DRAWN BY: STRUCTURAL BY Ben Sparks Ben Sparks

FINALS DATE: 13 / Nov / 06

CONNECTIONS, WALL, & HEADER DESIGN IS BASED

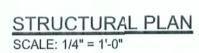
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE

JOB #L218303

ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING

JOB NUMBER: 611103 DRAWING NUMBER

OF 5 SHEETS



---979LB UPLIFT

USE H2.5A (48016) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM

CONNECTIONS UNLESS NOTED OTHERWISE