



SOUTH ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



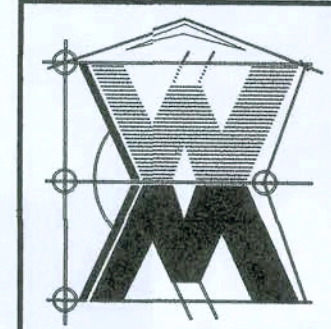
REVISIONS
May 04, 2009



FRONT & REAR ELEVATIONS
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:
ANTHONY & NICOLE SKOWRON
PROJECT ADDRESS: EAST 2.5 ACRES OF LOT 10, ROSE CREEK PLANTATION, COLUMBIA COUNTY, FL

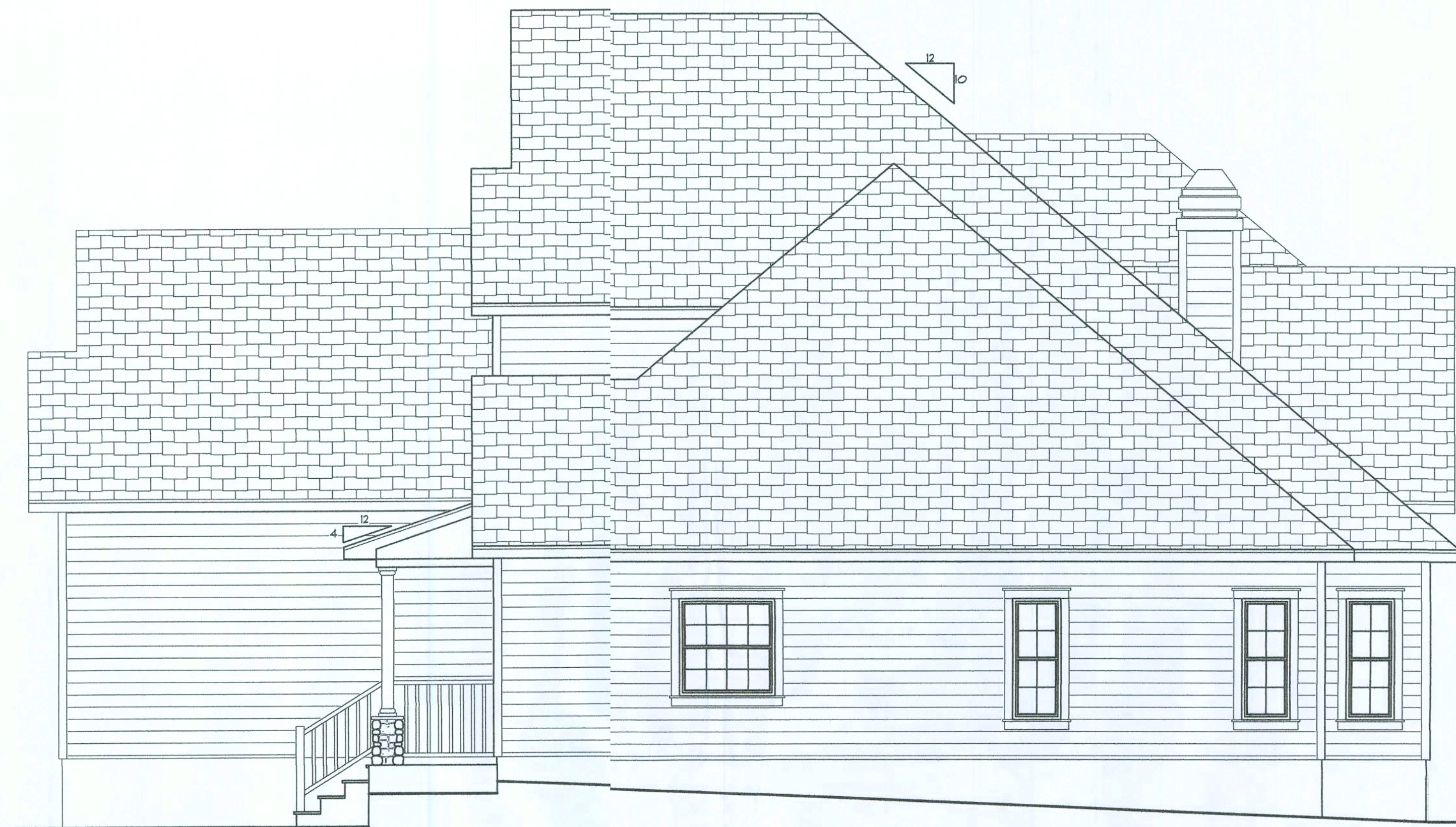
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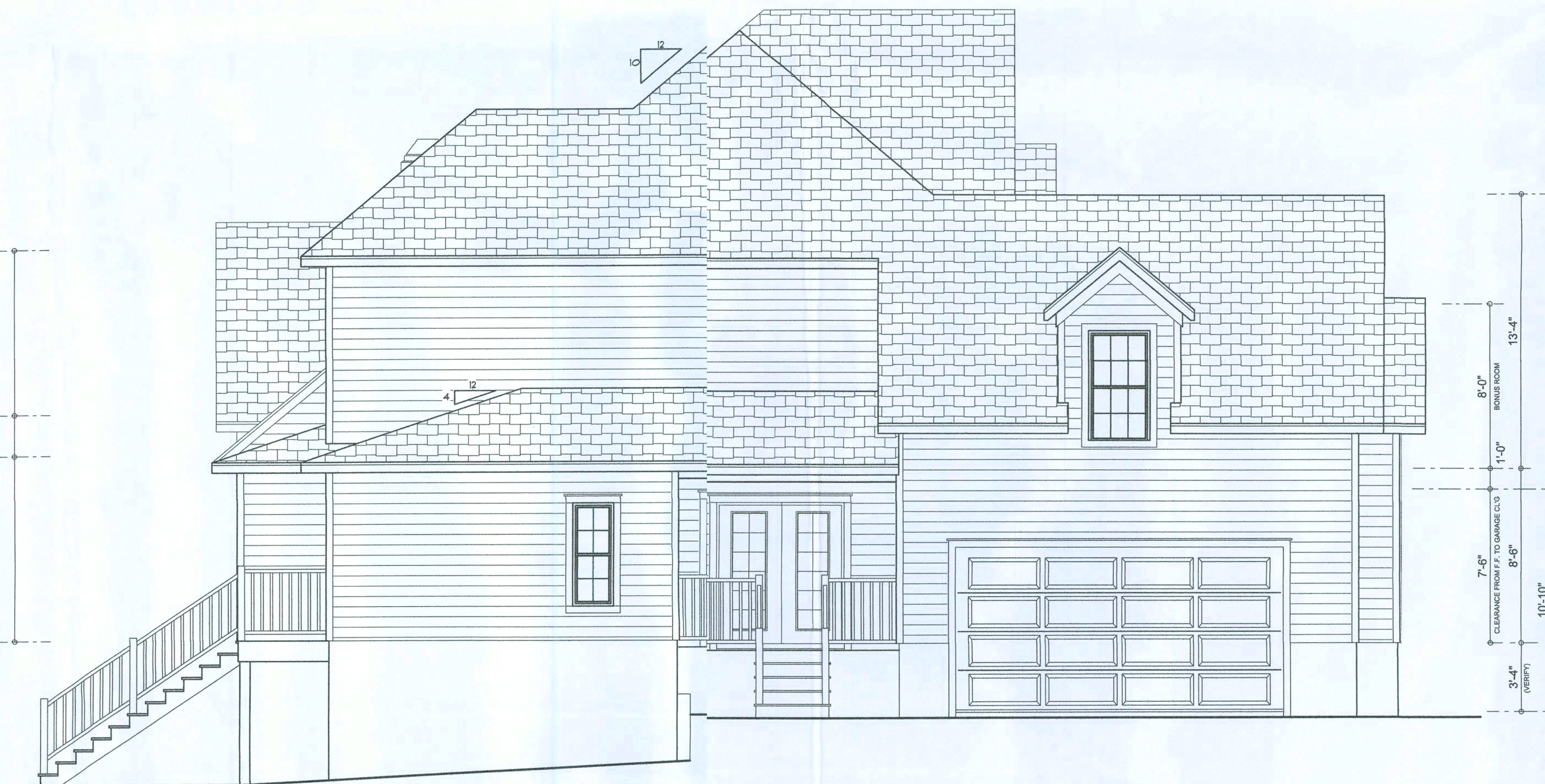
JOB #NUMBER
090307

SHEET NUMBER
A.1
OF 4 SHEETS

Will C. Myers



WEST ELEVATION
SCALE: 1/4" = 1'-0"



EAST ELEVATION
SCALE: 1/4" = 1'-0"

REVISIONS
May 04, 2009

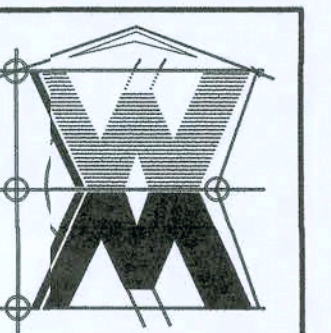
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

LEFT & RIGHT ELEVATIONS
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:
**ANTHONY & NICOLE
SKOWRON**

PROJECT ADDRESS: EAST 2.5 ACRES OF LOT 10, ROSE CREEK PLANTATION, COLUMBIA COUNTY, FL

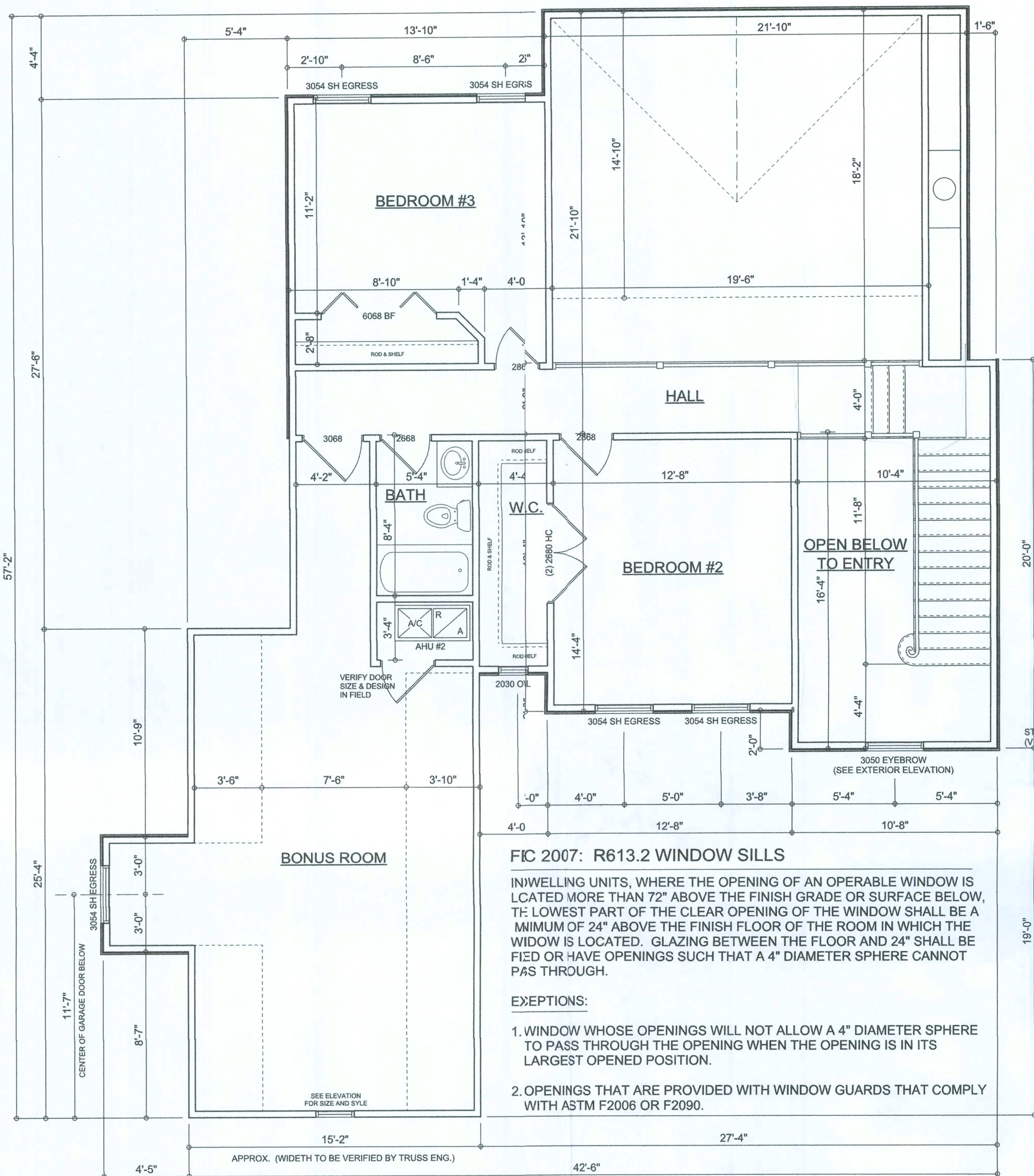
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SHEET NUMBER
A.2
OF 4 SHEETS

William C. Myers



SECOND FLOOR PLAN

SCALE: 1/4" = 1'-0"

HANDRAIL @ 34" ABOVE THE PLANE OF NOSING

LINE OF APPLIED FINISH

18R @ 1.33" = 11'-0" TOTAL RISE, ITT AT 10" = 170" TOTAL RUN

TYP. STAIR DETAIL

SCALE: 3/4" = 1'-0"

TRIPLE P/T 2X10 CARRIAGE
CONT. P/T 2X4 KICK PLATE
EXTENDED CARRIAGE ALANDINGS

AREA SUMMARY

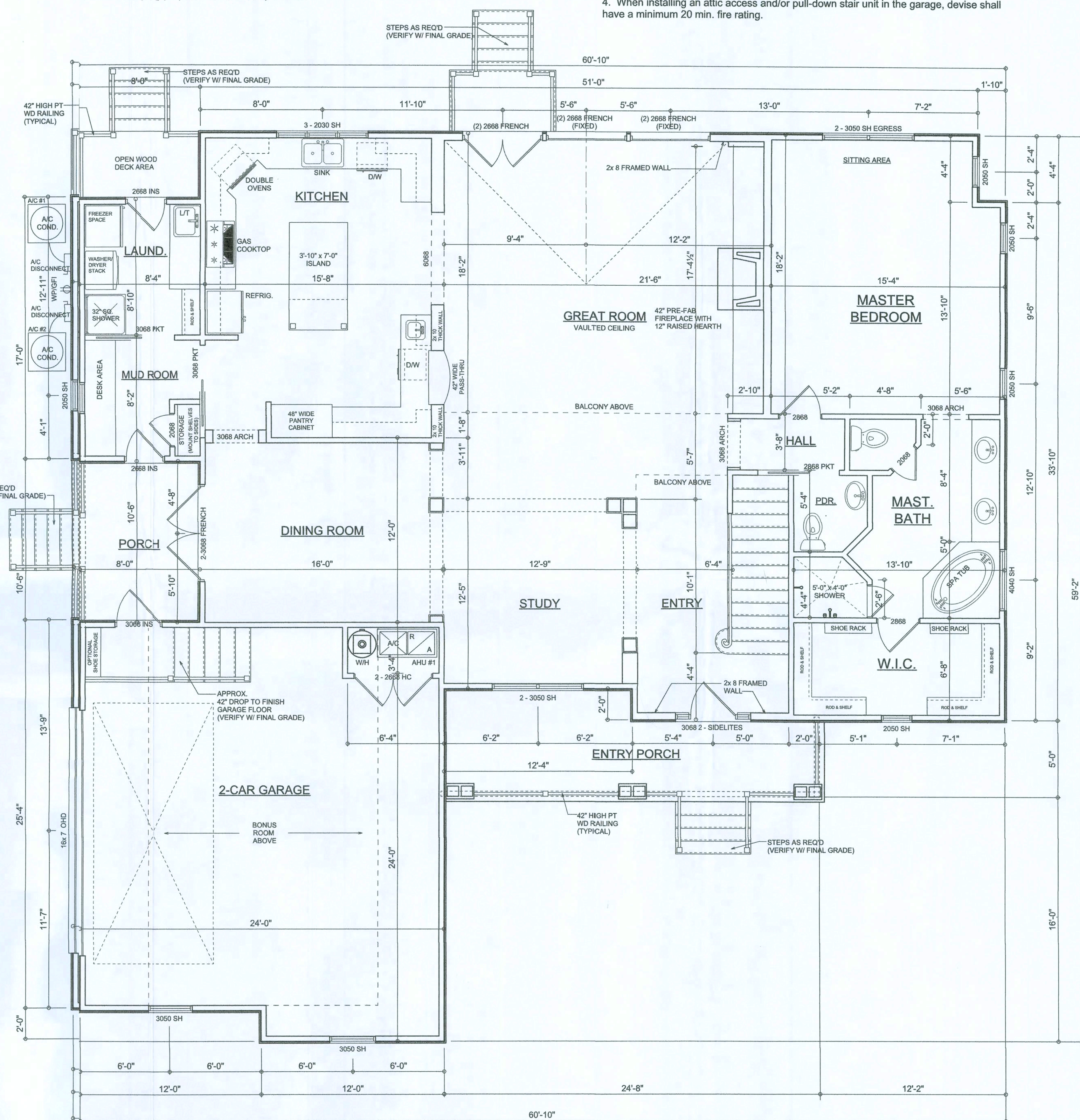
LIVING AREA	2023	S.F.
SECOND FLOOR LIVING	597	S.F.
TOTAL LIVING AREA	2620	S.F.
BONUS ROOM AREA	457	S.F.
GARAGE AREA	623	S.F.
ENTRY PORCH AREA	146	S.F.
SIDE PORCH AREA	83	S.F.
COVERED PORCH AREA	87	S.F.
TOTAL AREA	4016	S.F.

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.

2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
 3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
 4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.



FIRST FLOOR PLAN

SCALE: 1/4" = 1'-0"

REVISIONS	DATE
May 19, 2009	

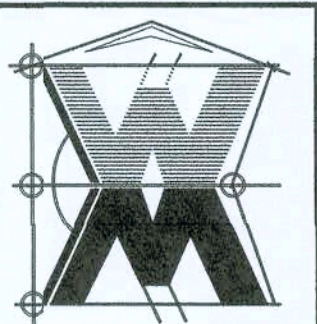
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

MAIN FLOOR PLAN
SCALE: 1/4" = 1'-0"

SECOND FLOOR PLAN
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:
ANTHONY & NICOLE SKOWRON
PROJECT ADDRESS: EAST 2.5 ACRES OF LOT 10, ROSE CREEK PLANTATION, COLUMBIA COUNTY, FL

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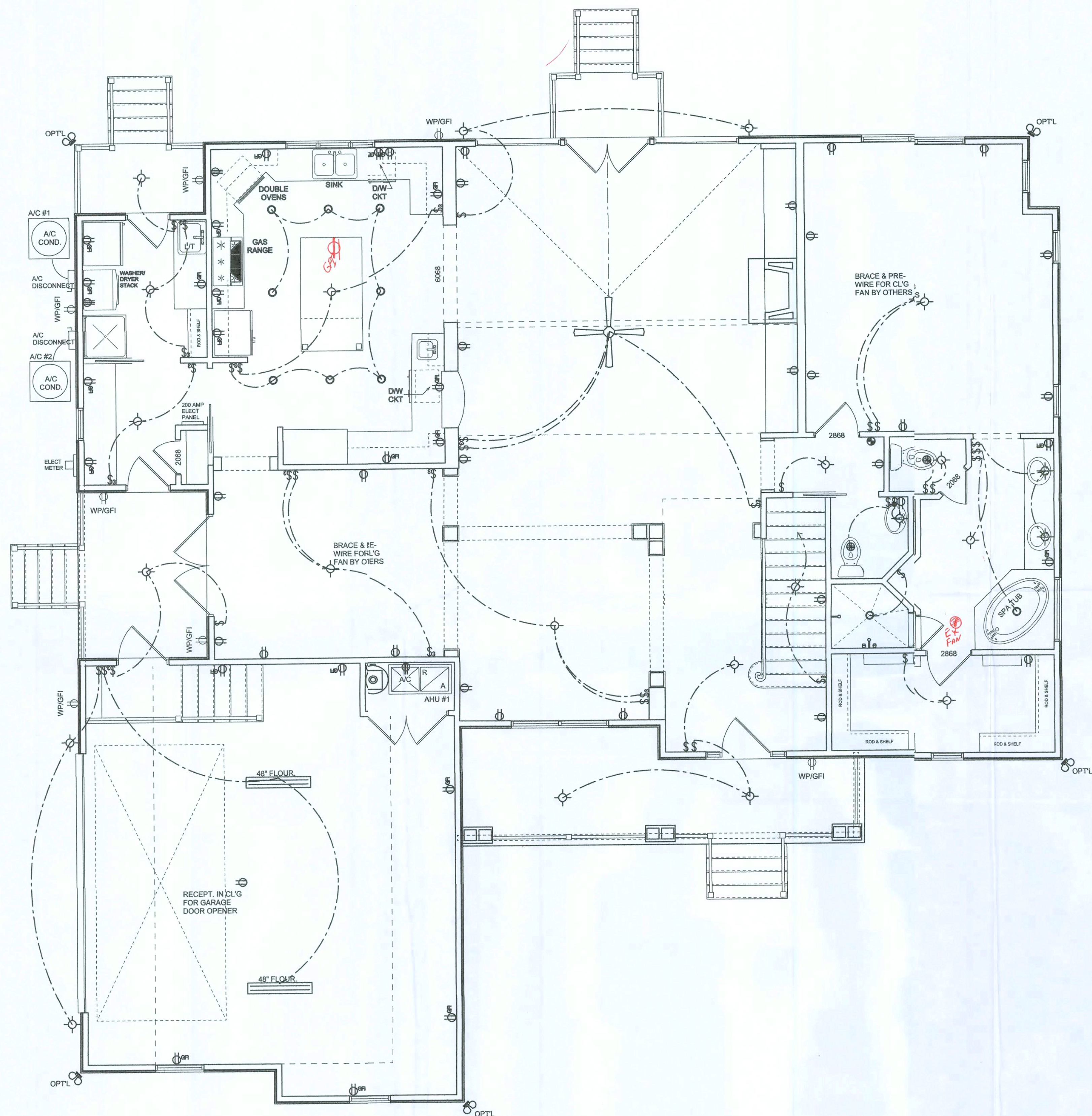


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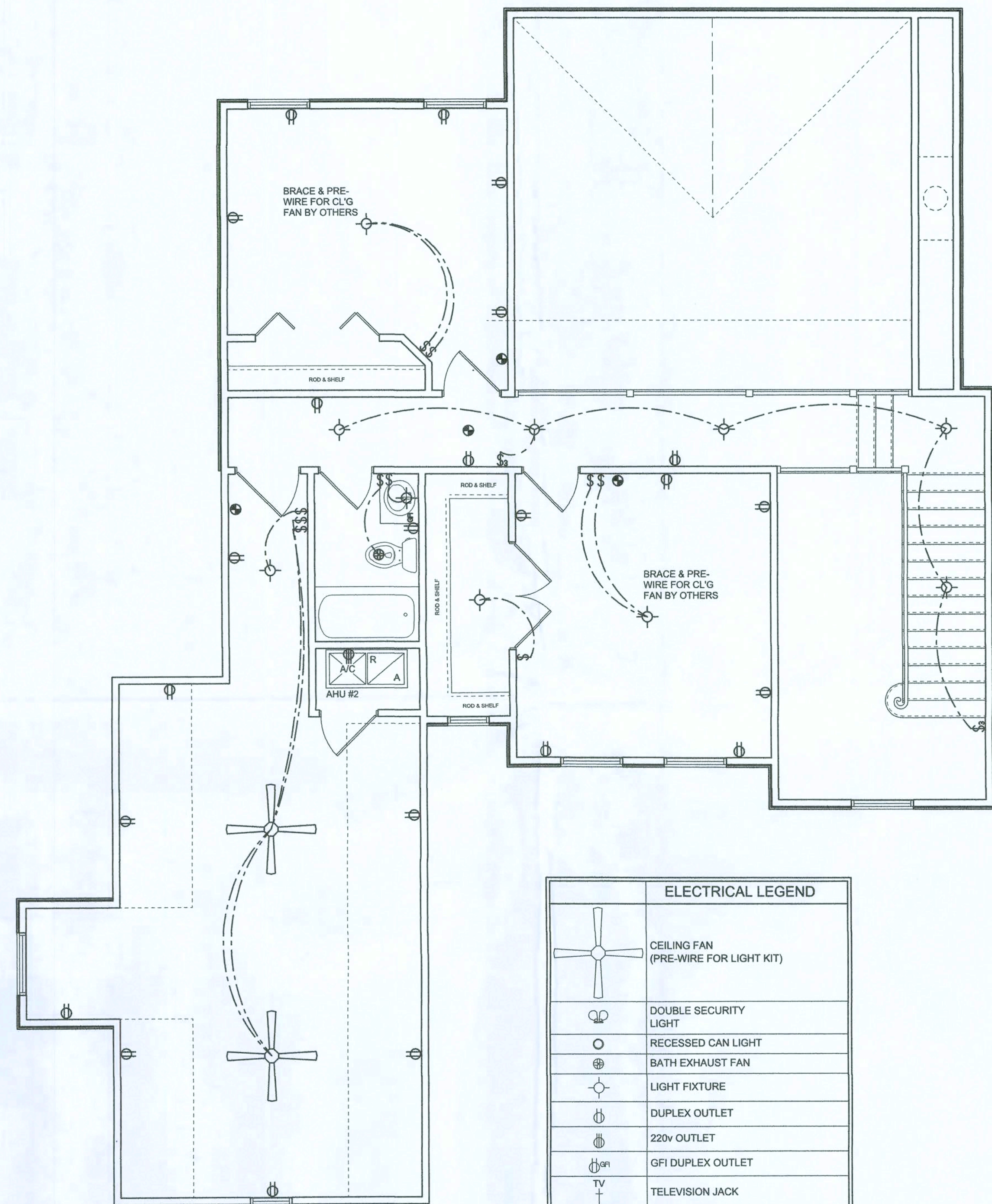
SHEET NUMBER

A.3
OF 4 SHEETS

William Myers



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"



2ND FLOOR ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:
ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTORS SHALL HAVE BATTERY BACKUP POWER AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

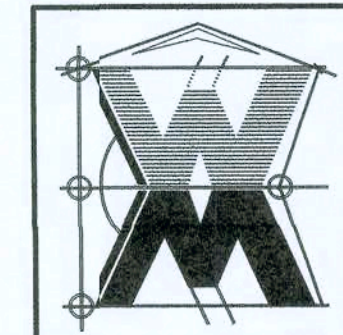
REVISIONS
May 04, 2003

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"
2ND FLOOR ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

A CUSTOM RESIDENCE FOR:
ANTHONY & NICOLE SKOWRON
PROJECT ADDRESS: EAST 2.5 ACRES OF LOT 10, ROSE CREEK PLANTATION, COLUMBIA COUNTY, FL

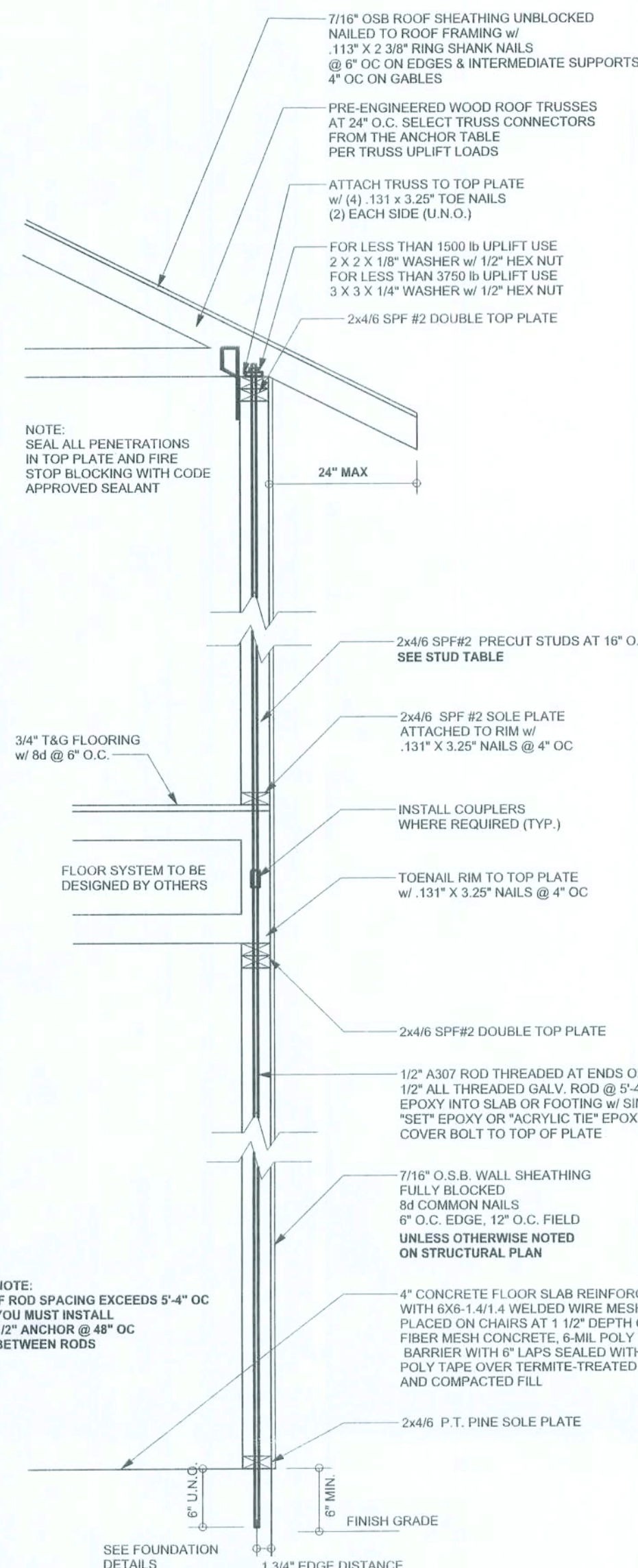
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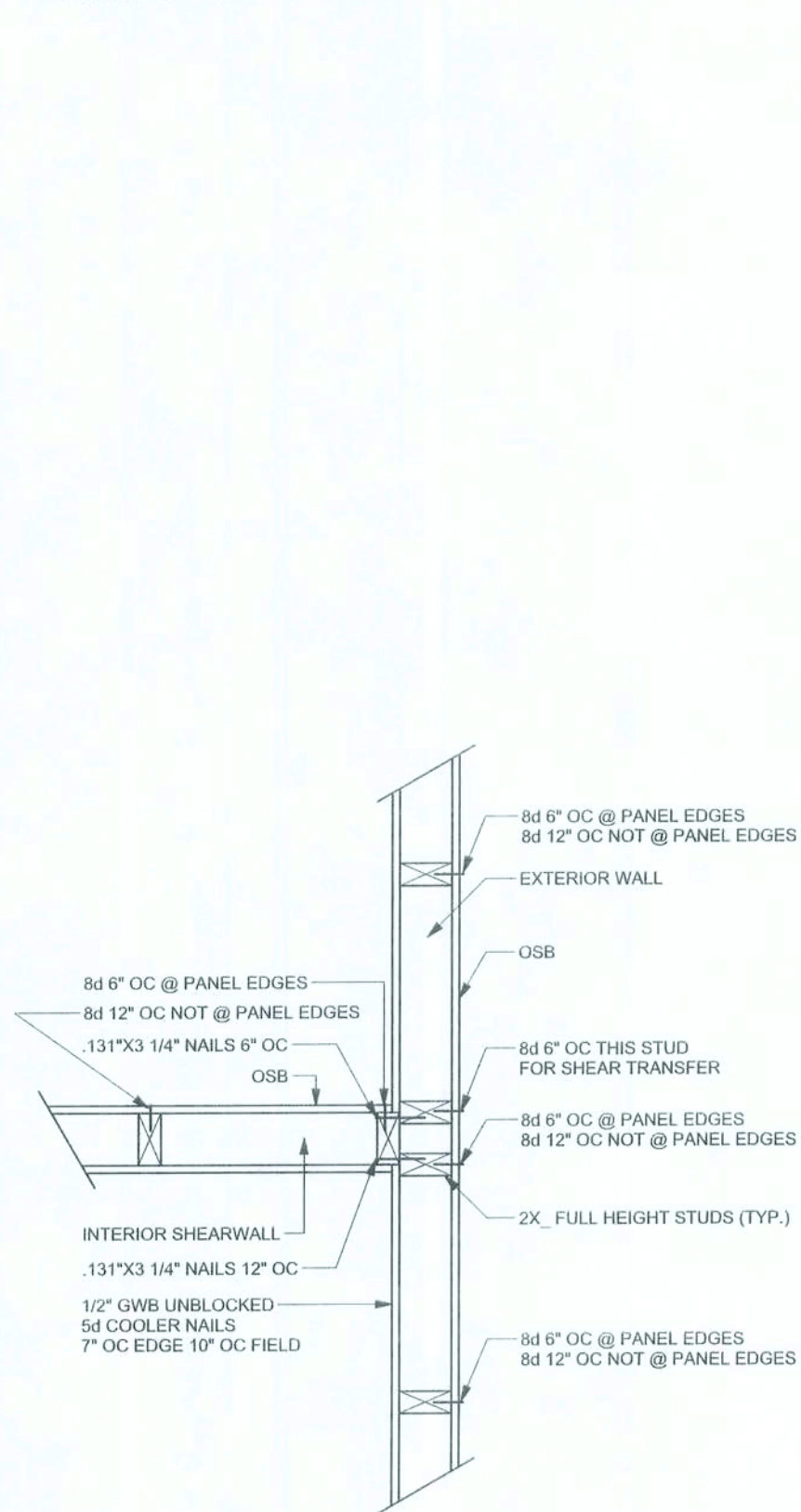
SHEET NUMBER
A.4
OF 4 SHEETS

W.M.C. Myers



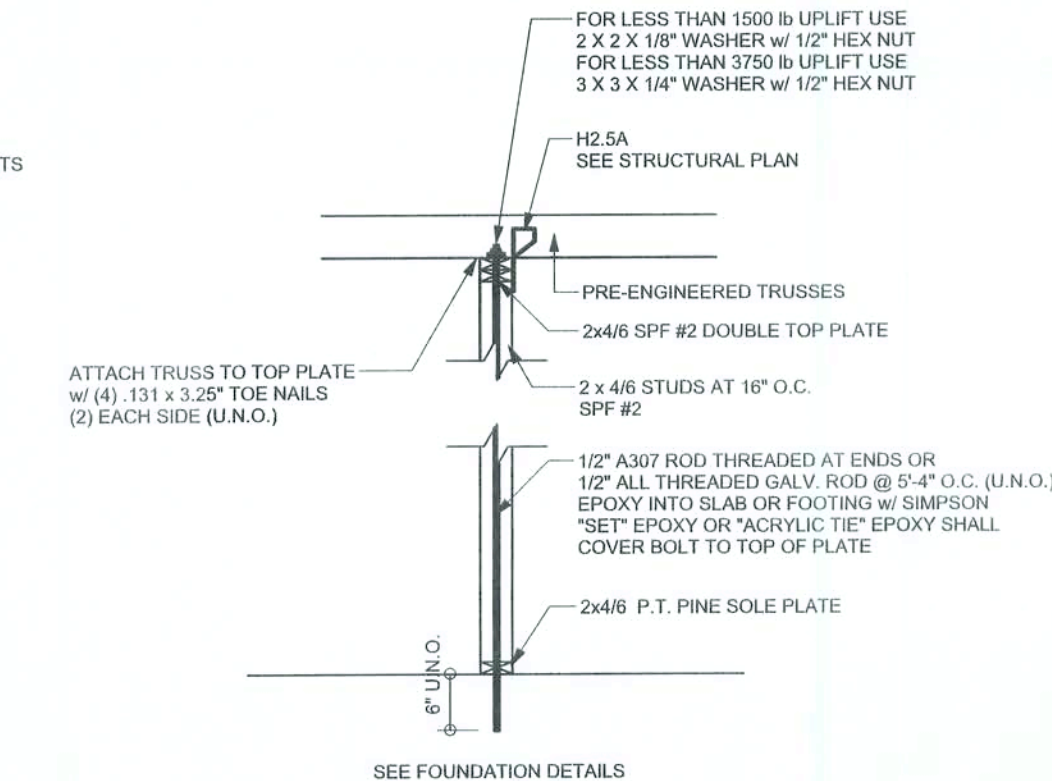
ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

TWO STORY WALL SECTION
SCALE: 3/4" = 1'-0"

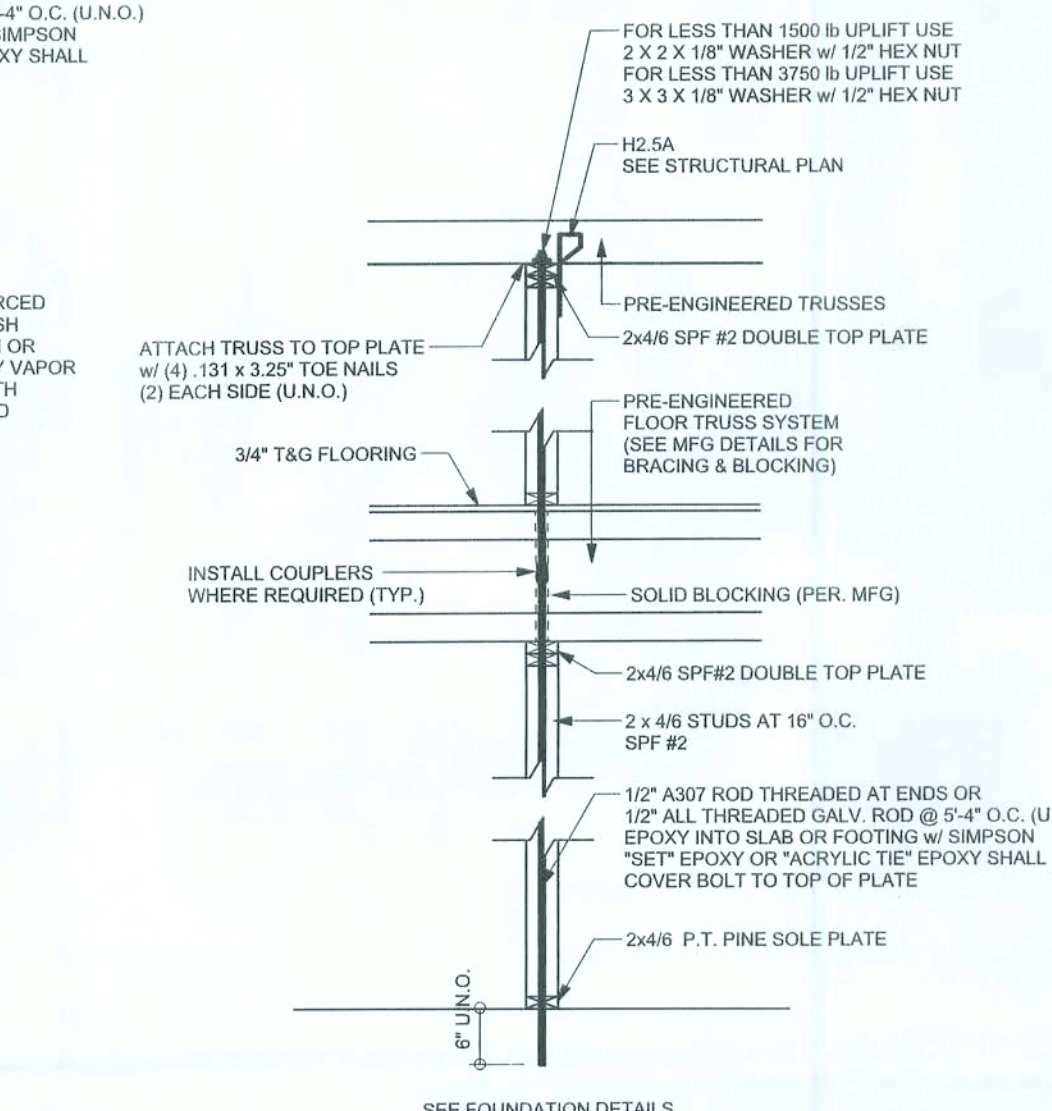


(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

(TYP.) CORNER FRAMING
WOOD FRAME



INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"



TWO STORY INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS		
RIDGE BOARD	2x6 SYP #2	2x4 SYP #2
RAFTER SPANS 20'-0" OR LESS	2x4 SYP #2	2x4 SPF #2
PURLINS / LATERAL BRACING	2x4 SYP #2	2x4 SPF #2
SLEEPERS	2x (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2x4 SYP #3	
CRIPPLES & BLOCKING	2x4 SYP #2 OR BETTER	
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL	

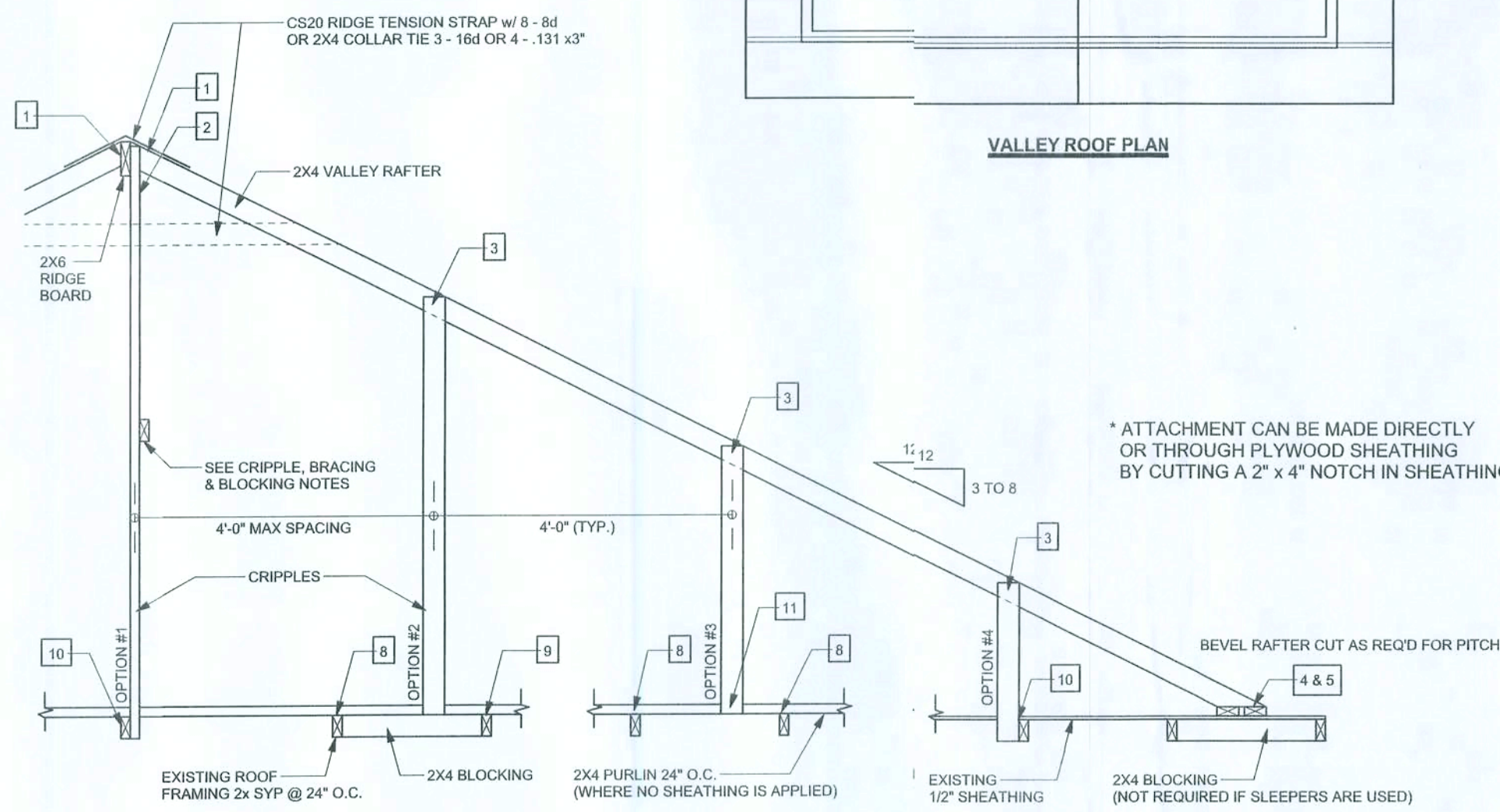
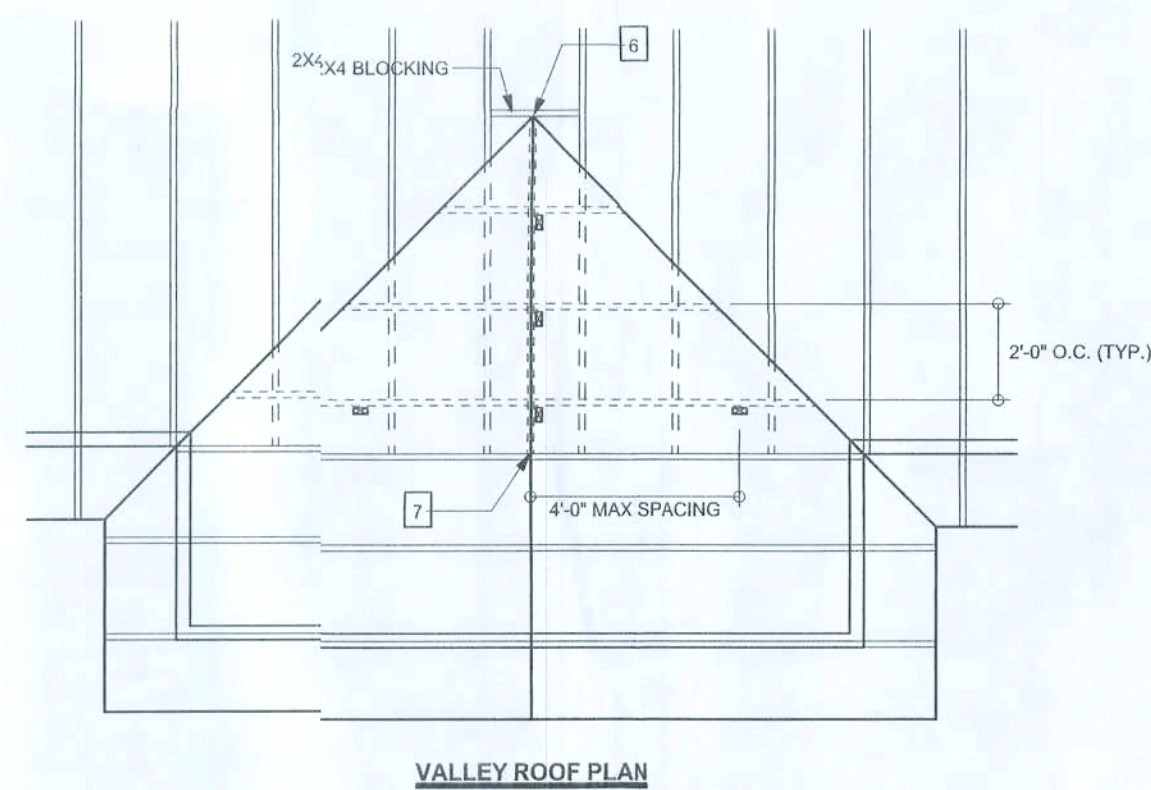
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-1" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-1" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. x 0.85 = 13'-6" O.C.



SECTION CUT PARALLEL TO VALLEY RAFTER

RETROFIT ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 255	H5	4-8d	4-8d	
< 360	< 265	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 800	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14 - 16d	14 - 16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MG1		22 - 10d	1-58" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-58" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-58" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-58" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4		6 - 10d, 1 1/2"	
< 1240	< 1065	SP4H		10 - 10d, 1 1/2"	
< 885	< 760	SP6		6 - 10d, 1 1/2"	
< 1240	< 1065	SP6H		10 - 10d, 1 1/2"	
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	16 - 8d		
< 1705	< 1705	CS16	28 - 8d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	8 - 16d		1/2" AB
< 2310	< 2310	LTT31	18 - 10d, 1 1/2"		1/2" AB
< 2775	< 2570	HDTA	2 - 58" BOLTS		5/8" AB
< 4175	< 3695	HT16	18 - 10d		5/8" AB
< 1400	< 1400	PAH42	16 - 16d		
< 3335	< 3335	HPAH22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		1/2" AB
< 2300	< 2300	ABU68	12 - 16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2 - 58" AB

VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TY) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

1 2X4 RAFTERS TO RIDGE	3 - 16d OR 6 - 131 x 3" TOE NAILS
2 CRIPPLE TO RIDGE	3 - 16d OR 6 - 131 x 3" FACE NAILS
3 CRIPPLE TO RAFTERS	3 - 16d OR 6 - 131 x 3" FACE NAILS
4 RAFTER TO SLEEPER OR BLOCKING	6 - 16d OR 12 - 131 x 3" TOE NAILS
5 SLEEPER TO TRUSS	4 - 16d OR 8 - 131 x 3" FACE NAILS EACH TRUSS
6 RIDGE BOARD TO ROOF BLOCK	3 - 16d OR 6 - 131 x 3" TOE NAILS
7 RIDGE BOARD TO TRUSS	3 - 16d OR 6 - 131 x 3" TOE NAILS
8 PURLIN TO TRUSS (TYP.)	4 - 16d OR 8 - 131 x 3" NAILS
9 TRUSS TO BLOCKING	3 - 16d OR 6 - 131 x 3" END NAILS
10 CRIPPLE TO TRUSS	3 - 16d OR 6 - 131 x 3" FACE NAILS
11 CRIPPLE TO PURLIN	3 - 16d OR 6 - 131 x 3" FACE NAILS

GENERAL NOTES

- MAXIMUM RAFTER SPANS: 6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2
- MAXIMUM ROOF AREA PER SUPPORT: 1002 IN ZONES 2 & 3, 2482 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1602 OR 2'-0" X 8'-0" SPAN = 1602)
- PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
- PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.
- THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
 - SPANS (DISTANCE BETWEEN HEELS) 40'-0" OR LESS
 - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
 - MAXIMUM WIND SPEED: 120 MPH
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
 - MAXIMUM TOTAL LOADING: 40 psf
 - MEETS REC 2007/ASCE 7-98 WIND REQUIREMENTS
 - EXPOSURE CATEGORY "B", I = 1.0, Kd = 1.0
 - ENCLOSED BUILDING

CRIPPLE, BRACING & BLOCKING NOTES

- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED w/ 2 - 10d NAILS OR 2X4 "T" OR SCAB NAIL TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 6" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBs OR BOTH FACES w/ "T" OR SCAB USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBRC 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGN IS FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND REACTION LOADS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 4 TIMES EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, P_c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14 FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWY CONTROL JOINTS IN SLAB ON GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40" DB (20" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

GULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4MI, E = 1800ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS, MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT EXHAUSTIVE. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 16" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 1/4", WITH 5/8" BOLTS TO BE 3" x 3" x 1/4", WITH 3/4" BOLTS TO BE 3" x 3" x 1/4", WITH 7/8" BOLTS TO BE 3" x 3" x 1/4", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBRC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBRC 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE TRUSS OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS FOR TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBRC 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS AND REACTION LOADS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBRC 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS NOT RESPONSIBLE FOR THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

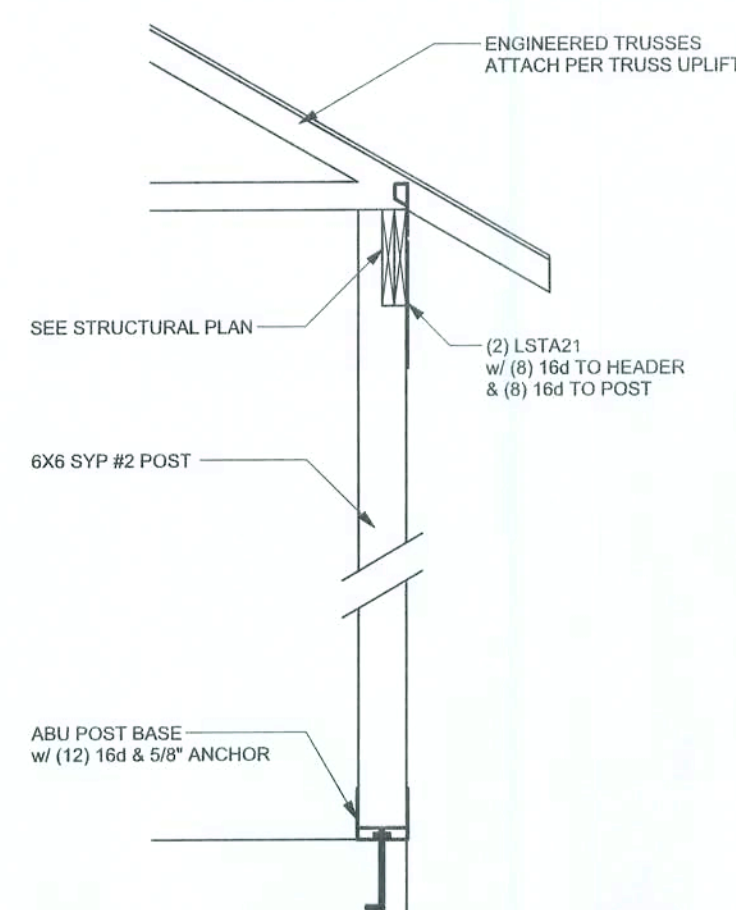
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS. MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 6 FT. NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 50 FT IN EXP. C AND <10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50' HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

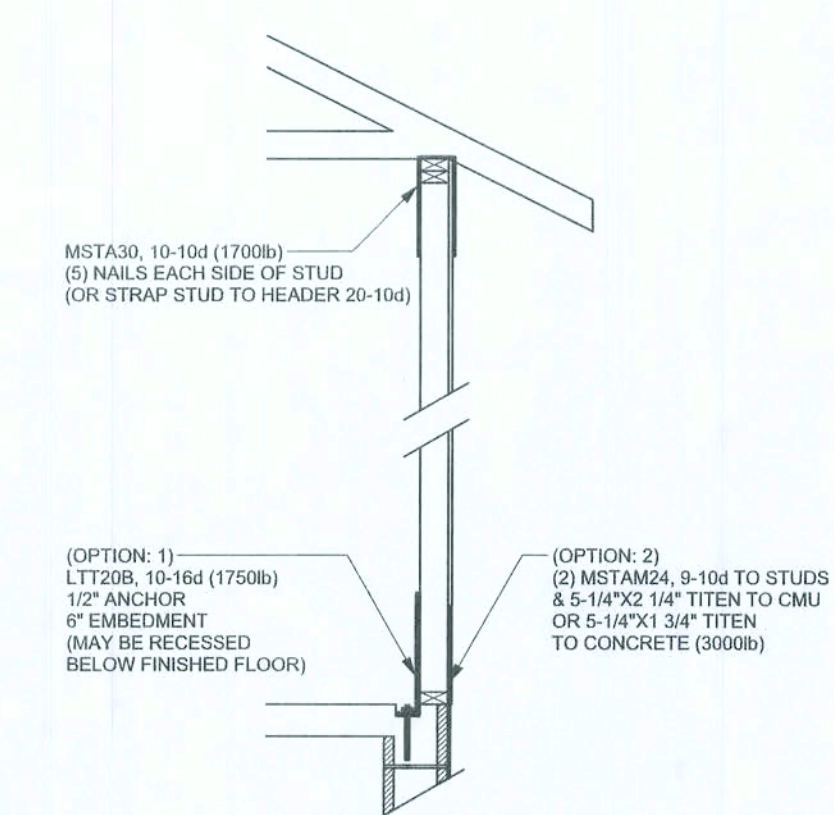
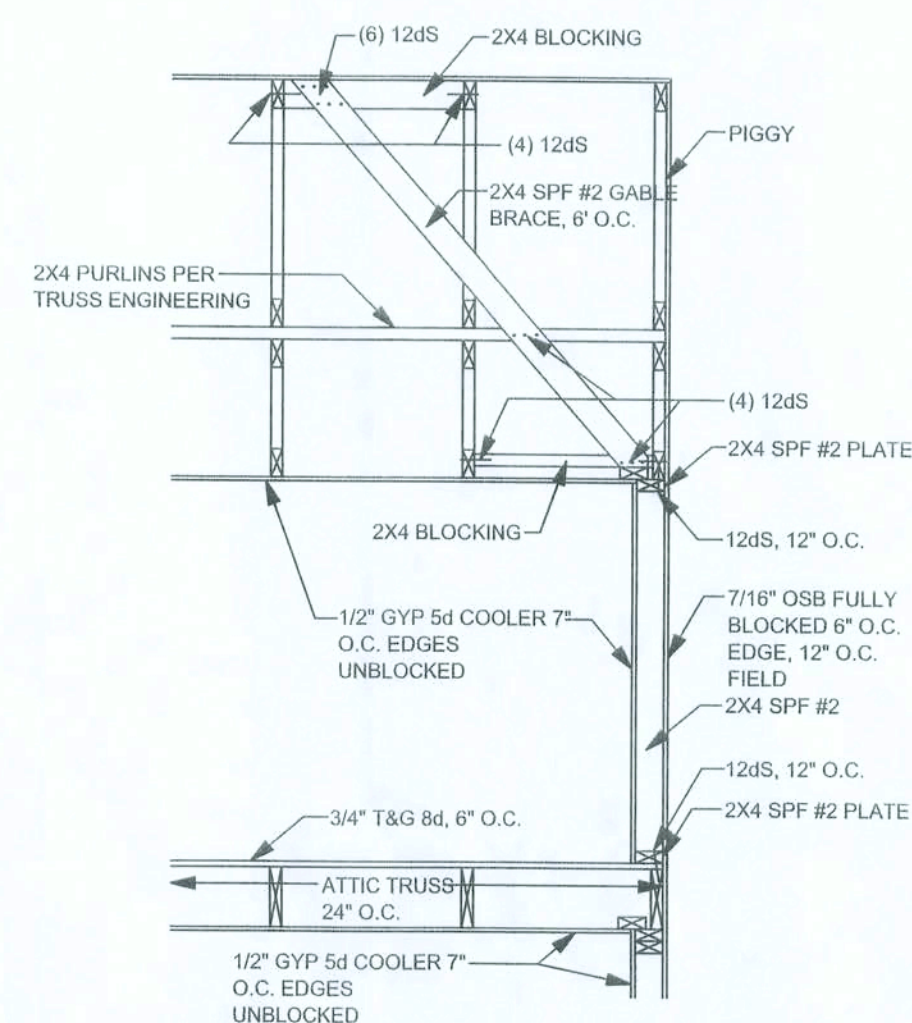
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = C
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = NA (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	10	20	30
1	27.8 - 30.5	25.3	25.3	
2	27.8 - 35.7	25.3	30.5	
2 Ohg	56.8		56.8	

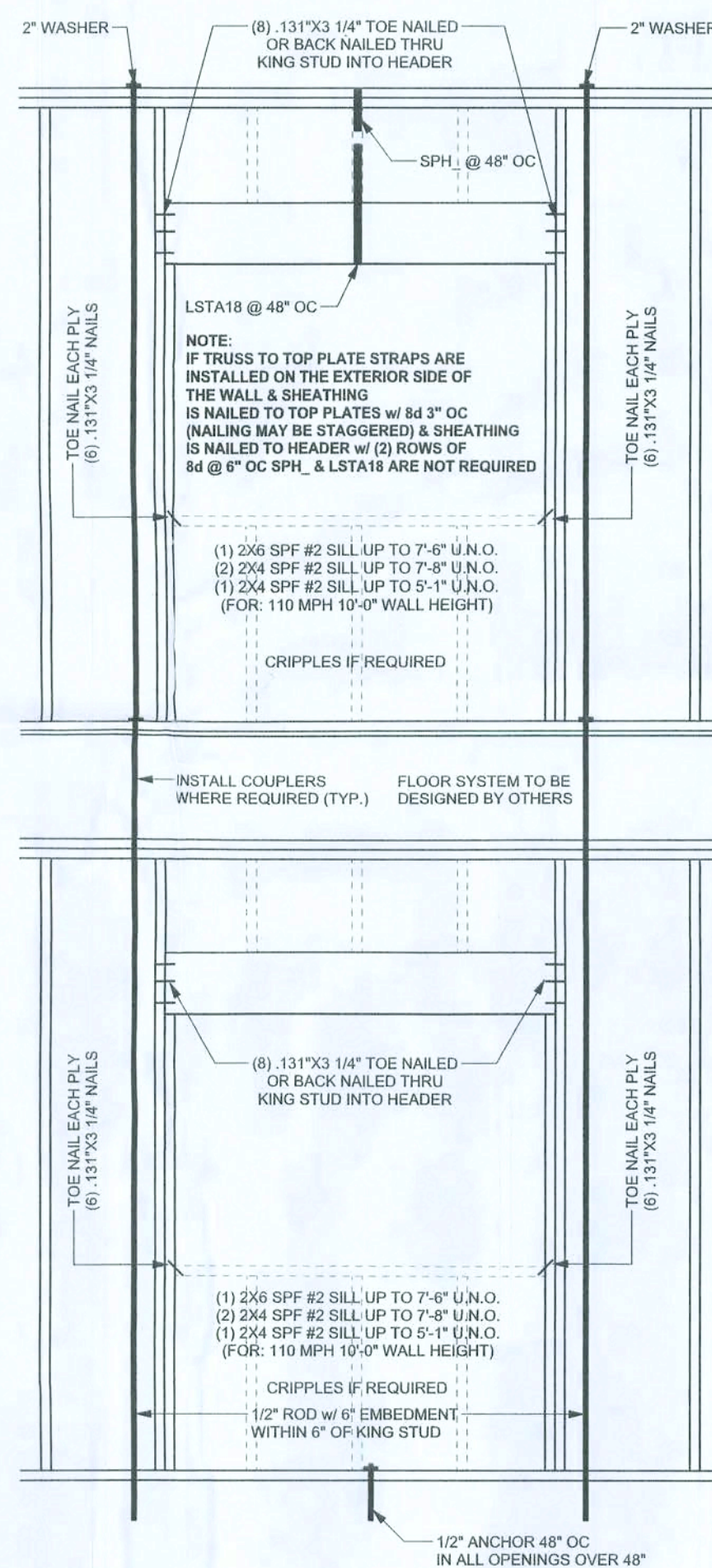


(TYP.) PORCH POST
ONE STORY WOOD

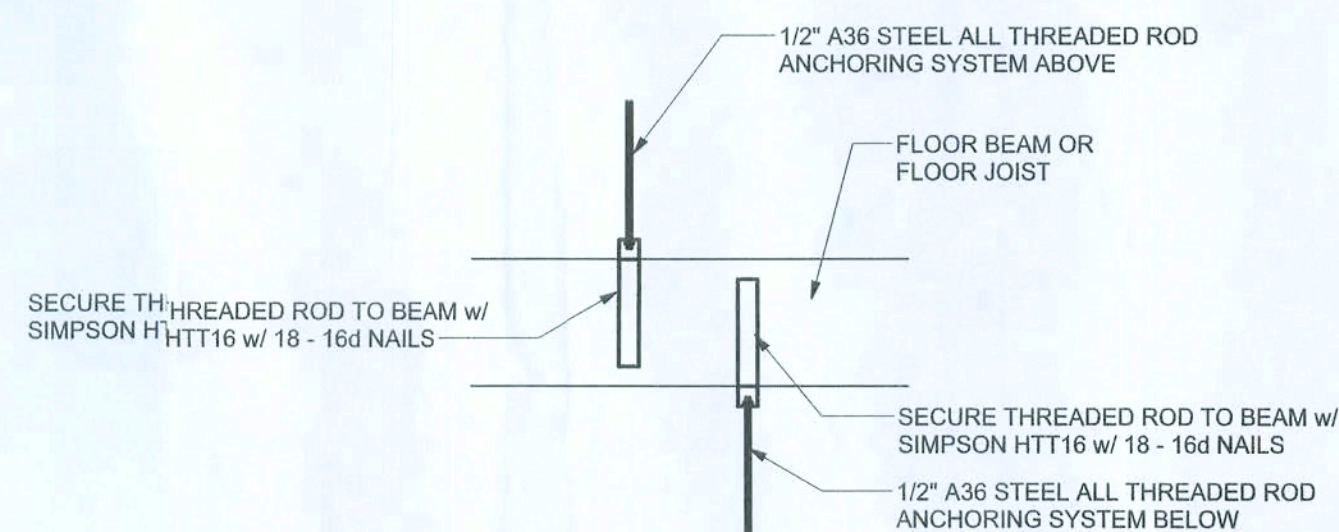
ALLOWABLE UPLIFT
1900 LB

W67 - BONUS ROOM / GABLE END BRACING

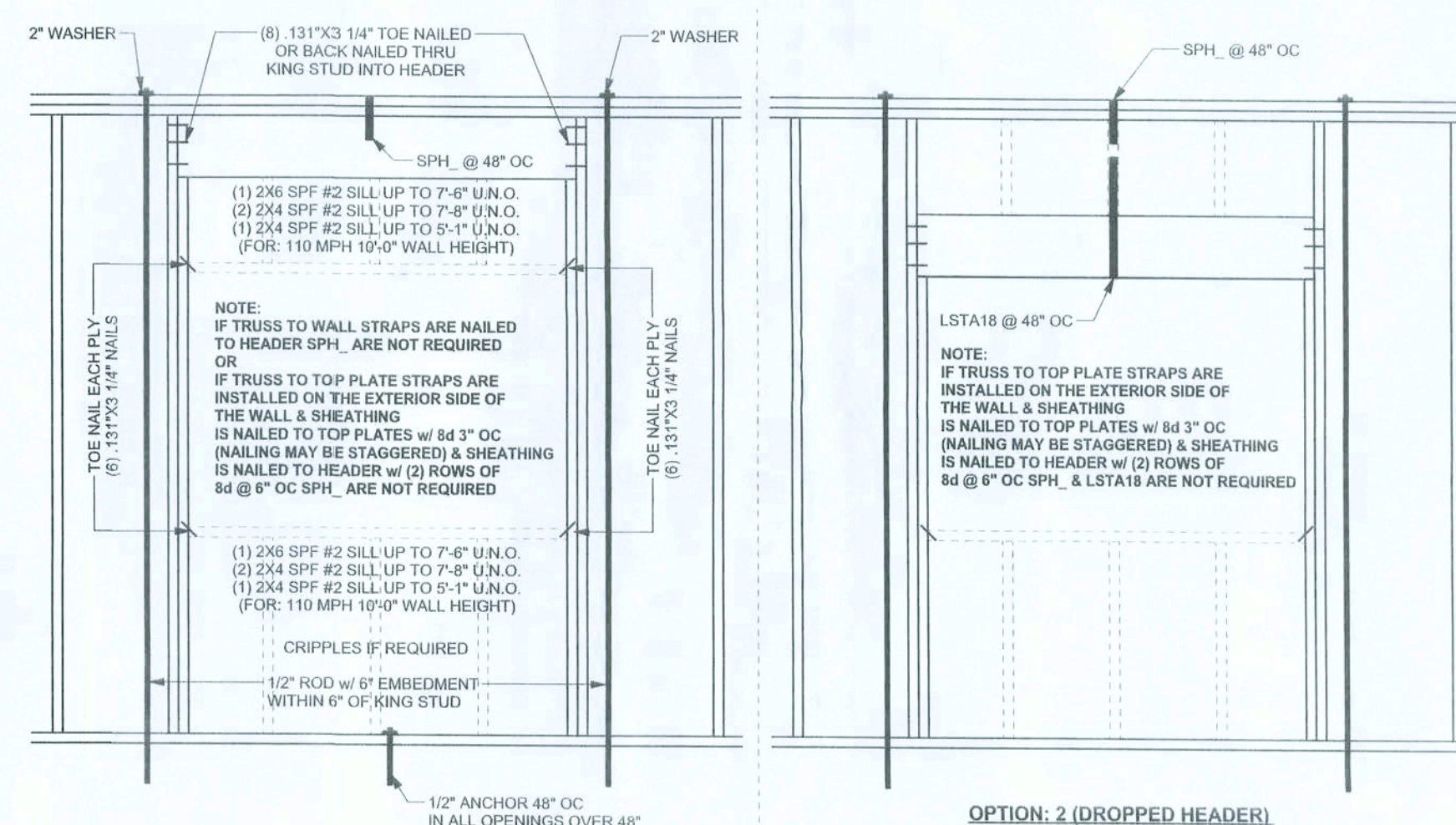
DOOR WIDTH	3/8"x4" LAG	16d STAGGER	(2) ROWS OF .131"x3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC
20'	12" OC	2.5" OC	2.5" OC



(TYP.) HEADER
TWO STORY WOOD FRAME w/ RODS



**OPTIONAL THREADED ROD TO
FLOOR BEAM OR FLOOR JOIST**
SCALE: 1/2" = 1'-0"



(TYP.) HEADER OPTION: 1 (FLUSH HEADER)
ONE STORY WOOD FRAME w/ RODS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

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P.E. 53915

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15MXX09
SFA

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PRINTED DATE: May 15, 2009	
DRAWN BY:	STRUCTURAL David Disosway

FINALS DATE:	
15May09	

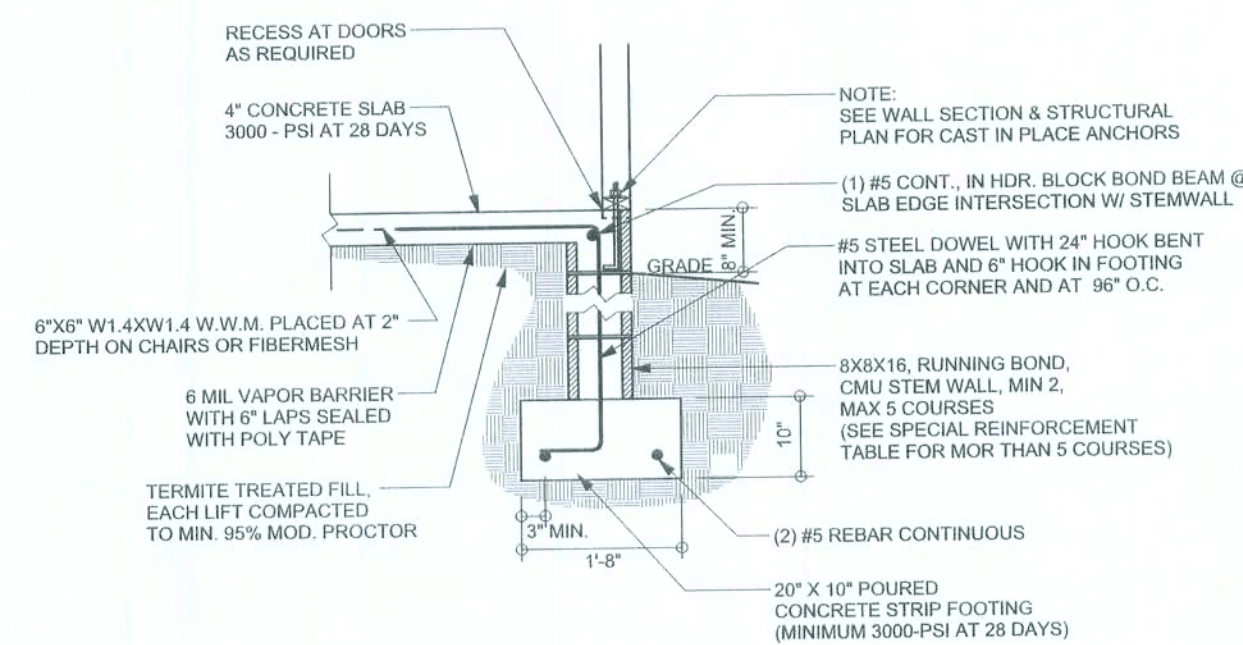
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904291

DRAINING NUMBER

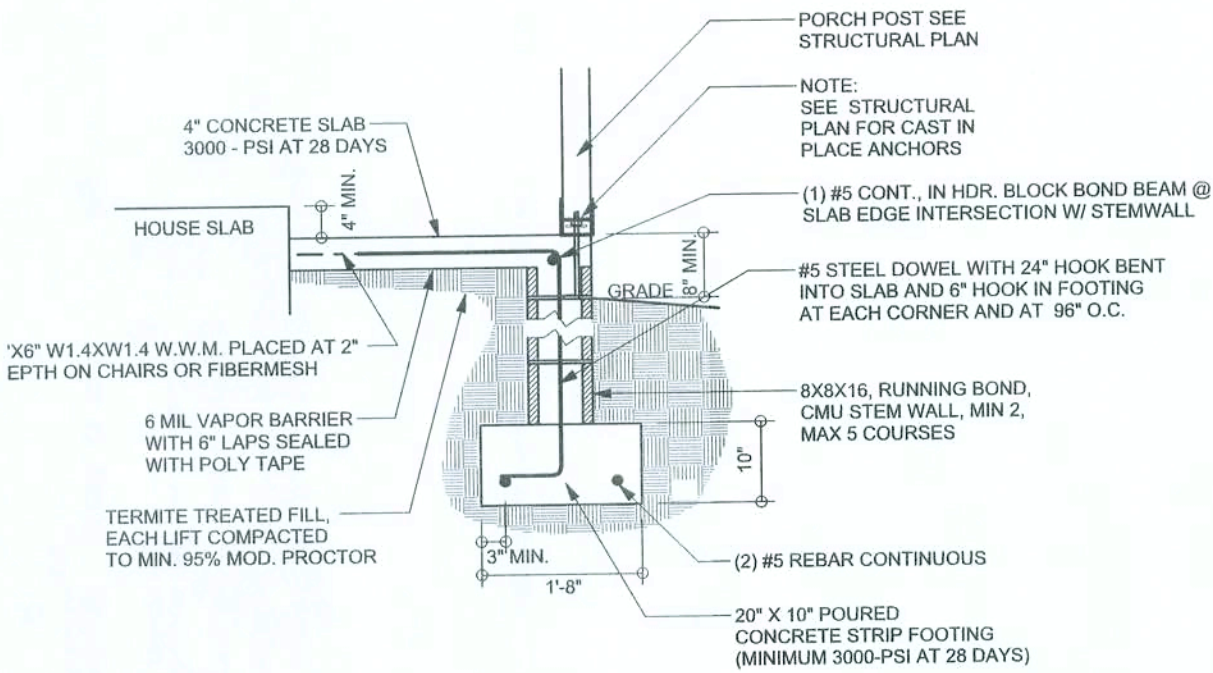
S-1.1
OF 4 SHEETS

REVISIONS	

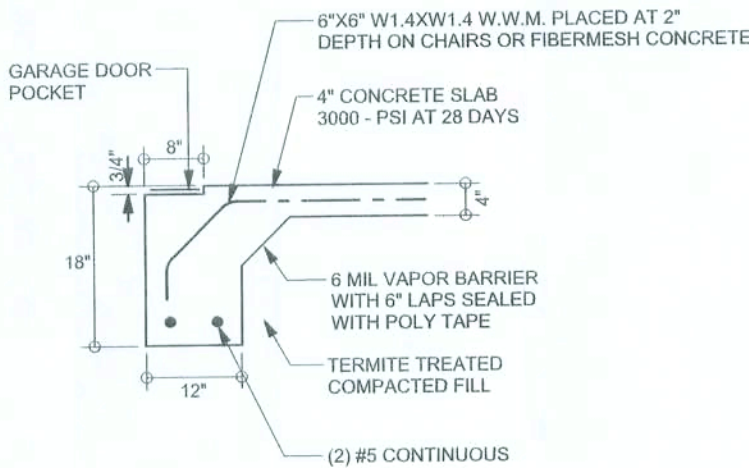
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



F9 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F12 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

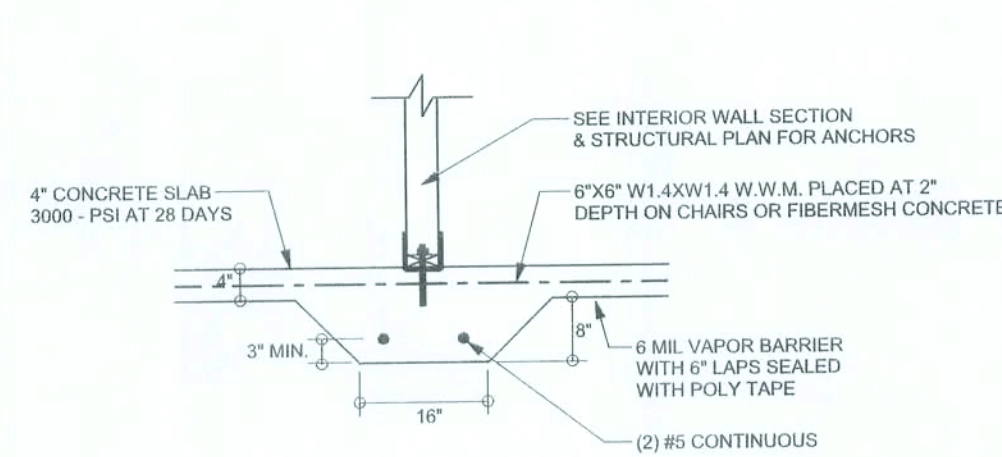


F4 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

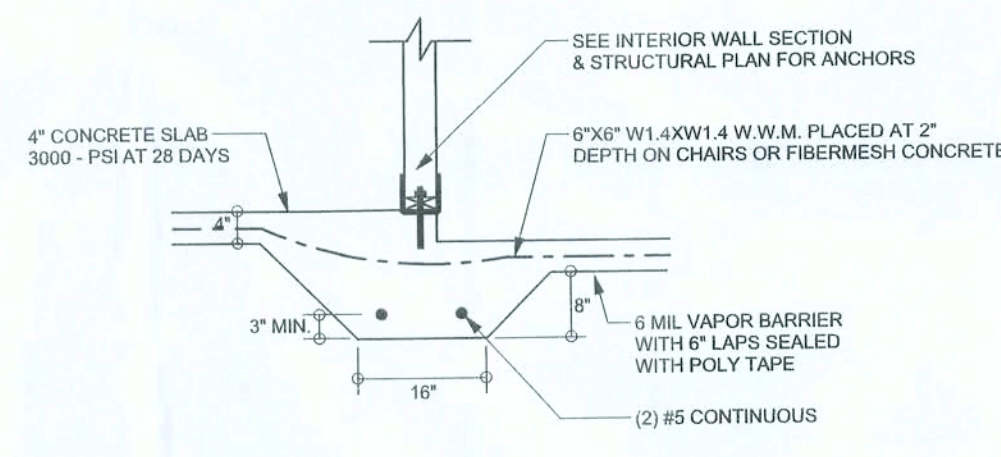
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 166 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

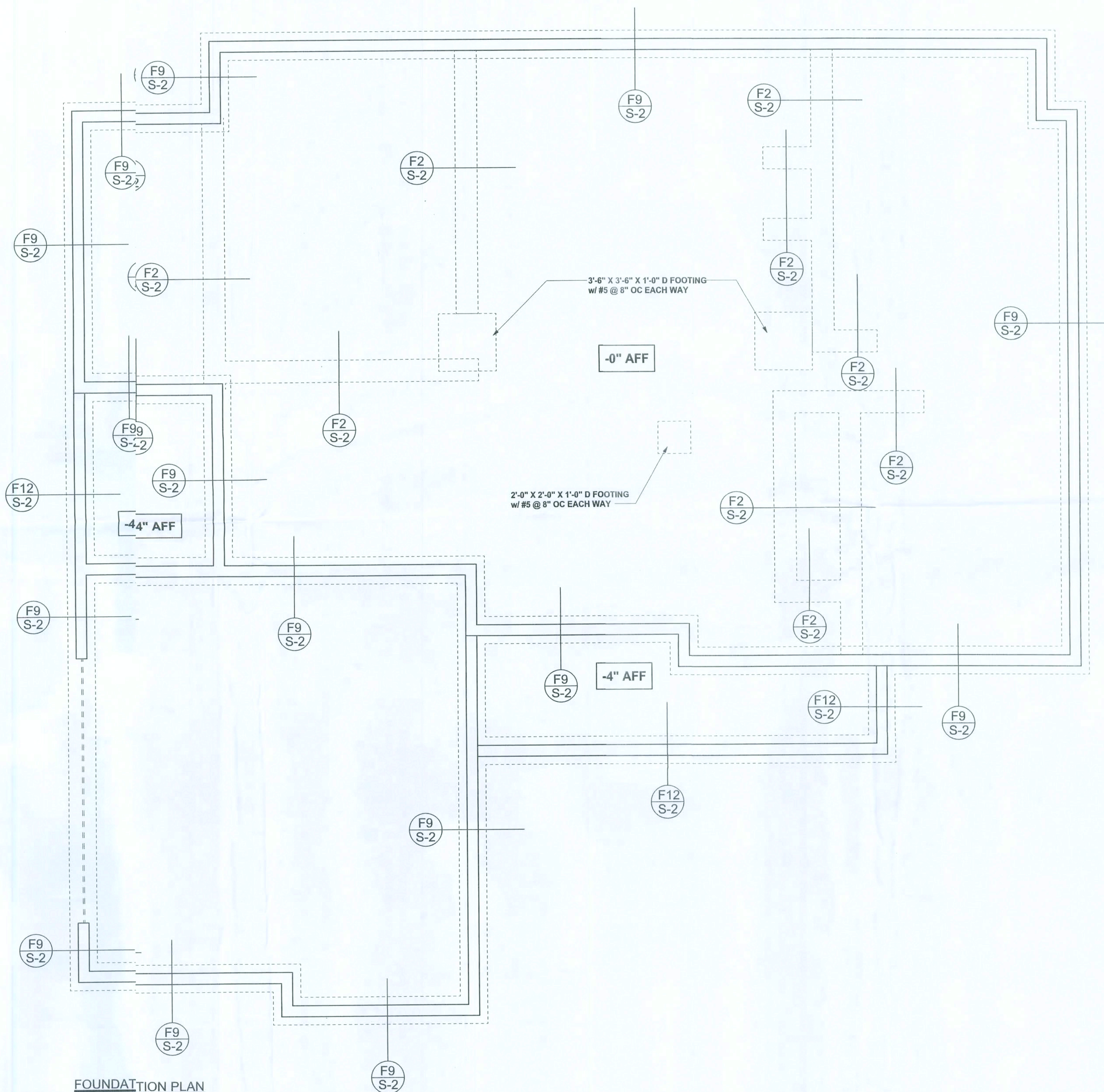
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS OF STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAI ENGINEER:
Mark Disoway, P.E.
No. 53915, KRS 868, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section RS01.2.1, Florida building code residential 507, to the best of my knowledge.

LIMITATION: This design is valid for one building, as specified location.

MARK DISOWAY
P.E. 53915

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15 MAY 09
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May 15, 2009

DRAWN BY: STRUCTURAL BY:
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FINAL DATE:
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JOB NUMBER:
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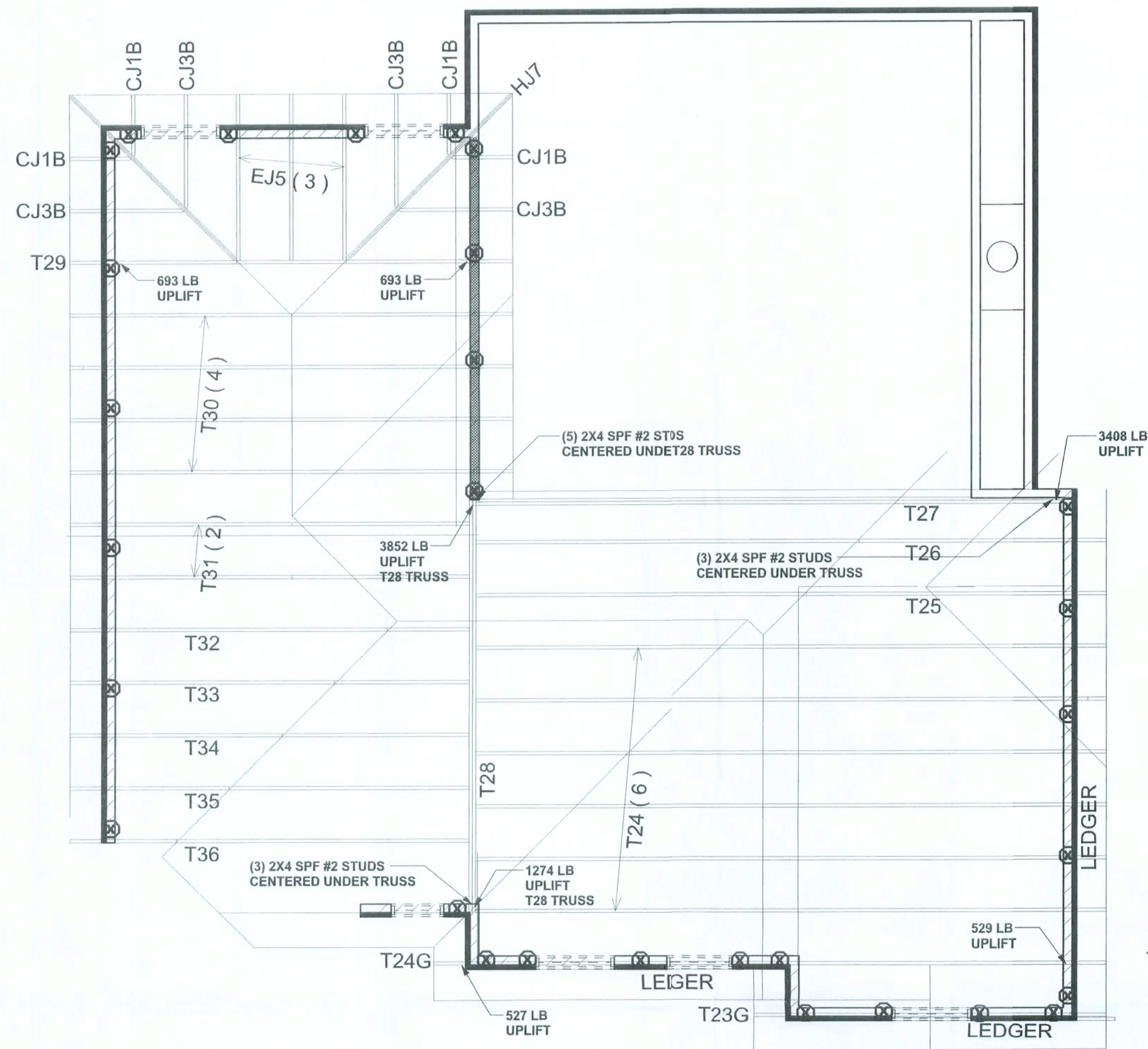
DRAWING NUMBER

S-2

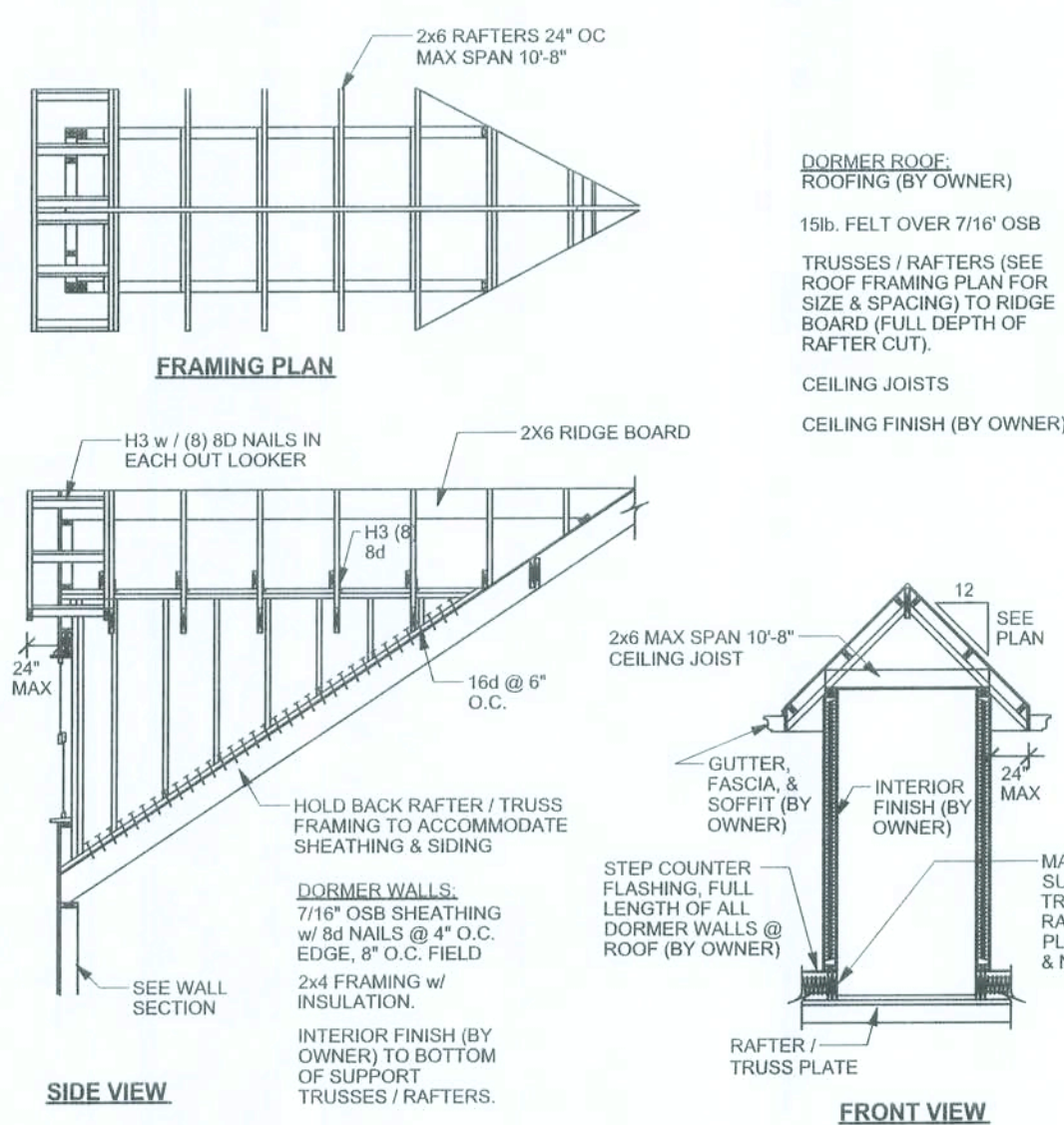
OF 4 SHEETS

USE H2.5¹/₄ (480lb) TO ATTACH ALL TRUSSES w/ UPLIFT TO WALLS AND PORCH BEAMS UNLESS NOTED OTHERWISE

NOTE: JACK STUDS MUST BE FULL HEIGHT TO TOP OF TRANSOM



2ND FLOOR STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"



DORMER ANCHORING DETAIL (ON ROOF)
SCALE: N.T.S.

1ST FLOOR STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-103. BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-E-01, BCSI-E-02, & BCSI-E-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND

	HEADER/BEAM CALL-OUT (U.N.O.)
	NUMBER OF KING STUDS (FULL LENGTH)
	NUMBER OF JACK STUDS (UNDER HEADER)
	SPAN OF HEADER
	SIZE OF HEADER MATERIAL
	NUMBER OF PLYS IN HEADER

THREADED ROD LEGEND

	INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD
	INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	46.5'	75.4'
LONGITUDINAL	38.9'	69.7'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB # 305006

REVISIONS	

SOFTPLAN
ARCHITECTURAL SOFTWARE

WINDLOAD ENGINEER:
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386-754-5419

DIMENSIONS:
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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Mark Drowsay
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SEL

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DRAWING NUMBER
S3
OF 4 SHEETS