

Project Information for:

L253081

Builder:

Richard Keen

Address:

162 Old Itchetucknee Road

...

Lake City, FL

County:

Columbia

Truss Count:

16

Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002
Truss Design Load Information:
Gravity: Wind:

Roof (psf): 42.0 Floor (psf): N/A Wind Standard: ASCE 7-02

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

James H. Johnston, III Florida Registered Residential Contractor License No. RR0066976

Wind Exposure:

Address: RT. #15 Box 3693 Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1887385	CJ1	8/30/07
2	J1887386	CJ3	8/30/07
3	J1887387	CJ5	8/30/07
4	J1887388	EJ5	8/30/07
5	J1887389	EJ7	8/30/07
6	J1887390	HIP1	8/30/07
7	J1887391	HJ7	8/30/07
8	J1887392	HJ9	8/30/07
9	J1887393	T01	8/30/07
10	J1887394	T02	8/30/07
11	J1887395	T03	8/30/07
12	J1887396	T04	8/30/07
13	J1887397	T05	8/30/07
14	J1887398	T05A	8/30/07
15	J1887399	T06	8/30/07
16	J1887400	T07	8/30/07



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L253081

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162 Old Itchetucknee Road

Lake City, FL

County:

Columbia

Truss Count:

16

Design Program: MiTek 20/20 6.3 Building Code:

FBC2004/TPI2002

Gravity:

Truss Design Load Information:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

August 30,2007

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

James H. Johnston, III Florida Registered Residential Contractor License No. RR0066976

Address: RT. #15 Box 3693 Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

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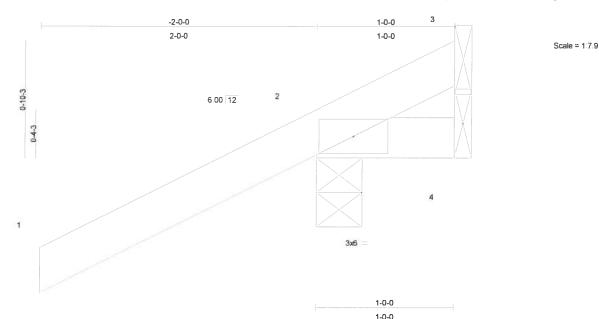
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No.	Drwg. #	Truss ID	Date
1	J1887385	CJ1	8/30/07
2	J1887386	CJ3	8/30/07
3	J1887387	CJ5	8/30/07
4	J1887388	EJ5	8/30/07
5	J1887389	EJ7	8/30/07
6	J1887390	HIP1	8/30/07
7	J1887391	HJ7	8/30/07
8	J1887392	HJ9	8/30/07
9	J1887393	T01	8/30/07
10	J1887394	T02	8/30/07
11	J1887395	T03	8/30/07
12	J1887396	T04	8/30/07
13	J1887397	T05	8/30/07
14	J1887398	T05A	8/30/07
15	J1887399	T06	8/30/07
16	J1887400	T07	8/30/07

Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887385
L253081	CJ1	JACK	12	1	
					Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Aug 30 17:36:22 2007 Page 1



LOADING	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	 Rep Stress Incr 	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/T	PI2002	(Mati	rix)						Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=257/0-4-0, 4=5/Mechanical, 3=-91/Mechanical

Max Horz 2=87(load case 6)

Max Uplift 2=-287(load case 6), 4=-9(load case 4), 3=-91(load case 1) Max Grav 2=257(load case 1), 4=14(load case 2), 3=128(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-70/76

BOT CHORD

2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 4 and 91 lb uplift at joint 3.

LOAD CASE(S) Standard



August 30,2007



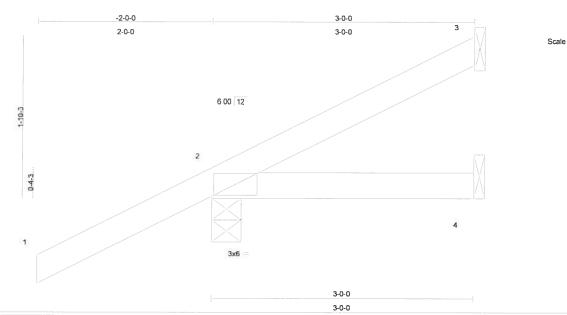
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887386
L253081	CJ3	JACK	12	1	
					Job Reference (optional)
Builders FiretS	ource Lake City El	32055 6	300 c Eeb 15 2006 N	MiTak In	dustries Inc. Thu Aug 20 12:26:44 2007 Page 1

15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:44 2007 Page 1



LOADING TCLL TCDL BCLL	(psf) 20.0 7.0 10.0	SPACING Plates increase Lumber increase * Rep Stress incr	2-0-0 1.25 1.25 YES	CSI TC BC WB	0.30 0.08 0.00	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.01 -0.01 -0.00	(loc) 2-4 2-4 3	I/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	Code FBC2004/TF	PI2002	(Mati	rix)						Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **BRACING**

TOP CHORD Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=251/0-4-0, 4=14/Mechanical

Max Horz 2=132(load case 6)

Max Uplift 3=-27(load case 7), 2=-240(load case 6), 4=-26(load case 4)

Max Grav 3=29(load case 1), 2=251(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-58/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4. Continued on page 2

August 30,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD		
					J1887386		
L253081	CJ3	JACK	12	1			
					Job Reference (optional)		
Builders FirstSource, Lake City, FI 32055			6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:44 2007 Page 2				

LOAD CASE(S) Standard

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Job Truss Truss Type RICHARD KEEN - OLD ITCHETUCKNEE RD Qty Ply J1887387 L253081 CJ5 **JACK** 8 Job Reference (optional) Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:44 2007 Page 1 -2-0-0 5-0-0 2-0-0 5-0-0 6.00 12 0-4-3 3x6 5-0-0 5-0-0 **SPACING** LOADING (psf) 2-0-0 CSI **DEFL** in (loc) I/defi L/d **PLATES GRIP** TCLL 20.0 Plates Increase 1.25 TC 0.30 Vert(LL) -0.032-4 >999 360 MT20 244/190 **TCDL** 7.0 Lumber Increase 1.25 BC 0.16 Vert(TL) -0.05>999 240 2-4 **BCLL** 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 19 lb LUMBER **BRACING** TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 5-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=296/0-4-0, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-86(load case 6), 2=-201(load case 6)

Max Grav 3=102(load case 1), 2=296(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-87/36

BOT CHORD 2-4=0/0

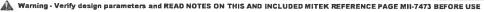
JOINT STRESS INDEX

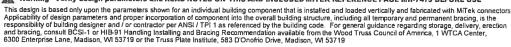
2 = 0.15

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3 and 201 lb uplift at joint 2. Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD			
1					J1887387			
L253081	CJ5	JACK	8	1				
					Job Reference (optional)			
Builders FirstSource, Lake City, FI 32055			6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:44 2007 Page 2					

LOAD CASE(S) Standard

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Job Truss Truss Type Qty RICHARD KEEN - OLD ITCHETUCKNEE RD J1887388 L253081 EJ5 **JACK** Job Reference (optional) Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:44 2007 Page 1 -2-0-0 5-0-0 2-0-0 5-0-0 Scale = 1:16.9 6.00 12 5-0-0 5-0-0 LOADING (psf) **SPACING** 2-0-0 CSI L/d **PLATES GRIP** DEFL in (loc) I/defl **TCLL** 20.0 1.25 Plates Increase TC 0.30 Vert(LL) 0.09 2-4 >672 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.24 Vert(TL) -0.05 2-4 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 3 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 19 lb LUMBER **BRACING** TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 5-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=296/0-4-0, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-261(load case 6), 4=-46(load case 4) Max Grav 3=102(load case 1), 2=296(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-87/36

BOT CHORD

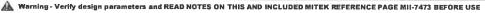
2-4=0/0

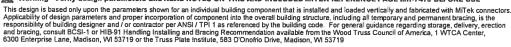
JOINT STRESS INDEX

2 = 0.15

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4. Continued on page 2







Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
			-		J1887388
L253081	EJ5	JACK	4	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:45 2007 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
L253081	EJ7	MONO TRUSS	20	1	J1887389
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:45 2007 Page 1

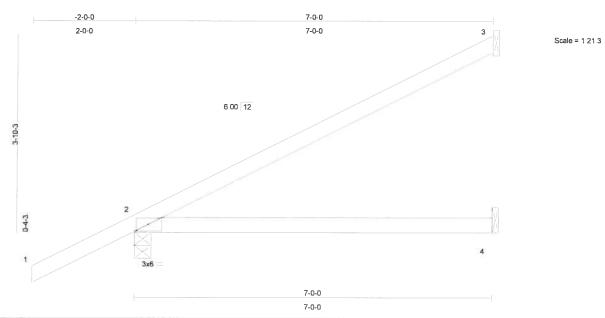


Plate	Offsets	(X.Y):	[2:0-2-12,0-1-8]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>506	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical

Max Horz 2=161(load case 6)

Max Uplift 3=-84(load case 6), 2=-140(load case 6)

Max Grav 3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-119/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.70

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb Contributed is in \$3.20 140 lb up fit at joint 2.

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August 30,2007



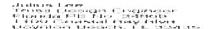
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building odde. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
L253081	EJ7	MONO TOLICS	00		J1887389
L253061	EJ/	MONO TRUSS	20	1	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:45 2007 Page 2

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
1.050004	LUDA	CARLE	2		J1887390
L253081	HIP1	GABLE	2	1	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:46 2007 Page 1

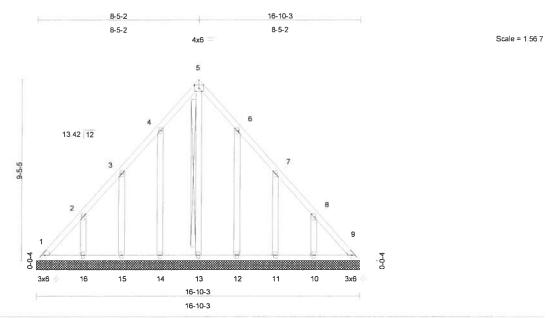


Plate Of	ffsets (X,Y	'): [5:Edge,0-1-14]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	n/a	· -	n/a	999	MT20	244/19
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	гіх)						Weight: 114 lb	

1	4.1	M	Ю	0
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TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -

5-13

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=78/16-10-3, 9=78/16-10-3, 13=83/16-10-3, 14=128/16-10-3,

15=123/16-10-3, 16=150/16-10-3, 12=128/16-10-3, 11=123/16-10-3,

10=150/16-10-3

Max Horz 1=326(load case 5)

Max Uplift 1=-112(load case 4), 9=-62(load case 5), 14=-165(load case 6),

15=-177(load case 6), 16=-208(load case 6), 12=-163(load case 7),

11=-178(load case 7), 10=-208(load case 7)

Max Grav 1=234(load case 5), 9=202(load case 7), 13=206(load case 7), 14=133(load

case 10), 15=123(load case 1), 16=150(load case 10), 12=133(load case

11), 11=123(load case 1), 10=150(load case 11)

Arabanea Lemen Ananger Grandagan Lengtingangu Albertakan Mili Politi dindpeksib Lukinda Caramburah dinang dinband

August 30,2007

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887390
L253081	HIP1	GABLE	2	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:46 2007 Page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-329/179, 2-3=-220/158, 3-4=-119/145, 4-5=-71/202, 5-6=-71/202, 6-7=-72/79, 7-8=-153/91, 8-9=-300/112
BOT CHORD 1-16=-75/248, 15-16=-75/248, 14-15=-75/248, 13-14=-75/248, 12-13=-75/248, 11-12=-75/248, 10-11=-75/248, 9-10=-75/248

9-10=-/5/246

WEBS 5-13=-203/0, 4-14=-112/176, 3-15=-106/194, 2-16=-118/202, 6-12=-112/174, 7-11=-106/195, 8-10=-118/202

JOINT STRESS INDEX

1 = 0.23, 2 = 0.33, 3 = 0.33, 4 = 0.33, 5 = 0.34, 6 = 0.33, 7 = 0.33, 8 = 0.33, 9 = 0.23, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33 and 16 = 0.33

NOTES

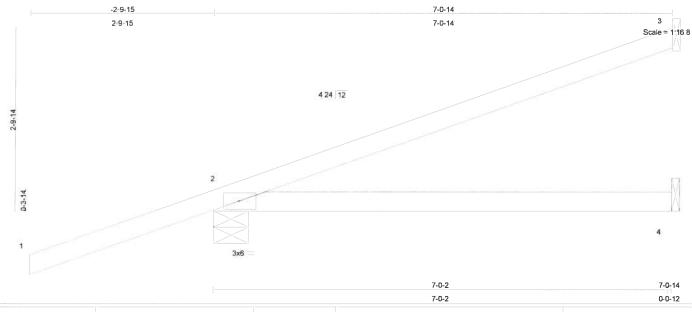
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 1, 62 lb uplift at joint 9, 165 lb uplift at joint 14, 177 lb uplift at joint 15, 208 lb uplift at joint 16, 163 lb uplift at joint 12, 178 lb uplift at joint 11 and 208 lb uplift at joint 10.

LOAD CASE(S) Standard





Job Truss Truss Type Qty Ply RICHARD KEEN - OLD ITCHETUCKNEE RD J1887391 L253081 HJ7 MONO TRUSS 2 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:47 2007 Page 1 -2-9-15 7-0-14



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.10	2-4	>773	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.13	2-4	>618	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 26 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

7-0-14 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=183/Mechanical, 2=341/0-6-6, 4=37/Mechanical

Max Horz 2=168(load case 3)

Max Uplift 3=-153(load case 3), 2=-338(load case 3), 4=-55(load case 6) Max Grav 3=183(load case 1), 2=341(load case 1), 4=96(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-71/42

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.51

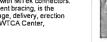
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 Ib uplift at joint 3, 338 lb uplift at joint 2 and 55 lb uplift at joint 4.

August 30,2007

Continued on page 2

🗥 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887391
L253081	HJ7	MONO TRUSS	2	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:47 2007 Page 2

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-3=-95(F=-21, B=-21), 2=0(F=5, B=5)-to-4=-18(F=-4, B=-4)

Audinarea (Leonor Francia (Leonorgian d'Enggarga cab Escalacia (Refui Paleo) de Electrica (Leonorgia (Refui Paleo) de Leonorgia (Leonorgia (Leonorgia) de La disentation



Job		Truss		Truss Ty	rpe	C	ty F	Ply	RIC	HARD K	EEN - OL	D ITCHETUC	KNEE RD J1887392
L253081		HJ9		MONO T	TRUSS	4		1					01007032
Builders Fi	rstSource,	Lake City, FI	32055		6.3	300 s Feb 15 20	006 Mil	Fek In			e (optiona Thu Aug :		007 Page 1
	-2-9-	15	E		4-3-0						9-10-13		
	2-9-1	15			4-3-0						5-7-13		4 Scale = 1 21 2
													i.J
					4	1 24 12							
4						3	x6						
3-8-14						3							
			2										
0-3-14						0		wa harea war-a wa					
1			3x6			7 2x4							6 5 3x6 = 0.10.13
			JAO		4-3-0	244				9	-5-9		3x6 9-10-13 9-19-10-15
					4-3-0						-2-9		0-4-6-0-2
													0-0-12
LOADING TCLL	(psf) 20.0	SPACING Plates Incre	220	2-0-0 1.25	CSI TC 0.61	DEFL Vert(LL)	0.0		(loc) 6-7	I/defl >999	L/d 360	PLATES MT20	GRIP 244/190
TCDL	7.0	Lumber Inci		1.25	BC 0.40		-0.1		6-7	>986	240	IVITZU	244/150
BCLL		Rep Stress		NO	WB 0.34				5	n/a	n/a		
BCDL	5,0	Code FBC2	2004/TPI	2002	(Matrix)							Weight: 4	5 lb
LUMBER						BRACIN	-						
		SYP No.2 SYP No.2				TOP CH	ORD			ral wood c purlins		ng directly ap	plied or
WEBS		1 SYP No.3				BOT CH	ORD					lied or 10-0-0) oc
-						_ 0 . 011					>>		

bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=458/0-6-6, 5=217/Mechanical

Max Horz 2=270(load case 3)

Max Uplift 4=-232(load case 3), 2=-284(load case 3), 5=-61(load case 3)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/50, 2-3=-642/116, 3-4=-105/65

BOT CHORD

2-7=-305/593, 6-7=-305/593, 5-6=0/0

WEBS

3-7=0/189, 3-6=-618/317

JOINT STRESS INDEX

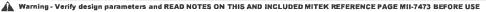
2 = 0.78, 3 = 0.16, 6 = 0.17 and 7 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 4, 284 lb uplift at joint 2 and 61 lb uplift at joint 5.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

An Albanea (League) Privata Calendrican (Entrephysioner) Plaine o Azon (Pala Balo) - 19-8 (1949) Privata (Pala Balo) - 1940 - 1940 - 1940 Privata (Pala Balo) - 1940 - 1940 - 1940 - 1940 Privata (Pala Balo) - 1940







Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887392
L253081	HJ9	MONO TRUSS	4	1	Joh Deference (actional)
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:47 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

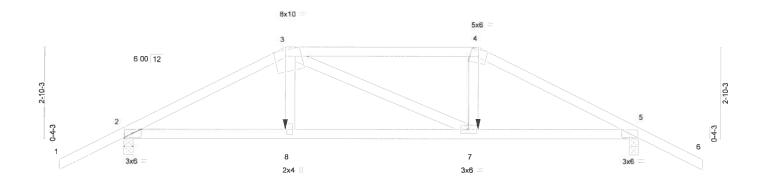
Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Judinary, Larger
Transpared Conservation (Linguistics or Albertalia) Filter (1-1860) Children (1-1860)



Job	Truss	Truss Type	C	Qty	Ply	RICHARD KEEN - OLD ITCI	HETUCKNEE RD J1887393
L253081	T01	HIP	1	}	1		
						Job Reference (optional)	
Builders FirstSo	ource, Lake City, FI 3	32055	6.300 s Feb 15 20	006 N	liTek In	dustries, Inc. Thu Aug 30 12:	36:48 2007 Page 1
-2-0-0	5-0-	0	11-0-0			16-0-0	18-0-0
2-0-0	5-0-	0	6-0-0			5-0-0	2-0-0 Scale = 1.33.9



5-0-0	5-1-12	10-10-4	11:0-0	16 0-0	
5-0-0	0-1-12	5-8-8	0-1-12	5-0-0	

Plate Offsets	(X Y):	[3:0-4-3,Edge]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.09	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.12	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.09	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	P12002	(Mat	rix)	, ,					Weight: 72 lb	

	 W 40	-	$\overline{}$	
,	 М	ĸ	_	×

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-8-11 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-6-15 oc

bracing.

REACTIONS (lb/size) 2=934/0-3-8, 5=934/0-3-8

Max Horz 2=-73(load case 6)

Max Uplift 2=-733(load case 5), 5=-733(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1516/1056, 3-4=-1314/987, 4-5=-1516/1056, 5-6=0/47

BOT CHORD 2-8=-910/1299, 7-8=-923/1313, 5-7=-884/1299

WEBS 3-8=-220/260, 4-7=-226/285, 3-7=-77/78

JOINT STRESS INDEX

2 = 0.65, 3 = 0.57, 4 = 0.61, 5 = 0.65, 7 = 0.18 and 8 = 0.18

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Example 2 crushing capacity of 565.00 psi

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August 30,2007

🞪 Warning - Vorify dosign paramotors and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building design read / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887393
L253081	T01	HIP	1	1	
					Job Reference (optional)
Duildoss EiratCou	con Lako City El 1	22055 6.3	200 a Eab 45 2006 N	AiTak In	dustrian Inc. Thu Aug 20 12:26:49 2007 Dags 2

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:48 2007 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 733 lb uplift at joint 2 and 733 lb uplift at joint 5.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

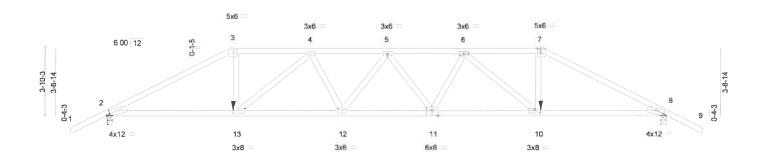
Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-10, 7-8=-17(F=-7), 5-7=-10

Concentrated Loads (lb)

Vert: 8=-187(F) 7=-187(F)



Job	Truss	Truss Type		Qty	Ply	RICHARD KI	EEN - OLD ITCHET	UCKNEE RD J188739
L253081	T02	HIP		2	1			0.00.00
						Job Referenc	e (optional)	
Builders FirstSou	rce, Lake City, FI 32	2055	6.300 s F	eb 15 2006	MiTek In	ndustries, Inc.	Thu Aug 30 12:36:4	9 2007 Page 1
-2-0-0	7-0-0	11-3-0	15-6-0	19-9-0		24-0-0	31-0-0	33-0-0
2-0-0	7-0-0	4-3-0	4-3-0	4-3-0		4-3-0	7-0-0	2-0-0



7-0-0	13-0-0	18-0-0	24-0-0	31-0-0	
7-0-0	6-0-0	5-0-0	6-0-0	7-0-0	

Plate Offsets (X,Y): [2:0-1-12,Edge], [3:0-2-8,0-2-1], [7:0-2-8,0-2-1], [8:0-1-12,Edge], [10:0-3-8,0-1-8], [11:0-4-0,Edge], [13:0-3-8,0-1-8]

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.33	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.64	11-12	>580	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.20	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 149 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

2-8-7 oc purlins, except

2-0-0 oc purlins (2-7-4 max.): 3-7.

BOT CHORD Rigid ceiling directly applied or 5-3-1 oc

bracing.

REACTIONS (lb/size) 2=2148/0-3-8, 8=2148/0-3-8

Max Horz 2=-77(load case 6)

Max Uplift 2=-677(load case 5), 8=-677(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-4107/1319, 3-4=-3648/1221, 4-5=-4895/1619, 5-6=-4895/1619,

6-7=-3648/1221, 7-8=-4107/1319, 8-9=0/47

BOT CHORD 2-13=-1141/3587, 12-13=-1537/4706, 11-12=-1644/5080, 10-11=-1507/4706,

8-10=-1108/3587

WEBS 3-13=-455/1461, 4-13=-1479/564, 4-12=-80/441, 5-12=-338/171, 5-11=-338/171,

6-11=-80/441, 6-10=-1479/564, 7-10=-455/1461

Indicate Large Tennescon Conguestics Tennescon Tennescon Pill Polici Marroccy (1970) and tennescon Conguestics (1970) and tennescon

JOINT STRESS INDEX

2 = 0.77, 3 = 0.96, 4 = 0.44, 5 = 0.39, 6 = 0.44, 7 = 0.96, 8 = 0.77, 10 = 0.92, 11 = 0.96, 12 = 0.44 and 13 = 0.92

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp CoRingslose (34) FRS; Lumber DOL=1.60 plate grip DOL=1.60.

August 30,2007



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J188739
L253081	T02	HIP	2	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:49 2007 Page 2

NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 677 lb uplift at joint 2 and 677 lb uplift at joint 8.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

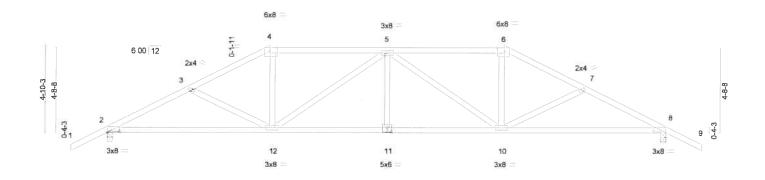
Vert: 1-3=-54, 3-7=-118(F=-64), 7-9=-54, 2-13=-10, 10-13=-22(F=-12), 8-10=-10

Concentrated Loads (lb)

Vert: 13=-411(F) 10=-411(F)



Job	Truss	Tr	russ Type	Qty	Ply	RICHARD KEEN - 0	OLD ITCHETU	CKNEE RD J1887395
L253081	T03	Н	IP .	2	1			31007333
						Job Reference (option	nal)	
Builders FirstSou	ırce, Lake City,	FI 32055	6.300 s	Feb 15 2006 i	MiTek In	dustries, Inc. Thu Au	g 30 12:36:50	2007 Page 1
-2-0-0	4-8-0	9-0-0	15-6-0	22-0	0-0	26-4-0	31-0-0	33-0-0
2-0-0	4-8-0	4-4-0	6-6-0	6-6	-0	4-4-0	4-8-0	2-0-0 Scale = 1:60:3



		9-0-0		6-6-0		6-6-0		9-0-0		
Plate Offsets (X,Y)		[2:0-8-4,0-0-10], [4:0-4-10,E	dge], [6:0-4-10,I	Edge], [8:0-8-4	4,0-0-10], [11:0	0-3-0,0-3-0)]		
LOADING	G (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	-0.15 8-10	>999	360	MT20	244/190

22-0-0

TCLL 20.0 Plates Increase 1.25 0.32Vert(LL) -0.15 8-10 >999 360 M120 7.0 TCDL Lumber Increase 1.25 BC 0.54 Vert(TL) -0.292-12 >999 240 * Rep Stress Incr **BCLL** 10.0 YES WB 0.49 0.08 Horz(TL) 8 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 157 lb

15-6-0

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 5tructural wood sheathing directly applied or 4-7-5 oc purlins, except 2-0-0 oc purlins (5-1-2 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 7-5-11 oc bracing

REACTIONS (lb/size) 2=1099/0-3-8, 8=1099/0-3-8

Max Horz 2=-88(load case 7)

9-0-0

Max Uplift 2=-270(load case 6), 8=-270(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1808/946, 3-4=-1596/855, 4-5=-1410/828, 5-6=-1410/828,

6-7=-1596/855, 7-8=-1808/946, 8-9=0/47

BOT CHORD 2-12=-679/1550, 11-12=-701/1728, 10-11=-701/1728, 8-10=-679/1550

WEBS 3-12=-178/179, 4-12=-143/415, 5-12=-478/217, 5-11=0/150, 5-10=-478/217,

6-10=-143/415, 7-10=-178/179

JOINT STRESS INDEX

2 = 0.71, 3 = 0.33, 4 = 0.69, 5 = 0.56, 6 = 0.69, 7 = 0.33, 8 = 0.71, 10 = 0.56, 11 = 0.40 and 12 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

8) Rinvide adeguate drainage to prevent water ponding.

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31-0-0

August 30,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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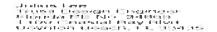
Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887395
L253081	T03	HIP	2	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:50 2007 Page 2

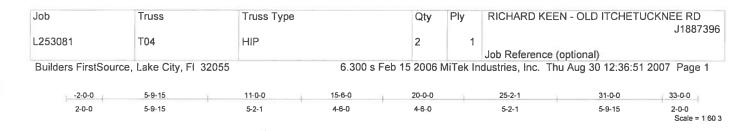
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 270 lb uplift at joint 2 and 270 lb uplift at joint 8.

LOAD CASE(S) Standard







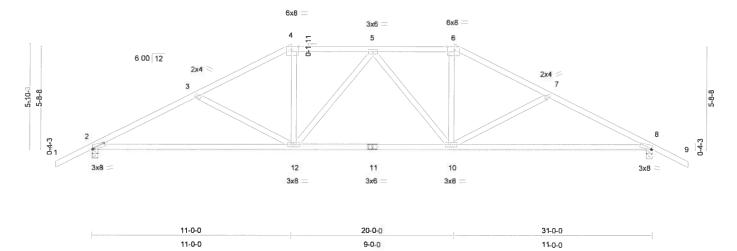


Plate Of	ffsets (X,Y): [2:0-0-10,Edge], [4	1:0-4-10,E	dge], [6	:0-4-10,I	Edge], [8:0-0-	10,Edge]				
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.31	2-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.57	2-12	>643	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	,					Weight: 156 lb	

2 X 4 SYP No.2
2 X 4 SYP No.2
2 X 4 SYP No.3

LUMBED

BRACING

TOP CHORD Structural wood sheathing directly applied or

4-6-10 oc purlins, except

2-0-0 oc purlins (5-6-3 max.): 4-6.

BOT CHORD Rigid ceiling directly applied or 7-7-3 oc

bracing.

REACTIONS (lb/size) 2=1098/0-4-0, 8=1098/0-4-0

Max Horz 2=100(load case 6)

Max Uplift 2=-285(load case 6), 8=-285(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1774/957, 3-4=-1470/817, 4-5=-1274/794, 5-6=-1274/794,

6-7=-1470/817, 7-8=-1774/957, 8-9=0/47

BOT CHORD 2-12=-678/1516, 11-12=-504/1376, 10-11=-504/1376, 8-10=-678/1516

WEBS 3-12=-295/274, 4-12=-146/397, 5-12=-275/117, 6-10=-146/397, 7-10=-295/274,

5-10=-275/117

JOINT STRESS INDEX

2 = 0.91, 3 = 0.33, 4 = 0.63, 5 = 0.39, 6 = 0.63, 7 = 0.33, 8 = 0.91, 10 = 0.56, 11 = 0.47 and 12 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

∂ hravide adequate drainage to prevent water ponding.

Altelates Levers
"Frequency Commercial Engirements
#Fitters date Prim Fitter In-1888 (ND)
1-8 (SA) In-material France Fitters
#Alterations at the Commercial Engire

August 30,2007

🚵 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

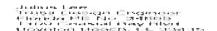


Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887396
L253081	T04	HIP	2	1	
					Job Reference (optional)
Builders FirstSource, Lake City, FI 32055			s Feb 15 2006 N	1iTek In	dustries, Inc. Thu Aug 30 12:36:51 2007 Page 2

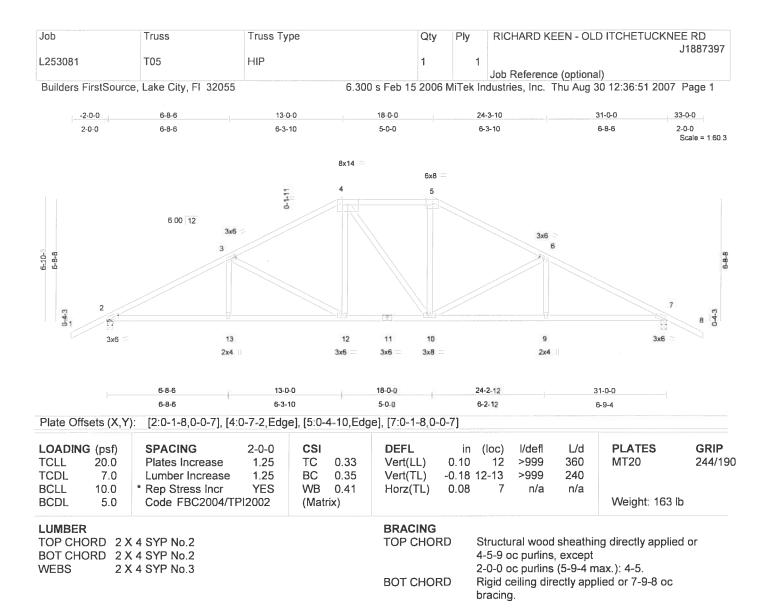
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2 and 285 lb uplift at joint 8.

LOAD CASE(S) Standard







REACTIONS (lb/size) 2=1099/0-3-8, 7=1099/0-3-8

Max Horz 2=112(load case 6)

Max Uplift 2=-296(load case 6), 7=-296(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1794/938, 3-4=-1342/793, 4-5=-1141/779, 5-6=-1342/793,

6-7=-1794/938, 7-8=0/47

BOT CHORD 2-13=-657/1524, 12-13=-657/1524, 11-12=-371/1140, 10-11=-371/1140,

9-10=-657/1524, 7-9=-657/1524

WEBS 3-13=0/213, 3-12=-444/326, 4-12=-122/309, 4-10=-152/153, 5-10=-122/309,

6-10=-444/326, 6-9=0/213

JOINT STRESS INDEX

2 = 0.77, 3 = 0.39, 4 = 0.68, 5 = 0.68, 6 = 0.39, 7 = 0.77, 9 = 0.33, 10 = 0.56, 11 = 0.37, 12 = 0.34 and 13 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Coffiling designed for C-C for members and forces, and for MWFRS for reactions specified.

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August 30,2007

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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
L253081	T05	HIP	1	1	J1887397
					Job Reference (optional)
Builders FirstSource	6.300 s Fe	eb 15 2006 N	/liTek In	dustries, Inc. Thu Aug 30 12:36:51 2007 Page 2	

NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2 and 296 lb uplift at joint 7.

LOAD CASE(S) Standard



Job	Truss	Truss Type		Qty	Ply	RICHARD	KEEN - OL	D ITCHETUCKN	IEE RD J1887398
253081	T05A	PIGGYBACK S	CISSOR	1	1	Job Refere	nce (entions	-1\	01001000
Builders FirstSou	ce, Lake City, FI 32055		6.300 s Ap	r 19 2006	MiTek In	dustries, Inc.	Thu Aug 3	0 17:39:21 2007	Page 1
-2-0-0	6-11-4	13-0-0	18-0-0	9	24-0	-12	31	-0-0	33-0-0
2-0-0	6-11-4	6-0-12	5-0-0		6-0-	-12	6-	11-4	2-0-0 Scale = 1 59
			2.0						
			8x14 =	8x14					
1		11.11	4	5					r
	6.00 12								
		6				3x6			
8-8-9	3		业业	HEL		- 1			
i 40			11 12 5x6	10		3 42			
			3x6 =	3x6 1-0-0			4	7	3-9-8
2		13			8	9			8 4.3
3x8		3.00 12				13-	1-0	ு⊡ 5x12 MT2∫H	
	800	13-0-0	15-6-0 17	18-0-0 -11-0	23-0-0	25-1	11-0	31-0-0	
	8-0-0	5-0-0		-5-0 0-1-0	5-0-0	2-1		5-1-0	
Plate Offsets (X, Y	'): [4:0-7-2,Edge], [5:0	-7-2,Edge], [7:0-9-	2,0-0-0]						
OADING (psf)	SPACING	2-0-0 CSI	DE	FL.	in (l	oc) l/defl	L/d	PLATES	GRIP
TCLL 20.0 TCDL 7.0	Plates Increase Lumber Increase	1.25 TC 1.25 BC		rt(LL) rt(TL)	0.37 -0.66	11 >983 11 >558	360 240	MT20 MT20H	244/190 187/143
3CLL 10.0	* Rep Stress Incr	YES WB		rz(TL)	0.49	7 n/a	n/a	WITZOIT	10//143
3CDL 5.0	Code FBC2004/T	PI2002 (Ma	rix)					Weight: 173	lb
JODE 5.0									
.UMBER	X 4 SYP No.2			ACING P CHORI				directly applied	

WEBS 2 X 4 SYP No.3 2 X 4 SYP No.3 OTHERS

2-0-0 oc purlins (3-8-10 max.): 4-5.

Rigid ceiling directly applied or 5-7-5 oc bracing.

Except:

BOT CHORD

1 Row at midpt

7-10

REACTIONS (lb/size) 2=1099/0-3-8, 7=1099/0-3-8

Max Horz 2=111(load case 6)

Max Uplift 2=-296(load case 6), 7=-296(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3262/1565, 3-4=-2549/1189, 4-5=-2733/1294, 5-6=-2549/1189,

6-7=-3262/1565, 7-8=0/46

BOT CHORD 2-13=-1249/2910, 12-13=-1252/2912, 11-12=-752/2297, 10-11=-752/2297,

9-10=-1252/2912, 7-9=-1249/2910

WEBS 3-13=0/202, 3-12=-619/485, 4-12=-131/304, 4-11=-233/731, 5-11=-233/731, 5-10=-131/304,

6-10=-619/485, 6-9=0/202

JOINT STRESS INDEX

2 = 0.77, 3 = 0.41, 4 = 0.74, 5 = 0.74, 6 = 0.41, 7 = 0.78, 9 = 0.34, 10 = 0.38, 11 = 0.83, 12 = 0.38, 13 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 17 = 0.34, 18 = 0.340.34, 18 = 0.34, 19 = 0.34 and 20 = 0.34Jidhus Esse Terres Cossess Circumpter Microsoft Circ States Microsoft States Microsoft States

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is Confinite of page 2 for members and forces, and for MWFRS for reactions specified.

August 30,2007

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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
L253081	T05A	PIGGYBACK SCISSOR	1	1	J1887398
L253061	105A	FIGGIBACK SCISSOR	1		Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Aug 30 17:39:21 2007 Page 2

NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2 and 296 lb uplift at joint 7.

LOAD CASE(S) Standard



Job Truss Truss Type Qty Ply RICHARD KEEN - OLD ITCHETUCKNEE RD J1887399 L253081 T06 PIGGYBACK SCISSOR 2 1 Job Reference (optional) 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:53 2007 Page 1 Builders FirstSource, Lake City, FI 32055 -2-0-0 8-0-0 15-0-0 16-0-0 23-0-0 31-0-0 2-0-0 8-0-0 7-0-0 1-0-0 7-0-0 8-0-0 Scale = 1:58.4 5x8 5x8 6 00 12 5x6 5x6 7-10-3 10 11 5x6 3x6 3x6 12 2x4 2x4 3.00 12 3x8 8-0-0 14-0-2 15-6-0 16-11-14 23-0-0 31-0-0 8-0-0 6-0-2 1-5-14 1-5-14 6-0-2 8-0-0 Plate Offsets (X,Y): [3:0-3-0,0-3-4], [6:0-3-0,0-3-4] LOADING (psf) CSI **DEFL PLATES GRIP** SPACING 2-0-0 I/defl L/d in (loc) 20.0 244/190 **TCLL** Plates Increase 1.25 TC 0.69 Vert(LL) 0.39 11 >933 360 MT20 **TCDL** 7.0 Lumber Increase 1.25 BC 0.76 Vert(TL) -0.64 11-12 >579 240 * Rep Stress Incr **BCLL** 10.0 YES WB 0.55 Horz(TL) 0.47 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 Weight: 148 lb (Matrix) **BRACING** LUMBER TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 2-7-13 oc purlins, except WERS 2 X 4 SYP No.3 2-0-0 oc purlins (3-11-8 max.): 4-5. Rigid ceiling directly applied or 4-9-14 oc **BOT CHORD** bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 7=979/0-3-8

Max Horz 2=134(load case 6)

Max Uplift 2=-307(load case 6), 7=-212(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3238/1670, 3-4=-2424/1250, 4-5=-2084/1178, 5-6=-2378/1228,

6-7=-3282/1744

BOT CHORD 2-12=-1410/2887, 11-12=-1411/2881, 10-11=-804/2089, 9-10=-768/2025,

8-9=-1484/2924, 7-8=-1489/2934

WEBS 3-12=0/230, 3-11=-729/551, 4-11=-179/358, 4-10=-94/423, 5-10=-337/753,

5-9=-204/376, 6-9=-814/656, 6-8=0/234

JOINT STRESS INDEX

2 = 0.74, 3 = 0.81, 4 = 0.66, 5 = 0.66, 6 = 0.81, 7 = 0.74, 8 = 0.33, 9 = 0.39, 10 = 0.73, 11 = 0.39 and 12 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Cofficial designed for C-C for members and forces, and for MWFRS for reactions specified.

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August 30,2007



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Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
					J1887399
L253081	T06	PIGGYBACK SCISSOR	2	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:54 2007 Page 2

NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 2 and 212 lb uplift at joint 7.

LOAD CASE(S) Standard





Job	Truss	Truss Ty	rpe	(Qty F	Ply	RIC	HARD K	EEN - OL	D ITCHETUCKN	IEE RD J1887400
L253081	T07	SCISSO	RS	7	'	1		Referenc	e (optiona	al)	
Builders FirstSou	rce, Lake City, FI 32055		6.30	00 s Feb 15 2	006 Mi	Tek Ir				30 12:36:54 200	7 Page 1
-2-0-0	8-0-0		15-6-0			3-0-0				31-0-0	
2-0-0	8-0-0		7-6-0		7	-6-0				8-0-0	Scale = 1:57.4
				5x6 =							
		6	00 12	4							
		5x6						5x6			
9		3						5			
7											
				8 5x8 =				12.			wp
		9						7			6 6
F ₁ 2		2x4						2x4			04-3
3x8		3.00 12									3×8
	8-0-0		15-6-0		23	3-0-0				31-0-0	
DI-1- O#1- (V	8-0-0	00000	7-6-0	,	7-	-6-0				8-0-0	
Plate Offsets (X,											
LOADING (psf) TCLL 20.0	SPACING Plates Increase	2-0-0 1.25	CSI TC 0.69	DEFL Vert(LL)	0.4		(loc) 8-9	l/defl >896	L/d 360	PLATES MT20	GRIP 244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.76	Vert(TL)	-0.6	68	8-9	>546	240		4
BCLL 10.0 BCDL 5.0	* Rep Stress Incr Code FBC2004/TI	YES PI2002	WB 0.95 (Matrix)	Horz(TL) 0.4	49	6	n/a	n/a	Weight: 136	lb
LUMBER				BRACIN	IG						
TOP CHORD 2 BOT CHORD 2				TOP CH	ORD			ral wood		ng directly appli	ed or
	X 4 SYP No.3			вот сн	ORD					olied or 4-9-14 o	C

bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 6=979/0-3-8

Max Horz 2=139(load case 6)

Max Uplift 2=-309(load case 6), 6=-214(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3239/1678, 3-4=-2265/1155, 4-5=-2265/1156, 5-6=-3283/1749

BOT CHORD 2-9=-1418/2888, 8-9=-1419/2884, 7-8=-1489/2927, 6-7=-1493/2935

WEBS 3-9=0/243, 3-8=-905/655, 4-8=-708/1543, 5-8=-949/724, 5-7=0/246

JOINT STRESS INDEX

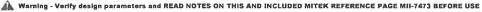
2 = 0.74, 3 = 0.81, 4 = 0.70, 5 = 0.81, 6 = 0.74, 7 = 0.33, 8 = 0.89 and 9 = 0.33

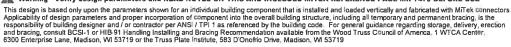
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2









Job	Truss	Truss Type	Qty	Ply	RICHARD KEEN - OLD ITCHETUCKNEE RD
L253081	T07	SCISSORS	7	1	J1887400
L200001	107	001000110	,	'	Job Reference (optional)
Puildore FiretCour	on Lake City El	22055	200 a Eab 15 2006 N	KITOL IN	dustrian Inc. Thu Aug 20 12:26:54 2007 Dags 2

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Aug 30 12:36:54 2007 Page 2

NOTES

- 5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 2 and 214 lb uplift at joint 6.

LOAD CASE(S) Standard



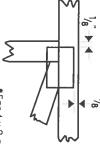


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless Dimensions are in inches. Apply dimensions indicate otherwise. securely seat. plates to both sides of truss and



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



This symbol indicates the connector plates required direction of slots in

PLATE SIZE

4 × 4

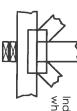
to slots. dimension is the length paralle perpendicular to slots. Second The first dimension is the width

LATERAL BRACING



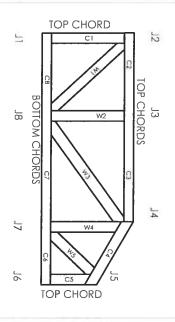
continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA

96-31, 96-67

IC80

3907, 4922

SBCCI

9667, 9432A

WISC/DILHR 960022-W, 970036-N

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561





MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 2 Cut members to bear tightly against each other.
- 4. ယ Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- Ç Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 0 Unless expressly noted, this design is not preservative treated lumber. applicable for use with fire retardant or
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- shown indicate minimum plating requirements. Plate type, size and location dimensions
- Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 tt. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 12. Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- 13. Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate engineer. without prior approval of a professional
- Care should be exercised in handling erection and installation of trusses.

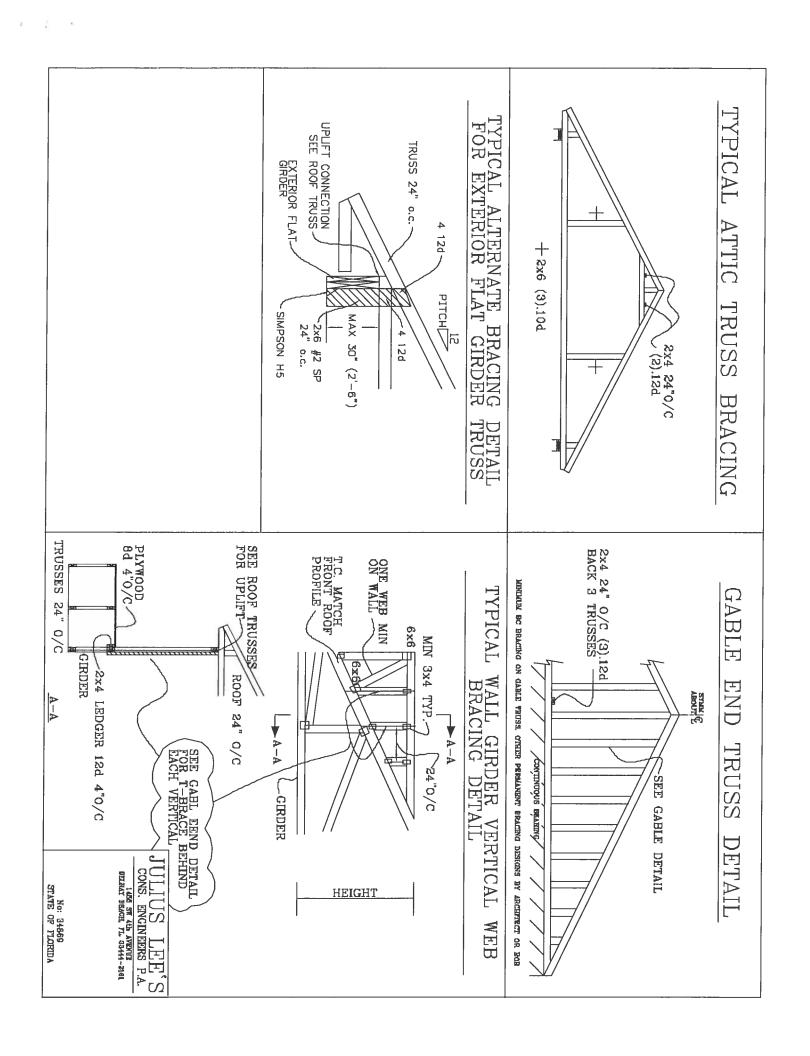
© 1993 MiTek® Holdings, Inc.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

	DIAGONAL HEA VERTICAL LENI VERTICAL LENI DIAGONAL HEA AT BACH END. TOYAL LENGTH VERTIC IN TAB	MAX GABLE VERTICAL LENGTH
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ASCET-02-0. 11/26/03	CAMBLE DAW DUPTNETS WAND THANK \$ 0" OPERBANG, OR 12" PLYWOOD OVERHANG. ANTIACH RAGH "L" BRACE WITE 10d NAILS AF 9 FOR (1) "L" BRACES WITE 10d NAILS AF 9 FOR (1) "L" BRACES SPACE NAILS AF 8" 0 IN 18" END ZONES AND 6" O.C. BETWERN 1 IN 18" END ZONES AND 6" O.C. BETWERN 1 IN 18" END ZONES AND 6" O.C. BETWERN 1 IN 18" END ZONES AND 6" O.C. BETWERN 1 IN 18" END ZONES AND 6" O.C. BETWERN 1 IN 18" END ZONES AND 6" O.C. BETWERN 1 IN 18" END ZONES AND 2 IN 18" END ZONES AND 1 IN 18" END ZONES AND 2 IN 18" EN	BRACING GROUP SPECIES AND GRADE GROUP A: GROUP A: GROUP A: GROUP A: HEM-FIR AZ STANDARD DOUGLAS FIR-LARCH STANDARD DOUGLAS FIR-LARCH GROUP B: HEM-FIR HI & BIR STANDARD GROUP B: HEM-FIR HI & BIR HI & BIR STANDARD GROUP B: HEM-FIR HI & BIR HI & BIR STANDARD GROUP B: HEM-FIR HI & BIR HI & BIR STANDARD GROUP B: HEM-FIR HI & BIR HI & BIR GROUP B: HEM-FIR HI & BIR GROUP B: HEM-FIR HI & BIR
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HERL PLATES.	PEAK, SPUICE, AND HEEL I
TRUBS DEBIGN YOR	REFER TO COMMON TRUES
2.5X4	REATER THAN 11' 0"
2%4	REATER THAN 4 0, BUT
IX4 OR EXS	SES THAN 4' 0"
NO BPLICE	ARMINCAT TRACIH
E SIZES	GABLE VERTICAL PLATE SIZES

DIAGONAL BEACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WICH DIAGONAL ERACE IS USED. CONTOCT HIACONAL BEACE TOR SEGS AT EACH EVID. MAX WEB TOTAL LEXICIH IS 14". MAX **GABLE** VERTICAL LENGTH SPACING SPECIES GRADE VERTICAL LENGTH IN TABLE ABOVE. 12" 24" O.C. O.C. 16 O.C. GABLE VERTICAL MODERATE OF VERTICAL SPF SPF SPF DFL DFL SP SP SP H HH H E ASCE NAOHG STANDARD STANDARD #1 #2 #3 STUD #3 STUD STANDARD STANDARD STANDARD STANDARD STANDARD STUD 13 WEST. 7-02: 2 2 E BRACE WAYARONCE: TRUSSES REGURE EXTREME CARE IN FARRICATING, HANDLING, SADPONG, DATALING AND BRACKS. REPER TO BOST 1-43 GUILING COMPIDENT SAFETY REPORTION, PUBLISHED BY TPE CRRUSS PLATE INSTITUTE, 383 DENOTROL DE, SUITÉ 280, MANISON, AC 327(9) AND VICA (AUDID TRUSS COUNCED TO MEDICA, 680 ENTERPRISE LM, MOISON, AT 537(9) FOR SAFETY PARCILLES PRIDE TO PERFORMING THESE TWICTIONS, UNLESS OTHERWISE DATACHED STRUCTURAL PARCES AND BOTTOM COURS SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PARCES AND BOTTOM COURS SHALL HAVE A PROPERTY ATTACHED RELIEF. GABLE TRUBS 7 G C O W 130 GROUP A (1) 1X4 "L" BRACE • (1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• (1) 2X6 ZX4 SP OR DT-L #Z OB BETTER DIAGONAL BRACE; BINGLE OR DOUBLE CUT (AS SHOWN) AT UPPER BND 4 4 5 MPH GROUP H 77 5 6 6 7 4 5 6 7 10 6 5 5 6 6 6 6 6 7 1 6 6 6 7 10 6 6 6 7 10 6 6 6 7 10 6 6 7 10 6 6 7 10 6 6 7 10 WIND GROUP A 요 나 II 이 의 명 SPEED, GROUP B B' 11" ~ u u u u REFER TO 30, LUORY 10" GROUP A 9' 10" 9' 10" 9' 10" 9' 10" 7º 10° CHART ABOVE FOR MAX GABLE VERTICAL LENGTH MEAN 10 6 0, EX4 #EN OR BETTER сридующе вкужимся GROUP B 10' 7' 9' 5" 9' 10" 9' 10" HEIGHT, **+** C CONS. ENGINEERS P.A. GROUP A 12' 11" 1456 SW 413 AVENUE DELRAY BEACH, PL 33444-EIGI 11, 4, 9, 9, 12, 11, No: 34869 STATE OF FLORIDA Ð "L" BRACE • (2) ZXB ENCLOSED, GROUP B 13 11 1 15 4" 12 11" 12' 10" GROUP A 14 0° Ŋ 909 F, **—** NAX. MAX GROUP 14' 0" 14' 0" 14' 0" 14' 0" 14' 0" BRACE 14 0 $\|$ 0,0 14. 0 TOT. 1.00, SPACING Ē ATTACH EACH 'L' BRACE WITE 104 MAIS. * FOR (1) 'L' BRACE; SPACE WALES AF 2' O.C. * FOR (2) 'L' BRACES; SPACE WALES AF 2' O.C. ** FOR (2) 'L' BRACES; SPACE WALES AT 3' O.C. IN 18' END ZONES AND 6' O.C. BETWEEN ZONES. CABLE END SUPPORTS TOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 180 FLF OVER CONTINUOUS BEARING (6 PSF *C DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS 1/240. MINBER LENGTH. T. BRACING MUST BE A MINIMUM OF 80% OF WEB DOUGLAS FIR-LARCH 43 9TUD STANDARD SPRUCE-PINE-TIR 11 / 42 STANDARD 43 STUD PLYTOOD OVERHANG. BRACING EXPOSURE CABLE TRUSS DETAIL SOUTHING PING 60 GREATER THAN 11' 6" GREATER THAN 11' 6" 24.0 PEAK, SPIJCE, AND HEEL PLATES. PSS THAN 4 0" GABLE VERTICAL PLATE SIZES PSF GROUP SPECIES REF DATE DWC mark dad over 20, e HI & HIR GROUP B: GROUP α DOUGLAS FIR-LARCH 11/26/03 SOUTHERN PONE #3 STUD STANDARD ASCET-02-CAB13030 Α 72 72 NO SPLICE Ð HELK-PIR 2.5X4 NOTES: 22 STANDARD GRADES: 3



TOP CHORD CHORD WEBS 2X4 2X4 2X4 はがは 222 BETTER BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

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SPACE PIGGYBACK VERTICALS AT 4' OC MAX. REFER TO SEALED DESIGN FOR DASHED PLATES.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURILINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY HE APPLIED HENDATH THE TOP CHORD OF SUPPORTING THUSS

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-93, CLOSED BIDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I EXP C, WIND TO DI=5 PSF, WIND BC DI=5 PSF

110 MPH WIND, 30' MBAN HGT, SHG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PSF, WIND HC DL-5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-88, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF WIND HC DL=6 PSF

FRONT FACE (5,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX EITHER PLATE LOCATION IS ACCEPTABLE XAX ∇ 20' FLAT TOP CHORD MAX SPAN 13/20 Ø MAX SIZE OF ZXIZ#2 OR BETTER Ш D-SPLICE

ATTACH TRUEQUAL, PER CONNECT	H
LOX PLATES WITH (6) 0.120° X 1.375" NAILS, OR FACE PER PLY. (4) NAILS IN EACH MEMBER TV ED. REFER TO DRAWING 160 TL FOR THULOX	4X8 OR 3X6 TRULOX AT 4' DC, HOTATED VEHTICALLY

Ħ a

5X4

9X9

9X6

EXG.1

1.5X4

1.5X4 933

1.5X4

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4X8

9X9

9X9

9X9

2X4

2.5X4

2.6X4

3X5

10' TO 14' MEMBER.	7'9" TO 10' MEMBER.	NO 1x4	WEB LENGIH	1
OR BETTER, AND 80% LENGTH OF ATTACH WITH 18d NAILS AT 4° OF	ATTACH WITH 8d NAJ	ING PRACE. SAME GRADE, SPECIES AS	REQUIRED BRACING	WEB BRACING CHART

00	ATTACH TEETH TO PAHRICATION. ATTA (4) 0.120" X 1.378" PIGGYBACK SPECIAL SPACE 4" OC OR LI
4 0 4 0	THE PIGGYBACK TACH TO SUPPOR S" NAILS PER FA IAL PLATE TO EAC LESS.
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a 0	TIME OF USS WITH PLY. APPLY FACE AND

* PIGGYBACK SPECIAL PLATE

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							*ATTACH PIGGYBACK WITH 3X6 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.
WYVARKINGOM TRAKSES REQUIRE EXTREME LARE IN FABRICATING, HAMILING, SUIPPING, INSTALLING AND BRACING, REFER TO INSECT INCOME COMPONENT SAFETY THEOREMY, PUBLICADE BY THE CRUENS FLARE DAYLING, SECRETING, COUNCIL TO AND AND ATTAC COURT TRAKES COUNCIL OF AMERICA, CASO INTERPRISE IN, HAMISON, M. 237199 FMP SAFETY PRACTICES PRIDE TO PERFORME THEORY INTERPRISE DAYLING, THEORY SAFETY PRACTICES PRIDE TO PERFORME STRUCTURAL PANCES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING.							
STATE OF FLORIDA				1812-775 T. TEPER AND IN	CONS. ENGINEERS P.A.	C, HHI SIII IIII	THIS DRAWII
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	T. CO DOM. PAC.	PSF	1.33 DUR. FAC.	55 PSF AT	MAX LOADING	NG REPLACES DRAWINGS 6
			-ENG JL	DRWGMITEK STD PIGGY	DATE 11/26/03	REF PIGGYBACK	THIS DRAWING REPLACES DRAWINGS 634,016 834,017 & 847,045

VALLEYTRUSS DETAIL

TOP CHORD BOT CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
2X4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FRON A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SHC 110 MPH, ASCE 7-93 110 MPH WIND OR (3) 16d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9". UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"—BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC. OR CONTINUOUS LATERAL BRACING.

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS HENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

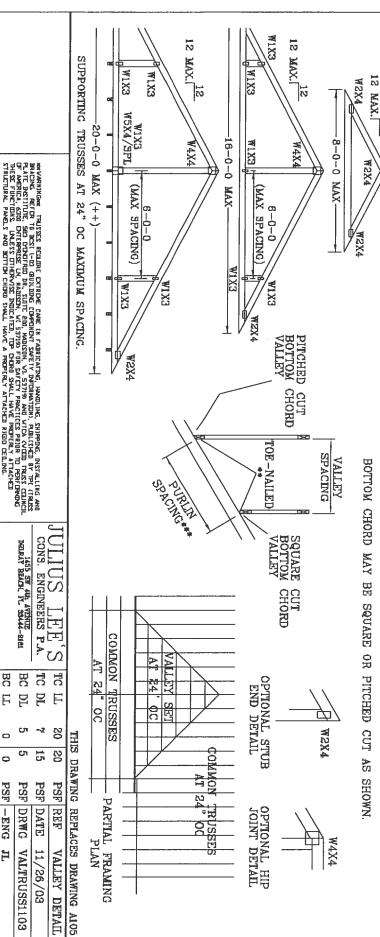
LARGER AS REQ'D

12

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XAM

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".



DETAYA BEYCH, U.C. 2011/0-8101

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VALTRUSS1103

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> PSF PSF PSF DRWG

No: 34869 STATE OF FLORIDA

SPACING

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DUR.FAC. 1.25 TOT. LD

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 — EDGE DISTANCE. END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

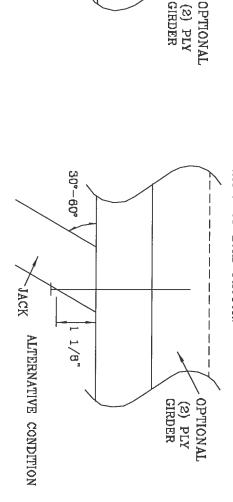
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM LATERAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

TUMBER OF		SOUTHERN PINE	DOUGLAS FIR-LARCH	FIR-LARCE		HEM-FIR	SPRUCE	SPRUCE PINE FIR
TOE-NAILS	1 PLY	2 PLIES 1 PLY		2 PLIES	1 PLY	2 PLIES	1 PLY	SEITA S
ผ	187#	256#	181#	234#	156#	203#	154#	189#
ω	296#	383#	271#	351#	234#	304#	230#	#882
4.	394#	511#	361#	468#	312#	406#	307#	397#
ຜ	493#	639#	452#	585#	390#	507#	384#	498#
ALL VALUE	ES MAY BI	MULTIPLI	PA AB OF	ROPRIATE	DURATION	ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR	CALUE	

GIRDER

(2) PLY



1/B"

JACK

THIS DRAWING REPLACES DRAWING 784040

			STRUCTURAL PANELS AND BOTTON GOIRD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING	PLATE INSTITUTE, 383 D'INCERCI DR. SUITE 260, NADISON, UT. 33719) AND VICA (MOD) TRUSS COLNCIL DE ARCRICA, 6500 ENICOPENEZ LM, NADISON, UT. 33719) FIDE SMEETY PRACTICES PRIDE TO ERECHANG THESE FUNCTIONS. UNLESS OTHERWISE INDICATED. THIS CHERD WALL HAVE PREPER IN ATTACKED.	****VARATIKO*** TRUSSES REGUIRE EXTREKE CARE IN FAIRGEATING, HANDLING, SUPPING, INSTALLING AND BRACING. REFER TO BESI 1-03 CHILLING COMPINENT SAFETY (NFIDWAITING), PUBLISHED BY TRY CRAUSS		
STATE OF FLORIDA	No: 34889			DELPAY BEACH, FL SHIII-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S	
SPACING	DUR. FAC. 1.00	TOT. LD.	BC LL	BC DL	TC DL	TC LL	
	1.00	PSF	PSF	PSF	PSF	PSF	
			-ENG JL			REF	
			IL	DRWG CNTONAIL1103	DATE 11/26/03	TOE-NAIL	

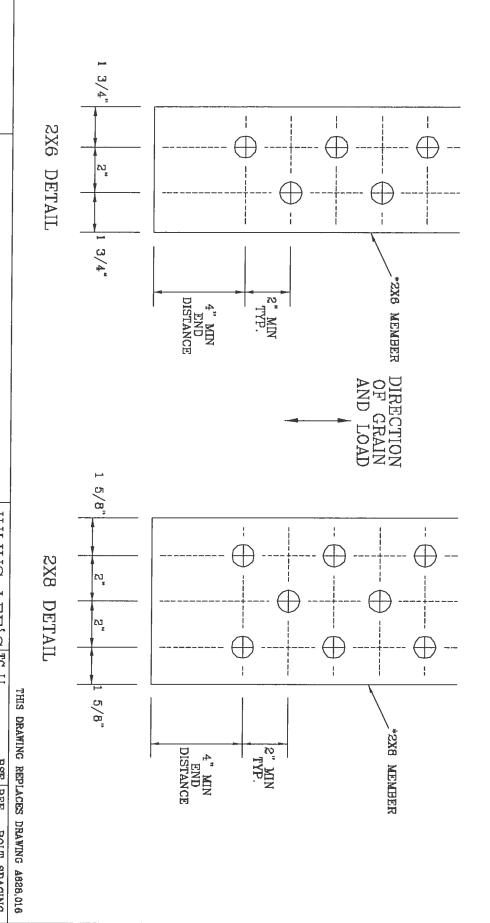
DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



WARNING* TRUSSES REQUIRE EXTEDE CARE (IN FABRICATING, HANDLING, SUDPRING, INSTALLING AND PARCING, REFER TO BOSI, I-PO GRUILLING COPERINDT SAFETY DEFENDING IN PUBLICATION PY FIX (TRUSS CALINCI).

PLAIT INSTITUTE, 595 CONTRIPRING DEE, DUTIE 200, MULTIEN, PARCINCES PRIDER TO PRESE CALINCI).

PLAIT INSTITUTE, 595 CONTRIPRING SAFETY PROCESSES PRIDER TO PRESENCE OF APPENDING TO PRESENCE OF APPENDING TO PROCESSES PRIDER TO PROCESS

			_	NO.	TRUSS ANO	
STATE OF FLORIDA	V-, 24020			DELEAT SEACH, FL 33444-2161	CONS. ENGINEERS P.A.	
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL		TC LL
		PSF	PSF	PSF	PSF	PSE REE
			-ENG JL	DRWG	DATE	REF
			IL.	CNB0LTSP1103	11/26/09	BOLT SPACING

TRULOX CONNECTION)FTAI

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (ϕ) .

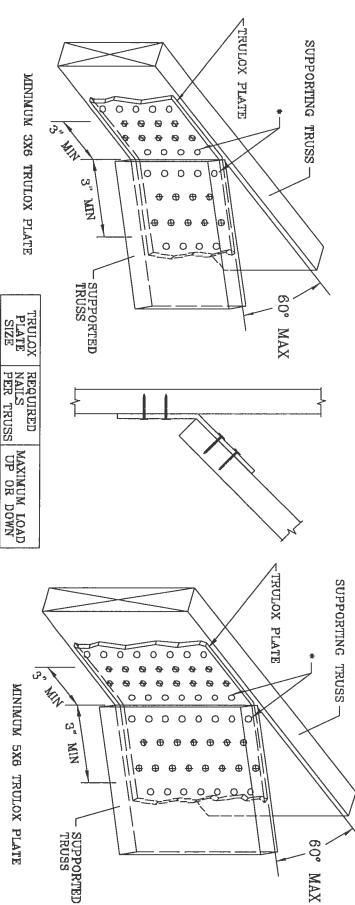
NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

THIS DETAIL FOR LUMBER, PLATES, AND OTHER REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN

MAX



3X6 **6X**8

15 9

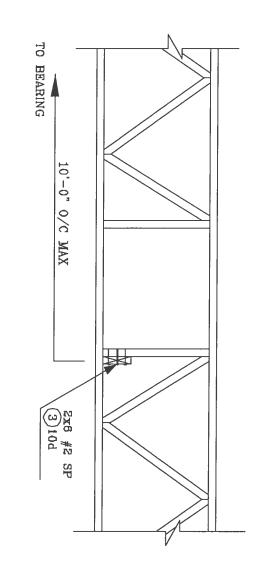
#066 350#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

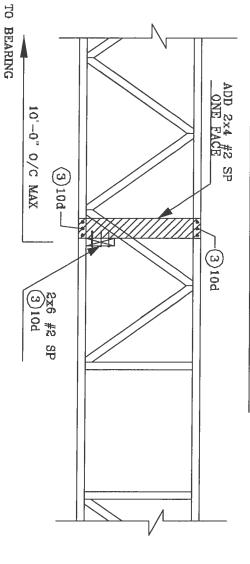
MINIMUM 5X6 TRULOX PLATE

JULIUS CONS. ENGIN DETRYL BEYOK 11" 38444-8161 No: 34869 STATE OF FLORIDA ENGINEERS PA. REF -ENG DRWG DATE CNTRULOX1103 11/26/03 TRULOX

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S CONS. ENGINEERS P.A.

1456 SW 45h AVENUE
DEEMAY BEAGH, 7L 30441-2161

No: 34869 STATE OF FLORIDA