

Wind Load Analysis and Certification

Rozear Residence by Hartley Brother, Inc

2023 Florida Building Code section 1609 according to ASCE 7-22

Ultimate Design Wind Speed (Vult) = 130 MPH (3 second gust)

Nominal Design Wind Speed (Vasd)) = 101 MPH

Risk Category = II

Exposure Category = B, Enclosed Building

Applicable Internal Pressure Coefficient = .18

Design Wind Pressure for use of External Components (Components and Cladding) = +32.1psf, -43.3psf

Overhead Garage Door: +15.2psf, -16.9psf



Roof Decking

7/16" or 5/8" OSB or 1/2", 5/8" or 3/4" CDX Decking; 48"x96" Sheets, Perpendicular to Roof Framing Members

8d common (.131" dia) or 8d ring-shank (.113" dia.) nails at 4" O.C. on Ends, 8" O.C. in Interior

Trusses or Rafters at 2' O.C. (horizontal distance), No Intermediate Blocking Required

Rafters: 2x6 SYP #2 up to 10' horizontal span, 2x8 SYP #2 up to 14' horizontal span

Shear Wall Segments

7/16" OSB or 1/2" CDX plywood, 48" Wide Sheets - Sheathing Continuous from Top Plate down to Pressure Treated Sole Plate Bearing on Foundation.

8d common (.131" dia) nails at 3" O.C. on Edges and Ends, 8" O.C. in Interior

Transverse Shearwall = 60', Longitudinal Shearwall = 50'

2x4 SPF (No. 1&2) Studs at 16" O.C., up to 12'

2x6 SPF (No. 1&2) Studs at 16" O.C., up to 17'

See attached detail for stud and jack requirements for wall openings

Nail Together Double Top Plate with two (2) 12-d comm nails every 16" O.C. (SYP top plates)

Other Wall Segments - Same as Shear Walls

Gabled End Wall Framing

Balloon Frame (see details) or see attached alternate detail. This includes porch walls parallel to trusses.

Gable overhangs without out-lookers limited to 12" total overhang

Special Notes: All headers and beams to be double 2x12 SP#2 except as shown on attached plans. All LVL plies to be 1-3/4" wide 2800Fb-1.9E or better. All girders and beams to have a min of two (2) 2x4 SP#2 or two (2) 2x6 SP#2 studs under each bearing point. **This structural and windload analysis is based on the attached truss layout. Any deviation from the attached layout invalidates this structural and windload analysis.**

Footings and Foundations (Based on Truss Engineering)

20" deep x 14" wide monolithic with 2-#5's, Continuous, 3000 psi Concrete

Garage Mono: depth of garage mono footer shall be a min of 18" below the garage slab and any curb shall be a min of 6" wide and a maximum of 6" deep or 8" wide and 8" deep. Any curb deeper than this requires special engineering design.

or: 20" Wide x 10" Deep 3000 psi Concrete Strip Footing with 2-#5's, Continuous

8"x8"x16" Concrete Masonry Stemwall, Minimum 2 Courses, Maximum 4 Courses, Fully Grouted, except sections over 3 courses need only cells with rebar to be grouted. 1-#5 Vertical Dowel at Corners **and 6'-0" O.C.** (10" hook top and bottom) (min 25" lap all #5 rebar) Max distance between top of garage floor and top of stemwall = 18" **(1) #5 continuous top**

course. All 4" slabs requires 6x6 WWM

Interior footers: 16" wide by 10" deep (including 4" slab) with 2-#5's, Continuous,

Note: It is the contractors responsibility to install all necessary interior footers per truss manufacturers requirements based on the attached truss layouts and any interior shearwalls.

Porch Footers: 16" deep x 14" wide monolithic with 2-#5's, Continuous or see above or: 8" wide by 8" deep bell footing with 1-#5, Continuous with minimum of 30"x30" x 15" pad under each post (w/ 3- #5 each way)

Note: footer design based on continuous bearing of 2000 psf. Footers for any concentrated loads greater than 10,000 lbs must be reviewed with windload engineer. Movement – The information presented in this document is not calculated or intended for the use or purpose of mitigating or addressing unsuitable soils or subsurface conditions in any way or manner, whatsoever.

Hurricane-Resistance Hardware (Based on Truss Engineering)

Truss Clips/Headers/Girders/Posts/Beams /Top and Bottom of Wall Unit - See Table

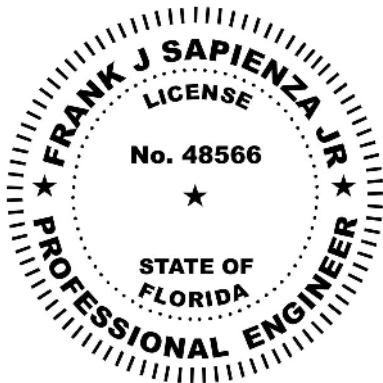
Note: regarding strapping of headers and beams, 50% of the strap shall be on the header/beam and 50% of the strap is on the supporting member below it (jacks and posts).

Anchor Bolts- 1/2"Dia. x 10" J Bolts (with min 7.5" embedment) at 48"O.C. (First bolt at 9" from Corner, then 48" O.C.) and at each end of Each Opening (2" round or square washers).

I hereby certify that the accompanying Wind Load Analysis for the **Rozear Residence**, demonstrates compliance with the 2023 FBC section 1609 according to ASCE 7-22, to the best of my knowledge.

Frank J. Sapienza Jr.
License Professional Engineer
Florida License Number 48566

This item has been digitally signed and sealed by FRANK J SAPIENZA JR PE using Digital Signature.
Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



Frank J
Sapienza Jr

Digitally signed by
Frank J Sapienza Jr
Date: 2024.09.13
09:33:12 -04'00'

HOLD-DOWN TABLE**Wood Sections**

	Uplift Force Lbs	Top Connector Simpson **	Rating Lbs	Bottom Connector Simpson **	Rating Lbs
HEADERS					
	up to 455 lbs	LSTA9	775	H3	455
	up to 910 lbs	LSTA12	970	2-H3	910
	up to 1235 lbs	LSTA18	1235	LTTP2	1680
	up to 1940 lbs	2-LSTA12	1940	LTTP2	2275
	up to 2470 lbs	2-LSTA18	2470	HTT4	3000
	up to 3000 lbs	3-LSTA18	3705	HTT4	3000
	up to 3705 lbs	3-LSTA18	3705	HTT5	4350

To determine uplift force on header at each end, total the uplifts for each truss resting on the header and divide by 2 (assumes uniform load) **Note:** must use proper bolt anchors sufficient to support required load

Trusses/Girders - Uplift

up to 600 lbs - use H2.5A top, no special device required at bottom
over 600 lbs but under 990 lbs use H10 top, no special device required at bottom
up to 1230 lbs use H6 or equivalent at top and LTTP2 at bottom
up to 1680 lbs use 2-H6 or equivalent at top and LTTP2 at bottom
up to 2460 lbs use 2-H6 or equivalent at top and HTT4 bottom
up to 3690 lbs use 3-H6 or equivalent at top and HTT5 bottom
Must Use proper bolt anchors

Note: it is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation

Strap rafters to truss or at each end with min uplift resistance of 450 lbs each end

Strap ridge beam at each end with min uplift resistance of 1800 lbs

Note: Three (3) 12d comm toenails (2 on one side, one on the other) required per truss/rafter per bearing point into plate to resist both lateral loads (wall to truss) and transverse loads

Horizontal Resistance (from truss loads) - Note: these devices are in addition to required toe-nails

up to 110 lbs - use H2.5A	Note: hardware to be used must satisfy both uplift and horizontal resistance, combination of devices is acceptable
up to 525 lbs use H10	
up to 1090 lbs use H10 plus A23	

	top		bottom	
BEAM SEATS	LSTA18*	1235	LTTP2*	1680
POSTS	2-LSTA18	2400	ABU44 or ABU66	2200
	* or per truss engineering		Must Use proper bolt anchors	

STUDS

Wall Sheathing Nailing Adequate Exterior Walls bottom (8d nails at 3" O.C.), must cover sill plate

Wall Sheathing Nailing Adequate Exterior Walls Top (8d nails at 3" O.C.), as long as sheathing covers top plate, otherwise use SP2 @32" O.C. in addition to sheathing nailing,

Use SP2 top and SP1 bottom each stud an anchor bolts @ 32" O.C. for all interior load bearing walls that have uplift. Interior anchor bolts to be 1/2" x 10" A307 or 1/2" x 6" Titan HD with 2" washers

Please Note: All Beams must be sheathed or strapped to double top plate (if applicable)

An equivalent device of same or other manufactures can be substituted for any of the devices specified on this page as long as it meets the required load capacities

Note: For nailing into SPF members, multiply table values by .86

Number of Jack and Stud Requirements per Opening Width
2x4 or 2x6 SPF #1&2 Construction — max Wall Height=12'
(based on 16" O.C. Stud Spacing)

Header		
Jacks		
Opening Width	#of Jacks	#of Studs
up to 4'	1	1
up to 6'	2	1
up to 9'	2	2
up to 12'	3	2
up to 14'	3	3
up to 18'	4	3
over 18' must be engineered		
Opening Width		
Studs		

Note — Based on uniform loads. Heavy concentrated loads require engineering review

Acceptable Framing Method for Balloon Framed Gable End-Wall with trusses

Balloon Frame with 2x4 SPF No.1&2 @ 16" O.C. with the Following Conditions:

Up to 12' - Block at 8'

Over 12' but Under 14' - 2x4 SYP #2 at 16" O.C. and Block at 4',8'&12'

Over 14' but Under 17' - Double 2x4 SYP #2 at 16" O.C. and block at 4',8',12'&16'

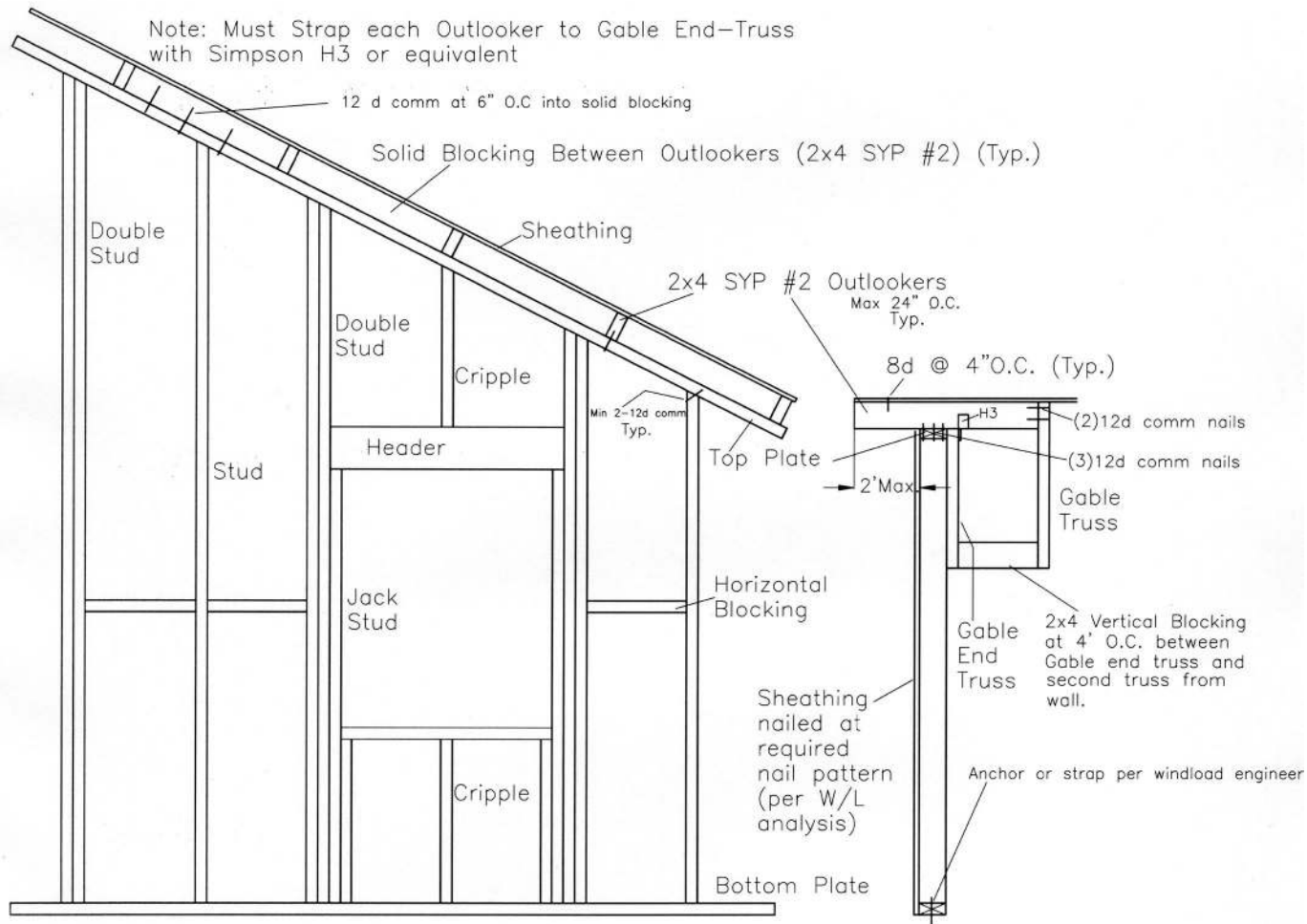
Over 17' but Under 20' - Triple 2x4 SYP #2 at 16" O.C. and block at 4',8',12'&16'

Over 20' but Under 23' - Quadruple 2x4 SYP #2 at 16" O.C. and block at 4',8',12',16'&20'

Over 23' - Must be Engineered

In all cases a minimum of a double full length stud is
required at each side of openings such as doors and windows

Blocking must be parallel to top and bottom plates with a minimum
of 2-12d comm nails



F. Sapienza, P.E.

Acceptable Framing Method for Balloon Framed Gable End-Wall

Balloon Frame with 2x6 SYP No.2, @ 16" O.C. with the Following Conditions:

Up to 18' - Block at 8' and 16'

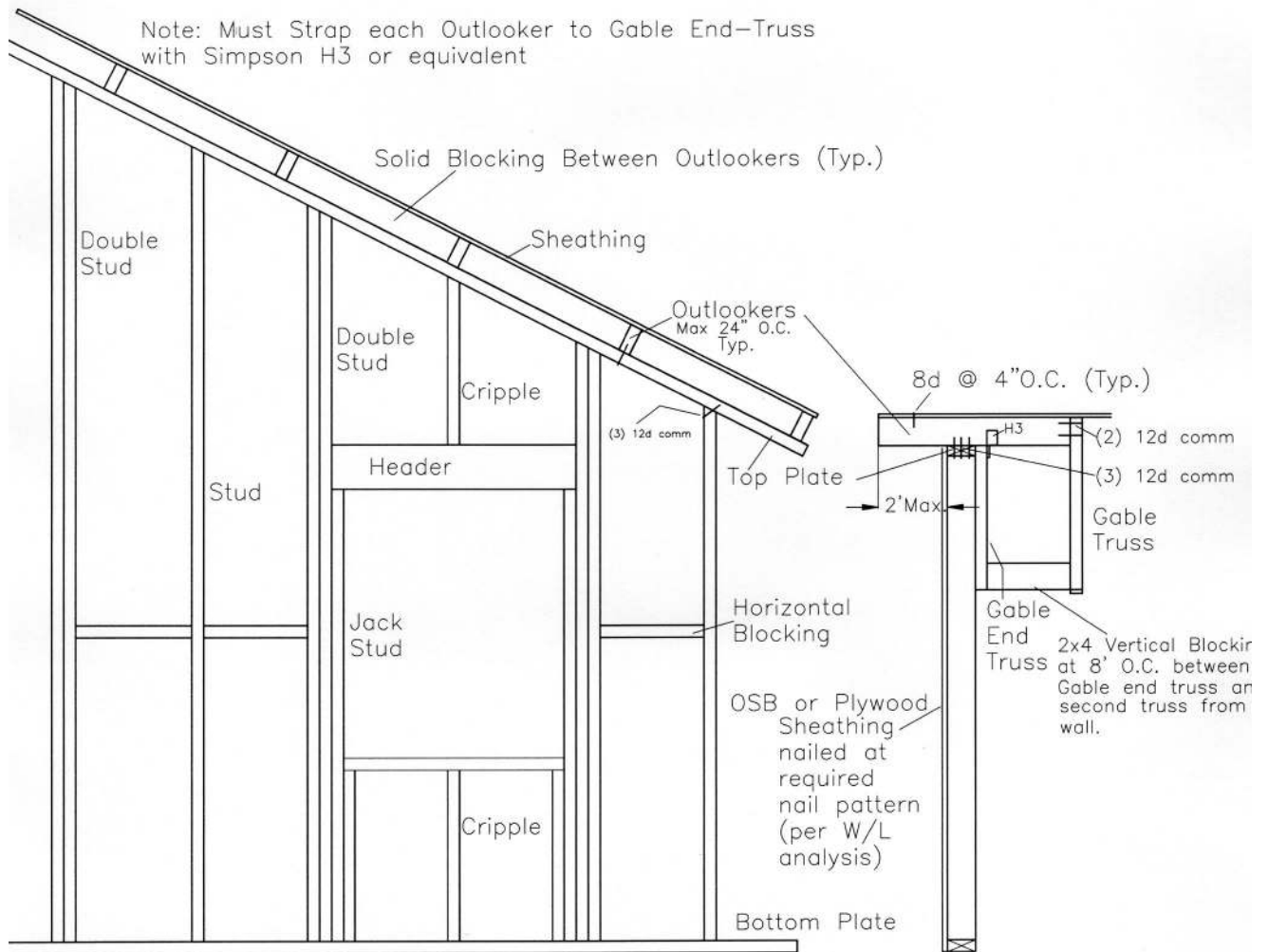
Over 18' but Under 21' - Double stud and block at 8' & 16'

Over 21' but Under 24' - Triple SYP #2 and block at 4', 8', 12' & 16'

Over 24' - Must be Engineered

In all cases a minimum of a double full length stud is required at each side of openings such as doors and windows

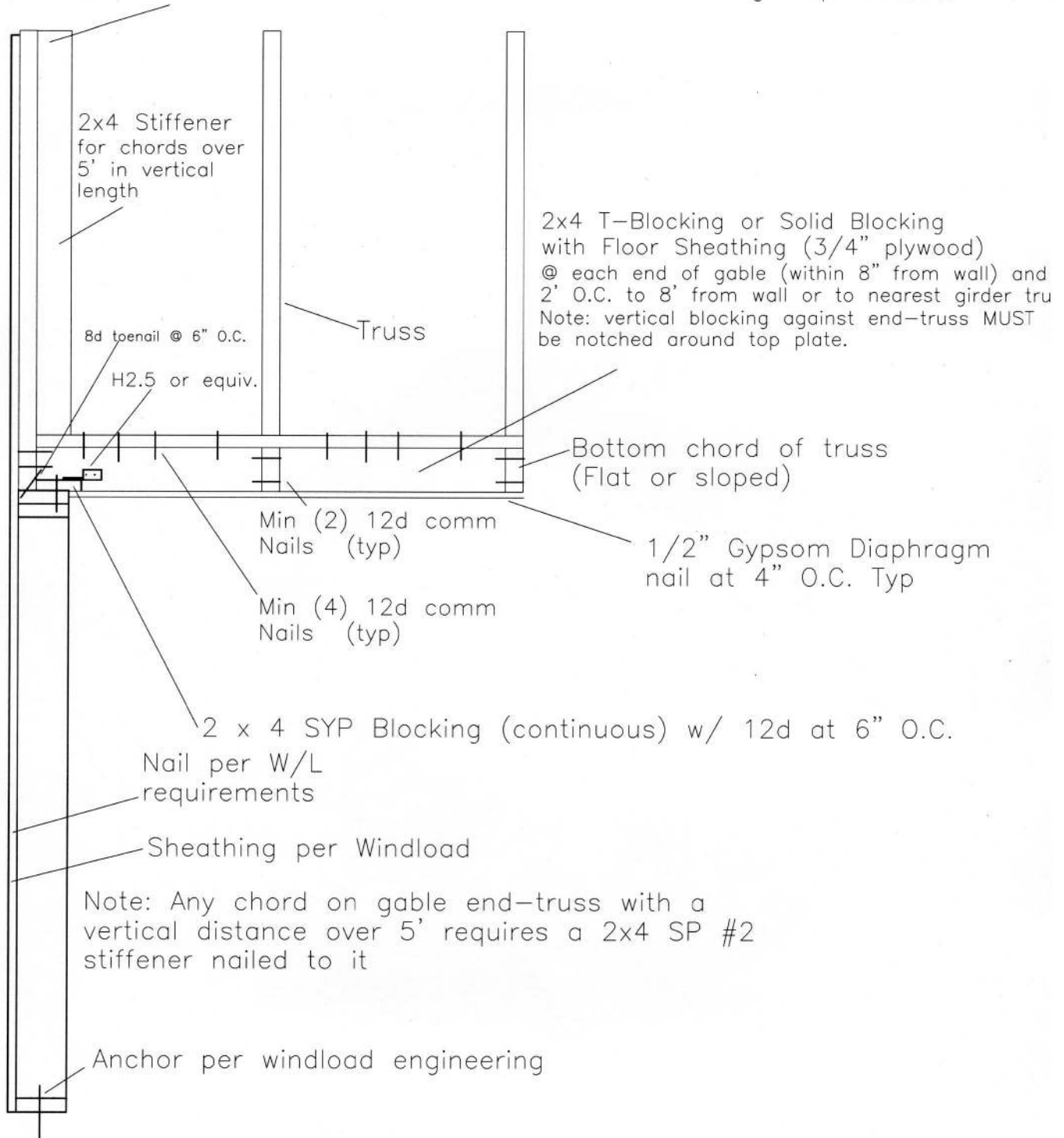
Blocking must be parallel to top and bottom plates with a minimum of 3-12d comm nails



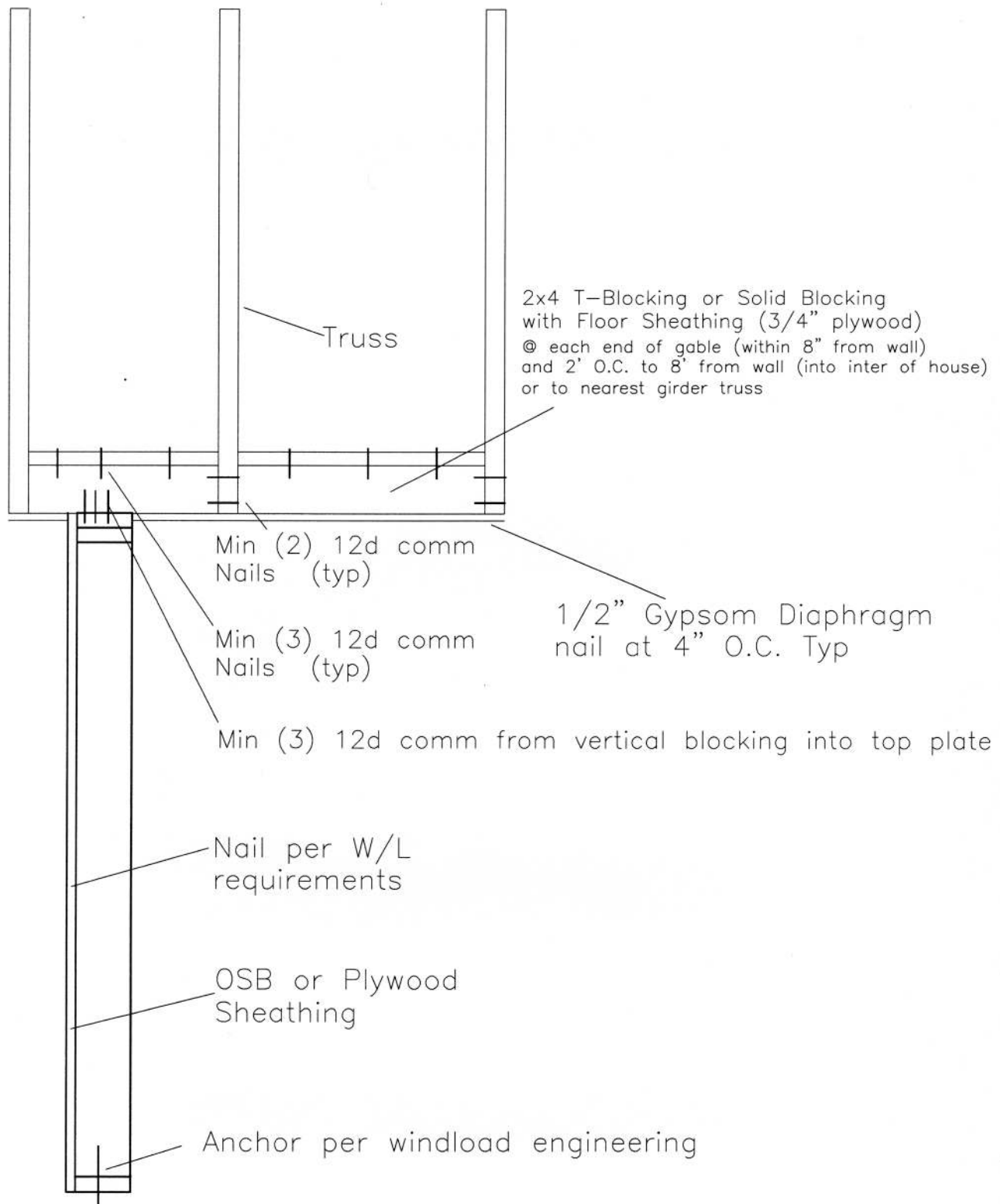
F. Sapienza, P.E.

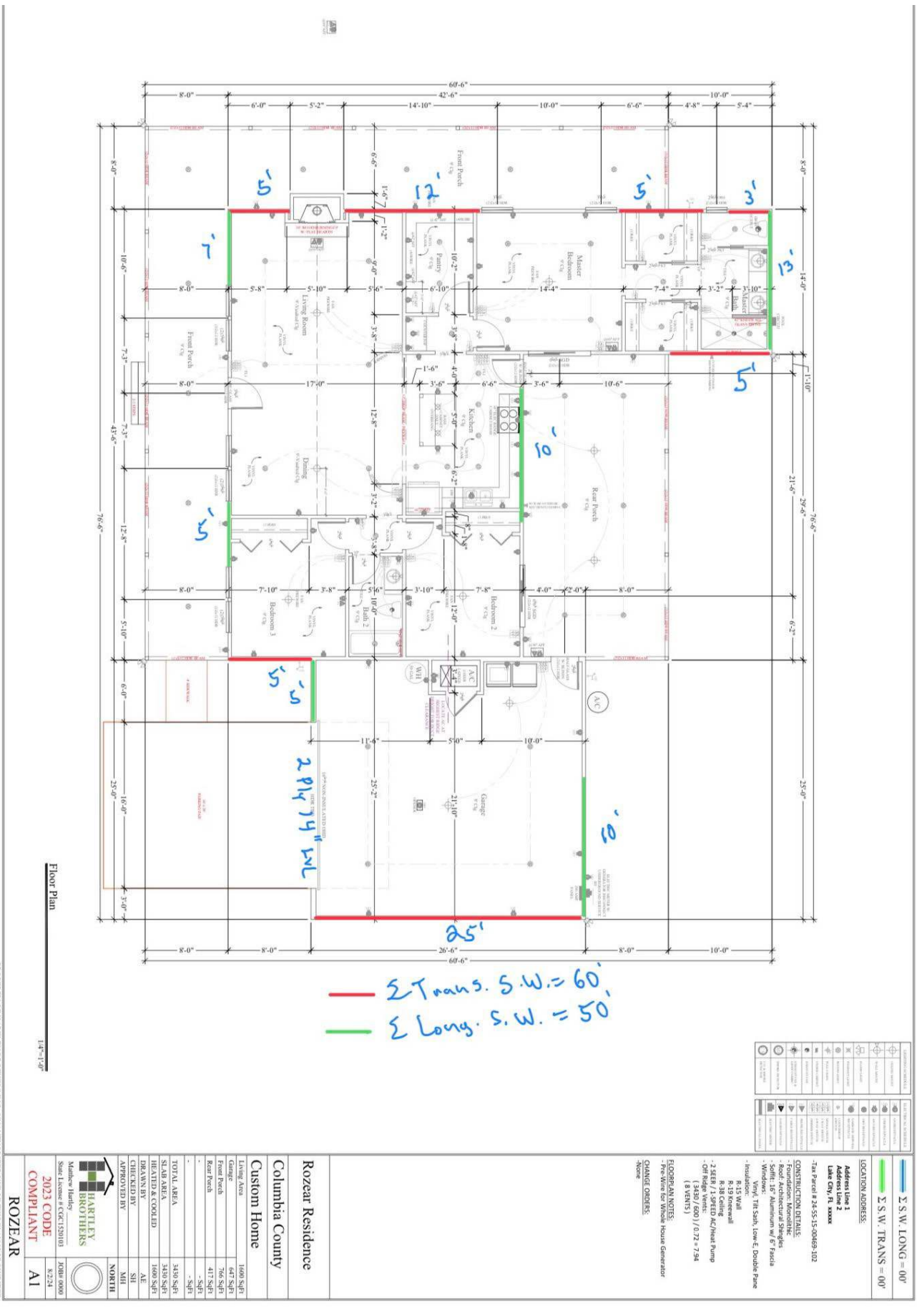
Gable Endwall Framing with Gable End-Truss

See Balloon Framed Detail for Outlooker framing requirements



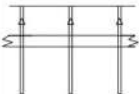
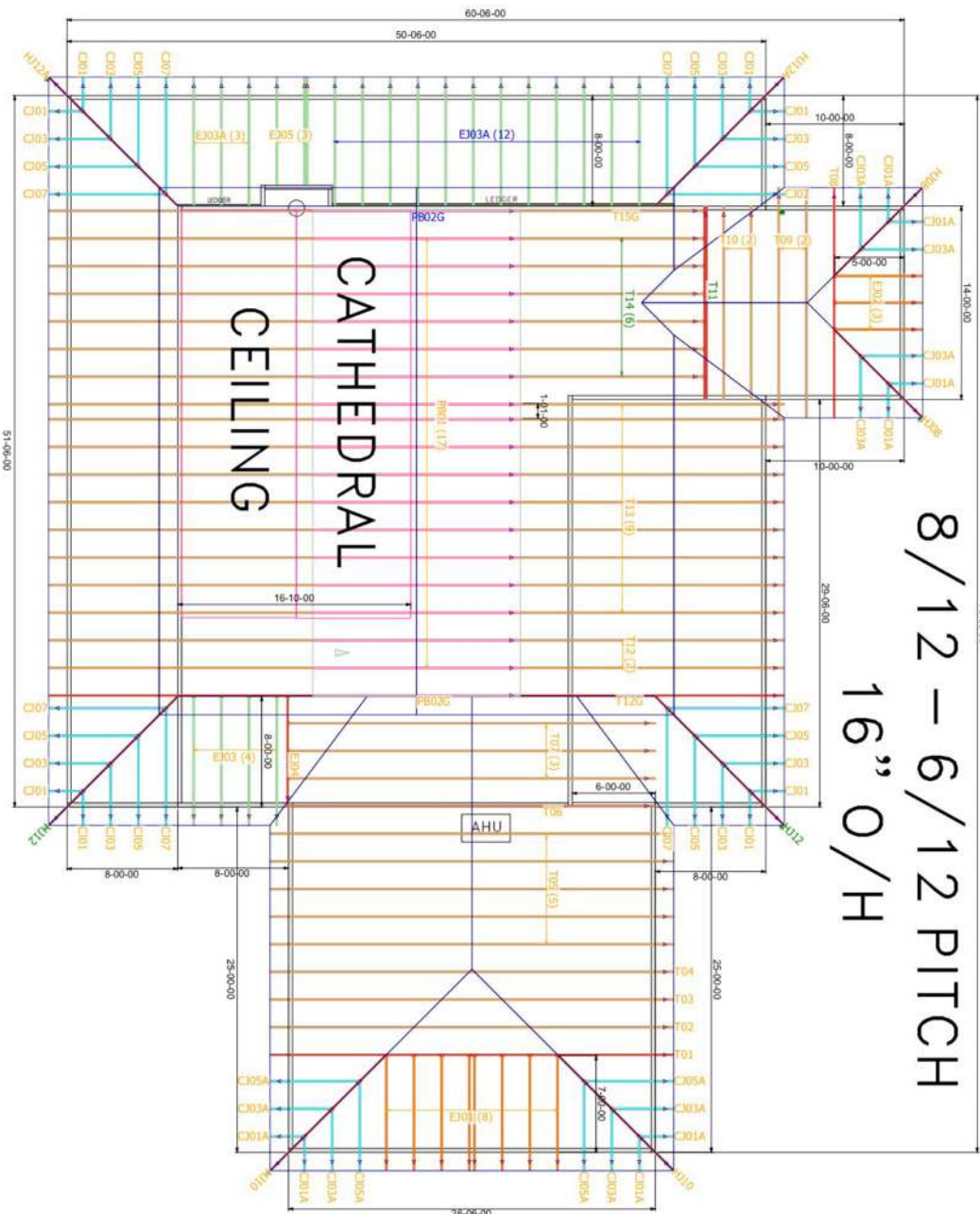
Porch Interior Wall Detail





2 S.W. LONG = 00'	
2 S.W. TRANS = 00'	
LOCATION ADDRESS:	
Address line 1	
Address line 2	
Lake City, FL 32008	
Tax Parcel # 24-55-15-00469-102	
CONSTRUCTION DETAILS:	
Foundation: Monolithic	
Roof: Architectural Shingles	
Siding: 1/2" Aluminum w/ 6" Faccia	
Windows: Vinyl, 1/2" Sash, Low E, Double Pane	
Insulation:	
R-15 Walls	
R-30 Ceiling	
2 SEER / 1 SPEED AC/Heat Pump	
- Off Road Vents	
- 1/2" SDR 35 (1000) / 0.72 = 7.94	
(8 EVENTS)	
ELECTRICAL NOTES:	
Per NEC & Florida House Generator	
CHANGE ORDERS:	
None	
Rozear Residence	
Columbia County	
Custom Home	
Living Area	
Garage	
Front Porch	
Rear Porch	
TOTAL AREA	
SLAB AREA	
HEATED & COOLED	
DRAWN BY	
CHECKED BY	
APPROVED BY	
NORTH	
HARTLEY BROTHERS	
Matthew Hartley	
State License # CC-1530103	
2023 CODE COMPLIANT	
A1	

MITEK PLATE APPROVAL #'S 2197.2-2197.4, BOISE EWP PRODUCT #'S LVL FL1644-R2, BCI JOISTS FL1392-R2



THE ARROW HEAD AT THE
END OF THE TRUSS ON
PLAN LAYOUT
CORRESPONDS WITH THE
INDIVIDUAL TRUSS
IDENTIFICATION TAG AS AN
AID TO IDENTIFY THE
TRUSS BEING REFERRED TO
WHEN SETTING THE
STRUCTURE

General Notes

- 1. The ANSHUT 1-2002 all "Truss to Rafter" connection is to be used for the connection of the Building Trusses, and the trusses shall be installed in accordance with the manufacturer's specifications for all trusses.
- 2. Trusses are to be 2" x 6" U.S.S.
- 3. All trusses are to be Simpson or equivalent U.S.S.O.
- 4. Trusses are to be installed in accordance with the manufacturer's specifications for all trusses.
- 5. Trusses are not designed to support loads other than the dead load of the trusses themselves.

Notes

No back trusses will be required by Building Trusses unless approved in writing from the Manufacturer.

402-503 1011

Any truss is to be installed in truss plates. Any ANY truss which must have an additional truss applied that is not shown on the truss design drawings shall be installed in accordance with the Manufacturer's specifications for all trusses.

It is the responsibility of the Contractor to make sure the placement of trusses are adjusted for plumbness, slope, and type of truss.

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Lake City

PHONE: 386-753-6894

FAX: 386-753-7973

Jacksonville

PHONE: 904-772-6100

FAX: 904-772-1973

Tallahassee

PHONE: 850-576-5177

HARTLEY BROTHERS

Rozeare Rees

Model

Custom

Notes for

8-28-24

KLH

4152007

Sheet 1 of 2

N/A

N/A

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