Attention Wegie

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000001280

DATE: 12/18/2006 BUILDING PERMIT NO. 25320

APPLICANT MIKE TODD PHONE 755-4387

ADDRESS 129 NE COLBURN AVE LAKE CITY FL 32055

OWNER GEROGE BROOKS ETHERIDGE JR PHONE 752-2752

ADDRESS 171 NE COLBURN AVE LAKE CITY FL 32055

CONTRACTOR MIKE TODD PHONE 755-4387

LOCATION OF PROPERTY 90 EAST, L COLBURN AVE, WAREHOUSE ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT DUVAL HEIGHTS 6-11

PARCEL ID # 33-3S-17-06484-000

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: [signature]

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED ______ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS:

SIGNED: [signature] DATE: 3-15-07

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Attention: Wesie

Columbia County Building Department
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ADDRESS 129 NE COLBURN AVE LAKE CITY FL 32055

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LOCATION OF PROPERTY 90 EAST, L COLBURN AVE, WAREHOUSE ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT DUVAL HEIGHTS 6-11

PARCEL ID # 33-3S-17-06484-000

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: X

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

____ APPROVED ____ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: ____

SIGNED: ____ DATE: ____

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

RECEIVED
DEC 19 2006
By: ____

COLUMBIA COUNTY GATEWAY TO FLORIDA

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 061-38 Date Received 11-16-06 By LH Permit # 1280 / 25320

Application Approved by - Zoning Official BLK Date 11.11.06 Plans Examiner OKJTH Date 12-12-06

Flood Zone X Development Permit N/A Zoning CI Land Use Plan Map Category COMMERCIAL

Comments ~~FW~~, ~~Well~~ Under 1 acre NO SOP Required

Compaction test results given CELL 867-0477

Applicants Name Mike Todd Construction INC Phone 386 755 4387

Address 129 NE Colburn Avenue, Lake City FL 32055

Owners Name George Brooks Etheridge JR Phone 386 752-2752

911 Address 171 NE Colburn Avenue

Contractors Name Mike Todd Construction, INC Phone 386 7554387

Address 129 NE Colburn Avenue, Lake City, FL 32055

Fee Simple Owner Name & Address n/a

Bonding Co. Name & Address n/a

Architect/Engineer Name & Address n/a

Mortgage Lenders Name & Address n/a

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 33-3S-17-06489-000 Estimated Cost of Construction 100,000

Subdivision Name Duval Heights N1/2 6,9,11 Lot 7,8 Block C Unit ___ Phase ___

Driving Directions East on Hwy 90 TO NE Colburn Ave - Turn (L) - warehouse on Right

Type of Construction Comm. metal bldg Number of Existing Dwellings on Property 1

Total Acreage .92 Lot Size 125 x 340 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 40' Side 127' Side 290' Rear 50'

Total Building Height 14'4" Number of Stories 1 Heated Floor Area -0- TOTAL 5,000 Roof Pitch 2/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CGC062097

Competency Card Number ___

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

NOTARY PUBLIC STATE OF FLORIDA (407) 398-0153 TERESA N PIERCE MY COMMISSION # DD474390 EXPIRES: Sept. 21, 2009 Florida Notary Service.com

Sworn to (or affirmed) and subscribed before me this 16 day of November 2006.

Personally known ✓ or Produced Identification ___

Notary Signature

FW ADDED THEREAFTER 12-12-06

NOTICE OF COMMENCEMENT

To Whom It May Concern:

The undersigned hereby informs you that improvements will be made to certain real property, and in accordance with Section 713.13, Florida Statutes, the following information is stated in this Notice of Commencement.-

DESCRIPTION OF REAL PROPERTY TO BE IMPROVED: 33-3S-17-06484-000, DUVAL HEIGHTS S/D, LOT 6, 7, 8, BLOCK C

GENERAL DESCRIPTION OF IMPROVEMENTS: WAREHOUSE

OWNER: GEORGE BROOKS ETHERIDGE, JR

ADDRESS: 171 NE COLBURN AVENUE, LAKE CITY, FLORIDA 32055

OWNER'S INTEREST IN THE SITE OF THE IMPROVEMENTS (IF OTHER THAN FEE SIMPLE TITLE HOLDER):

ADDRESS:

CONTRACTOR: MIKE TODD CONSTRUCTION, INC

ADDRESS: 129 NE COLBURN AVENUE, LAKE CITY, FLORIDA 32055

SURETY ON ANY PAYMENT BOND: N/A

Any person within the State of Florida designated by owner upon whom notices or other documents may be served under Part 1 of Chapter 713, Florida Statutes, which service shall constitute service upon owner:

NAME:

ADDRESS:

Inst:2006029675 Date:12/18/2006 Time:15:41
DC,P.DeWitt Cason,Columbia County B:1105 P:539

In addition to himself/herself, owner designates the following person to receive a copy of the Lienor's notice as provided in Section 713.06(2)(b), Florida Statutes:

NAME: MIKE TODD CONSTRUCTION, INC

ADDRESS: 129 NE COLBURN AVENUE, LAKE CITY, FLORIDA 32055

This Notice of Commencement shall expire upon completion of contract.

Sworn to and subscribed before me this 18 day of Decembe, 2006.

Teresa N Pierce
Notary Public
My Commission Expires: Sept 21, 2009

NOTARY PUBLIC STATE OF FLORIDA
TERESA N. PIERCE
MY COMMISSION # DD474390
EXPIRES: Sept. 21, 2009
(407) 398-0153 Florida Notary Service.com

25320

Cal-Tech Testing, Inc.

• Engineering
• Geotechnical
• Environmental
Laboratories

P.O Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3833 • Fax(386)752-5456
8919 Distribution Ave. S., Unit #6, Jacksonville, FL 32257 • Tel(904)262-4046 • Fax(904)262-4047
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3486 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 06-655
DATE TESTED: 12/18/06
DATE REPORTED: 12/18/06

PROJECT:	Ethridge Furniture Wharehouse, Lake City, FL
CLIENT:	Mike Todd Construction, 129 NE Colburn Ave , Lake City, FL 32055
GENERAL CONTRACTOR:	Mike Todd Construction
EARTHWORK CONTRACTOR:	Mike Todd Construction
INSPECTOR:	Chad Day

ASTM METHOD	SOIL USE
(D-2922) Nuclear	OTHER

SPECIFICATION REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft³)	MOISTURE PERCENT	DRY DENSITY (lb/ft³)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
1		SW Corner 20' North	12"	106.4	3.9	102.4	1	107.5	95.3%
2		SW Corner 50' North	12"	111.3	8.2	102.9	1	107.5	95.7%
3		NW Corner 18' East	12"	109.8	5.6	104.0	1	107.5	96.7%
4		SE Corner 60' North	12"	108.7	5.5	103.0	1	107.5	95.8%
5		SE Corner 30' North	12"	107.6	4.1	103.4	1	107.5	96.2%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft³)	OPT. MOIST.	TYPE
1		Grey Sand w/Silt	107.5	12.5	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Reviewed By:

Linda M. Creamer
President - CEO

Date:
Florida Registration No:

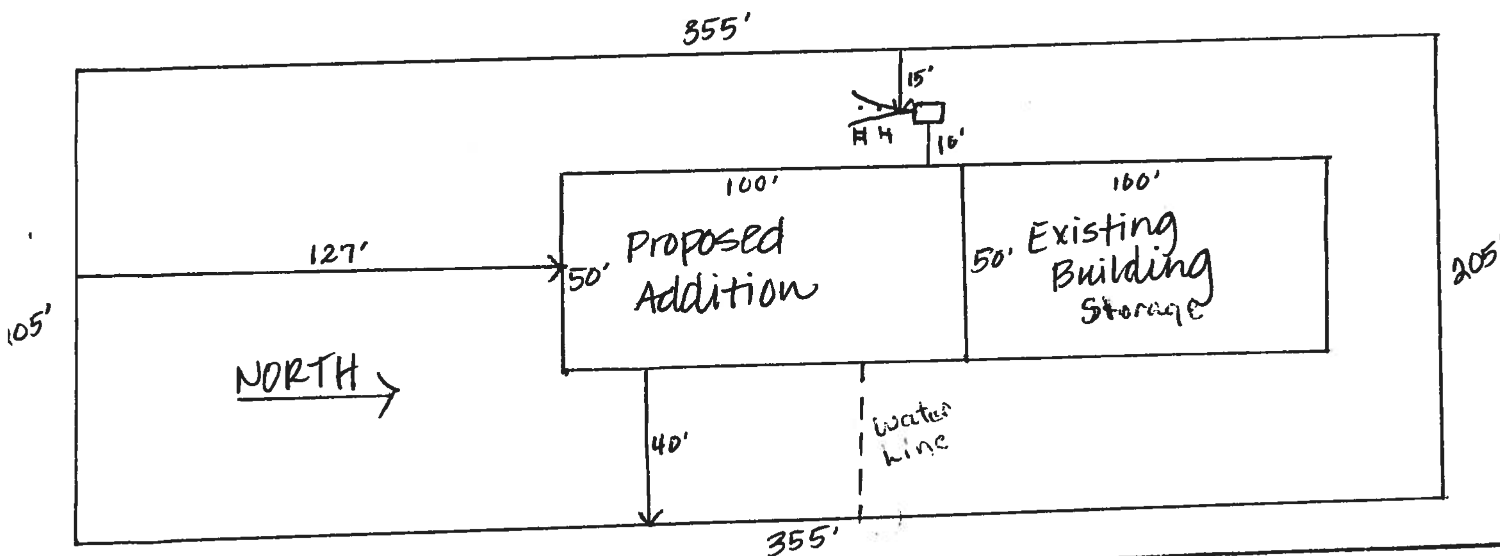
ss

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-1017N

PART II - SITEPLAN

e: 1 inch = 50 feet.



otes:

ite Plan submitted by: Rock D [signature]

MASTER CONTRACTOR

lan Approved ✓ Not Approved

Date NOV 0 2 2006 11/1/06

y [signature] Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

H 4015, 10/96 (Replaces HRS-H Form 4016 which may be used)
Stock Number: 5744-002-4015-6)

Columbia County Property Appraiser

DB Last Updated: 10/4/2006

2006 Proposed Values

Parcel: 33-3S-17-06484-000

Tax Record | Property Card | Interactive GIS Map | Print

Owner & Property Info

<< Prev Search Result: 5 of 6 Next >>

Owner's Name	ETHERIDGE JR GEORGE BROOKS
Site Address	DOR 2000
Mailing Address	1052 SW MAIN BLVD LAKE CITY, FL 32025
Description	N1/2 LOT 6 & ALL LOTS 7 & 8, BLOCK C, DUVAL HEIGHTS S/D. ORB 447-532,715-699, 720-764, 752-1094, 763-737, TAX DEED 850-1320, 859-1324, THRU 859-1328,

Use Desc. (code)	WAREHOUSE- (004800)
Neighborhood	33317.08
Tax District	2
UD Codes	MKTA03
Market Area	06
Total Land Area	0.482 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$11,340.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (1)	$67,118.00
XFOB Value	cnt: (2)	$1,000.00
Total Appraised Value		$79,458.00

Just Value	$79,458.00
Class Value	$0.00
Assessed Value	$79,458.00
Exempt Value	$0.00
Total Taxable ue	$79,458.00

911 address

171 NE Colburn Ave

Sales History

Sale Date	Bool	ual	Sale RCode	Sale Price
5/27/1998	859/		01	$26,000.00
12/15/1997	850/		01	$18,600.00
8/5/1992	763		01	$0.00

Building Characteris

Bldg Item	Blc	ed S.F.	Actual S.F.	Bldg Value
1	PREF M E	000	5000	$67,118.00
	Note: All	dimensions.		

Extra Features & Ou

Code	Desc				s	Condition (% Good)
0166	CONC,PAVM				x 0	(.00)
0294	SHED WOOD/	1998	$500.00	1.000	16 x 20 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
004800	WAREHOUSE (MKT)	21000.000 SF - (.482AC)	1.00/1.00/1.00/.90	$0.54	$11,340.00

Columbia County Property Appraiser DB Last Updated: 10/4/2006

<< Prev 5 of 6 Next >>

Columbia County Property Appraiser

DB Last Updated: 10/4/2006

2006 Proposed Values

Parcel: 33-3S-17-06490-000 Tax Record Property Card Interactive GIS Map Print

<< Prev Search Result: 12 of 19 Next >>

Owner & Property Info

Owner's Name	TODD FRANK A III & REBECCA E
Site Address	DUVAL HEIGHTS
Mailing Address	207 NW PALMETTO BLVD LAKE CITY, FL 32055
Description	LOTS 1 & 2 BLOCK F DUVAL HEIGHTS S/D EX N 52 FT. ORB 447-532, 560-304, 693-566, 715-699, 720-764, 752-1094, 763-737, 859-1324, 1326,

Use Desc. (code)	VACANT (000000)
Neighborhood	33317.08
Tax District	2
UD Codes	MKTA03
Market Area	06
Total Land Area	0.303 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$7,286.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$7,286.00

Just Value	$7,286.00
Class Value	$0.00
Assessed Value	$7,286.00
Exempt Value	$0.00
Total Taxable Value	$7,286.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
5/27/1998	859/1326	WD	V	U	02	$55,000.00
8/5/1992	763/737	QC	V	U	01	$0.00
10/29/1991	752/1094	QC	V	U	01	$0.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	13200.000 SF - (.303AC)	1.00/1.00/.92/1.00	$0.55	$7,286.00

Columbia County Property Appraiser DB Last Updated: 10/4/2006

<< Prev 12 of 19 Next >>

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

#25320

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB100078 Company Phone No. 386-755-2311
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Mike Todd Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 129 N.E. Colbert Ave Lake City, FL

Type of Construction (More than one box may be checked) ☐ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside 1'0 24 Inside 1' 2 Type of Fill Dirt

Section 4: Treatment Information

Date(s) of Treatment(s) 1-4-07
Brand Name of Product(s) Used Typar T-C
EPA Registration No. 53883-92
Approximate Final Mix Solution % 0.25 %
Approximate Size of Treatment Area: Sq. ft. 5400 Linear ft. 300 Linear ft. of Masonry Voids 250
Approximate Total Gallons of Solution Applied 945
Was treatment completed on exterior? ☐ Yes ☑ No
Service Agreement Available? ☐ Yes ☑ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments Treated Addition to existing building

Name of Applicator(s) Steve Brannon Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Date

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

BSX - BUILDING SYSTEMS EXPRESS
Division of Metal Buildings, Inc.
115 Fairbanks Ave.
P.O. Box 860
Thomasville, GA 31799-0860
Phone: 229-228-1949
Fax: 229-226-6874

RICHARD A. POWELL
PROFESSIONAL ENGINEER
196 Union Hill Road
Pelham, GA 31779

LETTER OF DESIGN CERTIFICATION

Date: 10/17/2006

Customer: Mr. Lee Sapp
L & L Construction
524 NE Carr Court
Lake City, FL 32055

RE: 50' W x 100' L x 10' EH Building
Columbia County, FL
BSX Job No. XF61046

To whom it may concern:

This is to certify that the pre-fabricated metal building system, described and detailed on the documents listed below, has been designed utilizing the criteria listed on pages 1 and 2 of the attached "General Notes / Specifications". This certification is limited to the structural design of the steel framing and cladding provided and / or manufactured by BSX. This specifically does NOT include any determination of the suitability of any existing structure or foundations to resist any additional loadings as may be imposed by the new BSX structure. Design of the interaction of the new BSX structure with any existing structures is the responsibility of the owner or the owner's design professional (i.e., NOT the undersigned, nor BSX). The supplying of erection drawings and / or engineering data for the pre-fabricated metal building system does not imply or constitute an agreement that BSX or the undersigned is acting as the engineer-of-record or prime professional for the construction project. It is the responsibility of the owner or the owner's design professional to insure that his or her project specifications, and other other aspects of overall project design, are adequate for the intended building use and are in compliance with all applicable requirements of any governing authorities. When the governing building code or any governing authority requires preparation of quality assurance plans and/or special inspections for wind and seismic force resisting systems, the preparation and implementation of such plans and inspections shall be the responsibility of the owner or the owner's design professional (i.e., NOT the undersigned, nor BSX). The following is a list of the pre-fabricated metal building system construction / design documents issued by BSX for general use on this project:

General Notes / Specifications, pages 1 and 2

Reactions for Foundation Design, pages 1 thru 3

Anchor Bolt Location Drawings AB1 & AB2

Erection Drawings E1 THRU E6

When formally requested, two copies of the above documents, along with design calculations and shop fabrication drawings, will be furnished directly to the engineer-of-record or prime professional for his or her review. If this has been requested, one set of these documents shall be returned to BSX noted with comments for any required actions. If additional copies of these documents are desired, they can be furnished on a fee basis.

If there are any questions or should any additional information be needed regarding this project, please contact BSX. Business hours are from 8:00 a.m. to 5:00 p.m., Monday through Thursday, and from 8:00 a.m. to 4:00 p.m. on Fridays.

Respectfully Yours

R A Powell 10/20/2006

Richard A. Powell, P.E., FL License No. 40676

General Notes / Specifications for PRE-FABRICATED METAL BUILDING SYSTEM

Project: 50' W x 100' L x 10' EH Building
Customer: L & L Construction
Job No: XF61046

1. DESIGN REFERENCES AND STANDARDS

1.1 2004 Florida Building Code

1.2 ASCE 7-02 "Minimum Design Loads for Buildings and Other Structures"

1.3 MBMA "Low Rise Building Systems Manual" 2002 Edition

1.4 AISC "Specification for Structural Steel Buildings" Allowable Stress Design and Plastic Design, June 1, 1989 with Commentary, and with Supplement No. 1, December 17, 2001

1.5 AISC "Seismic Provisions for Structural Steel Buildings", April 15, 1997 with Supplement No. 2, November 10, 2000

1.6 RCSC "Specification for Structural Joints Using ASTM A325 or A490 Bolts" Allowable Stress Design, June 23, 2000

1.7 AISI "North American Specification for the Design of Cold-Formed Steel Structural Members", 2001 Edition

1.8 SDI "Diaphragm Design Manual", 1st, 2nd & 3rd Editions

1.9 ANSI / AWS D1.1-2000 "Structural Welding Code - Steel"

1.10 ANSI / AWS D1.3-1998 "Structural Welding Code - Sheet Steel"

2. DESIGN LOADS

2.1 Roof dead load taken as the weight of the framing and cladding (1.71 psf + rafter weight).

2.2 No collateral load (i.e., additional dead load) has been specified in the order documents for this building.
No special concentrated loads have been specified or included for the design of the roof system (i.e., roof top HVAC units, hoisting systems, etc.).

2.3 Design for roof live load is based on the provisions of section 1604.6 of reference 1.1 above. Roof live loads are as follows:

20.00 psf for members supporting up to 200 sf
Varies for members supporting 201 to 600 sf
12.00 psf for members supporting over 600 sf

2.4 Per section 1605 of reference 1.1, there are no snow load provisions to be applied to this structure.

2.5 Design for wind load is based on the lowrise provisions of section 6.0 of reference 1.2 above. Wind load design data is as follows:

Basic wind speed, V, is 100 mph (3-sec gust)
Wind importance factor, Iw, is 1.00
Wind exposure category is B
Internal pressure coefficients, GCpi, are +0.18; -0.18

2.6 Per reference 1.1, there are no seismic provisions to be applied to this structure.

2.7 The load combinations used for design are those specified in section 1609.4 of reference 1.1, and section 2.4 of reference 1.2.

3. STRUCTURAL STEEL FRAMING

3.1 The steel framing members are designed to meet the standards, loadings and criteria established in sections 1 and 2 of these general notes. The framing system is a combination of Type 1 and Type 2 construction as defined in section A2 of reference 1.4. The system has been designed to resist the stresses determined from a first-order elastic analysis. Except where noted otherwise on the drawings or where otherwise indicated by the reactions furnished for foundation design, column bases have been treated as frictionless hinges in the analysis. Lateral stability for the system as a whole is provided by the following:

Rigid frame action of clearspan gable frames parallel to endwalls
Metal panel shear wall diaphragm parallel to sidewalls

Roof panels and/or bridging provide lateral stability for roof purlins, which in turn provide lateral stability for their supporting rafters. Where present, wall panels and/or bridging provide lateral stability for wall girts, which in turn provide lateral stability for their supporting columns.

3.2 Fabrication shall be in accordance with references 1.3 and 1.4 above. Unless noted otherwise in the order documents, all hot-rolled steel members shall be cleaned in accordance with SSPC - SP2 or SP7, and shall receive one shop coat of standard red primer of not less than 1.0 mil dry film thickness. Painted cold-formed components (i.e., cees, zees, etc.) shall receive one coat of polyester-based red primer, not less than 0.5 mil dry film thickness, oven baked prior to cold-forming by the component manufacturer. After cutting and welding operations, damaged areas of the oven baked primer on cold-formed members will be cleaned in accordance with SSPC-SP2 and touched up with same primer as applied to hot-rolled members.

Note: The primer shop coat applied to the structural members is designed to provide minimal short term protection of the steel during shipping and erection. If a greater level of protection is needed or desired, such protection shall be designed and provided by others, not by the building manufacturer.

3.3 Erection shall be in accordance with these plans and Section IV of reference 1.3 above. All A325 bolts need only be tightened to the snug-tight condition, as defined in reference 1.6 above, unless noted otherwise on the drawings. All field welding shall be performed by certified welders using low hydrogen electrodes.

Erection shall be performed by a qualified erector using proper tools and equipment. It shall be the responsibility of the erector to comply with all applicable laws and safety regulations. It shall be the responsibility of the erector to determine and provide any and all temporary bracing and/or securing of the system as required for stability during the entire erection process. One recognized reference on the design of erection bracing for low-rise steel buildings is AISC Design Guide No. 10.

3.4 Materials

W Shapes	ASTM A992 or A572-50 Fy = 50 ksi
Channels, angles, plates & bars	ASTM A36 Fy = 36 ksi
Cold-Formed Cees, Zees & Angles (Painted)	ASTM A1011, Gr.55 Fy = 55 ksi
Cold-Formed Cees, Zees & Angles (Galvanized	ASTM A653, Gr. 50 Fy = 50 ksi

Structural bolts for primary framing shall be 5/8 inch diameter or larger, as noted on the drawings, and shall meet ASTM A325.
Structural bolts for secondary framing (e.g., purlins and girts) shall be 1/2 inch diameter, and shall meet ASTM A307 or ASTM A325 as noted on the drawings.

Welding electrodes for shop and field welds shall conform to one of the following:

SMAW process	AWS A5.1, E70XX
GMAW process	AWS A5.18, E70S-X
FCAW process	AWS A5.20, E7XT-X

4. METAL ROOF AND WALL COVERING AND TRIM

4.1 Design

The metal roof and wall systems are designed to meet the loadings, standards and criteria listed in sections 1 and 2 of these general notes. Where metal roof and wall systems are used as diaphragms, they have been designed in accordance with the general procedure outlined in reference 1.8, modified as needed to comply with the more recent requirements of references 1.2 and 1.7.

Note: When section 2.6 and/or section 3.1 of these general notes indicate that metal wall panels will be used as shear wall diaphragms, it is absolutely essential that the panels are properly fastened to their supports and to each other in order to provide the strength and stiffness required of the diaphragm assembly.

The minimum standards for fastening of metal wall panels for this project are given in sections 4.4 and 4.6 of theses general notes. When the fastening requirements are different than these minimum standards, the required fastening schedule(s) shall be clearly noted on the erection drawing panel plans.

4.2 Materials

Unless noted otherwise on the drawings, the exposed surfaces of all metal roof and wall panels and trim shall be either clear acrylic coated or factory painted GALVALUME®. Acrylic coated GALVALUME® shall have a Coating Class AZ55. Factory painted GALVALUME® shall have a minimum Coating Class AZ50. GALVALUME® coated steel for roof and wall panels shall conform to ASTM A792 Grade 80. GALVALUME® coated steel for flashing and trim shall conform to ASTM A792 Grade 50. Unless noted otherwise in the order documents, factory painted GALVALUME® panels and trim shall have a baked-on silicone polyester finish coat applied to one side and a baked-on straight polyester wash coat applied to the other side. Including the primer coat, thickness of the finish coat and the wash coat shall be a nominal 1.0 and 0.5 mils respectively.

Note: GALVALUME® is a registered trademark of BIEC International, Inc.

4.3 Roof Panels

Roof panels shall be "Super Span" or "Super Span-X" panel as manufactured by WHIRLWIND BUILDING COMPONENTS or an essentially equivalent panel produced by a number of other component manufacturers and generally referred to as "R" or "PBR" panel. Unless noted otherwise on the drawings, roof panels shall be secured to framing members as follows:

Panel to framing connections at interior lines of support to be 12 inches o.c. beginning 2-1/2 to 3 inches from centerline of major panel rib. This results in one fastener for each major panel rib.

Panel to framing connections at panel ends to be in a 5-7-5-7 inch pattern beginning 2-1/2 to 3 inches from centerline of major panel rib. This results in two fasteners for each major panel rib.

Panel to panel connections (i.e., side laps) shall be approximately 20 inches o.c. with one fastener located in line with the panel to framing fasteners.

The first panel run at each end of the building shall be fastened to the endwall rafter, rake angle or other shear collecting member at approximately 20 inches o.c.

Where called for in the order documents and indicated on the drawings, a fiberglass insulation blanket not more than 6 inches thick may be installed between the panels and supporting framing.

4.4 Wall Panels

When provided, wall panels shall be "Super Span" panel as manufactured by WHIRLWIND BUILDING COMPONENTS or an essentially equivalent panel produced by a number of other component manufacturers and generally referred to as "R" panel. Unless noted otherwise on the drawings, wall panels shall be secured to framing members as follows:

Panel to framing connections at interior lines of support to be 12 inches o.c. beginning 2-1/2 to 3 inches from centerline of major panel rib. This results in one fastener for each major panel rib.

Panel to framing connections at panel ends to be in a 5-7-5-7 inch pattern beginning 2-1/2 to 3 inches from centerline of major panel rib. This results in two fasteners for each major panel rib.

Panel to panel connections (i.e., side laps) shall be approximately 24 inches o.c. with one fastener located in line with the panel to framing fasteners.

Panel runs at each end of the building and each side of openings shall be fastened to columns, door jambs or other shear collecting members at approximately 24 inches o.c.

Where called for in the order documents and indicated on the drawings, a fiberglass insulation blanket not more than 6 inches thick may be installed between the panels and supporting framing.

4.5 Roof Panel Fasteners

Roof panel to framing fasteners shall be No. 12-14 x 1-1/4 inch self-drilling, self-tapping, hex head, plated steel screw with a separate 5/8 inch O.D. formed steel washer and a neoprene sealing washer.

OR

When called for in the order documents, roof panel to framing fasteners shall be No. 12-14 x 1-1/4 inch self-drilling, self-tapping plated screw with an EPDM sealing washer housed in an undercut head (i.e., ZAC screw as manufactured by Construction Fasteners or equivalent long-life screw).

Roof panel sidelap fasteners shall be No. 14 x 7/8 inch screws of the same type as roof panel to framing fastener.

Where an insulation blanket is to be installed between the panels and supporting framing, greater length panel-to-framing screws will be furnished as needed based on the thickness of the insulation blanket.

4.6 Wall Panel Fasteners

Wall panel to framing fasteners shall be No. 12-14 x 1 inch self-drilling, self-tapping, hex head, plated steel screws (no washer) with heads painted to approximately match color of wall panels.

OR

When called for in the order documents, wall panel to framing fasteners shall be No. 12-14 x 1 inch self-drilling, self-tapping, hex head, plated steel screw with a separate 5/8 inch O.D. formed steel washer and a neoprene sealing washer.

Wall panel sidelap fasteners shall be No. 14 x 7/8 inch screws of the same type as wall panel to framing fastener.

Where an insulation blanket is to be installed between the panels and supporting framing, greater length panel-to-framing screws will be furnished as needed based on the thickness of the insulation blanket.

5. FOUNDATIONS AND BUILDING ANCHORAGE

5.1 Foundations

Unless noted otherwise in the order documents, the building manufacturer is not responsible for the design, materials and workmanship of the foundation. The anchor bolt plans prepared by the building manufacturer are intended to show only location, diameter and projection of anchor rods required to attach the building to the foundation. The manufacturer shall be responsible for providing the loads imposed on the foundation by the metal building structure. The manufacturer shall meet this requirement by furnishing the service level reactions on a load case basis, thereby allowing the foundation designer to generate as many different load combinations as needed using the principle of superpositioning. The furnishing of service level reactions on a load case basis, as opposed to a load combination basis, will also allow the foundation designer to apply the appropriate load factors for each load case in a given combination if load factored procedures will be used to design the foundation. If requested, the manufacturer will provide reactions for the allowable stress design combinations specified in section 2.7 of these notes.

5.2 Anchor Rods

Unless noted otherwise in the order documents, anchor rods shall be furnished by others, not by the building manufacturer.

The minimum anchor rod diameters indicated on the anchor bolt plans have been determined using either the shear friction approach of ACI 349 or the trilinear equation given in the AISC-ASD Specification Table J3.3 for threaded parts. Unless noted otherwise on the drawings, determination of required anchor rod diameters has assumed the use of material meeting ASTM A307, A36 or A1554 Grade 36.

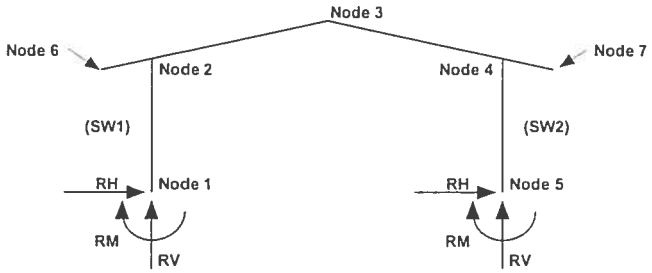
Only the interaction of shear and tension as governed by the steel strength of the anchor material has been considered in the determination of the indicated anchor diameters. Other considerations that may affect anchorage strength (i.e., concrete breakout strength of an anchor or group of anchors in shear, tension or combined shear and tension, etc.) are an integral part of concrete pedestal / footing design, and therefore must be investigated by the foundation designer, not the building manufacturer. The foundation designer may refer to section 1912 of FBC-2004 for allowable stress design of anchorage to concrete. However, this procedure is very restrictive due to the short embedment lengths listed for use in Table 1912.2. Therefore, the foundation designer may wish to use the strength design method of ACI 318-02, Appendix D, to obtain more practical results.

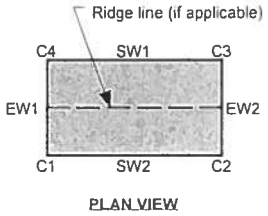
Note: If the foundation designer determines that larger diameter anchors than those indicated on the manufacturer's drawings are necessary, the purchaser shall notify the building manufacturer of the required changes, prior to returning a signed authorization to proceed with fabrication, so that column base plate design and details can be revised to accomodate the larger anchors.

REACTIONS FOR FOUNDATION DESIGN

Project: 50' W x 100' L x 10' EH Building
Customer: L & L Construction
Job No: XF61046

FRAME REACTIONS BY LOAD CASE (based on loading at Line 4):





PLAN VIEW

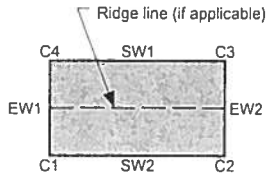
NOTE: Positive reactions act in the direction indicated above.

Load Case No.	DESCRIPTION	Node 1 RH (kips)	Node 1 RV (kips)	Node 1 RM (kip-ft)	Node 5 RH (kips)	Node 5 RV (kips)	Node 5 RM (kip-ft)	Equilibrium Check Sum VF (kips)	Sum HF (kips)	Sum M (kip-ft)
1	Dead Load	1.16	1.95	0.00	-1.16	1.95	0.00	0	0	0
2	Collateral Load									
3	Live Load	5.52	8.42	0.00	-5.52	8.42	0.00	0	0	0
4	Transverse Wind, w/ I.P. (SW1 windward)	-6.06	-8.79	0.00	3.33	-6.70	0.00	0	0	0
5	Transverse Wind, w/ I.P. (SW2 windward)	-3.33	-6.70	0.00	6.06	-8.79	0.00	0	0	0
6	Transverse Wind, w/ I.S. (SW1 windward)	-4.41	-4.95	0.00	1.67	-2.86	0.00	0	0	0
7	Transverse Wind, w/ I.S. (SW2 windward)	-1.67	-2.86	0.00	4.41	-4.95	0.00	0	0	0
8	Longitudinal Wind, w/ I.P. (SW1 windward)	-4.52	-10.19	0.00	4.68	-9.36	0.00	0	0	0
9	Longitudinal Wind, w/ I.P. (SW2 windward)	-4.68	-9.36	0.00	4.52	-10.19	0.00	0	0	0
10	Longitudinal Wind, w/ I.S. (SW1 windward)	-2.87	-6.35	0.00	3.03	-5.52	0.00	0	0	0
11	Longitudinal Wind, w/ I.S. (SW2 windward)	-3.03	-5.52	0.00	2.87	-6.35	0.00	0	0	0
12										
13										
14										
15										
16										
17										
18										
19										
20										
21										
22										
23										
24										

REACTIONS FOR FOUNDATION DESIGN

Project: 50' W x 100' L x 10' EH Building
Customer: L & L Construction
Job No: XF61046

COLUMN REACTIONS FOR ENDWALL 1 (By Load Case):



PLAN VIEW

Standard Load Cases

No.	Description
1	Dead load, D
2	Collateral load, C
3	Roof live load, Lr
4	Snow load, S
5	Roof wind load pressure, Wp
6	Roof wind load suction, Ws
7	Wall load pressure
8	Wall load suction
9	Vertical seismic, 0.2*SDS*(D+C)
10	Wall seismic, Max(0.40*Ie*SDS*wall weight, 0.10*wall weight)

NOTES:

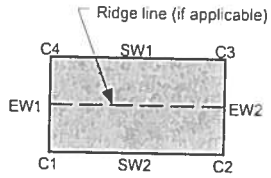
1. Positive axial reactions indicate a compressive force at the column base.
2. Negative axial reactions indicate an up-lift force at the column base.
3. Column 1 is at Sidewall 1 (i.e., SW1) with other column numbers assigned in sequential order towards Sidewall 2 (i.e., SW2).

Column No.	CASE 1		CASE 2		CASE 3		CASE 4		CASE 5	
	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)
1	0.00	0.23	NA	NA	0.01	1.85	NA	NA	0.00	0.93
2	0.00	0.58	NA	NA	0.02	4.68	NA	NA	0.01	2.39
3	0.00	0.58	NA	NA	0.02	4.68	NA	NA	0.01	2.39
4	0.00	0.23	NA	NA	0.01	1.85	NA	NA	0.00	0.93
5	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
6	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
7	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
8	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
9	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
10	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
11	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
12	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
13	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
14	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
15	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
16	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
17	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
18	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
19	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
20	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
21	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA

Column No.	CASE 6		CASE 7		CASE 8		CASE 9		CASE 10	
	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)	Shear (kips)	Axial (kips)
1	0.01	-1.96	0.58	0.00	0.73	0.00	NA	NA	NA	NA
2	0.02	-5.02	1.35	0.00	1.59	0.00	NA	NA	NA	NA
3	0.02	-5.02	1.35	0.00	1.59	0.00	NA	NA	NA	NA
4	0.01	-1.96	0.58	0.00	0.73	0.00	NA	NA	NA	NA
5	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
6	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
7	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
8	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
9	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
10	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
11	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
12	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
13	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
14	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
15	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
16	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
17	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
18	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
19	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
20	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
21	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA

REACTIONS FOR FOUNDATION DESIGN

Project: 50' W x 100' L x 10' EH Building
Customer: L & L Construction
Job No: XF61046



PLAN VIEW

METAL PANEL SHEAR WALL DIAPHRAGMS:

Wall Location	Mean Wall Hgt. (feet)	PH Wind (lbs)	PH Seismic (lbs)	Fv Wind (lbs / lf)	Fv Seismic (lbs / lf)	Lmin (Feet)	fv Wind (lbs / lf)	fv Seismic (lbs / lf)	CF Wind (lbs)	CF Seismic (lbs)	Le (Feet)	
EW1	12.08333	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	Diaphragm bracing not used at Endwall 1
EW2	12.08333	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	Diaphragm bracing not used at Endwall 2
SW1	10	1,805	NA	138	NA	13.08	18.3	NA	183	NA	98.5	O.K.
SW2	10	1,805	NA	138	NA	13.08	23.0	NA	230	NA	78.5	O.K.

Definitions:

PH = Maximum horizontal force, wind or seismic, to be resisted by diaphragm.

Fv = Allowable unit shear for specifed wall panel (factor-of-safety is 2.65 when failure mode is shear buckling; otherwise 2.35 for wind and 2.5 for seismic). It should be recognized that there is no standard allowable unit shear for a given panel profile and gage. The strength of a diaphragm system can be limited by the strength of the connections (i.e., connections at panel edge, panel side laps, or panel ends), local buckling in the panels, or by general plate-like buckling of the entire diaphragm area.

fv = Computed unit shear in diaphragm = PH / Le

Lmin = Minimum effective sheeted length of wall (Le) required to resist PH (Lmin = PH / Fv, but not less than 1/2 of wall height or 6 feet). Values of Lmin indicated above have been rounded up to the nearest foot.

Le = Effective sheeted length of wall. This is generally taken as the full wall length, less the sum of the width of all openings in the wall and all wall segments between openings that are less than 1/2 the wall height or 6 feet in length. Where this approach is taken, diaphragm boundary members (i.e., chord members) are required at each side of each opening; otherwise, diaphragms must be evaluated for stress concentrations that will ocurr at openings.

CF = Axial load (i.e., Chord force) in diaphragm boundary member, for wind or seismic, based on length Le.

NOTE: If openings other than those indicated on the building manufacturer's panel plans are to be installed, the diaphragm must be evaluated for the resulting vertical and horizontal stress concentrations by a qualified design professional. To avoid the possibility of underdesigned diaphragms, the purchaser should provide the building manufacturer with detailed information on the size and exact location of all planned openings so that their effect can be evaluated in the original design of the pre-fabricated steel building system. The introduction of unplanned openings into the building system may, in some cases, result in a loss of column stability under lateral or lateral-torsional buckling, with possible catastrophic results.

0611.38

SUWANNEE
RIVER
WATER
MANAGEMENT
DISTRICT
9225 CR 49
LIVE OAK, FLORIDA 32060
TELEPHONE: (386) 362-1001
TELEPHONE: 800-226-1066
FAX (386) 362-1056

NOTICED GENERAL PERMIT

PERMITTEE:
MIKE TODD CONSTRUCTION
129 NE COLBURN AVE.
LAKE CITY, FL 32055

PERMIT NUMBER: ERP06-0610
DATE ISSUED: 11/30/2006
DATE EXPIRES: 11/30/2009
COUNTY: COLUMBIA
TRS: S33/T3S/R17E

PROJECT: MIKE TODD CONSTRUCTION

Approved entity to whom operation and maintenance may be transferred pursuant to rule 40B-4.1130, Florida Administrative Code (F.A.C.):

MIKE TODD
MIKE TODD CONSTRUCTION
129 NE COLBURN AVE.
LAKE CITY, FL 32055

Based on information provided, the Suwannee River Water Management District's (District) rules have been adhered to and an environmental resource noticed general permit is in effect for the permitted activity description below:

Construction and operation of an additional 5,000 square feet of impervious surface on a total project area of 0.25 acres in a manner consistent with the application package submitted by Mike Todd on November 20, 2006.

It is your responsibility to ensure that adverse off-site impacts do not occur either during or after construction. Any additional construction or alterations not authorized by this permit may result in flood control or water quality problems both on and off site and will be a violation of District rule.

You or any other substantially affected persons are entitled to request an administrative hearing or mediation. Please refer to enclosed notice of rights.

This permit is issued under the provisions of chapter 373, F.S., chapter 40B-4, and chapter 40B-400, F.A.C. A noticed general permit authorizes the construction, operation, maintenance, alteration,

Bldg. Dept

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

December 5, 2006

Mike Todd Construction
129 NE Colburn Avenue
Lake City, Florida 32055

Attention: Mike Todd

Reference: Commercial Metal Building
Colburn Avenue
Lake City, Florida
Cal-Tech Project No. 06-655

Dear Mr. Todd,

Cal-Tech Testing, Inc. has completed the subsurface investigation and engineering evaluation for the proposed building at the above referenced location. Our work was performed in conjunction with and authorized by you.

Introduction

We understand you will construct a single-story, block and metal-frame commercial building, with a plan area of approximately 5,000 square feet. Support for the structure is to be provided by conventional, shallow spread footings. We understand that the design bearing pressure for the foundations is 2,000 pounds per square foot (psf). Detailed foundation loads have not been provided; however, we assume column and wall loads will not exceed 60 kips and 2.0 kips per foot, respectively.

The purposes of our investigation were to evaluate the existing subgrade soils for an allowable bearing pressure of 2,000 psf and to present recommendations for foundation design and construction.

Site Investigation

The subsurface conditions were investigated by performing four (4) Standard Penetration Test borings advanced to depths of 10 to 21 feet. The borings were performed at the approximate locations indicated on the attached Report of Soil Borings, and were located in the field by the client.

The Standard Penetration Test (ASTM D-1586) is performed by driving a standard split-barrel sampler into the soil by blows of a 140-pound hammer falling 30 inches. The number of blows required to drive the sampler 1 foot, after seating 6 inches, is designated the penetration resistance, or N-value; this value is an index to soil density or consistency.

Findings

The soil borings generally encountered very loose to loose fine sands (SP) from the ground surface to a depth of about nine feet. This was underlain by loose to medium dense fine sands (SP) to the termination depth.

Ground water was encountered at depths of seven to 12.5 feet in Borings B-1 through B-3, and no groundwater was encountered in Boring B-4.

For a more detailed description of the subsurface conditions encountered, please refer to the attached Report of Soil Borings. Note that the transition between soil layers may be gradual and not abrupt as indicated by the logs; therefore, the thickness of soil layers should be considered approximate.

Discussion and Recommendations

The site soils appear to be loose to very loose near the ground surface and increase in density with depth. Based upon these findings, moderate site improvement should be performed; however, it is our opinion the site soils are suitable to provide support for the building using conventional, shallow spread footings. We concur that the foundations may be sized using a maximum soil bearing pressure of 2,000 psf; however, we recommend foundations have minimum widths of 18 and 24 inches for strip and isolated footings, respectively, even though the allowable soil bearing pressure may not be developed. The bottoms of foundations should be embedded a minimum of 18 inches below the lowest adjacent grade (finished surface grade, for example).

Due to the generally loose to very loose condition of the immediate bearing soils, we believe it would be beneficial to proof-roll and then proof-compact the bearing soils in all foundation and floor slab areas. These bearing soils should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density to a depth of at least two feet. Compaction of the bearing soils will reduce settling of the foundations and thereby reduce the likelihood of distress in the structure.

Our evaluation is based upon subsurface conditions encountered at this site and as presented within this report. However, subsurface conditions may exist that differ from our findings. We request that we be notified if substantially different subsurface conditions are encountered.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer, CEO

Linda Creamer
President / CEO

Robert W Clark

Robert W. Clark, P.E. 12/5/06
Geotechnical Engineer
Registered Florida No. 52210

COLUMBIA COUNTY BUILDING DEPARTMENT
Mike Todd (Etheridge Furniture storage addition)
COMMERCIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 WITH AMENDMENTS

ALL REQUIREMENTS LISTED ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INCLUDE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2004 WITH AMENDMENTS BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SIGNATURE AND SEAL OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA. THE FOLLOWING BASIC WIND SPEED AS PER SECTION 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ---------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ---------- 110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing a floor plan, site plan, foundation plan, floor/roof framing plan or truss layout, wall sections and all exterior elevations with the following criteria and documents:

Applicant	Plans Examiner	
☐	X	All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans. 5,000 sq.ft addition onto an existing 5,000 sq.ft Building (S1 storage)
☐	X☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed. Nicholas Geisler (foundation) Steel Building by Richard A. Powell
☐	☐	**Two (2) Copies of Approved Site Plan**
☐	☐	**Minimum Type Construction** (FBC chapter 6) Type V Building meeting the requirements of table 602 >30 Feet setback from property lines
☐	☐	**Wind Load Engineering Summary, calculations and any details required:** a) Plans or specifications must state compliance with FBC Section 1609 b) The following information must be shown as per section 1603.1.4 FBC 1. Basic wind speed (MPH) 100 MHP 2. Wind importance factor (I) and building category 3. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated B 4. The applicable internal pressure coefficient 5. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☐	☐	**Fire Resistant Construction Requirements shall include:**
☐	☐	a) Fire resistant separations (listed system)
☐	☐	b) Fire resistant protection for type of construction Zero hours > 30' Property
☐	☐	c) Protection of openings and penetrations of rated walls (listed systems)
☐	☐	d) Fire blocking and draft-stopping No attics and concealed roof spaces
☐	☐	e) Calculated fire resistance Zero hours > 30' Property

Fire Suppression Systems shall include: (To be reviewed by Fire Department)

- ☐ ☐ a) Fire sprinklers
- ☐ ☐ b) Fire alarm system (early warning) with name of licensed installer. If not shown on plans or not known at time of permitting, a separate permit shall be required by the licensed installer
- ☐ ☐ c) Smoke evacuation system schematic
- ☐ ☐ d) Stand-pipes
  - Pre-engineered system
  - Riser diagram

Life Safety Systems shall include: (To be reviewed by Fire Department) Ok Fire Department

- ☐ ☐ a) Occupancy load and egress capacity
- ☐ ☐ b) Early warning
- ☐ ☐ c) Smoke control
- ☐ ☐ d) Stair pressurization
- ☐ ☐ e) Systems schematic

Occupancy Load/Egress Requirements shall include:

- ☐ ☐ a) Occupancy load (gross and net)
- ☐ ☐ b) Means of egress Storage one required
  - exit access, exit and exit discharge
- ☐ ☐ c) Stair construction/geometry and protection
- ☐ ☐ d) Doors
- ☐ ☐ e) Emergency lighting and exit signs not required
- ☐ ☐ f) Specific occupancy requirements
  1. Construction requirements
  2. Horizontal exits/exit passageways

Structural Requirements shall include:

- ☐ ☐ a) Soil conditions/analysis
- ☐ ☐ b) Show type of termite treatment (termiticide or alternative method)
- ☐ ☐ c) Design loads
- ☐ ☐ d) Wind requirements
- ☐ ☐ e) Building envelope
- ☐ ☐ f) Structural calculations
- ☐ ☐ g) Foundations
- ☐ ☐ h) Wall systems
- ☐ ☐ i) Floor systems
- ☐ ☐ j) Roof systems
- ☐ ☐ k) Threshold inspection plan (if applicable)
- ☐ ☐ l) Stair systems

Materials shall include:

- ☐ ☐ a) Wood
- ☐ ☐ b) Steel
- ☐ ☐ c) Aluminum
- ☐ ☐ d) Concrete
- ☐ ☐ e) Plastic
- ☐ ☐ f) Glass (mfg. Listing for wind zone including details for installation and attachments
- ☐ ☐ g) Masonry
- ☐ ☐ h) Gypsum board and plaster

☐	☐	i) Insulating (mechanical)
☐	☐	j) Roofing (mfg. Listed system for wind zone with installation and attachments)
☐	☐	k) Insulation
		Accessibility Requirements shall include: Not required
☐	☐	a) Site requirements
☐	☐	b) Accessible route
☐	☐	c) Vertical accessibility
☐	☐	d) Toilet and bathing facilities
☐	☐	e) Drinking fountains
☐	☐	f) Equipment
☐	☐	g) Special occupancy requirements
☐	☐	h) Fair housing requirements
		Interior Requirements shall include:
☐	☐	a) Interior finishes (flame spread/smoke develop)
☐	☐	b) Light and ventilation
☐	☐	c) Sanitation
		Special Systems shall include:
☐	☐	a) Elevators
☐	☐	b) Escalators
☐	☐	c) Lifts
		Swimming Pools – Commercial – Plans shall be signed and sealed by a Professional Engineer registered in the State of Florida and approved by the Department of Business and Professional Regulation/Health Department Indicating compliance with the Florida Administrative Code, Chapter 64E-9 And Section 424 of the Florida Building Code
		Electrical:
☐	☐	a) Electrical wiring, services, feeders and branch circuits, over-current protection, grounding, wiring methods and materials, GFCIs
☐	☐	b) Equipment
☐	☐	c) Special Occupancies
☐	☐	d) Emergency Systems
☐	☐	e) Communication Systems
☐	☐	f) Low Voltage
☐	☐	g) Load calculations
☐	☐	h) Riser diagram
		Plumbing:
☐	☐	a) Minimum plumbing facilities
☐	☐	b) Fixture requirements
☐	☐	c) Water supply piping
☐	☐	d) Sanitary drainage
☐	☐	e) Water heaters
☐	☐	f) Vents
☐	☐	g) Roof drainage
☐	☐	h) Back flow prevention
☐	☐	i) Irrigation
☐	☐	j) Location of water supply
☐	☐	k) Grease traps
☐	☐	l) Environmental requirements
☐	☐	m) Plumbing riser

Mechanical:

- ☐ ☐ a) Energy calculation (signed and sealed by Architect or Engineer, registered in the State of Florida)
- ☐ ☐ b) Exhaust systems (clothes dryer exhaust, kitchen equipment exhaust, Specialty equipment exhaust)
- ☐ ☐ c) Equipment
- ☐ ☐ d) Equipment location
- ☐ ☐ e) Make-up air
- ☐ ☐ f) Roof mounted equipment
- ☐ ☐ g) Duct systems
- ☐ ☐ h) Ventilation
- ☐ ☐ i) Combustion air
- ☐ ☐ j) Chimneys, fireplaces and vents
- ☐ ☐ k) Appliances
- ☐ ☐ l) Boilers
- ☐ ☐ m) Refrigeration
- ☐ ☐ n) Bathroom ventilation
- ☐ ☐ o) Laboratory

Gas:

- ☐ ☐ a) Gas piping
- ☐ ☐ b) Venting
- ☐ ☐ c) Combustion air
- ☐ ☐ d) Chimney's and vents
- ☐ ☐ e) Appliances
- ☐ ☐ f) Type of gas
- ☐ ☐ g) Fireplaces
- ☐ ☐ h) LP tank locations
- ☐ ☐ i) Riser diagram/shut offs

☐ ☐ **Disclosure Statement for Owner Builders**

☐ ☐ ***Notice of Commencement Required Before Any Inspections will be Done**

☐ ☐ **Private Potable Water:**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used