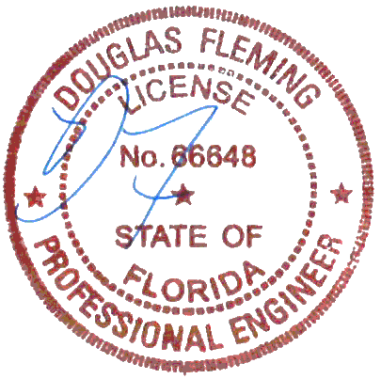


Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

COA #0 278
Florida Certificate of Product Approval #FL1999

03/31/2025

This item has been digitally signed by Douglas Fleming on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Site Information:	**Page 1:**
Customer: Seminole Trusses, Inc.	*Job Number:* B60881a
Job Description: PIZZAGALLI RESIDENCE	
Address: 245 SW COLONY GLEN, Lake City, FL 32024	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	*IntelliVIEW Version:* 24.02.00C *JRef #:* 1Y8R8570002
Wind Standard: ASCE 7-22 *Wind Speed (mph):* 120 *Building Type:* Closed	*Design Loading (psf):* 37.00

This package contains general notes pages, 22 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	090.25.1314.04223	A1 40'3" Common
3	090.25.1314.12967	A1b 40'3" Common
5	090.25.1314.20120	A2-G 40'3" Gable
7	090.25.1314.34037	B1 28'8" Common Girder
9	090.25.1314.53657	B3-SG 28'8" Gable
11	090.25.1313.32200	C2-G 16'11" Gable
13	090.25.1313.33987	D2-G 8' Gable
15	090.25.1313.35880	E2-G 5'10" Gable
17	090.25.1313.53843	PB2-G 9' Gable
19	090.25.1313.56527	V2 19'11"8 Valley
21	090.25.1313.58363	V4 11'11"8 Valley
23	REPCHRD1014	
25	VALTN220723	
27	CNNAILSP1014	

Item	Drawing Number	Truss
2	090.25.1314.09080	A1a 40'3" Common
4	090.25.1314.17950	A1c 40'3" Common
6	090.25.1314.23957	A2a-G 40'3" Gable
8	090.25.1314.35273	B2 28'8" Common
10	090.25.1313.30780	C1 16'11" Common
12	090.25.1313.33110	D1 8'4" Mono
14	090.25.1313.34923	E1 5'10" Mono
16	090.25.1313.36817	PB1 9' Common
18	090.25.1313.55457	V1 23'11"8 Valley
20	090.25.1313.57407	V3 15'11"8 Valley
22	090.25.1313.59293	V5 7'11"8 Valley
24	VAL180220723	
26	GBLDIAG220923	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:
The bearing area factor, Cb, is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:
Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
C = Coated lumber.
C-AT = AtTEK coated lumber.
C-FX = FX Lumber Guard coated lumber.
C -TE = TechWood 4400 coated lumber.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-BF = Boraflame Fire Retardant Treated lumber
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-ON = OnWood Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

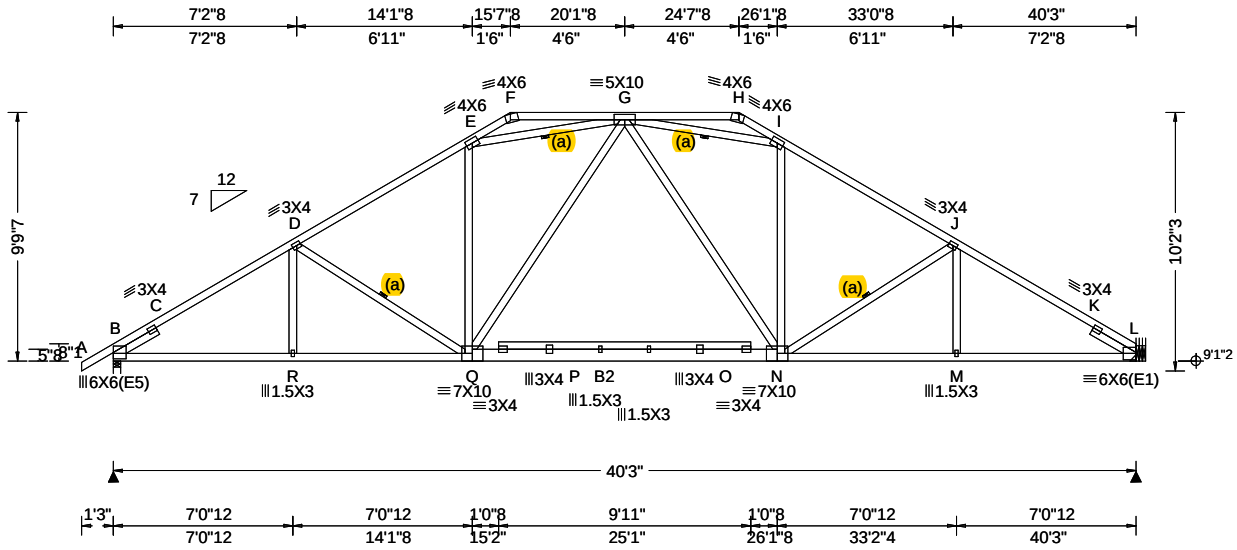
Key to Terms (continued):
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN: 29878 COMN FROM: RJL	Ply: 1 Qty: 14	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** A1 40'3" Common	Cust: R 857 JRef:1Y8R8570002 T10 DrwNo: 090.25.1314.04223 SSB / DF 03/31/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.03 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.297	P	999	360
VERT(CL): 0.502	P	961	240
HORZ(LL): 0.076	L	-	-
HORZ(TL): 0.129	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.659
Max BC CSI: 0.685
Max Web CSI: 0.893

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1851	/-	/-	/798	/140	/150
L	1773	/-	/-	/790	/129	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 2.2 (Truss)
L Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	286	- 2945	I - J	266	- 2510
C - D	242	- 2895	J - K	243	- 2903
D - E	265	- 2508	K - L	338	- 2953

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - R	2434	- 198	O - N	1963	- 209
R - Q	2432	- 199	N - M	2442	- 201
Q - P	1963	- 209	M - L	2444	- 200
P - Q	2909	- 1144			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - Q	61	- 425	G - I	177	- 1837
E - Q	529	- 17	N - I	532	- 18
E - G	177	- 1840	N - J	70	- 435

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	-1.25 to	56 plf at	40.25
BC: From	5 plf at	-1.25 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	14.12
BC: From	60 plf at	14.12 to	60 plf at	26.12
BC: From	20 plf at	26.12 to	20 plf at	40.25

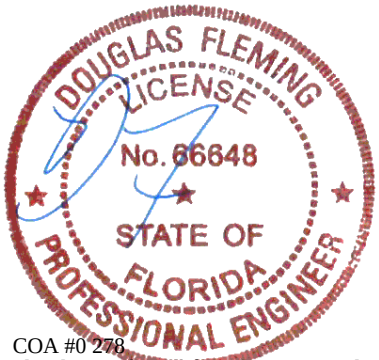
Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
03/31/2025

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

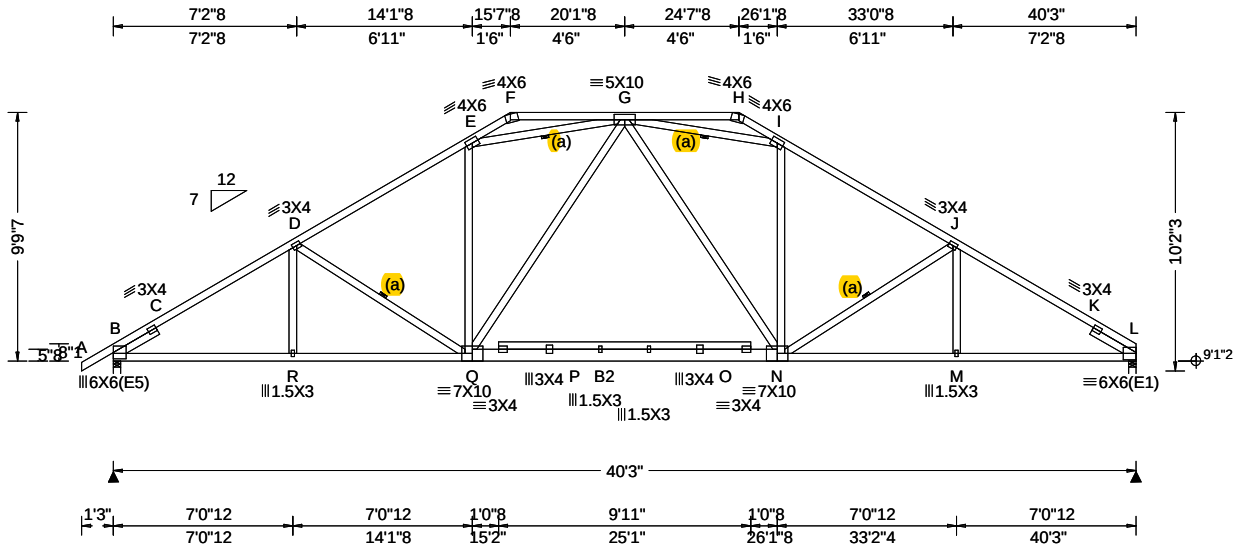
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29880 COMN FROM: RJL	Ply: 1 Qty: 3	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** A1a 40'3" Common	Cust: R 857 JRef:1Y8R8570002 T9 DrwNo: 090.25.1314.09080 SSB / DF 03/31/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.03 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.297	P	999	360
VERT(CL): 0.502	P	961	240
HORZ(LL): 0.076	L	-	-
HORZ(TL): 0.129	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.659
Max BC CSI: 0.685
Max Web CSI: 0.893

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1851	/-	/-	/798	/140	/150
L	1773	/-	/-	/790	/129	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 2.2 (Truss)
L Brg Wid = 3.5 Min Req = 2.1 (Truss)
Bearings B & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	286	- 2945	I - J	266	- 2510
C - D	242	- 2895	J - K	243	- 2903
D - E	265	- 2508	K - L	338	- 2953

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - R	2434	- 198	O - N	1963	- 209
R - Q	2432	- 199	N - M	2442	- 201
Q - P	1963	- 209	M - L	2444	- 200
P - Q	2909	- 1144			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - Q	61	- 425	G - I	177	- 1837
E - Q	529	- 17	N - I	532	- 18
E - G	177	- 1840	N - J	70	- 435

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
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Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Special Loads

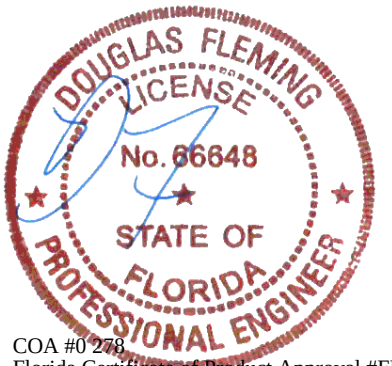
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	-1.25 to	56 plf at	40.25
BC: From	5 plf at	-1.25 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	14.12
BC: From	60 plf at	14.12 to	60 plf at	26.12
BC: From	20 plf at	26.12 to	20 plf at	40.25

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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Florida Certificate of Product Approval #FL1999
03/31/2025

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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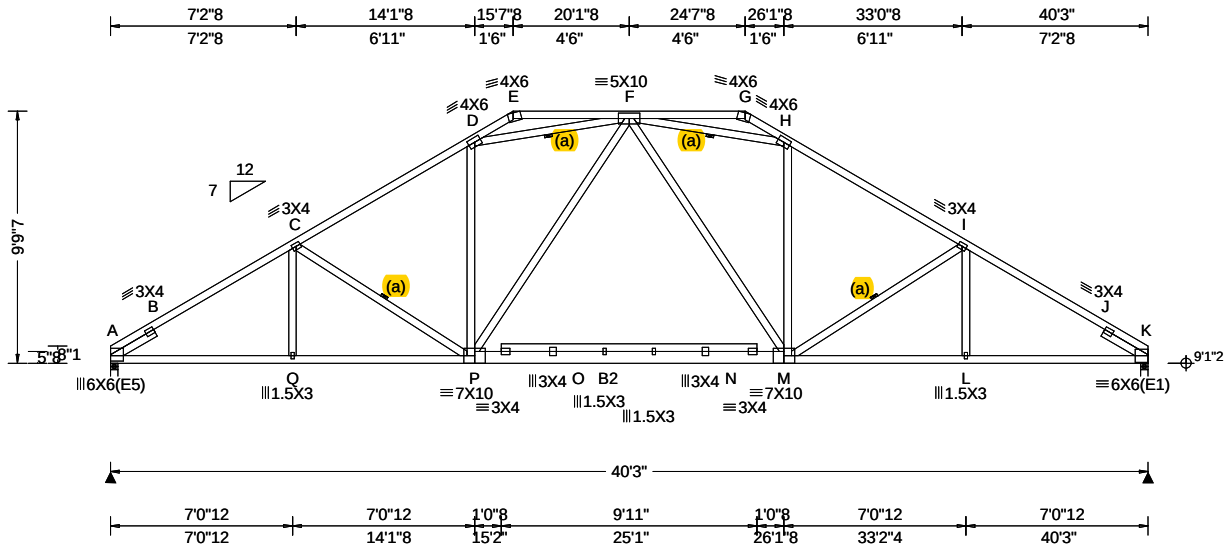
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29882 COMN FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** A1b 40'3" Common	Cust: R 857 JRef:1Y8R8570002 T8 DrwNo: 090.25.1314.12967 SSB / DF 03/31/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.03 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.296	O	999	360
VERT(CL): 0.502	O	961	240
HORZ(LL): 0.076	K	-	-
HORZ(TL): 0.129	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.659
Max BC CSI: 0.685
Max Web CSI: 0.892

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1774	/-	/-	/790	/129	/142
K	1774	/-	/-	/790	/129	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 2.1 (Truss)
K Brg Wid = 3.5 Min Req = 2.1 (Truss)
Bearings A & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	338	- 2955	H - I	266	- 2512
B - C	243	- 2905	I - J	243	- 2905
C - D	266	- 2512	J - K	338	- 2955

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - Q	2446	- 200	N - M	1965	- 210
Q - P	2444	- 201	M - L	2444	- 201
P - O	1965	- 210	L - K	2446	- 200
O - P	2913	- 1147			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - P	70	- 435	F - H	177	- 1839
D - P	533	- 18	M - H	533	- 18
D - F	177	- 1839	M - I	70	- 435

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Special Loads

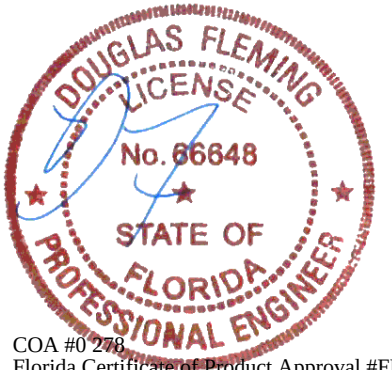
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	0.00 to	56 plf at	40.25
BC: From	20 plf at	0.00 to	20 plf at	14.12
BC: From	60 plf at	14.12 to	60 plf at	26.12
BC: From	20 plf at	26.12 to	20 plf at	40.25

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
03/31/2025

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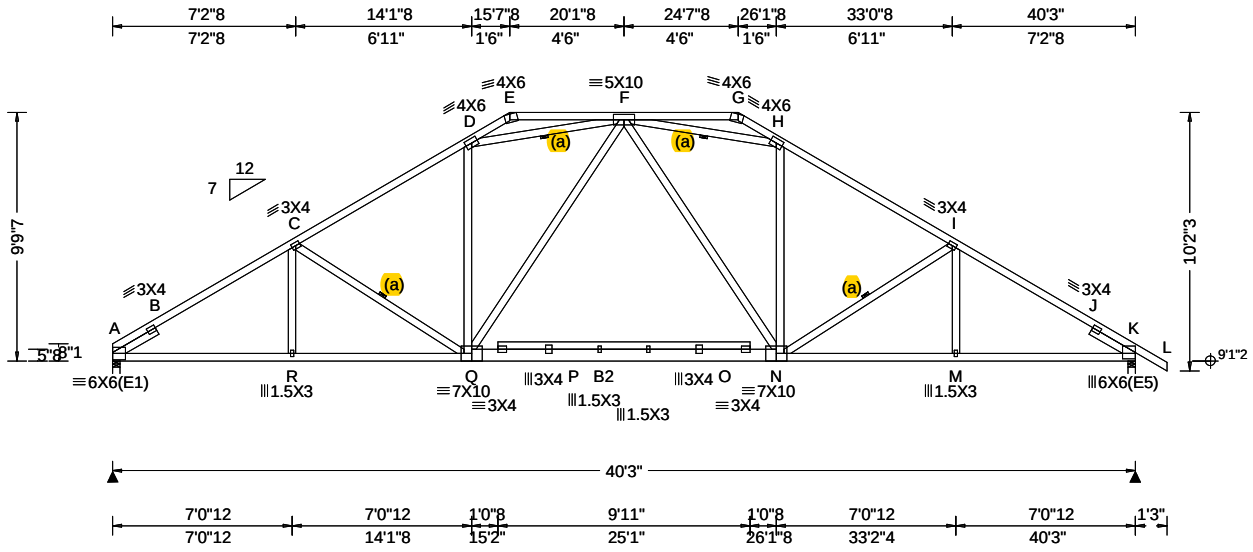
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29884 COMN FROM: RJL	Ply: 1 Qty: 6	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** A1c 40'3" Common	Cust: R 857 JRef:1Y8R8570002 T6 DrwNo: 090.25.1314.17950 SSB / DF 03/31/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.03 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.297 P	999	360
VERT(CL):	0.502 P	961	240
HORZ(LL):	0.076 K	-	-
HORZ(TL):	0.129 K	-	-

Creep Factor: 2.0
Max TC CSI: 0.659
Max BC CSI: 0.685
Max Web CSI: 0.893

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1773	/-	/-	/790	/129	/150
K	1851	/-	/-	/798	/129	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 2.1 (Truss)
K Brg Wid = 3.5 Min Req = 2.2 (Truss)
Bearings A & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	338	- 2953	H - I	266	- 2508
B - C	243	- 2903	I - J	243	- 2895
C - D	266	- 2510	J - K	286	- 2945

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - R	2444	- 200	O - N	1963	- 210
R - Q	2442	- 201	N - M	2432	- 201
Q - P	1963	- 210	M - K	2434	- 200
P - Q	2911	- 1146			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - Q	70	- 435	F - H	177	- 1840
D - Q	532	- 18	N - H	529	- 18
D - F	177	- 1837	N - I	62	- 425

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Special Loads

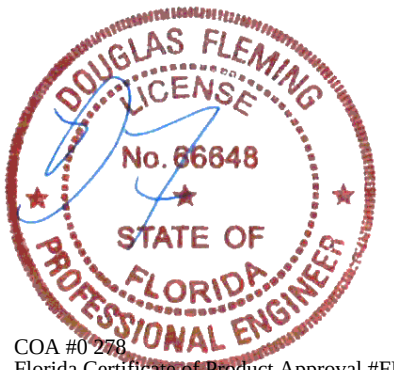
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	0.00 to	56 plf at	41.50
BC: From	20 plf at	0.00 to	20 plf at	14.12
BC: From	60 plf at	14.12 to	60 plf at	26.12
BC: From	20 plf at	26.12 to	20 plf at	40.25
BC: From	5 plf at	40.25 to	5 plf at	41.50

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
03/31/2025

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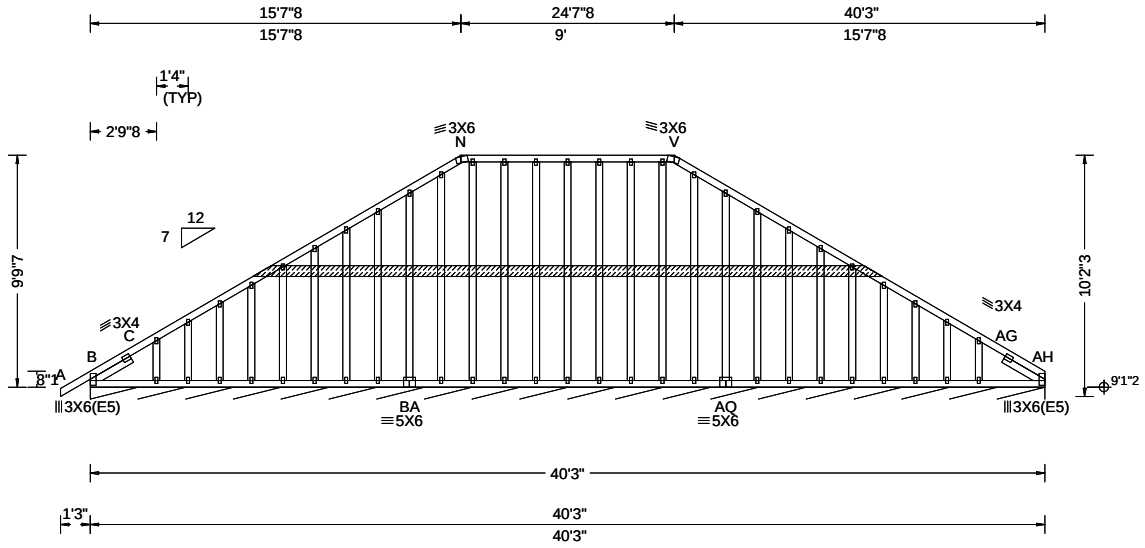
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29829 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** A2-G 40'3" Gable	Cust: R 857 JRef:1Y8R8570002 T7 DrwNo: 090.25.1314.20120 SSB / DF 03/31/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.003 AG 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.008 AG 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 AG - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.004 AG - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.091
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.043
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.417
	C&C Dist a: 4.03 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AH*107	/-	/-		/39	/2	/4

Wind reactions based on MWFRS
AH Brg Wid = 483 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/852.

Additional Notes

See DWG GBLDIAG220923 for gable stiffback and diagonal bracing details.

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

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Florida Certificate of Product Approval #FL1999
03/31/2025

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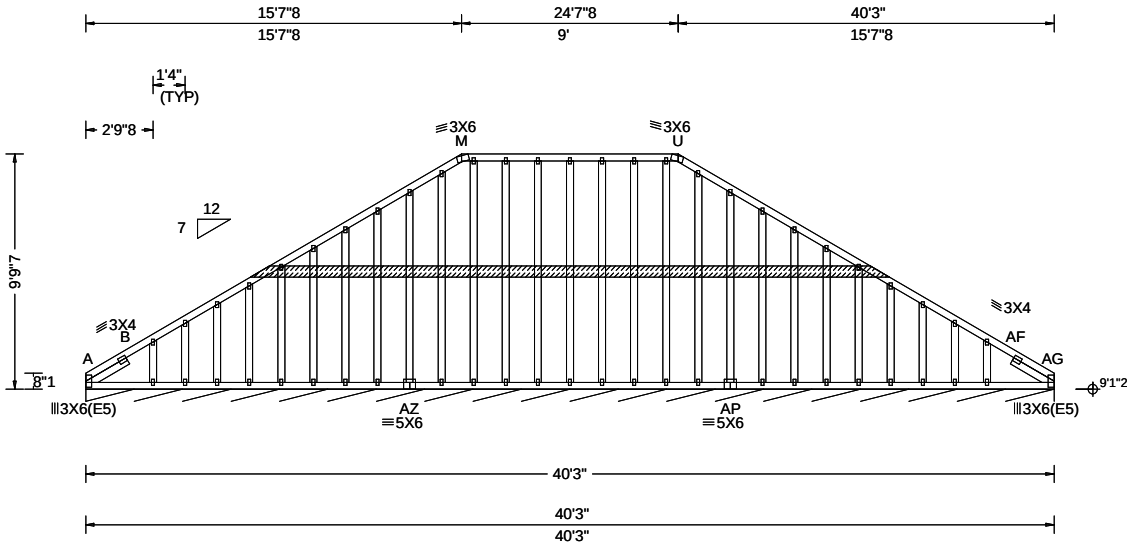
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29832 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** A2a-G 40'3" Gable	Cust: R 857 JRef:1Y8R8570002 T12 DrwNo: 090.25.1314.23957 SSB / DF 03/31/2025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.03 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 B 999 360 VERT(CL): 0.008 B 999 240 HORZ(LL): 0.002 B - - HORZ(TL): 0.005 B - - Creep Factor: 2.0 Max TC CSI: 0.086 Max BC CSI: 0.043 Max Web CSI: 0.416 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AG*104	/-	/-		/38	/2	/4

Wind reactions based on MWFRS
AG Brg Wid = 483 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

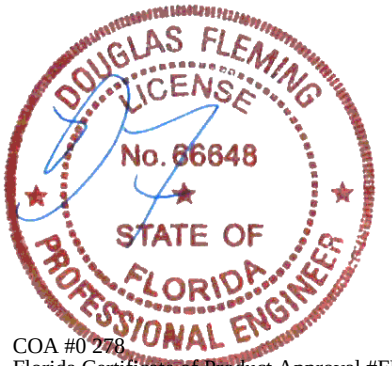
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/852.

Additional Notes

See DWG GBLDIAG220923 for gable stiffback and diagonal bracing details.

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0 278
Florida Certificate of Product Approval #FL1999
03/31/2025

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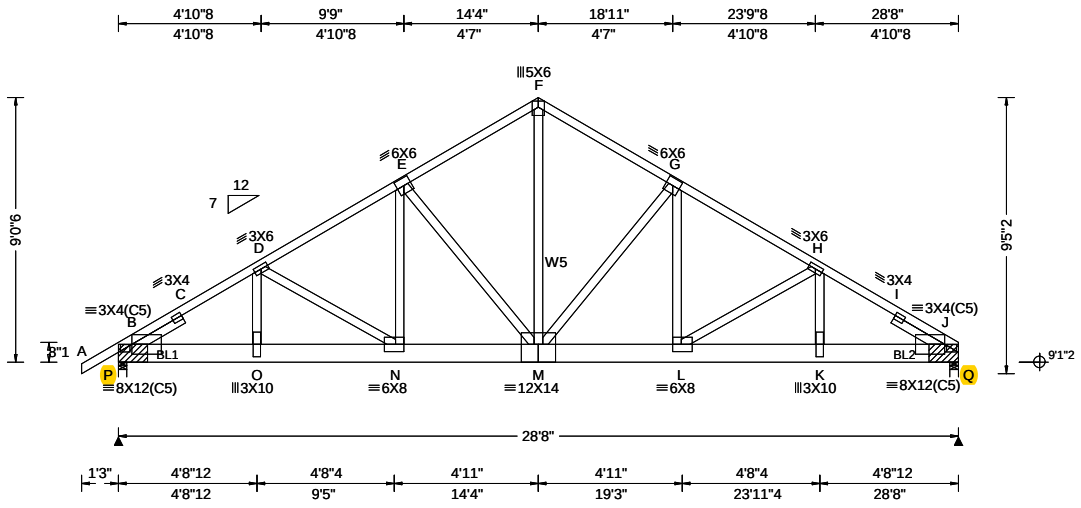
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29850 COMN FROM: RJL	Ply: 3 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** B1 28'8" Common Girder	Cust: R 857 JRef:1Y8R8570002 T11 DrwNo: 090.25.1314.34037 SSB / DF 03/31/2025	

3 Complete Trusses Required



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.188	M	999	360
VERT(CL): 0.345	M	977	240
HORZ(LL): 0.050	D	-	-
HORZ(TL): 0.092	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.610
Max BC CSI: 0.483
Max Web CSI: 0.721

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity	
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
P	12785 /-	/-	/-	/953	/-
Q	14004 /-	/-	/-	/1032	/-

Wind reactions based on MWFRS
P Brg Wid = 3.5 Min Req = -
Q Brg Wid = 3.5 Min Req = -
Bearings P & Q are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	492	-6641	F - G	323	-4339
C - D	488	-6617	G - H	419	-5642
D - E	419	-5643	H - I	489	-6618
E - F	323	-4339	I - J	493	-6642

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - O	5659	-416	M - L	4790	-353
O - N	5603	-413	L - K	5606	-414
N - M	4791	-353	K - J	5653	-417

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
O - D	1043	-66	M - G	126	-1696
D - N	65	-882	G - L	1889	-128
N - E	1891	-128	L - H	66	-888
E - M	126	-1699	H - K	1042	-66
F - M	4252	-303			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x8 SP SS Dense;
Webs: 2x4 SP #3; W5 2x4 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 3.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 56 plf at -1.25 to 56 plf at 28.67
BC: From 5 plf at -1.25 to 5 plf at 0.00
BC: From 10 plf at 0.00 to 10 plf at 28.67
BC: 1773 lb Conc. Load at 2.06, 4.06, 6.06, 8.06 10.06,12.06,14.06,16.06,18.06,20.06,22.06,24.06 26.10,28.10

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.

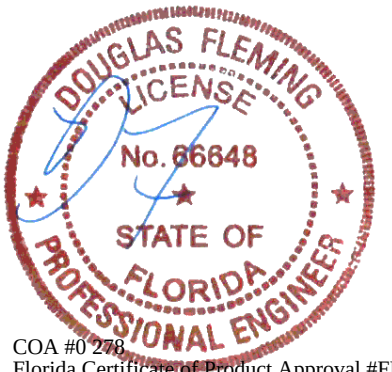
It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

Bearing Block(s)

Brg blocks:0.128"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
1	0.000'	1	12"	19	Rigid Surface
2	28.375'	2	12"	15	Rigid Surface

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.



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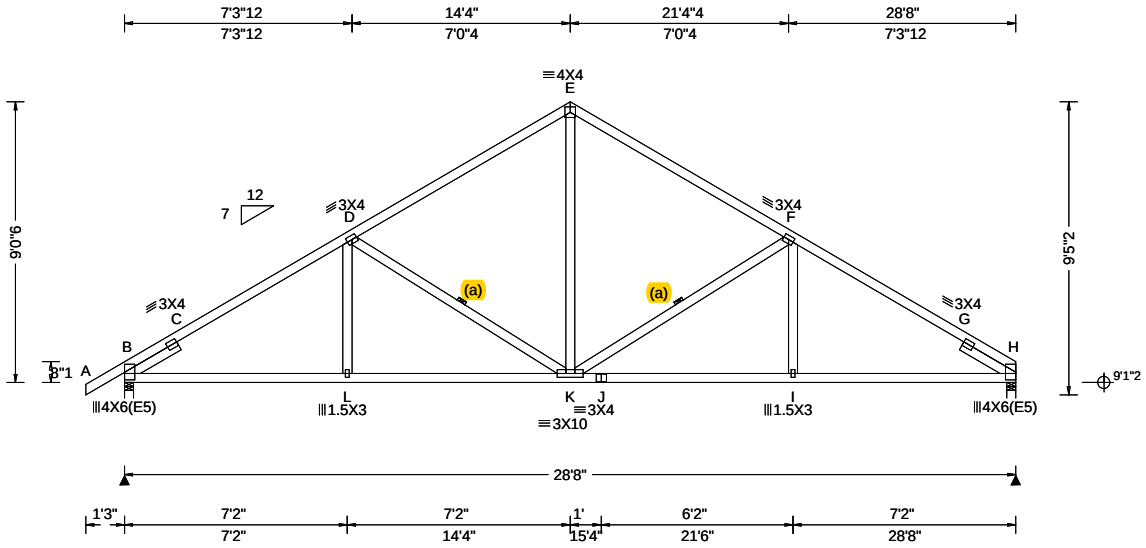
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29852 COMN FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** B2 28'8" Common	Cust: R 857 JRef:1Y8R8570002 T3 DrwNo: 090.25.1314.35273 SSB / DF 03/31/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.083	G	999	360
VERT(CL): 0.158	G	999	240
HORZ(LL): 0.044	C	-	-
HORZ(TL): 0.084	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.640
Max BC CSI: 0.483
Max Web CSI: 0.457

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1170	/-	/-	/563	/10	/132
H	1091	/-	/-	/556	/2	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	236	-1840	E - F	100	-1178
C - D	51	-1644	F - G	58	-1650
D - E	97	-1177	G - H	284	-1885

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - L	1366	0	J - I	1370	0
L - K	1362	0	I - H	1374	0
K - J	1370	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - K	68	-520	E - K	702	0
K - F	70	-529			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

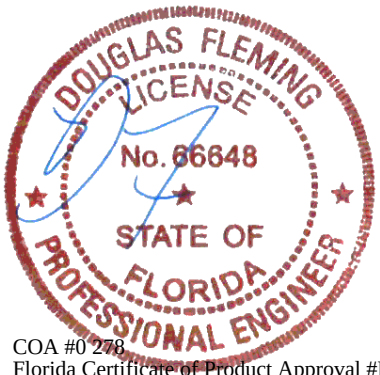
Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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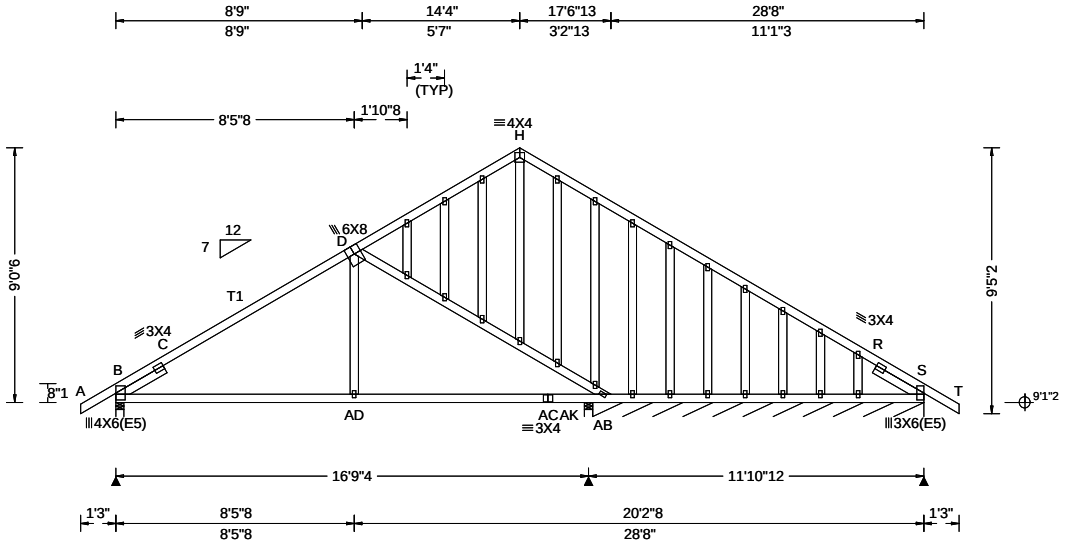
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29862 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** B3-SG 28'8" Gable	Cust: R 857 JRef:1Y8R8570002 T5 DrwNo: 090.25.1314.53657 SSB / DF 03/31/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.156 C	999	360
VERT(CL):	0.309 C	652	240
HORZ(LL):	0.096 C	-	-
HORZ(TL):	0.191 C	-	-

Creep Factor: 2.0
Max TC CSI: 0.876
Max BC CSI: 0.641
Max Web CSI: 0.579

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1187	/-	/-	/451	/28	/172
AK	449	/-	/-	/225	/-	/-
S*	183	/-	/-	/59	/21	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
AK Brg Wid = 3.5 Min Req = 1.5 (Truss)
S Brg Wid = 141 Min Req = -
Bearings B, AK, & AK are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	560	- 2109	H - R	116	- 494
C - D	111	- 1477	R - S	56	- 539
D - H	113	- 634			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AD	1175	- 26	AC-AB	2352	- 47
AD-AC	1176	- 24	AB- S	395	- 42

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
D -AB	224	- 1083

Lumber

Top chord: 2x4 SP #1; T1 2x4 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

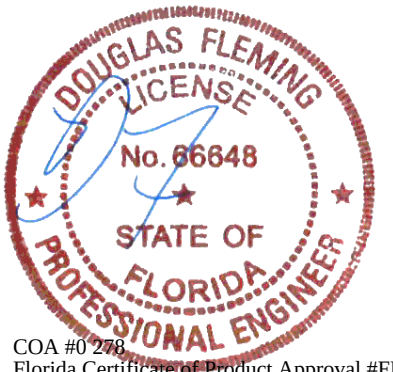
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/194.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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03/31/2025

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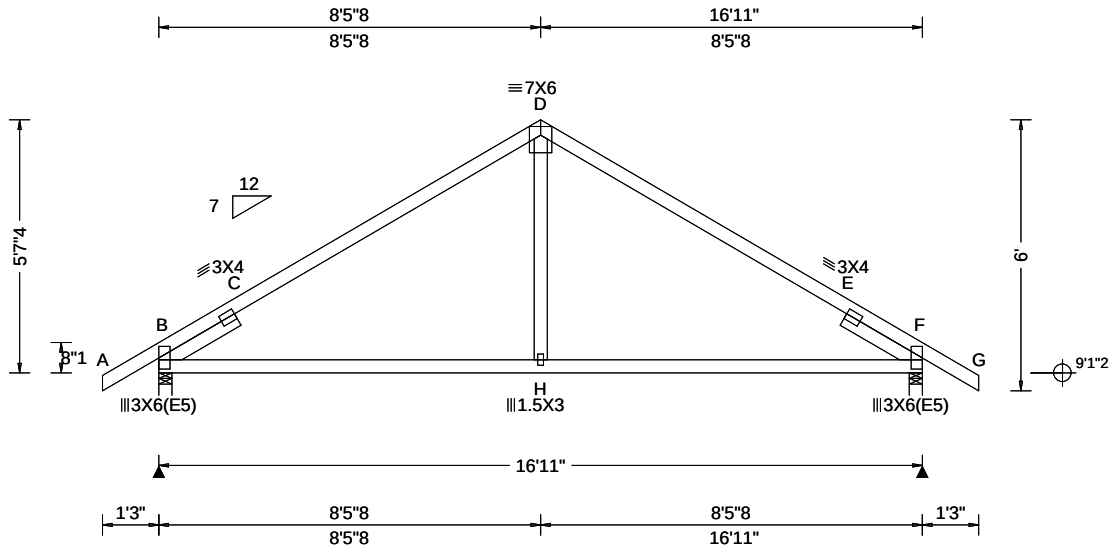
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29854 COMN FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** C1 16'11" Common	Cust: R 857 JRef:1Y8R8570002 T1 DrwNo: 090.25.1313.30780 SSB / DF 03/31/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.168	C	999	360
VERT(CL): 0.212	C	957	240
HORZ(LL): 0.104	C	-	-
HORZ(TL): 0.131	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.688
Max BC CSI: 0.735
Max Web CSI: 0.333

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	816	/-	/-	/334	/9	/73
F	816	/-	/-	/334	/9	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	632	-1375	D - E	136	-952
C - D	136	-952	E - F	630	-1375

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - H	764	-9	H - F	764	-9

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
D - H	389	0

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

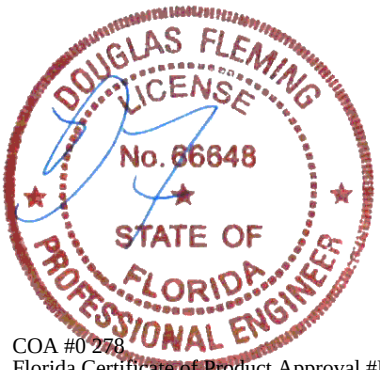
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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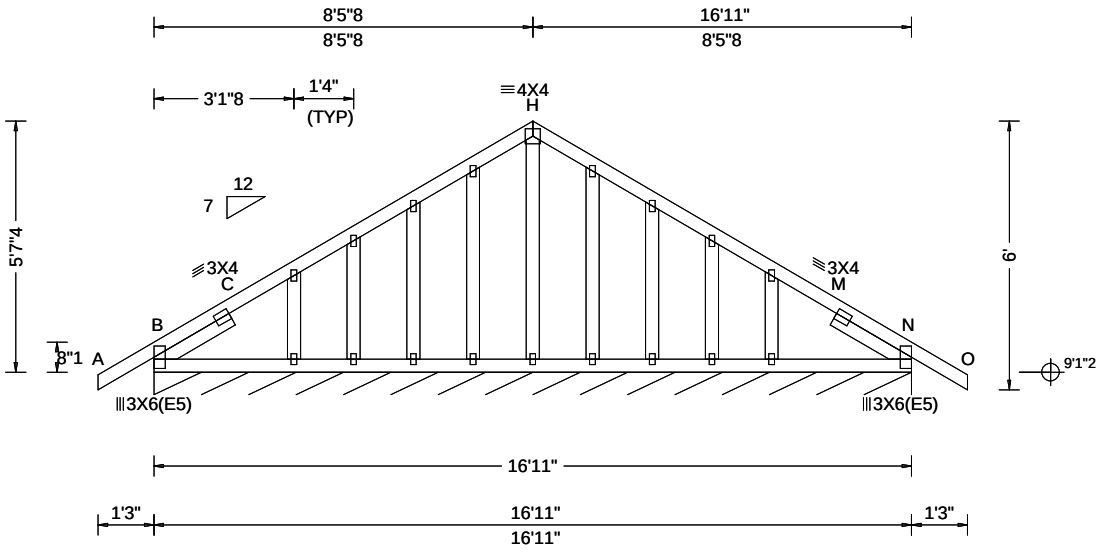
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29858 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** C2-G 16'11" Gable	Cust: R 857 JRef:1Y8R8570002 T2 DrwNo: 090.25.1313.32200 SSB / DF 03/31/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.005 M 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.010 M 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.003 M - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.006 M - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.111
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.054
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.398
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	116	/-	/-	/46	/6	/6

Wind reactions based on MWFRS
B Brg Wid = 203 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

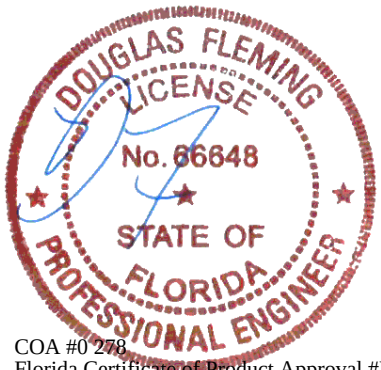
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/463.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



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03/31/2025

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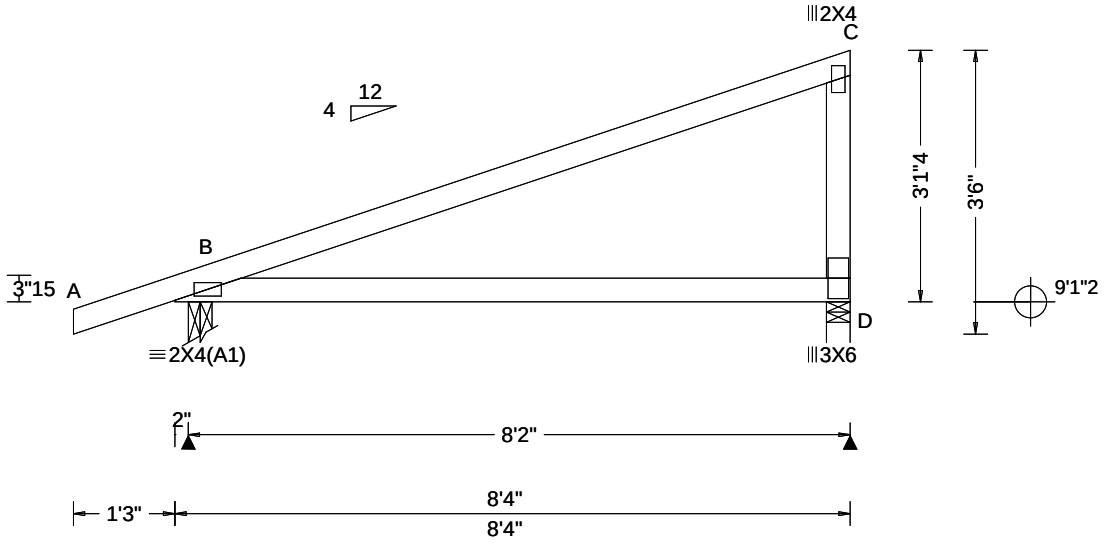
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29864 MONO FROM: RJL	Ply: 1 Qty: 7	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** D1 8'4" Mono	Cust: R 857 JRef:1Y8R8570002 T18 DrwNo: 090.25.1313.33110 SSB / DF 03/31/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.021 B - - HORZ(TL): 0.038 B - - Creep Factor: 2.0 Max TC CSI: 0.713 Max BC CSI: 0.526 Max Web CSI: 0.316 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	399	/-	/-	/166	/5	/63
D	298	/-	/-	/155	/13	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & D are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

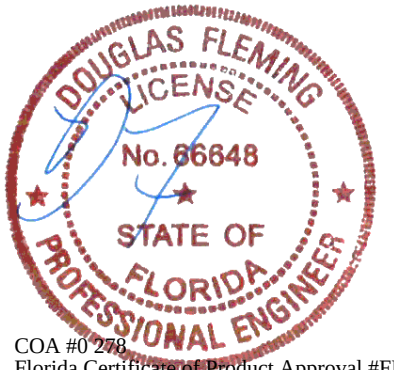
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
03/31/2025

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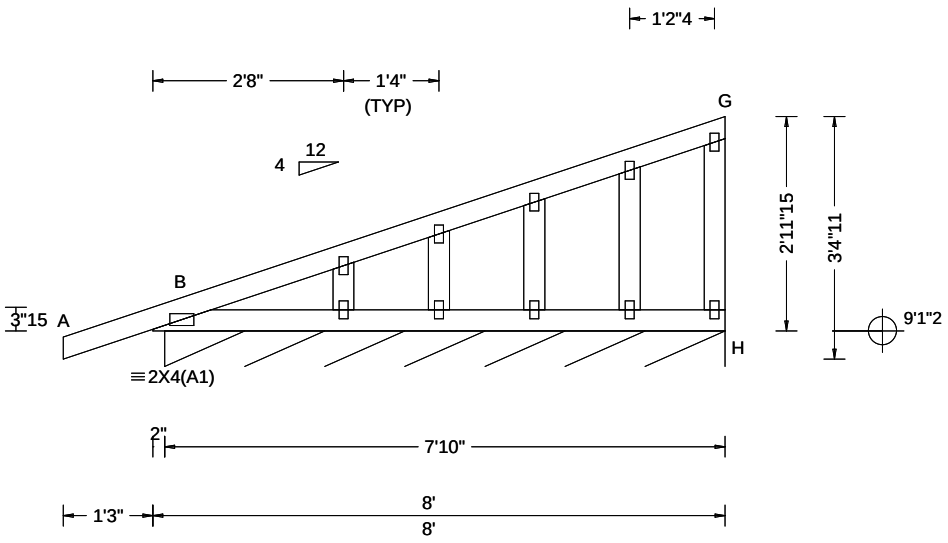
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29867 GABL FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** D2-G 8' Gable	Cust: R 857 JRef:1Y8R8570002 T20 DrwNo: 090.25.1313.33987 SSB / DF 03/31/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 B 999 240 HORZ(LL): 0.000 B - - HORZ(TL): 0.002 F - - Creep Factor: 2.0 Max TC CSI: 0.096 Max BC CSI: 0.027 Max Web CSI: 0.444 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	92	/-	/-	/40	/23	/15

Wind reactions based on MWFRS
B Brg Wid = 94.0 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

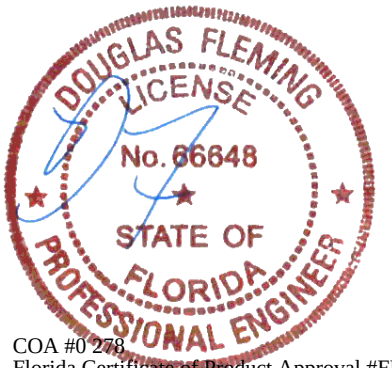
Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0 278
Florida Certificate of Product Approval #FL1999
03/31/2025

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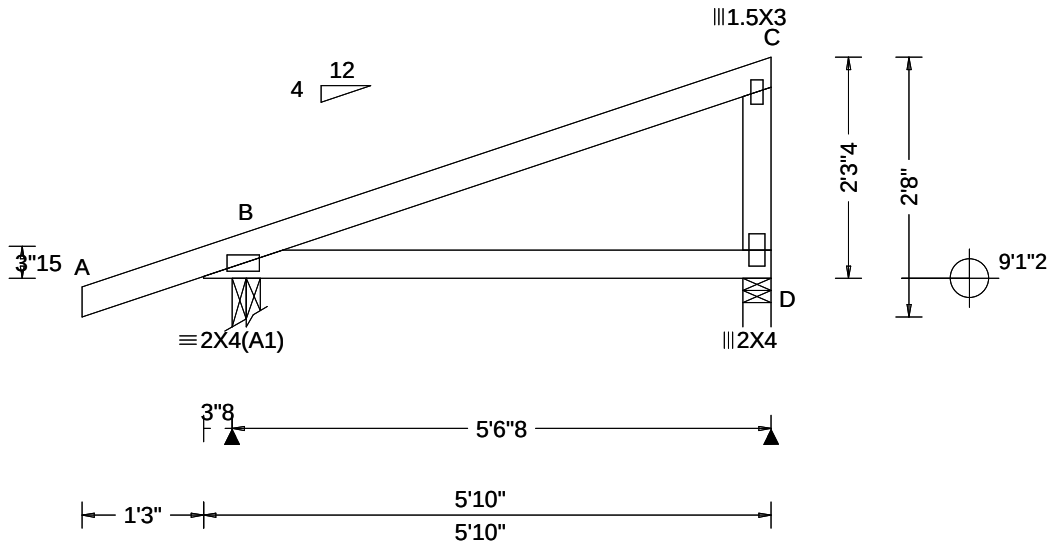
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29869 MONO FROM: RJL	Ply: 1 Qty: 4	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** E1 5'10" Mono	Cust: R 857 JRef:1Y8R8570002 T19 DrwNo: 090.25.1313.34923 SSB / DF 03/31/2025



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria
PP Deflection in loc L/defl L/#
VERT(LL): NA
VERT(CL): NA
HORZ(LL): 0.005 B - -
HORZ(TL): 0.010 B - -
Creep Factor: 2.0
Max TC CSI: 0.295
Max BC CSI: 0.234
Max Web CSI: 0.098

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	315	/-	/-	/125	/-	/35
D	194	/-	/-	/105	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & D are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

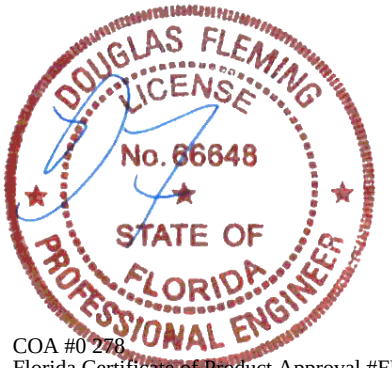
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
03/31/2025

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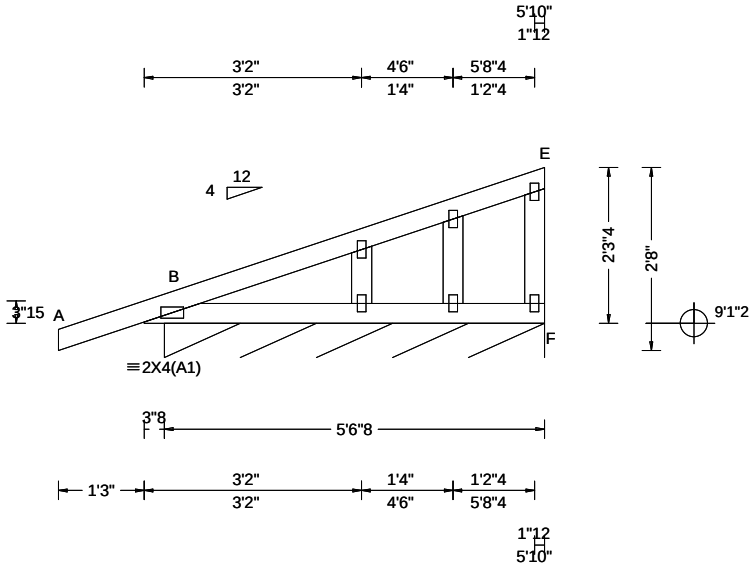
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29871 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** E2-G 5'10" Gable	Cust: R 857 JRef:1Y8R8570002 T4 DrwNo: 090.25.1313.35880 SSB / DF 03/31/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 10.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.001 B	999	360
VERT(CL):	0.002 B	999	240
HORZ(LL):	0.000 B	-	-
HORZ(TL):	0.001 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.114
Max BC CSI: 0.038
Max Web CSI: 0.032

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	97	/-	/-	/42	/14	/12

Wind reactions based on MWFRS
B Brg Wid = 66.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

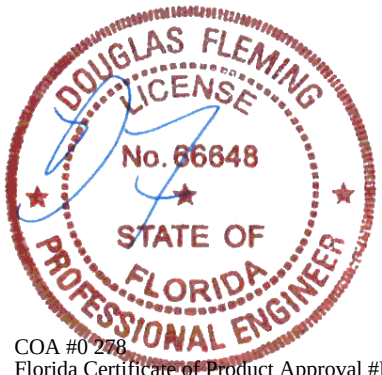
Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0 278
Florida Certificate of Product Approval #FL1999
03/31/2025

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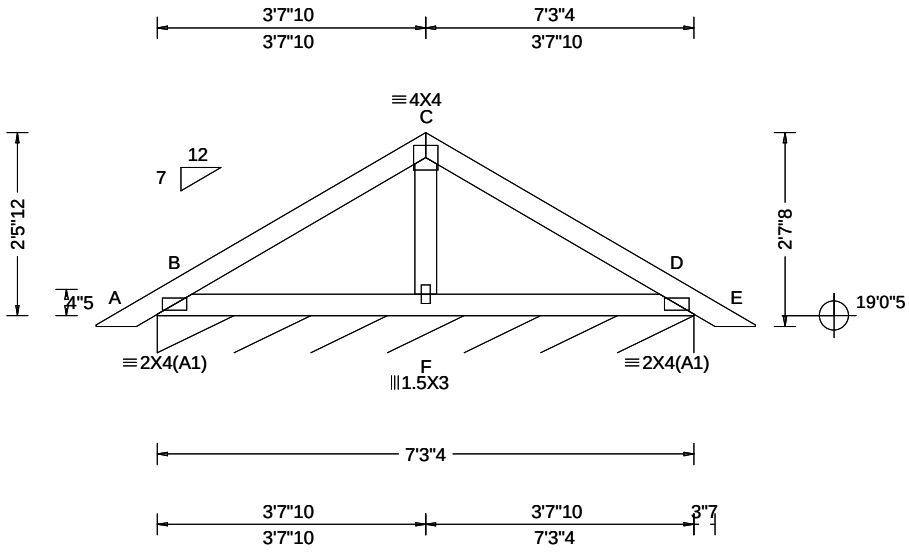
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29873 COMN FROM: RJL	Ply: 1 Qty: 24	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** PB1 9' Common	Cust: R 857 JRef:1Y8R8570002 T14 DrwNo: 090.25.1313.36817 SSB / DF 03/31/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 20.20 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.002 D	999	360
VERT(CL):	0.003 D	999	240
HORZ(LL):	-0.002 D	-	-
HORZ(TL):	0.002 D	-	-

Creep Factor: 2.0
Max TC CSI: 0.119
Max BC CSI: 0.110
Max Web CSI: 0.011

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	86	/-	/-	/39	/6	/5

Wind reactions based on MWFRS
B Brg Wid = 87.3 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Refer to DWG PB160220723 for piggyback details.

COA #0 278
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03/31/2025

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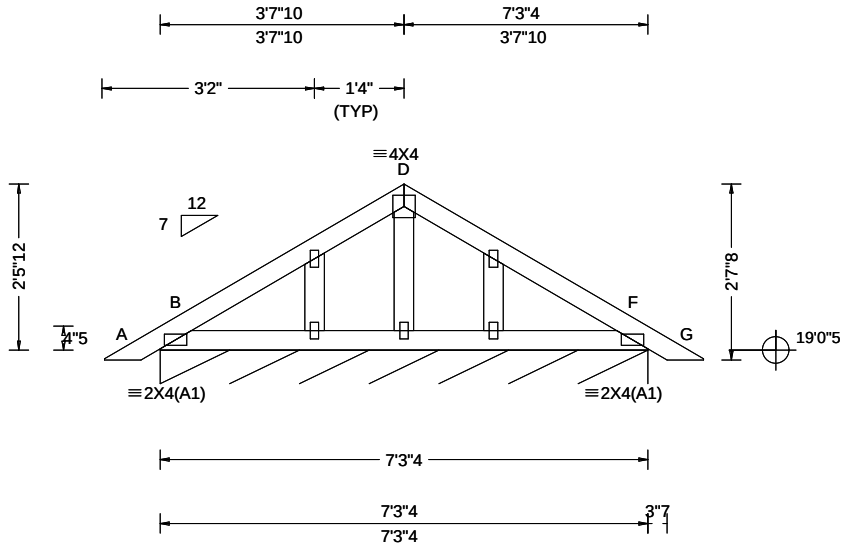
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29876 GABL FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** PB2-G 9' Gable	Cust: R 857 JRef:1Y8R8570002 T13 DrwNo: 090.25.1313.53843 SSB / DF 03/31/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 20.20 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	F	999	360
VERT(CL): 0.001	F	999	240
HORZ(LL): -0.000	F	-	-
HORZ(TL): 0.001	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.032
Max BC CSI: 0.025
Max Web CSI: 0.033

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	91	/-	/-	/41	/29	/5

Wind reactions based on MWFRS
B Brg Wid = 87.3 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

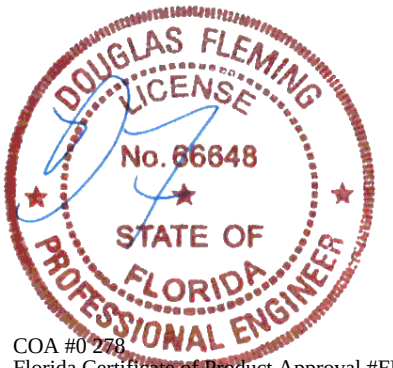
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Refer to DWG PB160220723 for piggyback details.



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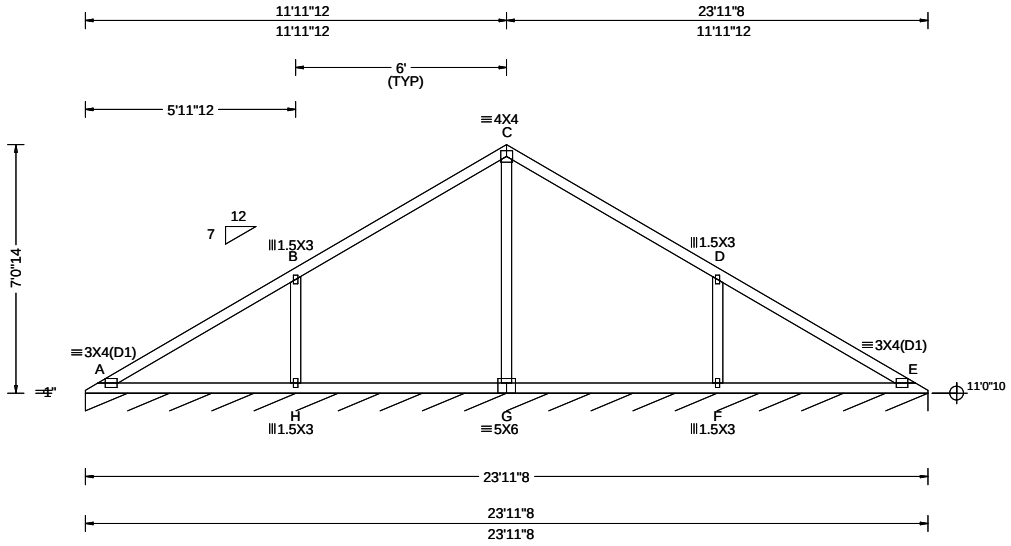
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29837 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** V1 23'11"8 Valley		Cust: R 857 JRef:1Y8R8570002 T27 DrwNo: 090.25.1313.55457 SSB / DF 03/31/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.013 A 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.026 A 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.005 A - -
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.010 A - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.499
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.273
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: No	Max Web CSI: 0.297
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	76	/-	/-	/36	/-	/4

Wind reactions based on MWFRS
E Brg Wid = 287 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - H	125	-395	F - D	125	-395

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

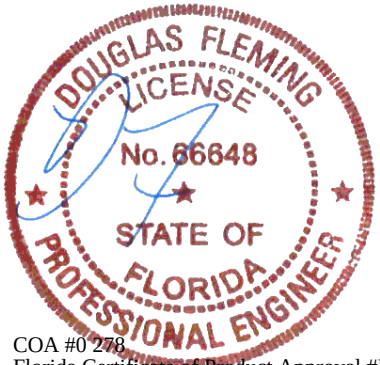
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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03/31/2025

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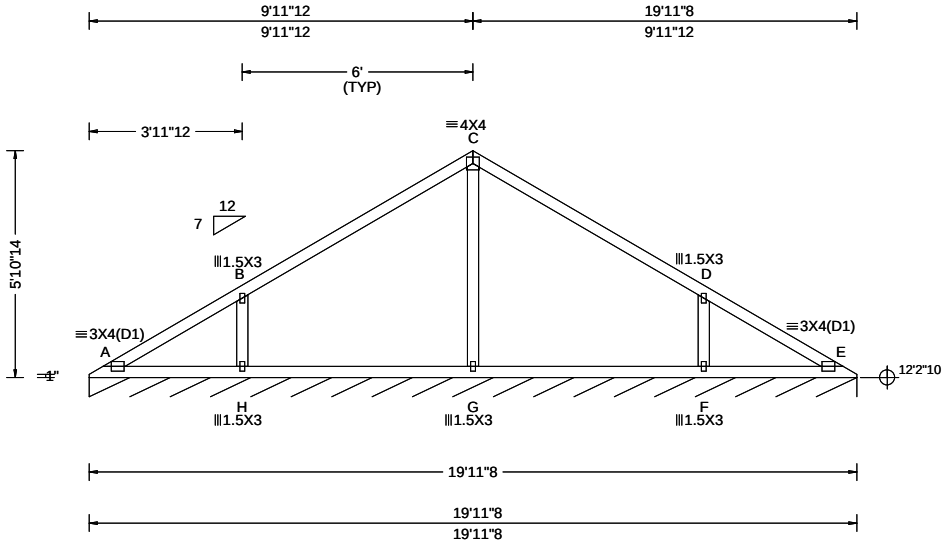
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29839 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** V2 19'11"8 Valley		Cust: R 857 JRef:1Y8R8570002 T30 DrwNo: 090.25.1313.56527 SSB / DF 03/31/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.32 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 A 999 360 VERT(CL): 0.003 A 999 240 HORZ(LL): -0.001 A - - HORZ(TL): 0.001 E - - Creep Factor: 2.0 Max TC CSI: 0.432 Max BC CSI: 0.213 Max Web CSI: 0.153 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	76	/-	/-	/36	/-	/4

Wind reactions based on MWFRS
E Brg Wid = 239 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

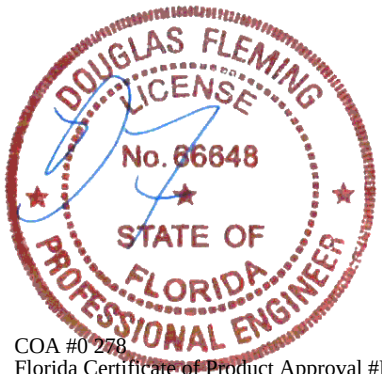
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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155 Harlem Ave
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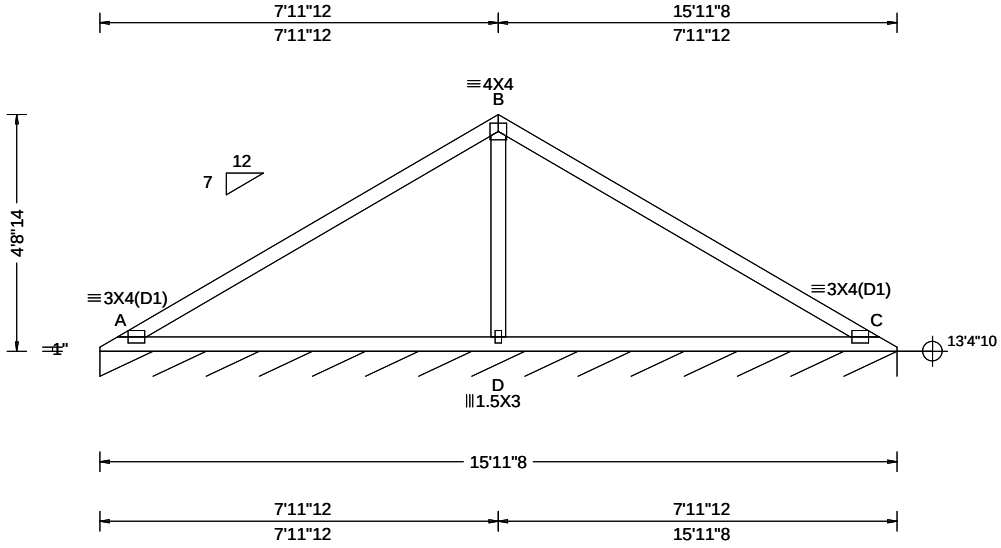
Glenview, IL 60025

SEQN: 29841 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** V3 15'11"8 Valley	Cust: R 857 JRef:1Y8R8570002 T28 DrwNo: 090.25.1313.57407 SSB / DF 03/31/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.90 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.046	C	999	360
VERT(CL): 0.088	C	999	240
HORZ(LL): -0.022	C	-	-
HORZ(TL): 0.042	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.705
Max BC CSI: 0.614
Max Web CSI: 0.349

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

Loc	Gravity R+	Gravity / R-	Gravity / Rh	Non-Gravity / Rw	Non-Gravity / U	Non-Gravity / RL
C*	76	/-	/-	/36	/-	/4

Wind reactions based on MWFRS
C Brg Wid = 191 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	597	-92	B - C	597	-91

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - D	123	-435	D - C	123	-435

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - D	157	-878

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

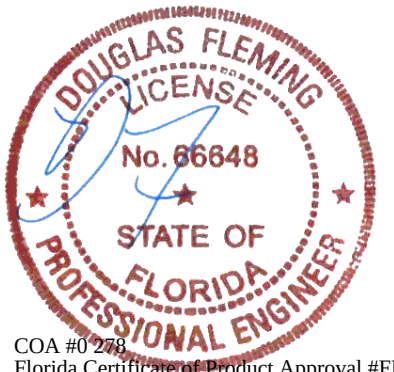
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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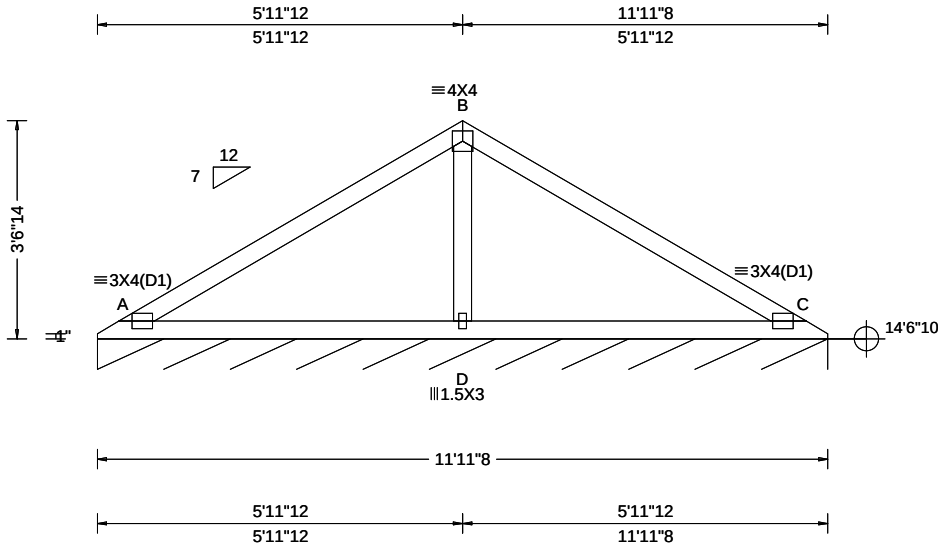
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29843 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** V4 11'11"8 Valley	Cust: R 857 JRef:1Y8R8570002 T31 DrwNo: 090.25.1313.58363 SSB / DF 03/31/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs), or *=PLF**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.49 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.019 A 999 360 VERT(CL): 0.037 A 999 240 HORZ(LL): -0.009 C - - HORZ(TL): 0.017 C - - Creep Factor: 2.0 Max TC CSI: 0.381 Max BC CSI: 0.340 Max Web CSI: 0.141 VIEW Ver: 24.02.00C.1213.15	Gravity / Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 76 /- /- /36 /- /4 Wind reactions based on MWFRS C Brg Wid = 143 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# **Maximum Web Forces Per Ply (lbs)** Webs Tens.Comp. B - D 145 -549

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

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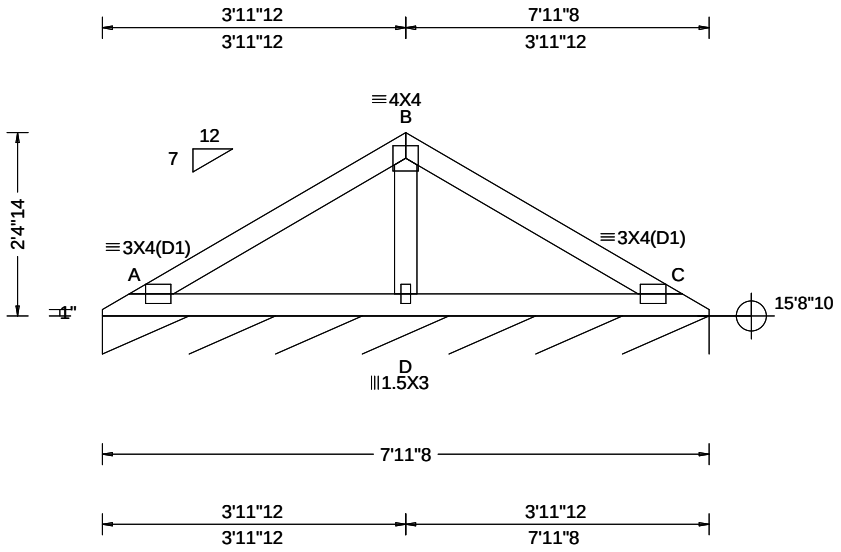
North Building, 4th Floor
Glenview, IL 60025

SEQN: 29845 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60881a PIZZAGALLI RESIDENCE **Truss Label:** V5 7'11"8 Valley	Cust: R 857 JRef:1Y8R8570002 T29 DrwNo: 090.25.1313.59293 SSB / DF 03/31/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.07 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.008 C 999 360 VERT(CL): 0.011 C 999 240 HORZ(LL): -0.004 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.146 Max BC CSI: 0.139 Max Web CSI: 0.057 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	76	/-	/-	/35	/-	/4

Wind reactions based on MWFRS
C Brg Wid = 95.5 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

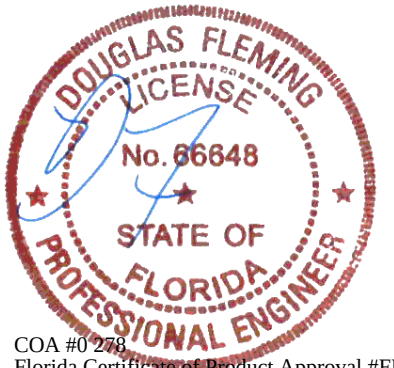
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section

(L) = Minimum nailing distance on each side of damaged area (B)

(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = (2)(L) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.
Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

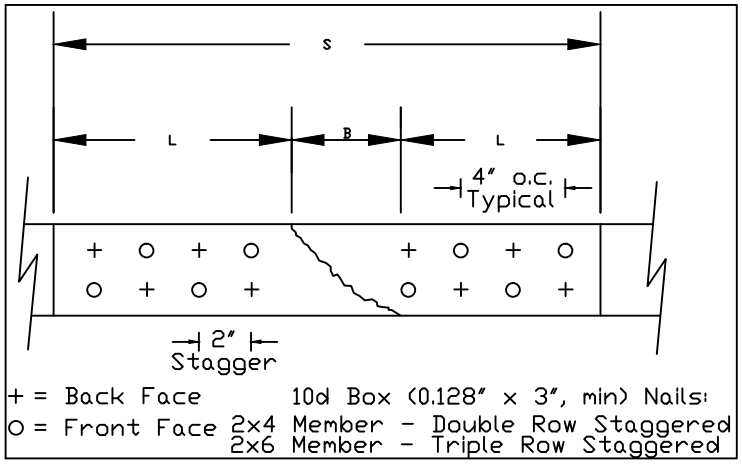
This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

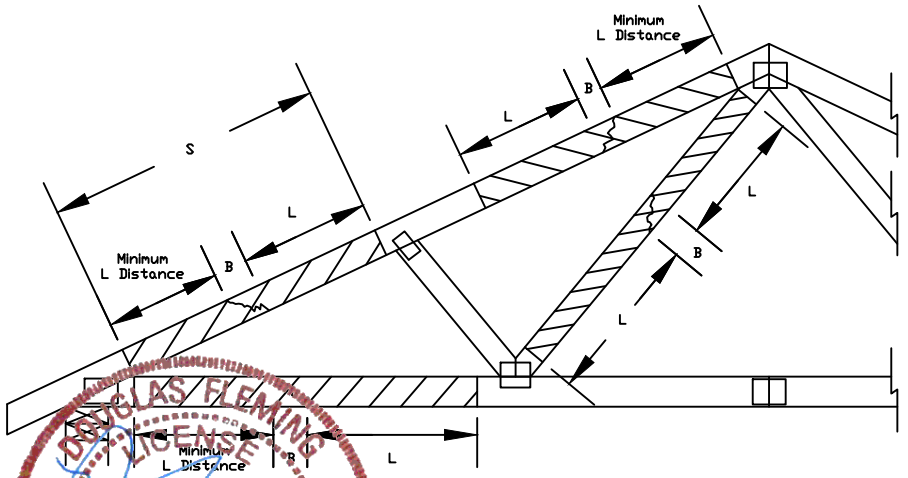
Broken chord may not support any tie-in loads.

Load Duration = 0%
Member forces may be increased for Duration of Load

Member	Size	L	Maximum Member Axial Force SPF-C	HF	DF-L	SYP
Web Only	2x4	12"	620#	635#	730#	800#
Web Only	2x4	18"	975#	1055#	1295#	1415#
Web or Chord	2x4	24"	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30"	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36"	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42"	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48"	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#



Nail Spacing Detail



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AN ITW COMPANY
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North Building, 4th Floor
Glenview, IL 60025

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/31/2025

COA #0 278

SPACING 24.0" MAX

REF	MEMBER REPAIR
DATE	10/01/14
DRWG	REPCHRD1014

Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
535# connection or with (1) Simpson H2.5A or equivalent connector for
ASCE 7-22 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
Or
ASCE 7-22 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

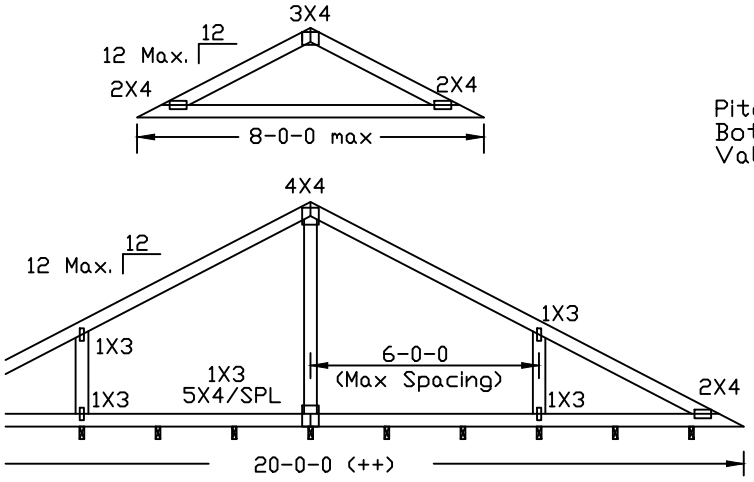
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

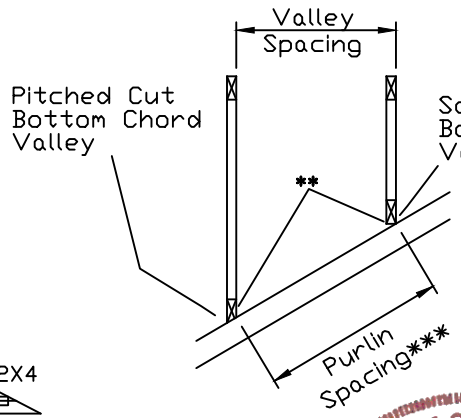
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

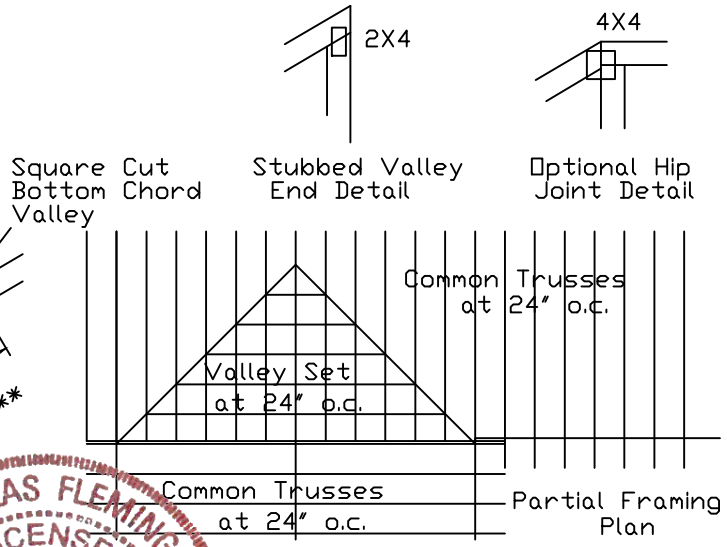
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



Supporting trusses at 24" o.c. maximum spacing.





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03/31/2025

COA #0 278

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TC LL	30	30	40 PSF
TC DL	20	15	7 PSF
BC DL	10	10	10 PSF
BC LL	0	0	0 PSF
TOT. LD.	60	55	57 PSF
DUR.FAC.	1.25/1.33	1.15	1.15
SPACING		24.0"	

REF	VALLEY DETAIL
DATE	07/03/2023
DRWG	VAL180220723

Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location:
140 mph for SP (G = 0.55, min.),
125 mph for DF-L (G = 0.50, min.), or
105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

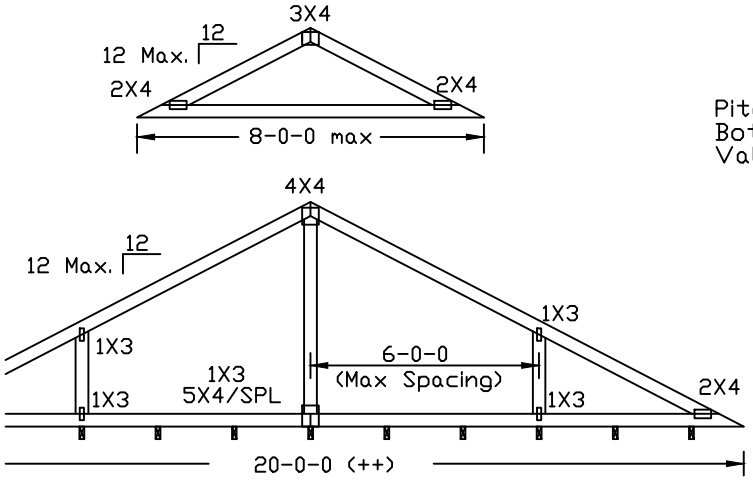
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



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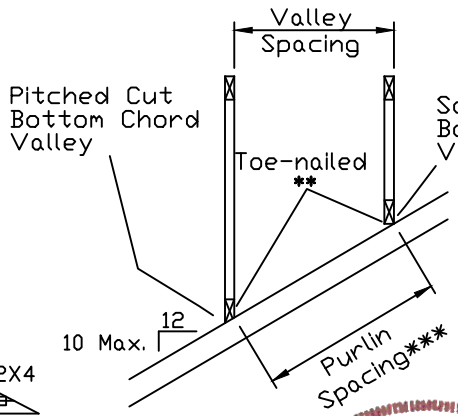

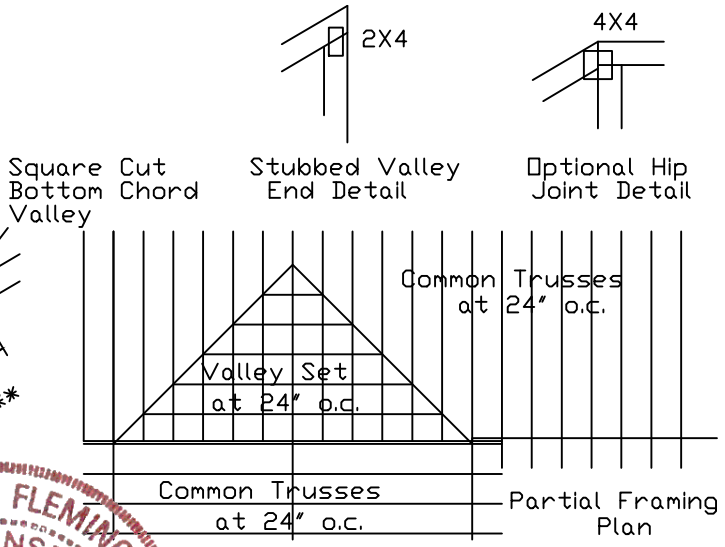

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/31/2025
COA #0 278

TC LL	30	30	40PSF
TC DL	20	15	7PSF
BC DL	10	10	10 PSF
BC LL	0	0	0PSF
TOT. LD.	60	55	57PSF
DUR.FAC.	1.25/1.33	1.15	1.15
SPACING		24.0"	

REF	VALLEY DETAIL
DATE	07/03/2023
DRWG	VALTN220723

Gable End Wind Bracing Details - Stiffback w/ Diagonal Bracing

Apply single or double stiffback as per Engineer's sealed truss design referencing this detail.

Refer to Engineer's sealed truss design for additional information not provided on this detail.

The required locations for lateral restraint or bracing depicted on this detail are for the permanent lateral transfer and support to transfer load and reduce buckling lengths. Details shall be specified by the Building Designer or other Registered Design Professional. This Detail does not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Refer to Building Designer for conditions not addressed by this detail.

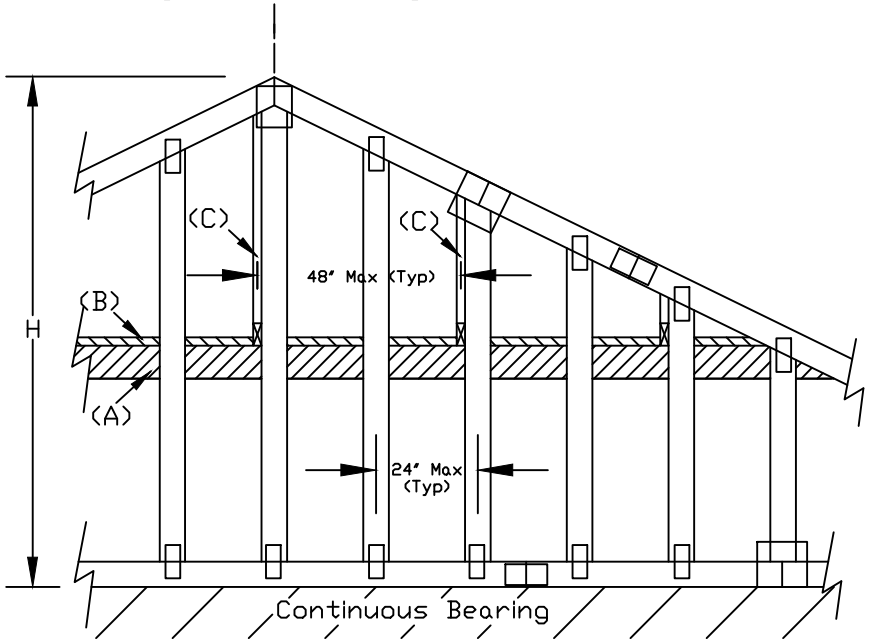
Gable Lateral Bracing Components

(A) Stiffback. Provide connection to each intersecting stud and chord.

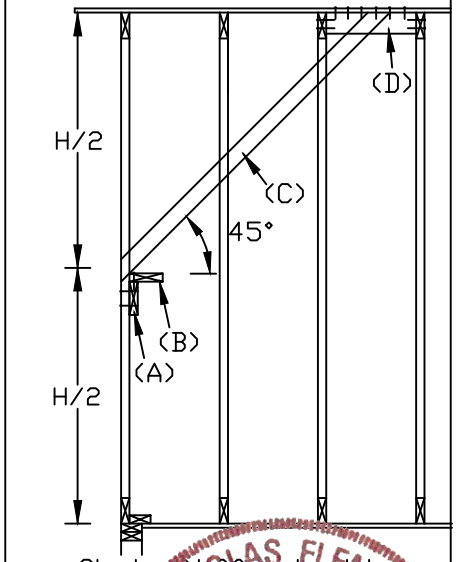
(B) L-reinforcement. Provide connection to narrow edge of stiffback.

(C) Diagonal brace. Provide connection to gable stud at bottom end and to blocking at top end.

(D) Blocking, cut to fit tight between trusses. Attach blocking to trusses at each end and to roof sheathing.

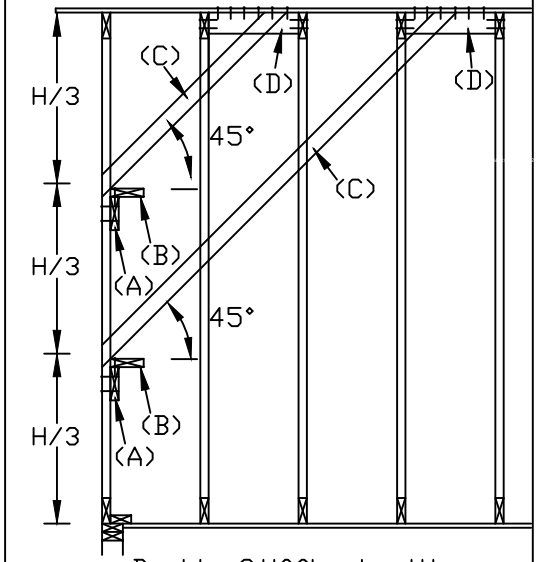


Single Stiffback



Single Stiffback with Diagonal Bracing Spaced at 1/2 Truss Height

Double Stiffback



Double Stiffback with Diagonal Bracing Spaced at 1/3 Truss Height

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/31/2025

COA #0 278

MAX. TOT. LD.

MAX. SPACING

REF	GE STIFFBACK
DATE	09/27/2023
DRWG	GBLDIAG220923

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

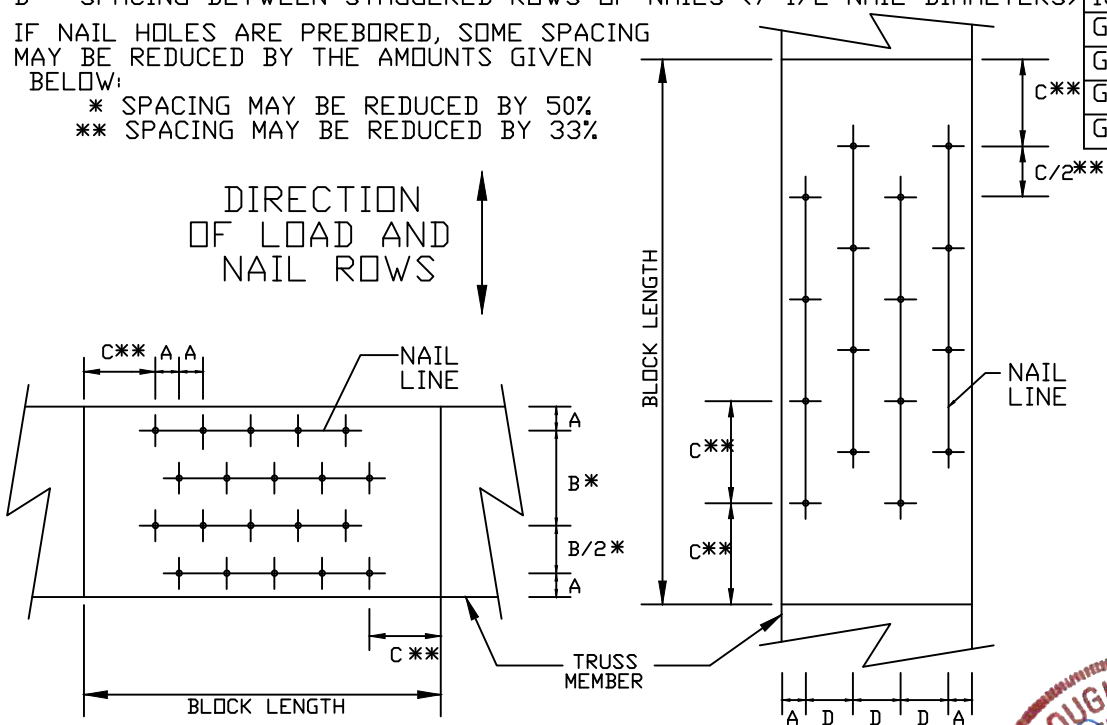
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A - EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

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REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014