

DATE 10/16/2008

Columbia County Building Permit

PERMIT

This Permit Must Be Prominently Posted on Premises During Construction

000027430

APPLICANT PATRICK WILSON PHONE 352 206-5459
ADDRESS 6800 SOUTHPOINT PARKWAY JACKSONVILLE FL 32216
OWNER MARONDA HOMES PHONE 904 296-1490
ADDRESS 353 SW MULBERRY DRIVE LAKE CITY FL 32055
CONTRACTOR THEODORE BROCK PHONE 904 296-1490
LOCATION OF PROPERTY 90W, TL ON 247S, TR ON 252B, TL ON TIMBER RIDGE,
TL ON MULBERRY DR., 1ST LOT ON LEFT
TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 154800.00
HEATED FLOOR AREA 2527.00 TOTAL AREA 3096.00 HEIGHT 22.00 STORIES 1
FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB
LAND USE & ZONING RSF-2 MAX. HEIGHT _____
Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 10-4S-16-02856-140 SUBDIVISION TIMBERLANDS
LOT 40 BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES 0.50

000001680 _____ CBC1256382 _____
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
CULVERT 08-597 BK HD Y
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: ELEVATION CONFIRMATION LETTER REQUIRED AT SLAB,

Check # or Cash 941616

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
date/app. by date/app. by date/app. by
Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
date/app. by date/app. by date/app. by
Framing _____ Rough-in plumbing above slab and below wood floor _____
date/app. by date/app. by
Electrical rough-in _____ Heat & Air Duct _____ Peri. beam (Lintel) _____
date/app. by date/app. by date/app. by
Permanent power _____ C.O. Final _____ Culvert _____
date/app. by date/app. by date/app. by
M/H tie downs, blocking, electricity and plumbing _____ Pool _____
date/app. by date/app. by
Reconnection _____ Pump pole _____ Utility Pole _____
date/app. by date/app. by date/app. by
M/H Pole _____ Travel Trailer _____ Re-roof _____
date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 775.00 CERTIFICATION FEE \$ 15.48 SURCHARGE FEE \$ 15.48
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ _____
FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 905.96
INSPECTORS OFFICE _____ CLERKS OFFICE _____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0809-52 Date Received 9/24 By JW Permit # 1680/27430
 Zoning Official BLK Date 26.09.08 Flood Zone X-100 Land Use RES Low Density Zoning RSF-2
 FEMA Map # N/A Elevation N/A MFE 74.0 ft River N/A Plans Examiner HD Date 9-25-08
 Comments Elevation Confirmation Letter Required at S/S
☐ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel # _____
☐ Dev Permit # _____ ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter
 IMPACT FEES: EMS \$29.88 Fire \$78.63 Corr \$409.16 Road/Code \$1,046.00 / 210
 School \$9,500.00 = TOTAL \$3,063.67

Septic Permit No. 08-0597 Fax (904) 332-10375
 Name Authorized Person Signing Permit Theodore Brock / Patrick Wilson Phone (904) 290-1490
 Address 10800 Southpoint Pkwy #300 Jacksonville, FL 32216
 Owners Name Maronda Homes Inc. of Florida Phone (904) 290-1490
 911 Address 353 SW Mulberry Drive Lake City, FL 32055
 Contractors Name Theodore C. Brock Phone (904) 290-1490
 Address 10800 Southpoint Pkwy #300 Jacksonville, FL 32216
 Fee Simple Owner Name & Address N/A
 Bonding Co. Name & Address N/A
 Architect/Engineer Name & Address Thomas Ponce 4005 Maronda Way Sanford, FL 32711
 Mortgage Lenders Name & Address Bank of America 250 Park Ave S #400 Winter Park, FL 32789

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 10-45-10-02850-140 Estimated Cost of Construction \$116,100.00
 Subdivision Name Timberlands Lot 40 Block _____ Unit _____ Phase _____
 Driving Directions Hwy 90, left on 247 South; Right on 252B; left on Timber Ridge, left on SW Mulberry Drive, 1st lot on left.

Number of Existing Dwellings on Property 0
 Construction of Residential Single Family Dwelling Total Acreage 1/2 Lot Size _____
 Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 22'
 Actual Distance of Structure from Property Lines - Front 50'0" Side 12'0" Side 4'0" Rear 46'0"
 Number of Stories 1 Heated Floor Area 2527 Total Floor Area 3096 Roof Pitch 12/6

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
 Page 1 of 2 (Both Pages must be submitted together.) 90596 Revised 1-10-08

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

Steve Hagg

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Theodore C. Brock

Contractor's License Number CBC1256382
Columbia County
Competency Card Number _____

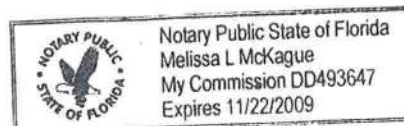
Affirmed under penalty of perjury to by the Contractor and subscribed before me this 22 day of July, 2008.

Personally known X or Produced Identification _____

State of Florida Notary Signature (For the Contractor)

Melissa L. McKague

SEAL:



This Instrument Prepared by and Return to :

Amy Wesp
SOUTHERN TITLE HOLDING
COMPANY, LLC.
3943 BAY MEADOWS ROAD
JACKSONVILLE, Florida 32217

as a necessary incident to the fulfillment of conditions
contained in a title insurance commitment issued by it.

Property Appraisers Parcel I.D. (Folio) Number(s):

R02856-000

Grantee(s) I.D.#(s):

File No: JX0812085

Inst: 200812010775 Date: 6/8/2008 Time: 1:05 PM
Doc Stamp-Deed: 6283.00
DC, P. DeWitt Cason, Columbia County Page 1 of 1 8:1151 P.2385

WARRANTY DEED
(CORPORATION)

This Warranty Deed Made this 27th day of May, 2008, by RML HOLDINGS INC., A FLORIDA CORPORATION, and having its place of business at 703 NW BLACKBERRY CIRCLE, LAKE CITY, Florida 32055, hereinafter called the grantor,

to MARONDA HOMES, INC. OF FLORIDA, A FLORIDA CORPORATION, whose post office address is: 11200 ST. JOHNS INDUSTRIAL PARKWAY, JACKSONVILLE, FLORIDA 32246, hereinafter called the grantee,

WITNESSETH: That said grantor, for and in consideration of the sum of ~~\$48,000~~ \$899,000.00 Dollars and other valuable considerations, receipt whereof is hereby acknowledged, by these presents grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz: LOTS 1, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 24, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, AND 41, OF TIMBERLANDS, PHASE 1, ACCORDING TO PLAT THEREOF AS RECORDED IN PLAT BOOK 9, PAGE 26 AND 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007, reservations, restrictions and easements of record, if any.

(Wherever used herein the terms "grantor" and "grantee" included all the parties to this instrument, and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation.)

In Witness Whereof, the Grantor has caused these presents to be executed in its name, and its corporate seal to be hereunto affixed, by its proper officers thereunto duly authorized, the day and year first above written.

Signed, sealed and delivered in our presence:

ATTEST:

Secretary

RML HOLDINGS INC.

Witness Signature:

Printed Name:

WORTH D. MORRIS
WORTH D. MORRIS

Witness Signature:

Printed Name:

Jody M. Coble
Jody M. Coble

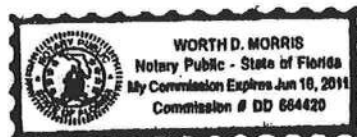
BY:

ROBERT R. LARDIZABAL, PRESIDENT

STATE OF FLORIDA
COUNTY OF DUVAL

The foregoing instrument was acknowledged before me this 28th day of May, 2008, by ROBERT R. LARDIZABAL as PRESIDENT of RML HOLDINGS INC., A FLORIDA CORPORATION, on behalf of the corporation. He/she is personally known to me or who has produced driver license(s) as identification.

My Commission Expires:



Printed Name: WORTH D. MORRIS
Notary Public
Serial Number



STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 08-0597

----- PART II - SITE PLAN -----

Scale: Each block represents 5 feet and 1 inch = 50 feet.

See
Attached
Site
Plan

Notes: _____

Site Plan submitted by: Patricia Wilson Signature

Plan Approved ☒ Not Approved ☐ Agent Title

By Mr. D. Zander Columbia County Health Department Date 9/11/08

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA

S.W. MULBERRY DRIVE

60' RIGHT-OF-WAY

1) MARIONDA HOMES

BUILDING SETBACK NOTE:
BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT 25',
REAR 15', SIDE 10'

BENCHMARK NOTE:

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BROTT SURVEYING (PLANNING SUPERVISOR) DATUM UNKNOWN.

SURVEYOR NOTES:

- 2) ALL UTILITIES AND OR MAN-MADE FEATURES, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR, THE BOUNDARY OF THE SUBJECT PROPERTY IS SHOWN AS BEING THE SAME AS THE DISCREPANCY AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING, STAKE, LINES DERIVED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO APPROVAL BY THE CITY OF CHICAGO, ILL. IN ORDER TO BE CONSIDERED AS A PART OF ANY NEW CONSTRUCTION. THE CONTRACTED FOR THE CURRENT STAKE REQUIREMENTS.
- 5) THE MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN DERIVED FROM THIS SURVEYOR, UNLESS OTHERWISE DERIVED HEREON.

CURVE TABLE

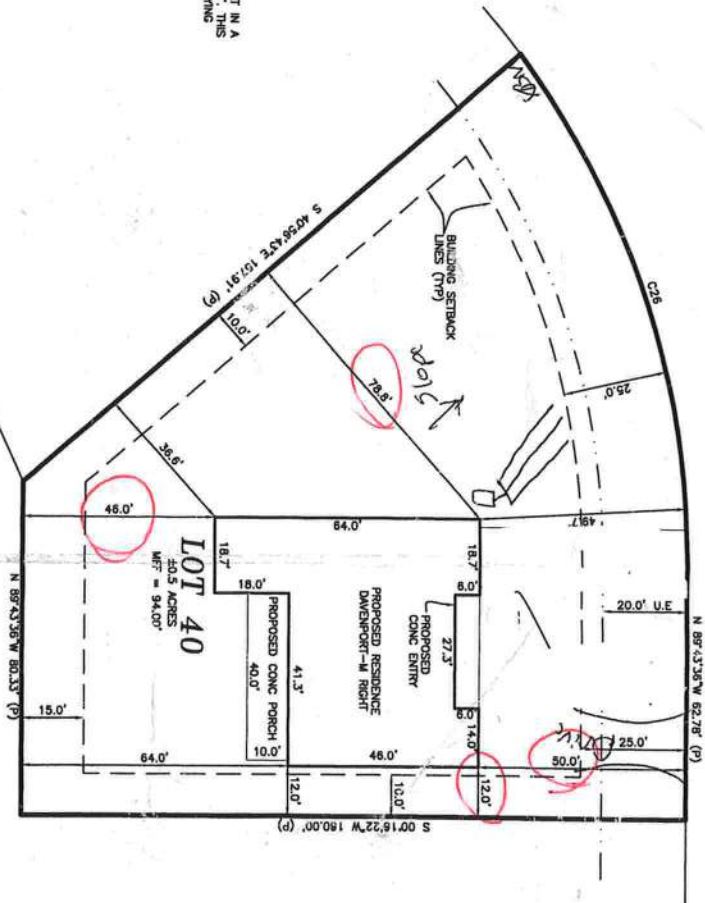
CURVE	RADIUS	TANGENT	LENGTH	DELTA	CHORD	CHORD BEARING
C25	200.00'	67.79'	130.72'	37°26'54.7"	128.41'	N 71°32'57" E

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 1200707-0175-B, DATED 1-6-89, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SHOWN ON THE FLOOD INSURANCE RATE MAP. THE FEDERAL FLOOD INSURANCE MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURETY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURETOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.



2	= FOUNDATION	2	= FOUNDATION
1	= IDENTIFICATION	1	= IDENTIFICATION
0	= FOUND 1/2" REBAR & CAP	0	= FOUND 1/2" REBAR & CAP
0	= SET 1/2" REBAR & CAP	0	= SET 1/2" REBAR & CAP
0	= FOUND 3/4" IRON PIPE	0	= FOUND 3/4" IRON PIPE
0	= FOUND 4" CONC. MON.	0	= FOUND 4" CONC. MON.
0	= NO IDENTIFICATION	0	= NO IDENTIFICATION
0	= SET 4" CONC. MON.	0	= SET 4" CONC. MON.
X	= SET NAIL & DISK P.S.M. 5582	X	= SET NAIL & DISK P.S.M. 5582
X	= FOUND NAIL & DISK	X	= FOUND NAIL & DISK
0	= FOUND 6" x 6" S.R.D.	0	= FOUND 6" x 6" S.R.D.
0	= R/O MON.	0	= R/O MON.
0	= CAT RISE	0	= CAT RISE
0	= TELEPHONE PEDISTAL	0	= TELEPHONE PEDISTAL
0	= WOOD POWER POLE	0	= WOOD POWER POLE
0	= AIR CONDITONER	0	= AIR CONDITONER
0	= CALCULATED FROM MEASURED	0	= CALCULATED FROM MEASURED
0	= CABLE TELEVISION	0	= CABLE TELEVISION
0	= CONCRETE SLAB	0	= CONCRETE SLAB
0	= CONCRETE FLOOR	0	= CONCRETE FLOOR
0	= CONCRETE FOUNDATION	0	= CONCRETE FOUNDATION
0	= CONCRETE	0	= CONCRETE
0	= ELECTRIC	0	= ELECTRIC
0	= ELEVATION	0	= ELEVATION
0	= FENCE	0	= FENCE
0	= FLOOR	0	= FLOOR
0	= LICENSED SURVEYOR BUSINESS	0	= LICENSED SURVEYOR BUSINESS
0	= FIELD MEASURED	0	= FIELD MEASURED
0	= MANHOLE	0	= MANHOLE
0	= PLUMBING UTILITIES	0	= PLUMBING UTILITIES
0	= PLAY BOOK	0	= PLAY BOOK
0	= PLUMB UTILITIES EASEMENT	0	= PLUMB UTILITIES EASEMENT
0	= TOPSOIL	0	= TOPSOIL
0	= TYPICAL	0	= TYPICAL
0	= WATER	0	= WATER
0	= WATER LETTER	0	= WATER LETTER
0	= WATER VALVE	0	= WATER VALVE

[illegible]

THIS IS NOT A BOUNDARY SURVEY
CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 58-617, FLORIDA ADMINISTRATIVE CODE.

DATE: 8/14/08

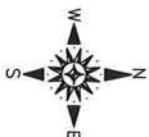


BRINKMAN SURVEYING & MAPPING INC.
4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'	THE BENCHMARK IN QUALITY SERVICE™	DRAWN BY: ZL
DATE: 8/14/08		CHECKED BY: J.B.

PREPARED FOR: MARONDA	DRAWING NUMBER 157-08
-----------------------	--------------------------

LOT FOURTY (40) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA



IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA

S.W. MULBERRY DRIVE
60' RIGHT-OF-WAY



1) MARONDA HOMES

BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT 25',
REAR 15', SIDE 10'

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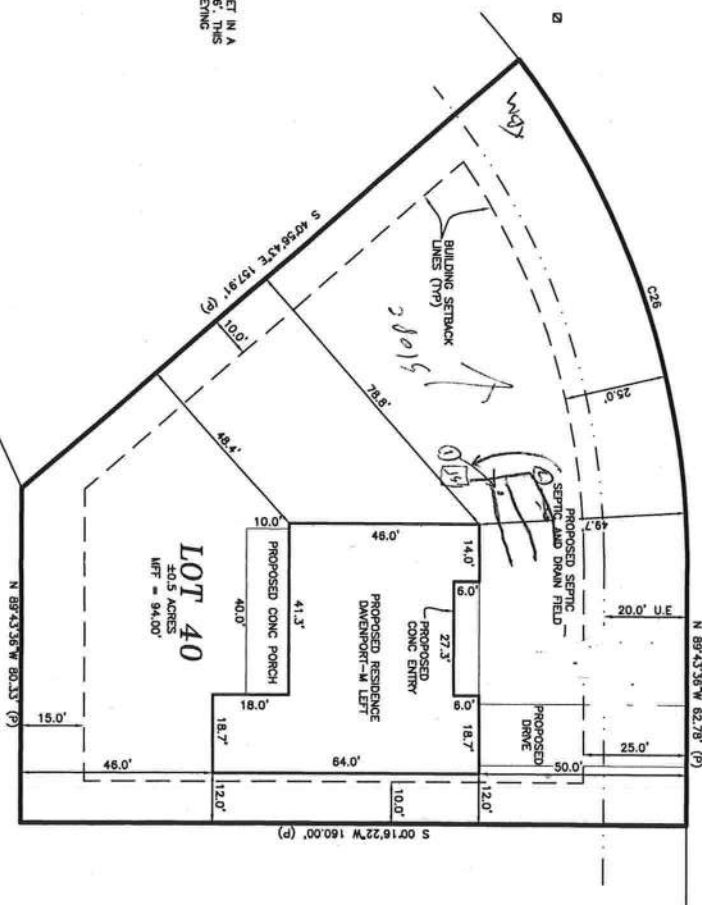
- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE PRIOR TO ANY NEW CONSTRUCTION. THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).

6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.



CURVE TABLE				
CURVE	RADIUS	TANGENT	LENGTH	DELTA
C26	200.00'	67.79'	130.72'	37°26'54.7"
				CHORD
				128.41'
				CHORD BEARING
				N 71°32'57" E

IN THE OPINION OF THIS SURETOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE 1.

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FILL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

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CERTIFICATE OF SURVEYOR:**

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I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HERON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 417.22, FLORIDA STATUTES, AND CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE.

BY: _____
DATE: _____

DATE _____



BRINKMAN SURVEYING & MAPPING INC.

PHONE: (352) 374-7707 FAX: (352) 374-8757

FAX: (352) 374-8757

SCALE: 1" = 30'	"THE BENCHMARK IN QUALITY SERVICE"	DRAWN BY: ZL
DATE: 8/14/08		CHECKED BY: J.B.

1000

CHECKED BY: J.B.

PREPARED FOR: MARONDA

DRAWING NUMBER
167-08

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

401 TM

Project Name: **DAVENPORT GAINESVILLE**
Address: **353 SW Mulberry Dr**
City, State: **Lake City, FL 32055**
Owner: **ELECTRIC**
Climate Zone: **North**

Builder: **MARONDA HOMES**
Permitting Office: **Columbia**
Permit Number: **27430**
Jurisdiction Number: **221000**

1. New construction or existing New ☐
2. Single family or multi-family Single family ☐
3. Number of units, if multi-family 1 ☐
4. Number of Bedrooms 4 ☐
5. Is this a worst case? Yes ☐
6. Conditioned floor area (ft²) 2527 ft² ☐
7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default)
 - a. U-factor: Description Area
 - (or Single or Double DEFAULT) 7a (Single Default) 237.0 ft² ☐
 - b. SHGC:
 - (or Clear or Tint DEFAULT) 7b. (Clear) 237.0 ft² ☐
8. Floor types
 - a. Slab-On-Grade Edge Insulation R=0.0, 211.0(p) ft ☐
 - b. N/A ☐
 - c. N/A ☐
9. Wall types
 - a. Concrete, Int Insul, Exterior R=4.1, 1173.0 ft² ☐
 - b. Frame, Wood, Adjacent R=13.0, 326.0 ft² ☐
 - c. N/A ☐
 - d. N/A ☐
 - e. N/A ☐
10. Ceiling types
 - a. Under Attic R=19.0, 2650.0 ft² ☐
 - b. N/A ☐
 - c. N/A ☐
11. Ducts
 - a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 250.0 ft ☐
 - b. N/A ☐

12. Cooling systems
 - a. Central Unit Cap: 47.5 kBtu/hr ☐
 - SEER: 13.00 ☐
 - b. N/A ☐
 - c. N/A ☐
13. Heating systems
 - a. Electric Heat Pump Cap: 47.5 kBtu/hr ☐
 - HSPF: 8.00 ☐
 - b. N/A ☐
 - c. N/A ☐
14. Hot water systems
 - a. Electric Resistance Cap: 50.0 gallons ☐
 - EF: 0.90 ☐
 - b. N/A ☐
 - c. Conservation credits ☐
 - (HR-Heat recovery, Solar
 - DHP-Dedicated heat pump)
15. HVAC credits PT, ☐
 - (CF-Ceiling fan, CV-Cross ventilation,
 - HF-Whole house fan,
 - PT-Programmable Thermostat,
 - MZ-C-Multizone cooling,
 - MZ-H-Multizone heating)

Glass/Floor Area: 0.09

Total as-built points: 31338
Total base points: 32964

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Wayne Campbell

DATE: 07/12/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Melissa Mykane

DATE: 07/12/08

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT										
GLASS TYPES														
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt			Area X SPM X SOF = Points						
.18	2527.0	18.59	8456.0	1.Single, Clear	W	1.0	6.0	71.0	43.84	0.97	3020.0			
				2.Single, Clear	S	1.0	6.0	16.0	40.81	0.94	614.0			
				3.Single, Clear	E	1.0	6.0	78.0	47.92	0.97	3618.0			
				4.Single, Clear	W	1.0	8.0	40.0	43.84	0.99	1734.0			
				5.Single, Clear	N	1.0	6.0	32.0	21.73	0.97	677.0			
				As-Built Total:			237.0			9663.0				
WALL TYPES Area X BSPM = Points				Type	R-Value			Area X SPM = Points						
Adjacent	326.0	0.70	228.2	1. Concrete, Int Insul, Exterior		4.1	1173.0	1.13	1331.4					
Exterior	1173.0	1.70	1994.1	2. Frame, Wood, Adjacent		13.0	326.0	0.60	195.6					
Base Total:				1499.0			2222.3			As-Built Total:			1499.0	1527.0
DOOR TYPES Area X BSPM = Points				Type				Area X SPM = Points						
Adjacent	18.0	2.40	43.2	1.Exterior Wood			20.0	6.10	122.0					
Exterior	38.0	6.10	231.8	2.Adjacent Wood			18.0	2.40	43.2					
				3.Exterior Wood			18.0	6.10	109.8					
Base Total:				56.0			275.0			As-Built Total:			56.0	275.0
CEILING TYPES Area X BSPM = Points				Type	R-Value			Area X SPM X SCM = Points						
Under Attic	2527.0	1.73	4371.7	1. Under Attic		19.0	2650.0	2.34 X 1.00	6201.0					
Base Total:				2527.0			4371.7			As-Built Total:			2650.0	6201.0
FLOOR TYPES Area X BSPM = Points				Type	R-Value			Area X SPM = Points						
Slab	211.0(p)	-37.0	-7807.0	1. Slab-On-Grade Edge Insulation		0.0	211.0(p)	-41.20	-8693.2					
Raised	0.0	0.00	0.0											
Base Total:				-7807.0			As-Built Total:			211.0			-8693.2	
INFILTRATION Area X BSPM = Points							Area X SPM = Points							
2527.0 10.21 25800.7							2527.0 10.21 25800.7							

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 33318.7				Summer As-Built Points: 34773.4						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
33318.7	0.3250		10828.6	(sys 1: Central Unit 47500btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 34773 1.00 (1.08 x 1.147 x 0.86) 0.260 0.950 9206.6 34773.4 1.00 1.072 0.260 0.950 9206.6						

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2527.0	20.17	9175.0	1.Single, Clear	W	1.0	6.0	71.0	28.84	1.01	2063.0
				2.Single, Clear	S	1.0	6.0	16.0	20.24	1.03	332.0
				3.Single, Clear	E	1.0	6.0	78.0	26.41	1.02	2093.0
				4.Single, Clear	W	1.0	8.0	40.0	28.84	1.00	1157.0
				5.Single, Clear	N	1.0	6.0	32.0	33.22	1.00	1063.0
				As-Built Total:							237.0
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	326.0	3.60	1173.6	1. Concrete, Int Insul, Exterior		4.1	1173.0	6.42	7530.7		
Exterior	1173.0	3.70	4340.1	2. Frame, Wood, Adjacent		13.0	326.0	3.30	1075.8		
Base Total:		1499.0	5513.7	As-Built Total:		1499.0		8606.5			
DOOR TYPES Area X BWPM = Points				Type	Area X WPM = Points						
Adjacent	18.0	11.50	207.0	1.Exterior Wood			20.0	12.30	246.0		
Exterior	38.0	12.30	467.4	2.Adjacent Wood			18.0	11.50	207.0		
				3.Exterior Wood			18.0	12.30	221.4		
Base Total:		56.0	674.4	As-Built Total:		56.0		674.4			
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	2527.0	2.05	5180.4	1. Under Attic		19.0	2650.0	2.70 X 1.00	7155.0		
Base Total:		2527.0	5180.4	As-Built Total:		2650.0		7155.0			
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	211.0(p)	8.9	1877.9	1. Slab-On-Grade Edge Insulation		0.0	211.0(p)	18.80	3966.8		
Raised	0.0	0.00	0.0								
Base Total:			1877.9	As-Built Total:		211.0		3966.8			
INFILTRATION Area X BWPM = Points				Area X WPM = Points							
	2527.0	-0.59	-1490.9				2527.0	-0.59	-1490.9		

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Winter Base Points:		20930.4		Winter As-Built Points:			25619.7			
Total Winter Points	X System Multiplier	=	Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Heating Points
				(sys 1: Electric Heat Pump 47500 btuh ,EFF(8.0) Ducts:Unc(S),Con(R),Int(AH),R6.0						
20930.4	0.5540		11595.5	25619.7	1.000	(1.060 x 1.169 x 0.88)	0.426	0.950		11357.7
				25619.7	1.00	1.095	0.426	0.950		11357.7

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit	= Total
4		2635.00	10540.0	50.0	0.90	4		1.00	2693.56	10774.2
				As-Built Total:						10774.2

CODE COMPLIANCE STATUS													
BASE					AS-BUILT								
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
10829		11595		10540		32964	9207		11358		10774		31338

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.7

The higher the score, the more efficient the home.

ELECTRIC, , , ,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 47.5 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	2527 ft ²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 47.5 kBtu/hr
(or Single or Double DEFAULT) 7a(Sngle Default)	237.0 ft ²		HSPF: 8.00
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT) 7b. (Clear)	237.0 ft ²	c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 211.0(p) ft	a. Electric Resistance	Cap: 50.0 gallons
b. N/A			EF: 0.90
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Concrete, Int Insul, Exterior	R=4.1, 1173.0 ft ²	(HR-Heat recovery, Solar	
b. Frame, Wood, Adjacent	R=13.0, 326.0 ft ²	DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits	PT,
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=19.0, 2650.0 ft ²	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 250.0 ft			
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Melinda Morgan Date: 07/22/08

Address of New Home: 353 SW Mulberry Drive City/FL Zip: Lake City, FL 32055



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5.2)



Project Summary
Entire House
Maronda Homes

Job:
Date:
By: G. Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-3913 Fax: 407-321-0064

Project Information

For: DAVENPORT

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Heating Summary

Structure	34215 Btuh
Ducts	5655 Btuh
Central vent (100 cfm)	4048 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	43918 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft ²)	2527	2527
Volume (ft ³)	20216	20216
Air changes/hour	0.32	0.16
Equiv. AVF (cfm)	108	54

Heating Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Model N4H348AKA

Efficiency	8.3 HSPF
Heating input	
Heating output	0 Btuh @ 47°F
Temperature rise	0 °F
Actual air flow	0 cfm
Air flow factor	0.000 cfm/Btuh
Static pressure	0.60 in H2O
Space thermostat	

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Sensible Cooling Equipment Load Sizing

Structure	19692 Btuh
Ducts	12208 Btuh
Central vent (100 cfm)	1860 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	32747 Btuh

Latent Cooling Equipment Load Sizing

Structure	1895 Btuh
Ducts	2875 Btuh
Central vent (100 cfm)	3516 Btuh
Equipment latent load	8286 Btuh
Equipment total load	41034 Btuh
Req. total capacity at 0.76 SHR	3.6 ton

Cooling Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Cond N4H348AKA
Coil FSM4X4800A

Efficiency	13 SEER
Sensible cooling	35340 Btuh
Latent cooling	11160 Btuh
Total cooling	46500 Btuh
Actual air flow	1460 cfm
Air flow factor	0.046 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.80

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



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Right-Suite Residential 6.0.90 RSR21115

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Building Analysis Entire House Maronda Homes

Job:
Date:
By: G. Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-3913 Fax: 407-321-0064

Project Information

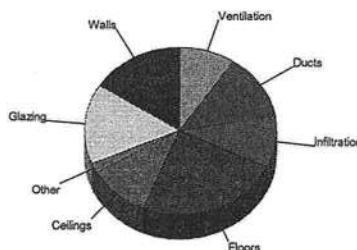
For: DAVENPORT

Design Conditions

Location:		Indoor:		Heating	Cooling
Gainesville, FL, US		Indoor temperature (°F)		70	75
Elevation: 151 ft		Design TD (°F)		37	17
Latitude: 30°N		Relative humidity (%)		30	50
		Moisture difference (gr/lb)		10.6	52.0
Outdoor:		Heating	Cooling		
Dry bulb (°F)		33	92		
Daily range (°F)		-	19 (M)		
Wet bulb (°F)		-	77		
Wind speed (mph)		15.0	7.5		
		Infiltration:			
		Method		Simplified	
		Construction quality		Average	
		Fireplaces		0	

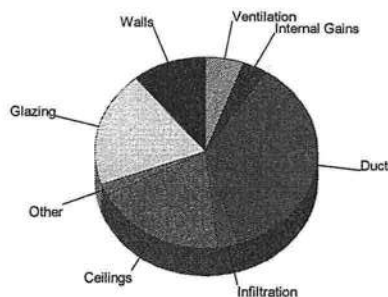
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.4	7021	16.0
Glazing	47.0	6814	15.5
Doors	14.4	808	1.8
Ceilings	1.8	4607	10.5
Floors	17.4	10602	24.1
Infiltration	2.4	4364	9.9
Ducts		5655	12.9
Piping		0	0.0
Humidification		0	0.0
Ventilation		4048	9.2
Adjustments		0	0.0
Total		43918	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.3	3684	10.9
Glazing	45.6	6613	19.6
Doors	11.4	637	1.9
Ceilings	2.6	6555	19.4
Floors	0.0	0	0.0
Infiltration	0.6	1003	3.0
Ducts		12208	36.2
Ventilation		1860	5.5
Internal gains		1200	3.6
Blower		0	0.0
Adjustments		0	0.0
Total		33760	100.0



Overall U-value = 0.163 Btuh/ft²-°F

Data entries checked.



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Right-Suite Residential 6.0.90 RSR21115

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Duct System Summary

Entire House

Maronda Homes

Job:
Date:
By: G. Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-3913 Fax: 407-321-0064

Project Information

For: DAVENPORT

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.46 / 0.14 in H2O	0.46 / 0.14 in H2O
Lowest friction rate	0.100 in/100ft	0.100 in/100ft
Actual air flow	1460 cfm	1460 cfm
Total effective length (TEL)	26 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER BDR	h 626.5	240	240	0.100	8	x0	VIFx	5.0	0.0	st5
MASTER BATH	h 294.7	75	75	0.100	6	x0	VIFx	10.0	0.0	st5
MASTER TOILET	h 111.6	25	25	0.100	4	x0	VIFx	20.0	0.0	st5
LIVING ROOM	h 310.7	155	155	0.100	7	x0	VIFx	5.0	0.0	st4
FAMILY ROOM	c 413.9	165	165	0.100	7	x0	VIFx	5.0	0.0	st4
DINING ROOM	h 434.1	125	125	0.100	6	x0	VIFx	5.0	0.0	st1
NOOK	h 265.3	125	125	0.100	6	x0	VIFx	20.0	0.0	st1
KITCHEN	c 262.7	100	100	0.100	6	x0	VIFx	10.0	0.0	st1
BDR #4	h 325.2	130	130	0.100	6	x0	VIFx	10.0	0.0	ST2
bDR #3	h 198.4	90	90	0.100	6	x0	VIFx	5.0	0.0	ST2
BATH	h 86.7	20	20	0.100	4	x0	VIFx	15.0	0.0	ST2
BDR #2	c 410.9	160	160	0.100	7	x0	VIFx	15.0	0.0	ST2
UTILITY	h 90.7	50	50	0.100	4	x0	VIFx	10.0	0.0	ST3

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	101.0	101.0	0.100	590	16	0 x 0	VinIFlx	
ST2	Peak AVF	400	400	0.100	517	10	0 x 0	VinIFlx	
ST3	Peak AVF	50	50	0.100	284	4	0 x 0	VinIFlx	
st5	Peak AVF	660	660	0.100	562	14	0 x 0	VinIFlx	st4
st4	Peak AVF	340	340	0.100	683	12	0 x 0	VinIFlx	st1

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1460	1460	6.0	2.308	669	20	x0 0		VIFx	

Columbia County Building Department Culvert Permit

Culvert Permit No.
000001680

DATE 10/16/2008 PARCEL ID # 10-4S-16-02856-140

APPLICANT PATRICK WILSON PHONE 352 206-5459

ADDRESS 6800 SW SOUTHPOINT PARKWAY JACKSONVILLE FL 32216

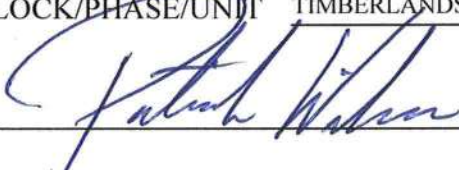
OWNER MARONDA HOMES PHONE 904 296-1490

ADDRESS 353 SW MULBERRY DRIVE LAKE CITY FL 32055

CONTRACTOR THEODORE BROCK PHONE 904 296-1490

LOCATION OF PROPERTY 90W, TL ON 247 S, TR ON 252B, TL ON TIMBER RIDGE, TL ON
MULBERRY DRIVE, 1ST LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBERLANDS 40

SIGNATURE 

INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALLATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



NOTICE OF COMMENCEMENT

Tax Parcel Identification Number 10-4S-10-62850-140

County Clerk's Office Stamp or Seal

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): 401 Timberlands
a) Street (job) Address: 353 SW Midway Drive
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
a) Name and address: Maronda Homes Inc of FL 10800 Southpoint Pkwy #300 Jax FL 32216
b) Name and address of fee simple titleholder (if other than owner):
c) Interest in property:
4. Contractor Information
a) Name and address: Maronda Homes Inc of FL 10800 Southpoint Pkwy #300 Jax FL 32216
b) Telephone No.: (904) 296-1490 Fax No. (Opt.): (904) 332-0375
5. Surety Information
a) Name and address:
b) Amount of Bond:
c) Telephone No.: Fax No. (Opt.):
6. Lender
a) Name and address:
b) Phone No.:
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
a) Name and address: Southern Title Holding Co LLC 3945 Baymeadows Rd Jax FL 32217
b) Telephone No.: (904) 739-2205 Fax No. (Opt.):
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b), Florida Statutes:
a) Name and address:
b) Telephone No.: Fax No. (Opt.):
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager
Theodore C. Brock
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 22 day of July, 20 08, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney
fact) for Maronda Homes Inc of Florida (name of party on behalf of whom instrument was executed).

Personally Known ☒ OR Produced Identification Type

Notary Signature [Signature] Notary Stamp or Seal:

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
Signature of Natural Person Signing (in line #10 above.)

Maronda Systems

Maronda Systems 4005 Maronda Way

Sanford FL 32771

(407) 321-0064

Fax (407) 321-3913

Engineer/Architect of Record:

Design Criteria: TPI Design: Matrix Analysis MiTek software

PLAN JOB #	LOT	ADDRESS	DIV/SUB	MODEL
9TM04001	40-1	353 SW MULBERRY DR	JAX-9TM	DVPM4 RIGHT

DAVENPORT A-M

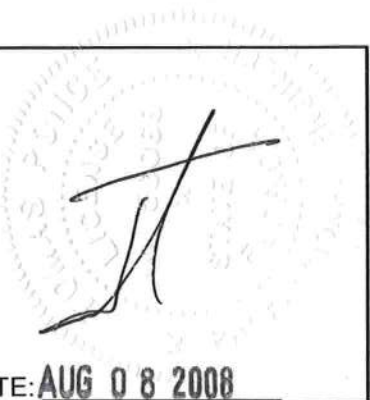
This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone.

Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek/Trenco.

I, the Architect/Engineer of Record for the above referenced lot Have reviewed the package and confirmed that it matches the physical and structural Parameters found on the set of permit drawings.

Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. Dwgs:	46
Layout	10/18/07		S1	11/13/07		Roof Loads-	
V	07/27/05		T1	11/13/07		TC Live:	16.0 psf
HIP	11/02/06		T2	11/13/07		TC Dead:	7.0 psf
B1	11/13/07		V1S	11/13/07		BC Live:	10.0 psf
BGRD1	01/10/08		V2S	11/13/07		BC Dead:	10.0 psf
G1	11/13/07		V3S	11/13/07		Total	43.0 psf
G2	11/13/07		V4S	11/13/07			
GRD1	11/13/07		V6	11/13/07		DurFac- Lbr:	1.25
GRD2	11/13/07		V7	11/13/07		DurFac- Plt:	1.25
H1S	11/13/07		V8	11/13/07		O.C. Spacing:	24.0"
H2S	11/13/07						
H3S	11/13/07						
H4S	11/13/07						
H5S	11/13/07						
H6	11/13/07						
HGRD1	11/13/07						
HS1	11/13/07						
HS2	11/13/07						
HS3	11/13/07						
HS4	11/13/07						
HS5	11/13/07						
HS6S	11/13/07						
HS7S	11/13/07						
HS8S	11/13/07						
HSGRD1S	11/13/07						
J	11/13/07						
J1	11/13/07						
J2	11/13/07		INV #	DESC	QNTY		
J3	11/13/07		50060.0048	THD26	15		
JGRD1	11/13/07		50060.0047	THD28			
JS	11/13/07		50060.0050	THDH28-3	1		
JS1	11/13/07		50060.0110	JUS26			
JS2	11/13/07		50060.0058	THJ26	3		
JS3	11/13/07		50060.0049	THD28-2	2		
JSA	11/13/07						
JSGRD1	11/13/07						
			SEAT PLATES		150		



HARDWARE LEGEND

- 1 HUS26
- 2 HUS28
- 3 JUS26
- 4 MP6F
- 5 MPA1 & MPA1F
- 6 SKH26 L/R
- 7 SKH26 L/R
- 8 SUS26
- 9 SUS28
- 10 THD26
- 11 THD28
- 12 THD28-2
- 13 THD28-3
- 14 THD48
- 15 THU26**
- 16 LTW12

HARDWARE MANUFACTURED
BY USP
* HARDWARE MANUFACTURED
BY SIMPSON
* HARDWARE MANUFACTURED
BY CLEVELAND

AUG 08 2008

DAVENPORT ELEVATION "A" "M"-FL

GARAGE : RIGHT

Maronda Homes

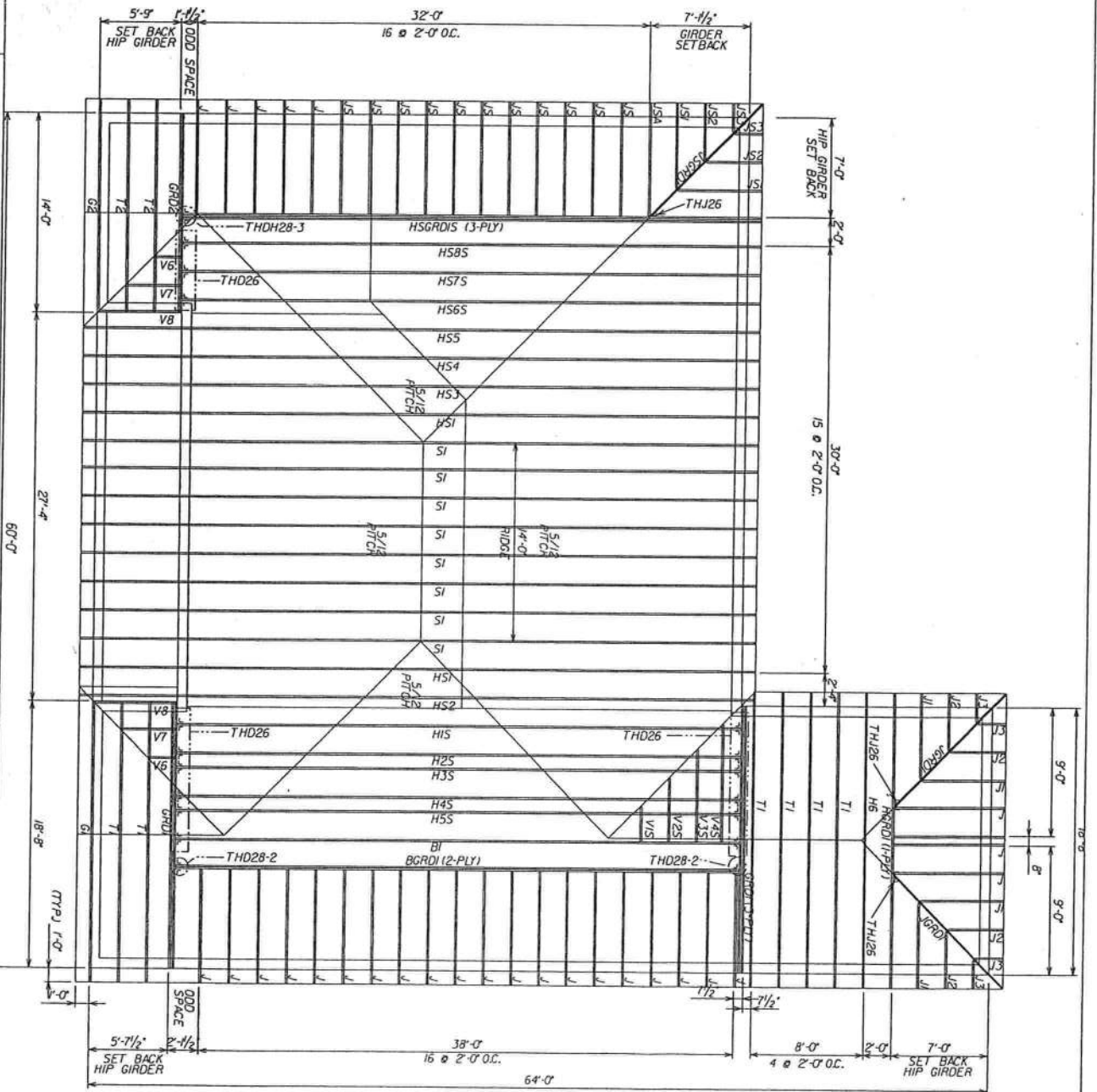
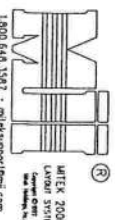
Copyright © 2007 MARONDA HOMES
(407) 321-0064 4005 MARONDA WAY SANFORD, FL 32010A

DESIGNER: WCA
CHECKER: MIKE

LOADING-FBC2004/TP12002

SCALE: 1/8" = 1'-0"
DATE: 10/18/2007

TC LIVE	16.00	SNOW LOAD	0.00
TC DEAD	7.00	LUMBER DOL	1.25
BC LIVE	10.00	PLATE DOL	1.25
BC DEAD	10.00	WIND	1.25
TOTAL	43.00	SPACING	2'-0"





Trenco

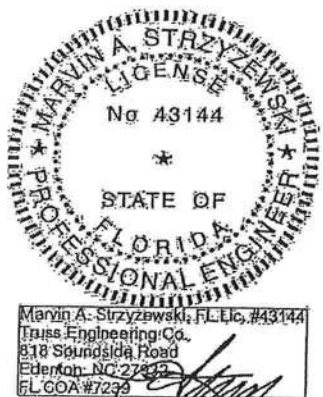
818 Soundside Rd
Edenton, NC 27932

Re: DAVENPORT
DAVENPORT_125

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Pages or sheets covered by this seal: E4513785 thru E4513864

My license renewal date for the state of Florida is February 28, 2009.



November 13, 2007

Strzyzewski, Marvin

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.
Engineering services provided by Truss Engineering Company.

RE: DAVENPORT - DAVENPORT_125

Trenco

818 Soundside Rd
Edenton, NC 27932

Site Information:

Project Customer: Maronda Homes Project Name: Maronda Homes
Lot/Block: Sanford Subdivision: Sanford
Address: Sanford
City: Sanford State: Florida

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 7.0
Wind Code: ASCE 7-02 Wind Speed: 125 mph Floor Load: N/A psf
Roof Load: 43.0 psf

This package includes 1 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date
1	E4606947	BGRD1	1/10/08

The truss drawing(s) referenced above have been prepared by
TRENCO under my direct supervision based on the parameters
provided by Maronda Homes-Pittsburgh, PA.

Truss Design Engineer's Name: Strzyzewski, Marvin
My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of
professional engineering responsibility solely for the truss
components shown. The suitability and use of this component
for any particular building is the responsibility of the building
designer, per ANSI/TPI-1 Chapter 2.



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 10, 2008

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

Warning! The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

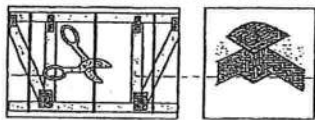
Warning! Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

NOTAS GENERALES

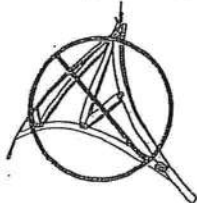
Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arrioste Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.



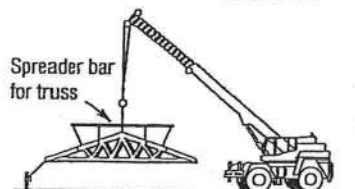
HANDLING — MANEJO

Warning! Avoid lateral bending. — Evite la flexión lateral.



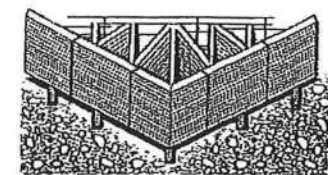
Warning! Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



Warning! Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



Warning! If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

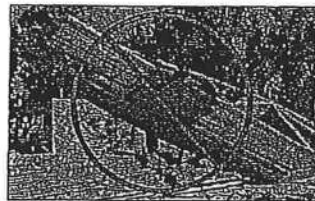
Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostres de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenamiento de los trusses en área de trabajo.

Warning! Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



Warning! Do not store on uneven ground.

No almacene en tierra desigual.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES

RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

Warning! Don't overload the crane. ¡Advertencia! No sobrecargue la grúa!

Warning! Never use banding alone to lift a bundle. Do not lift a group of individually banded bundles.

Nunca use sólo los empaques para levantar un paquete. No levante un grupo de empaques individuales.

Warning! A single lift point may be used for bundles with trusses up to 45'.

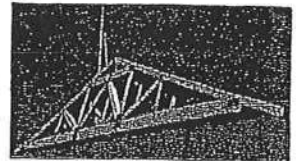
Two lift points may be used for bundles with trusses up to 60'.

Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.

Puede usar dos puntos de levantar para paquetes más de 60 pies.

Use por lo menos tres puntos de levantar para paquetes más de 60 pies.



Warning! Do not over load supporting structure with truss bundle.

¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

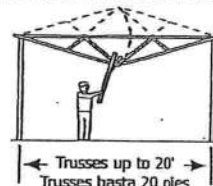
Warning! Place truss bundles in stable position. Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND

INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

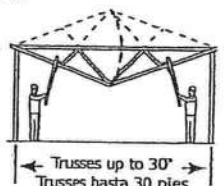
Warning! Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



Warning! Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALES

Warning! Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalada y el truss está asegurado en los soportes.

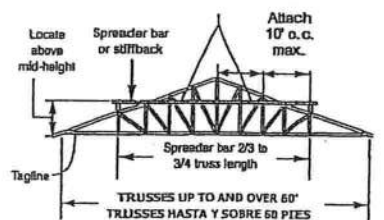
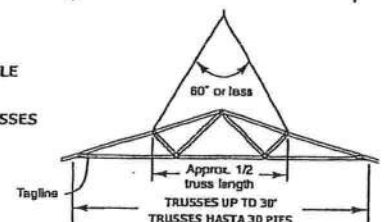
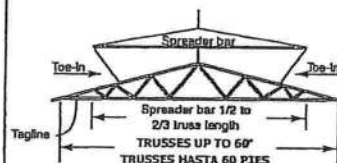
Warning! Using a single pick-point at the peak can damage the truss.

¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



TEMPORARY RESTRAINT & BRACING

RESTRICCIÓN Y ARRIOSTRE TEMPORAL

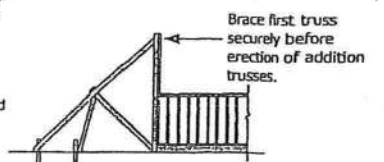
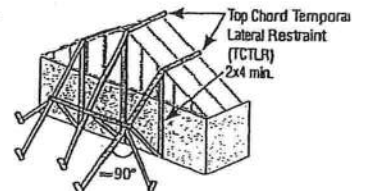
Warning! Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resumen BCSI B2 - Restricción/Arrioste Temporal y Instalación de los Trusses para más información.

Warning! Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).

Warning! Do not walk on unbraced trusses. No camine en trusses sueltos.



STEPS TO SETTING TRUSSES

AS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

- 1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estabilizar los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

- Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resumen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES

RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

- This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

TOP CHORD - CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCLTR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30'	10' o.c. max.
Hasta 30 pies	10 pies máximo
30' to 45'	8' o.c. max.
30 a 45 pies	8 pies máximo
45' to 60'	6' o.c. max.
45 a 60 pies	6 pies máximo
60' to 80'*	4' o.c. max.
60 a 80 pies*	4 pies máximo

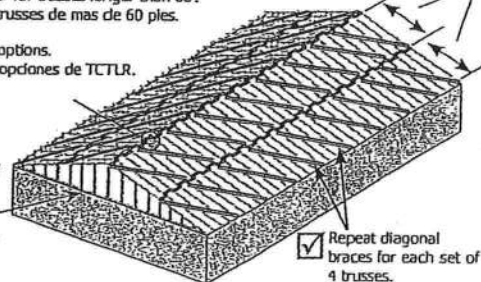
*Consult a Professional Engineer for trusses longer than 60'.

*Consulte a un Ingeniero para trusses de mas de 60 pies.

- See BCSI-B2 for TCLTR options.
Vea el BCSI-B2 para las opciones de TCLTR.

- Refer to BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for Gable End Frame restraint/bracing/reinforcement information.

Para información sobre restricción/arriostre/refuerzo para armazón de hastial vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.

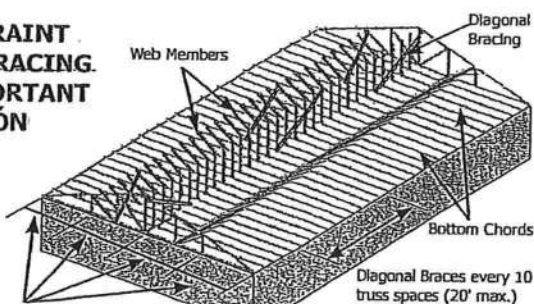


Ground bracing not shown for clarity.

- Repeat diagonal braces for each set of 4 trusses.
Repita los arriostres diagonales para cada grupo de 4 trusses.

WEB MEMBER PLANE - PLANO DE LOS MIEMBROS SECUNDARIOS

LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!



10'-15' max. Same spacing as bottom chord Lateral Restraint

Some chord and web members not shown for clarity.

BOTTOM CHORD - CUERDA INFERIOR

Lateral Restraints - 2x4x12' or greater lapped over two trusses.

Bottom chords

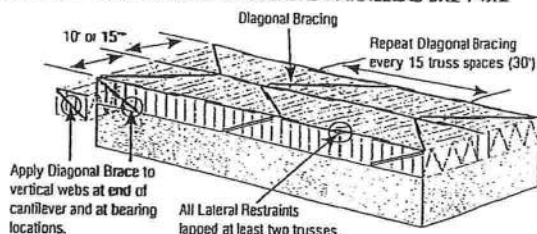
Diagonal Braces every 10 truss spaces (20' max.)

Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

- Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses for more information.

Vea el resumen BCSI-B7 - Restricción y Arriostre Temporal y Permanente de Trusses de Cuerdas Paralelas para más información.



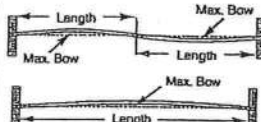
Apply Diagonal Brace to vertical webs at end of cantilever and at bearing locations.

All Lateral Restraints lapped at least two trusses.

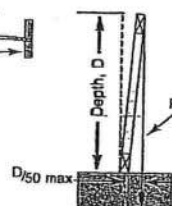
*Top chord Temporary Lateral Restraint spacing shall be 10' o.c. max. for 3x2 chords and 15' o.c. for 4x2 chords.

INSTALLING - INSTALACION

- Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



- Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.



D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥8'

Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥33.3'

CONSTRUCTION LOADING - CARGA DE CONSTRUCCION

- Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

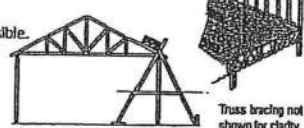
- Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.

Material	Height
Gypsum Board	12"
Plywood or OSB	16"
Asphalt Shingles	2 bundles
Concrete Block	8"
Clay Tile	3-4 ties high



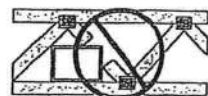
- Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.
- Never stack materials near a peak.
Nunca amontone material cerca del pico.
- Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.
- Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.



ALTERATIONS - ALTERACIONES

- Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.
Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

- Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.
No corte, altere o perforo ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.



- Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the Contractor. Thus, WITCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.



6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbciindustry.com



TRUSS PLATE INSTITUTE
218 N. Lee St., Ste. 312 • Alexandria, VA 22314
703/683-1010 • www.tpinst.org

B3WARN11x17 20061115

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

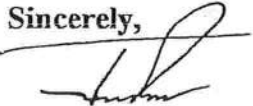
From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

Date: 11/1/06

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the client by the client and the client's acceptance of this information as a basis for the design and construction of the component. The client is responsible for the accuracy of the information and the design and construction of the component. The client is responsible for the accuracy of the information and the design and construction of the component. The client is responsible for the accuracy of the information and the design and construction of the component.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies shall be reported to the designer immediately. The fabricator shall be responsible for the accuracy of the information and the design and construction of the component. The fabricator shall be responsible for the accuracy of the information and the design and construction of the component. The fabricator shall be responsible for the accuracy of the information and the design and construction of the component.

PRECAUTIONARY NOTES

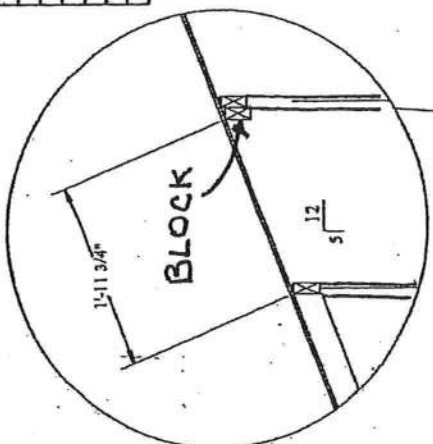
All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during building and handling, delivery and installation to avoid damage. Temporary and permanent bracing for building trusses in a straight and plumb position and for reading lateral forces shall be designed and installed by the fabricator. Careful attention to erection procedures shall be followed. No temporary bracing for trusses shall be used. No temporary bracing for trusses shall be used. No temporary bracing for trusses shall be used.

HIP TRUSS BLOCKING REQUIREMENTS

APA-FORM NO. TT-083
SPAN RATING AND BLOCKING RECOMMENDATIONS FOR USE OVER HIP-ROOFS SUPPORT AT 24" O.C

Roof Slope	24/12	32/12	40/20	48/24
< 1:12	OK	OK	OK	OK
< 2:12	OK	OK	OK	OK
< 3:12	OK	OK	OK	OK
< 4:12	OK	OK	OK	OK
< 5:12	OK	OK	OK	OK
< 6:12	OK	OK	OK	OK
< 7:12	OK	OK	OK	OK
< 8:12	OK	OK	OK	OK
< 9:12	OK	OK	OK	OK
< 10:12	OK	OK	OK	OK
< 11:12	OK	OK	OK	OK
< 12:12	OK	OK	OK	OK
< 13:12	OK	OK	OK	OK
< 14:12	OK	OK	OK	OK

NOTE: SHADED AREA REQUIRES 2X LUMBER BLOCKING BETWEEN TRUSSES AT UN-SUPPORTED PANEL EDGES IN HIP ROOF AREAS



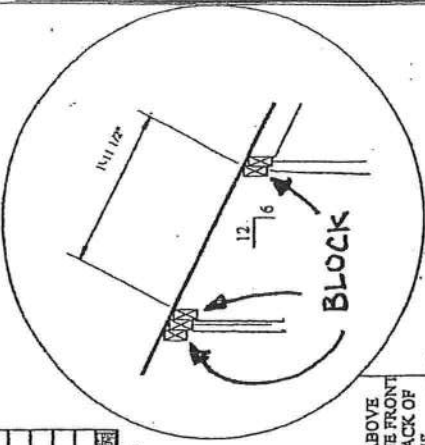
2X BLOCKING TYP
NAILED W/ 10D 12" O.C.

5/12 (1) 2X
TO THE FRONT OF
HIP TRUSS.

7/16" OSB ROOF
SHEATHING

6/12 OR ABOVE
(2) 2X ONE FRONT
& ONE BACK OF
HIP TRUSS

NOTE: NO BLOCKING IS REQUIRED ON 4/12 AND BELOW PITCHED ROOFS



PRE-ENG'D TRUSSES

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce bracing length. Provision must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 or TPI-02/FBC-04)
D'Onofrio Drive, Madison, Wisconsin 53719. Contact Building Designer/Truss Plate Institute, TPI is located at 383 Component Engineering by: Truss Engineering Co., P.A., 118 Soundside Rd., Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #00050068
367 Medallion Pl. Chuluota, FL 32766

Eng Job: WO: HIPDETAIL

Dwg: TI: HIP

Dsgnr: TLY Chk: 11/2/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Flt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

v4.7.32-0

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513785
DAVENPORT	B1	SPECIAL	1	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, FL						

7.010 s Oct 18 2007 MITek Industries, Inc. Tue Nov 13 08:17:33 2007 Page 1



Scale = 1:71.0

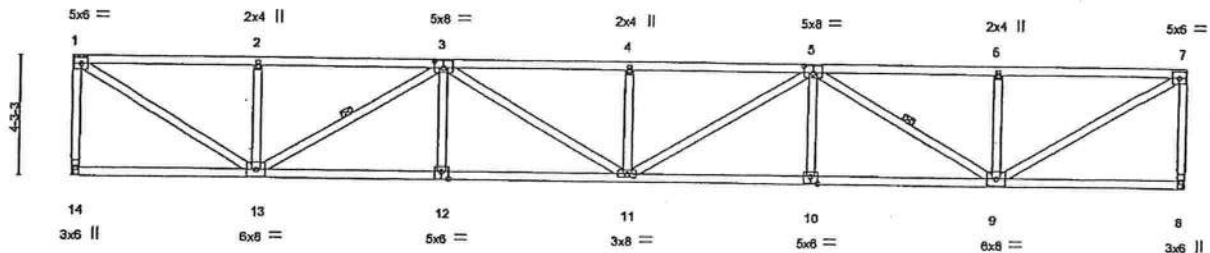


Plate Offsets (X,Y): [3-0-4-0-0-3-0], [5-0-4-0-0-3-0], [10-0-3-0-0-3-0], [12-0-3-0-0-3-0]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 16.0	Plates Increase	1.25	TC 0.70	Ver(L/L)	-0.40 11-12	>899
TCCL 7.0	Lumber Increase	1.25	BC 0.83	Ver(T/L)	-0.80 11-12	>597
BCCL 10.0	Rep Stress Incr	YES	WB 0.66	Horz(T/L)	0.18 8	n/a
BCDL 10.0	Code FBC2004/TP12002		(Matlrx)			n/a
						Weight: 223 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-2 oc purlins, except and verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-0 oc bracing.
WEBS 1 Row at midpt 3-13, 5-9

REACTIONS (lb/size) 14=1707/Mechanical, 8=1707/Mechanical
Max Uplift 14=-418(LC 4), 8=-418(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-14=-1577/646, 1-2=-2415/872, 2-3=-2415/872, 3-4=-4336/1565, 4-5=-4336/1565, 5-6=-2415/872, 6-7=-2415/872, 7-8=-1577/646
BOT CHORD 13-14=-25/68, 12-13=-1393/3856, 11-12=-1393/3856, 10-11=-1393/3856, 9-10=-1393/3856, 8-9=-25/68
WEBS 1-13=-992/2749, 2-13=-307/297, 3-13=-1677/806, 3-12=-0/266, 3-11=-201/559, 4-11=-306/288, 5-11=-201/559, 5-10=-0/266, 5-9=-1677/806, 6-9=-307/297, 7-9=-992/2749

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 418 lb uplift at joint 14 and 418 lb uplift at joint 8.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, F.L.E.C. #43144
Truss Engineering Co.,
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A L&T A Division
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4606947
DAVENPORT	BGRD1	SPECIAL	1	2	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Jan 10 09:25:26 2008 Page 1						

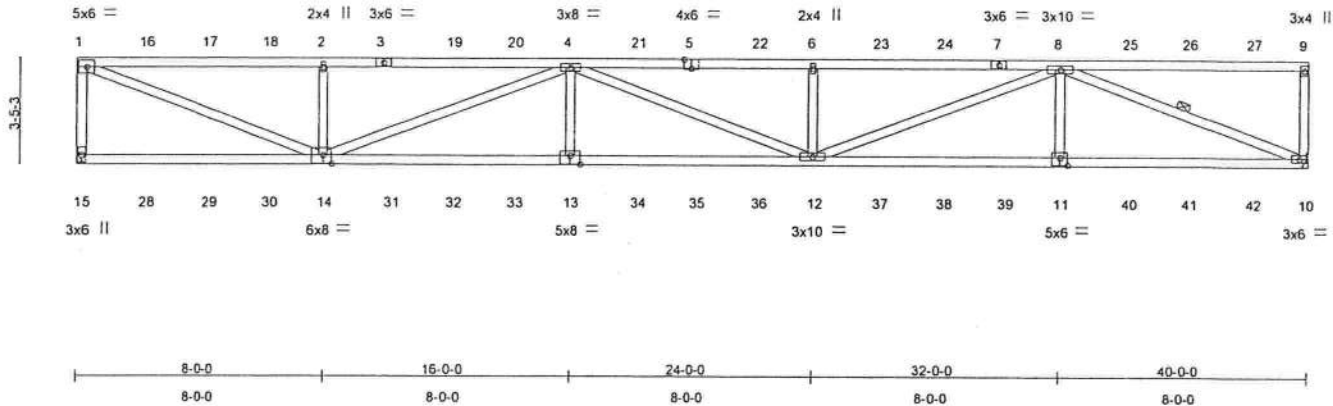
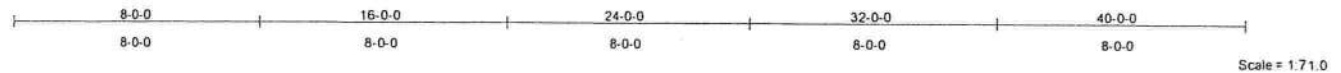


Plate Offsets (X,Y): [5:0-3-0 Edge], [11:0-3-0-0-3-0], [13:0-4-0-0-3-4], [14:0-3-8-0-3-4]

LOADING (psf)	SPACING	1-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.83	Vert(LL)	0.75 12-13	>635	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.82	Vert(TL)	-1.10 12-13	>432	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.87	Horz(TL)	0.18 10	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 410 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-2-0 oc bracing.
WEBS 1 Row at midpt 8-10

REACTIONS

(lb/size) 15=3030/Mechanical, 10=2848/Mechanical
Max Uplift 15=-1833(LC 9), 10=-1697(LC 9)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-15=-2676/1670, 1-16=-6079/3645, 16-17=-6079/3645, 17-18=-6079/3645, 2-18=-6079/3645, 2-3=-6079/3645, 3-19=-6079/3645, 19-20=-6079/3645, 4-20=-6079/3645, 4-21=-9072/5440, 5-21=-9072/5440, 5-22=-9072/5440, 6-22=-9072/5440, 6-23=-9072/5440, 23-24=-9072/5440, 7-24=-9072/5440, 7-8=-9072/5440, 8-25=-228/137, 25-26=-228/137, 26-27=-228/137, 9-27=-228/137, 9-10=-250/195
BOT CHORD 15-28=-137/229, 28-29=-137/229, 29-30=-137/229, 14-30=-137/229, 14-31=-5446/9081, 31-32=-5446/9081, 32-33=-5446/9081, 13-33=-5446/9081, 13-34=-5446/9081, 34-35=-5446/9081, 35-36=-5446/9081, 12-36=-5446/9081, 12-37=-3644/6078, 37-38=-3644/6078, 38-39=-3644/6078, 11-39=-3644/6078, 11-40=-3644/6078, 40-41=-3644/6078, 41-42=-3644/6078, 10-42=-3644/6078
WEBS 1-14=-3778/6301, 2-14=-569/466, 4-14=-3225/1934, 4-13=-243/603, 4-12=-9/6, 6-12=-574/463, 8-12=-1930/3216, 8-11=-235/595, 8-10=-6301/3777

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1833 lb uplift at joint 15 and 1697 lb uplift at joint 10.

Continued on page 2



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 10, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4606947
DAVENPORT	BGRD1	SPECIAL	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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NOTES

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 98 lb down and 83 lb up at 0-1-12, 98 lb down and 83 lb up at 2-2-4, 98 lb down and 83 lb up at 4-2-4, 98 lb down and 83 lb up at 6-2-4, 98 lb down and 83 lb up at 8-2-4, 98 lb down and 83 lb up at 10-2-4, 98 lb down and 83 lb up at 12-2-4, 98 lb down and 83 lb up at 14-2-4, 98 lb down and 83 lb up at 16-2-4, 98 lb down and 83 lb up at 18-2-4, 98 lb down and 83 lb up at 20-2-4, 98 lb down and 83 lb up at 22-2-4, 98 lb down and 83 lb up at 24-2-4, 98 lb down and 83 lb up at 26-2-4, 98 lb down and 83 lb up at 28-2-4, 98 lb down and 83 lb up at 30-2-4, 98 lb down and 83 lb up at 32-2-4, 98 lb down and 83 lb up at 34-2-4, and 98 lb down and 83 lb up at 36-2-4, and 98 lb down and 83 lb up at 38-2-4 on top chord, and 110 lb down and 72 lb up at 0-1-12, 110 lb down and 72 lb up at 2-2-4, 110 lb down and 72 lb up at 4-2-4, 110 lb down and 72 lb up at 6-2-4, 110 lb down and 72 lb up at 8-2-4, 110 lb down and 72 lb up at 10-2-4, 110 lb down and 72 lb up at 12-2-4, 110 lb down and 72 lb up at 14-2-4, 110 lb down and 72 lb up at 16-2-4, 110 lb down and 72 lb up at 18-2-4, 110 lb down and 72 lb up at 20-2-4, 110 lb down and 72 lb up at 22-2-4, 110 lb down and 72 lb up at 24-2-4, 110 lb down and 72 lb up at 26-2-4, 110 lb down and 72 lb up at 28-2-4, 110 lb down and 72 lb up at 30-2-4, 110 lb down and 72 lb up at 32-2-4, 110 lb down and 72 lb up at 34-2-4, and 110 lb down and 72 lb up at 36-2-4, and 110 lb down and 72 lb up at 38-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-9=-23, 10-15=-20

Concentrated Loads (lb)

Vert: 15=-110(F) 1=-98(F) 3=-98(F) 14=-110(F) 2=-98(F) 4=-98(F) 13=-110(F) 6=-98(F) 12=-110(F) 11=-110(F) 8=-98(F) 5=-98(F) 7=-98(F) 16=-98(F) 17=-98(F) 18=-98(F) 19=-98(F) 20=-98(F) 21=-98(F) 22=-98(F) 23=-98(F) 24=-98(F) 25=-98(F) 26=-98(F) 27=-98(F) 28=-110(F) 29=-110(F) 30=-110(F) 31=-110(F) 32=-110(F) 33=-110(F) 34=-110(F) 35=-110(F) 36=-110(F) 37=-110(F) 38=-110(F) 39=-110(F) 40=-110(F) 41=-110(F) 42=-110(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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ENGINEERING BY
TRENCO
A MITek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513787
DAVENPORT	G1	GABLE	1	1	Job Reference (optional)	
Maronde Homes Inc., Sanford, FL						

7.010 s Oct 18 2007 Mitek Industries, Inc. Tue Nov 13 08:17:35 2007 Page 1

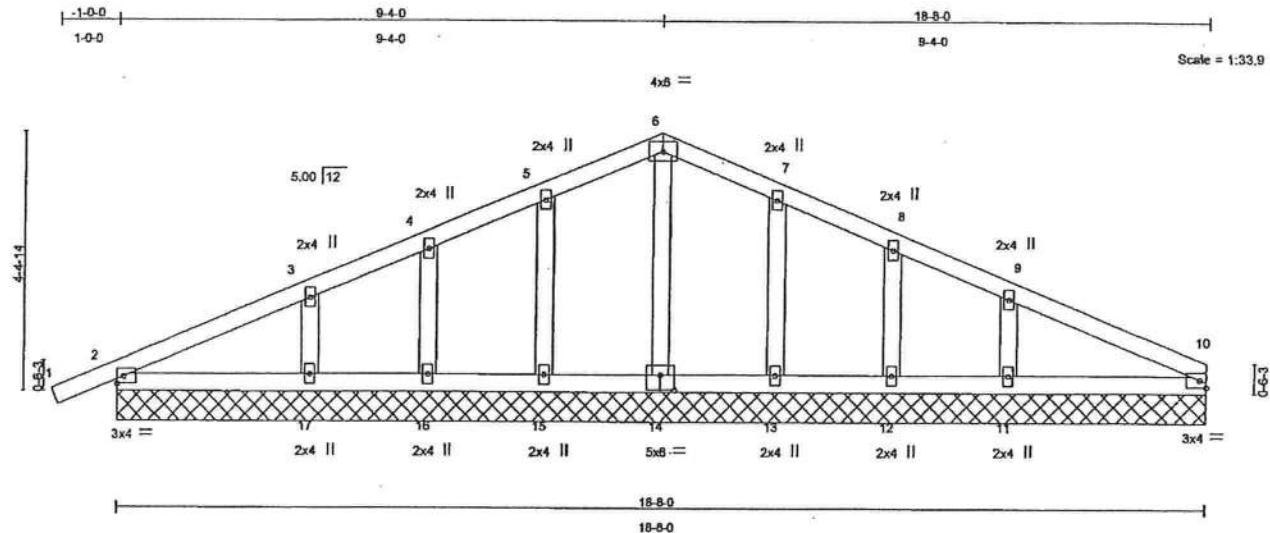


Plate Offsets (X,Y): [14-0-3-0-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.10	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.09	Vert(TL)	0.01	1	n/r	120		
BCLL 10.0	Rep Stress Incr	NO	WB 0.03	Horz(TL)	0.00	10	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
										Weight: 85 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=121/18-8-0, 2=179/18-8-0, 14=160/18-8-0, 15=183/18-8-0, 16=132/18-8-0, 17=277/18-8-0, 13=185/18-8-0, 12=125/18-8-0, 11=283/18-8-0
Max Horz 2=93(LC 6)
Max Uplift 10=32(LC 7), 2=115(LC 6), 15=102(LC 6), 16=82(LC 6), 17=151(LC 6), 13=104(LC 7), 12=71(LC 7), 11=176(LC 7)
Max Grav 10=121(LC 1), 2=179(LC 1), 14=160(LC 1), 15=187(LC 10), 16=132(LC 1), 17=277(LC 10), 13=188(LC 11), 12=125(LC 1), 11=293(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/10, 2-3=76/34, 3-4=32/89, 4-5=24/122, 5-6=26/179, 6-7=28/180, 7-8=24/120, 8-9=33/70, 9-10=46/33
BOT CHORD 2-17=0/83, 16-17=0/83, 15-16=0/83, 14-15=0/83, 13-14=0/83, 12-13=0/83, 11-12=0/83, 10-11=0/83
WEBS 6-14=83/0, 5-15=101/137, 4-16=72/119, 3-17=145/197, 7-13=101/139, 8-12=69/112, 9-11=153/213

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 10, 115 lb uplift at joint 2, 102 lb uplift at joint 15, 82 lb uplift at joint 16, 151 lb uplift at joint 17, 104 lb uplift at joint 13, 71 lb uplift at joint 12 and 176 lb uplift at joint 11.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

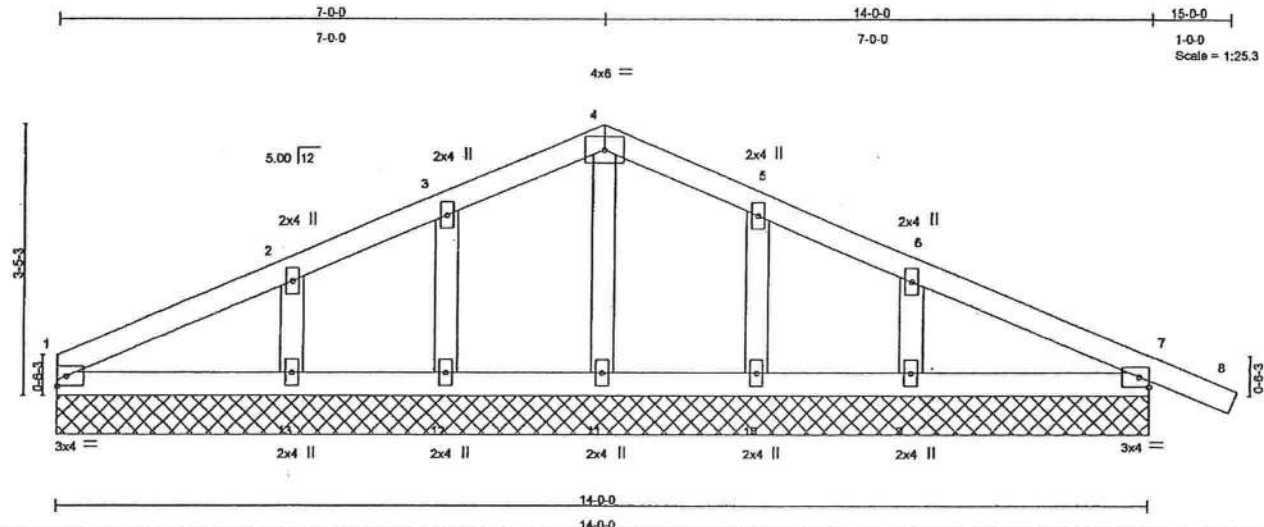
November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE NIT-7473 BEFORE USE.
Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oonorio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITEK COMPANY
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513788
DAVENPORT	G2	GABLE	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.10	Vert(LL) 0.00	8	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.07	Vert(TL) 0.00	8	n/r	120		
BCLL 10.0	Rep Stress Incr NO	WB 0.03	Horz(TL) 0.00	7	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)						
Weight: 59 lb								

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purfins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 1=109/14-0-0, 7=169/14-0-0, 11=180/14-0-0, 12=137/14-0-0, 13=266/14-0-0, 10=143/14-0-0, 9=249/14-0-0
Max Horz 1=78(LC 7)
Max Uplift 1=39(LC 7), 7=138(LC 7), 12=78(LC 6), 13=161(LC 6), 10=88(LC 7), 9=131(LC 7)
Max Grav 1=109(LC 1), 7=169(LC 1), 11=180(LC 1), 12=141(LC 10), 13=266(LC 1), 10=147(LC 11), 9=249(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=48/42, 2-3=32/88, 3-4=26/151, 4-5=26/152, 5-6=31/88, 6-7=37/26, 7-8=0/10
BOT CHORD 1-13=0/75, 12-13=0/75, 11-12=0/75, 10-11=0/75, 9-10=0/75, 7-9=0/75
WEBS 4-11=82/23, 3-12=79/122, 2-13=138/210, 5-10=82/120, 6-9=131/193

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2'-0" oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1, 138 lb uplift at joint 7, 78 lb uplift at joint 12, 161 lb uplift at joint 13, 88 lb uplift at joint 10 and 131 lb uplift at joint 9.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and 8 CSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenon, NC 27032

Maronda Homes, Inc. Sanford, FL

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November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. A additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult: ANSIP101 Quality Criteria, D58-89 and BC51 Building Component Safety Information, available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Metek Affiliat

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513789
DAVENPORT	GRD1	COMMON	2	3	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-23, 4-7=-23, 1-7=-20

Concentrated Loads (lb)

Vert: 10=-1828(F) 8=-1943(F) 11=-4002(F) 13=-1910(F) 14=-1932(F) 15=-1945(F) 16=-1976(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
 Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/ITPI Quality Criteria, D58-89 and IC311 Building Component Safety Information available from Truss Plate Institute, 553 D'Oroffio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A MITEK ALUMA
 616 Soundside Road
 Eden, NC 27632

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513790
DAVENPORT	GRD2	COMMON	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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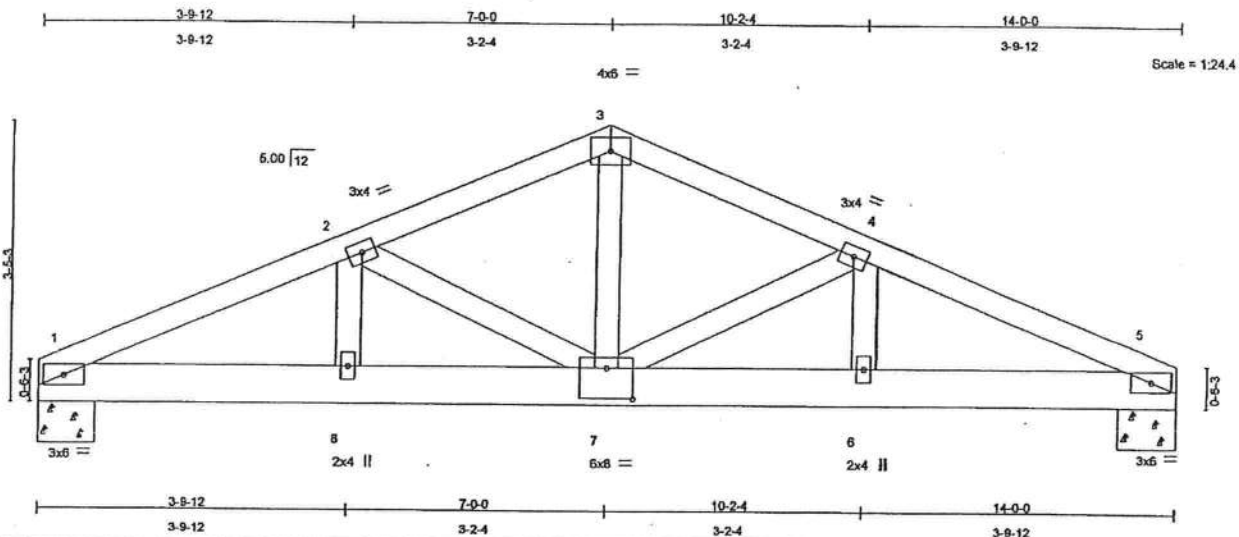


Plate Offsets (X,Y): [7-0-4-0-4-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/def	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.16	Vert(LL)	-0.06	7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.41	Vert(TL)	-0.11	7	>999	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.34	Horz(TL)	0.03	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 147 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.2
 WEBS 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-11-12 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=2430/0-8-6, 5=2430/0-8-6
 Max Horz 1=46(LC 5)
 Max Uplift 1=807(LC 5), 5=807(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=5038/1699, 2-3=5098/1792, 3-4=5098/1792, 4-5=5038/1700
 BOT CHORD 1-8=1528/4479, 7-8=1528/4479, 6-7=1481/4479, 5-6=1481/4479
 WEBS 2-6=0/72, 2-7=154/360, 3-7=1276/3707, 4-7=156/360, 4-8=0/73

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 6 - 2 rows at 0-3-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise);
 Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 807 lb uplift at joint 1 and 807 lb uplift at joint 5.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3714 lb down and 1394 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=46, 3-5=46, 1-5=40
 Concentrated Loads (lb)
 Vert: 7=3714(F)



Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL CO#7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
 Design valid for use only with Mitek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53717.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513802
DAVENPORT	H1S	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

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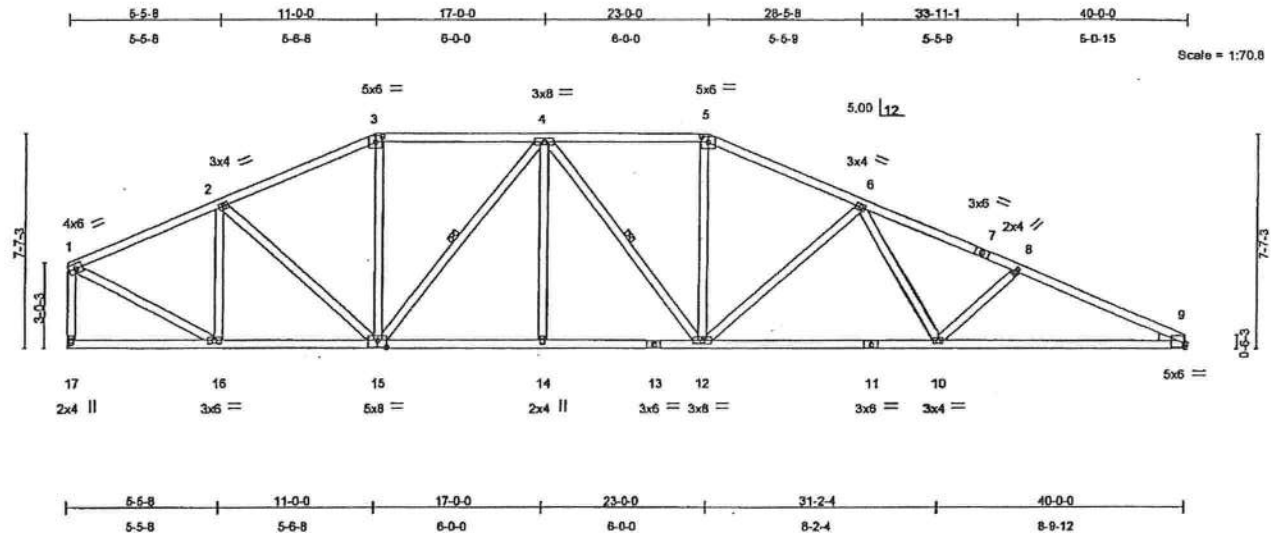


Plate Offsets (X,Y): [3-0-3-0-0-2-4], [5-0-3-0-0-2-4], [9-0-0-1-0-1-11], [15-0-4-0-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.59	Vert(LL)	-0.26 10-12	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.91	Vert(TL)	-0.54 10-12	>889	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.56	Horz(TL)	-0.15 17	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight 236 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2
 WEDGE
 Right: 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-8-8 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-8-8 oc bracing.
 WEBS 1 Row at midpt 4-12, 4-15

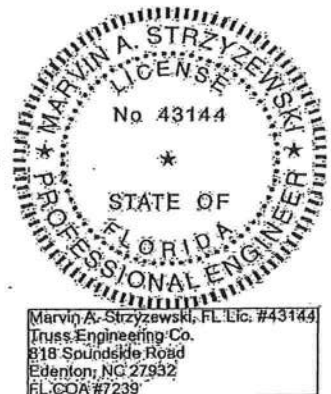
REACTIONS (lb/size) 9=1711/Mechanical, 17=1711/Mechanical
 Max Horz 9=184(LC 7)
 Max Uplift 9=306(LC 7), 17=281(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 5-6=2648/946, 6-7=3318/1112, 7-8=3371/1095, 8-9=3545/1185, 3-4=1895/782, 4-5=2323/922, 1-2=1799/620, 2-3=2091/796, 1-17=1803/584
 BOT CHORD 18-17=28/71, 15-16=515/1617, 14-15=718/2305, 13-14=718/2305, 12-13=718/2305, 11-12=934/2788, 10-11=934/2788, 9-10=1137/3198
 WEBS 8-10=191/234, 6-10=67/578, 6-12=646/304, 5-12=125/690, 4-12=129/87, 4-14=0/218, 4-15=726/235, 3-15=67/521, 2-15=72/431, 2-18=594/328, 1-16=550/1744

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 306 lb uplift at joint 9 and 281 lb uplift at joint 17.

LOAD CASE(S) Standard



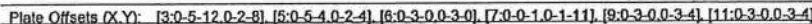
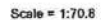
November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE M17473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-87 and TC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncio Drive, Madison, WI 53719.

TRENCO
 A Mitek Atlanta
 818 Spunkside Road
 Edenton, NC 27932

Maronde Homes Inc., Sanford, FL

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Weight: 218 lb

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Right: 2 X 4 SYP No.3

TOP CHORD	Structural wood sheathing directly applied or 2-7-4 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	1 Row at midpt 6-8, 2-12

Max Horiz 7=171(LC 7)
Max Uplift 7=290(LC 7), 12=306(LC 5)

TOP CHORD 6-8=2797/1008, 6-7=3576/1172, 3-4=2636/1039, 4-5=2636/1039, 1-2=187/52, 2-3=2078/777, 1-12=177/100
BOT CHORD 11-12=498/1421, 10-11=588/1892, 9-10=819/2534, 8-9=1112/3222, 7-8=1112/3222
WEBS 6-8=0/334, 6-9=773/323, 5-9=37/622, 5-10=72/290, 4-10=392/356, 3-10=360/1012, 3-11=160/201, 2-11=134/717,
2-12=1873/721

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 290 lb uplift at joint 7 and 308 lb uplift at joint 12.

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 Edenton, NC 27932
 FL COA #7239

November 13, 2007

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and practices that shift the location of responsibility of building designer - not just design. Bracing shown is for temporary support of structure only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS1/PTI Quality Criteria, D58-89 and BCSP Building Component Safety Information, available from Trussteel Institute, 583 D'Oroville Drive, Madison, WI 53719.

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513804
DAVENPORT	HIS	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:17:48 2007 Page 1

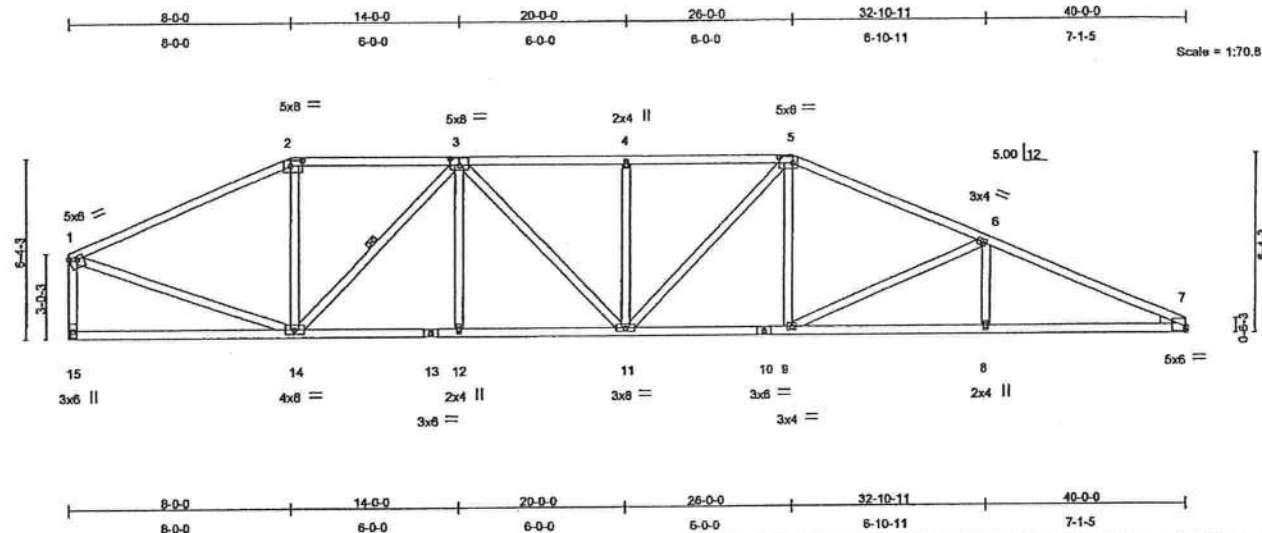


Plate Offsets (X, Y): [1:0-3-0-0-1-8], [2:0-5-0-0-2-4], [3:0-4-0-0-3-0], [5:0-5-12-0-2-8], [7:0-0-1-0-1-11]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 18.0	Plates Increase	1.25	TC 0.58	Vert(LL)	-0.24	8-9 >999
TCDL 7.0	Lumber Increase	1.25	BC 0.89	Vert(TL)	-0.50	8-9 >962
BCLL 10.0	Rep Stress Incr	YES	WB 0.64	Horz(TL)	-0.16	15 n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)			n/a
				PLATES	GRIP	
				MT20	244/190	
				Weight: 224 lb		

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2
 WEDGE
 Right: 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-9-9 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-8-13 oc bracing.
 WEBS 1 Row at midpt 3-14

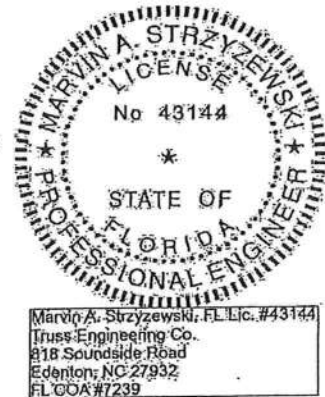
REACTIONS (lb/size) 7=1711/Mechanical, 15=1711/Mechanical
 Max Horz 7=-165(LC 7)
 Max Uplift 7=-285(LC 4), 15=-319(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 5-6=-2878/1039, 6-7=-3803/1188, 2-3=-1884/755, 3-4=-2849/1105, 4-5=-2849/1105, 1-2=-2079/747, 1-15=-1545/618
 BOT CHORD 14-15=-83/161, 13-14=-860/2586, 12-13=-860/2586, 11-12=-860/2586, 10-11=-861/2611, 9-10=-861/2611, 8-9=-1132/3246, 7-8=-1132/3246
 WEBS 8-8=0/318, 6-9=-714/298, 5-9=-44/548, 5-11=-128/467, 4-11=-288/255, 3-11=-129/401, 3-12=0/223, 3-14=-1102/372, 2-14=0/524, 1-14=-571/1791

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 7 and 319 lb uplift at joint 15.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL GOA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTE REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 563 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A MITek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513805
DAVENPORT	H4S	HIP	1	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, FL						7.010 & Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:49 2007 Page 1

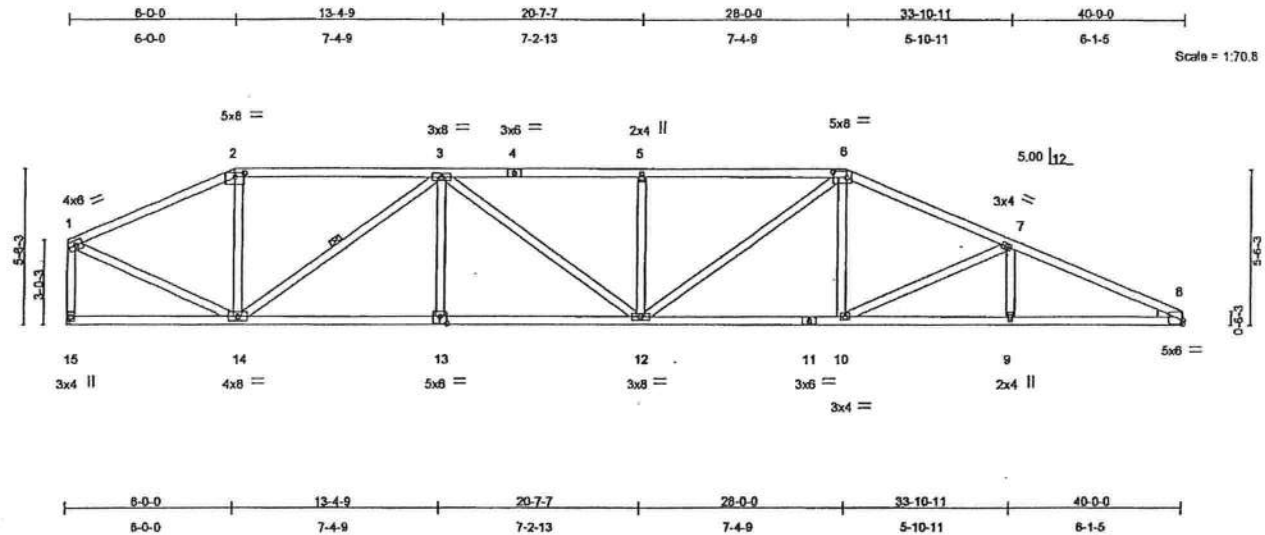


Plate Offsets (X,Y): [2-0-4-0-0-1-13], [6-0-5-12-0-2-8], [8-0-0-1-0-1-11], [13-0-3-0-0-3-0]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	L/defl
TCLL 16.0	Plates Increase	1.25	TC 0.43	Vert(LL)	-0.30 10-12	>899 240
TCDL 7.0	Lumber Increase	1.25	BC 0.82	Vert(TL)	-0.61 10-12	>786 180
BCLL 10.0	Rep Stress Incr	YES	WB 0.50	Horz(TL)	-0.18 15	n/a n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)			
						Weight: 216 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2
 WEDGE
 Right 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-11-12 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-8-12 oc bracing.
 WEBS 1 Row at midpt 3-14

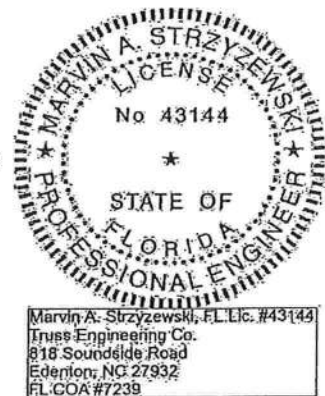
REACTIONS (lb/size) 8=1711/Mechanical, 15=1711/Mechanical
 Max Horz 8=152(LC 7)
 Max Uplift 8=303(LC 4), 15=344(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 6-7=-3074/1107, 7-8=-3825/1212, 2-3=-1729/692, 3-4=-3325/1265, 4-5=-3325/1265, 5-6=-3325/1265, 1-2=-1908/692, 1-15=-1599/615
 BOT CHORD 14-15=-2872, 13-14=-1023/2963, 12-13=-1023/2963, 11-12=-958/2806, 10-11=-958/2806, 9-10=-1160/3263, 8-9=-1160/3263
 WEBS 7-9=0/255, 7-10=-519/223, 8-10=-12/485, 8-12=-235/748, 5-12=-348/317, 3-12=-157/467, 3-13=0/292, 3-14=-1559/549, 2-14=0/456, 1-14=-808/1813

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at Joint 8 and 344 lb uplift at Joint 15.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with Mitek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Alliance
 818 Soundside Road
 Edenport, NC 27932

Job	Truss	Truss Type	City	Ply	DAVENPORT_125	E4513608
DAVENPORT	HSS	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						
7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:50 2007 Page 1						

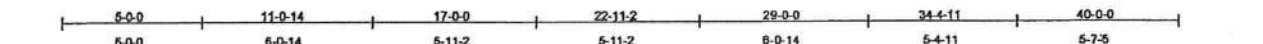
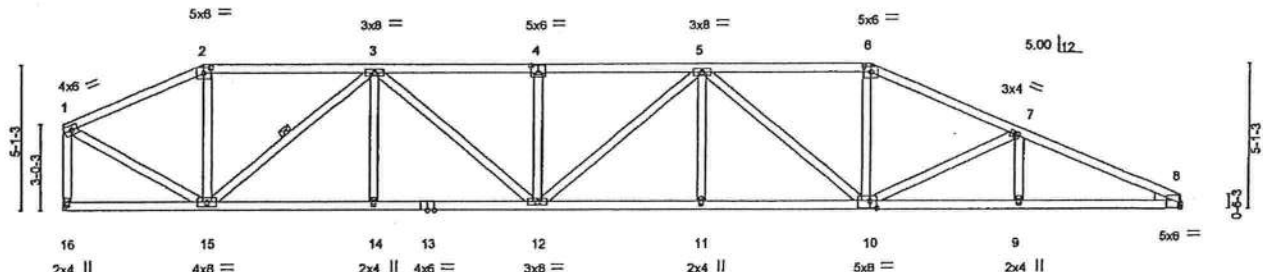
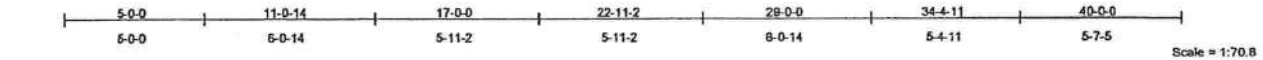


Plate Offsets (X,Y): [2-0-3-0-0-2-4], [4-0-3-0-0-3-0], [8-0-3-0-0-2-4], [8-0-0-1-0-1-11], [10-0-2-12-0-3-0]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	L/defl
TCLL 16.0	Plates Increase	1.25	TC 0.36	Vert(LL)	-0.31 11-12	>999 240
TCDL 7.0	Lumber Increase	1.25	BC 0.78	Vert(TL)	-0.62 11-12	>773 180
BCLL 10.0	Rep Stress Incr	YES	WB 0.78	Horz(TL)	-0.19 16	n/a n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)			
						Weight: 222 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-9 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-8-12 oc bracing.
WEBS 2 X 4 SYP No.2	WEBS 1 Row at midpt 3-15
WEDGE	
Right: 2 X 4 SYP No.3	

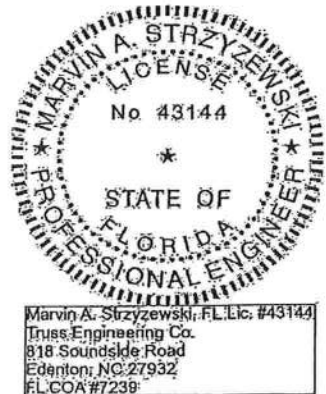
REACTIONS (lb/size) 8=1711/Mechanical, 16=1711/Mechanical
Max Horiz=146(LC 7)
Max Uplift=312(LC 4), 16=356(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 6-7=-3162/1137, 7-8=-3836/1224, 2-3=-1800/838, 3-4=-3504/1324, 4-5=-3504/1324, 5-6=-2876/1094, 1-2=-1759/642, 1-16=-1616/615
BOT CHORD 15-16=-20/52, 14-15=-1003/2871, 13-14=-1003/2871, 12-13=-1003/2871, 11-12=-1232/3515, 10-11=-1232/3515, 9-10=-1176/3271, 8-9=-1176/3271
WEBS 7-9=0/229, 7-10=-438/200, 6-10=-199/828, 5-10=-904/296, 5-11=0/241, 5-12=-29/59, 4-12=-269/251, 3-12=-290/827, 3-14=0/240, 3-15=-1662/592, 2-15=-10/451, 1-15=-597/1755

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.80 plate grip DOL=1.80. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for design to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 8 and 356 lb uplift at joint 16.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSI-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Alliance
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513807
DAVENPORT	H&	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7,010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 06:17:50 2007 Page 1

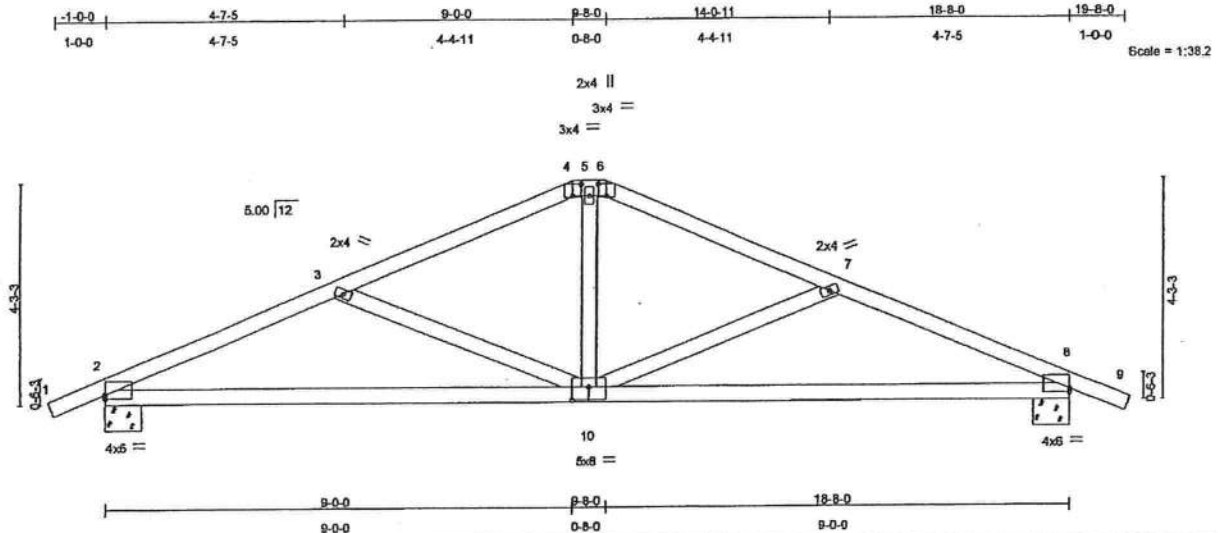


Plate Offsets (X,Y): [2-0-0-1-0-0-15], [4-0-2-0-0-2-11], [6-0-2-0-0-2-11], [8-0-0-1-0-0-15], [10-0-4-0-0-3-0]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 16.0	Plates Increase	1.25	TC 0.27	Vert(LL)	-0.13 8-10	>999 240
TCDL 7.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.30 8-10	>720 180
BCLL 10.0	Rep Stress Incr	YES	WB 0.18	Horz(TL)	0.03 8	n/a n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)			
				PLATES	GRIP	
				MT20	244/190	
				Weight: 82 lb		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=838/0-8-6, 8=838/0-8-6
Max Horz 2=71(LC 6)
Max Uplift 2=227(LC 6), 8=227(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/10, 2-3=-1278/432, 3-4=-1059/317, 4-5=-937/332, 5-6=-937/332, 6-7=-1059/317, 7-8=-1278/432, 8-9=0/10
BOT CHORD 2-10=-299/1133, 8-10=-299/1133
WEBS 3-10=-244/191, 7-10=-244/191, 5-10=-28/558

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 2 and 227 lb uplift at joint 8.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M3-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/APA Quality Criteria, D58-87 and 3CSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53717.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	City	Pty	DAVENPORT_125	E4513812
DAVENPORT	HGRD1	HIP	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Tue Nov 13 08:17:54 2007 Page 1

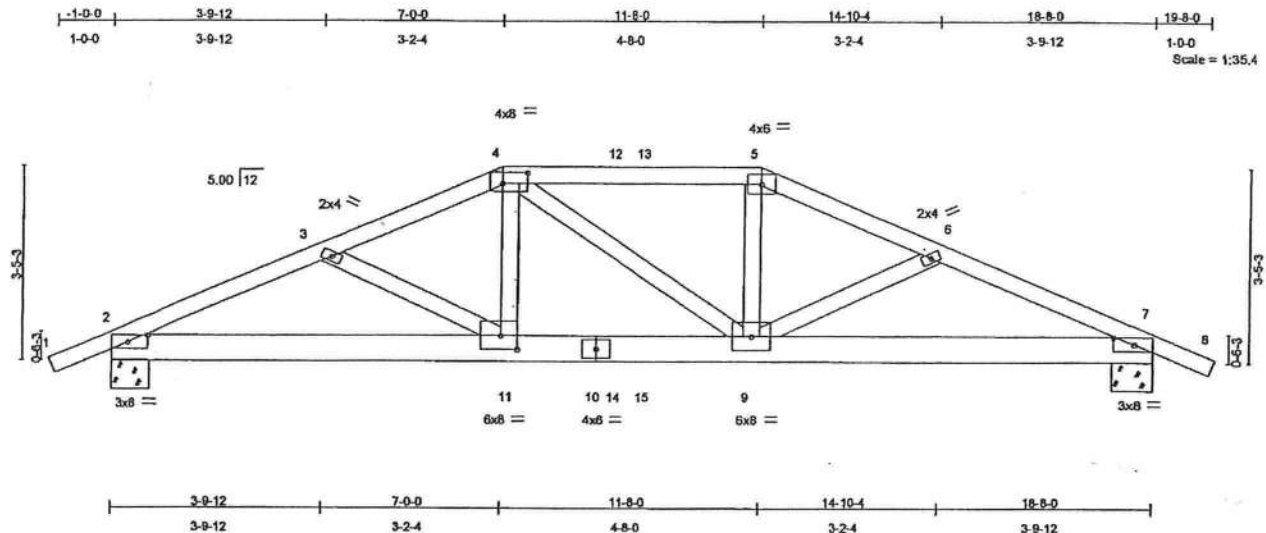


Plate Offsets (X,Y): [2-0-4-4-0-1-8], [4-0-5-4-0-2-4], [7-0-4-4-0-1-8], [11-0-3-8-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCCL 18.0	Plates Increase	1.25	TC 0.44	Vert(LL)	0.13	9-11	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.59	Vert(TL)	-0.22	9-11	>999	180		
BCCL 10.0	Rep Stress Incr	NO	WB 0.17	Horz(TL)	0.06	7	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 102 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-8 oc bracing.

REACTIONS (lb/size) 2=1807/0-8-6, 7=1807/0-8-6
Max Horz 2=61(LC 7)
Max Uplift 2=855(LC 7), 7=855(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/14, 2-3=3481/1661, 3-4=3522/1704, 4-12=3288/1623, 12-13=3288/1623, 5-13=3288/1623, 5-6=3519/1702, 6-7=3480/1680, 7-8=0/14
BOT CHORD 2-11=1478/3082, 10-11=1541/3291, 10-14=1541/3291, 14-15=1541/3291, 9-15=1541/3291, 7-9=1417/3081
WEBS 3-11=115/298, 4-11=388/931, 4-9=73/67, 5-9=388/928, 6-9=114/284

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 855 lb uplift at joint 2 and 855 lb uplift at joint 7.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 98 lb down and 83 lb up at 7-0-0, 98 lb down and 83 lb up at 9-0-12, and 98 lb down and 83 lb up at 9-7-4, and 98 lb down and 83 lb up at 11-6-0 on top chord, and 663 lb down and 404 lb up at 7-0-0, 110 lb down and 72 lb up at 9-0-12, and 110 lb down and 72 lb up at 9-7-4, and 663 lb down and 404 lb up at 11-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular; Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)

Vert: 1-4=48, 4-5=48, 5-8=48, 2-7=40

Concentrated Loads (lb)

Vert: 4=98(B) 5=98(B) 11=663(B) 9=663(B) 12=98(B) 13=98(B) 14=110(B) 15=110(B)



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

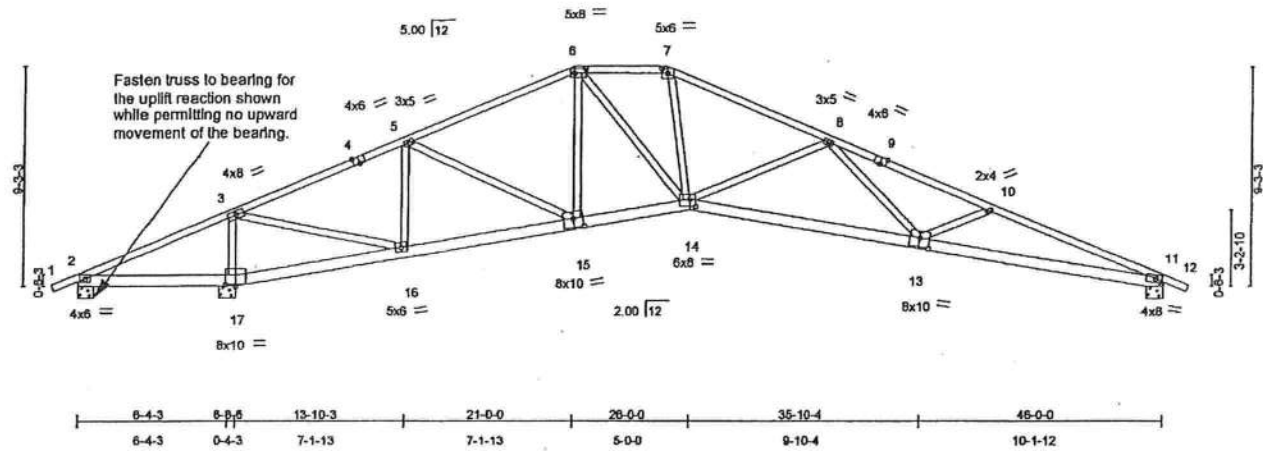
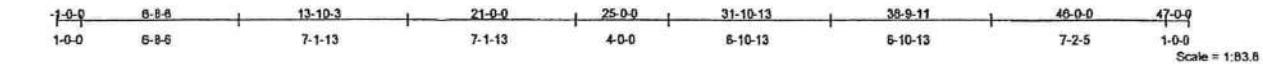
November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MD-7473 BEFORE USE.
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ENGINEERED BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513815
DAVENPORT	HS1	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL			7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:57 2007 Page 1			



Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513816
DAVENPORT	HS2	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 15 2007 MTEK Industries, Inc. Tue Nov 13 08:17:58 2007 Page 1

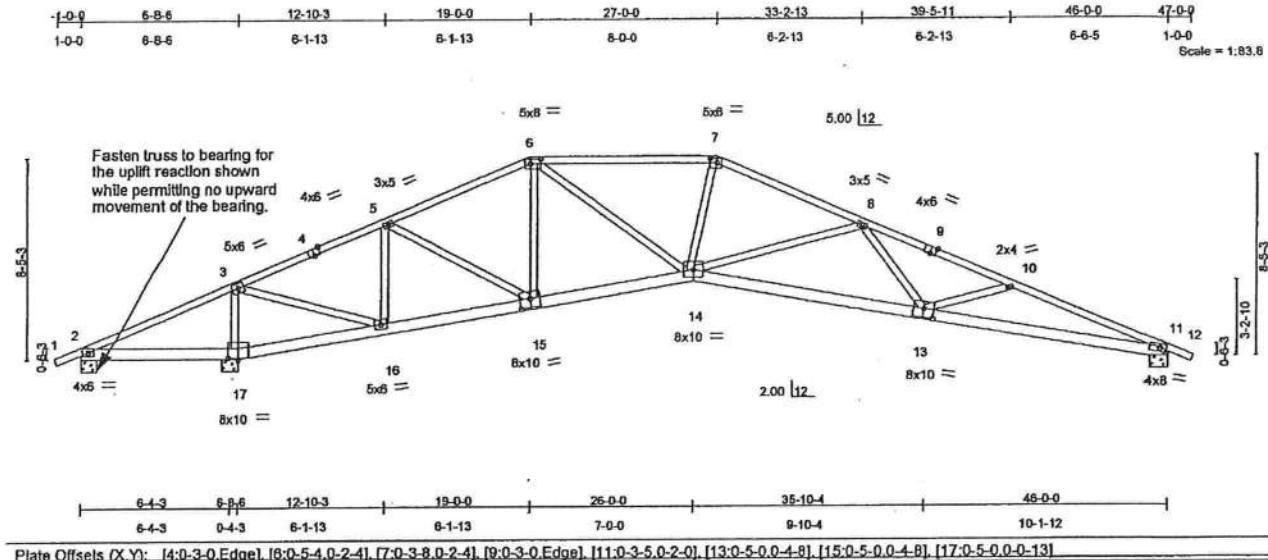


Plate Offsets (X Y): [4:0-3-0-Edge] [6:0-5-4-0-2-4] [7:0-3-8-0-2-4] [9:0-3-0-Edge] [11:0-3-5-0-2-0] [13:0-5-0-0-4-8] [15:0-5-0-0-4-8] [17:0-5-0-0-13]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	L/d
TCCL 18.0	Plates Increase	1.25	TC 0.74	Vert(LL)	-0.36 13-14	>999 240
TCDL 7.0	Lumber Increase	1.25	BC 0.94	Vert(TL)	-0.72 13-14	>652 180
BCCL 10.0	Rep Stress Incr	YES	WB 0.90	Horz(TL)	-0.28 17	n/a n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)			
						Weight: 265 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-11 oc bracing.

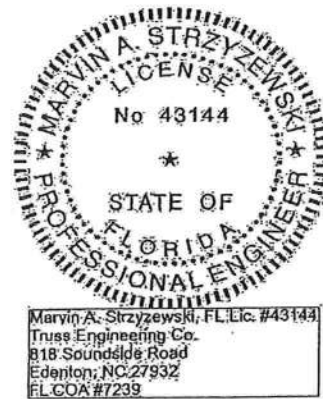
REACTIONS (lb/size) 17=3204/0-8-6, 2=741/0-8-0, 11=1563/0-8-6
Max Horz 11=137(LC 6)
Max Uplift 17=566(LC 5), 2=799(LC 11), 11=368(LC 7)
Max Grav 17=3204(LC 1), 2=45(LC 7), 11=1563(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 7-8=2819/907, 8-9=4181/1218, 9-10=4224/1208, 10-11=4432/1437, 11-12=0/11, 6-7=2705/902, 1-2=0/14, 2-3=878/2453, 3-4=848/279, 4-5=785/297, 5-6=1817/881
BOT CHORD 13-14=911/3584, 11-13=1217/4053, 16-17=2355/768, 15-16=43/758, 14-15=261/1669, 2-17=2169/701
WEBS 10-13=135/314, 8-13=7660, 8-14=977/423, 7-14=48/744, 6-14=293/1348, 6-15=423/217, 5-15=253/1105, 5-16=1173/481, 3-16=813/3094, 3-17=2463/873

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 11 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 17, 799 lb uplift at joint 2 and 368 lb uplift at joint 11.

LOAD CASE(S) Standard



November 13, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

TRENCO
A Mitek Alliance

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513817
DAVENPORT	HS3	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:59 2007 Page 1

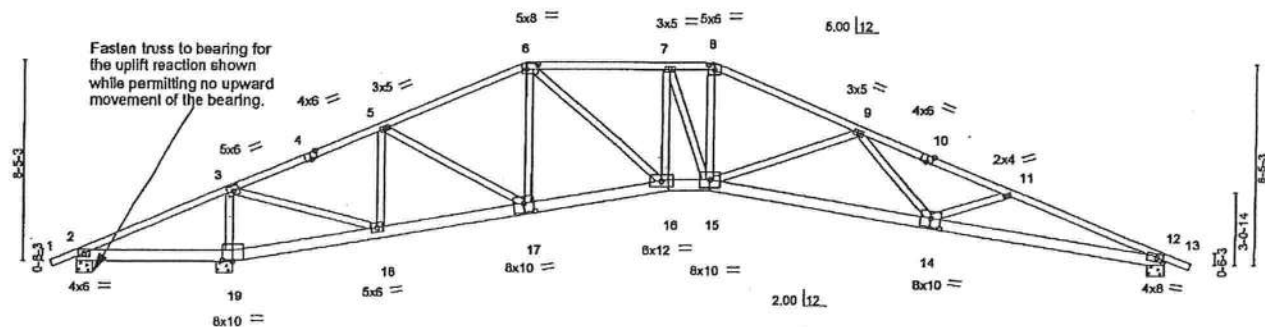
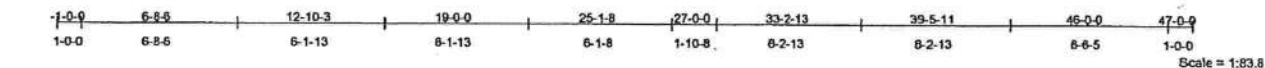


Plate Offsets (X,Y): [4:0-3:0 Edge], [6:0-5:12:0-2-8], [8:0-3:0:0-2-4], [10:0-3:0 Edge], [12:0-3:5:0-2-0], [14:0-5:0:0-4-8], [17:0-5:0:0-4-8], [19:0-5:0:0-13]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 16.0	Plates Increase	1.25	TC 0.78	Vert(LL)	-0.35 14-15	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.94	Vert(TL)	-0.70 14-15	>669
BCLL 10.0	Rep Stress Incr	YES	WB 0.65	Horz(TL)	-0.27 19	n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)			
						Weight: 277 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-8-2 oc purlins.
BOT CHORD 2 X 6 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-9-13 oc bracing.
WEBS 2 X 4 SYP No.2	

REACTIONS (lb/size) 19=3199/0-8-6, 2=736/0-8-0, 12=1564/0-8-6
Max Horiz 12=137(LC 6)
Max Uplift 19=566(LC 5), 2=794(LC 11), 12=368(LC 7)
Max Grav 19=3199(LC 1), 2=43(LC 7), 12=1564(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 8-9=2927/926, 9-10=4197/1239, 10-11=4280/1228, 11-12=4448/1424, 12-13=0/11, 6-7=2553/893, 7-8=2688/806,
1-2=0/14, 2-3=877/2443, 3-4=850/282, 4-5=790/301, 5-6=1819/656
BOT CHORD 14-15=923/3559, 12-14=1205/4064, 15-16=479/2539, 18-19=2346/768, 17-18=477/763, 16-17=252/1655,
2-19=2160/701
WEBS 11-14=110/288, 9-14=19/866, 9-15=899/434, 8-15=181/895, 7-15=130/386, 7-16=523/188, 6-16=311/1240,
6-17=393/209, 5-17=239/1092, 5-18=1178/483, 3-18=818/3092, 3-19=2457/872

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Bearing at joint(s) 12 considers parallel to grain value using ANS/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 19, 794 lb uplift at joint 2 and 368 lb uplift at joint 12.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL CO#7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and brock, consult ANS/TPI1 Quality Criteria, B58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513818
DAVENPORT	HS4	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:59 2007 Page 1

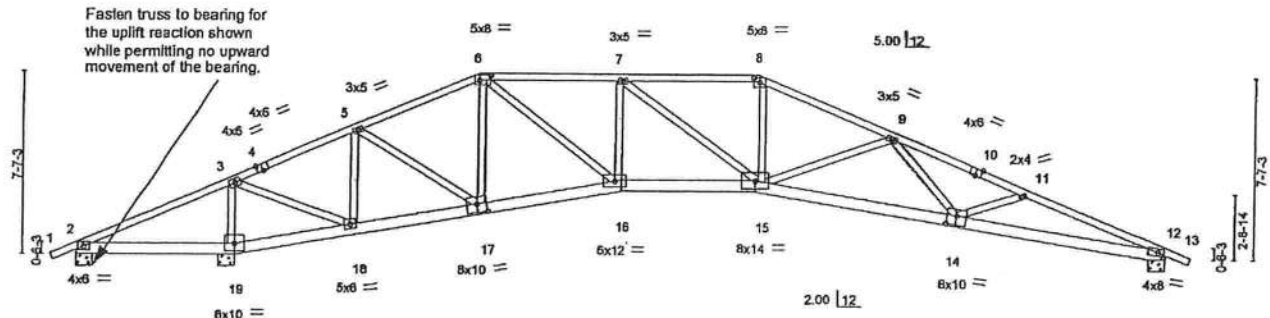


Plate Offsets (X,Y):	[4-0-3-0-Edge]	[6-0-5-12-0-2-8]	[8-0-3-0-0-2-4]	[10-0-3-0-Edge]	[12-0-3-5-0-2-0]	[14-0-5-0-0-4-8]	[17-0-5-0-0-4-8]
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/def	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.77	Vert(LL)	-0.33 14-15	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.95	Vert(TL)	-0.67 14-15	>703	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.54	Horz(TL)	-0.27 19	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 271 lb	

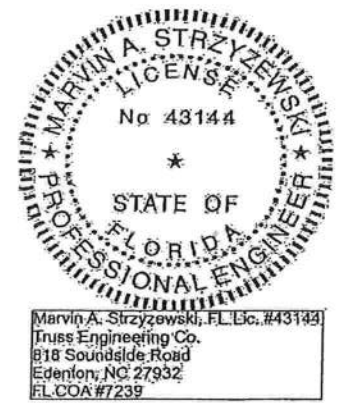
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-9-7 oc purlins.
BOT CHORD 2 X 6 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-7-2 oc bracing.
WEBS 2 X 4 SYP No.2	

REACTIONS (lb/size) 19=3302/0-8-6, 2=825/0-8-0, 12=1550/0-8-6
 Max Horz 12=124(LC 6)
 Max Uplift 19=627(LC 5), 2=898(LC 11), 12=355(LC 7)
 Max Grav 19=3302(LC 1), 2=37(LC 7), 12=1550(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 8-9=-3213/1045, 9-10=-4213/1271, 10-11=-4262/1260, 11-12=-4361/1419, 12-13=0/11, 6-7=-2630/947, 7-8=-2890/1005
 , 1-2=0/14, 2-3=-754/2661, 3-4=-445/154, 4-5=-391/170, 5-6=-1537/588
 BOT CHORD 14-15=-988/3658, 12-14=-1202/3991, 15-16=-587/2815, 18-19=-2551/842, 17-18=0/387, 16-17=-216/1389,
 2-19=-2360/773
 WEBS 11-14=-28/233, 9-14=-5/514, 9-15=-719/351, 8-15=-164/950, 7-15=-163/391, 7-16=-513/309, 6-16=-457/1589,
 6-17=-819/279, 6-17=-343/1320, 5-18=-1379/527, 3-18=-794/2939, 3-19=-2528/890

- NOTES
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 627 lb uplift at joint 19, 898 lb uplift at joint 2 and 355 lb uplift at joint 12.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, F.L.E., #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL COA #7239

November 13, 2007

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513819
DAVENPORT	HSS	SPECIAL	1	1	Job Reference (optional)	
Meranda Homes Inc., Sanford, FL						

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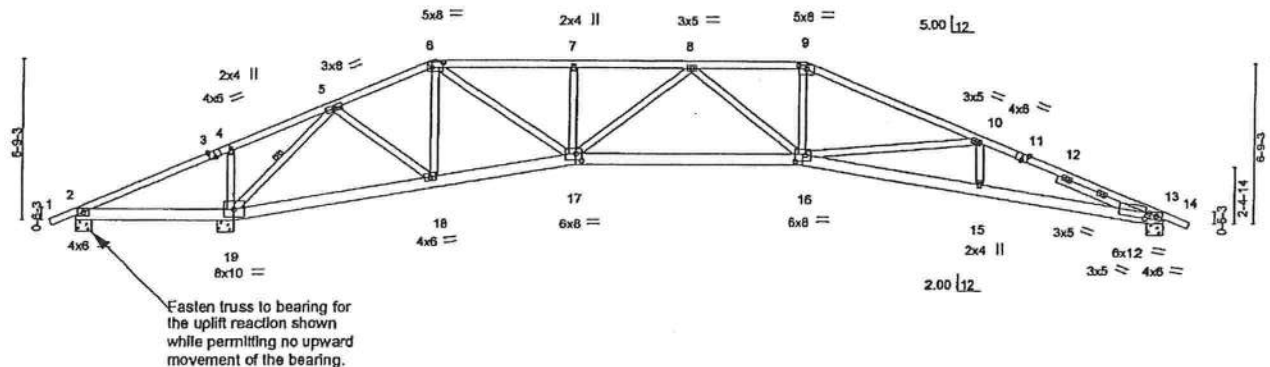
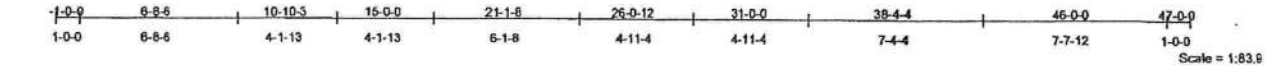


Plate Offsets (X,Y):	[3-0-3-0,Edge]	[8-0-5-12,0-2-8]	[9-0-4-0-0-1-13]	[11-0-3-0,Edge]	[13-0-1-11,0-3-0]	[16-0-5-4-0-3-4]	[17-0-2-12,0-3-8]
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Vdefl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.81	Vert(LL)	-0.36 16-17	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.77	Vert(TL)	-0.73 16-17	>645	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.62	Horz(TL)	-0.30 19	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						Weight: 269 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD 2 X 6 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-8-3 oc bracing.
WEBS 2 X 4 SYP No.2	WEBS 1 Row at midpt 5-19
SLIDER Right 2 X 4 SYP No.2 4-0-13	

REACTIONS (lb/size) 13=1561/0-8-6, 19=3294/0-8-6, 2=829/0-8-0
Max Horz 13=112(LC 6)
Max Uplift 13=345(LC 7), 19=665(LC 5), 2=927(LC 11)
Max Grav 13=1561(LC 1), 19=3294(LC 1), 2=48(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 9-10=3803/1159, 10-11=4334/1396, 11-12=4361/1387, 12-13=4429/1378, 13-14=0/11, 6-7=2748/990,
7-8=2748/990, 8-9=3312/1136, 1-2=0/14, 2-3=783/2559, 3-4=759/2613, 4-5=636/2525, 5-6=1281/507
BOT CHORD 15-16=1168/4071, 13-15=1165/4049, 16-17=844/3150, 18-19=199/109, 17-18=176/1177, 2-19=2321/709
WEBS 10-15=0/222, 10-16=713/350, 8-18=82/244, 8-17=542/235, 7-17=268/250, 6-17=590/1939, 6-18=856/361,
5-18=344/1502, 5-19=3368/1036, 4-19=405/314, 9-18=165/1031

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 13 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 13, 865 lb uplift at joint 19 and 927 lb uplift at joint 2.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mittek connections. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, D58-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

TRENCO
ENGINEERING BY
A MITTEK AFFILIATE

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513821
DAVENPORT	HS7S	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL			7.010 s Oct 16 2007 MTEK Industries, Inc. Tue Nov 13 08:18:02 2007 Page 1			

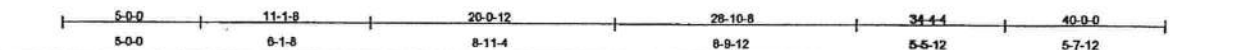
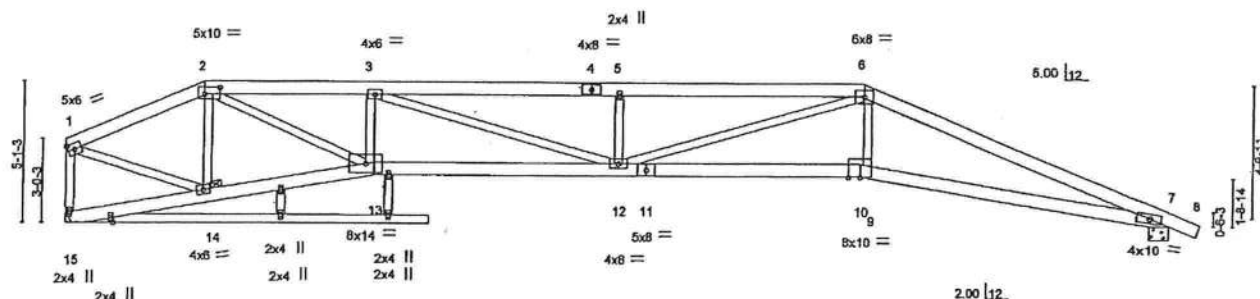
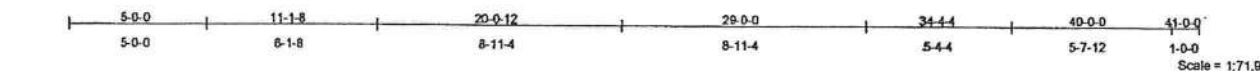


Plate Offsets (X,Y): [2-0-6-12-0-2-12]							
LOADING (psf)	SPACING	2-0-0	CSJ	DEFL	In (loc)	I/defl	L/d
TCLL 16.0	Plates Increase	1.25	TC 0.56	Vert(LL)	-0.51 10-12	>922	240
TCDL 7.0	Lumber Increase	1.25	BC 0.74	Vert(TL)	-1.02 10-12	>463	180
BCLL 10.0	Rep Stress Incr	YES	WB 0.69	Horz(TL)	-0.41 15	n/a	n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)				
							Weight: 279 lb

LUMBER
TOP CHORD 2 X 6 SYP No.2
BOT CHORD 2 X 6 SYP No.2 *Except*
7-10 2 X 6 SYP No.1D, 17-18 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-9 oc bracing.
JOINTS 1 Brace at J(s): 14

REACTIONS (lb/size) 7=1766/0-8-6, 15=1698/Mechanical
Max Horz 7=164(LC 7)
Max Uplift 7=373(LC 5), 15=353(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 6-7=5354/1809, 7-8=0/16, 2-3=4755/1754, 3-4=5899/2145, 4-5=5899/2145, 5-6=5899/2145, 1-2=2139/768, 1-15=1612/612
BOT CHORD 9-10=1634/4675, 7-9=1688/4929, 12-13=1635/4718, 11-12=1645/4758, 10-11=1645/4758, 14-15=24/74, 13-14=656/1952
WEBS 6-9=176/1245, 6-12=428/1444, 5-12=420/373, 3-12=436/1267, 3-13=747/471, 2-13=1123/3154, 2-14=786/402, 1-14=679/2005

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 373 lb uplift at joint 7 and 353 lb uplift at joint 15.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and 8CS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

816 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513822
DAVENPORT	HS65	SPECIAL	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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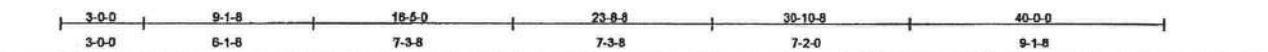
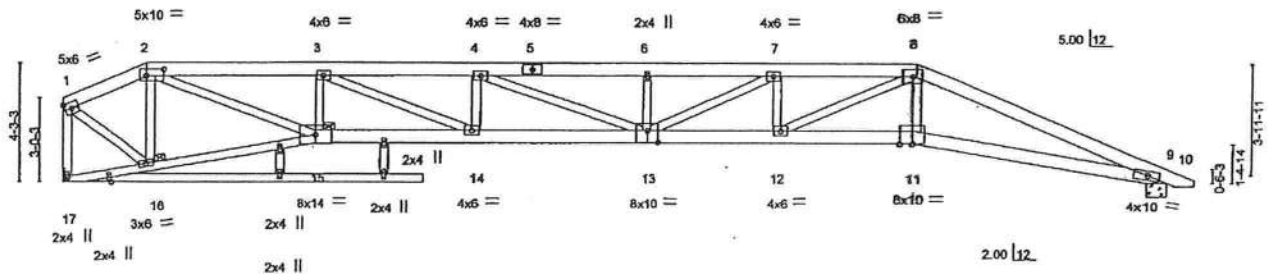
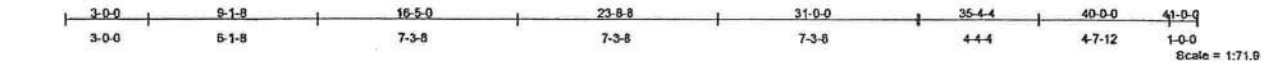


Plate Offsets (X,Y): [2-0-7-4-0-2-12], [13-0-5-0-0-5-0]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d
TCLL 16.0	Plates Increase	1.25	TC 0.38	Vert(LL)	-0.60 13-14	>790	240
TCDL 7.0	Lumber Increase	1.25	BC 0.80	Vert(TL)	-1.19 13-14	>399	180
BCLL 10.0	Rep Stress Incr	YES	WB 0.84	Horz(TL)	-0.45 17	n/a	n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)				
							Weight: 276 lb

LUMBER	BRACING
TOP CHORD 2 X 6 SYP No.2 *Except*	TOP CHORD Structural wood sheathing directly applied or 3-0-2 oc purlins, except end verticals.
2-5 2 X 6 SYP No.1D, 5-8 2 X 6 SYP No.1D	
BOT CHORD 2 X 6 SYP No.1D *Except*	BOT CHORD Rigid ceiling directly applied or 5-6-1 oc bracing.
15-17 2 X 4 SYP No.2, 19-20 2 X 4 SYP No.2	JOINTS 1 Brace at Jt(s): 15, 16
WEBS 2 X 4 SYP No.2	

REACTIONS (lb/size) 9=1745/0-8-6, 17=1699/Mechanical
Max Horz 9=140(LC 7)
Max Uplift 9=372(LC 5), 17=378(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 8-9=5377/1878, 9-10=0/8, 2-3=4787/1754, 3-4=8560/2398, 4-5=8954/2537, 5-6=8954/2537, 6-7=8952/2535, 7-8=8264/2285, 1-2=1542/560, 1-17=1672/609

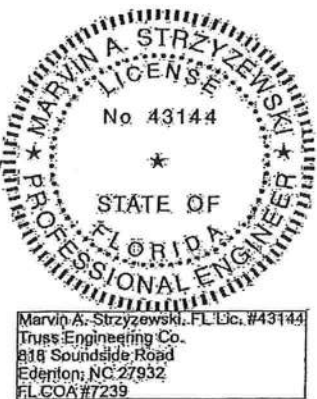
BOT CHORD 9-11=1768/4956, 14-15=1726/4939, 13-14=2340/8560, 12-13=2229/6204, 11-12=1745/4917, 16-17=6/28, 15-16=489/1408

WEBS 3-15=972/509, 2-15=1315/3882, 2-16=1073/510, 1-16=620/1759, 4-14=433/312, 3-14=667/1873, 6-13=287/252, 4-13=151/432, 7-12=570/277, 7-13=279/815, 8-12=537/1662, 8-11=212/1137

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 9 and 378 lb uplift at joint 17.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.

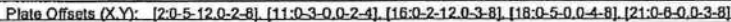
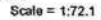
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D51-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Maronda Homes Inc., Sanford, FL

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Weight: 732 lb

BRACING

BOT CHORD

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

Max Horz 13=64(LC 6)

Max Uplift: 13=1416 (LC 6), 24=1181 (LC 5)
Max Cost: 13=2853 (LC 1), 24=2022 (LC 0)

Max Gravity=2857(LC 1), 24=2922(LC 9)

FORCES (lb) - Maximum Compression/Maximum Tension

WEBS

NOTES

- Continued on page 2



November 13, 2007

1

**ENGINEERING BY
TRENCO**

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513623
DAVENPORT	HSGRD1S	SPECIAL	1	3	Job Reference (optional)	

Meronde Homes Inc., Sanford, FL

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NOTES

- 8) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1416 lb uplift at joint 13 and 1181 lb uplift at joint 24.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 119 lb down and 114 lb up at 33-0-0, 108 lb down and 122 lb up at 30-11-4, 108 lb down and 122 lb up at 28-11-4, 108 lb down and 122 lb up at 26-11-4, 108 lb down and 122 lb up at 24-11-4, 108 lb down and 122 lb up at 22-11-4, 108 lb down and 122 lb up at 20-11-4, 108 lb down and 122 lb up at 18-11-4, 108 lb down and 122 lb up at 16-11-4, 108 lb down and 122 lb up at 14-11-4, 108 lb down and 122 lb up at 12-11-4, 108 lb down and 122 lb up at 10-11-4, 86 lb down and 1 lb up at 8-11-4, 86 lb down and 1 lb up at 6-11-4, 86 lb down and 1 lb up at 4-11-4, and 86 lb down and 1 lb up at 2-11-4, and 118 lb down and 1 lb up at 1-0-0 on top chord, and 690 lb down and 518 lb up at 33-0-0, 98 lb down at 30-11-4, 98 lb down at 28-11-4, 98 lb down at 26-11-4, 98 lb down at 24-11-4, 98 lb down at 22-11-4, 98 lb down at 20-11-4, 98 lb down at 18-11-4, 98 lb down at 16-11-4, 98 lb down at 14-11-4, 98 lb down at 12-11-4, 98 lb down at 10-11-4, 105 lb down and 84 lb up at 8-11-4, 105 lb down and 84 lb up at 6-11-4, 105 lb down and 84 lb up at 4-11-4, and 105 lb down and 84 lb up at 2-11-4, and 105 lb down and 84 lb up at 0-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 11-14=23, 2-11=23, 1-2=23, 13-16=20, 16-20=20, 20-21=20, 21-24=20

Concentrated Loads (lb)

Vert: 11=119(B) 8=108(B) 2=86(B) 4=86(B) 23=105(B) 25=86(B) 26=86(B) 27=86(B) 28=108(B) 29=108(B) 30=108(B) 31=108(B) 32=108(B) 33=108(B) 34=108(B) 35=108(B) 36=108(B) 37=108(B) 38=690(B) 39=98(B) 40=98(B) 41=98(B) 42=98(B) 43=98(B) 44=98(B) 45=98(B) 46=98(B) 47=98(B) 48=98(B) 49=98(B) 50=105(B) 51=105(B) 52=105(B) 53=105(B)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.

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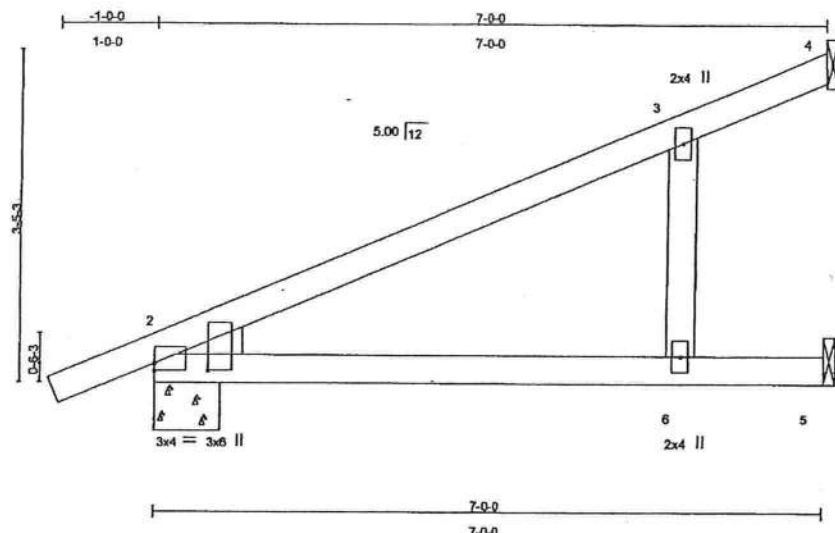
ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Eden, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513824
DAVENPORT	J	MONO TRUSS	29	1	Job Reference (optional)	

Maronde Homes Inc., Sanford, FL

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Scale = 1:20.6

Plate Offsets (X,Y):		[2-0-0-0-0-15] [2-0-0-14-0-6-12]	
LOADING (psf)	SPACING	2-0-0	CSI
TCLL 18.0	Plates Increase	1.25	TC 0.39
TCDL 7.0	Lumber Increase	1.25	BC 0.44
BCLL 10.0	Rep Stress Incr	YES	WB 0.02
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)
			DEFL
			In (loc) I/defl L/d
			Vert(LL) -0.12 2-6 >639 240
			Vert(TL) -0.28 2-6 >309 180
			Horz(TL) 0.01 4 n/a n/a
			PLATES GRIP
			MT20 244/190
			Weight: 28 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purfins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=132/Mechanical, 2=355/0-8-6, 5=145/Mechanical
Max Horz 2=143(LC 6)
Max Uplift 4=-19(LC 6), 2=-127(LC 6), 5=-72(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/10, 2-3=-106/26, 3-4=-17/48
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-8=-17/165

NOTES (5)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 4, 127 lb uplift at joint 2 and 72 lb uplift at joint 5.
- 5) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 4 and 5.

LOAD CASE(S) Standard



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November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

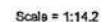
Design valid for use only with Mitek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.18	Vert(LL)	-0.02	2-4	>999	240	MT20
TCOL 7.0	Lumber Increase	1.25	BC 0.20	Vert(TL)	-0.06	2-4	>937	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						Weight: 18 lb

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

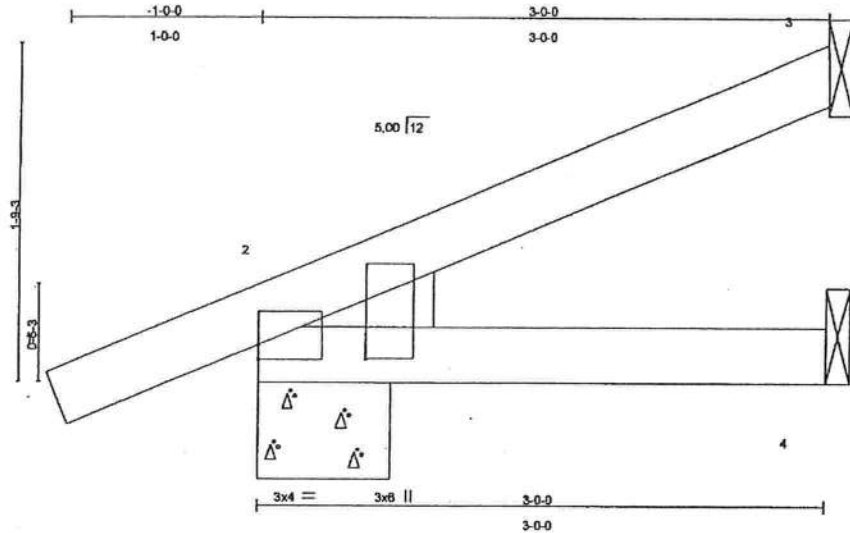
LOAD CASE(S) Standard



WARNING - verify design parameters data shown on this label for each unique parameter and location. This design is valid for use only with MITEC connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component in a responsible manner of building design is the responsibility of the fabricator. The fabricator is responsible for the design of the building component. Additional bracing may be required to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/PTI Quality Criteria, D58-B9 and BC511 Building Component Safety Information, available from Trust Pole Institute, 583 D'Oroville Drive, Madison, WI 53719.

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513826
DAVENPORT	J2	JACK	4	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL			7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:18:05 2007 Page 1			



Scale = 1:10.4

Plate Offsets (X,Y): [2-0-0-0-0-15] [2-0-0-14-0-8-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

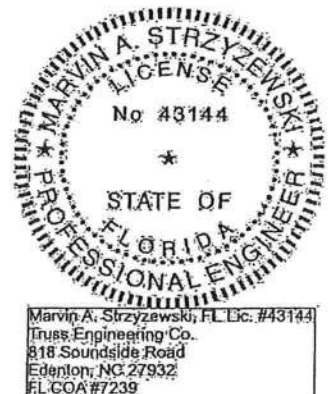
REACTIONS (lb/size) 3=43/Mechanical, 2=193/0-8-6, 4=52/Mechanical
Max Horz 2=74(LC 6)
Max Uplift 3=38(LC 6), 2=112(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/10, 2-3=37/15
BOT CHORD 2-4=0/0

NOTES (5)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3 and 112 lb uplift at joint 2.
- 5) Attach with (2) 18d Common Toe-Nails (0.182"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



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November 13, 2007

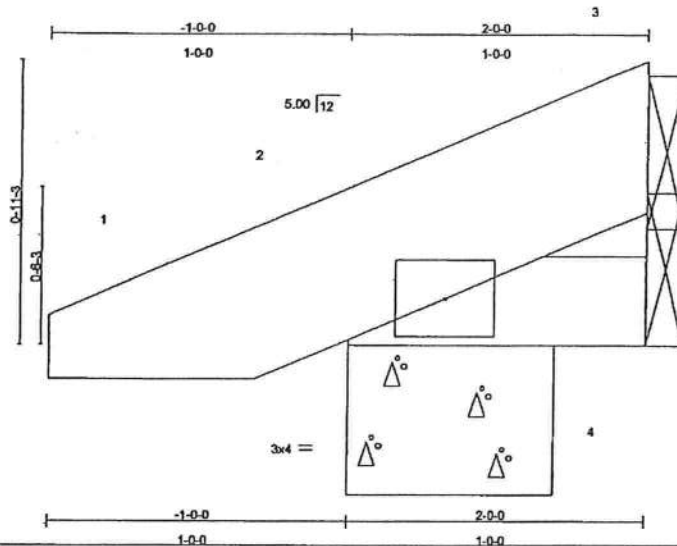
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED REFERRED PAGE MI-7473 BEFORE USE.

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and 8CS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

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Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513827
DAVENPORT	JS	JACK	4	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL		7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:18:05 2007 Page 1				



Scale = 1/8"

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.02	Vert(LL)	-0.00	2	>889	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>889	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 7 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purtins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=86/0-8-8, 4=18/Mechanical, 3=8/Mechanical
Max Horz 2=32(LC 6)
Max Uplift 2=62(LC 6), 3=11(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/7, 2-3=17/2
BOT CHORD 2-4=0/0

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 2 and 11 lb uplift at joint 3.
- 5) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



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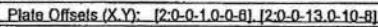
November 13, 2007

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LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.2

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 9-7-1 oc bracing.

REACTIONS (lb/size) 5=547/Mechanical, 2=519/1-0-10
Max Horz 2=142(LC 3)
Max Up lift 5=297(LC 7), 2=256(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/11, 2-7=-877/337, 7-8=-843/380, 3-8=-812/338, 3-9=-119/63, 4-9=-64/3, 4-5=-128/160
BOT CHORD	2-10=-424/795, 10-11=-424/795, 6-11=-424/795, 6-12=-424/795, 5-12=-424/795
WEBS	3-6=0/277, 3-5=-776/402

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.80 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 5 and 258 lb uplift at joint 2.
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 44 lb up at 4-4-12, 44 lb up at 4-4-12, 50 lb down and 117 lb up at 7-2-11, 50 lb down and 117 lb up at 7-2-11, and 44 lb down at 1-6-12, and 44 lb down at 1-6-12 on top chord, and 22 lb up at 1-6-12, 22 lb up at 1-6-12, 12 lb down at 4-4-12, 12 lb down at 4-4-12, and 52 lb down at 7-2-11, and 52 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular; Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=46, 2-5=40
Concentrated Loads (lb)
Vert: 6=6(F=3, B=3) 9=101(F=50, B=50) 10=43(F=22, B=22) 11=24(F=12, B=12) 12=104(F=52, B=52)



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November 13, 2007

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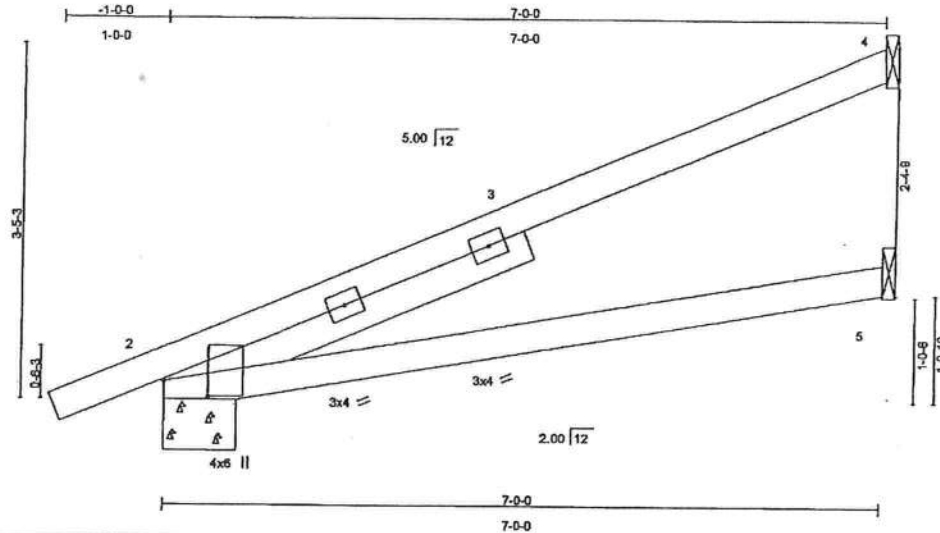
ENGINEERING BY
TRENCO
A MITCHELL GROUP COMPANY

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Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513834
DAVENPORT	JS	SPECIAL	11	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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Scale = 1:19.3

Plate Offsets (X,Y): [2.0-1-13.0-5-2]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.47	Vert(LL)	-0.12	2-5	>690	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.45	Vert(TL)	-0.30	2-5	>276	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.02	4	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 29 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
SLIDER Left 2 X 4 SYP No.2 3-8-9

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 4=154/Mechanical, 2=351/0-8-8, 5=138/Mechanical
Max Horz 2=141(LC 6)
Max Uplift 4=140(LC 6), 2=109(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/6, 2-3=88/0, 3-4=78/47
BOT CHORD 2-5=18/18

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSVTPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 4 and 109 lb uplift at joint 2.
- 6) Attach with (2) 18d Common Toe-Nails (0.162"x3.5") at joints 4 and 5.

LOAD CASE(S) Standard



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November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.

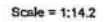
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7,010 s Oct 16 2007 MiTek Industries, Inc. Tue Nov 13 08:18:06 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	l/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.03	2-5	>999	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.23	Vert(TL)	-0.08	2-5	>772	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.01	4	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						Weight: 21 lb

TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

- 1) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCOL=4.2psf$; $BCOL=6.0psf$, Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 4 and 98 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 4 and 5.

November 13, 2007

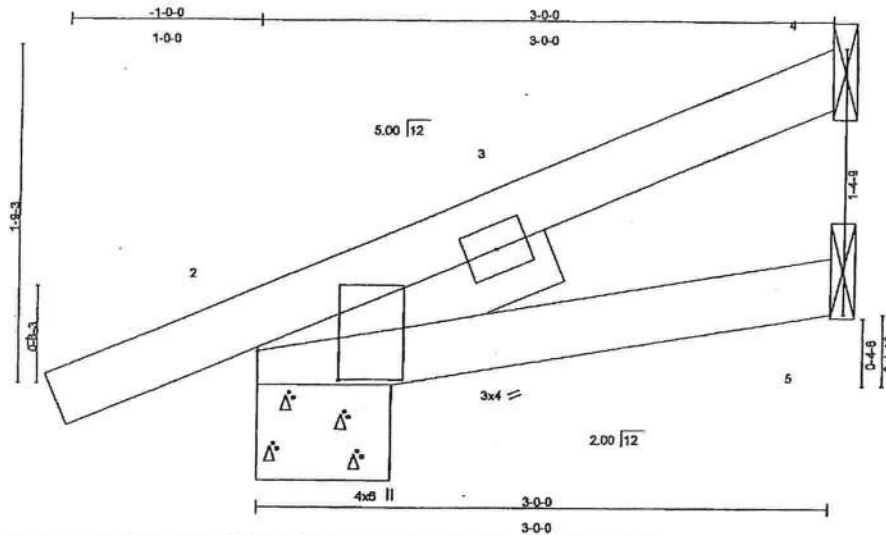
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MIT-7473 BEFORE USE.

Design valid for use only with Mitex connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP11 Quality Criteria, D58-89 and BC311 Building Component Safety Information, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513838
DAVENPORT	JS2	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						
7,010 s Oct 16 2007 Mikek Industries, Inc. Tue Nov 13 08:18:09 2007 Page 1						



Scale = 1/10.4

Plate Offsets (X,Y): [2:0-1-13:0-5-2]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 18.0	Plates Increase	1.25	TC 0.08	Vert(LL)	-0.00	2-5 >999
TCDL 7.0	Lumber Increase	1.25	BC 0.08	Vert(TL)	-0.01	2-5 >999
BCCL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	4 n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)			n/a
						Weight: 14 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
SLIDER Left 2 X 4 SYP No.2 1-7-9

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=57/Mechanical, 2=185/0-8-6, 5=58/Mechanical
Max Horz 2=72(LC 6)
Max Uplift 4=53(LC 6), 2=88(LC 6)

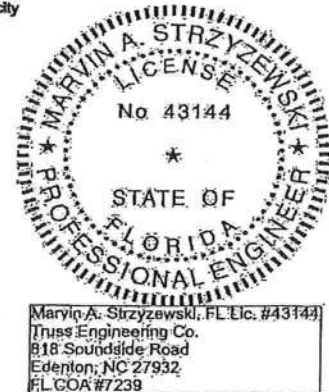
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/6, 2-3=42/0, 3-4=28/16
BOT CHORD 2-5=8/8

NOTES (8)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 4 and 88 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 4 and 5.

LOAD CASE(S) Standard



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FL GOA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotof Drive, Madison, WI 53719.

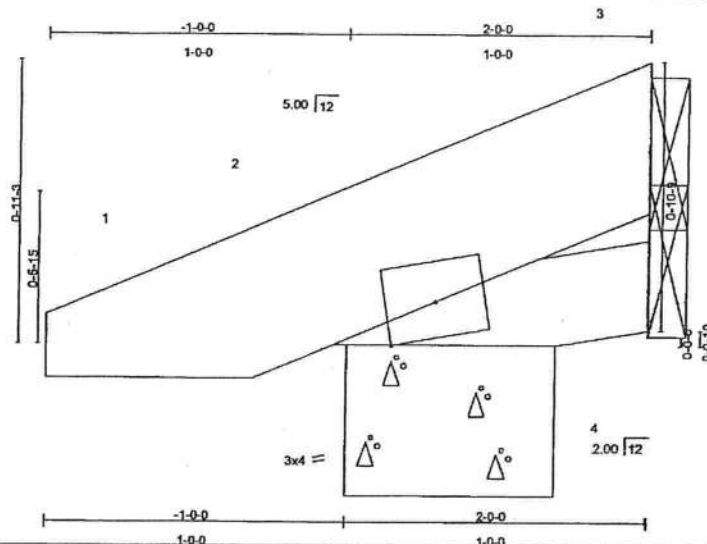
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513837
DAVENPORT	J63	SPECIAL	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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Scale = 1/8"

LOADING (psf)	SPACING	CSI	DEFL	In (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.06	Vert(LL)	-0.00	2	>999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(TL)	-0.00	2	>999		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TPI2002							
							Weight: 7 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=19/Mechanical, 2=84/0-8-6, 3=11/Mechanical
Max Horz 2=29(LC 6)
Max Uplift 2=54(LC 6), 3=14(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/2, 2-3=18/4
BOT CHORD 2-4=3/3

NOTES (7)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSVIPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 2 and 14 lb uplift at joint 3.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



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November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE ME-7473 BEFORE USE.
Design valid for use only with MITek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSVIPI Quality Criteria, D58-87 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
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A MITek Affiliate

818 Soundside Road
Edenton, NC 27932

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.10	2-5	>816	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.24	2-5	>328	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.01	Horz(TL)	0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						Weight 34 lb	

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/6, 2-3=85/0, 3-4=74/44
BOT CHORD 2-5=17/17
WEBS 4-5=146/165

- 1) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 2 and 93 lb uplift at joint 5.

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November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-74YS BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Mitek designer. Bracing shown is for lateral support of individual web member only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSII/PTI Quality Criteria, D38-89 and BCS11 Building Component Safety Information, available from Truss Plate Institute, 583 O'Conor Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MILK ADRIANO
518 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513639
DAVENPORT	USGRD1	SPECIAL	1	1	Job Reference (optional)	
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Maronda Homes Inc., Sanford, FL

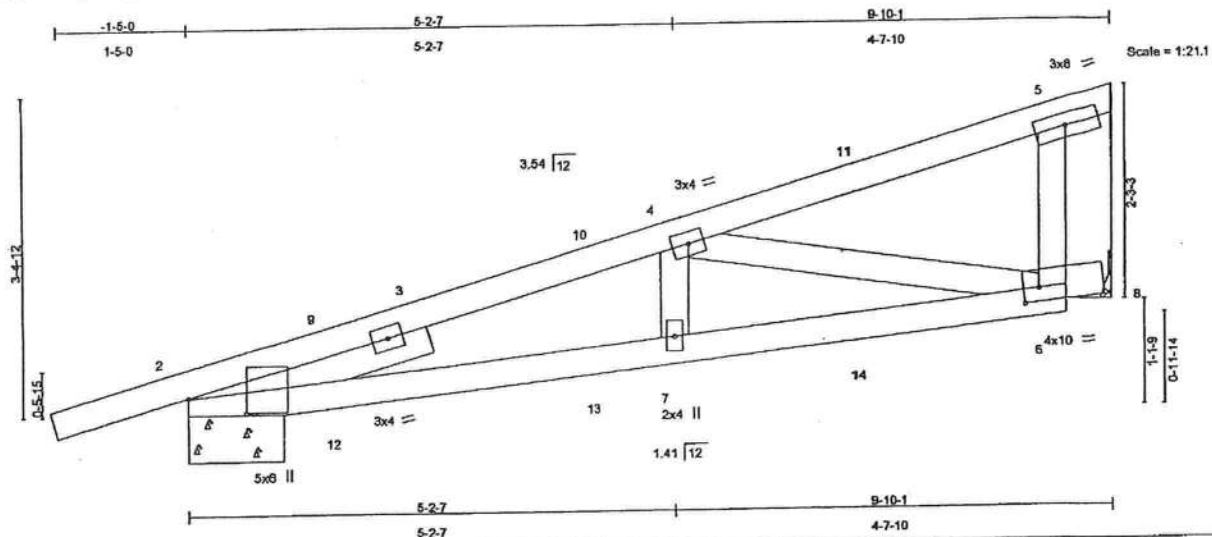


Plate Offsets (X, Y): [2-0-2-0-0-7-6], [6-0-2-0-0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCCL	16.0	Plates Increase	1.25	TC 0.32	Vert(LL)	-0.04	6-7	>999	240	MT20
TCDL	7.0	Lumber Increase	1.25	BC 0.39	Vert(TL)	-0.08	6-7	>999	180	
BCCL	10.0	Rep Stress Incr	NO	WB 0.24	Horz(TL)	0.02	8	n/a	n/a	
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 50 lb	244/190

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2 *Except*
 5-6 2 X 6 SYP No.2
 SLIDER Left 2 X 4 SYP No.2 2-7-11

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-6-7 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 7-5-5 oc bracing.

REACTIONS (lb/size) 2=550/0-11-14, 8=562/Mechanical
 Max Horz 2=133(LC 3)
 Max Uplift 2=273(LC 7), 8=334(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/6, 2-9=-1307/640, 3-9=-1272/657, 3-10=-1272/658, 4-10=-1241/632, 4-11=-348/215, 5-11=-288/149,
 5-8=-189/462
 BOT CHORD 2-12=-703/1201, 12-13=-697/1208, 7-13=-697/1222, 7-14=-706/1203, 6-14=-701/1227
 WEBS 4-7=0/254, 4-6=-913/501, 5-8=-596/368, 6-8=-172/203

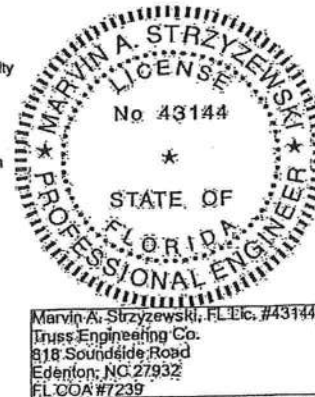
NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise);
 Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSVIPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 2 and 334 lb uplift at joint 8.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 11 lb down and 65 lb up at 4-4-12, 11 lb down and 65 lb up at 4-4-12, 61 lb down and 132 lb up at 7-2-11, 61 lb down and 132 lb up at 7-2-11, and 43 lb down at 1-6-13, and 43 lb down at 1-6-13 on top chord, and 21 lb up at 1-6-13, 21 lb up at 1-6-13, 18 lb down at 4-4-12, 18 lb down at 4-4-12, and 58 lb down at 7-2-11, and 58 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular; Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vert 1-5=-46, 2-6=-40

Continued on page 2



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 818 Soundside Road
 Edenton, NC 27932
 FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITek REFERENCE PAGE MD-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSVIPI Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 O'Donohue Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A MITek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513839
DAVENPORT	JSGRD1	SPECIAL	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 10=-22(F=-11, B=-11) 11=-121(F=-61, B=-61) 12=41(F=21, B=21) 13=-36(F=-18, B=-18) 14=-116(F=-58, B=-58)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and IBC11 Building Component Safety Information available from Truss Plate Institute, 583 O'Neale Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Eden, NC 27032

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513843
DAVENPORT	51	SPECIAL	8	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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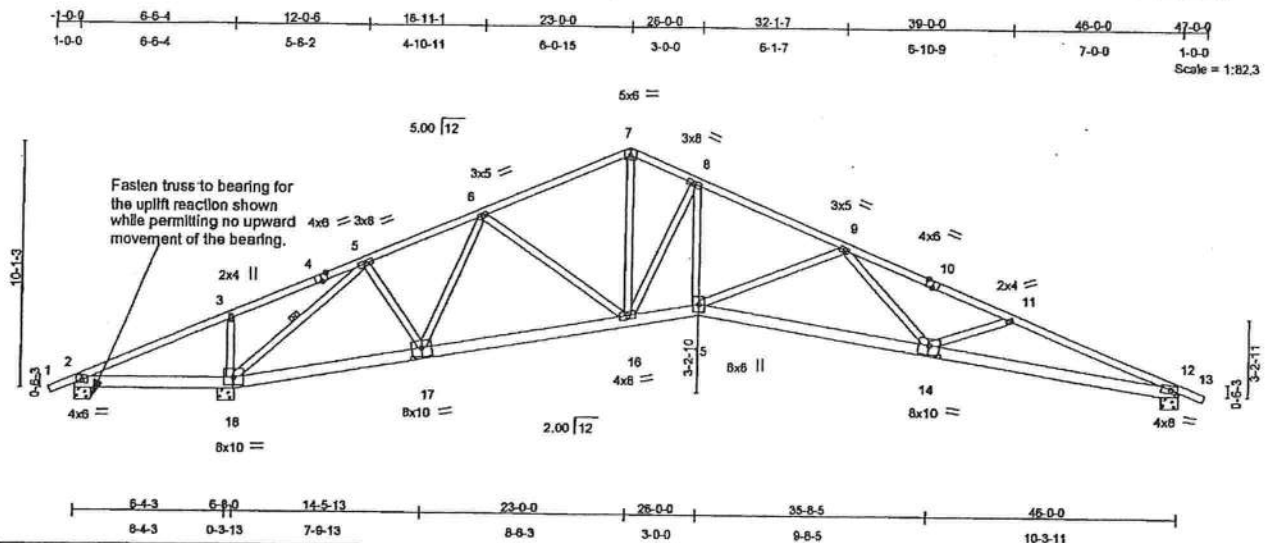


Plate Offsets (X,Y): [4:0-3-0-Edge], [10:0-3-0-Edge], [12:0-3-5-0-2-0], [14:0-6-0-0-4-8], [17:0-5-0-0-4-8]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl
TCLL 16.0	Plates Increase	1.25	TC 0.77	Vert(LL)	-0.38 14-15	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.95	Vert(TL)	-0.76 14-15	>816
BCLL 10.0	Rep Stress Incr	YES	WB 0.87	Horz(TL)	-0.32 18	n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)			
						Weight: 278 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-6-8 oc purfins.
BOT CHORD 2 X 6 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-4-2 oc bracing.
WEBS 2 X 4 SYP No.2 *Except*	WEBS 1 Row at midpt 5-18
8-18 2 X 4 SYP No.1D	

REACTIONS (lb/size) 2=625/0-8-0, 18=3055/0-8-6, 12=1598/0-8-6
Max Horz 12=182(LC 6)
Max Uplift 2=653(LC 11), 18=583(LC 6), 12=390(LC 7)
Max Grav 2=64(LC 7), 18=3055(LC 1), 12=1598(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/14, 2-3=618/2149, 3-4=466/2002, 4-5=455/2090, 5-6=1386/489, 6-7=2057/686, 7-8=2021/710,
8-9=2888/854, 9-10=4278/1198, 10-11=4337/1186, 11-12=4570/1410, 12-13=0/11
BOT CHORD 2-18=1893/647, 17-18=411/727, 16-17=240/1800, 15-16=401/2605, 14-15=845/3528, 12-14=1191/4183
WEBS 3-18=391/323, 5-18=3513/938, 5-17=131/1055, 6-17=799/305, 6-16=614/70, 7-16=358/1318, 8-16=1816/451,
8-15=308/1593, 9-15=930/458, 9-14=54/797, 11-14=178/331

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 12 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 653 lb uplift at joint 2, 583 lb uplift at joint 18 and 390 lb uplift at joint 12.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-1473 BEFORE USE.
Design valid for use only with MITek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, D58-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513844
DAVENPORT	T1	COMMON	8	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

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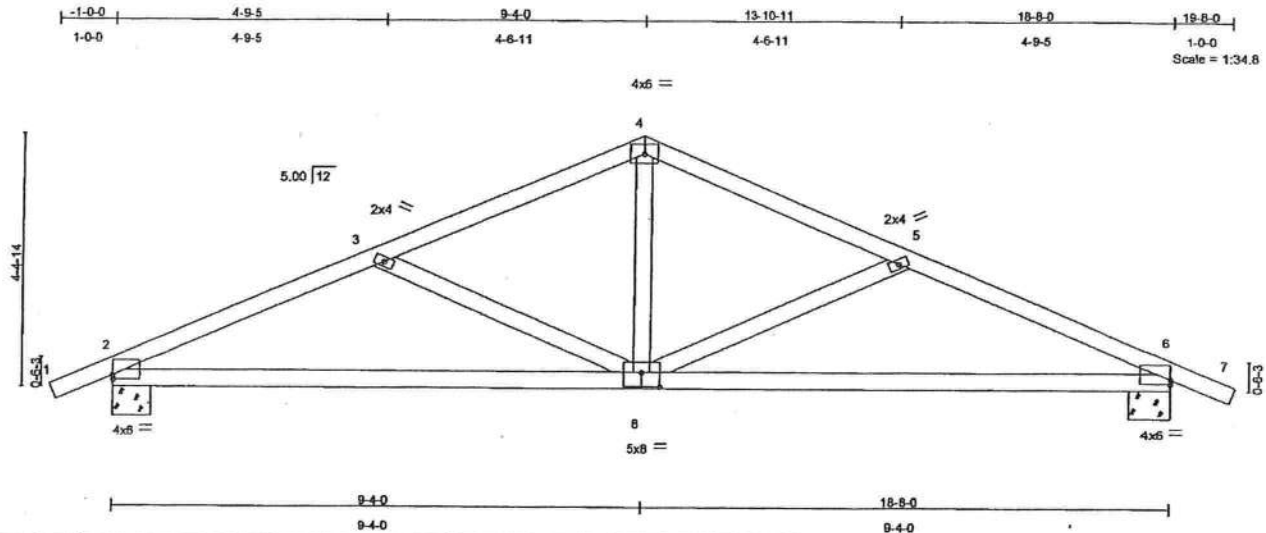


Plate Offsets (X,Y): [2-0-0-1-0-0-15] [6-0-0-1-0-0-15] [8-0-4-0-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (oc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.27	Vert(LL)	-0.13	6-8	>999	240	MT20
TCCL 7.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.30	6-8	>713	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.11	Horz(TL)	0.03	6	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 82 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=838/0-8-6, 6=838/0-8-8
Max Horz 2=73(LC 6)
Max Uplift 2=230(LC 6), 6=230(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/10, 2-3=1276/426, 3-4=1056/313, 4-5=1056/313, 5-6=1276/426, 6-7=0/10
BOT CHORD 2-8=293/1131, 6-8=293/1131
WEBS 3-8=246/193, 4-8=37/573, 5-8=246/193

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2 and 230 lb uplift at joint 6.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITEK Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513845
DAVENPORT	T2	COMMON	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

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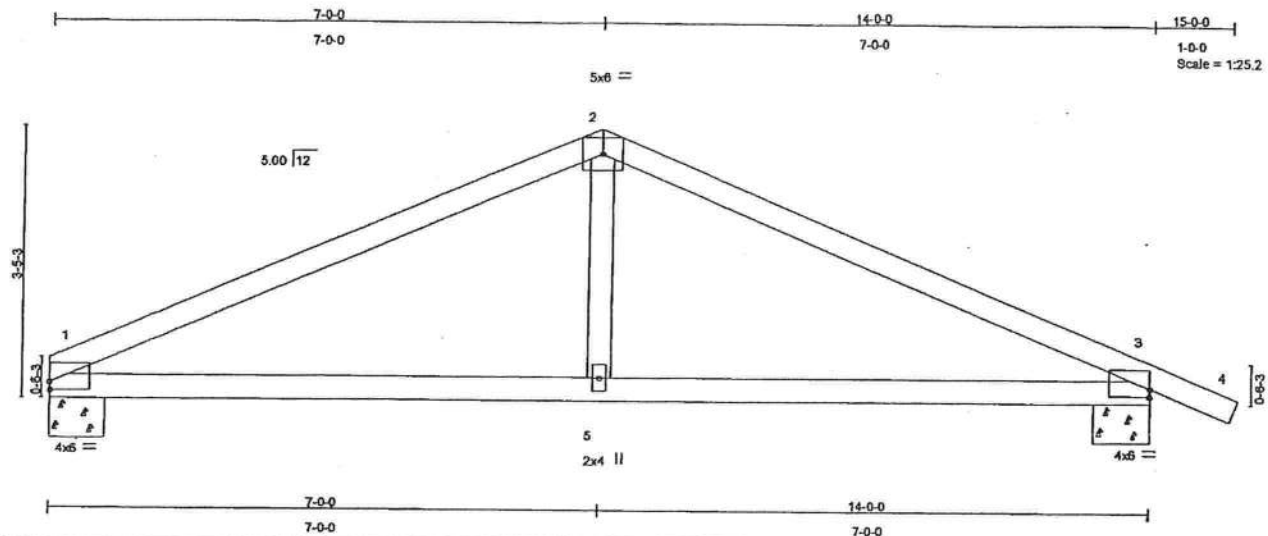


Plate Offsets (X,Y): [1:0-0-1-0-1-3], [3:0-0-1-0-1-3]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCCL 16.0	Plates Increase 1.25	TC 0.31	Vert(LL)	-0.08	1-5	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.53	Vert(TL)	-0.15	1-5	>999	180		
BCCL 10.0	Rep Stress Incr YES	WB 0.06	Horz(TL)	0.02	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002	(Matrix)							
									Weight: 50 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purfins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 1=570/0-8-6, 3=641/0-8-6
 Max Horz 1=83(LC 7)
 Max Uplift 1=109(LC 6), 3=193(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=862/268, 2-3=877/268, 3-4=0/10
 BOT CHORD 1-5=137/746, 3-5=137/746
 WEBS 2-5=0/332

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 1 and 193 lb uplift at joint 3.

LOAD CASE(S) Standard



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November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, B58-81 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERED BY
TRENCO
 A MITek Affiliate

818 Soundside Road
 Edenton, NC 27932

Maronda Homes Inc., Sanford, FL

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Weight: 7 lb

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Mitek design. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSIP/TPI Quality Criteria, DSI-89 and BCSI Building Component Safety Information, available from Truss Plate Institute, 583 O'Donofre Drive, Madison, WI 53719.

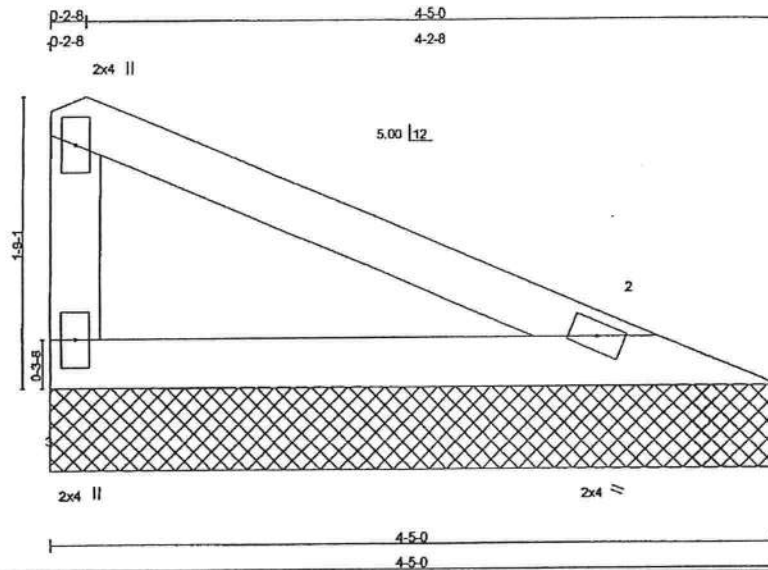
ENGINEERING BY
TRENCO
A MITEC AFFILIATE

818 Soundside Road
Eden, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513857
DAVENPORT	V2S	VALLEY	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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Scale: 1"=1'

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.13	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.12	Vert(TL)	n/a	-	n/a		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	2	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 14 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=152/4-5-0, 2=152/4-5-0
Max Horz 3=81(LC 7)
Max Uplift 3=50(LC 7), 2=25(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-3=-81/82, 1-2=-46/25
BOT CHORD 2-3=-0/76

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at Joint 3 and 25 lb uplift at Joint 2.

LOAD CASE(S) Standard



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FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D38-87 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

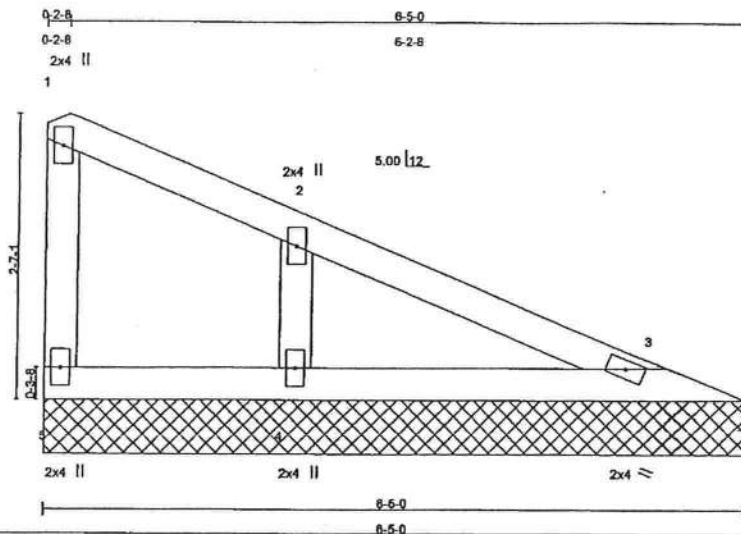
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513858
DAVENPORT	V3S	GABLE	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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Scale = 1:18.1

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.10	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.08	Vert(TL)	n/a	-	n/a		
BCLL 10.0	Rep Stress Incr	YES	WB 0.03	Horz(TL)	0.00	3	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 23 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=48/6-5-0, 3=118/6-5-0, 4=310/6-5-0
Max Horz 5=96(LC 7)
Max Uplift 5=18(LC 7), 4=102(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-5=-26/29, 1-2=-28/5, 2-3=-99/36
BOT CHORD 4-5=-0/117, 3-4=-0/117
WEBS 2-4=-166/184

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 5 and 102 lb uplift at joint 4.

LOAD CASE(S) Standard



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FL.GOA #7239

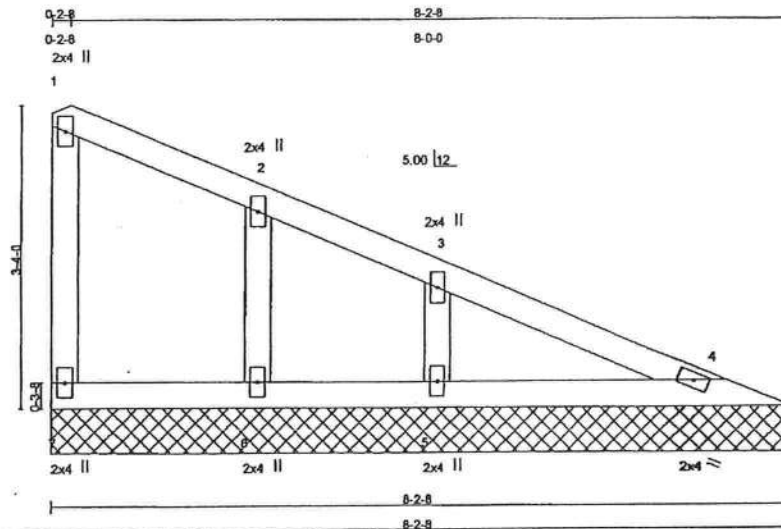
November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITek REFERENCE PAGE MI-1473 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the sector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D38-87 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513859
DAVENPORT	V4S	GABLE	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL			7,010 s Oct 18 2007 Mitek Industries, Inc. Tue Nov 13 08:18:22 2007 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.08	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.07	Vert(TL)	n/a	-	n/a		
BCLL 10.0	Rep Stress Incr	YES	WB 0.02	Horz(TL)	0.00	4	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrx)						Weight: 33 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

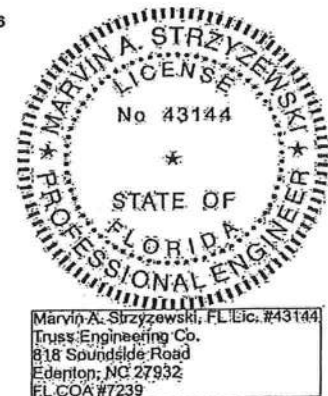
REACTIONS (lb/size) 7=80/8-2-8, 4=113/8-2-8, 6=166/8-2-8, 5=273/8-2-8
Max Horz 7=127(LC 7)
Max Uplift 7=26(LC 7), 6=54(LC 7), 5=90(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-7=43/46, 1-2=25/13, 2-3=66/12, 3-4=139/33
BOT CHORD 6-7=0/152, 5-6=0/152, 4-5=0/152
WEBS 2-8=89/96, 3-5=146/159

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 7, 54 lb uplift at joint 6 and 90 lb uplift at joint 5.

LOAD CASE(S) Standard



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Truss Engineering Co.
818 Spaulds Road
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FL COA #7239

November 13, 2007

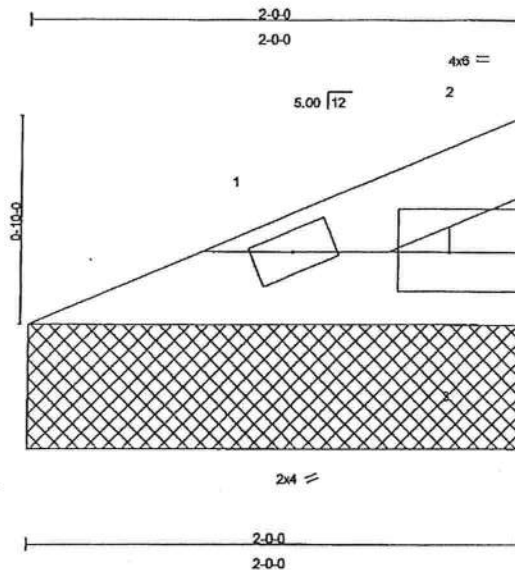
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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ENGINEERING BY
TRENCO
A Mitek Alliance
818 Spaulds Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513860
DAVENPORT	V6	VALLEY	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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Scale: 1.5"=1'

Plate Offsets (X,Y): [2:Edge,0-1-14]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	Vdefl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.01	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00		n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 5 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=37/2-0-0, 3=37/2-0-0
Max Horz 1=19(LC 6)
Max Uplift 1=4(LC 6), 3=14(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=15/8, 2-3=20/25
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 1 and 14 lb uplift at joint 3.

LOAD CASE(S) Standard



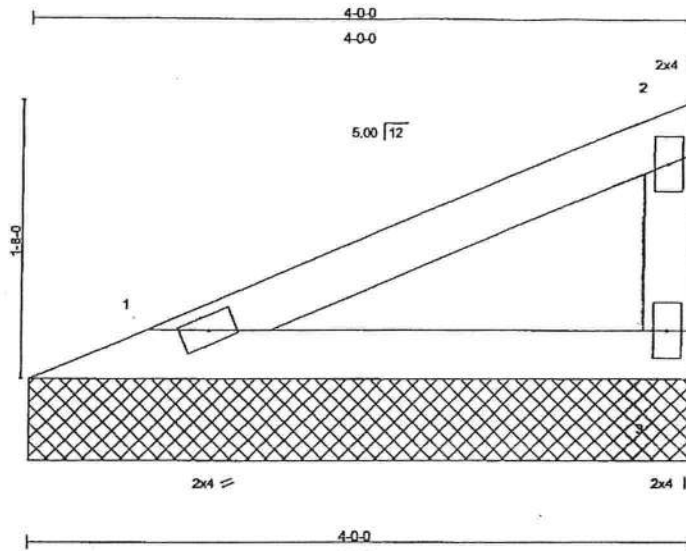
November 13, 2007

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ENGINEERING BY
TRENCO
A MITek Alliance

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513861
DAVENPORT	V7	VALLEY	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 11:45:33 2007 Page 1



Scale: 1"=1'

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.10	Ver(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.09	Ver(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00		n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=134/4-0-0, 3=134/4-0-0
Max Horz 1=54(LC 6)
Max Uplift 1=22(LC 6), 3=44(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=40/22, 2-3=72/81
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) **WARNING:** Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 44 lb uplift at joint 3.

LOAD CASE(S) Standard



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818 Soundside Road
Edenton, NC 27932
FL COR #7239

November 13, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. The design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TPI1 Quality Criteria, DSB-89 and CCSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

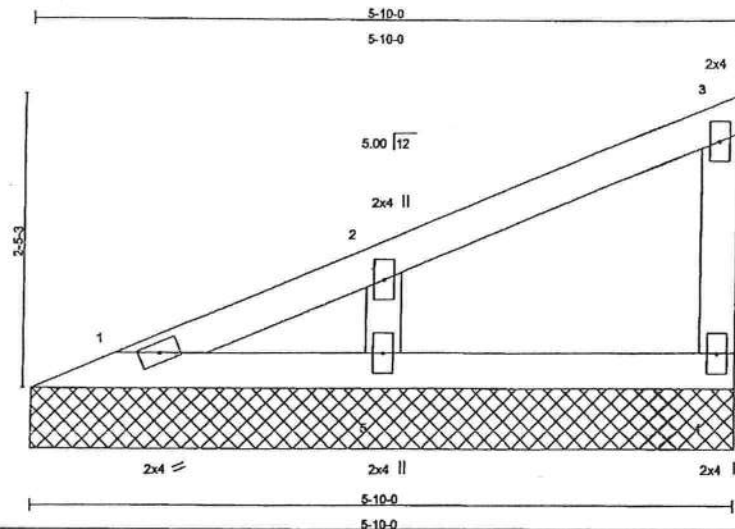
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513882
DAVENPORT	V8	VALLEY	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

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Scale = 1:16.4

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.07	Vert(TL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.08	Vert(TL)	n/a	-	n/a	999		
BCCL 10.0	Rep Stress Incr	YES	WB 0.02	Horz(TL)	0.00	-	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matlrx)							
									Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=83/5-10-0, 4=94/5-10-0, 5=269/5-10-0
Max Horz 1=86(LC 6)
Max Uplift 4=31(LC 6), 5=88(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-99/26, 2-3=-31/15, 3-4=-51/57
BOT CHORD 1-5=0/0, 4-5=0/0
WEBS 2-5=-144/161

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 4 and 88 lb uplift at joint 5.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-B9 and BC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate

818 Soundside Road
Edenton, NC 27932

FAX: (352) 372-2721

COLUMBIA COUNTY FLORIDA DEPARTMENT OF BUILDING AND ZONING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-140

Building permit No. 000027430

Use Classification SFD, UTILITY

Fire: 57.78

Permit Holder THEODORE BROCK

Waste: 150.75

Owner of Building MARONDA HOMES

Total: 208.53

Location: 353 SW MULBERRY DR., LAKE CITY, FL

Date: 01/13/2009

Tony Dick

Building Inspector

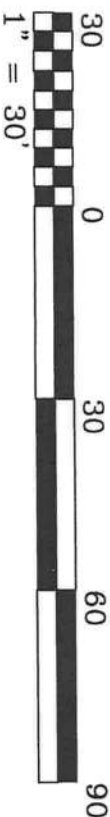
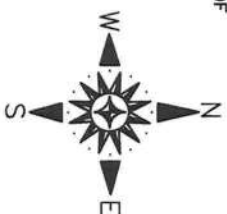


POST IN A CONSPICUOUS PLACE
(Business Places Only)

LEGAL DESCRIPTION:
LOT TWENTY-EIGHT (28) OF TIMBERLANDS PHASE
1st AS PER PLAT THEREOF, AS RECORDED IN PLAT
BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF
COLUMBIA COUNTY, FLORIDA.

BOUNDARY SURVEY

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA



27430

CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT
25', REAR 15', SIDE 10'

BEARING NOTE:

BEARINGS SHOWN HEREON ARE REFERRED TO AN
ASSUMED VALUE OF S. 89°43'38" E., FOR THE
CENTERLINE OF S.W. MULBERRY DRIVE.

BENCHMARK NOTE:

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A
8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS
INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING
(PLATTING SURVEYOR) DATUM UNKNOWN.

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

FLOOD NOTE:

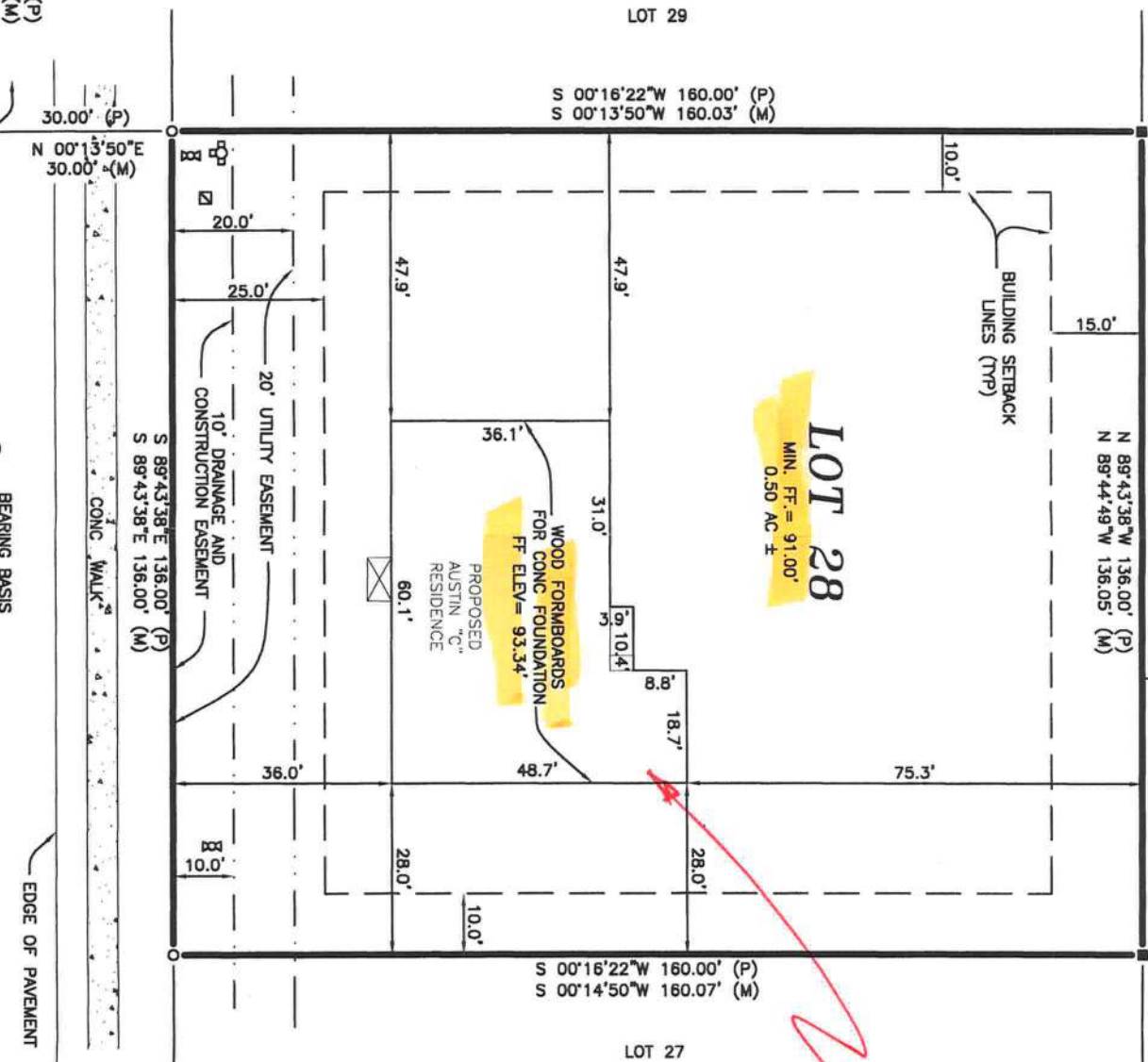
IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS DETERMINED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

SW MULBERRY DRIVE

±21' ASPHALT ROAD
60' RIGHT-OF-WAY

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.



LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- = FOUND 1/2" REBAR & CAP L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- ✕ = FOUND NAIL & DISK P.S.M. 5757
- = FOUND 4" x 4" CONC. MON. P.S.M. 5757
- ⊞ = CATV RISER
- ⊞ = TELEPHONE PEDESTAL
- ⊞ = WOOD POWER POLE
- ⊞ = FIRE HYDRANT
- ⊞ = WATER VALVE

ABBREVIATIONS:

- A/C = AIR CONDITIONER
- ASPH = ASPHALT
- C = CALCULATED FROM MEASURED
- CATV = CABLE TELEVISION
- C/B = CONCRETE BLOCK
- CLF = CHAIN LINK FENCE
- CM = CONCRETE MONUMENT
- CONC = CONCRETE
- ELEC = ELECTRIC
- ELEV = ELEVATION
- FND = FOUND
- FNC = FENCE
- LB = LICENSED SURVEYOR BUSINESS
- (M) = FIELD MEASURED
- MH = MANHOLE
- O.U. = OVERHEAD UTILITIES
- P = PLAT
- PB = PLAT BOOK
- P.U.E. = PUBLIC UTILITIES EASEMENT
- TRANS = TRANSFORMER
- TYP = TYPICAL
- WM = WATER METER
- WV = WATER VALVE

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G17-9, FLORIDA ADMINISTRATIVE CODE.

BY: *James E. Brinkman* PSM - FLA. CERT# 5582

DATE: 12/31/08



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'
DATE: 12/30/2008
FIELD WORK COMPLETED ON 12/29/2008
PREPARED FOR: MARONDA

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL
CHECKED BY: J.B.

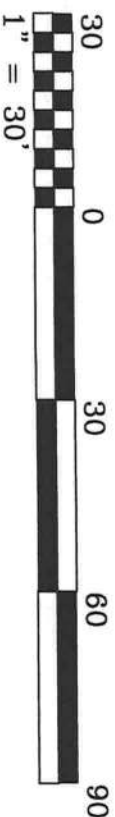
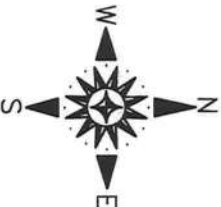
FIELDBOOK 99, PAGE 35

DRAWING NUMBER 199-08

LEGAL DESCRIPTION:
LOT FOURTY (40) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

BOUNDARY SURVEY

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



BEARING BASIS
S 89°43'38"E 717.21'(P)
S 89°43'38"E 717.23'(M)

CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

BENCHMARK NOTE:

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN.

CURVE TABLE			
CURVE	RADIUS	TANGENT	LENGTH
C26(P)	200.00'	67.79'	130.72'
C26(M)	200.00'	67.79'	130.72'

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-8-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

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NOT A PART

LOT 40

±0.5 ACRES
MFT = 94.00'

N 89°43'36"W 80.33' (P)
N 89°43'36"W 80.33' (M)

RETENTION AREA

LEGEND:

- = SET 1/2" REBAR & CAP
- LB. 6894
- ✕ = FOUND NAIL & DISK
- ⊠ = TELEPHONE PEDESTAL
- ⊞ = WOOD POWER POLE

ABBREVIATIONS:

- A/C = AIR CONDITIONER
- ASPH = ASPHALT
- C = CALCULATED FROM PLAT
- CATV = CABLE TELEVISION
- C/B = CONCRETE BLOCK
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BY: *James E. Brinkman* PSM - FLA. CERT# 5582

DATE: 10/21/08

BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL

DATE: 8/14/08

CHECKED BY: J.B.

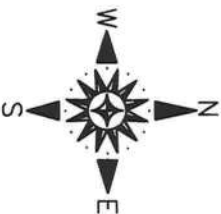
FIELD WORK COMPLETED ON 10/13/2008

FIELDBOOK 97, PAGE 74

PREPARED FOR: MARONDA

DRAWING NUMBER 167-08

LEGAL DESCRIPTION:
LOT FOURTY (40) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA

S.W. MULBERRY DRIVE
60' RIGHT-OF-WAY



CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:

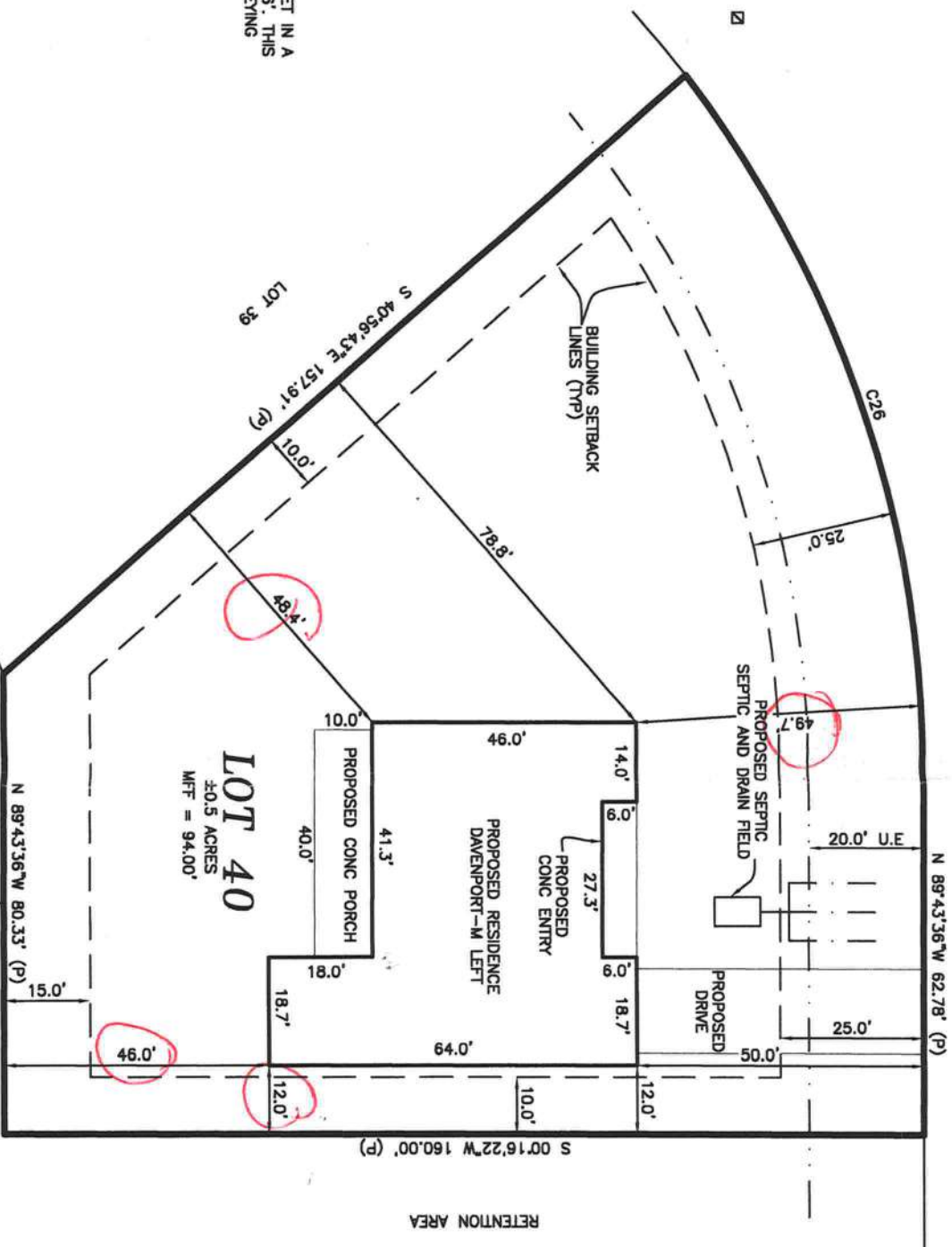
BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

BENCHMARK NOTE:

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SURVEYOR NOTES:

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CURVE TABLE

CURVE	RADIUS	TANGENT	LENGTH	DELTA	CHORD	CHORD BEARING
C26	200.00'	67.79'	130.72'	37°28'54.7"	128.41'	N 71°32'57" E

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0173-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

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LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
 - = FOUND 1/2" REBAR & CAP
 - = FOUND 1/2" REBAR & CAP
 - = SET 1/2" REBAR & CAP
 - = FOUND 3/4" IRON PIPE
 - = FOUND 4" X 4" CONC. MON.
 - = SET 4" X 4" CONC. MON.
 - = SET 4" X 4" CONC. MON.
 - ✕ = SET NAIL & DISK P.S.M. 5582
 - ✕ = FOUND NAIL & DISK
 - ✕ = FOUND 6" X 6" S.R.D.
 - ⊠ = FOUND 6" X 6" S.R.D.
 - ⊠ = CATV RISER
 - ⊠ = TELEPHONE PEDESTAL
 - ⊠ = WOOD POWER POLE
- ABBREVIATIONS:**
- A/C = AIR CONDITIONER
 - ASPH = ASPHALT
 - C = CALCULATED FROM MEASURED
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 - TRANS = TRANSFORMER
 - TYP = TYPICAL
 - WM = WATER METER
 - WV = WATER VALVE

THIS IS NOT A BOUNDARY SURVEY CERTIFICATE OF SURVEYOR:

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 84917-6, FLORIDA ADMINISTRATIVE CODE.

BY: *James E. Brinkman*
JAMES E. BRINKMAN, P.S.M. F.L.A. CERT# 5562
DATE: 8/17/08

BRINKMAN SURVEYING & MAPPING INC.
4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'	"THE BENCHMARK IN QUALITY SERVICE"	DRAWN BY: ZL
DATE: 8/14/08		CHECKED BY: J.B.
FIELD WORK COMPLETED ON ****	FIELD BOOK **, PAGE **	
PREPARED FOR: MARONDA		DRAWING NUMBER 167-08

**HOMETEAM**

PEST DEFENSE®

27430

TREATMENT WORKORDER☐ Termite Baiting System w/Tubes-under-the slab☐ Treat Only☒ Tubes-under-the slab and Treat☐ Bora-Care

DATE CALLED IN:	12/3	DATE OF SCHEDULE:	1/5/09
TIME CALLED IN:		TIME SCHEDULE:	8:30

JOB NAME: Miranda		SUBDIVISION: Timberland	
JOB ADDRESS: 252 SW Mulberry Lake City 3224			
BILLING NAME:		BILLING PHONE:	
BILLING ADDRESS:			
CALLED IN BY:		PHONE: 600277518	PERMIT NUMBER:

LOT & MODEL NUMBER: 28

DATE & TIME COMPLETED: 1/5/09 10:45

SQUARE FOOT: _____ LINEAR FOOT: _____ BLOCKVOIDS: _____

SLAB TYPE: _____ TYPE OF FILL: _____

APPROX. DEPTH OF FOOTING: Outside: _____ Inside: _____

☐ Addition ☐ Spot Treat ☐ Pool Addition ☐ Driveway☐ Final/Completion ☐ Other _____

PESTICIDE USED: Tera-X Pro TOTAL APPLIED: 120 gals

PERCENT (%) USED: 10 STICKER POSTED: Y

PRICE PER SQ. FT. =	TOTAL FOR P.T.		
	ADDITIONAL		
	TAX:		
/ /	TOTAL AMOUNT	\$	

X _____ X TECHNICIAN: [Signature]

I hereby acknowledge the satisfactory completion of the above described work

GT 23 / TCI

12/05