

Project Information for:

L246428

Builder:

Prudential Builders

Lot:

11

Subdivision:

Hunnington Place

County:

Columbia

Truss Count:

36

Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002
Truss Design Load Information:

Gravity:

Wind

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

Justin Fitzhugh Florida Contractor License No. CRC1328401

Address: P.O. Box 3333 Lake City, Florida

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Drwg. #

J1870334

J1870335

J1870336

J1870337

J1870338

J1870339

J1870340

J1870341 V9G

Truss ID

T16

V12

V4

V₆

V8

T16G

V15G

Date

7/27/07

7/27/07

7/27/07

7/27/07

7/27/07

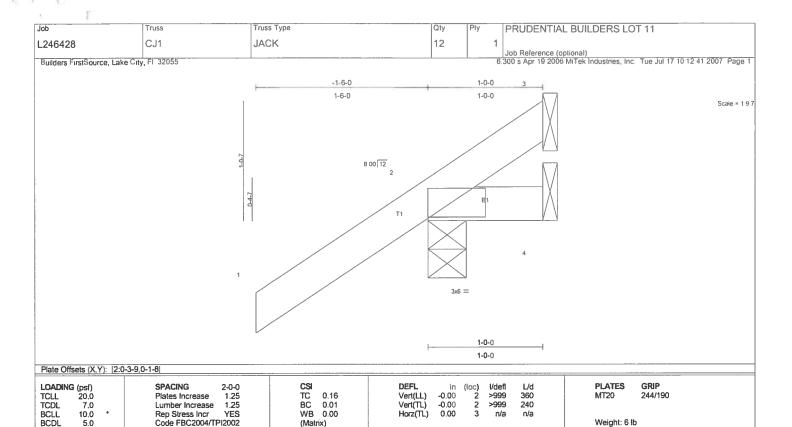
7/27/07

7/27/07

7/27/07

No.	Drwg. #	Truss ID	Date	No
1	J1870306	CJ1	7/27/07	29
2	J1870307	CJ3	7/27/07	30
3	J1870308	CJ5	7/27/07	31
4	J1870309	EJ5	7/27/07	32
5	J1870310	EJ7	7/27/07	33
6	J1870311	EJ7A	7/27/07	34
7	J1870312	HJ2	7/27/07	35
8	J1870313	HJ8	7/27/07	36
9	J1870314	HJ9	7/27/07	
10	J1870315	PB01	7/27/07	
11	J1870316	PB02	7/27/07	
12	J1870317	PB03	7/27/07	
13	J1870318	T01	7/27/07	
14	J1870319	T02	7/27/07	
15	J1870320	T03	7/27/07	
16	J1870321	T04	7/27/07	
17	J1870322	T05	7/27/07	
18	J1870323	T06	7/27/07	
19	J1870324	T07	7/27/07	
20	J1870325	T08	7/27/07	
21	J1870326	T09	7/27/07	
22	J1870327	T09A	7/27/07	
23	J1870328	T10	7/27/07	
24	J1870329	T11	7/27/07	
25	J1870330	T12	7/27/07	
26	J1870331	T13	7/27/07	
27	J1870332	T14	7/27/07	
28	J1870333	T15	7/27/07	





5.0

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 6 lb

REACTIONS (lb/size) 2=180/0-4-0, 4=5/Mechanical, 3=-41/Mechanical

Max Horz 2=94(load case 6)
Max Uplift2=-201(load case 6), 4=-11(load case 4), 3=-41(load case 1)
Max Grav 2=180(load case 1), 4=14(load case 2), 3=70(load case 6)

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/44, 2-3=-55/48 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Extenor(2) zone; porch left and right exposed; Lumber

- DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 2) "This truss has been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.

 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 201 lb uplift at joint 2, 11 lb uplift at joint 4 and 41 lb uplift at joint 3.

Ε Job Truss Truss Type PRUDENTIAL BUILDERS LOT 11 Qty L246428 CJ3 JACK 10 Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 12 45 2007 Page 1 Builders FirstSource, Lake City, Fl 32055 -1-6-0 3-0-0 1-6-0 3-0-0 3 Scale 3/4"=1 8 00 12 В1 3-0-0 3-0-0 Plate Offsets (X,Y): [2:0-3-9,0-1-8] LOADING (psf) in (loc) .01 2-4 .01 2-4 .00 3 SPACING 2-0-0 1.25 CSI TC BC WB DEFL I/defi **PLATES** GRIP TCLL TCDL 20.0 Plates Increase 0.01 360 244/190 0.18 Vert(LL) >999 MT20 -0.01 -0.00 7.0 Lumber Increase 1.25 0.08 Vert(TL) >999 240 BCLL 10.0 Rep Stress Incr YES 0.00 Horz(TL) n/a n/a Code FBC2004/TPI2002 **BCDL** 5.0 Weight: 13 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=206/0-4-0, 4=14/Mechanical

Max Horz 2=154(load case 6) Max Uplif13=47(load case 6), 2=177(load case 6), 4=-33(load case 4) Max Grav 3=48(load case 1), 2=206(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/45, 2-3=-60/19 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3, 177 lb uplift at joint 2 and 33 lb uplift at joint 4.

Truss Type PRUDENTIAL BUILDERS LOT 11 Job Truss JACK 10 CJ5 L246428 1 | Job Reference (optional) | 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10:12 49 2007 | Page 1 Builders FirstSource, Lake City, FI 32055 -1-6-0 5-0-0 1-6-0 5-0-0 Scale ≈ 1.22.5 8 00 12 3,45 ≂ 5-0-0 5-0-0 Plate Offsets (X,Y): [2:0-3-9,0-1-8] LOADING (psf) CSI TC BC SPACING PLATES 2-0-0 (loc) 2-4 I/defl 1.25 1.25 >671 TCLL TCDL 20.0 0.22 0.24 Vert(LL) Vert(TL) 244/190 Plates Increase 0.09 360 MT20

BCLL BCDL LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

10.0

P

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 19 lb

-0.05

-0.00

>999

240

REACTIONS (Ib/size) 3=113/Mechanical, 2=258/0-4-0, 4=24/Mechanical Max Horz 2=215(load case 6)

Lumber Increase

Rep Stress Incr YES Code FBC2004/TPI2002

Max Upini3=121(load case 6), 2=-198(load case 6), 4=-56(load case 4) Max Grav 3=113(load case 1), 2=258(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/45, 2-3=-100/50 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.15

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 3, 198 lb uplift at joint 2 and 56 lb uplift at joint 4.

WB 0.00

(Matrix)

Job Truss Type Qty PRUDENTIAL BUILDERS LOT 11 5 EJ5 MONO TRUSS L246428 1 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 12 53 2007 Page 1 Builders FirstSource, Lake City, FI 32055 -1-6-0 5-8-0 1-6-0 5-8-0 Scale = 1 27 0 8 00 12 5-8-0 5-8-0 Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING	G (psf)	SPACING 2-0-0	CSI	DEFL in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plates Increase 1.25	TC 0.27	Vert(LL) 0.12 2-6 >518 360 MT20 244/190	
TCDL	7.0	Lumber Increase 1.25	BC 0.28	Vert(TL) -0.07 2-6 >932 240	
BCLL	10.0 *	Rep Stress Incr YES	WB 0.04	Horz(TL) 0.00 n/a n/a	
BCDL	5.0	Code FBC2004/TPI2002	(Matrix)	Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

Structural wood sheathing directly applied or 5-8-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. TOP CHORD **BOT CHORD**

REACTIONS (ib/size) 2=271/0-4-0, 6=159/0-4-0 Max Horz 2=237(load case 6) Max Uplif2=-198(load case 6), 6=-199(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/45, 2-3=-112/56, 3-4=-2/0 BOT CHORD 2-6=0/0, 5-6=0/0

TOP CHORD BOT CHORD WEBS 3-6=-130/151

JOINT STRESS INDEX

2 = 0.15, 3 = 0.08 and 6 = 0.09

NOTES

- NOTES

 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for a 10.0 psf bottom chord live load noncoursent with any other live loads.
 2) *This truss has been designed for a 10.0 psf bottom chord live load noncoursent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 198 lb uplift at joint 2 and 199 lb uplift at joint 6.

PRUDENTIAL BUILDERS LOT 11 Joh russ Truss Type Qty EJ7 13 L246428 MONO TRUSS 1 Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 12 56 2007 Page 1 Builders FirstSource, Lake City, FI 32055 -1-6-0 7-0-0 1-6-0 7-0-0 Scale = 1:28 9 Camber = 1/16 in 8.00 12 0.4.7 7-0-0 7-0-0 Plate Offsets (X,Y): [2:0-0-12,Edge] CSI TC 0.41 BC 0.29 WB 0.00 LOADING (psf) TCLL 20.0 2-0-0 1.25 in 0.09 (loc) 2-4 l/defl >877 L/d 360 **SPACING** DEFL **PLATES** GRIP 244/190 Plates Increase Vert(LL) MT20 TCDL Lumber Increase 1.25 Vert(TL) -0.17 >479 240 BCLL BCDL Rep Stress Incr YES Code FBC2004/TPI2002 -0.00 10.0 YES Horz(TL) 3 n/a n/a

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

5.0

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 26 lb

REACTIONS (lb/size) 3=157/Mechanical, 2=318/0-4-0, 4=49/Mechanical

Max Horz 2=198(load case 6)

Max Uplifi3=-106(load case 6), 2=-90(load case 6)
Max Grav 3=157(load case 1), 2=318(load case 1), 4=95(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-133/68 BOT CHORD 2-4=0/0

JOINT STRESS INDEX 2 = 0.78

- NOME: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 106 lb uplift at joint 3 and 90 lb uplift at joint 2.

(Matrix)

Job Truss Type PRUDENTIAL BUILDERS LOT 11 5 **SPECIAL** 1246428 EJ7A Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 13:02 2007 Page 1 Builders FirstSource, Lake City, FI 32055 -1-6-0 2-4-0 7-0-0 1-6-0 4-8-0 Scale = 1 28 9 Camber = 1/16 in 8 00 12 D-4-Z 7-0-0 2-4-0 2-4-0 4-8-0 Plate Offsets (X,Y): [2:0-3-9,0-1-8], [3:0-4-9,0-0-14], [6:0-2-14,Edge LOADING (psf) CSI TC in (loc) 0.**1**6 5-6 SPACING DEFL. **PLATES** GRIP 1.25 1.25 244/190 0.32 Vert(LL) >516 360 MT20 TCLL 20.0 Plates Increase TCDL 7.0 Lumber Increase Vert(TL) -0.18 >455 240 BCLL BCDL 10.0 Rep Stress Incr YES Code FBC2004/TPI2002 WB 0.00 Horz(TL) 0.07 5 n/a n/a Weight: 28 lb (Matrix)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 "Except" B2 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=139/Mechanical, 2=318/0-4-0, 5=67/Mechanical

Max Horz 2=198(load case 6)
Max Uplift4=-88(load case 6), 2=-90(load case 6), 5=-11(load case 6) Max Grav 4=139(load case 1), 2=318(load case 1), 5=91(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/45, 2-3=-269/0, 3-4=-103/63

BOT CHORD 2-7=-142/168, 6-7=-1/50, 3-6=-1/101, 5-6=0/0

JOINT STRESS INDEX

2 = 0.34, 3 = 0.37, 6 = 0.75 and 7 = 0.45

NOTES

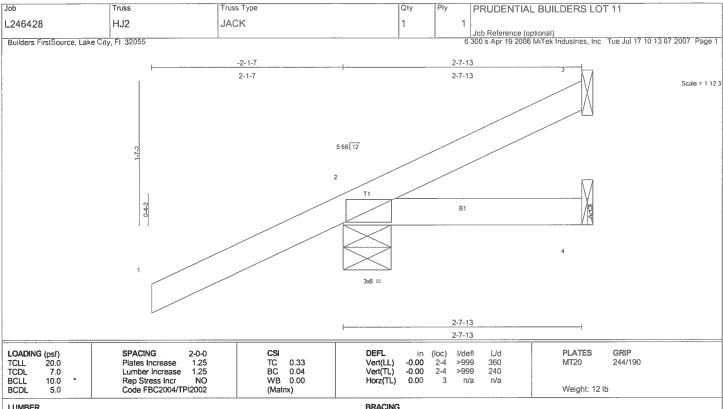
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is

- designed for C-C for members and forces, and for MWFRS for reactions specified.

 2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 4, 90 lb uplift at joint 2 and 11 lb uplift at joint



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-7-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 3=-66/Mechanical, 2=213/0-6-6, 4=35/Mechanical

Max Horz 2=49(load case 5)
Max Uplif13=-66(load case 1), 2=-219(load case 5), 4=-23(load case 3)
Max Grav 3=99(load case 3), 2=213(load case 1), 4=36(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/48, 2-3=-32/47 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.09

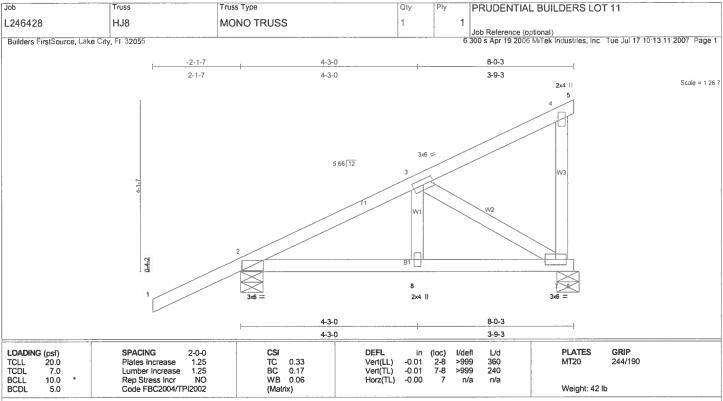
- NOTES

 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 3, 219 lb uplift at joint 2 and 23 lb uplift at joint 4.
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf) Vert: 2=0(F=5, B=5)-to-4=-45(F=-18, B=-18)



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=322/0-6-7, 2=314/0-6-7

Max Horz 2=244(load case 5)

Max Uplift7=-323(load case 5), 2=-261(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1.2=0/48, 2-3=-254/70, 3-4=-80/42, 4-5=-8/0, 4-7=-167/176
BOT CHORD 2-8=-216/205, 7-8=-216/205, 6-7=0/0
WEBS 3-8=-62/114, 3-7=-212/224

JOINT STRESS INDEX

2 = 0.61, 3 = 0.11, 4 = 0.62, 7 = 0.26 and 8 = 0.08

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

- 2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 ib uplift at joint 7 and 261 lb uplift at joint 2.
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

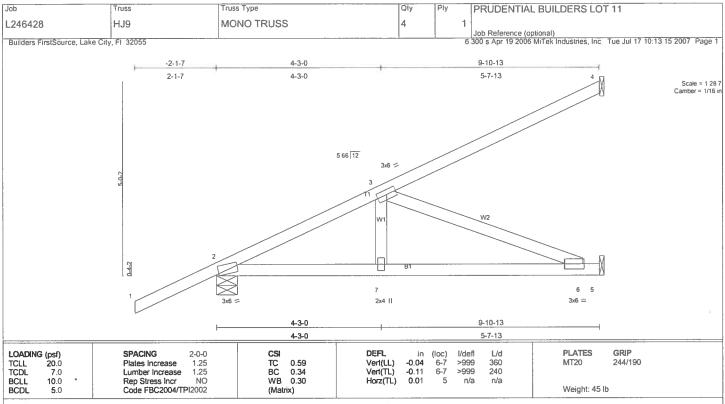
LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-106(F=-26, B=-26), 4=-66(F=-26, B=-26)-to-5=-70(F=-28, B=-28), 2=0(F=5, B=5)-to-6=-20(F=-5, B=-5)



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=264/Mechanical, 2=409/0-6-7, 5=231/Mechanical Max Horz 2=343(load case 5)

Max Uplift4=-266(load case 5), 2=-186(load case 5), 5=-96(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/48, 2-3=-552/57, 3-4=-149/82 BOT CHORD 2-7=-330/491, 6-7=-330/491, 5-6=0/0

WEBS 3-7=0/196, 3-6=-527/355

JOINT STRESS INDEX

2 = 0.85, 3 = 0.15, 6 = 0.14 and 7 = 0.14

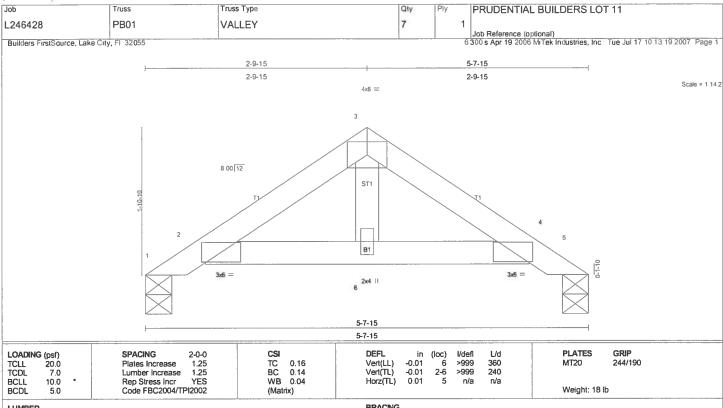
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60. 2) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 4, 186 lb uplift at joint 2 and 96 lb uplift at joint 5.
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3 BRACING

Structural wood sheathing directly applied or 5-7-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. TOP CHORD BOT CHORD

REACTIONS (lb/size) 1=171/0-4-0, 5=171/0-4-0 Max Horz 1=49(load case 5)

Max Uplift1=-35(load case 6), 5=-35(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=84/53, 2-3=-254/142, 3-4=-254/142, 4-5=-84/53 BOT CHORD 2-6=-69/205, 4-6=-69/205 TOP CHORD BOT CHORD

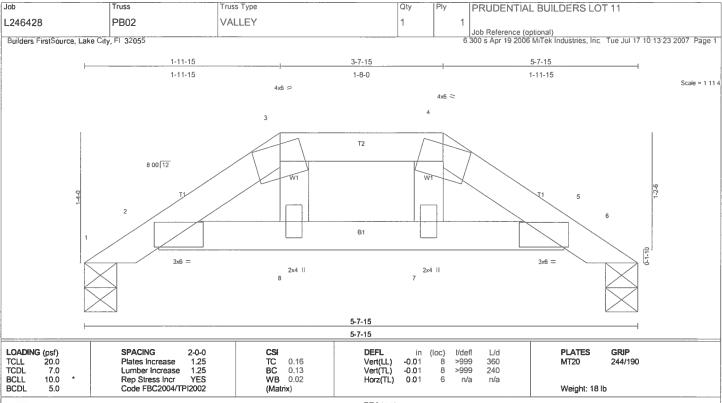
WEBS 3-6=-37/112

JOINT STRESS INDEX

2 = 0.40, 3 = 0.14, 4 = 0.40 and 6 = 0.08

- NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) All bearings are assumed to be SYP No.2 crushing capacity of 555.00 psi

- 4) All bealings at joint(s) 1,5 considers parallel to grain value using ANSUTPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1 and 35 lb uplift at joint 5.
 7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

1

BRACING TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-7-15 oc purlins Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=171/0-4-0, 6=171/0-4-0 Max Horz 1=34(load case 5)

Max Uplift1=-31(load case 6), 6=-31(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-84/62, 2-3=-292/205, 3-4=-251/204, 4-5=-292/205, 5-6=-84/62

BOT CHORD 2-8=-139/242, 7-8=-142/251, 5-7=-139/242 3-8=-17/67, 4-7=-17/67

WEBS

JOINT STRESS INDEX

2 = 0.35, 3 = 0.09, 4 = 0.09, 5 = 0.35, 7 = 0.05 and 8 = 0.05

- NOTES

 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 3) Provide adequate drainage to prevent water ponding.

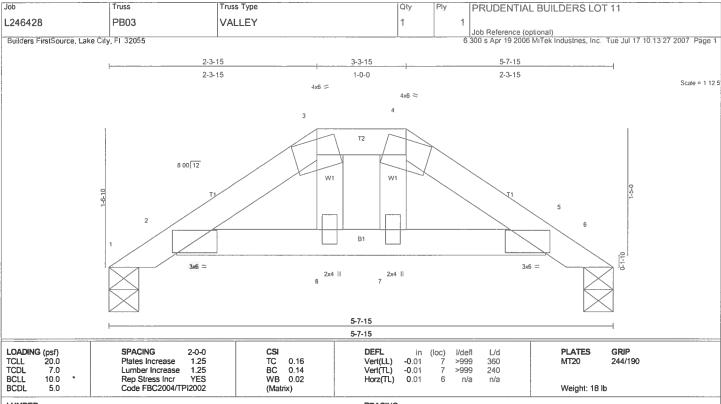
 4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 6) Bearing at joint(5) 1, 6 considers parallel to grain value using ANSUTPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1 and 31 lb uplift at joint 6.

 8) SEE MTEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-7-15 oc purlins Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=171/0-4-0, 6=171/0-4-0 Max Horz 1=40(load case 5)

Max Uplift1=-33(load case 6), 6=-33(load case 7)

FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-84/59, 2-3=-265/173, 3-4=-223/178, 4-5=-265/173, 5-6=-84/59
BOT CHORD 2-8=-102/216, 7-8=-104/223, 5-7=-102/216
WEBS 3-8=-18/59, 4-7=-18/59

JOINT STRESS INDEX

2 = 0.37, 3 = 0.09, 4 = 0.09, 5 = 0.37, 7 = 0.04 and 8 = 0.04

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

obesigned for C-C for members and orders, and for movers for reactions specified.

3) Provide adequate drainage to prevent water ponding.

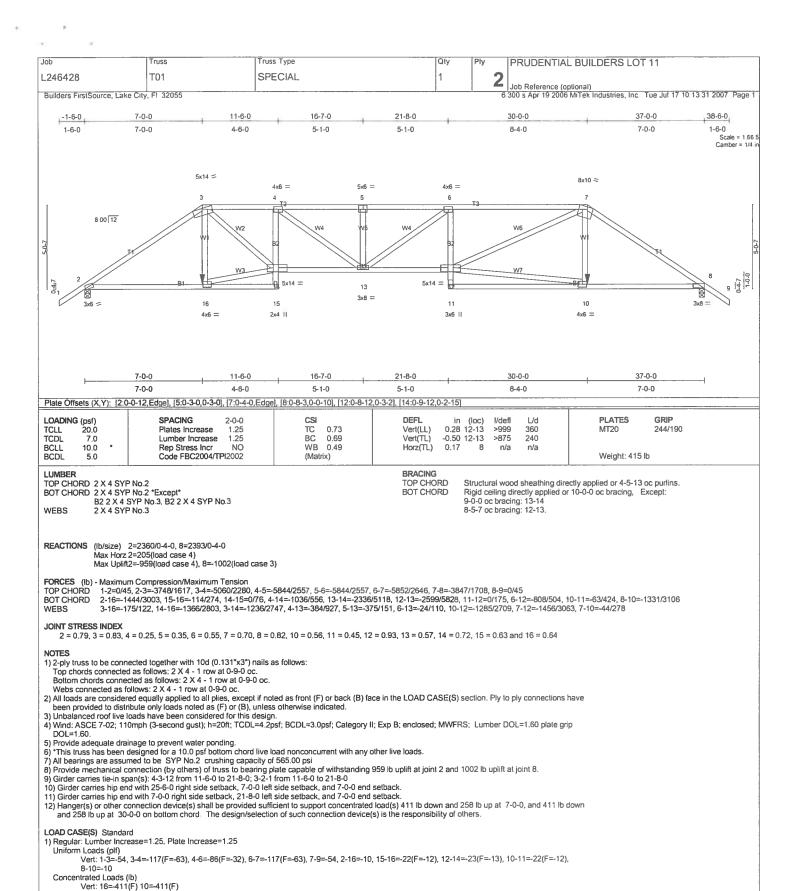
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

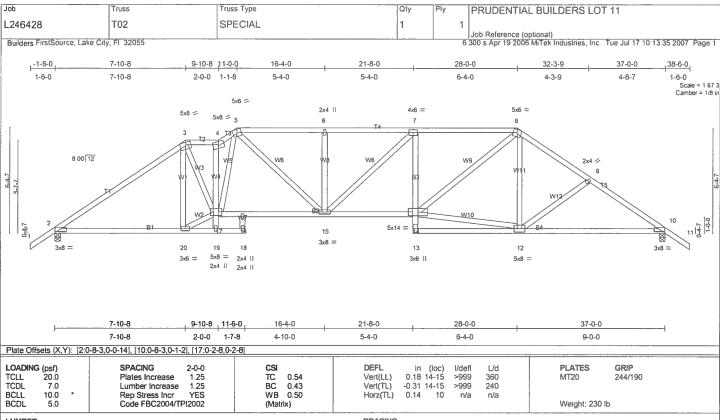
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 1 and 33 lb uplift at joint 6.

8) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS





TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except*

B3 2 X 4 SYP No.3 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-11-2 oc purlins. Rigid ceiling directly applied or 7-1-12 oc bracing.

REACTIONS (lb/size) 2=1263/0-4-0, 10=1263/0-4-0 Max Horz 2=-167(load case 4)

Max Uplift2=-283(load case 6), 10=-300(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1811/858, 3-4=-1868/972, 4-5=-2112/1089, 5-6=-2103/1126, 6-7=-2103/1126, 7-8=-2057/1110, 8-9=-1646/852, 9-10=-1836/888, 10-11=0/45 BOT CHORD

2-20=-514/1401, 19-20=-7/29, 18-19=0/0, 16-17=-638/1772, 15-16=-638/1772, 14-15=-785/2065, 13-14=0/79, 7-14=-405/240, 12-13=-40/120, 10-12=-580/1448
3-20=-526/231, 3-17=-373/1075, 17-19=-33/47, 4-17=-812/338, 5-15=-225/550, 6-15=-279/201, 7-15=-64/110, 12-14=-407/1215, 8-14=-439/980, 8-12=-85/147, 9-12=-162/171, 5-17=-198/480,

WEBS

17-20=-567/1529, 16-18=-13/15

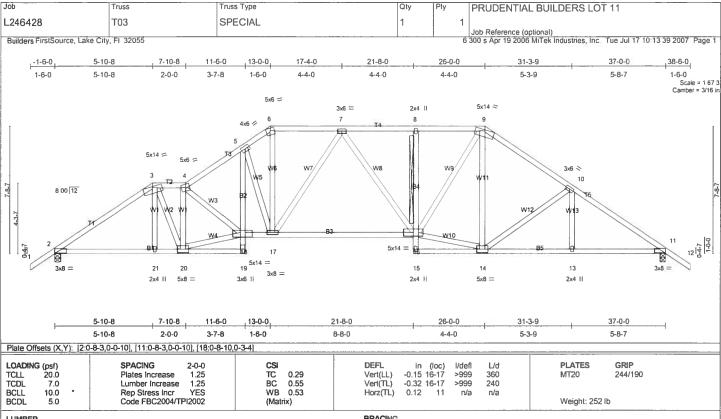
JOINT STRESS INDEX

2 = 0.70, 3 = 0.86, 4 = 0.87, 5 = 0.51, 6 = 0.34, 7 = 0.25, 8 = 0.54, 9 = 0.34, 10 = 0.73, 12 = 0.56, 13 = 0.27, 14 = 0.87, 15 = 0.57, 16 = 0.34, 17 = 0.93, 18 = 0.34, 19 = 0.34 and 20 = 0.88

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Extenor(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 283 lb uplift at joint 2 and 300 lb uplift at joint 10.



TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 "Except" B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 4-2-14 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. Except: T-Brace: 2 X 4 SYP No.3 - 8-16

REACTIONS (lb/size) 2=1263/0-4-0, 11=1263/0-4-0 Max Horz 2=204(load case 5)

Max Uplift2=-296(load case 6), 11=-280(load case 7)

FORCES (lb)

1-2=0145, 2-3=-1844/857, 3-4=-1742/915, 4-5=-2076/1047, 5-6=-1838/992, 6-7=-1549/863, 7-8=-1627/905, 8-9=-1618/903, 9-10=-1543/821, 10-11=-1851/856, 11-12=0/45 TOP CHORD BOT CHORD

2-21=-540/1443, 20-21=-539/1446, 19-20=-60/201, 18-19=0/77, 5-18=-266/478, 17-18=-579/1692, 16-17=-556/1665, 15-16=0/54, 8-16=-236/147, 14-15=-70/47, 13-14=-545/1452,

11-13=-545/1452

3-21=0/173, 3-20=-331/700, 4-20=-1125/515, 18-20=-663/1644, 4-18=-121/149, 5-17=-510/344, 6-17=-424/814, 7-17=-324/203, 7-16=-138/116, 14-16=-311/1201, 9-16=-285/772, **WEBS**

9-14=-140/128, 10-14=-296/226, 10-13=0/178

JOINT STRESS INDEX

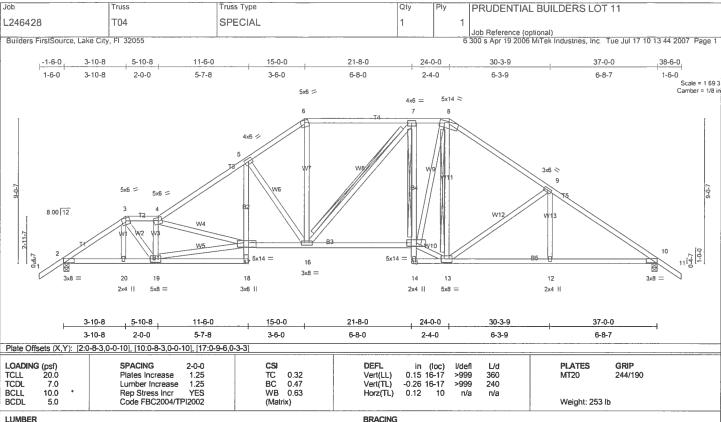
2 = 0.81, 3 = 0.97, 4 = 0.56, 5 = 0.46, 6 = 0.47, 7 = 0.43, 8 = 0.34, 9 = 0.80, 10 = 0.43, 11 = 0.81, 13 = 0.34, 14 = 0.58, 15 = 0.44, 16 = 0.51, 17 = 0.67, 18 = 0.73, 19 = 0.29, 20 = 0.81 and 21 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2 and 280 lb uplift at joint 11.



TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

WEBS

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-1-6 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. Except: T-Brace: 2 X 4 SYP No.3 - 7-15

T-Brace 2 X 4 SYP No.3 - 7-16, 8-13

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.

with 4in minimum end distance.

Brace must cover 90% of web length.

(lb/size) 2=1263/0-4-0, 10=1263/0-4-0 REACTIONS

Max Horz 2=241(load case 5)
Max Uplift2=-308(load case 6), 10=-293(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/45, 2-3=-1857/838, 3-4=-2078/1031, 4-5=-2139/1017, 5-6=-1663/894, 6-7=-1360/801, 7-8=-1340/793, 8-9=-1456/783, 9-10=-1834/837, 10-11=0/45 2-20=-545/1460, 19-20=-546/1458, 18-19=-54/202, 17-18=0/100, 5-17=-216/517, 16-17=-572/1710, 15-16=-357/1355, 14-15=-13/7, 7-15=-370/260, 13-14=-65/71, 12-13=-517/1433, TOP CHORD BOT CHORD

10-12=-517/1433 3-20=0/71, 3-19=-535/1080, 4-19=-1235/633, 17-19=-847/1976, 4-17=-449/319, 5-16=-620/378, 6-16=-285/590, 7-16=-157/155, 13-15=-241/1169, 8-15=-296/829, 8-13=-273/141,

9-13=-381/288, 9-12=0/213

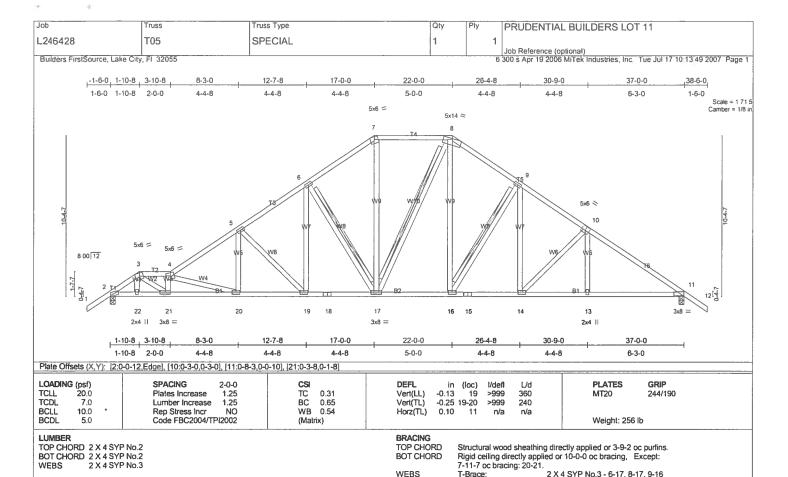
JOINT STRESS INDEX

2 = 0.81, 3 = 0.46, 4 = 0.59, 5 = 0.59, 6 = 0.56, 7 = 0.27, 8 = 0.95, 9 = 0.43, 10 = 0.81, 12 = 0.34, 13 = 0.54, 14 = 0.34, 15 = 0.63, 16 = 0.58, 17 = 0.62, 18 = 0.44, 19 = 0.92 and 20 = 0.34

WEBS

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 293 lb uplift at joint 10.



REACTIONS (lb/size) 2=1263/0-4-0, 11=1263/0-4-0

Max Horz 2=-278(load case 3)

Max Uplift2=-317(load case 5), 11=-303(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/45, 2-3=-178/7303, 3-4=-2756/543, 4-5=-2157/427, 5-6=-1696/373, 6-7=-1337/338, 7-8=-1067/319, 8-9=-1299/322, 9-10=-1551/339, 10-11=-1836/332, 11-12=0/45 2-22=-362/1389, 21-22=-363/1389, 20-21=-633/2864, 19-20=-368/1766, 18-19=-255/1353, 17-18=-255/1353, 16-17=-99/1035, 15-16=-116/1228, 14-15=-116/1228, 13-14=-168/1432, 14-15=-168/1432, 14-15=-168/1432, 14-15=-168/1432, 14-15=-168/1432, 14-15=-168/1432, 14-15=-168/1432, 14-168/1432, 14-15=-168/1432, 14-15=-168/1432, 14-15=-168/1432, 14-1 BOT CHORD

11-13=-168/1433

3-22=3/35, 3-21=-361/1675, 4-21=-982/240, 4-20=-1147/277, 5-20=-57/397, 5-19=-574/198, 6-19=-110/434, 6-17=-547/233, 7-17=-119/448, 8-17=-159/213, 8-16=-136/361, 9-16=-371/195, WEBS

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.

,with 4in minimum end distance. Brace must cover 90% of web length.

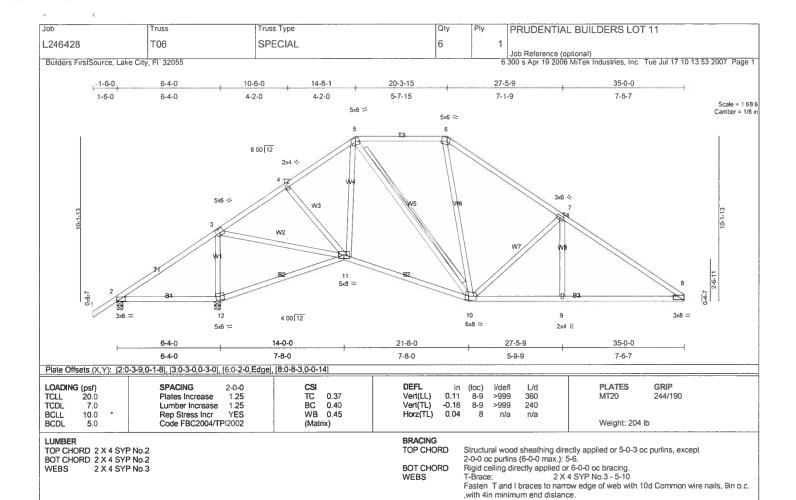
9-14=-77/261, 10-14=-294/147, 10-13=0/176

JOINT STRESS INDEX

2 = 0.77, 3 = 0.52, 4 = 0.58, 5 = 0.43, 6 = 0.43, 7 = 0.38, 8 = 0.83, 9 = 0.43, 10 = 0.37, 11 = 0.81, 13 = 0.34, 14 = 0.37, 15 = 0.41, 16 = 0.44, 17 = 0.63, 18 = 0.47, 19 = 0.37, 20 = 0.35, 21 = 0.75 and 22 =

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) All plates are 3x6 MT20 unless otherwise indicated.

- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 2 and 303 lb uplift at joint 11.



Brace must cover 90% of web length

REACTIONS (lb/size) 8=867/Mechanical, 2=77/0-4-0, 12=1371/0-4-0

Max Horz 2=291(load case 5)
Max Uplift8=-222(load case 7), 2=-182(load case 4), 12=-350(load case 6)
Max Grav 8=867(load case 1), 2=169(load case 10), 12=1371(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/45, 2-3=-187/419, 3-4=-936/460, 4-5=-797/465, 5-6=-647/538, 6-7=-908/557, 7-8=-1303/608 2-12=-264/154, 11-12=-316/186, 10-11=-150/653, 9-10=-386/995, 8-9=-386/995 TOP CHORD BOT CHORD

3-12=-1184/567, 3-11=-289/1024, 4-11=-173/165, 5-11=-42/179, 5-10=-219/170, 7-10=-446/358, 7-9=0/216, 6-10=-61/218

JOINT STRESS INDEX

2 = 0.60, 3 = 0.60, 4 = 0.34, 5 = 0.36, 6 = 0.69, 7 = 0.43, 8 = 0.65, 9 = 0.34, 10 = 0.60, 11 = 0.96 and 12 = 0.58

NOTES

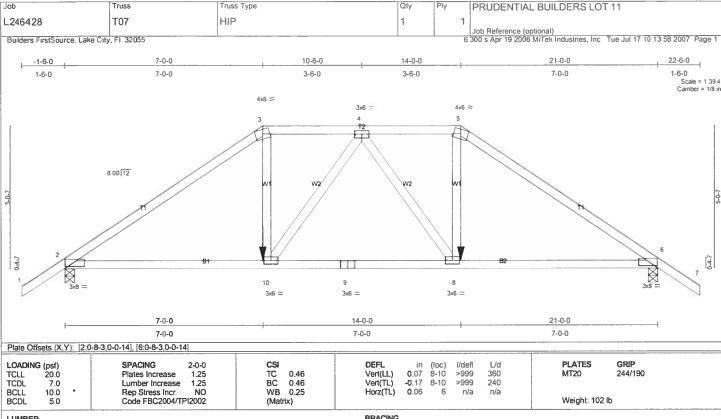
Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 8, 182 lb uplift at joint 2 and 350 lb uplift at joint 12.

 7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 BRACING

Structural wood sheathing directly applied or 4-0-2 oc purlins. Rigid ceiling directly applied or 6-10-1 oc bracing. TOP CHORD

BOT CHORD

REACTIONS (lb/size) 2=1422/0-4-0, 6=1422/0-4-0

Max Horz 2=-130(load case 3)
Max Uplift2=-618(load case 5), 6=-618(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-2105/907, 3-4=-1672/807, 4-5=-1672/807, 5-6=-2105/907, 6-7=0/45
BOT CHORD 2-10=-784/1650, 9-10=-852/1780, 6-9=-852/1780, 6-8=-681/1650

BOT CHORD 3-10=-398/768, 4-10=-300/276, 4-8=-300/276, 5-8=-398/768 WEBS

JOINT STRESS INDEX

2 = 0.78, 3 = 0.78, 4 = 0.42, 5 = 0.78, 6 = 0.78, 8 = 0.50, 9 = 0.68 and 10 = 0.50

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gusl); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 618 lb uplift at joint 2 and 618 lb uplift at joint 6.

7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 258 lb up at 14-0-0, and 411 lb down.

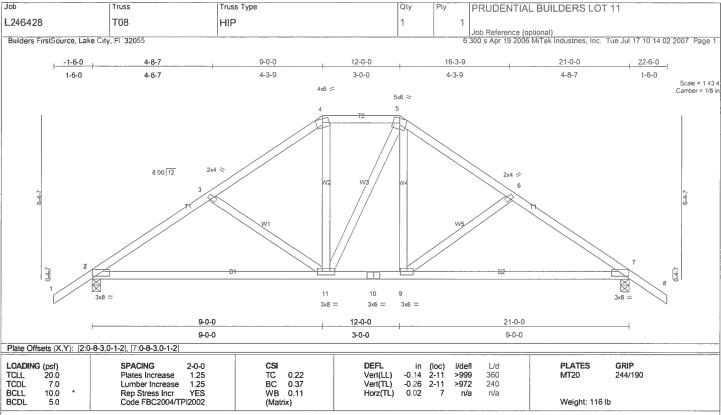
and 258 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-10=-10, 8-10=-22(F=-12), 6-8=-10

Concentrated Loads (lb) Vert: 10=-411(F) 8=-411(F)



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. TOP CHORD BOT CHORD

REACTIONS (lb/size) 2=751/0-4-0, 7=751/0-4-0

Max Horz 2=-167(load case 4)
Max Uplift2=-206(load case 6), 7=-206(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/45, 2-3=-952/440, 3-4=-738/387, 4-5=-559/377, 5-6=-746/392, 6-7=-953/439, 7-8=0/45 2-11=-215/730, 10-11=-51/557, 9-10=-51/557, 7-9=-215/730 TOP CHORD

BOT CHORD

WEBS 3-11=-214/201, 4-11=-76/226, 5-11=-103/107, 5-9=-81/221, 6-9=-211/200

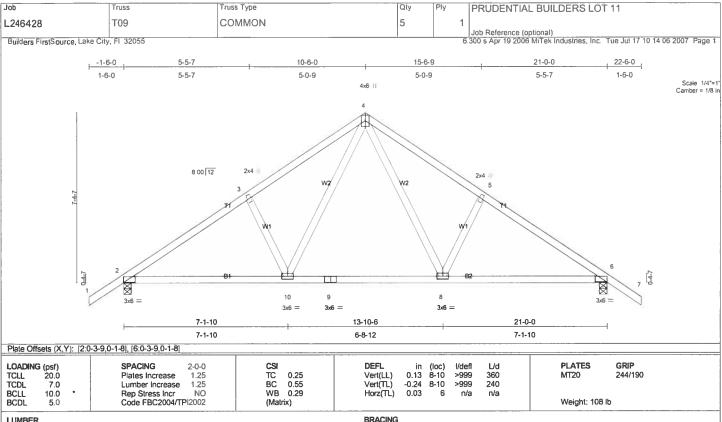
JOINT STRESS INDEX

2 = 0.73, 3 = 0.34, 4 = 0.30, 5 = 0.33, 6 = 0.34, 7 = 0.69, 9 = 0.35, 10 = 0.44 and 11 = 0.63

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 2 and 206 lb uplift at joint 7.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-5-11 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=920/0-4-0, 6=920/0-4-0

Max Horz 2=194(load case 5)

Max Uplift2=-258(load case 6), 6=-258(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD
BOT CHORD
1.2=0/45, 2-3=-1272/557, 3-4=-1142/613, 4-5=-1142/613, 5-6=-1272/557, 6-7=0/45
BOT CHORD
2.10=-300/977, 9-10=-90/672, 6-9=-90/672, 6-8=-300/977
WEBS
3.10=-222/218, 4-10=-268/515, 4-8=-268/515, 5-8=-222/218

JOINT STRESS INDEX

2 = 0.66, 3 = 0.34, 4 = 0.52, 5 = 0.34, 6 = 0.66, 8 = 0.46, 9 = 0.47 and 10 = 0.46

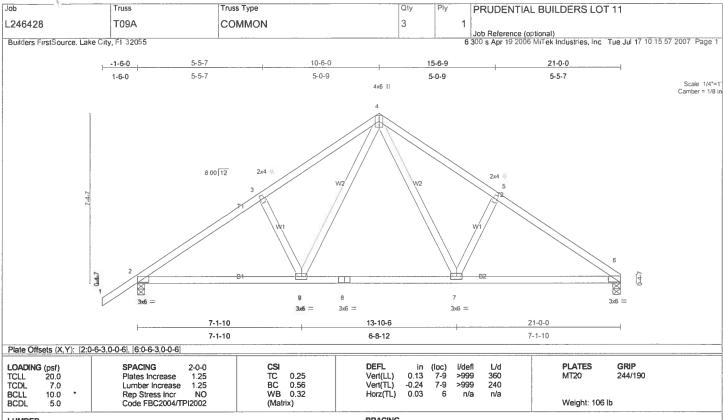
- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 2 and 258 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-60(F=-50), 6-8=-10



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-4-12 oc purlins. Rigid ceiling directly applied or 9-9-11 oc bracing.

REACTIONS (lb/size) 6=826/0-4-0, 2=923/0-4-0 Max Horz 2=214(load case 5)

Max Uplift6=-184(load case 7), 2=-259(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/45, 2-3=-1278/565, 3-4=-1148/622, 4-5=-1160/639, 5-6=-1288/581

BOT CHORD 2-9=-371/982, 8-9=-162/678, 7-8=-162/678, 6-7=-390/995 3-9=-222/219, 4-9=-265/514, 4-7=-296/534, 5-7=-232/233 WERS

JOINT STRESS INDEX

2 = 0.68, 3 = 0.34, 4 = 0.51, 5 = 0.34, 6 = 0.68, 7 = 0.46, 8 = 0.47 and 9 = 0.46

NOTES

- 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for reactions specified.

 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) All bearings are assumed to be \$14 NO.2 crushing capacity of 863.00 psi 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint 6 and 259 lb uplift at joint 2. 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (pif)

Vert: 1-4=-54, 4-6=-54, 2-9=-10, 7-9=-60(F=-50), 6-7=-10

Truss Type PRUDENTIAL BUILDERS LOT 11 Job Truss T10 HIP L246428 Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 14:17 2007 Page 1 Builders FirstSource, Lake City, Fl 32055 35-0-0 36-6-0 13-10-11 28-4-0 -1-6-0 21-1-5 6-8-0 1-6-0 6-8-0 7-2-11 7-2-11 7-2-11 6-8-0 1-6-0 5x8 = 5x6 = 2x4 || 8 00 12 Ø × 13 5x8 = 2x4 \\ 3x6 = 3x6 == 2x4 // 21-1-5 29-8-0 29-1,0-0 35-0-0 13-10-11 5-4-0 6-2-0 5-4-0 0-10-0 7-8-11 7-2-11 8-6-11 0-2-0 5-2-0 Plate Offsets (X,Y): [2:0-6-3,0-0-6], [4:0-3-0,0-3-0], [7:0-3-9,0-1-8] LOADING (psf) DEFL in (loc) 0.11 10-12 **PLATES** SPACING 1/defl L/d TC BC >999 360 MT20 244/190 0.41 Vert(LL) TCLL 20.0 Plates Increase 1.25 Lumber Increase 1.25 0.46 Vert(TL) -0.18 9-10 >999 240 WB 0.03 0.79 n/a n/a Horz(TL) BCLL 10.0 Rep Stress Incr YES Weight: 176 lb 5.0 Code FBC2004/TPI2002 (Matrix) LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 5-8-5 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. T-Brace: 2 X 4 SYP No.3 - 4-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. .with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (Ib/size) 7=-56/0-4-0, 9=1388/0-4-0, 2=650/0-4-0, 13=416/0-4-0 Max Horz 2=-124(load case 4)

Max Uplift?=-245(load case 10), 9=-530(load case 4), 2=-361(load case 5), 13=-75(load case 5) Max Grav 7=83(load case 5), 9=1388(load case 1), 2=650(load case 1), 13=416(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

6-7=-296/644, 7-8=0/45, 3-4=-1105/802, 4-5=-909/628, 5-6=-909/628, 1-2=0/45, 2-3=-721/679
2-14=-392/521, 13-14=-394/625, 12-13=-394/625, 11-12=-547/1112, 10-11=-547/1112, 9-10=-83/153, 7-9=-443/385
6-9=-1349/794, 6-10=-614/1162, 5-10=-400/267, 4-10=-241/213, 4-12=-296/175, 3-12=-230/647, 3-14=-356/21 BOT CHORD WEBS

JOINT STRESS INDEX

2 = 0.65, 3 = 0.57, 4 = 0.53, 5 = 0.34, 6 = 0.62, 7 = 0.66, 9 = 0.51, 10 = 0.54, 11 = 0.40, 12 = 0.38 and 14 = 0.34

1) Unbalanced roof live loads have been considered for this design.

3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 7, 530 lb uplift at joint 9, 361 lb uplift at joint 2 and 75 lb uptift at joint 13.

Job Truss Truss Type Qty PRUDENTIAL BUILDERS LOT 11 HIP L246428 T11 | Job Reference (optional) | 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 14 21 2007 | Page 1 Builders FirstSource, Lake City, Fl 32055 -1-6-0 14-6-11 30-5-9 35-0-0 36-6-0 1-6-0 Scale = 1 62 3 Camber = 1/8 in 1-6-0 4-6-7 4-1-9 5-10-11 5-10-11 5-10-11 4-1-9 4-6-7 5x6 = 2x4 II 5x6 > 8 00 12 2x4 \\ 16 15 13 12 11 3x8 = 6-2-0 14-6-11 20-5-5 28-10-0 29-10-0 35-0-0 6-2-0 8-4-11 5-10-11 8-4-11 1-0-0 5-2-0 Plate Offsets (X,Y): [2:0-6-3,0-0-6], [9:0-3-9,0-1-8] LOADING (psf) **SPACING** DEFL **PLATES** 2-0-0 in (loc) 1/defl GRIP TCLL TCDL TC BC 20.0 Plates Increase 1.25 0.29 Vert(LL) -0.12 12-13 >999 MT20 244/190 7.0 1.25 -0.25 12-13 Lumber Increase 0.45 Vert(TL) >999 240 BCLL Rep Stress Incr YES Code FBC2004/TPI2002 10.0 WB 0.82 Horz(TL) n/a n/a 5.0 (Matrix) Weight: 195 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: **BOT CHORD** 6-0-0 oc bracing: 2-16. T-Brace: 2 X 4 SYP No.3 - 6-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. **WEBS** with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=85/0-4-0, 16=1300/0-4-0, 9=738/0-4-0, 11=276/0-4-0

Max Horz 2=161(load case 5)

Max Uplift2=-167(load case 6), 16=-461(load case 5), 9=-229(load case 7), 11=-223(load case 4) Max Grav 2=109(load case 10), 16=1300(load case 1), 9=738(load case 1), 11=276(load case 1)

FORCES ((b) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-123/316, 3-4=-93/417, 4-5=-737/478, 5-6=-737/478, 6-7=-929/560, 7-8=-882/416, 8-9=-1003/380, 9-10=0/45

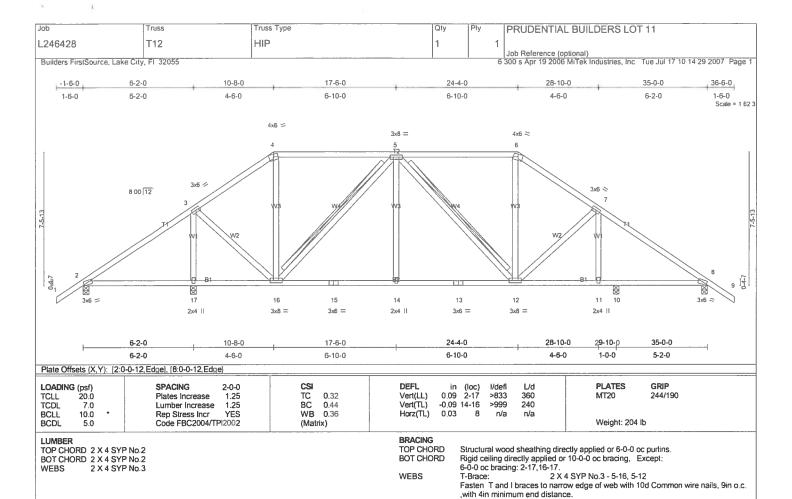
2-16=-245/190, 15-16=-86/165, 14-15=-242/929, 13-14=-242/929, 12-13=-120/708, 11-12=-173/757, 9-11=-173/757, 3-16=-191/215, 4-16=-1138/496, 4-15=-356/827, 5-15=-330/231, 6-15=-272/115, 6-13=-179/168, 7-13=-188/375, 7-12=-120/194, 8-12=-140/182 BOT CHORD WEBS

JOINT STRESS INDEX

 $2 = 0.63, \ 3 = 0.34, \ 4 = 0.43, \ 5 = 0.34, \ 6 = 0.37, \ 7 = 0.41, \ 8 = 0.34, \ 9 = 0.57, \ 12 = 0.47, \ 13 = 0.37, \ 14 = 0.36, \ 15 = 0.82 \ \text{and} \ 16 = 0.49 \ \text{and}$

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) All plates are 3x6 MT20 unless otherwise indicated.

- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 167 lb uplift at joint 2, 461 lb uplift at joint 16, 229 lb uplift at joint 9 and 223 lb uplift at joint 11.



Brace must cover 90% of web length.

REACTIONS (lb/size) 2=177/0-4-0, 17=1172/0-4-0, 8=682/0-4-0, 10=367/0-4-0

Max Horz 2=198(load case 5) Max Uplift2=-173(load case 6), 17=-395(load case 5), 8=-241(load case 7), 10=-209(load case 4) Max Grav 2=204(load case 10), 17=1172(load case 1), 8=682(load case 1), 10=367(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD

1-2=0/45, 2-3=-112/243, 3-4=-539/357, 4-5=-392/348, 5-6=-676/465, 6-7=-874/492, 7-8=-886/351, 8-9=0/45
2-17=-151/162, 16-17=-151/162, 15-16=-201/748, 14-15=-201/748, 13-14=-201/748, 12-13=-201/748, 11-12=-123/648, 10-11=-123/648, 8-10=-123/648

WEBS 3-17=-1094/522, 3-16=-181/691, 4-16=-40/97, 5-16=-549/251, 5-14=0/209, 5-12=-197/149, 6-12=-69/211, 7-12=-131/141, 7-11=-291/233

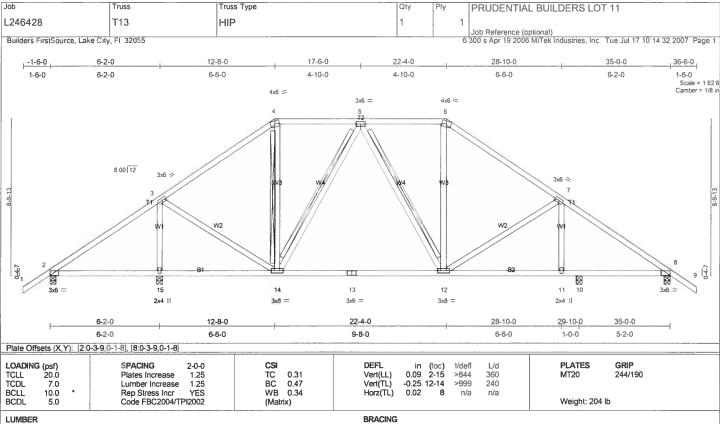
JOINT STRESS INDEX

 $2 = 0.75, \ 3 = 0.51, \ 4 = 0.50, \ 5 = 0.57, \ 6 = 0.50, \ 7 = 0.51, \ 8 = 0.75, \ 11 = 0.40, \ 12 = 0.68, \ 13 = 0.32, \ 14 = 0.34, \ 15 = 0.32, \ 16 = 0.68 \ and \ 17 = 0.40, \ 17 = 0.40, \ 18 =$

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 173 lb uplift at joint 2, 395 lb uplift at joint 17, 241 lb uplift at joint 8 and 209 lb uplift at joint 10.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

BRACING TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.
T-Brace: 2 X 4 SYP No.3 - 4-14, 5-14, 5-12

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.

,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 2=212/0-4-0, 15=1137/0-4-0, 8=724/0-4-0, 10=326/0-4-0

Max Horz 2=235(load case 5)

Max Uplif2=-167(load case 6), 15=-340(load case 5), 8=-251(load case 7), 10=-192(load case 4)
Max Grav 2=240(load case 10), 15=1137(load case 1), 8=724(load case 1), 10=326(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

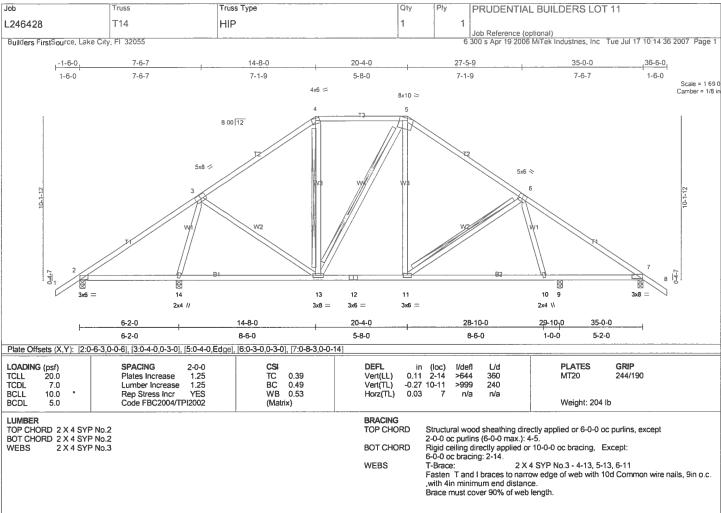
BOT CHORD WEBS

1-2=0/45, 2-3=-111/173, 3-4=-680/402, 4-5=-482/412, 5-6=-652/490, 6-7=-880/482, 7-8=-968/374, 8-9=0/45 2-15=-95/161, 14-15=-95/161, 13-14=-138/627, 12-13=-138/627, 11-12=-148/721, 10-11=-148/721, 8-10=-148/721 3-15=-1060/520, 3-14=-130/637, 4-14=-59/136, 5-14=-363/176, 5-12=-83/153, 6-12=-32/206, 7-12=-127/124, 7-11=-234/225

JOINT STRESS INDEX

2 = 0.67, 3 = 0.43, 4 = 0.67, 5 = 0.45, 6 = 0.67, 7 = 0.43, 8 = 0.67, 11 = 0.39, 12 = 0.61, 13 = 0.49, 14 = 0.61 and 15 = 0.39

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 167 lb uplift at joint 2, 340 lb uplift at joint 15, 251 lb uplift at joint 8 and 192 lb uplift at joint 10.



REACTIONS (lb/size) 2=205/0-4-0, 14=1145/0-4-0, 7=726/0-4-0, 9=323/0-4-0

Max Horz 2=272(load case 5)
Max Uplift2=159(load case 6), 14=-293(load case 6), 7=-218(load case 7), 9=-221(load case 4) Max Grav 2=241 (load case 10), 14=1145 (load case 1), 7=726 (load case 1), 9=323 (load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD
TOP CHORD
BOT CHORD
2-14=-107/166, 13-14=-160/185, 12-13=-40/584, 11-12=-40/584, 10-11=-120/756, 9-10=-47/706, 7-9=-47/706
WEBS
3-14=-107/1/513, 3-13=-22/387, 4-13=-84/139, 5-13=-265/120, 5-11=-73/304, 6-11=-223/176, 6-10=-197/284

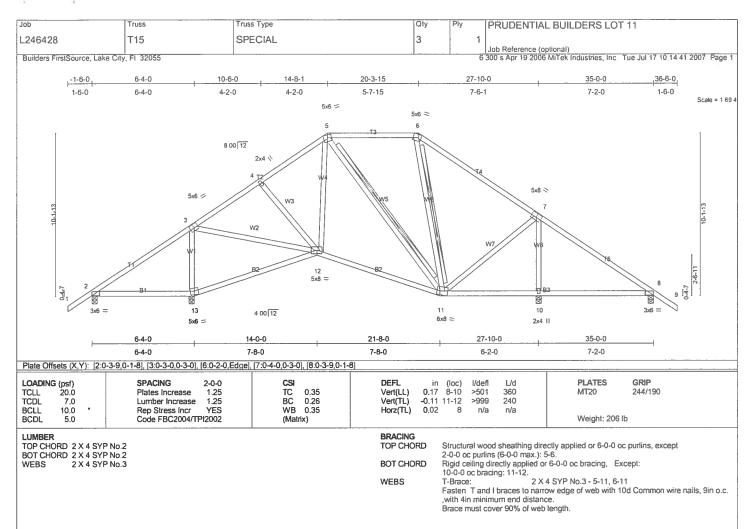
JOINT STRESS INDEX 2 = 0.64, 3 = 0.65, 4 = 0.68, 5 = 0.60, 6 = 0.84, 7 = 0.69, 10 = 0.34, 11 = 0.35, 12 = 0.22, 13 = 0.61 and 14 = 0.40

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 169 lb uplift at joint 2, 293 lb uplift at joint 14, 218 lb uplift at joint 7 and 221 lb uplift at joint 9.

7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.



REACTIONS (lb/size) 2=209/0-4-0, 13=976/0-4-0, 8=285/0-4-0, 10=928/0-4-0

Max Horz 2=-272(load case 4), 13=-333(load case 5), 8=-226(load case 7), 10=-310(load case 7) Max Grav 2=230(load case 10), 13=976(load case 1), 8=306(load case 11), 10=928(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/45, 2-3=-182/247, 3-4=-727/352, 4-5=-587/357, 5-6=-325/386, 6-7=-480/348, 7-8=-145/108, 8-9=0/45 2-13=-87/94, 12-13=-137/110, 11-12=-134/460, 10-11=-27/115, 8-10=-32/121 TOP CHORD

BOT CHORD

3-13=-856/368, 3-12=-115/647, 4-12=-174/167, 5-12=-36/237, 5-11=-196/155, 6-11=-190/67, 7-11=-72/360, 7-10=-839/445 WERS

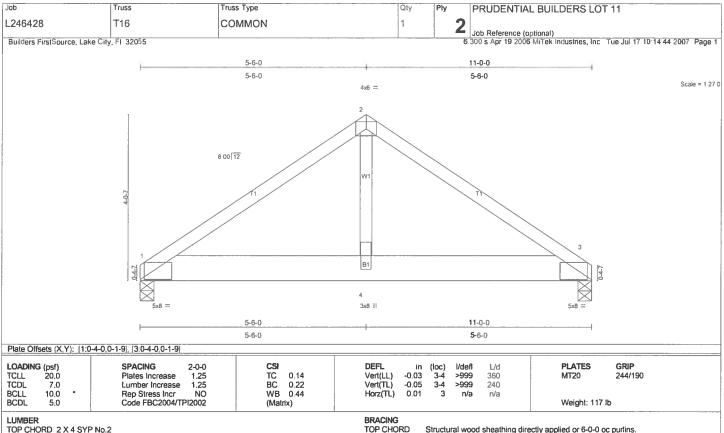
JOINT STRESS INDEX

2 = 0.60, 3 = 0.56, 4 = 0.34, 5 = 0.35, 6 = 0.74, 7 = 0.66, 8 = 0.63, 10 = 0.34, 11 = 0.62, 12 = 0.96 and 13 = 0.44

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 203 lb uplift at joint 2, 333 lb uplift at joint 13, 226 lb uplift at joint 8 and 310 lb uplift at joint 10.
- 7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 8 SYP 2400F 2.0E

WEBS 2 X 4 SYP No.3

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD

REACTIONS (lb/size) 1=2617/0-4-0, 3=2617/0-4-0 Max Horz 1=99(load case 4)

Max Uplift1=-743(load case 5), 3=-743(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-2847/816, 2-3=-2847/816 BOT CHORD 1-4=-638/2327, 3-4=-638/2327

WEBS 2-4=-776/2766

JOINT STRESS INDEX

1 = 0.37, 2 = 0.73, 3 = 0.37 and 4 = 0.45

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 1 row at 0-9-0 oc.

 - Bottom chords connected as follows: 2 X 8 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

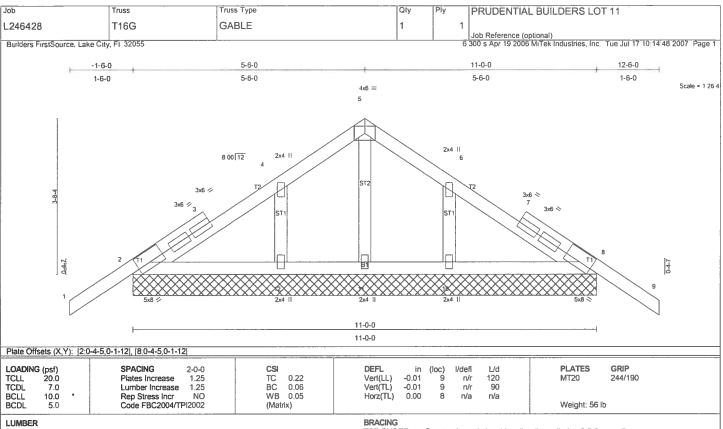
 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 743 lb uplift at joint 1 and 743 lb uplift at joint 3. 8) Girder carries tie-in span(s): 29-0-0 from 0-0-0 to 11-0-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54, 2-3=-54, 1-3=-437(F=-427)



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3 TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=293/11-0-0, 8=293/11-0-0, 11=173/11-0-0, 12=285/11-0-0, 10=285/11-0-0

Max Horz 2=-122(load case 4)

Max Uplift2=-165(load case 6), 8=-180(load case 7), 11=-10(load case 5), 12=-146(load case 6), 10=-149(load case 7) Max Grav 2=296(load case 10), 8=296(load case 11), 11=173(load case 1), 12=288(load case 10), 10=288(load case 11)

FORCES (tb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=7/70, 2-3=71/72, 3-4=-69/109, 4-5=-36/92, 5-6=-36/92, 6-7=-26/109, 7-8=-50/30, 8-9=-7/70

2-12=-27/127, 11-12=-27/127, 10-11=-27/127, 8-10=-27/127 5-11=-167/16, 4-12=-249/185, 6-10=-249/185 BOT CHORD

WEBS

JOINT STRESS INDEX

2 = 0.44, 3 = 0.00, 3 = 0.24, 3 = 0.24, 4 = 0.12, 5 = 0.08, 6 = 0.12, 7 = 0.00, 7 = 0.24, 7 = 0.24, 8 = 0.44, 10 = 0.11, 11 = 0.06 and 12 = 0.11

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 4) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) Gable requires continuous bottom chord bearing.

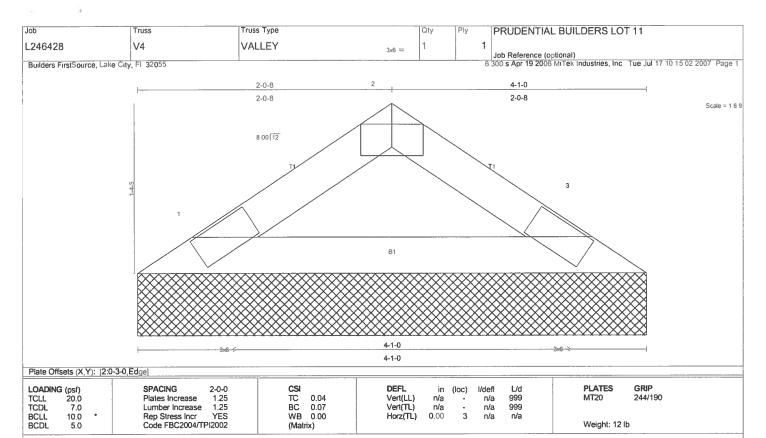
- 6) Gable study spaced at 2-0-0 oc.
 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 165 lb uplift at joint 2, 180 lb uplift at joint 8, 10 lb uplift at joint 11 , 146 lb uplift at joint 12 and 149 lb uplift at joint 10.

 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-5=-87(F=-33), 5-9=-87(F=-33), 2-8=-10



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-1-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=100/4-1-0, 3=100/4-1-0 Max Horz 1=29(load case 5) Max Uplift1=-21(load case 6), 3=-21(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-95/63, 2-3=-95/63 BOT CHORD 1-3=-23/63

JOINT STRESS INDEX

1 = 0.03, 2 = 0.02 and 3 = 0.03

- NOTES

 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) Gable requires continuous bottom chord bearing.

 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 1 and 21 lb uplift at joint 3.

PRUDENTIAL BUILDERS LOT 11 Job Truss Truss Type V6 L246428 VALLEY 1 Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10:15 05 2007 Page 1 Builders FirstSource, Lake City, FI 32055 3-3-13 6-7-10 3-3-13 Scale = 1 17 0 4x6 = 2 8.00 12 ST1 B1 6-7-10 6-7-10 LOADING (psf) TCLL 20.0 TCDL 7.0 CSI TC BC SPACING DEFL PLATES GRIP (loc) I/defl 1.25 1.25 0.10 244/190 Plates Increase Vert(LL) n/a n/a 999 MT20 0.06 Vert(TL) Lumber Increase n/a n/a BCLL BCDL 10.0 Rep Stress Incr YES Code FBC2004/TPI2002 WB 0.02 Horz(TL) 0.00 3 n/a n/a 5.0 (Matrix) Weight: 22 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=102/6-7-10, 3=102/6-7-10, 4=158/6-7-10

Max Horz 1=-53(load case 4)
Max Uplift1=-31(load case 6), 3=-36(load case 7), 4=-14(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-2=-61/43, 2-3=-61/43 1-4=-12/25, 3-4=-12/25 WEBS 2-4=-123/76

JOINT STRESS INDEX

1 = 0.03, 2 = 0.03, 3 = 0.03 and 4 = 0.04

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1 60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 Gable requires continuous bottom chord bearing.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 1, 36 lb uplift at joint 3 and 14 lb uplift at joint

Job Truss Type PRUDENTIAL BUILDERS LOT 11 V8 VALLEY 1 L246428 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 15:09 2007 Page 1 Builders FirstSource, Lake City, Fl 32055 4-0-8 8-1-0 4-0-8 4-0-8 Scale = 1 19 6 4x6 = 8 00 12 ST1 B1 3x6 % 2x4 || 8-1-0 8-1-0 LOADING (psf) SPACING DEFL **PLATES** GRIP 2-0-0 (loc) 1/defl L/d TC BC TCLL TCDL 20.0 1.25 1.25 0.09 Plates Increase Vert(LL) n/a 999 MT20 244/190 999 Lumber Increase Vert(TL) n/a n/a 10.0 Rep Stress Incr YES Code FBC2004/TPI2002 WB 0.03 3 Horz(TL) BCDL Weight: 28 lb (Matrix) LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=109/8-1-0, 3=109/8-1-0, 4=237/8-1-0

Max Horz 1:-66(load case 4)
Max Upifit1=-28(load case 6), 3=-33(load case 7), 4=-40(load case 6) Max Grav 1=110(load case 10), 3=110(load case 11), 4=237(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

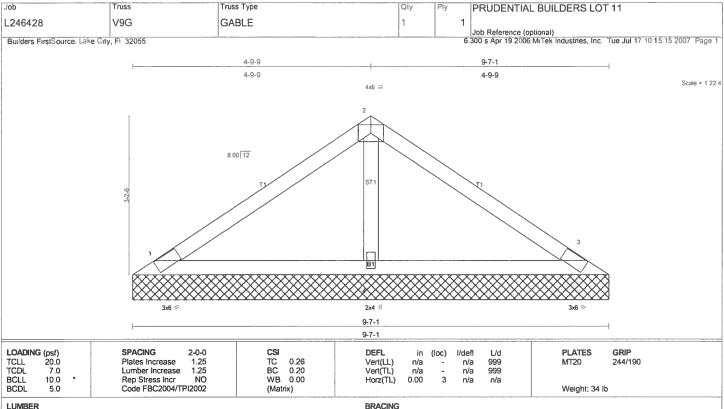
TOP CHORD BOT CHORD 1-2=-89/55, 2-3=-89/55 1-4=-15/35, 3-4=-15/35 WEBS 2-4=-170/106

JOINT STRESS INDEX

1 = 0.22, 2 = 0.32, 3 = 0.22 and 4 = 0.06

- NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate gnp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) Gable requires continuous bottom chord bearing.

- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1, 33 lb uplift at joint 3 and 40 lb uplift at joint



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD

REACTIONS (lb/size) 1=367/9-7-1, 3=367/9-7-1, 4=103/9-7-1

Max Horz 1=-100(load case 4)
Max Uplift1=-152(load case 6), 3=-152(load case 7), 4=-6(load case 6)

Max Grav 1=367(load case 1), 3=367(load case 1), 4=170(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-469/247, 2-3=-469/247 BOT CHORD 1-4=-137/313, 3-4=-137/313

JOINT STRESS INDEX

1 = 0.42, 2 = 0.50, 3 = 0.42 and 4 = 0.00

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) Gable requires continuous bottom chord bearing.
 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 152 lb uplift at joint 1, 152 lb uplift at joint 3 and 6 lb uplift at joint
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-87(F=-33), 2-3=-87(F=-33), 1-3=-10

PRUDENTIAL BUILDERS LOT 11 Job Truss Truss Type Qty V12 L246428 VALLEY Job Reference (optional)
6 300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 14:52 2007 Page 1 Builders FirstSource, Lake City, FI 32055 6-0-8 12-1-0 6-0-8 6-0-8 Scale = 1 27 0 2x4 | 8 00 12 12-1-0 12-1-0 CSI TC 0.10 BC 0.06 WB 0.05 LOADING (psf) **SPACING** DEFL. **PLATES** GRIP (loc) 244/190 20.0 1.25 1.25 TCLL Plates Increase Vert(LL) n/a n/a 999 MT20 TCDL. Lumber Increase n/a 999 Vert(TL) BCLL BCDL 10.0 Rep Stress Incr YES Code FBC2004/TPI2002 Horz(TL) 0.00 5 n/a n/a (Matrix) Weight: 49 lb LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=97/12-1-0, 5=97/12-1-0, 7=44/12-1-0, 6=237/12-1-0, 8=237/12-1-0

Max Horz 1=-103(load case 4)
Max Uplift1=-15(load case 7), 5=-10(load case 7), 6=-116(load case 7), 8=-116(load case 6)
Max Grav 1=97(load case 1), 5=97(load case 1), 7=63(load case 7), 6=237(load case 1), 8=237(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-75/65, 2-3=-54/103, 3-4=-54/103, 4-5=-59/50
BOT CHORD 1-8=-17/67, 7-8=-17/67, 6-7=-17/67, 5-6=-17/67
WEBS 3-7=-61/0, 4-6=-182/185, 2-8=-182/185

JOINT STRESS INDEX

1 = 0.20, 2 = 0.10, 3 = 0.04, 4 = 0.10, 5 = 0.20, 6 = 0.11, 7 = 0.02 and 8 = 0.11

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) "This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) Gable requires continuous bottom chord bearing.
 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 10 lb uplift at joint 5, 116 lb uplift at joint 6 and 116 lb uplift at joint 8.

Job Truss Truss Type Qtv PRUDENTIAL BUILDERS LOT 11 VALLEY L246428 V15G Job Reference (optional)
6 300 s Apr 19 2006 MiTek Industries, Inc. Tue Jul 17 10 14:56 2007 Page 1 Builders FirstSource, Lake City, Fl 32055 7-6-3 15-0-6 7-6-3 7-6-3 Scale = 1 32 6 4x6 = 8 00 12 2x4 II 2x4 || 2x4 II 2x4 II 15-0-6 15-0-6 CSI TC BC LOADING (psf) SPACING DEFL l/defl **PLATES** GRIP (loc) TCLL TCDL 20.0 7.0 1.25 1.25 244/190 Plates Increase 0.16 Vert(LL) n/a n/a 999 MT20 0.10 999 Lumber Increase Vert(TL) n/a n/a BCLL 10.0 Rep Stress Incr NO Code FBC2004/TPI2002 WB 0.09 Horz(TL) 0.00 5 n/a n/a Weight: 60 lb 5.0 (Matrix) LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3 TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=164/15-0-6, 5=164/15-0-6, 7=256/15-0-6, 6=390/15-0-6, 8=390/15-0-6

Max Horz 1=-163(load case 4)
Max Uplift1=-63(load case 7), 5=-50(load case 6), 7=-26(load case 6), 6=-278(load case 7), 8=-278(load case 6) Max Grav 1=164(load case 1), 5=164(load case 1), 7=256(load case 1), 6=397(load case 11), 8=397(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-130/102, 2-3=-142/134, 3-4=-142/128, 4-5=-130/53 BOT CHORD 1-8=-39/6, 7-8=-39/76, 5-7=-39/76 5-8=-39/76 WEBS 3-7=-235/56, 4-6=-336/280, 2-8=-336/281

JOINT STRESS INDEX

1 = 0.22, 2 = 0.16, 3 = 0.38, 4 = 0.16, 5 = 0.22, 6 = 0.16, 7 = 0.09 and 8 = 0.16

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gusl); h=20ff; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) Gable requires continuous bottom chord bearing.

 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 6) Provide mechanical connection (by others) of russ to bearing plate capable of withstanding 63 lb uplift at joint 1, 50 lb uplift at joint 5, 26 lb uplift at joint 7, 278 lb uplift at joint 6 and 278 lb uplift at joint 8.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-87(F=-33), 3-5=-87(F=-33), 1-5=-10

DIAGONAL BEACE OPTION:
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DOUBLED WICHN DIAGONAL
HRACE IS USED. CONNECT
HIAGONAL BEACE FOR SAGA
AT EACH YED. MAY WEB
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SP# #A ##. OR BETTER
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CUY (AS SHOWN) AT
UPPER END. MPH 0 0 0 0 0 0 0 GROUP B ~ 1 만 의 하 기 기 이 의 의 하 0 1 만 보고 다 의 의 의 WIND GROUP A (1) 2X4 "L" BRACE * (2) 2X4 "L" BRACE ** 5 8" 7' 11" 7' 11" 6' 10" 7' 11" SPEED GROUP B REPER 157 Ę, **⊢** ≅ ď GROUP A 10 10 10 5 5 10 5 5 10 5 5 10 5 5 10 5 5 10 5 5 10 5 5 10 10 5 MEAN CHART ABOVE FOR MAX GABLE VERTICAL LENGTH EX4 #EN OR BETTER CONLINION BRYKING GROUP B 10' 5" 11' 2" 11' 2" 10' 11" (**Q**) 5 5 HEIGHT, **⊕** GROUP A (1) 2X6 "L" BRACE * (2) CONS. 10° 4° 8° 10° 3° 13 13 13 15 15 6 8 8 8 8 8 9 No: 34869 STATE OF FLORIDA ENGINEERS P.A. ENCLOSED, 10. 4, 10. 4, 12. 4, 12. 4, 12. 4, 12. 5, 13 GROUP B 12' 3" 14' 0" 14' 0" 12' 6" 13['] в" 14 0 GROUP A 12 11 B 8 . מס Н 녆 MAX. MAX. HRACE GROUP B 14 D 14 D 14 D 13' 11" 13' 7" 11 B 11 14. 0° 1.00, TOT. SPACING 1 E ATTACH EACH 'L' BRACE WITH 10d NAILS.

* FOR (1) 'L' BRACE, SPACE WILL A FE 'D.C.

* FOR (2) 'L' BRACES, SPACE WALLS AT S' O.C.

IN 16' IND ZONES AND 6' D.C. BETWEEN ZONES.

IN 16' IND ZONES AND 6' D.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD THOM 4' 0" PROVIDE UPLIT CONNECTIONS FOR 136 FLF OVER CONTENIOUS BEARING (5 PSF PC DEAD LOAD). LIVE LOAD DEPLECTION CRETERIA IS L/240. MITABET LENGTH. I. BRACING MUST BE A MINIMUM OF 80% OF WEB DOUGLAS FIR-LARCH
#3
STUD
STANDARD SPRUCE-PINE-INE

#3 STUD

#3 STUD BRACING GROUP SPECIES PLYWOOD OVERHANG. EXPOSURE SOUTH MACHETURE CABLE TRUSS DETAIL NOTES: VENTYCAL LENGTH

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GREATER THAN 11' 0° 60 24.0 PEAK, SPIJCE, AND HEEL FLATES. GARLE VERTICAL PLATE SIZES PSF DATE REF GROUP DRWG #1 & BTR GROUP DOUGLAS FIR-LARCH
#1
#2 C MITEN SID CASSES 16 E HI SOUTHERN PINE
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ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

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	AL BRACE OPTION: LI LENCTH MAY BE D WHEN DIAGONAL IS USED. CONNECT AL HALE TON BEG H END. MAX WEB LENGTH IS 14". VERTICAL LENGTH BHOWN IN TABLE ABOVE. CONNECT DIAGONAL A' MIDPIDNT OF VERTICA	12" O.C		NG BLE	2X.
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MAX.	ictu.				(2) ZXB 'L' BRACE
TOT. LD.	MH- * *13 HB				1 3
. 60 PSF	DUTLIDORERS WITH 2' O' DYENBANG, DR 12" PUNPOOD OVERHANG. ** FOR (1) "L' BRACE WITH 10d NAILS. ** FOR (1) "L' BRACE REACE NAILS AV 2" O.C. DI 18" END ZONES AND 4" O.C. BETWEN ZONE ** FOR (2) "L' BRACES: BRACE RAILS AT 3" O.C. DI 18" END ZONES AND 6" O.C. BETWEN ZONE ** FOR (2) "L' BRACES: BRACE RAILS AT 3" O.C. DI 18" END ZONES AND 6" O.C. BETWEN ZONE ** FOR (2) "L' BRACES: BRACE RAILS AT 3" O.C. DEFENTATION AND 11 OF BOX OF WEB MEMBER LENGTH. CABLE VERTICAL PLATE SIZES	CABLE TRUSS DETAIL NOTES: LIVE LOAD DEPLECTION CRITERIA IS L/240. PROVIDE UPLIT CONNECTIONS FUR 180 FLF OVER CONTENUOUS BEARING (5 PSP YC DEAD LOAD). CABLE END SUPPORTS LOAD FROM 4: 0"	G SING NEGHTINGS	BRACING GROUP SPECIES AND GRADES: GROUP A: SPRUCE-PINE-FIR 11 / 42 STANDARD DOUGLAS FIR-LARCH STUD DOUGLAS FIR-LARCH STUD DOUGLAS FIR-LARCH STUD STANDARD STANDARD STANDARD STANDARD STANDARD STANDARD	
DATE DWG	THILDOWRES WITH E' O' DVERBANG, DR 12" FWOOD OVERHANG, CHE RACH "L' BRACE WITH 104 INLES. POR (1) "L' BRACE WITH 104 INLES. NO 18" END ZONES AND 4" O.C. BETWEEN 1" NO 18" END ZONES AND 4" O.C. BETWEEN 1" NO 18" END ZONES AND 6" O.C. BETWEEN 1" NO 18" END ZONES AND 6" O.C. BETWEEN 1" NO 18" END ZONES AND 6" O.C. BETWEEN 1" NO 18" END ZONES AND 6" O.C. BETWEEN 1" NO 18" END ZONES AND 6" O.C. BETWEEN 1" NO 18" END ZONES AND 18" END ZONES CABLE VERTICAL LENCH 1ND 18" END ZONE CREATER THAN 1" B" CREATER	TRUSS I	GROUP B HEM-FIR HISA-FIR HISA-	ROUP SPEC	:
	THE SAME SAME AND ACCOUNTS OF THE SAME AND ACCOUNTS OF THE SAME AND ACCOUNTS OF THE SAME ACCO	D FROM 4	PB: PIR DOUGLAS FI	CIES AND GRAD P A: HEM-FIR J2 STAND SOUTHERN PORE J3 STANDARD	
3/03 GABLE 50'1	TERBANG, DR 12" 1 10d NAILS. 2 0.C. HITHEN ZONE ACE MAILS AT 2" 0.C. 0 0.C HITHEN ZONE MILM OF 80% OF WEB MILM OF 80% OF WEB MIL PLATES. 10T ZM 2.5M 10T ZM 10T ZM 2.5M 10T ZM	NOTES: /240. 20 FLF OV	DOUGLAS FIR-LARCH	AND GRADES: HEM-FIR STUD TIPERN PINE #3 STUD #5 STUD #	
	DAR	- 5			

'L' BRACE WITH 104 NAILS.

'L' BRACE; GRACE NAILS AT 2° O.C.
D ZONES AND 4° O.C. BETWIEN ZONES.
L' BRACES: SPACE MAILS AT 3° O.C.
D ZONES AND 6° D.C. BETWIEN ZONES. IT CONNECTIONS JUR 180 FLF OVER
3 HEALING (6 PSF TC DEAD LAAD).

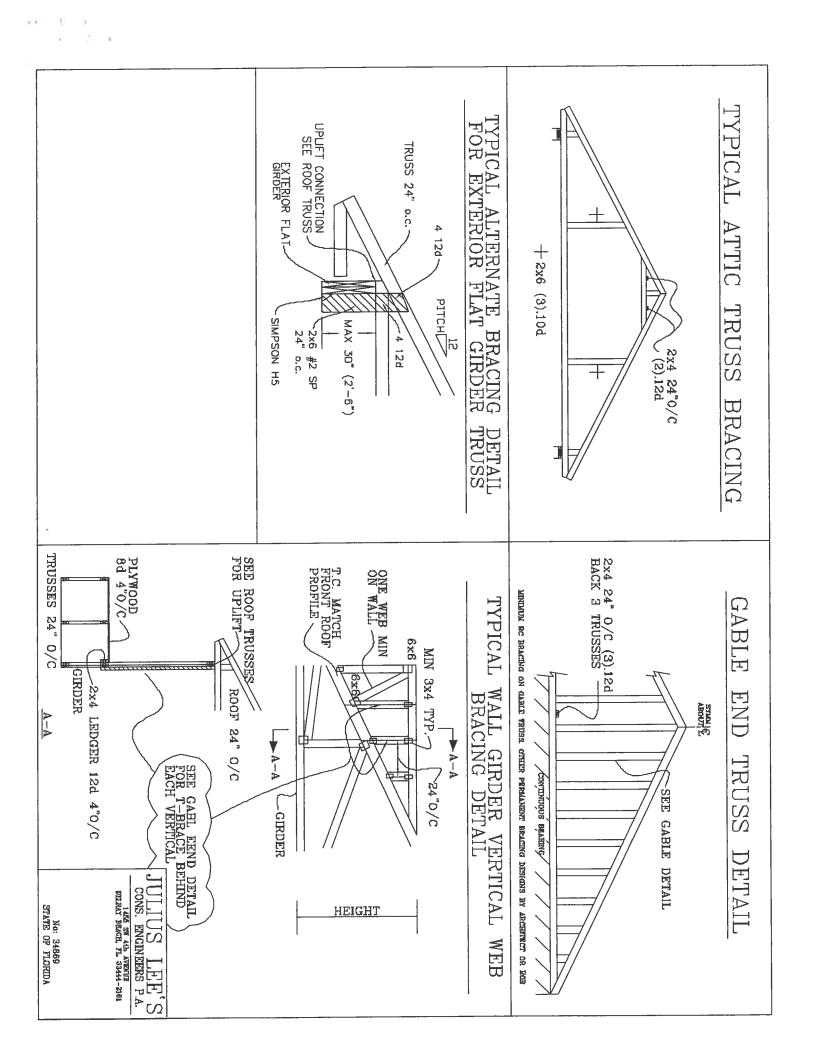
PPOSIIS LOAD FROM 4' 0"

THIN 2' 0" OVERHANG, DR 12"

VERHANG. ST BE A MINIMUM OF BOX OF WEB OCTION CRITERIA IS L/240.

PEAK, SPLICE, AND HEEL PLATES	REATER THAN 11' 6"	GREATER THAN 4' D', BUT	ESIS TRAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
THURS PREIGN FOR	2.5X4	2X4	1X4 DR EXT	NO SPLICE	IE SIZES

ASCB7-02-CAB13030



TOP CHORD CHORD WEBS 2X4 2X4 2X4 ಹನ್ನು 988 BETTER BETTER

TOINT

SPANS

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34

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52

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4X8

5X8

8X6

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EX3

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. 504

1.5X4

>

2X4

2.5X4

2.5X4

3X5

REFER TO SEALED DESIGN FOR DASHED PLATES

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

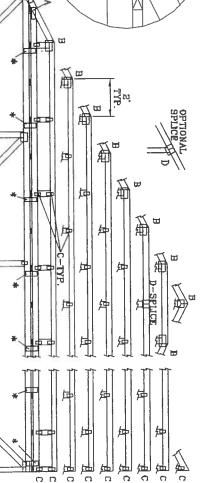
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY HE APPLIED HENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: 110 MPH WIND, 30' MBAN HGT, ASCE 7-93, CLOSED BIDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C. WIND TO DI=5 PSF, WIND BC DI=5 PSF

110 MPH WIND, 30' MBAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-88, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND TC DL=6 PSF, WIND BC DL=6 PSF CLOSED C,

FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. 20' FLAT TOP CHORD MAX SPAN MAX SIZE OF ZXIZ



NAX L 12 LOCATION IS
ACCEPTABLE

E	AXE OR 3XE TRULOX AT 4' OC, ROTATED VERTICALLY
ACH TRULOX PLATES JAL, PER FACE PER CONNECTED. REFER ORMATION.	PLATES WITH (8) 0.12D" X 1.375" NAILS, OR CE PER PLY. (4) NAILS IN EACH MEMBER TO REFER TO DRAWING 160 TL FOR TRULOX

ESE E

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9X9

933

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10' T	7'9" 1	o' To	A ETH	
10' TO 14'	7'9" TO 10'	0' TO 7'8"	LENGTH	
2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 18d NAILS AT 4° OC.	1x4 "T" I MEMBER. MEMBER.	NO BRACING	REQUIRED BRACING	WEB BRACING CHART

*
* PIGGYBACK
SPECIAL
PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

G C C

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8 1/4" અ

C

OB

	PANARHINGS TRUSSES REQUIRE EXTREME IN FABRICATING, HAMILING, SHIPPING, INSTALLING AND BRAIDING. REFER TO INSECT AND QUILITING COMPONENT SHETTY TREMBAYITHY, PAULICINED BY THE CIRILSS PLANT INSTITUTE, SEA DYDOFROU SK, SUINTE RUM, MAISON, V.J. 537999 AND VITAN CYDIM TRUSS CILLUROL OF ARRICA, ASIA EMPERICIS LA, HAMISON, V.J. 53799 FIRE SAFETY PROTECTES FROM TO PERFORMS THE SET FINCTITUS. LALEST CHICATING. DECEST CHICATING THE THE CHICA SHE SAFETY ANTACHED STORE AND BETTON CHICAD SHALL HAVE A PROPERLY ATTACHED STORE AND BETTON CHICAD SHALL HAVE A PROPERLY ATTACHED RIGHT CHICAGO.				
No: 34868 STATE OF FLORIDA		JULIUS LEE'S P.A. 1466 SW 4th AVENUE P.A. 1466 SW 4th AVENUE P.A. 1467 SW 4th AVENUE P.A.	THIS DRAWING		
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	MAX LOADING 55 PSF AT 1.33 DUR. FAC. 50 PSF AT 1.25 DUR. FAC.	ING REPLACES DRAWINGS		
		DATE 11/26/03 DRWGMITEK STD PIGGY -ENG JL	REPLACES DRAWINGS 634,018 634,017 & 847,045		

VALLEYTRUSS DETAIL

TOP CHORD CHORD WEBS 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR 2X4 SP #3 OR BETTER. BETTER

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE)
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: BUILDING, EXP. C, RESIDENTIAL, WIND TO DL=5 PSF. SHC 110 MPH, ASCE 7-83 110 MPH WIND OR (3) 16d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEH, VALLEY WEH, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY INSTALLATION TRUSS

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON SEALED DESIGN

* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

ENGINEERS' SEALED DESIGN.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

CUT FROM 2X6 OR LARGER AS REQ'D

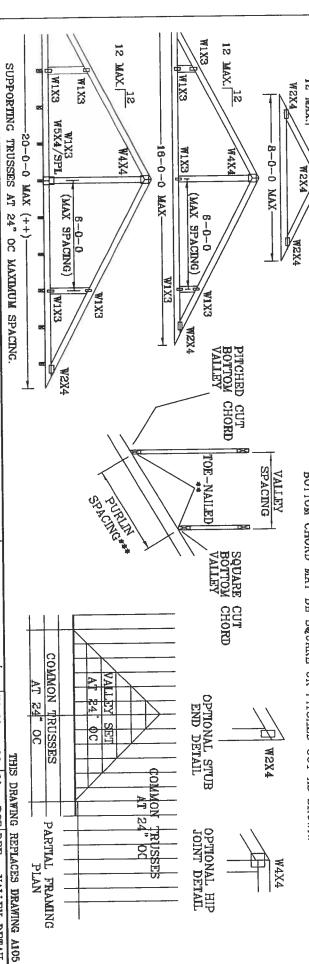
12 MAX.

12

4-0-0

MAX

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



WAVARNHAM TRUISSS REQUIRE DYTROKE EARE IN FARRENTIAG, HACHING SHIPPING, INSTALLING AN ANCING REFER TO XCST I-ING GUILLING COPORNOM SAFETY ARGUMATURIN, JOHN STEP THE YEAR ANCING REFER TO XCST I-ING GUILLING BY LUTTE BUT ANDISON, VI. 53799 FOR SAFETY PRACTICES PRIDE TO PERFORMENT FINET INCIDENT LANGUAGE, VI. 53799 FOR SAFETY PRACTICES PRIDE TO PERFORMENT ANTACKED THE SAFETY ANDISON, VI. 53799 FOR SAFETY PRACTICES PRIDE TO PERFORMENT AND SAFETY ANDISON CELLING.

						_
STATE OF FLORIDA	No. 94880			DELRAY BRACH, IL SSA44-SICI	INEERS P.A.	
SPA	DURI	TOT.	BC 1	BC 1	TC I	TC I
SPACING	DURFAC 1.25	TOT. LD.	F	ΔI	P	E
	S)	32 40	0	ייט	~Z	20
24:	1.25	40	0	Ç'n	15	88
		PSF	PSF	PSF	PSF	PSF REF
			-ENG JL	PSF DRWG	PSF DATE	REF
			T	VALTRUSS1103	11/26/09	VALLEY DETAIL

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE, PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

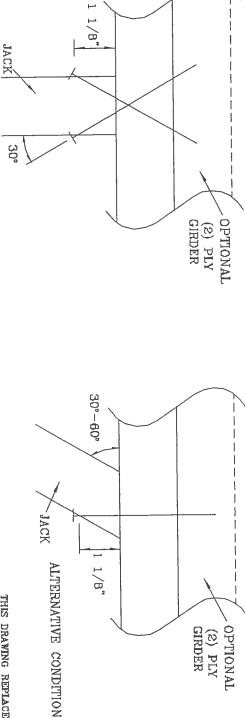
MAXIMUM LATERAL RESISTANCE 엵 16d (0.162"X3.5") COMMON TOE-NAILS

5 493#	4 394	3 296#	2 197#	TOE-NAILS 1	
₩	4#	6#	7#	PLY	SOUTHERN PINE
639#	511#	383#	256#	Sarra 3	
452#	361#	271#	181#	1 PLY	DOUGLAS
585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
390#	312#	234#	156#	1 PLY	HEM-FIR
507#	406#	304#	203#	2 PLIES	-FIR
384#	307#	230#	154#	1 PLY	SPRUCE PINE FIR
498#	397#	298#	199#	2 PLIES	PINE FIR

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR

GIRDER

OPTIONAL (2) PLY



THIS DRAWING REPLACES DRAWING 784040

STATE OF FLORIDA	No. 94800			1450 SY 4th AVENUS DELEAY SEACH, FL S3444-2161	CONS. ENGINEERS P.A.	V, HH, V
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG CNTONALL103	DATE 11/26/03	PSF REF TOE-NAIL

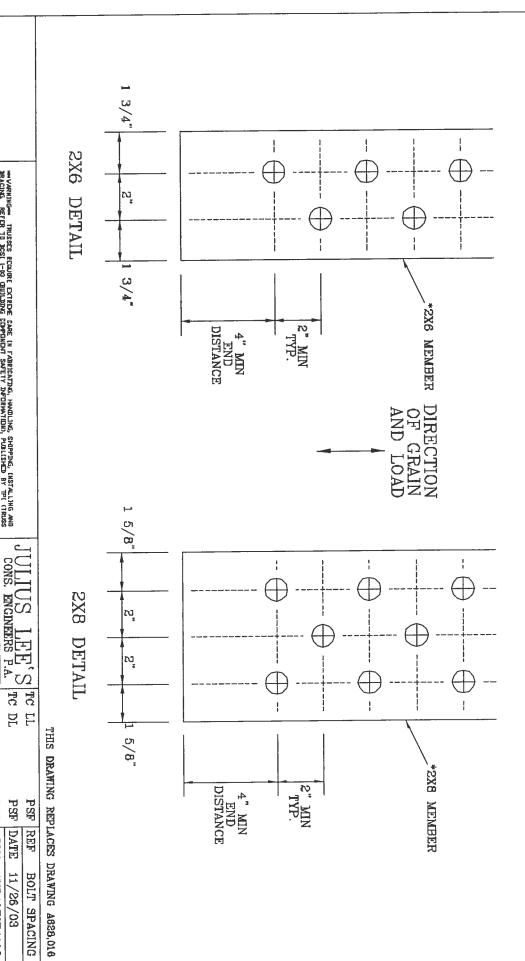
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



INVESTS REQUIRE EXTENDE DAME IN FABRINATIAG, HANDLING, SHIPPING, (METALLING).

RACING REFER TO DOS! 1-00 GUILDING EDPRIDIAT SAFETY DEPRINATION, PUBLISHO BY THE (IR PLAITE DASTITUE, 500 CHORITED DE, SUITE DRI, MUNISON, V.). 347799 AND LYTCA CYDID TRASS CAU DE AMERICA, 6300 ENTERPORSE LIV, MADISON, V.I. 33799 FOR SAFETY PACCIFICES PERIOR TO PERFORME THE SET ENVITORIS. UNLESS OFTENDENSE, V. MADISON, VI. 33799 FOR SAFETY PACCIFICES PERIOR TO PERFORME THE CHORICAL PANCIES AND MUTHER SHAPE PROPERLY ATTACHED STENDED CELING.

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TC DL

PSF

DATE

DRWG -ENG

> CNBOLTSP1103 11/26/03

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TOT. BC

PSF PSF PSF

DELRAY BEACH, FL 33444-2161

No: 34869 STATE OF FLORIDA

SPACING DUR. FAC

TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\(\phi\)).

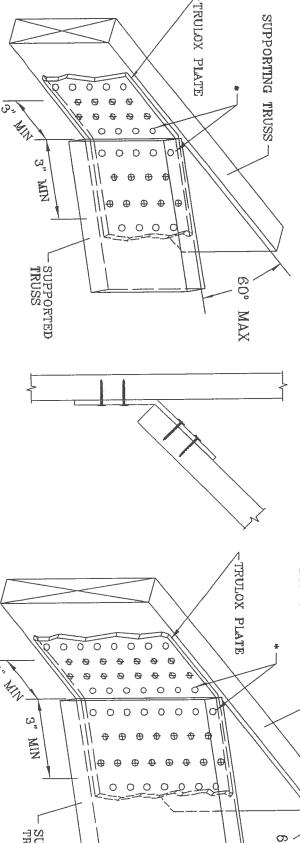
NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST

EXCEED THE TRULOX PLATE WIDTH

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

THIS DETAIL FOR LUMBER. REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN. PLATES, AND OTHER



MINIMUM 3X6 TRULOX PLATE

	_	
6X6	3X6	TRULOX PLATE SIZE
16	9	REQUIRED NAILS PER TRUSS
990#	350#	MAXIMUM LOAD UP OR DOWN

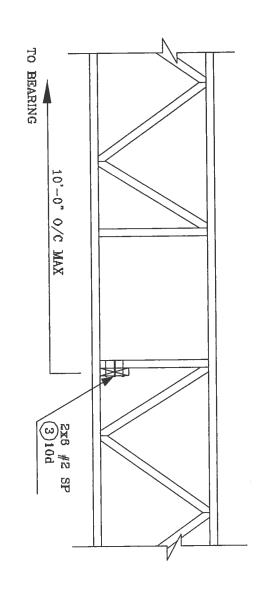
 NIM "S MIN	TRULOX PLATE	SUPPORTING TRUSS
SUPPORTED		60° MAX

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

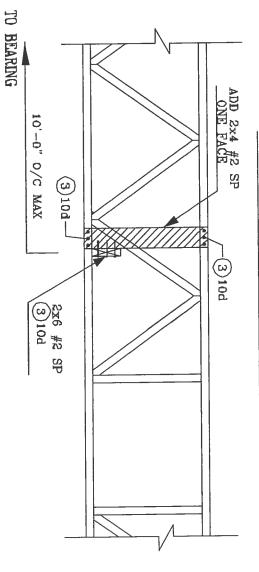
MINIMUM 5X6 TRULOX PLATE

			\neg
""HARRING" TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO SESS ;—DO GRUILDING COMPINENT SAFETY INFORMATION, PUBLISHED BY TRY (TRUSS PLACE BOSTITUTE, 383 DENDREMED BC, SMITE END, MARISON, VI. 39739) AND VICA (VOID TRUSS COUNCIL OF AMERICA, VI. 39739) AND VICA (VOID TRUSS COUNCIL OF AMERICA, VI. 39739) FOR SAFETY PRACTICES POIDT TO PERFERNING THESE FUNCTIONS. UNLESS OTHERWISE DIGITATED, TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND INTITAL CHERO SMALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND INTITAL CHERO SMALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND INTITAL CHERO SMALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND INTITAL CHERO SMALL HAVE A PROPERLY ATTACHED RIGHT CHERO.			
	DELEVAL BRACK IT BRAVE-EIGH	CONS. ENGINEERS P.A.	7 HH 1 2111 1111
	ם	ы	ਸ
ENG JL	ORWG CNTRULOX1103	DATE 11/26/03	REF TRULOX
		DELBAY BRACK, 11. 28444-22E1 —ENG	DATE CONS. ENGINEERS P.A. 1455 5W 444 AVENUE DELEAN BEAUCH, 71 28444-2255 —ENG

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S cons. engineers p.a.

No: 34869 STATE OF FLORIDA