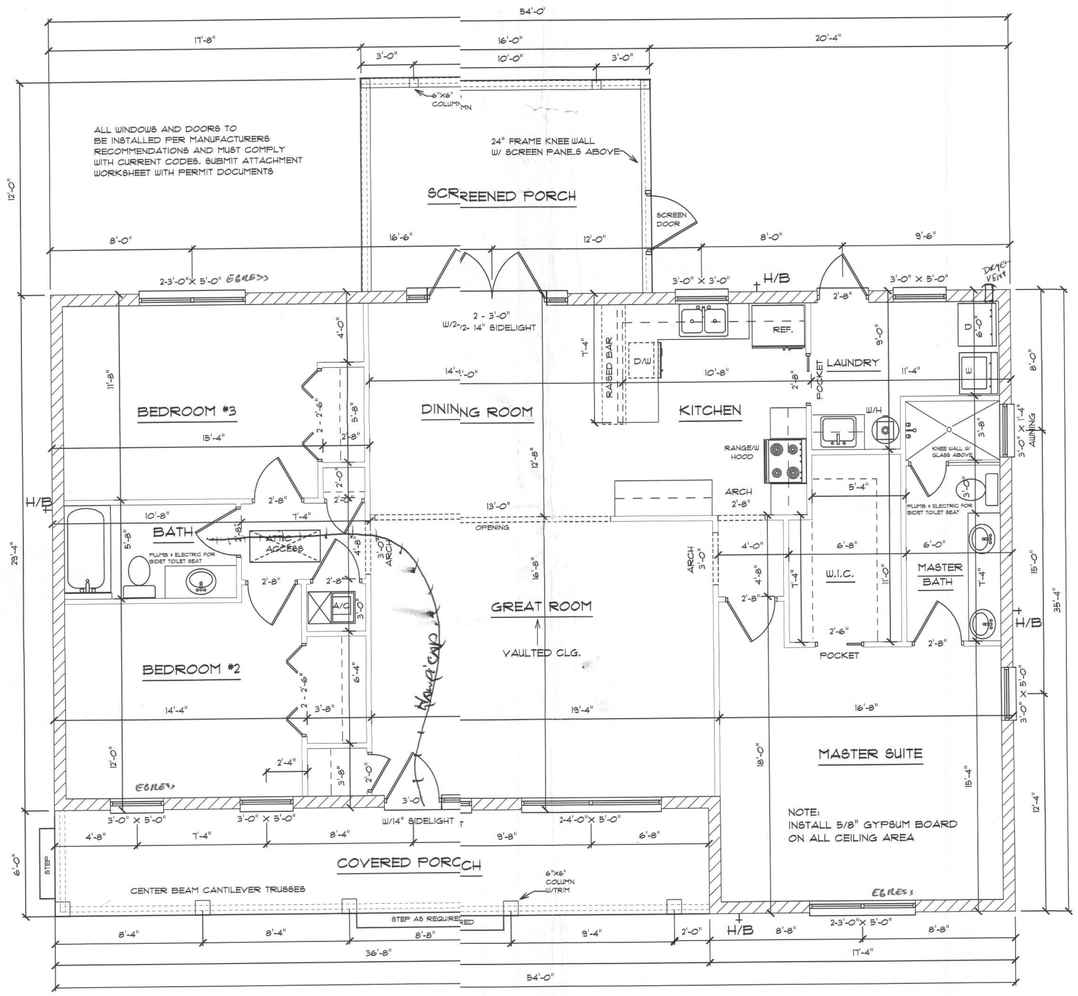


MAIN FLOOR PLAN

IT IS THE OWNER AND OR THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL STRUCTURAL ASPECTS OF THESE DRAWINGS. THIS INCLUDES BUT NOT LIMITED TO DIMENSIONS, WALL HEIGHTS AND MATERIAL, WINDOW SIZE AND LOCATION. ALSO ALL STATE AND LOCAL CODES MUST BE FOLLOWED BY ACCEPTING THESE PLANS OWNER ACCEPTS ALL RESPONSIBILITY AND DOES NOT HOLD COLACINO DRAFTING AND DESIGN LIABLE FOR ANY ERRORS OR OMISSIONS
PT LUMBER TO BE USED FOR CONTACT TO ALL GROUND

APPROX. AREA

LIVING	1688
COVERED PORCH	220
SCREENED PORCH	192
TOTAL	2100

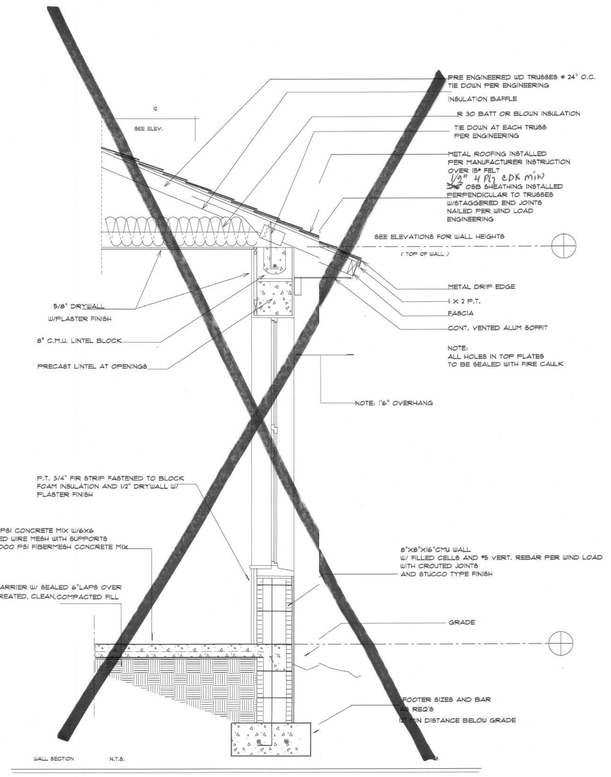
COLACINO
DRAFTING&DESIGN
PH. 352-472-3462

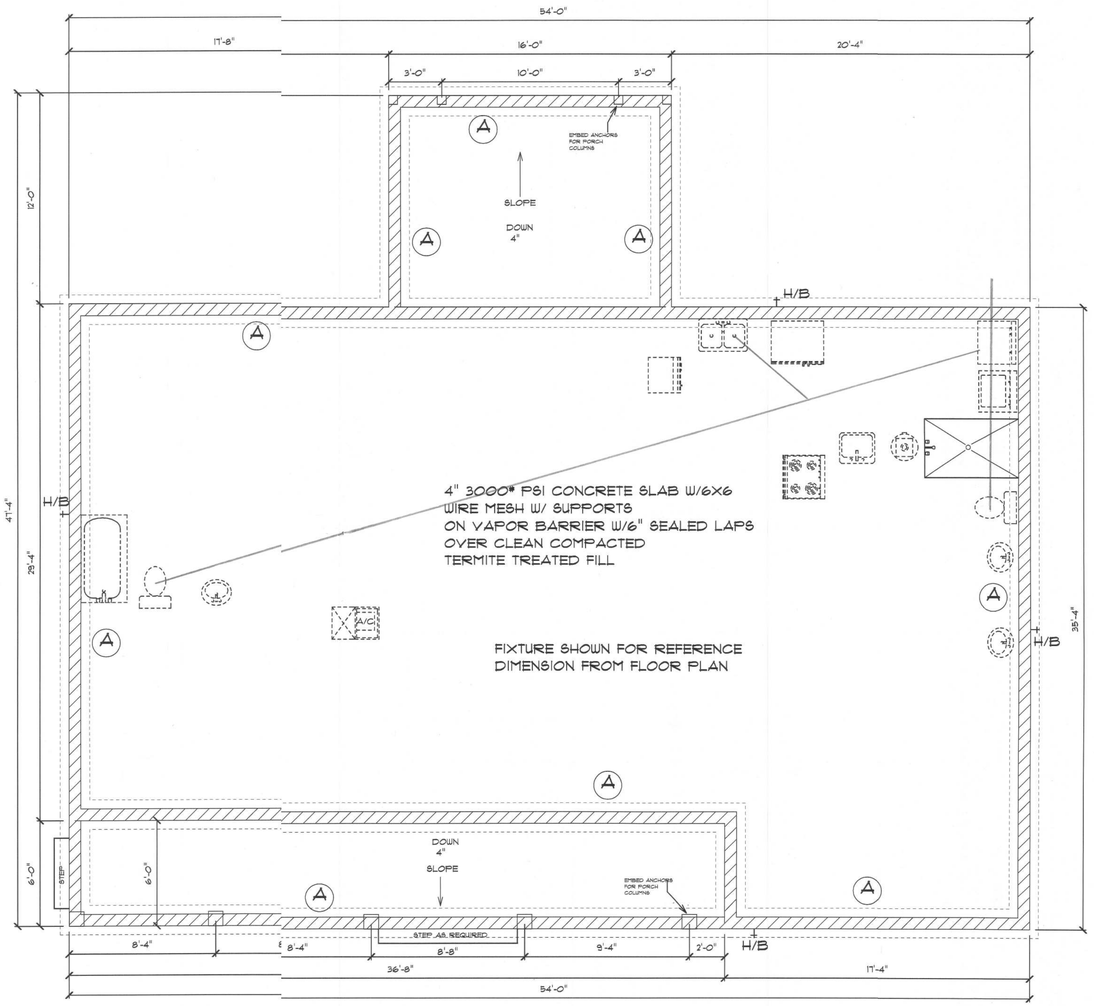
DATE 8/2/10
REVISED
REVISED

1

JENKINS RESIDENCE

SCALE 1/4" =1'
DRAWN BY P.A.C.
APPROVED





NOTE:
ALL FOOTING SIZES SHOWN ARE FOR STANDARD CONSTRUCTION AND MUST BE REVIEWED AND VERIFIED BY STRUCTURAL ENGINEER TO ASSURE ALL SIZES ARE ADEQUATE FOR SPECIFIC LOADING SITUATIONS OF BUILDING

NOTE:
VERIFY INTERIOR BEARING WALLS WITH ROOF TRUSS COMPANY

FOUNDATION PLAN

(A) 20" WIDE X 12" DEEP FOOTING WITH 2-#5 CONT. REBAR

(VERIFY ALL WITH ENGINEERING)

2

DATE 8/2/10
REVISED
REVISED
REVISED

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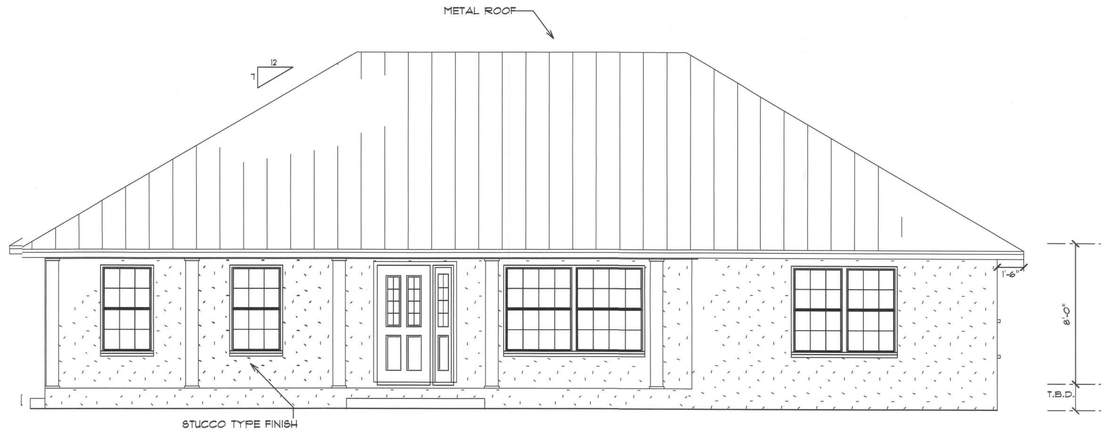
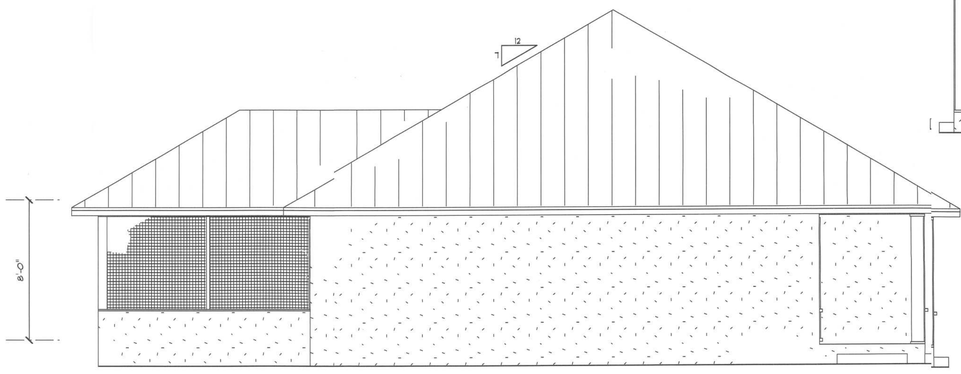
SCALE 1/4" =1'
DRAWN BY P.A.C.
APPROVED

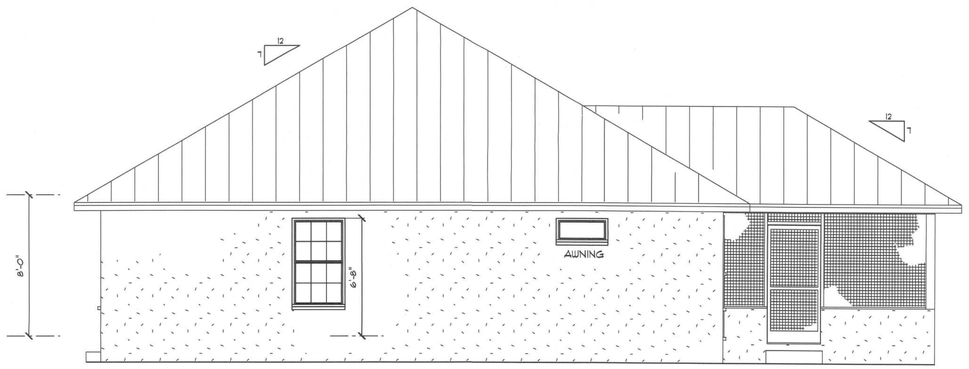
JENKINS RESIDENCE



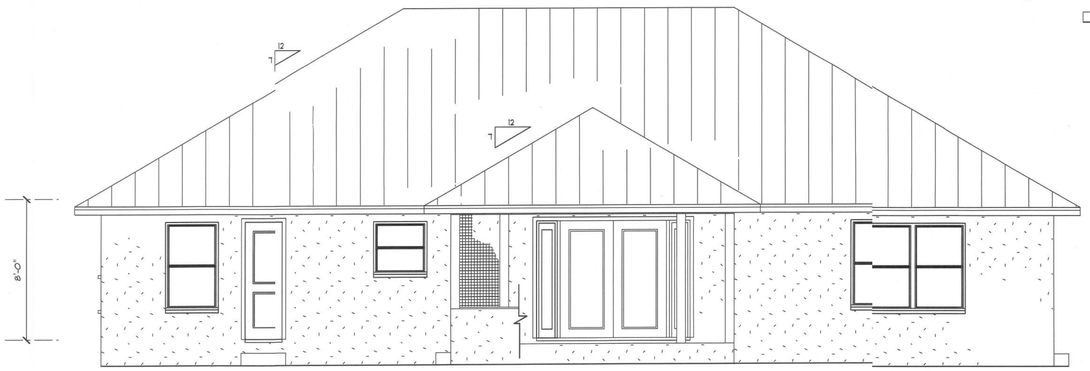
FRONT ELEVATION



LEFT ELEVATION



RIGHT ELEVATION



REAR ELEVATION

SCREEN NOT SHOWN FOR CLARITY

3

DATE 8/2/10
REVISED
PREVIOUSLY

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SCALE 1/4" =1'
DRAWN BY P.A.C.
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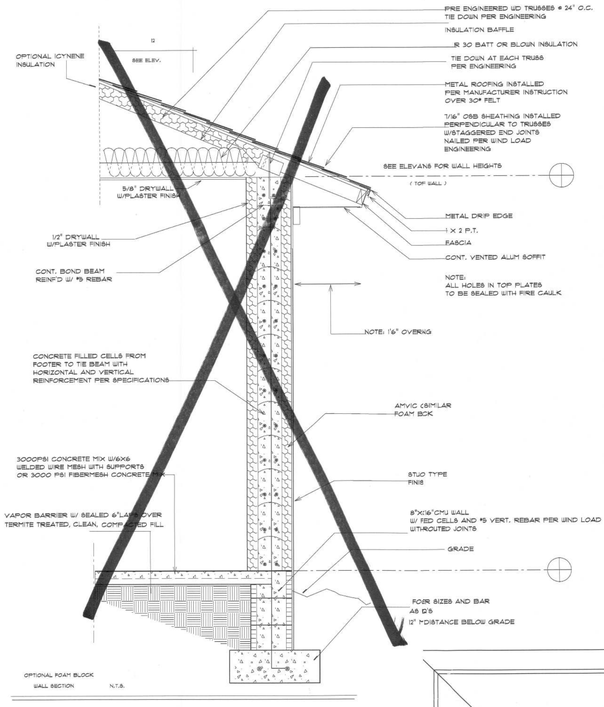
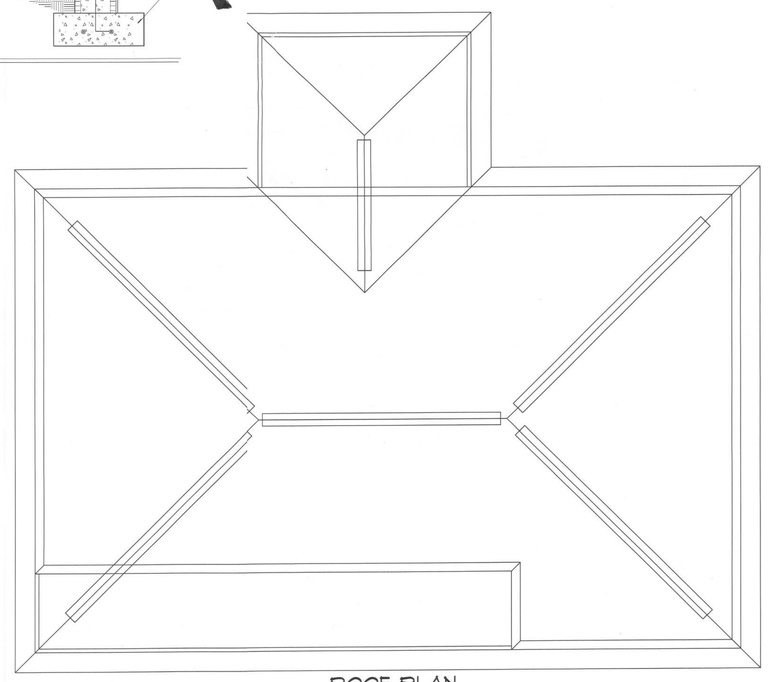
JENKINS RESIDENCE



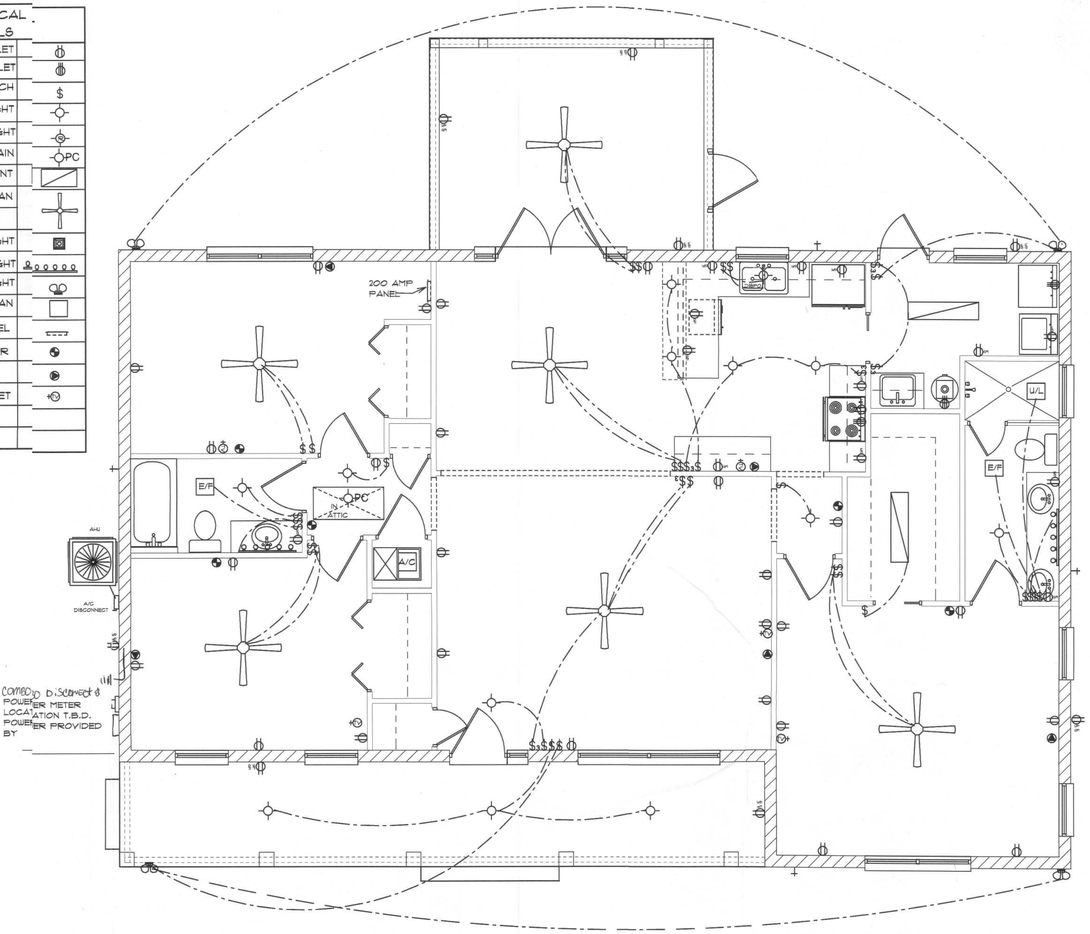


VENTING
2100 ⊤ 3.5 = 35
300 2 .1 = 35' RIDGE VENT

ROOF PLAN
3/16" SCALE

7/12 ROOF PITCH
1'-6" OVERHANGS

ELECTRICAL SYMBOLS	
OUTLET	
220 OUTLET	
SWITCH	$
LIGHT	
RECESS LIGHT	
PULL CHAIN	PC
FLUORESCENT	
CEILING FAN	
WALL MOUNT LIGHT	
VANITY LIGHT	
FLOOD LIGHT	
CLG. LITE/FAN	
PANEL	
SMOKE DETECTOR (IN COMMON)	
PHONE	
TV/CABLE/INTENET	



ELECTRIC PLAN IS A SUGGESTED LOCATION FOR SWITCHES PLUGS AND LIGHTS IT IS THE OWNER AND OR THE CONTRACTORS RESPONSIBILITY TO FOLLOW ALL STATE AND LOCAL CODES.

ELECTRICAL PLAN

SMOKE DETECTORS TO BE 110 WIRED W/ BATTERY BACK UP AS REQUIRED BY CODE

ALL OUTLETS TO BE ARC FAULT PROTECTED CIRCUIT

ALL RECEPTACLES NEAR WATER TO BE GFI

ALL EXTERIOR OUTLETS TO BE WATERPROOF GFI

PHONE AND TV OUTLETS PER OWNERS

Fire caulk all penetrations through top plate

UFER Ground to service 20' Re-Bar

BI'DES

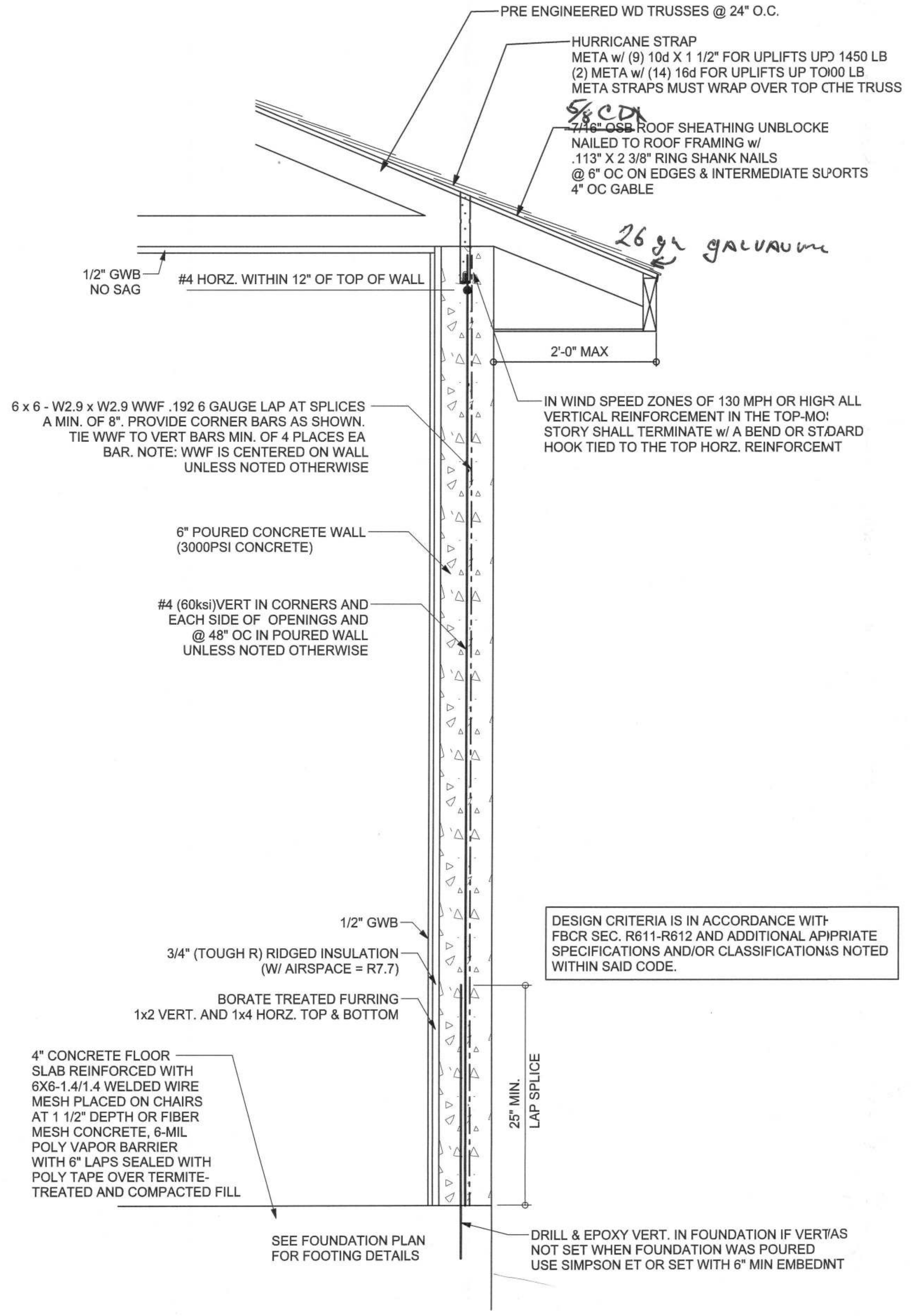
4

DATE 8/2/10
REVISED
REVISED

COLACINO
DRAFTING&DESIGN
PH. 352-472-3462

SCALE 1/4" =1'
DRAWN BY P.A.C.
APPROVED

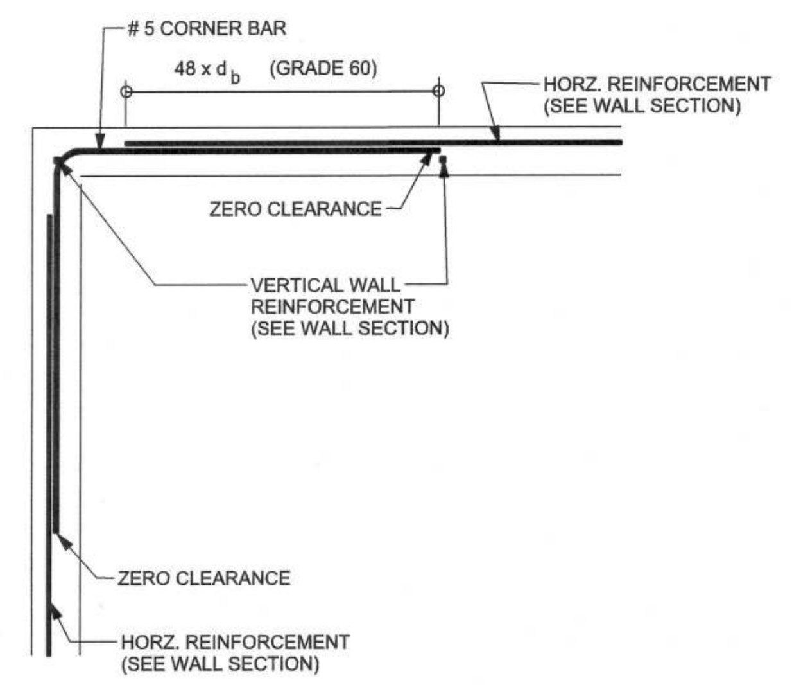
JENKINS RESIDENCE



DESIGN CRITERIA IS IN ACCORDANCE WITH FBCR SEC. R611-R612 AND ADDITIONAL APPRIATE SPECIFICATIONS AND/OR CLASSIFICATIONS NOTED WITHIN SAID CODE.

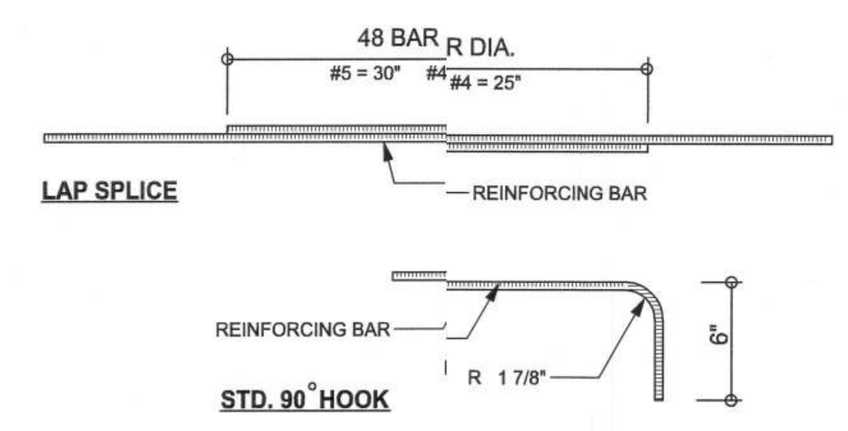
POURED CONCRETE WALL SECTION

SCALE: 1" = 1'-0"



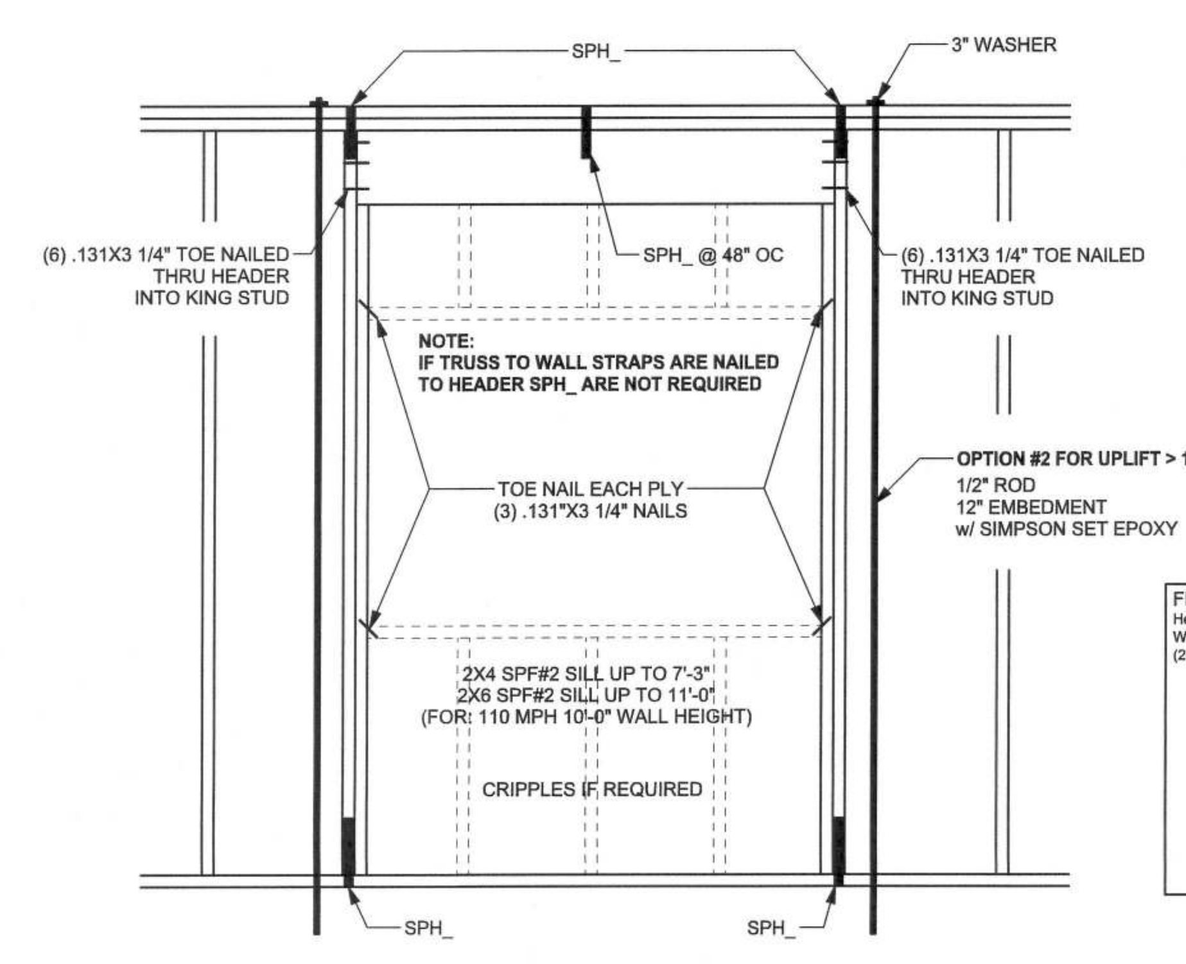
TYPICAL CORNER DETAIL

SCALE: 1/2" = 1'=0"



REBAR DETAILS

SCALE: N.T.S.



(TYP.) HEADER

ONE STORY WOOD FRAME

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

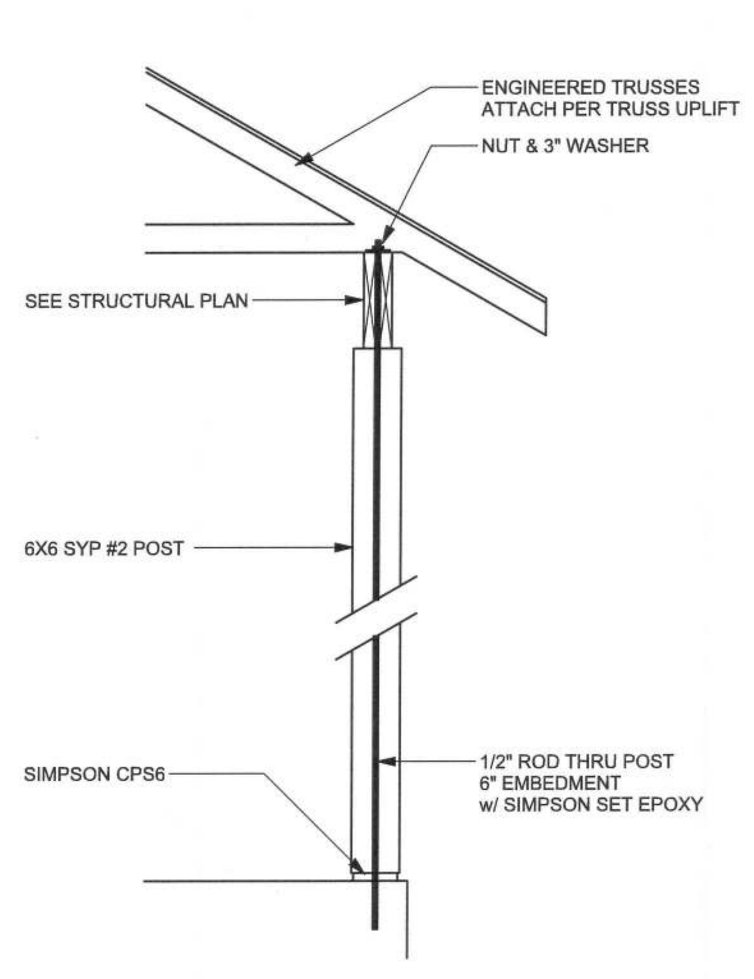
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

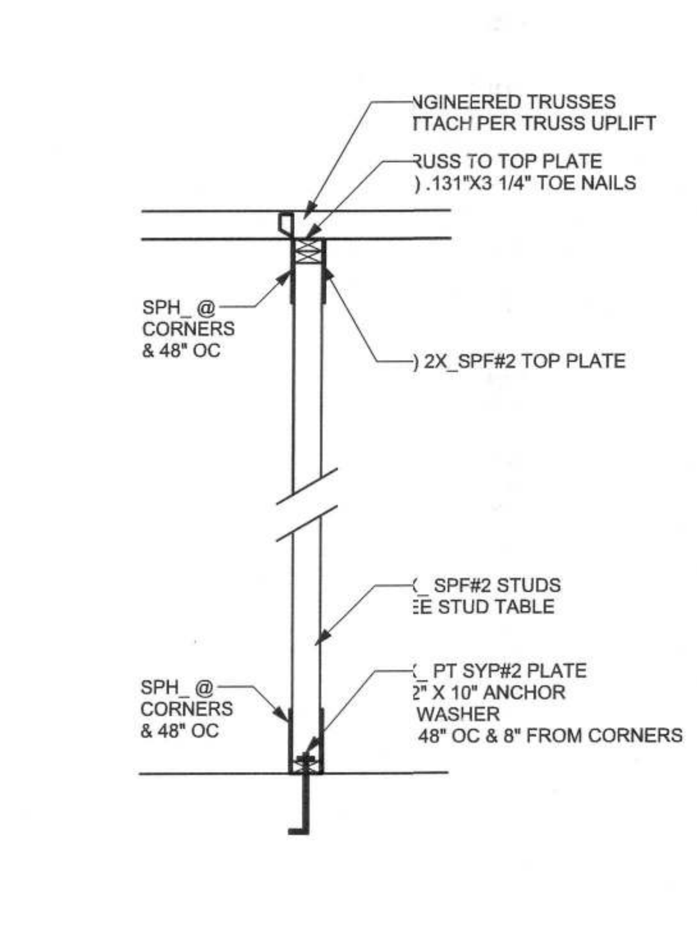
ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



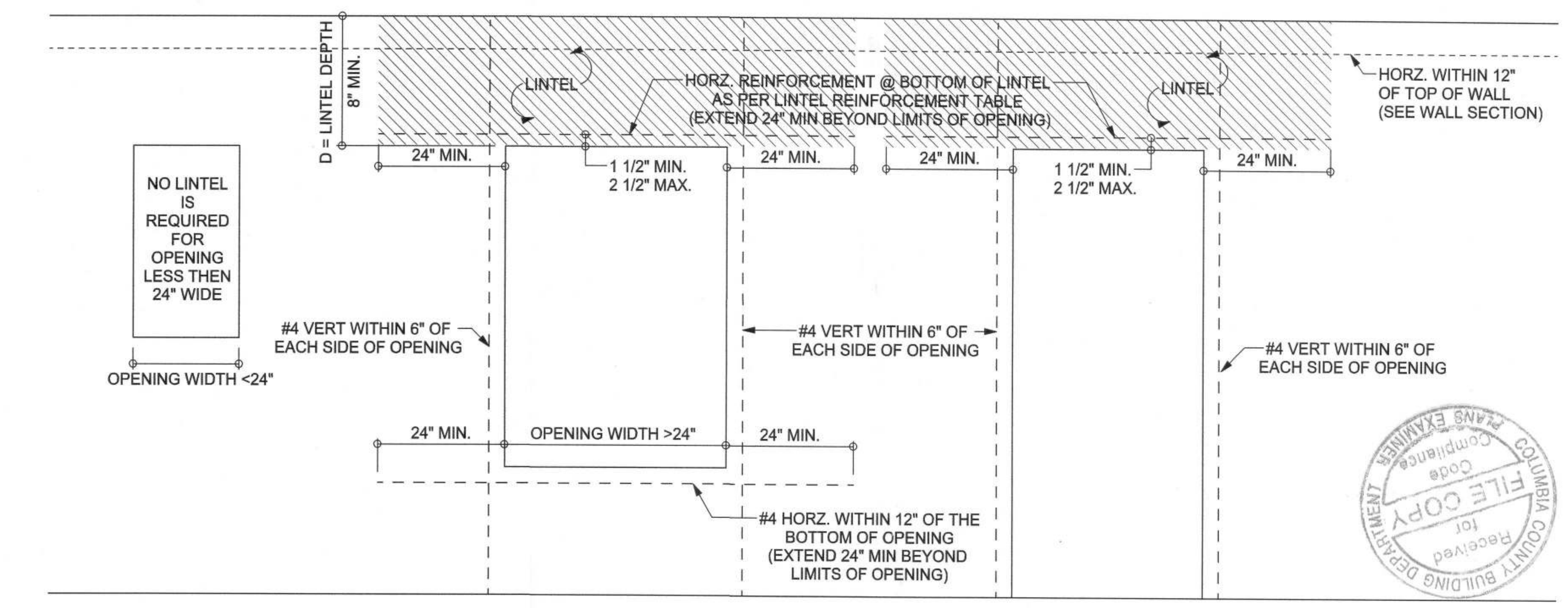
(TYP.) PORCH POST

ONE STORY WOOD



(TYP.) INTERIOR BEARING WALL

ONE STORY WOOD FRAME w/ STPS & ANCHORS



TYPICAL OPENING / LINTEL DETAIL

SCALE: 1/2" = 1'=0"

MAX. ALLOWABLE CLEAR SPANS FOR LINTELS WITHOUT STIRRUPS IN LOAD-BEARING WALLS (#4 BOTTOM LINTEL REINFORCEMENT)

MIN. LINTEL DEPTH, D	SUPPORTING FRAME ROOF ONLY			SUPPORTING FRAME 2nd STORY & ROOF			SUPPORTING ICF 2nd STORY & FRAME ROOF		
	MAX. ROOF / FLOOR CLEAR SPAN								
	24'	28'	34'	24'	28'	34'	24'	28'	34'
8"	3'-10"	2'-11"	2'-10"	2'-9"	2'-7"	2'-6"	2'-9"	2'-7"	2'-6"
12"	5'-1"	4'-10"	4'-8"	4'-8"	4'-5"	4'-3"	4'-2"	4'-0"	3'-10"
16"	7'-0"	7'-5"	6'-5"	5'-1"	4'-10"	4'-8"	4'-8"	4'-5"	4'-3"
20"	8'-11"	8'-6"	8'-2"	6'-7"	6'-3"	6'-0"	5'-11"	5'-8"	5'-5"
24"	10'-7"	10'-1"	9'-8"	8'-5"	7'-8"	7'-4"	7'-2"	6'-10"	6'-7"

MIMIMUM BOTTOM BAR LINTEL REINFORCEMENT FOR LARGE CLEAR SPANS WITH STIRRUPS IN LOAD-BEARING WALL

MAX. LINTEL CLEAR SPAN	MIN. LINTEL DEPTH	SUPPORTING FRAME ROOF ONLY	SUPPORTING FRAME 2nd STORY & ROOF	SUPPORTING ICF 2nd STORY & FRAME ROOF
		MAX. ROOF / FLOOR CLEAR SPAN = 34'		
12'-3"	20"	1-#6 ; 2-#4		
	24"	1-#5	2-#5	2-#6
16'-3"	24"	2-#5		

MAX. ALLOWABLE CLEAR SPANS FOR LINTELS WITH STIRRUPS IN LOAD-BEARING WALLS (#4 BOTTOM LINTEL REINFORCEMENT)

ALLOWABLE LINTELS CLEAR SPANS: 4'-8" / 1'-5": MIDDLE PORTION OF SPAN WHERE STIRRUPS ARE NOT REQUIRED

MIN. LINTEL DEPTH, D	SUPPORTING FRAME ROOF ONLY			SUPPORTING FRAME 2nd STORY & ROOF			SUPPORTING ICF 2nd STORY & FRAME ROOF		
	MAX. ROOF / FLOOR CLEAR SPAN								
	24'	28'	34'	24'	28'	34'	24'	28'	34'
8"	5'-8" / 2'-2"	5'-5" / 2'-0"	5'-2" / 1'-10"	4'-2" / 1'-2"	4'-0" / 1'-1"	3'-10" / 1'-0"	3'-9" / 1'-0"	3'-7" / 11"	3'-5" / 10"
12"	7'-4" / 3'-7"	7'-0" / 3'-3"	6'-8" / 3'-0"	5'-6" / 2'-0"	5'-3" / 1'-10"	5'-0" / 1'-8"	4'-11" / 1'-7"	4'-8" / 1'-5"	4'-6" / 1'-4"
16"	8'-7" / 4'-10"	8'-2" / 4'-5"	7'-10" / 4'-1"	6'-7" / 2'-9"	6'-3" / 2'-6"	6'-0" / 2'-4"	5'-10" / 2'-3"	5'-7" / 2'-1"	5'-4" / 1'-11"
20"	9'-8" / 6'-3"	9'-3" / 5'-9"	8'-10" / 5'-3"	7'-5" / 3'-7"	7'-1" / 3'-3"	6'-9" / 3'-0"	6'-8" / 2'-11"	6'-4" / 2'-7"	6'-1" / 2'-5"
24"	10'-7" / 7'-6"	10'-1" / 6'-10"	9'-8" / 6'-3"	8'-1" / 4'-4"	7'-9" / 4'-0"	7'-5" / 3'-8"	7'-4" / 3'-6"	7'-0" / 3'-2"	6'-8" / 2'-11"

MAX. ALLOWABLE CLEAR SPANS FOR LINTELS WITHOUT STIRRUPS IN GABLE END (NON-LOAD-BEARING WALLS) (#4 BOTTOM LINTEL REINFORCEMENT)

MIN. LINTEL DEPTH, D	SUPPORTING FRAME GABLE END WALL ONLY	SUPPORTING ICF SECOND STORY & GABLE END WALL
8"	16'-3"	4'-4"
12"	16'-3"	7'-0"
16"	16'-3"	9'-7"
20"	16'-3"	12'-0"
22"	16'-3"	14'-3"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = C

3.) WIND IMPORTANCE FACTOR = 1.0

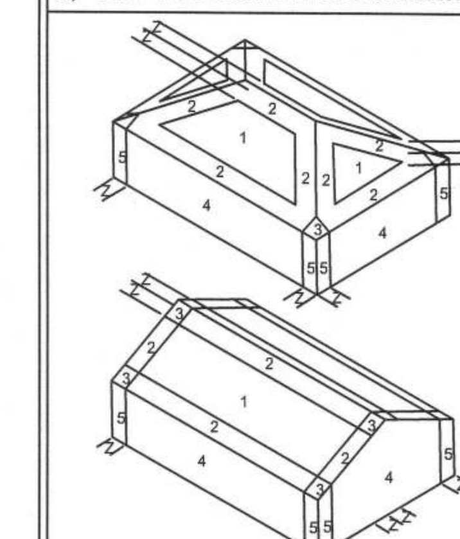
4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft2)			
	10		100	
1	27.8	-30.5	25.3	-25.3
2	27.8	-35.7	25.3	-30.5
2 O'hg		-56.8		-56.8
3	27.8	-35.7	25.3	-30.5
3 O'hg		-95.6		-59.3
4	30.5	-33.0	25.9	-28.5
5	30.5	-40.7	25.9	-31.6

Doors & Windows Worst Case (Zone 5, 10 ft2)	30.5	-40.7
8x7 Garage Door	27.3	-32.0
16x7 Garage Door	25.9	-29.4

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, PO 868, Lake City, FL 32056, 386-754-519

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, PE. for resolution. Do not proceed whout clarification.

CERTIFICATION: hereby certify that I have examined this plan and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 207, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specifid location.

MARK DISOSWAY P.E. 53915
November 11, 2010

Rosenboom Corstruction

Jenkins Residence

ADDRESS:
Columbii County, Florida

Mark Dsosway P.E.
P.O Box 868
Lake City Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: November 11, 2010

DRAWN BY:

STRUCTURAL BY: David Disosway

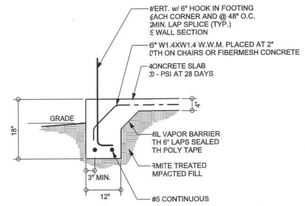
FINALS DATE: 11Nov10

JOB NUMBER: 1(11027

DRAWING NUMBER

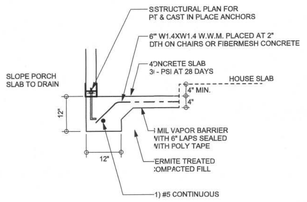
S-1

OF 3 SHEETS



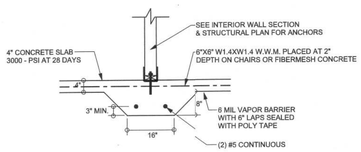
F1 S-2 MONOLITHIC OOTING

SCALE: 1/2" = 1'-0"



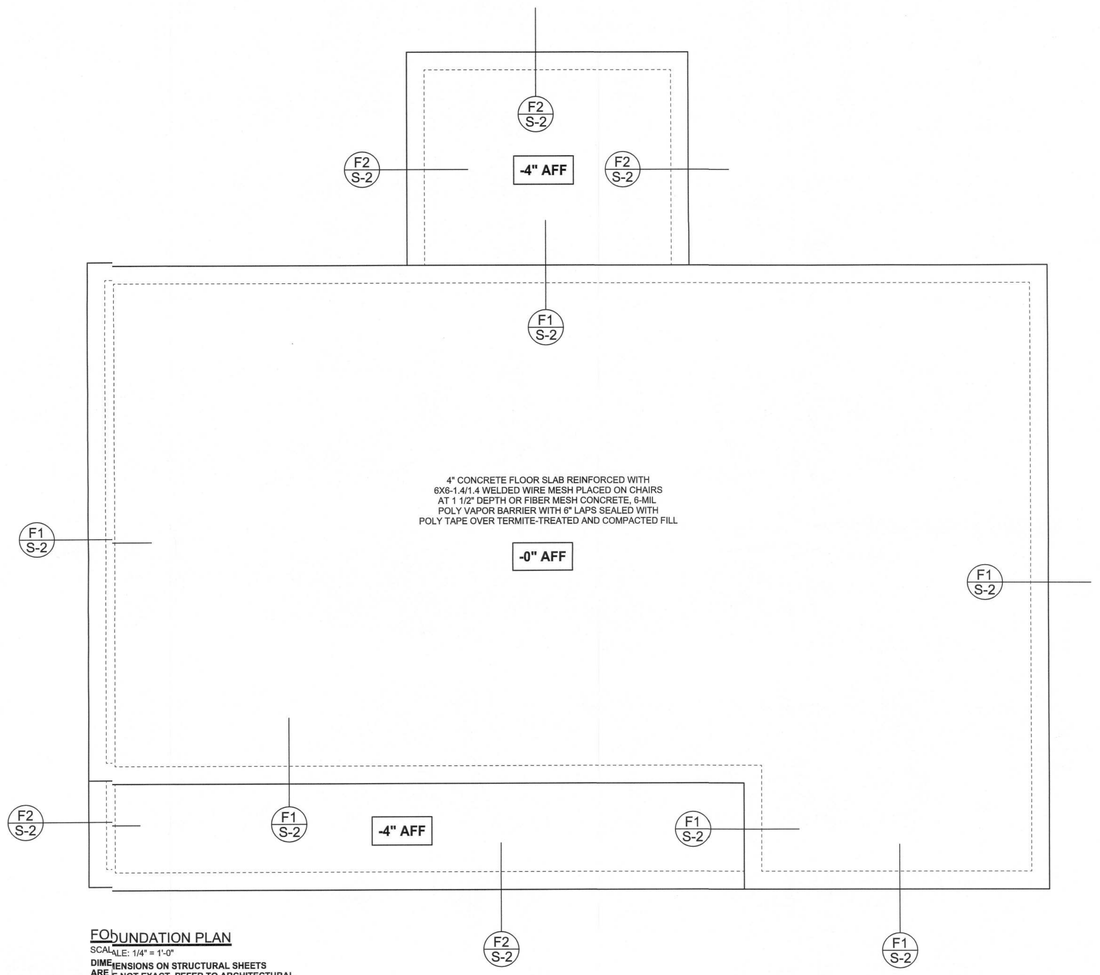
F2 S-2 PORCH FOOING

SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARNG FOOTING

SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.

REVISIONS	

SOFTPLAN ARCHITECTRAL DESIGN SOFTWARE

WINDLOAD ENGNEER: Mark Disosway, PE No.53915, PiB 868, Lake City, FL 32056, 386-754-419

DIMENSIONS:
Stated dimension supercede scaled dimensions. Refe all questions to Mark Disosway, I.E. for resolution. Do not proceed vihout clarification.

CERTIFICATION I hereby certify that I have examined this pln, and that the applicable portions of the pln, relating to wind engineering comply with secton R301.2.1, florida building code residential 007, to the best of my knowledge.

LIMITATION: Ths design is valid for one building, at spacied location.

MARK DISOSWAY P.E. 53915
STATE OF FLORIDA PROFESSIONAL ENGINEER
No 53915
November 11, 2010

Rosenboom Construction

Jenkirs Residence

ADDRESS:
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PRINTED DATE:
November 11, 2010

DRAWN BY	STRUCTURAL BY: David Discsway

FINALS DATE: 11Nov10

JOB NUMBER: 1011027

DRAWING NUMBER

S-2

OF 3 SHEETS

ANCHOR TABLE

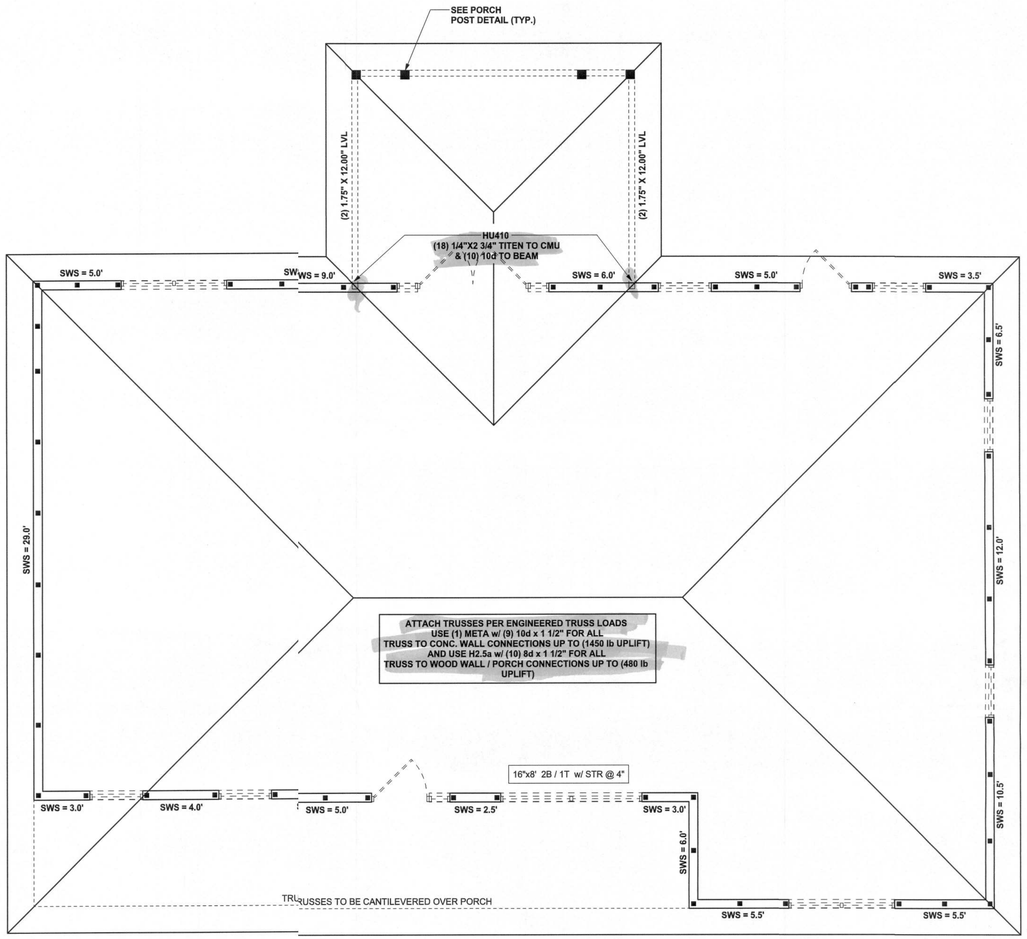
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			**TO FOUNDATION**
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			**TO STUDS**
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	**TO STUDS**		**TO FOUNDATION**
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HDZA	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

CONCRETE TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2x12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 ALL LOAD BEARING CONC. LINTELS SHALL BE A MIN. OF 16" DEEP w/ (1) #4 TOP AND BOTTOM NO STIRRUPS REQUIRED (U.N.O.)

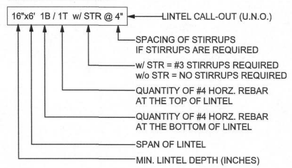
SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	POURED CONC. WALL
	INTERIOR BEARING WALL
	PORCH POST / PORCH HEADER BEAM

LINTEL LEGEND



TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	24.0'	64.0'
LONGITUDINAL	20.0'	57.0'

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Discoway, PE No.53915, POB 68, Lake City, FL 32056, 386-754-541

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer a questions to Mark Discoway, P.E for resolution. Do not proceed without clarification.

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CERTIFICATION: I ereby certify that I have examined this plan, nd that the applicable portions of the plan, elating to wind engineering comply with section:I301.2.1, florida building code residential 200, to the best of my knowledge.

LIMITATION: This dsign is valid for one building, at specifiedlocation.

MARK DISCOWAY P.E. 53915 LICENSED STATE OF FLORIDA PROFESSIONAL ENGINEER
November 11, 2010

Rosenboom Construction

JenkinsResidence

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PRINTED DATE: November 11, 2010

DRAWN BY: | STRUCTURAL BY: David Discoway

FINALS DATE: 11Nov10

JOB NUMBER: 10I1027

DRAWIIG NUMBER

S-3

OF : SHEETS