DATE 06/27/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024681

APPLICANT SUSAN FAIR PHONE 752-1711

ADDRESS 180 NW AMENITY COURT LAKE CITY FL 32055

OWNER CORNERSTONE DEVELOPERS PHONE 752-1711

ADDRESS 196 SW FIELDSTONE COURT LAKE CITY FL 32055

CONTRACTOR BRYAN ZECHER PHONE 752-8653

LOCATION OF PROPERTY 90W, TL ON HEATHRIDGE, TR ON FIELDSTONE, 5TH LOT ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 80400.00

HEATED FLOOR AREA 1608.00 TOTAL AREA 2095.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 18

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 33-3S-16-02438-165 SUBDIVISION EMERALD COVE

LOT 65 BLOCK PHASE 1 UNIT TOTAL ACRES

000001136		CBC054575	Susan Fair
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor

CULVERT	06-0592-N	BK	JH	
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD

Check # or Cash 2395

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 405.00 CERTIFICATION FEE $ 10.47 SURCHARGE FEE $ 10.47

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 525.94

INSPECTORS OFFICE _____ CLERKS OFFICE _____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application 525.94

Revised 9-23-04

For Office Use Only Application # 0606-61 Date Received 6-19-06 By CH Permit # 1136/24681

Application Approved by - Zoning Official BLK Date 23.06.06 Plans Examiner OK JTH Date 6-21-06

Flood Zone Xppl plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res Low Den.

Comments EH / NOC

Applicants Name SUSAN FAIR Phone 752-1711

Address 180 NW AMENITY CT. LAKE CITY, FL. 32055

Owners Name CORNERSTONE DEVELOPERS Phone 752-1711

911 Address 196 SW FIELDSTONE CT. LAKE CITY, FL. 32055

Contractors Name BRYAN ZECHER Phone 752-8653

Address PO BOX 815 LAKE CITY FL. 32056

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address MARK DISOSWAY PO BOX 868 LAKE CITY FL 32056

Mortgage Lenders Name & Address NA

Circle the correct power company - (FL Power & Light) - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 33-3S-16-02438-165 Estimated Cost of Construction $ 109000.00

Subdivision Name EMERALD COVE PHASE 1 Lot 65 Block ___ Unit ___ Phase ___

Driving Directions TAKE 90 TO SW HEATHRIDGE DR. TURN LEFT. GO TO 2ND STREET SW FIELDSTONE COURT. TURN RIGHT. 5TH LOT ON RIGHT.

Type of Construction FRAME & BRICK Number of Existing Dwellings on Property 0

Total Acreage 37 acres Lot Size 1/2 acre Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 35' ✓ Side 25' ✓ Side 25' ✓ Rear 108' ✓

Total Building Height 18'9" Number of Stories 1 Heated Floor Area 1608 Roof Pitch 6/12

Porch 19 GARAGE 468 TOTAL 2095

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CBC054575

Competency Card Number

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 16th day of June 20 06

Personally known ✓ or Produced Identification ___

Notary Signature

LEIGH ANN CANNON
Notary Public - State of Florida
My Commission Expires Dec 16, 2006
Commission # DD172314
Bonded By National Notary Assn

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Inst:2005026450 Date:10/24/2005 Time:13:06
Doc Stamp-Deed : 3628.80
DC,P.DeWitt Cason,Columbia County B:1062 P:2214

Property Appraiser's 02438-000
Identification Number 02421-000

TM File No: 05-652

WARRANTY DEED

This Warranty Deed, made this 24 day of October, 2005, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor, and CORNERSTONE DEVELOPERS, LLC, a Florida Limited Liability Company, whose post office address is P.O. Box 815, Lake City, Florida 32056, grantee.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lots 65,66,67,68,71,72,73,74,93,94,95,96,97 & 98, Emerald Cove, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 35-36, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(Signature of First Witness)

DeEtte F. Brown

(Typed Name of First Witness)

(Signature of Second Witness)

Karen M. Wright

(Typed Name of Second Witness)

D D P CORPORATION

BY: _____ (SEAL)
O. P. Daughtry, III,
President

(Corporate Seal)

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 14 day of October, 2005, by O. P. Daughtry, III, President of D D P Corporation, a Florida corporation, on behalf of said corporation, who is/are personally known to me or who has/have produced _____ as identification and who did not take an oath.

My Commission Expires:

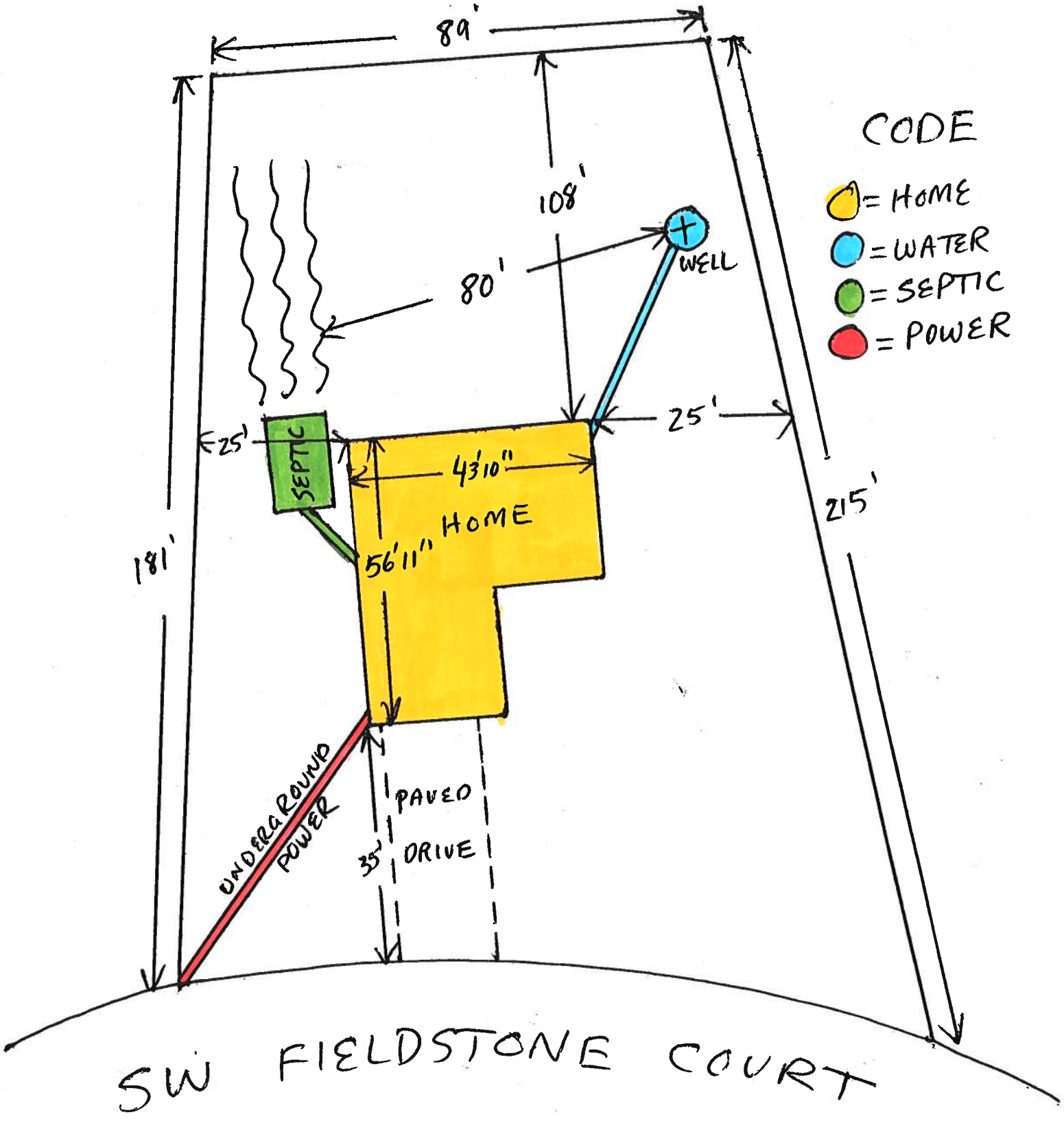
Notary Public
Printed, typed, or stamped name:

KAREN M. WRIGHT
Notary Public - State of Florida
My Commission Expires Jan 23, 2008
Commission # DD066234
Bonded By National Notary Assn.

Inst:2005026450 Date:10/24/2005 Time:13:06
Doc Stamp-Deed : 3628.80
DC,P.DeWitt Cason,Columbia County B:1062 P:2215


PART OF NORTHEAST 1/4
(NOT INCLUDED)
OWNER: DDP CORPORATION
ZONE: A-3
SECTION 32
SECTION 33
BASE BEARING
N 6°16'20"E
WEST LINE OF SECTION 33
ZONING:
RSF-2 - RESIDENTIAL SINGLE FAMILY 2
BUILDING SET BACKS:
FRONT: 25 FEET
SIDE: 10 FEET
REAR: 15 FEET
PART OF THE NORTHWEST 1/4
(NOT INCLUDED)
OWNER: DDP CORPORATION
ZONE: RSF-2
SW TIMBERLAND COURT
SW FIELDSTONE COURT
SW HEATHRIDGE DRIVE
SW WOODLEAF COURT
LOT 65
0.50± ACRES
CYPRESS LAKE PHASE 3
(PLAT BOOK 6, PAGE 80)
WEST LINE OF CYPRESS LAKE PHASE 3
NORTH LINE OF CYPRESS LAKE PHASE 3
30' DRAINAGE EASEMENT
ZONE: RSF-2
OWNER: SHAGUFTA CHOUDHURY
UNPLATTED

EMERALD COVE I
COLUMBIA COUNTY, FL.
LOT 65 SITE PLAN



HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
904 NW Main Blvd.
LAKE CITY, FLORIDA 32055

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

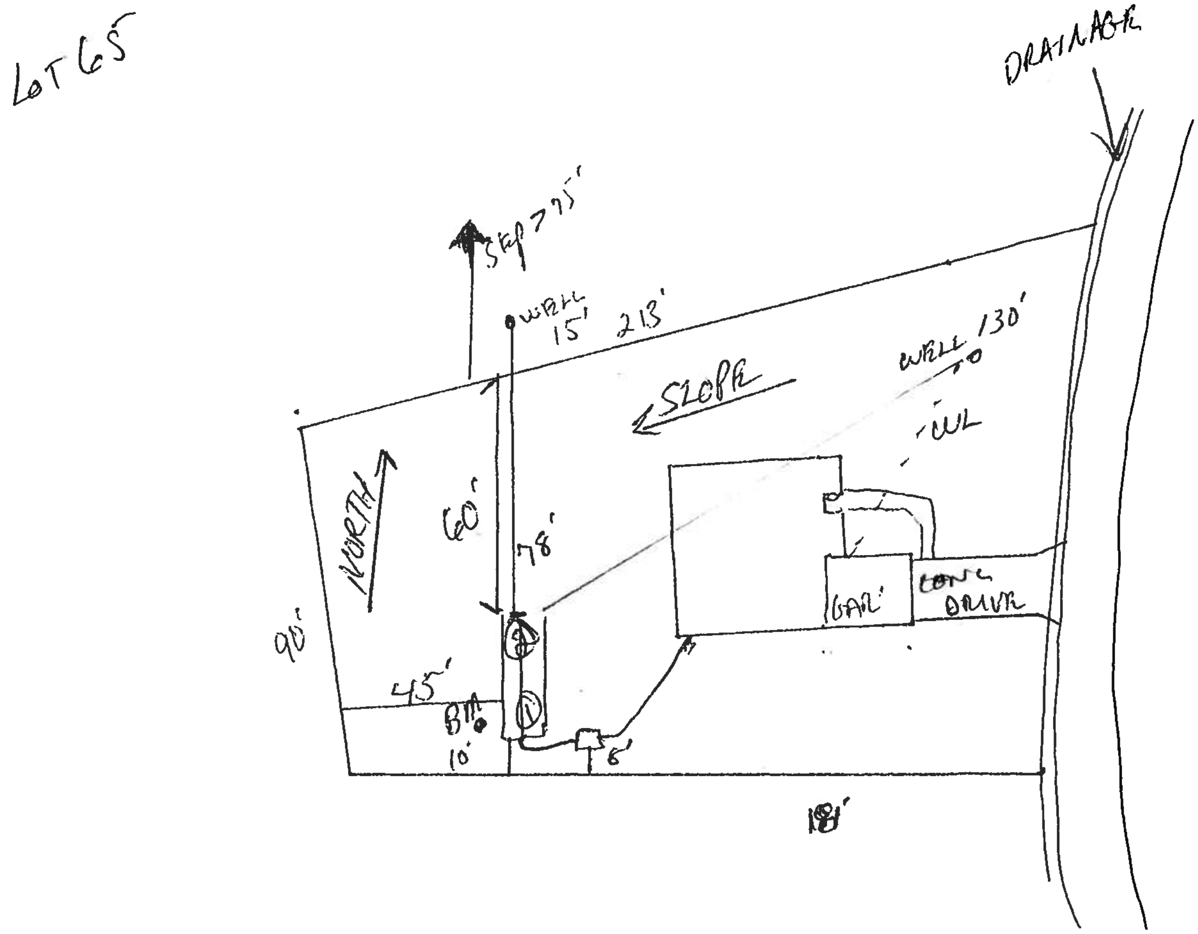
Donald D. Hall
DDH/jk

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION F0R ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0592N

--------------------------- PART II - SITEPLAN ---------------------------

Scale: 1 inch = 50 feet.



Notes: ____

Site Plan submitted by: Rock D [signature] MASTER CONTRACTOR

Plan Approved ✓ Not Approved_____ Date 6/26/06

By [signature] Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

FORM 600A-2004 EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**605112CornerstoneDevelopmentTheJaneModel**	Builder:	Brian Zecher
Address:	**Lot: 65, Sub: Emerald Cove, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	24681
Owner:	**The Jane Model**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	1608 ft²	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description Area (Dble Default) 112.7 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 112.7 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 175.0(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=13.0, 1046.3 ft²	
b.	Frame, Wood, Exterior	R=13.0, 160.0 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 1688.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 160.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 28.0 kBtu/hr	
		SEER: 10.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 28.0 kBtu/hr	
		HSPF: 7.00	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons	
		EF: 0.93	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.07

Total as-built points: 22034

Total base points: 24641

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Brian Zecher

DATE: 5-16-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Chris W. Ay

DATE: 5-16-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 65, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1608.0	20.04	5800.4	Double, Clear	E	1.5	5.5	45.0	42.06	0.90	1696.4	
				Double, Clear	E	1.5	6.5	20.0	42.06	0.93	779.5	
				Double, Clear	S	1.5	1.2	2.7	35.87	0.49	47.2	
				Double, Clear	W	1.5	5.5	30.0	38.52	0.90	1036.6	
				Double, Clear	N	1.5	5.5	15.0	19.20	0.93	267.3	
				As-Built Total:				**112.7**			**3827.0**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1046.3	1.50		1569.5	
Exterior	1206.3	1.70	2050.7	Frame, Wood, Exterior			13.0	160.0	1.50		240.0	
Base Total:	**1206.3**		**2050.7**	**As-Built Total:**				**1206.3**			**1809.5**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	1.60	32.0	Exterior Insulated				20.0	4.10		82.0	
Exterior	40.0	4.10	164.0	Exterior Insulated				20.0	4.10		82.0	
				Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**60.0**		**196.0**	**As-Built Total:**				**60.0**			**196.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1608.0	1.73	2781.8	Under Attic			30.0	1688.0	1.73 X 1.00		2920.2	
Base Total:	**1608.0**		**2781.8**	**As-Built Total:**				**1688.0**			**2920.2**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	175.0(p)	-37.0	-6475.0	Slab-On-Grade Edge Insulation			0.0	175.0(p	-41.20		-7210.0	
Raised	0.0	0.00	0.0									
Base Total:			**-6475.0**	**As-Built Total:**				**175.0**			**-7210.0**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1608.0	10.21	16417.7					1608.0	10.21		16417.7	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 65, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 20771.6			**Summer As-Built Points:**					**17960.3**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 28000 btuh ,SEER/EFF(10.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)					
			17960	1.00	(1.09 x 1.147 x 0.91)	0.341	1.000	6974.0
20771.6	**0.4266**	**8861.2**	**17960.3**	**1.00**	**1.138**	**0.341**	**1.000**	**6974.0**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 65, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	**1608.0**	**12.74**	**3687.5**	Double, Clear	E	1.5	5.5	45.0	18.79	1.04	880.6
				Double, Clear	E	1.5	6.5	20.0	18.79	1.03	387.4
				Double, Clear	S	1.5	1.2	2.7	13.30	3.11	111.8
				Double, Clear	W	1.5	5.5	30.0	20.73	1.03	639.3
				Double, Clear	N	1.5	5.5	15.0	24.58	1.00	369.8
				As-Built Total:				**112.7**			**2388.9**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1046.3	3.40		3557.4
Exterior	1206.3	3.70	4463.3	Frame, Wood, Exterior			13.0	160.0	3.40		544.0
Base Total:	**1206.3**		**4463.3**	**As-Built Total:**				**1206.3**			**4101.4**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	20.0	8.00	160.0	Exterior Insulated				20.0	8.40		168.0
Exterior	40.0	8.40	336.0	Exterior Insulated				20.0	8.40		168.0
				Adjacent Insulated				20.0	8.00		160.0
Base Total:	**60.0**		**496.0**	**As-Built Total:**				**60.0**			**496.0**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1608.0	2.05	3296.4	Under Attic			30.0	1688.0	2.05 X	1.00	3460.4
Base Total:	**1608.0**		**3296.4**	**As-Built Total:**				**1688.0**			**3460.4**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	175.0(p)	8.9	1557.5	Slab-On-Grade Edge Insulation			0.0	175.0(p	18.80		3290.0
Raised	0.0	0.00	0.0								
Base Total:			**1557.5**	**As-Built Total:**				**175.0**			**3290.0**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1608.0	-0.59	-948.7					1608.0	-0.59		-948.7

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 65, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**12552.0**	**Winter As-Built Points:**					**12788.0**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 28000 btuh ,EFF(7.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0					
12788.0	1.000	(1.069 x 1.169 x 0.93)	0.487	1.000	7239.9			
12552.0	**0.6274**	**7875.1**	**12788.0**	**1.00**	**1.162**	**0.487**	**1.000**	**7239.9**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 65, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	40.0	0.93	3		1.00	2606.67	1.00	7820.0
				As-Built Total:							**7820.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
8861	**7875**	**7905**	**24641**	**6974**	**7240**	**7820**	**22034**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 65, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.9

The higher the score, the more efficient the home.

The Jane Model, Lot: 65, Sub: Emerald Cove, Plat: , Lake City, FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 28.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 10.00
4.	Number of Bedrooms	3		b. N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1608 ft²		c. N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description Area	13.	Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 112.7 ft²		a. Electric Heat Pump	Cap: 28.0 kBtu/hr
	b. SHGC:				HSPF: 7.00
	(or Clear or Tint DEFAULT) 7b.	(Clear) 112.7 ft²		b. N/A	
8.	Floor types				
	a. Slab-On-Grade Edge Insulation	R=0.0, 175.0(p) ft		c. N/A	
	b. N/A				
	c. N/A		14.	Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 40.0 gallons
	a. Frame, Wood, Exterior	R=13.0, 1046.3 ft²			EF: 0.93
	b. Frame, Wood, Exterior	R=13.0, 160.0 ft²		b. N/A	
	c. N/A				
	d. N/A			c. Conservation credits	
	e. N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 1688.0 ft²	15.	HVAC credits	
	b. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 160.0 ft		MZ-C-Multizone cooling,	
	b. N/A			MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: 6-16-06

Address of New Home: 196 SW FIELDSTONE CT. City/FL Zip: LAKE CITY, FL 32055

NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

24681

Prepared by and after
recording return to:

William L. Joel
Stoneburner Berry & Simmons, P.A.
841 Prudential Drive, Suite 1400
Jacksonville, FL 32207

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DeWitt CASON, CLERK OF COURTS
By Bonnie Dow
Deputy Clerk
Date 7/10/06

Inst:2006016323 Date:07/10/2006 Time:11:13
_____DC,P.DeWitt Cason,Columbia County B:1089 P:464

Permit No. _____
Tax Folio No. _____

NOTICE OF COMMENCEMENT

STATE OF FLORIDA

COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvements will be made to certain real property, and in accordance with section 713 Florida Statutes, the following information is provided in this notice of commencement.

1. Description of property (legal description and address, if available):

Lot 65, Emerald Cove, Phase I, according to map or plat thereof as recorded in Plat Book 8, Pages 35 and 36, Public Records of Columbia County, Florida.

2. General description of improvements: Construction of residential dwellings

3. Owner Information:

(a) Name and Address: Cornerstone Developers, LLC, a Florida limited liability company
180 NW Amenity Court
Lake City, Florida 32025

(b) Owner's interest in the site of the improvements (if other than fee simple title holder):

(c) Name and Address of fee simple title holder (if other than owner):

4. Contractor:

(a) Name and Address: Bryan Zecher Construction Inc.
465 NW Orange Street
Lake City, FL 32055

(b) Phone No. 386-752-8653 Fax No. _____ (Optional, if service by fax is acceptable)

5. Surety on any payment bond: N/A

(a) Name and Address:
(b) Phone No. _____ Fax No. _____ (Optional, if service by fax is acceptable)
(c) Amount of bond $_____

6. Lender making loan for the construction of the improvements:

(a) Name and Address: First Horizon Home Loan Corporation
1051 Deerwood Park Boulevard
Building 200, Suite 115
Jacksonville, FL 32256
Attn: James J. O'Connor, Jr.

(b) Phone No. 904-998-5300 Fax No. _____ (Optional, if service by fax is acceptable)

7. Persons within the State of Florida designated by owner upon whom notices may be served as provided by Section 713.13(1)(a)7, Florida Statutes:

(a) Name and Address: Cornerstone Developers, LLC a Florida limited liability company
180 NW Amenity Court
Lake City, Florida 32025

(b) Phone No. 386- 752-1711 Fax No. _____ (Optional, if service by fax is acceptable)

8. In addition to himself, Owner designates the following person to receive a copy of the lienor's notice as provided in Section 713.13(1)(b), Florida Statute:

(a) Name and Address: James J. O'Connor, Jr.
First Horizon Home Loan Corporation
1051 Deerwood Park Boulevard
Building 200, Suite 115
Jacksonville, FL 32256

(b) Phone No. 904-998-5300 Fax No. _____ (Optional, if service by fax is acceptable)

9. Expiration date of notice of commencement (the expiration date is one (1) year from the date of recording unless a different date is specified):

CORNERSTONE DEVELOPERS, LLC

By: _____
Frank Soucinek, its sole Manager (SEAL)

(OWNER)

Sworn to and Subscribed before me this 21 day of June, 2006, by Frank Soucinek, the Manager of Cornerstone Developers, LLC. He is personally known or has produced _____ as identification.

Susan Ann Fair
Notary Public, State and County Aforesaid
Print Name:
My Commission Expires:
My Commission No.:
(NOTARIAL SEAL)

SUSAN ANN FAIR
MY COMMISSION # DD 342845
EXPIRES: August 14, 2008
Bonded Thru Notary Public Underwriters

POST A CERTIFIED COPY OF THE RECORDED NOTICE ON CONSTRUCTION SITE

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001136

DATE 06/27/2006 PARCEL ID # 33-3S-16-02438-165

APPLICANT SUSAN FAIR PHONE 752-1711

ADDRESS 180 NW AMENITY COURT LAKE CITY FL 32055

OWNER CORNERSTONE DEVELOPERS PHONE 752-1711

ADDRESS 196 SW FIELDSTONE COURT LAKE CITY FL 32055

CONTRACTOR BRYAN ZECHER PHONE 752-8653

LOCATION OF PROPERTY 90W, TL ON HEATHRIDGE, TR ON FIELDSTONE, 5TH LOT ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 65 1

SIGNATURE Susan Fair

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

Project Information for:	L164073		
Builder:	CORNERSTONE	Date:	5/19/2006
Lot:	LOT 65 EMERALD COVE	Start Number:	**2568**
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	28		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ZECHER, BRYAN C. CBC 054575
Address: PO BOX 815
LAKE CITY,FLORIDA 32056 Designer: **162**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

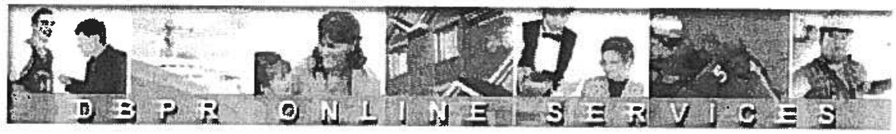
Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0519062568	5/19/2006				
2	CJ3	0519062569	5/19/2006				
3	CJ3A	0519062570	5/19/2006				
4	CJ5	0519062571	5/19/2006				
5	CJ5A	0519062572	5/19/2006				
6	EJ7	0519062573	5/19/2006				
7	EJ7A	0519062574	5/19/2006				
8	HJ9	0519062575	5/19/2006				
9	HJ9A	0519062576	5/19/2006				
10	T01	0519062577	5/19/2006				
11	T02	0519062578	5/19/2006				
12	T03	0519062579	5/19/2006				
13	T04	0519062580	5/19/2006				
14	T05	0519062581	5/19/2006				
15	T06	0519062582	5/19/2006				
16	T07	0519062583	5/19/2006				
17	T08	0519062584	5/19/2006				
18	T09	0519062585	5/19/2006				
19	T10	0519062586	5/19/2006				
20	T11	0519062587	5/19/2006				
21	T12	0519062588	5/19/2006				
22	T13	0519062589	5/19/2006				
23	T14	0519062590	5/19/2006				
24	T15	0519062591	5/19/2006				
25	T16	0519062592	5/19/2006				
26	T17	0519062593	5/19/2006				
27	T18	0519062594	5/19/2006				
28	T19	0519062595	5/19/2006				

MAY 19 2006

MyFlorida.com



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- Online Help

10:00:13 AM

Licensee Details

Licensee Information

Name:	**ZECHER, BRYAN CHRISTIAN (Primary Name)** **BRYAN ZECHER CONSTRUCTION INC (DBA**
Main Address:	**P O BOX 815** **LAKE CITY, Florida 32056**
Lic. Location:	**465 NW ORANGE ST** **LAKE CITY, FL 32055 United States** **Columbia**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC054575**
Status:	**Current, Active**
Licensure Date:	**12/05/1991**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	04/13/2004

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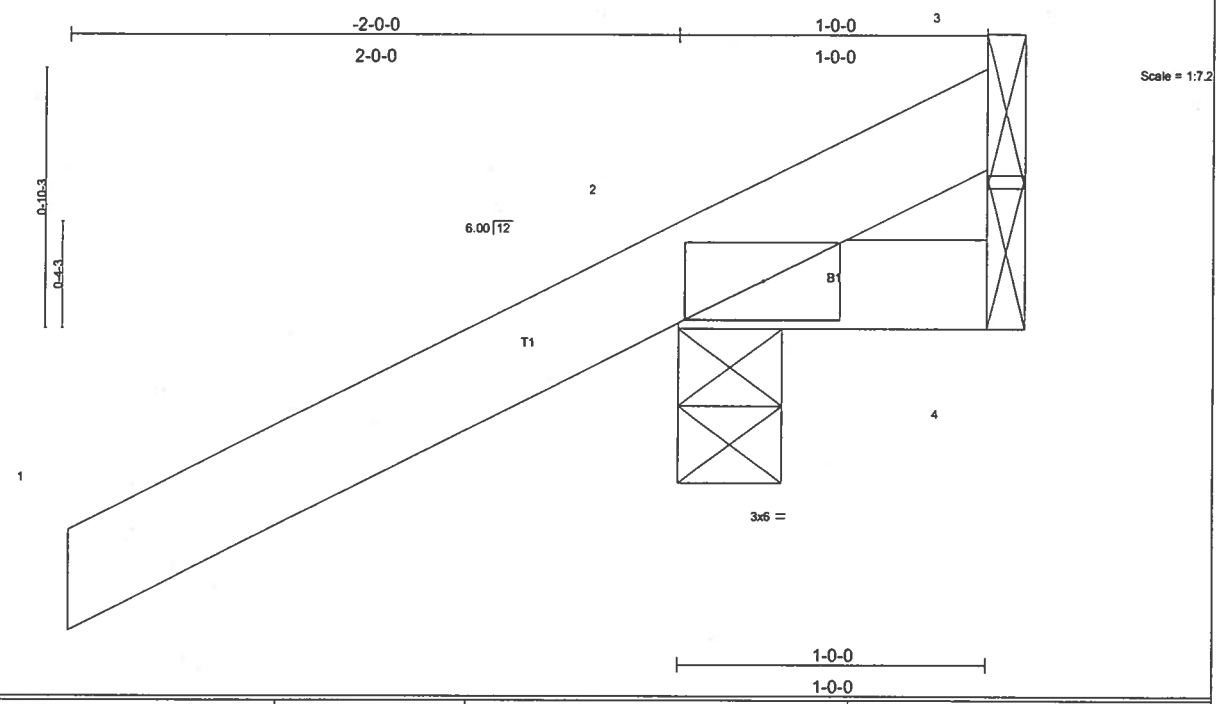
MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0519062568

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	CJ1	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:07 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-275(load case 5), 3=-91(load case 1)
Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/76
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 91 lb uplift at joint 3.

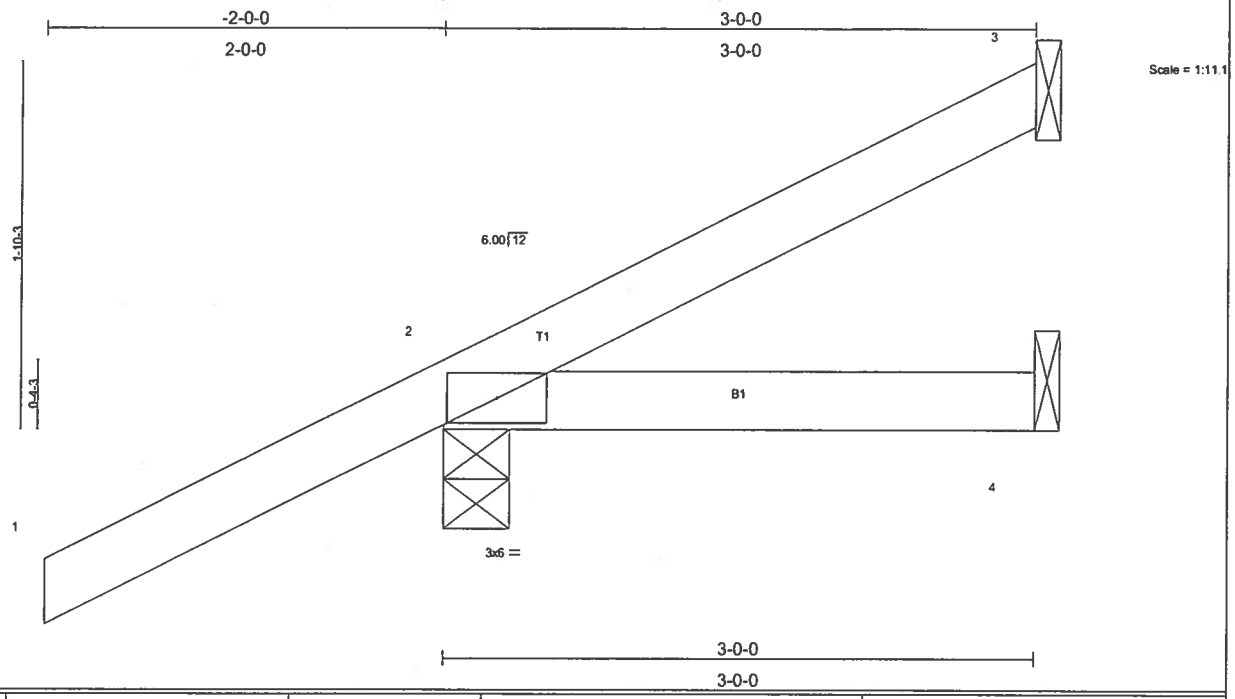
LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0519062569

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	CJ3	JACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:08 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-27(load case 6), 2=-205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3 and 205 lb uplift at joint 2.

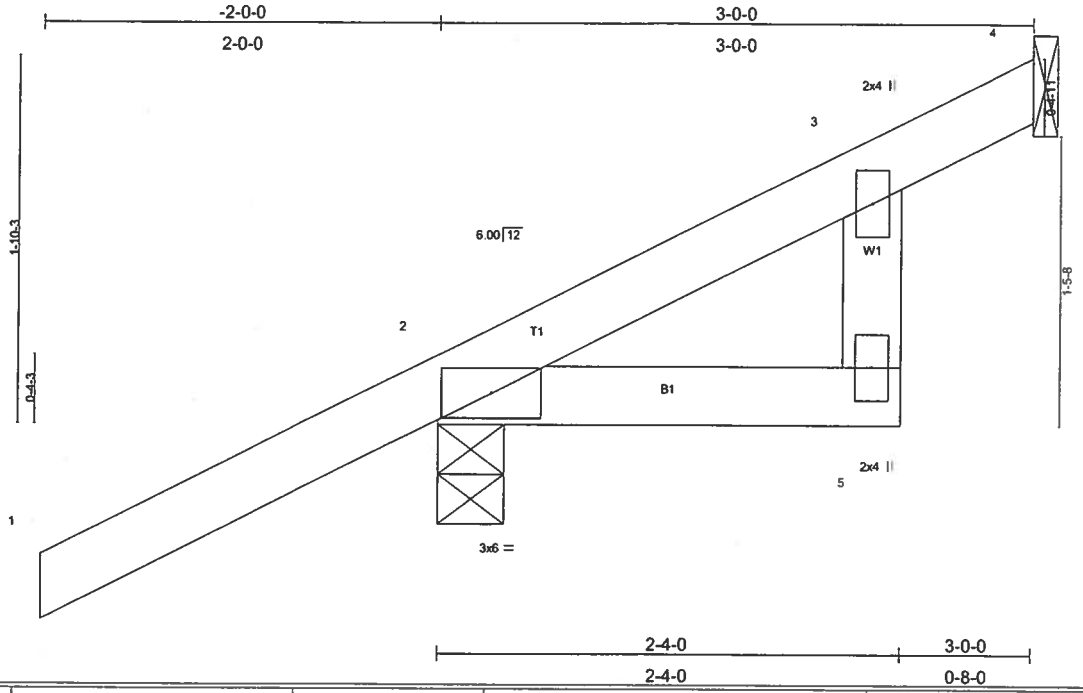
LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0519062570

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	CJ3A	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:08 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	5	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=51/Mechanical, 2=276/0-4-0
Max Horz 2=132(load case 5)
Max Uplift4=-23(load case 6), 2=-206(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-63/2, 3-4=-17/21
BOT CHORD 2-5=0/0
WEBS 3-5=0/30

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 4 and 206 lb uplift at joint 2.

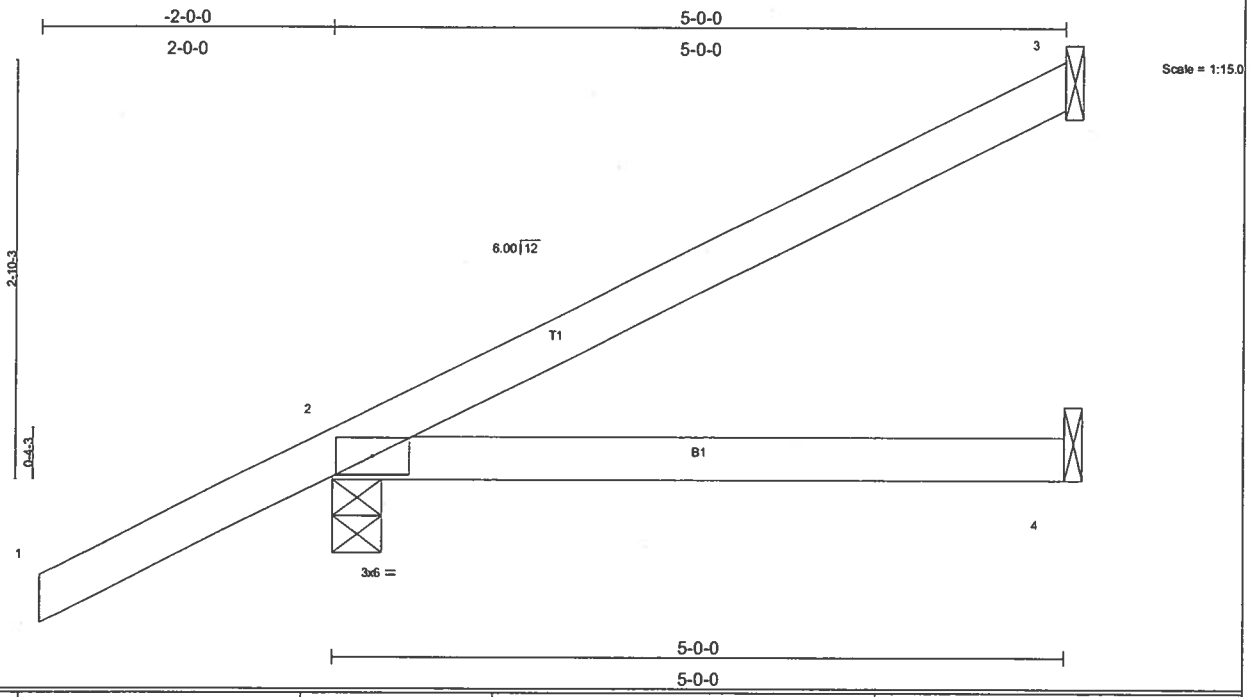
LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0519062571

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	CJ5	JACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:09 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-86(load case 5), 2=-201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3 and 201 lb uplift at joint 2.

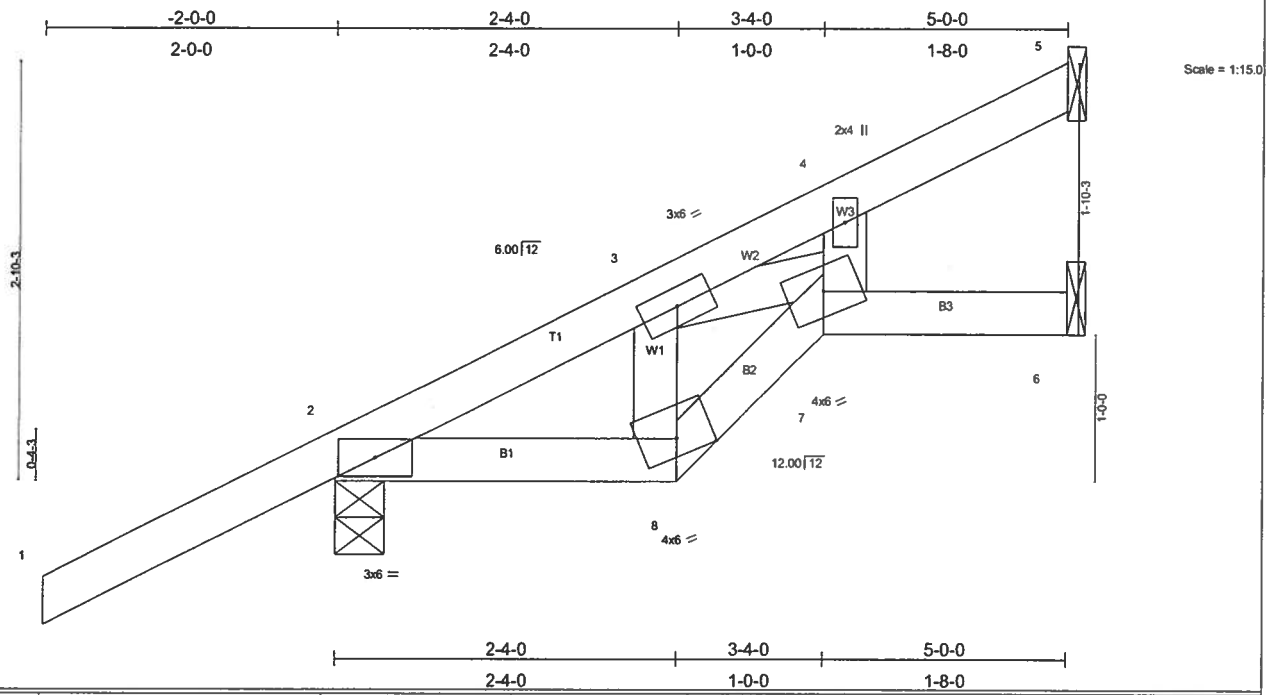
LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0519062572

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	CJ5A	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:10 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.03	7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.04	7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=150/Mechanical, 2=344/0-4-0, 6=24/Mechanical
Max Horz 2=178(load case 5)
Max Uplift5=-77(load case 5), 2=-201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-224/0, 3-4=-110/0, 4-5=-67/63
BOT CHORD 2-8=-74/147, 7-8=-93/199, 6-7=-0/0
WEBS 3-8=-86/74, 3-7=-100/53, 4-7=-63/186

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 5 and 201 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0519062573

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	EJ7	MONO TRUSS	30	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:10 2006 Page 1

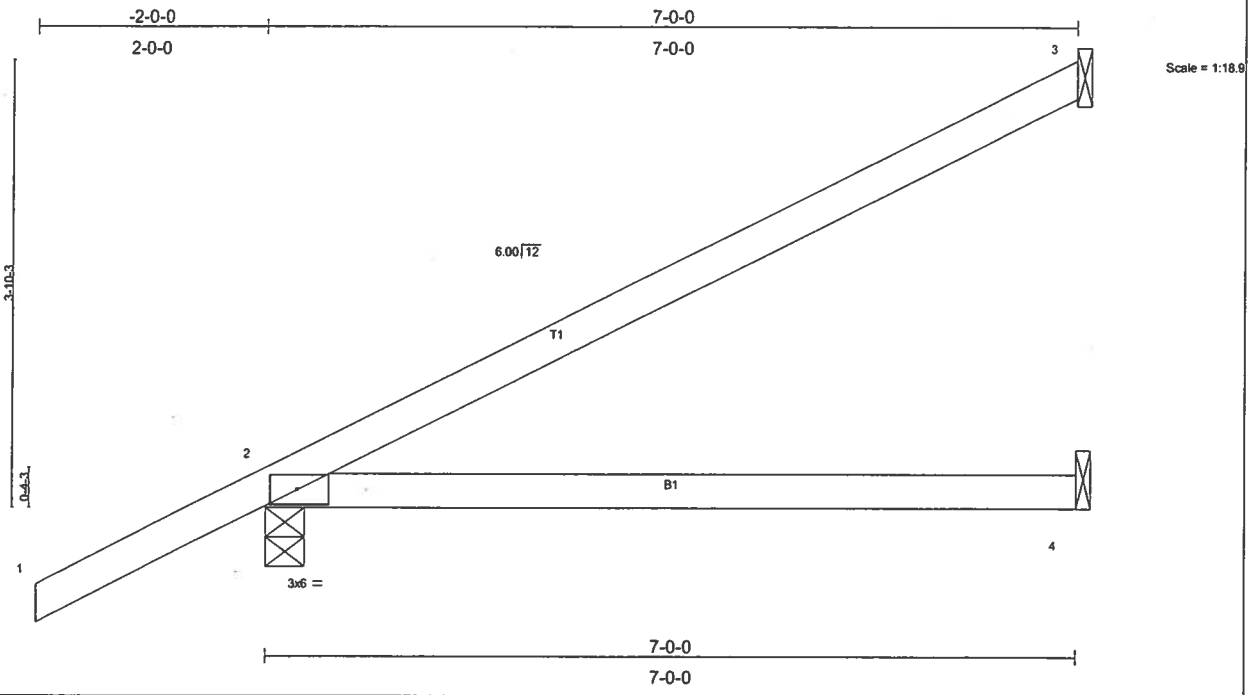


Plate Offsets (X,Y): [2:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.12	2-4	>674	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.20	2-4	>403	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-133(load case 5), 2=-211(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/58
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 3 and 211 lb uplift at joint 2.

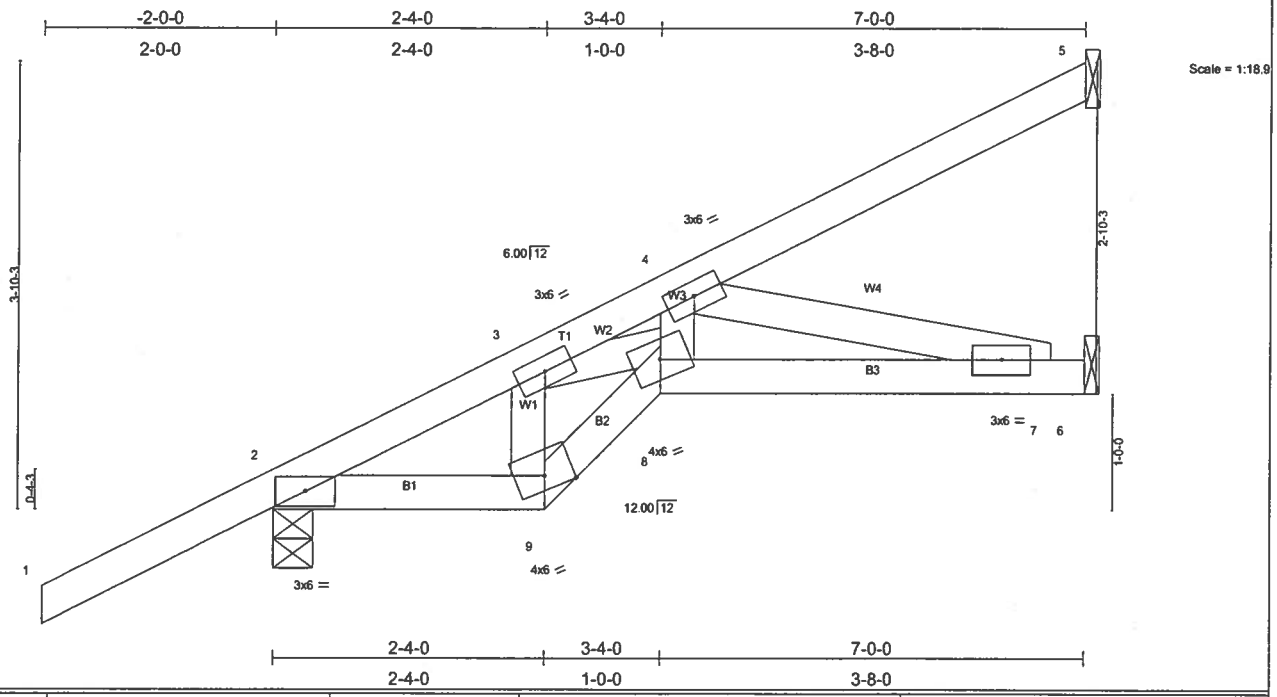
LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0519062574

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	EJ7A	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:11 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.02	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.04	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-4-14 oc bracing.

REACTIONS (lb/size) 5=85/Mechanical, 2=420/0-4-0, 6=181/Mechanical
Max Horz 2=224(load case 5)
Max Uplift5=-72(load case 5), 2=-211(load case 5), 6=-53(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-343/0, 3-4=-727/341, 4-5=-61/30
BOT CHORD 2-9=-142/258, 8-9=-171/346, 7-8=-449/594, 6-7=0/0
WEBS 3-9=-229/177, 3-8=-346/425, 4-8=-80/299, 4-7=-611/462

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 5, 211 lb uplift at joint 2 and 53 lb uplift at joint 6.

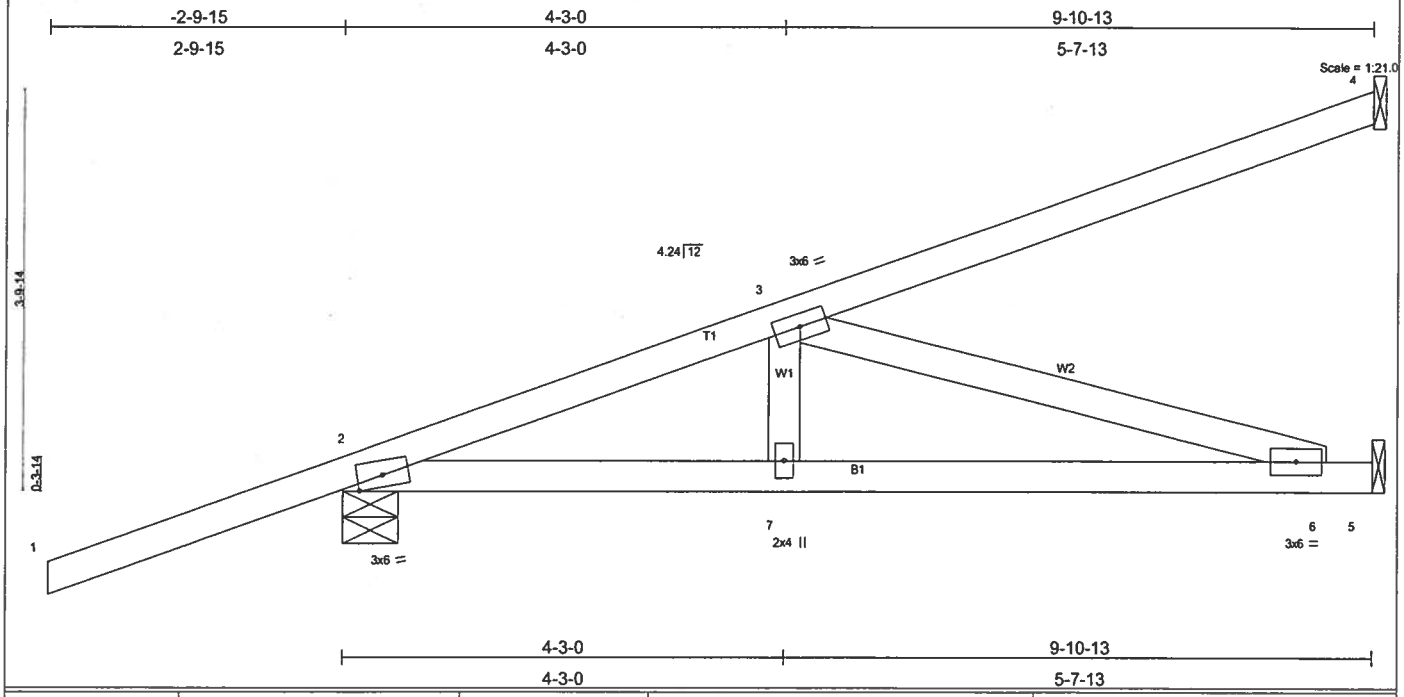
LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Dwg.#0519062575

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	HJ9	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:11 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.18	6-7	>623	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=537/0-6-6, 5=372/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-231(load case 2), 2=-284(load case 2), 5=-61(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-876/116, 3-4=-105/66
BOT CHORD 2-7=-304/809, 6-7=-304/809, 5-6=0/0
WEBS 3-7=0/187, 3-6=-843/317

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 284 lb uplift at joint 2 and 61 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0519062576

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	HJ9A	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:12 2006 Page 1

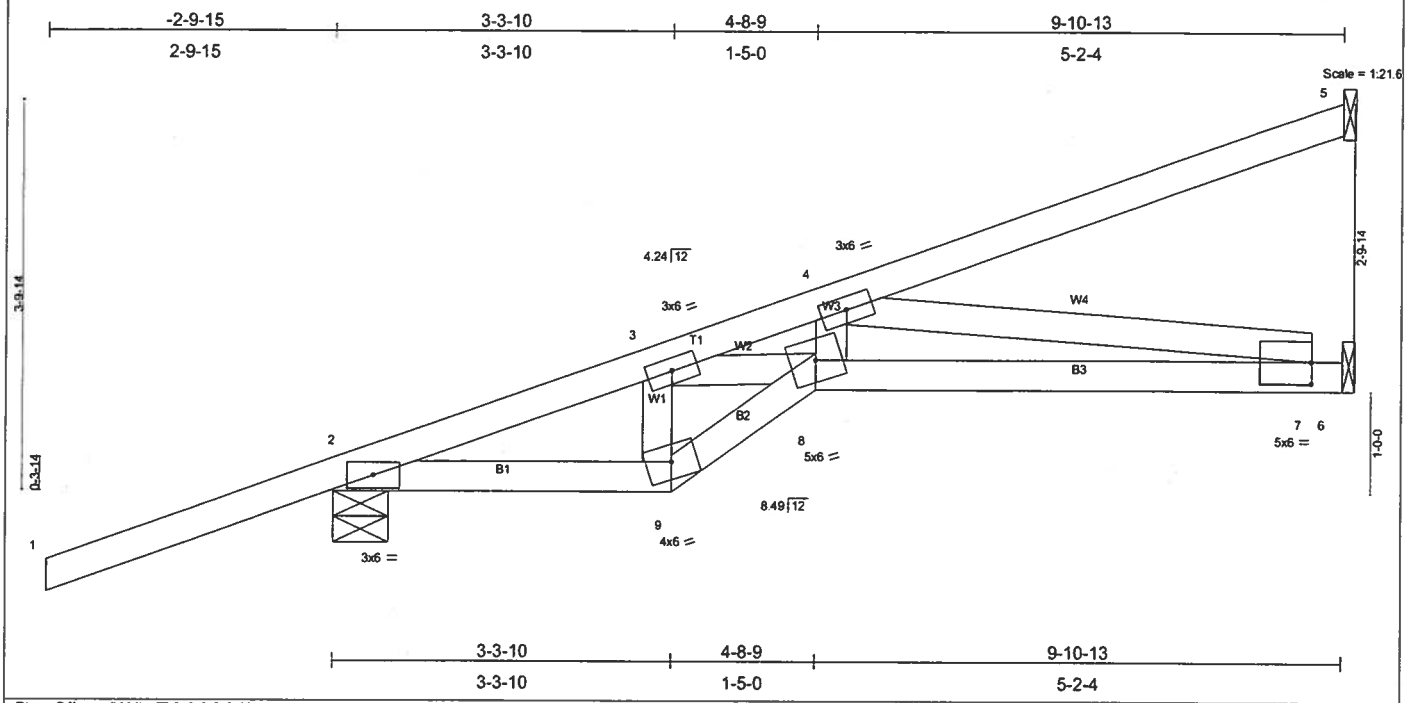


Plate Offsets (X,Y): [7:0-0-0,0-2-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.12	7-8	>995	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.19	7-8	>603	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.74	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
7-4-11 oc bracing: 7-8.

REACTIONS (lb/size) 5=248/Mechanical, 2=537/0-6-7, 6=394/Mechanical
Max Horz 2=269(load case 2)
Max Uplift5=-206(load case 2), 2=-284(load case 2), 6=-87(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-615/2, 3-4=-1827/527, 4-5=-94/61
BOT CHORD 2-9=-147/540, 8-9=-149/643, 7-8=-718/1656, 6-7=0/0
WEBS 3-9=-416/117, 3-8=-613/1265, 4-8=0/467, 4-7=-1677/727

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 5, 284 lb uplift at joint 2 and 87 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-5=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-9=-24(F=3, B=3), 9=-24(F=3, B=3)-to-8=-35(F=-2, B=-2), 8=-35(F=-2, B=-2)-to-6=-74(F=-22, B=-22)

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0519062577

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T01	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:13 2006 Page 1

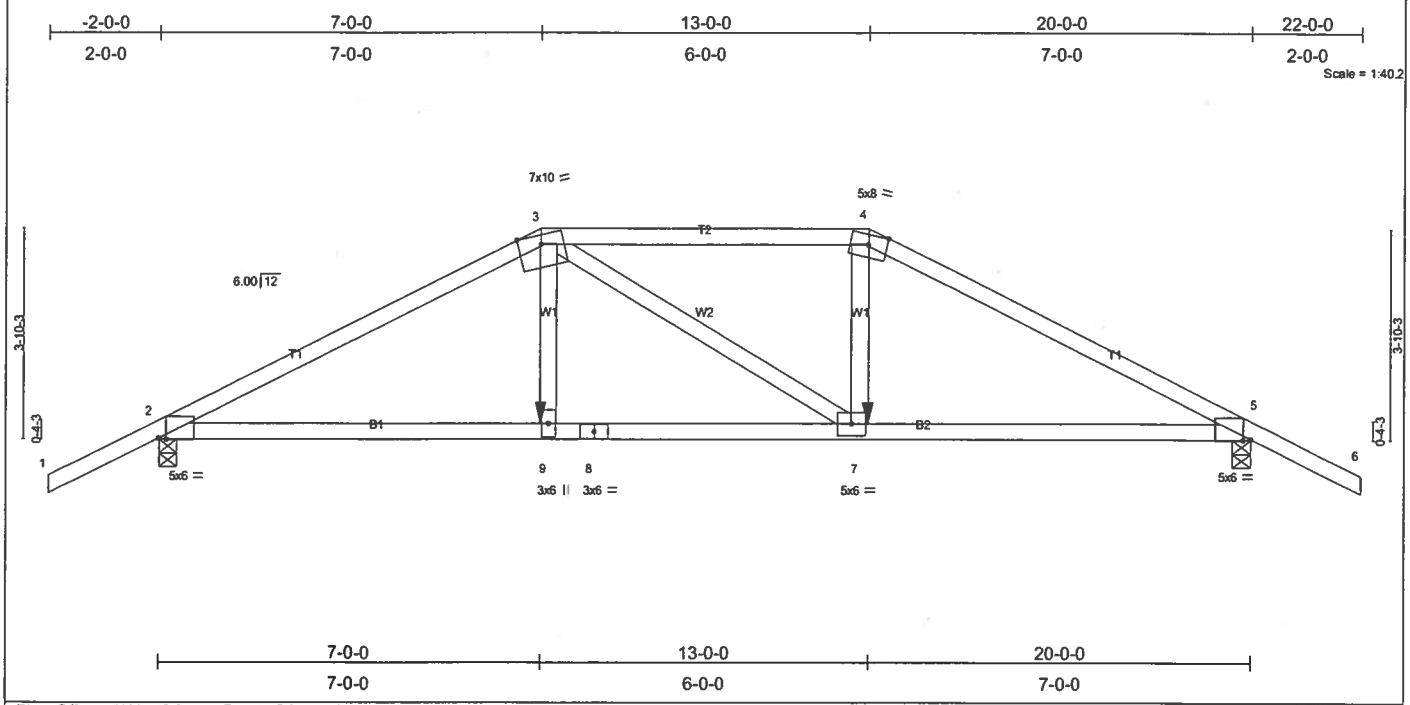


Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.17	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.27	7-9	>872	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.09	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 88 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-8 oc bracing.

REACTIONS (lb/size) 2=1773/0-4-0, 5=1773/0-4-0
Max Horz 2=87(load case 4)
Max Uplift2=-807(load case 4), 5=-807(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3138/1270, 3-4=-2758/1208, 4-5=-3140/1270, 5-6=0/47
BOT CHORD 2-9=-1071/2722, 8-9=-1080/2757, 7-8=-1080/2757, 5-7=-1030/2723
WEBS 3-9=-220/849, 3-7=-122/126, 4-7=-241/900

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 807 lb uplift at joint 2 and 807 lb uplift at joint 5.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 13-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-9=-30, 7-9=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 9=-539(F) 7=-539(F)

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

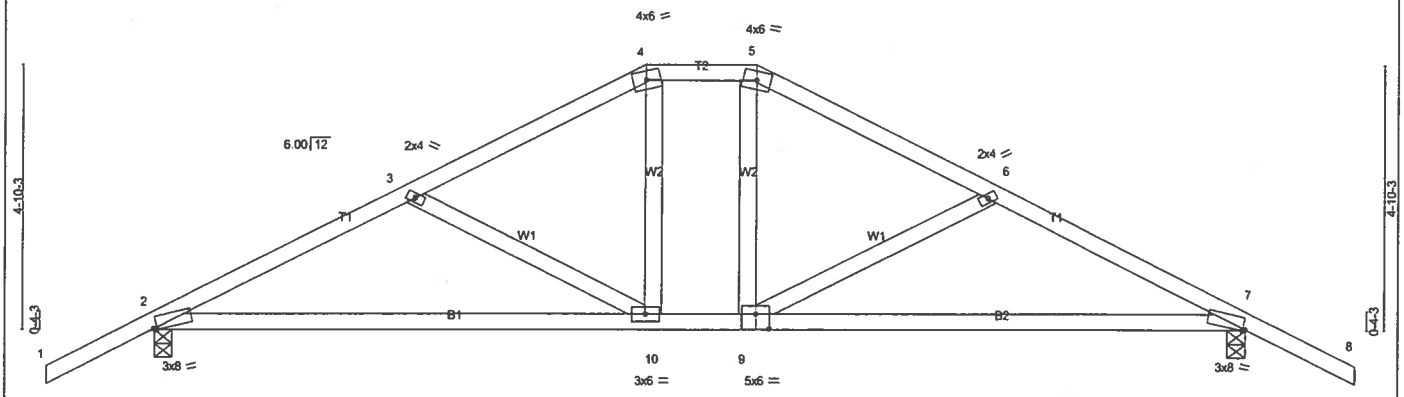
Dwg.#0519062578

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:14 2006 Page 1

-2-0-0	4-9-4	9-0-0	11-0-0	15-2-12	20-0-0	22-0-0
2-0-0	4-9-4	4-2-12	2-0-0	4-2-12	4-9-4	2-0-0

Scale = 1:40.2



9-0-0	11-0-0	20-0-0
9-0-0	2-0-0	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge], [9:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.18	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.28	2-10	>828	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 97 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=943/0-4-0, 7=943/0-4-0
Max Horz 2=-101(load case 6)
Max Uplift2=-390(load case 5), 7=-390(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1323/560, 3-4=-1062/446, 4-5=-906/449, 5-6=-1062/446, 6-7=-1323/560, 7-8=0/47
BOT CHORD 2-10=-337/1143, 9-10=-142/906, 7-9=-337/1143
WEBS 3-10=-306/225, 4-10=-63/290, 5-9=-63/290, 6-9=-306/225

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 2 and 390 lb uplift at joint 7.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0519062579

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T03	COMMON	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:15 2006 Page 1

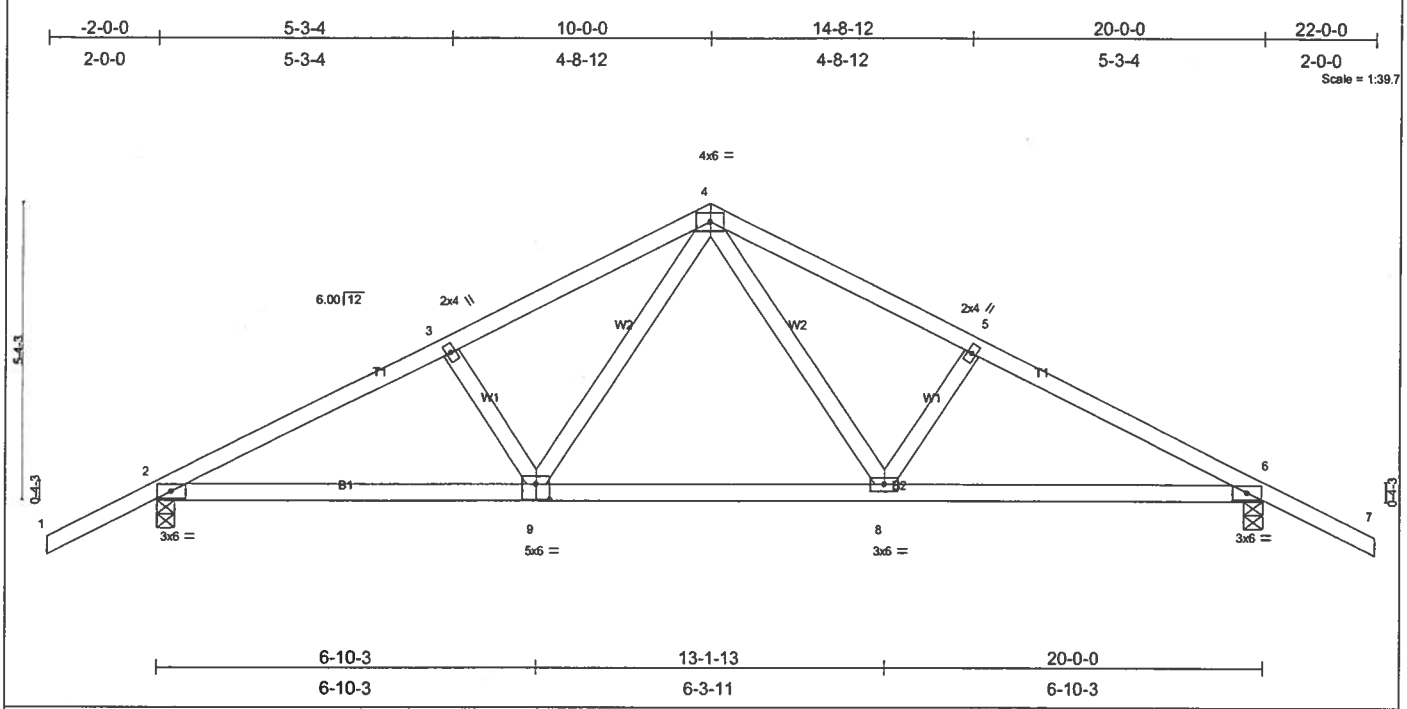


Plate Offsets (X,Y): [9:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.16	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.25	8-9	>932	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-2-0 oc bracing.

REACTIONS (lb/size) 2=1101/0-4-0, 6=1101/0-4-0
Max Horz 2=-108(load case 6)
Max Uplift2=-455(load case 5), 6=-455(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1728/704, 3-4=-1584/704, 4-5=-1584/704, 5-6=-1728/704, 6-7=0/47
BOT CHORD 2-9=-460/1475, 8-9=-232/1023, 6-8=-460/1475
WEBS 3-9=-219/204, 4-9=-238/663, 4-8=-238/663, 5-8=-219/204

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 455 lb uplift at joint 2 and 455 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-9=-30, 8-9=-80(F=-50), 6-8=-30

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

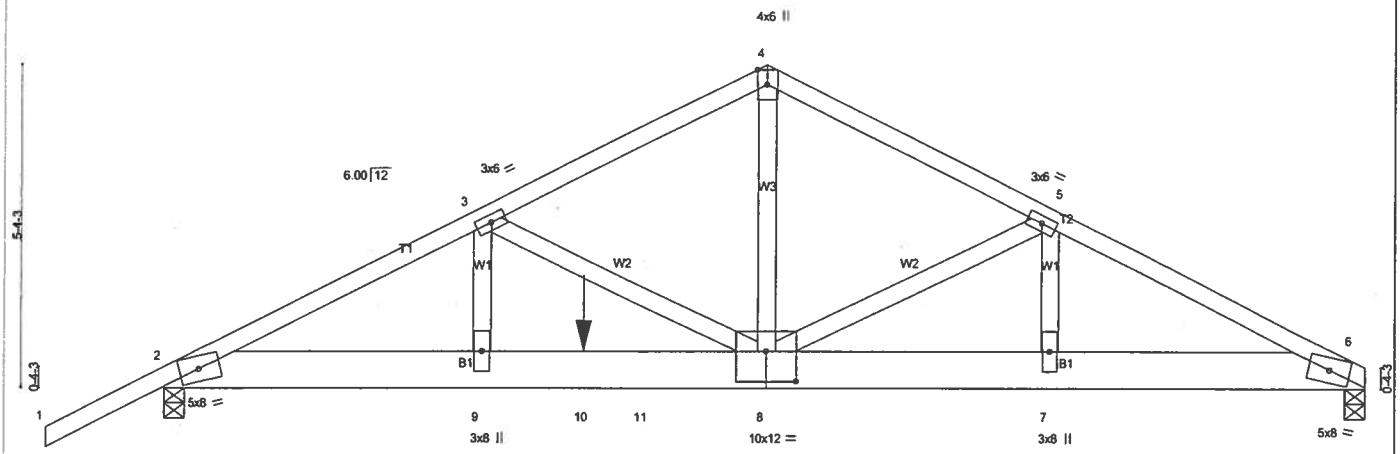
Dwg.#0519062580

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T04	COMMON	1	3	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:16 2006 Page 1

-2-0-0 | 5-3-4 | 10-0-0 | 14-8-12 | 20-0-0
2-0-0 | 5-3-4 | 4-8-12 | 4-8-12 | 5-3-4

Scale = 1:36.5



5-3-4 | 7-0-0 | 10-0-0 | 14-8-12 | 20-0-0
5-3-4 | 1-8-12 | 3-0-0 | 4-8-12 | 5-3-4

Plate Offsets (X,Y): [8:0-6-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.16	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.26	8-9	>904	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.93	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 378 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=8411/0-4-0, 2=6035/0-4-0
Max Horz 2=141(load case 4)
Max Uplift6=-3132(load case 5), 2=-2320(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-12877/4718, 3-4=-10150/3808, 4-5=-10154/3798, 5-6=-14202/5274
BOT CHORD 2-9=-4226/11477, 9-10=-4226/11477, 10-11=-4226/11477, 8-11=-4226/11477, 7-8=-4664/12695, 6-7=-4664/12695
WEBS 3-9=-768/2274, 3-8=-2763/1074, 4-8=-3223/8714, 5-8=-4145/1636, 5-7=-1330/3449

NOTES
1) 3-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 4 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3132 lb uplift at joint 6 and 2320 lb uplift at joint 2.
7) Girder carries tie-in span(s): 38-9-8 from 8-0-0 to 20-0-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3617 lb down and 1366 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-11=-30, 6-11=-796(F=-766)
Concentrated Loads (lb)
Vert: 10=-3617(F)

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

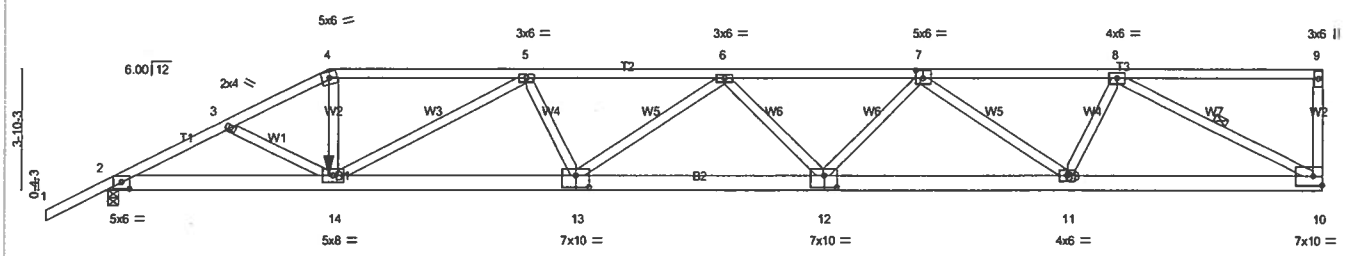
Dwg.#0519062581

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T05	MONO HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:16 2006 Page 1

-2-0-0	3-9-4	7-0-0	13-3-10	19-7-3	25-10-13	32-2-6	38-9-8
2-0-0	3-9-4	3-2-12	6-3-10	6-3-10	6-3-10	6-3-10	6-7-2

Scale = 1:70.1



3-9-4	7-0-0	14-10-8	22-9-0	30-7-8	38-9-8
3-9-4	3-2-12	7-10-8	7-10-8	7-10-8	8-2-0

Plate Offsets (X,Y): [2:0-3-0,0-2-9], [7:0-3-0,0-3-0], [12:0-5-0,0-4-8], [13:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.45	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.73	12-13	>637	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.14	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 457 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-0 oc bracing.
WEBS 1 Row at midpt 8-10

REACTIONS (lb/size) 10=3617/Mechanical, 2=3506/0-4-0
Max Horz 2=228(load case 4)
Max Uplift10=-1635(load case 3), 2=-1460(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-6962/2976, 3-4=-6916/3030, 4-5=-6302/2803, 5-6=-9635/4271, 6-7=-9766/4328, 7-8=-6671/2933, 8-9=-192/99, 9-10=-382/300
BOT CHORD 2-14=-2700/6131, 13-14=-4200/9230, 12-13=-4639/10123, 11-12=-4102/8937, 10-11=-2611/5680
WEBS 3-14=-148/73, 4-14=-957/2526, 5-14=-3362/1632, 5-13=-186/979, 6-13=-606/475, 6-12=-531/463, 7-12=-337/1234, 7-11=-2812/1450, 8-11=-779/2399, 8-10=-6234/2854

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) Refer to girder(s) for truss to truss connections.
6) All bearings are assumed to be SYP No.2
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1635 lb uplift at joint 10 and 1460 lb uplift at joint 2.
8) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-9=-120(F=-66), 2-14=-30, 10-14=-67(F=-37)
Concentrated Loads (lb)
Vert: 14=-539(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

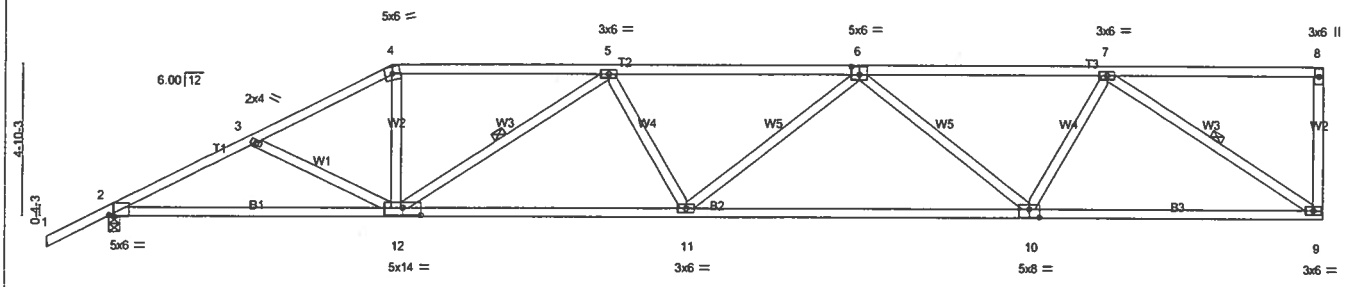
Dwg.#0519062582

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T06	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:17 2006 Page 1

-2-0-0	4-7-15	9-0-0	15-11-9	23-10-13	31-9-15	38-9-8
2-0-0	4-7-15	4-4-1	6-11-9	7-11-4	7-11-2	6-11-9

Scale = 1:70.1



9-0-0	18-5-0	29-4-10	38-9-8
9-0-0	9-5-0	10-11-10	9-4-15

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-3-0,0-3-0], [10:0-4-0,0-3-4], [12:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.51	10-11	>914	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.84	10-11	>551	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.18	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 199 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-15 oc bracing.
WEBS 1 Row at midpt 5-12, 7-9

REACTIONS (lb/size) 9=1613/Mechanical, 2=1736/0-4-0
Max Horz 2=272(load case 5)
Max Uplift9=-599(load case 4), 2=-564(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3010/1186, 3-4=-2807/1096, 4-5=-2499/1047, 5-6=-3422/1355, 6-7=-2526/970, 7-8=-66/17, 8-9=-170/116
BOT CHORD 2-12=-1199/2628, 11-12=-1389/3339, 10-11=-1351/3237, 9-10=-837/2037
WEBS 3-12=-177/183, 4-12=-244/914, 5-12=-1010/485, 5-11=0/176, 6-11=-10/240, 6-10=-925/496, 7-10=-281/1032, 7-9=-2371/987

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 599 lb uplift at joint 9 and 564 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

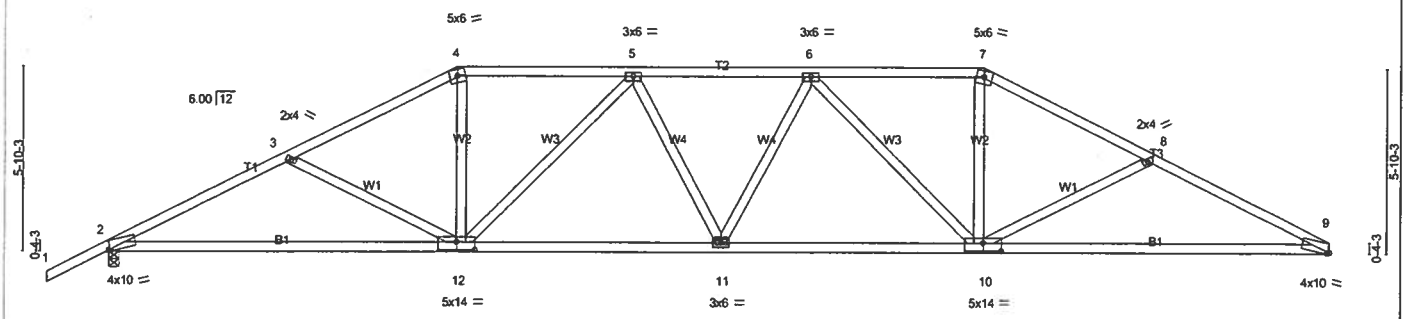
Dwg.#0519062583

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T07	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:18 2006 Page 1

-2-0-0	5-9-4	11-0-0	16-7-3	22-2-5	27-9-8	33-0-4	38-9-8
2-0-0	5-9-4	5-2-12	5-7-3	5-7-3	5-7-3	5-2-12	5-9-4

Scale = 1:69.9



11-0-0	19-4-12	27-9-8	38-9-8
11-0-0	8-4-12	8-4-12	11-0-0

Plate Offsets (X,Y): [2:0-0-13,Edge], [9:0-0-13,Edge], [10:0-7-0,0-3-0], [12:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.44	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.73	9-10	>631	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.16	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-15 oc bracing.

REACTIONS (lb/size) 9=1616/Mechanical, 2=1740/0-4-0
Max Horz 2=140(load case 5)
Max Uplift9=-466(load case 6), 2=-592(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2973/1247, 3-4=-2675/1109, 4-5=-2332/1053, 5-6=-2805/1244, 6-7=-2357/1077, 7-8=-2706/1140, 8-9=-3039/1311
BOT CHORD 2-12=-1014/2603, 11-12=-968/2747, 10-11=-977/2756, 9-10=-1088/2679
WEBS 3-12=-310/275, 4-12=-257/872, 5-12=-685/322, 5-11=-10/190, 6-11=-3/175, 6-10=-669/315, 7-10=-280/892, 8-10=-366/332

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 466 lb uplift at joint 9 and 592 lb uplift at joint 2.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

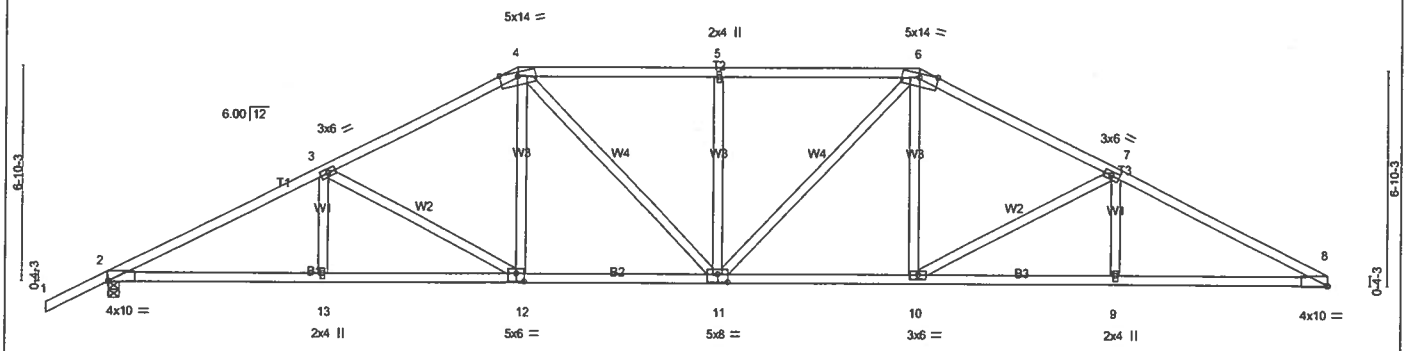
Dwg.#0519062584

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T08	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:19 2006 Page 1

-2-0-0	6-9-4	13-0-0	19-4-12	25-9-8	32-0-4	38-9-8
2-0-0	6-9-4	6-2-12	6-4-12	6-4-12	6-2-12	6-9-4

Scale = 1:69.9



6-9-4	13-0-0	19-4-12	25-9-8	32-0-4	38-9-8
6-9-4	6-2-12	6-4-12	6-4-12	6-2-12	6-9-4

Plate Offsets (X,Y): [2:Edge,0-0-4], [8:0-0-0,0-0-4], [11:0-4-0,0-3-0], [12:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.25	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.40	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 207 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-15 oc bracing.

REACTIONS (lb/size) 8=1616/Mechanical, 2=1740/0-4-0
Max Horz 2=154(load case 5)
Max Uplift8=-483(load case 6), 2=-609(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3051/1222, 3-4=-2468/1086, 4-5=-2413/1142, 5-6=-2413/1142, 6-7=-2484/1101, 7-8=-3113/1289
BOT CHORD 2-13=-985/2640, 12-13=-985/2640, 11-12=-708/2149, 10-11=-720/2163, 9-10=-1055/2709, 8-9=-1055/2709
WEBS 3-13=0/209, 3-12=-574/317, 4-12=-116/461, 4-11=-213/512, 5-11=-357/256, 6-11=-208/496, 6-10=-141/485, 7-10=-636/384, 7-9=0/228

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 483 lb uplift at joint 8 and 609 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33548

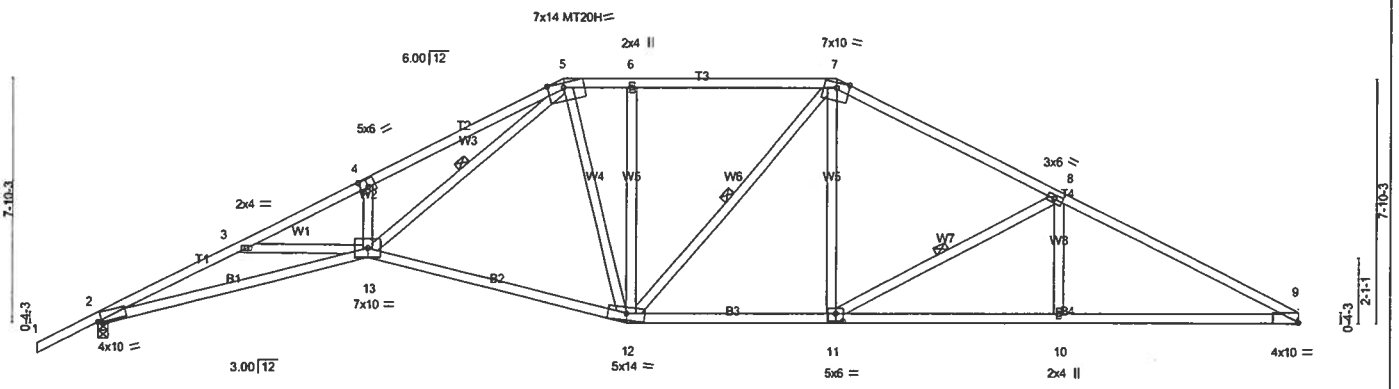
Dwg.#0519062585

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:19 2006 Page 1

-2-0-0	4-8-13	8-8-4	15-0-0	17-0-8	23-9-8	31-0-4	38-9-8
2-0-0	4-8-13	3-11-7	6-3-12	2-0-8	6-9-0	7-2-12	7-9-4

Scale = 1:70.9



8-8-4	17-0-8	23-9-8	31-0-4	38-9-8
8-8-4	8-4-4	6-9-0	7-2-12	7-9-4

Plate Offsets (X,Y): [2:0-1-6,Edge], [4:0-3-0,0-3-0], [5:0-6-3,Edge], [9:0-0-0,0-0-4], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.62	12-13	>752	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.99	12-13	>466	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.44	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-3 oc bracing.
WEBS 1 Row at midpt 5-13, 7-12, 8-11

REACTIONS (lb/size) 9=1616/Mechanical, 2=1740/0-4-0
Max Horz 2=167(load case 5)
Max Uplift9=-498(load case 6), 2=-625(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-5457/2217, 3-4=-5324/2104, 4-5=-5361/2265, 5-6=-2063/1034, 6-7=-2067/1035, 7-8=-2317/1058, 8-9=-3064/1283
BOT CHORD 2-13=-1930/4930, 12-13=-703/2202, 11-12=-640/2000, 10-11=-1037/2657, 9-10=-1037/2657
WEBS 3-13=-49/164, 4-13=-305/323, 5-13=-1366/3463, 5-12=-214/178, 6-12=-252/198, 7-12=-120/277, 7-11=-162/543, 8-11=-759/453, 8-10=0/272

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) All bearings are assumed to be SYP No.2
7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 498 lb uplift at joint 9 and 625 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33548

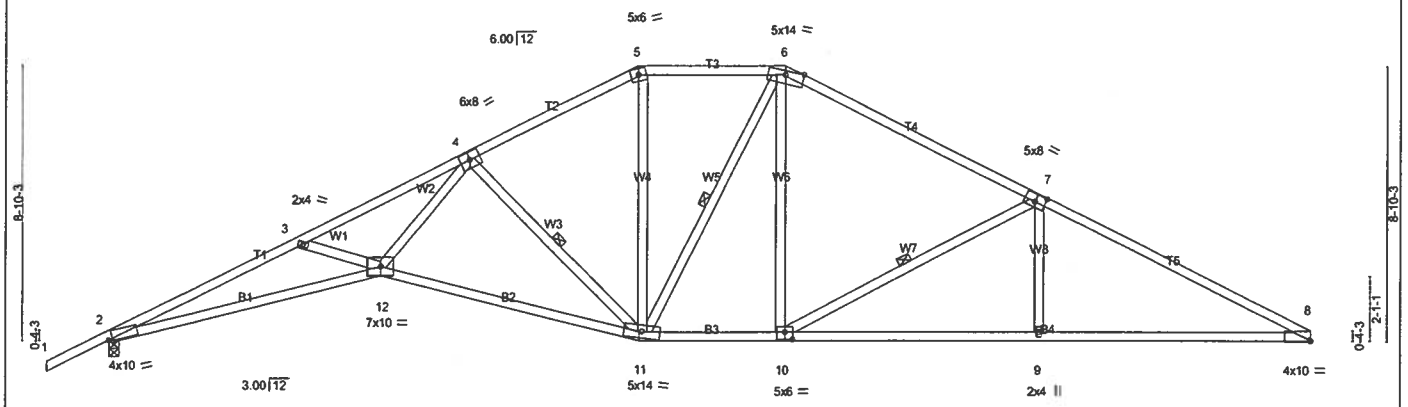
Dwg.#0519062586

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:20 2006 Page 1

-2-0-0	6-2-5	11-7-2	17-0-0	21-9-8	30-0-4	38-9-8
2-0-0	6-2-5	5-4-14	5-4-14	4-9-8	8-2-12	8-9-4

Scale = 1:70.9



8-8-4	17-0-8	21-9-8	30-0-4	38-9-8
8-8-4	8-4-4	4-9-0	8-2-12	8-9-4

Plate Offsets (X,Y): [2:0-1-6,Edge], [7:0-4-0,0-3-0], [8:0-0-0,0-0-4], [10:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	-0.57	11-12	>816	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.92	11-12	>504	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.42	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-11, 6-11, 7-10

REACTIONS (lb/size) 2=1740/0-4-0, 8=1616/Mechanical
Max Horz 2=181(load case 5)
Max Uplift2=-638(load case 5), 8=-512(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-5560/2277, 3-4=-5274/2101, 4-5=-2099/1029, 5-6=-1819/977, 6-7=-2142/1013, 7-8=-3012/1273
BOT CHORD 2-12=-1983/5036, 11-12=-1096/3042, 10-11=-555/1827, 9-10=-1016/2597, 8-9=-1015/2603
WEBS 3-12=-228/286, 4-12=-970/2861, 4-11=-1607/753, 5-11=-282/657, 6-11=-201/184, 6-10=-199/584, 7-10=-887/527, 7-9=0/317

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 638 lb uplift at joint 2 and 512 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#0519062587

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:21 2006 Page 1

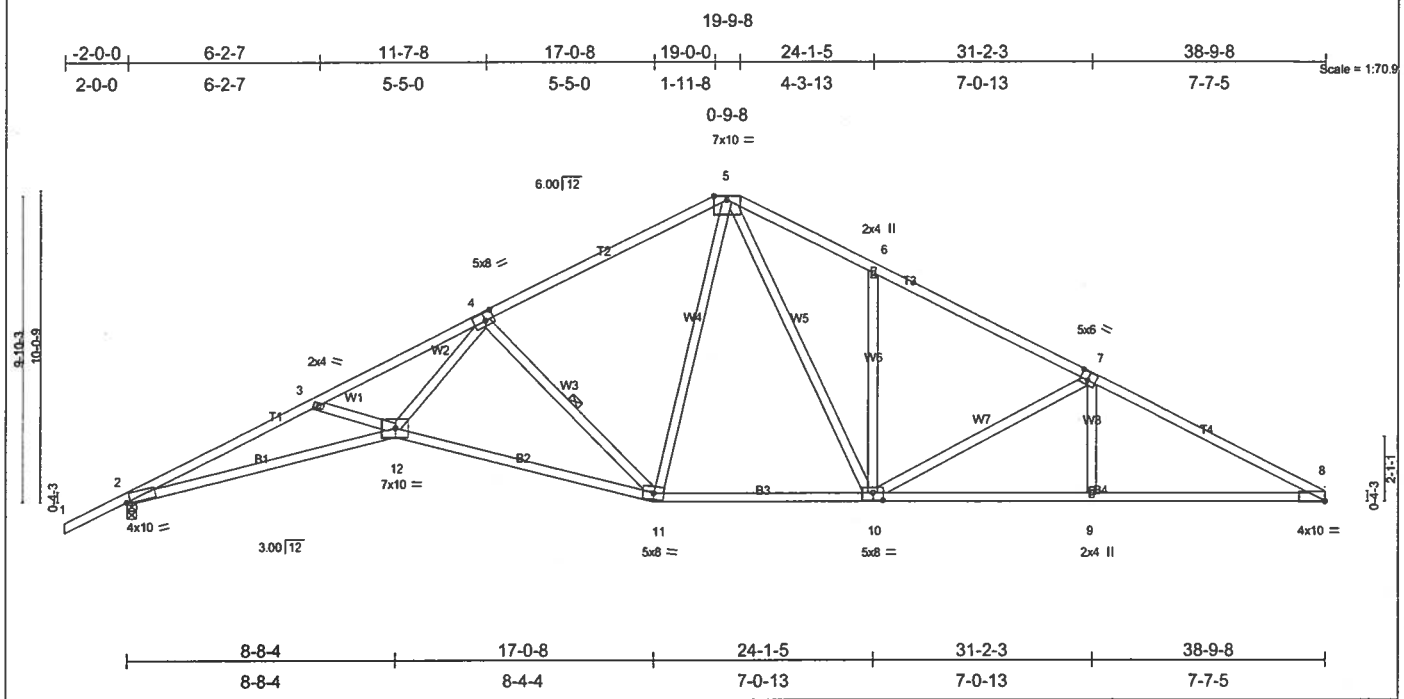


Plate Offsets (X,Y): [2:0-1-6,Edge], [4:0-3-8,0-3-0], [7:0-3-0,0-3-4], [8:0-0-0,0-0-4], [10:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.57	11-12	>817	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.92	11-12	>506	180		
BCLL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.42	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-8-6 oc bracing.
WEBS 1 Row at midpt 4-11

REACTIONS (lb/size) 2=1740/0-4-0, 8=1616/Mechanical
Max Horz 2=198(load case 5)
Max Uplift2=-652(load case 5), 8=-526(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-5544/2299, 3-4=-5279/2146, 4-5=-2091/1050, 5-6=-2296/1246, 6-7=-2347/1104, 7-8=-3073/1323
BOT CHORD 2-12=-2000/5019, 11-12=-1138/3059, 10-11=-463/1620, 9-10=-1075/2662, 8-9=-1074/2668
WEBS 3-12=-199/254, 4-12=-977/2841, 4-11=-1666/824, 7-10=-742/452, 7-9=0/262, 5-11=-294/744, 6-10=-306/277, 5-10=-497/949

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 652 lb uplift at joint 2 and 526 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

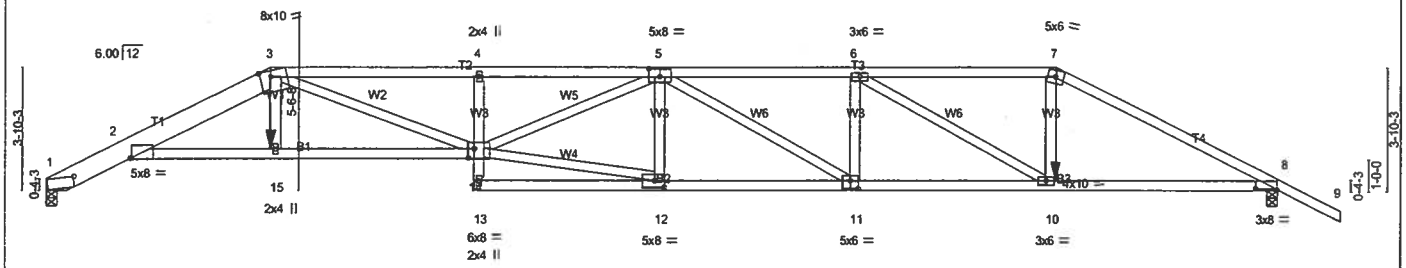
Dwg.#0519062588

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T12	SPECIAL	1	3	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:22 2006 Page 1

-2-0-0	7-0-0	13-5-0	19-3-0	25-5-6	31-9-8	35-0-4	38-9-8	40-9-8
2-0-0	7-0-0	6-5-0	5-10-0	6-2-6	6-4-2	3-2-12	3-9-4	2-0-0

Scale = 1:69.2



7-0-0	13-5-0	19-3-0	25-5-6	31-9-8	35-0-4	38-9-8
7-0-0	6-5-0	5-10-0	6-2-6	6-4-2	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-0-8,Edge], [3:0-4-3,Edge], [5:0-4-0,0-3-0], [7:31-0-10,0-8-0], [8:0-8-4,0-0-6], [11:0-3-0,0-3-0], [12:0-3-8,0-2-8], [14:0-2-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.57	12-13	>813	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.91	12-13	>506	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.60	Horz(TL)	0.36	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 599 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=3258/0-4-0, 8=3461/0-4-0
Max Horz 1=-110(load case 5)
Max Uplift1=-1229(load case 3), 8=-1411(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1262/545, 2-3=-9217/3608, 3-4=-12243/5082, 4-5=-12071/5024, 5-6=-8961/3855, 6-7=-6076/2605, 7-8=-6800/2838, 8-9=0/47
BOT CHORD 2-15=-3376/8704, 14-15=-3399/8781, 12-13=-191/488, 11-12=-4090/9874, 10-11=-3736/8961, 8-10=-2445/5972
WEBS 3-15=-382/1314, 3-14=-1791/3794, 13-14=-2/220, 4-14=-272/293, 5-14=-932/2408, 5-12=-1184/758, 5-11=-1085/408, 6-11=-125/944, 6-10=-3414/1516, 7-10=-953/2548, 12-14=-3956/9523

NOTES
1) 3-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All bearings are assumed to be SYP No.2
7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1229 lb uplift at joint 1 and 1411 lb uplift at joint 8.
9) Girder carries tie-in span(s): 5-10-11 from 7-0-0 to 13-5-0; 3-0-6 from 7-0-0 to 13-5-0
10) Girder carries hip end with 7-0-0 right side setback, 13-5-0 left side setback, and 7-0-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 204 lb up at 7-0-0, and 539 lb down and 277 lb up at 31-9-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-65, 2-3=-54, 3-4=-69(F=-15), 4-7=-117(F=-63), 7-9=-54, 2-15=-30, 14-15=-105(F=-75), 10-13=-65(F=-35), 8-10=-30
Concentrated Loads (lb)
Vert: 15=-539(F) 10=-539(F)

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

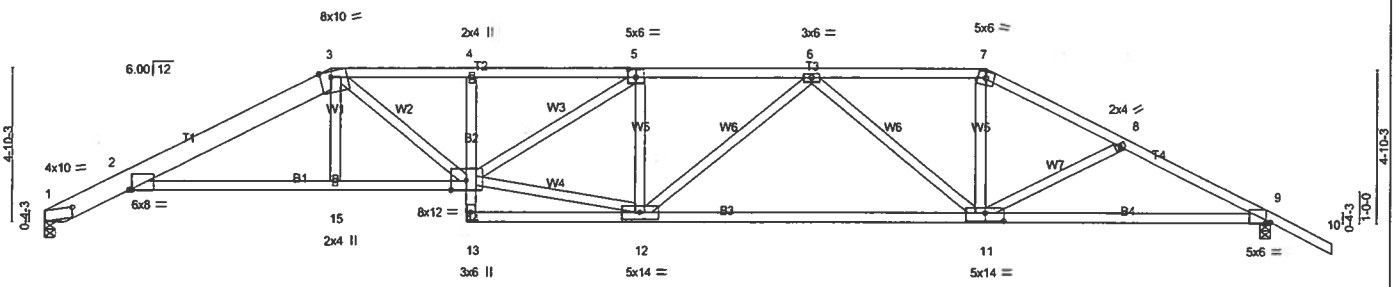
Dwg.#0519062589

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:23 2006 Page 1

2-0-0	2-4-0	3-4-0	9-0-0	13-5-0	18-10-8	24-3-2	29-9-8	34-0-4	38-9-8	40-9-8
2-0-0	2-4-0	1-0-0	5-8-0	4-5-0	5-5-8	5-4-10	5-6-6	4-2-12	4-9-4	2-0-0

Scale = 1:69.7



2-4-0	3-4-0	9-0-0	13-5-0	18-10-8	29-9-8	38-9-8
2-4-0	1-0-0	5-8-0	4-5-0	5-5-8	10-11-0	9-0-0

Plate Offsets (X,Y): [2:1-9-15,0-3-15], [2:0-1-0,Edge], [3:0-4-3,Edge], [5:0-3-0,0-3-0], [9:0-1-11,Edge], [11:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	-0.70	11-12	>662	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-1.15	11-12	>401	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.47	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 6-0-11 oc bracing. Except:
1 Row at midpt 2-15

REACTIONS (lb/size) 1=1566/0-4-0, 9=1734/0-4-0
Max Horz 1=-124(load case 6)
Max Uplift1=-444(load case 5), 9=-572(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-604/340, 2-3=-3574/1391, 3-4=-3905/1615, 4-5=-3892/1618, 5-6=-3459/1462, 6-7=-2481/1064, 7-8=-2822/1130, 8-9=-3034/1221, 9-10=0/47
BOT CHORD 2-15=-1084/3309, 14-15=-1085/3323, 13-14=0/53, 4-14=-118/140, 12-13=-118/286, 11-12=-1081/3152, 9-11=-921/2636
WEBS 3-15=-35/341, 3-14=-368/849, 12-14=-1063/3248, 5-14=-178/524, 5-12=-604/320, 6-12=-108/457, 6-11=-941/448, 7-11=-287/958, 8-11=-179/189

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 444 lb uplift at joint 1 and 572 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

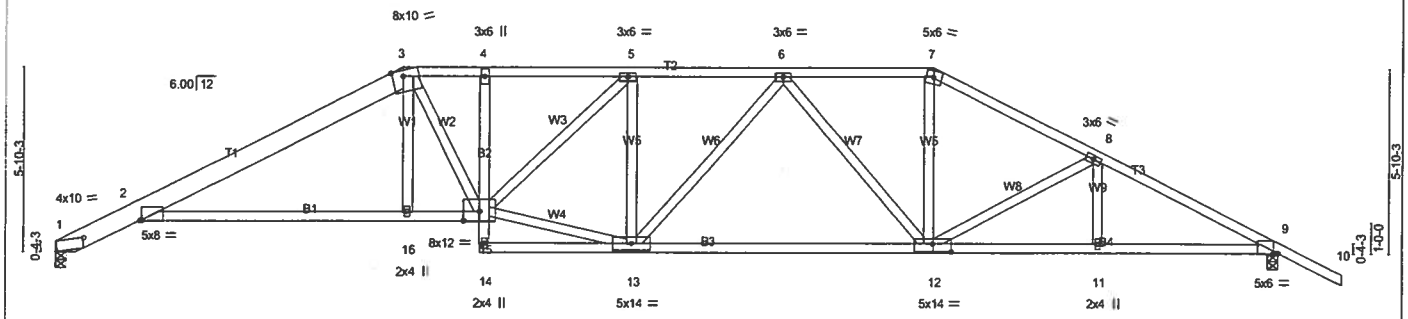
Dwg.#0519062590

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:24 2006 Page 1

2-0-0	2-4-0	3-4-0	11-0-0	13-5-0	18-2-8	23-0-0	27-9-8	33-0-4	38-9-8	40-9-8
2-0-0	2-4-0	1-0-0	7-8-0	2-5-0	4-9-8	4-9-8	4-9-8	5-2-12	5-9-4	2-0-0

Scale = 1:69.7



13-5-0	18-2-8	27-9-8	33-0-4	38-9-8
13-5-0	4-9-8	9-7-0	5-2-12	5-9-4

Plate Offsets (X,Y): [2:1-9-14,0-4-0], [2:0-0-8,Edge], [3:0-4-3,Edge], [9:0-1-11,Edge], [12:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	-0.76	2-16	>611	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-1.22	2-16	>377	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.50	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 225 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*
T2 2 X 4 SYP No.1D, T3 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 6-7-10 oc bracing. Except:
1 Row at midpt 2-16

REACTIONS (lb/size) 1=1566/0-4-0, 9=1734/0-4-0
Max Horz 1=-138(load case 6)
Max Uplift1=-463(load case 5), 9=-591(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-604/343, 2-3=-3285/1287, 3-4=-3022/1314, 4-5=-3026/1321, 5-6=-2821/1257, 6-7=-2304/1052, 7-8=-2639/1113, 8-9=-3057/1210, 9-10=0/47
BOT CHORD 2-16=-952/3002, 15-16=-954/3017, 14-15=-3/46, 4-15=-72/218, 13-14=-90/279, 12-13=-866/2665, 11-12=-906/2650, 9-11=-906/2650
WEBS 13-15=-825/2601, 5-15=-110/317, 5-13=-510/269, 6-13=-71/312, 6-12=-650/314, 7-12=-279/870, 8-12=-402/245, 8-11=0/128, 3-16=-56/467, 3-15=-199/207

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 463 lb uplift at joint 1 and 591 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

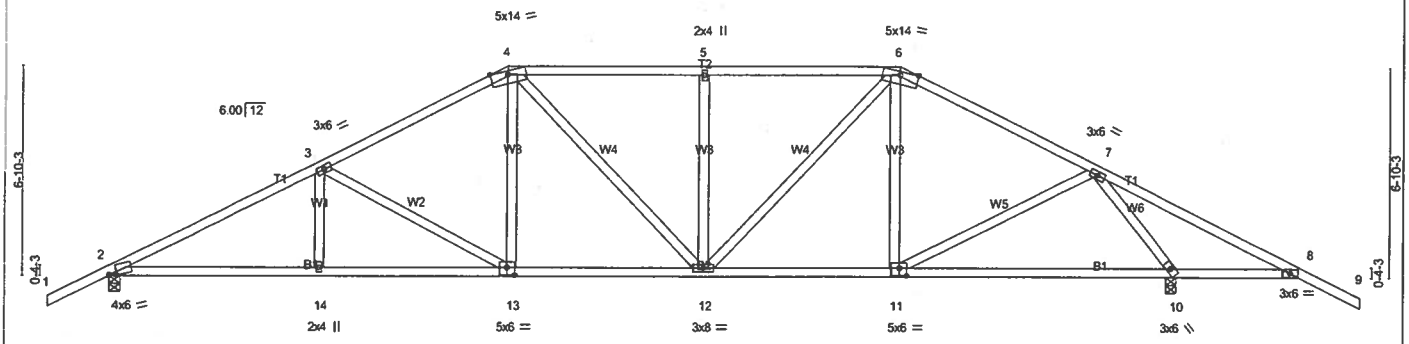
Dwg.#0519062591

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:24 2006 Page 1

-2-0-0	6-9-4	13-0-0	19-4-12	25-9-8	32-0-4	38-9-8	40-9-8
2-0-0	6-9-4	6-2-12	6-4-12	6-4-12	6-2-12	6-9-4	2-0-0

Scale = 1:71.7



6-9-4	13-0-0	19-4-12	25-9-8	34-7-8	38-9-8
6-9-4	6-2-12	6-4-12	6-4-12	8-10-0	4-2-0

Plate Offsets (X,Y): [2:0-2-12,0-0-11], [11:0-3-0,0-3-0], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.16	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.27	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.09	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 212 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=1537/0-4-0, 10=1932/0-4-0
Max Horz 2=-129(load case 6)
Max Uplift2=-561(load case 5), 10=-778(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2611/985, 3-4=-2024/844, 4-5=-1815/820, 5-6=-1815/820, 6-7=-1626/622, 7-8=-790/693, 8-9=0/47
BOT CHORD 2-14=-695/2250, 13-14=-695/2250, 12-13=-413/1752, 11-12=-238/1381, 10-11=-129/725, 8-10=-546/855
WEBS 3-14=0/209, 3-13=-579/323, 4-13=-117/468, 4-12=-154/238, 5-12=-357/257, 6-12=-287/673, 6-11=-96/193, 7-11=-298/739, 7-10=-2091/1320

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 561 lb uplift at joint 2 and 778 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

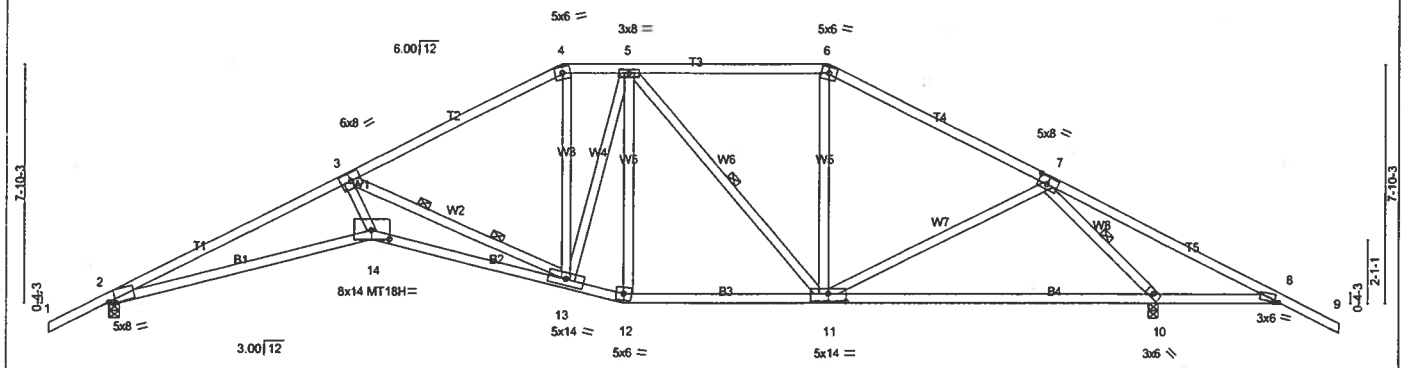
Dwg.#0519062592

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:25 2006 Page 1

-2-0-0	7-10-11	15-0-0	17-0-8	23-9-8	31-0-4	38-9-8	40-9-8
2-0-0	7-10-11	7-1-5	2-0-8	6-9-0	7-2-12	7-9-4	2-0-0

Scale = 1:72.6



8-8-4	15-0-0	17-0-8	23-9-8	29-2-8	34-7-8	38-9-8
8-8-4	6-3-12	2-0-8	6-9-0	5-5-0	5-5-0	4-2-0

Plate Offsets (X,Y): [2:0-2-6,Edge], [7:0-3-12,0-3-0], [8:0-1-12,0-0-7], [11:0-7-0,0-3-0], [14:0-7-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	-0.56	2-14	>742	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.90	2-14	>459	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.98	Horz(TL)	0.44	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 219 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 5-11, 7-10
2 Rows at 1/3 pts 3-13

REACTIONS (lb/size) 2=1539/0-4-0, 10=1930/0-4-0
Max Horz 2=-143(load case 6)
Max Uplift2=-575(load case 5), 10=-793(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4679/1661, 3-4=-1978/836, 4-5=-1703/826, 5-6=-1391/677, 6-7=-1646/671, 7-8=-817/685, 8-9=0/47
BOT CHORD 2-14=-1324/4227, 13-14=-1593/5276, 12-13=-336/1654, 11-12=-326/1594, 10-11=-199/981, 8-10=-527/890
WEBS 3-14=-699/2714, 3-13=-3787/1317, 4-13=-138/573, 5-13=-184/420, 5-12=-339/99, 5-11=-397/174, 6-11=-38/371, 7-11=-148/476, 7-10=-2191/1445

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 575 lb uplift at joint 2 and 793 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

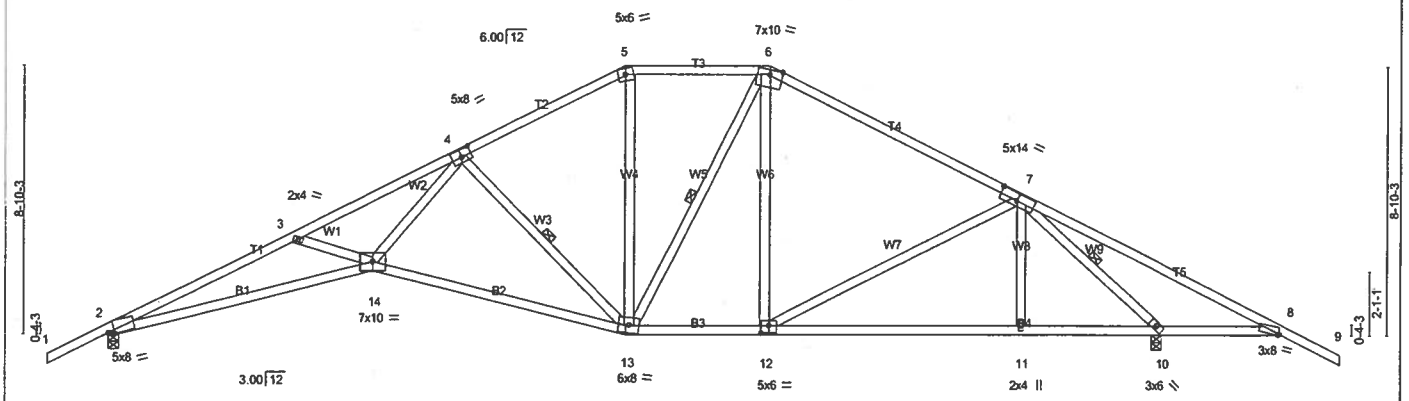
Dwg.#0519062593

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:26 2006 Page 1

-2-0-0	6-2-5	11-7-2	17-0-0	21-9-8	30-0-4	38-9-8	40-9-8
2-0-0	6-2-5	5-4-14	5-4-14	4-9-8	8-2-12	8-9-4	2-0-0

Scale = 1:72.6



8-8-4	17-0-8	21-9-8	34-7-8	38-9-8
8-8-4	8-4-4	4-9-0	12-10-0	4-2-0

Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-0-0,0-0-0], [4:0-4-0,0-3-0], [5:0-0-0,0-0-0], [7:0-7-0,0-3-0], [8:0-0-10,Edge], [12:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.46	13-14	>909	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.74	13-14	>560	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(TL)	0.33	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 219 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-13, 6-13, 7-10

REACTIONS (lb/size) 2=1539/0-4-0, 10=1930/0-4-0
Max Horz 2=-157(load case 6)
Max Uplift2=-587(load case 5), 10=-808(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4755/1757, 3-4=-4449/1568, 4-5=-1655/785, 5-6=-1420/757, 6-7=-1592/706, 7-8=-771/823, 8-9=0/47
BOT CHORD 2-14=-1428/4303, 13-14=-703/2513, 12-13=-219/1329, 11-12=-237/1208, 10-11=-236/1212, 8-10=-637/860
WEBS 3-14=-246/304, 4-14=-692/2493, 4-13=-1439/626, 5-13=-179/466, 6-13=-118/316, 6-12=0/179, 7-12=-113/198, 7-11=0/212, 7-10=-2528/1418

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 587 lb uplift at joint 2 and 808 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#0519062594

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:27 2006 Page 1

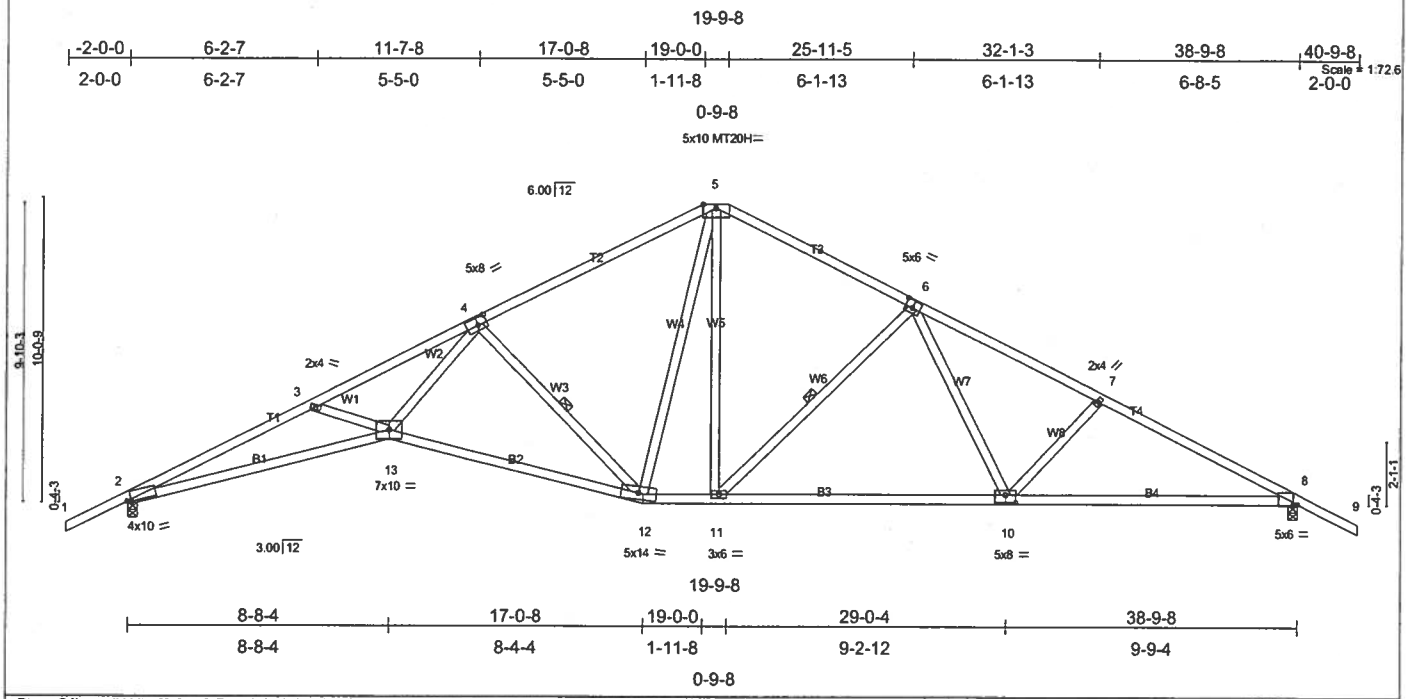


Plate Offsets (X,Y): [2:0-1-6,Edge], [4:0-3-8,0-3-0], [6:0-3-0,0-3-0], [8:0-1-11,Edge], [10:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.57	12-13	>815	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.92	12-13	>503	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.41	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-11-1 oc bracing.
WEBS 1 Row at midpt 4-12, 6-11

REACTIONS (lb/size) 2=1732/0-4-0, 8=1732/0-4-0
Max Horz 2=-174(load case 6)
Max Uplift2=-650(load case 5), 8=-650(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-5517/2194, 3-4=-5249/2035, 4-5=-2080/1039, 5-6=-1890/967, 6-7=-2733/1214, 7-8=-2963/1276, 8-9=0/47
BOT CHORD 2-13=-1825/4995, 12-13=-1009/3032, 11-12=-370/1626, 10-11=-679/2123, 8-10=-955/2584
WEBS 3-13=-198/263, 4-13=-891/2842, 5-11=-258/680, 5-12=-314/654, 4-12=-1653/773, 6-11=-722/430, 6-10=-165/593, 7-10=-304/298

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 650 lb uplift at joint 2 and 650 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

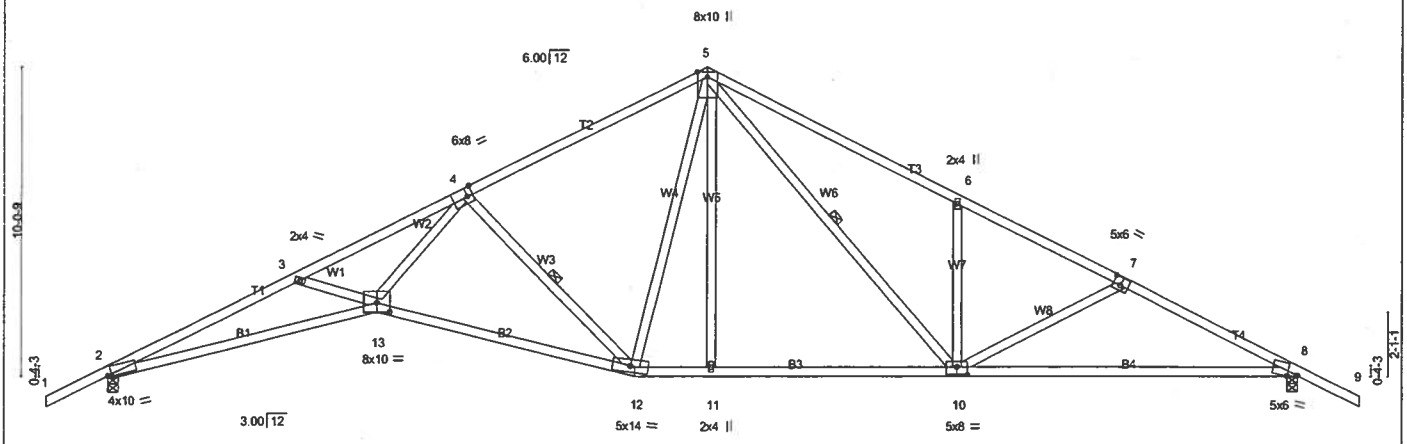
Dwg.#0519062595

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- LOT 65 EMERALD COVE
L164073	T19	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon May 15 10:31:28 2006 Page 1

-2-0-0	6-2-7	11-7-8	17-0-8	19-4-12	27-7-12	32-11-6	38-9-8	40-9-8
2-0-0	6-2-7	5-5-0	5-5-0	2-4-4	8-3-0	5-3-10	5-10-2	2-0-0

Scale = 1:71.7



8-8-4	17-0-8	27-7-12	38-9-8
8-8-4	8-4-4	10-7-4	11-1-12

Plate Offsets (X,Y): [2:0-1-6,Edge], [4:0-2-4,Edge], [7:0-3-0,0-3-0], [8:0-3-13,Edge], [10:0-4-0,0-3-0], [13:0-5-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.57	12-13	>814	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.92	12-13	>503	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.41	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 215 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-11-1 oc bracing.
WEBS 1 Row at midpt 5-10, 4-12

REACTIONS (lb/size) 2=1732/0-4-0, 8=1732/0-4-0
Max Horz 2=-174(load case 6)
Max Uplift2=-650(load case 5), 8=-650(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-5520/2199, 3-4=-5247/2034, 4-5=-2081/1039, 5-6=-2687/1375, 6-7=-2654/1168, 7-8=-2942/1286, 8-9=0/47
BOT CHORD 2-13=-1829/4998, 12-13=-1004/3027, 11-12=-381/1625, 10-11=-380/1627, 8-10=-967/2573
WEBS 3-13=-199/270, 5-10=-575/1108, 6-10=-389/392, 7-10=-288/245, 5-12=-289/694, 4-12=-1637/759, 4-13=-895/2844, 5-11=0/110

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 650 lb uplift at joint 2 and 650 lb uplift at joint 8.

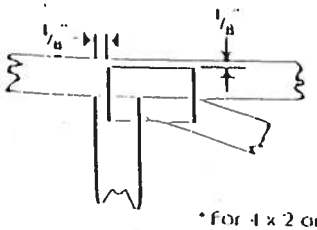
LOAD CASE(S) Standard

MAY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

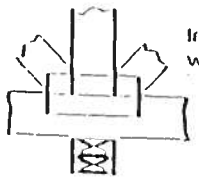
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

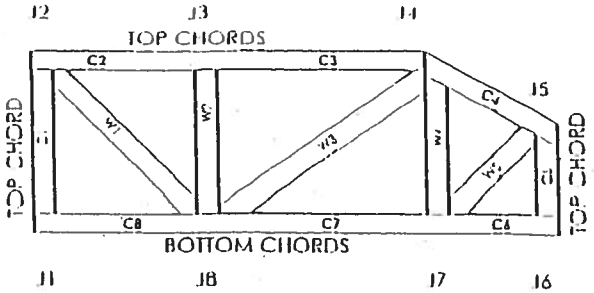
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

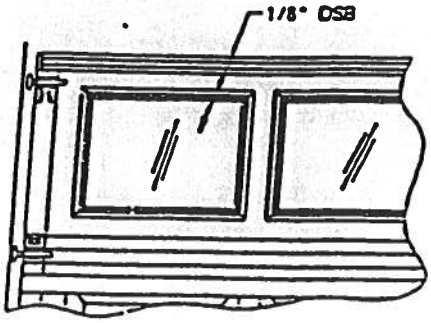
1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

GARAGE DOORS

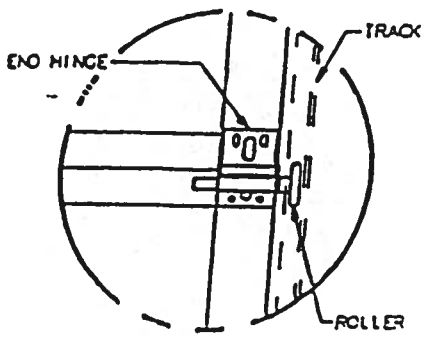
REVISIONS			
LETTER	DESCRIPTION	DATE	
A	REV PER EN 10130	3/06/98	
B	REV PER EN 10141	3/13/98	
C	REV PER EN 10132	4/8/98	

...UCTED WITH 8 OR MORE SECTIONS MUST BE
E STRUTS FOR THE ADDITIONAL INTERMEDIATE
ON THE THIRD SECTION.
..L SECTIONS LESS THAN 20.812" MUST BE
THE ACTUAL SECTION HEIGHT & 20.812".
1 X 5/8" LONG TYPE AB HEX HEAD METAL SCREW
WIND WARNINGS ISSUED.

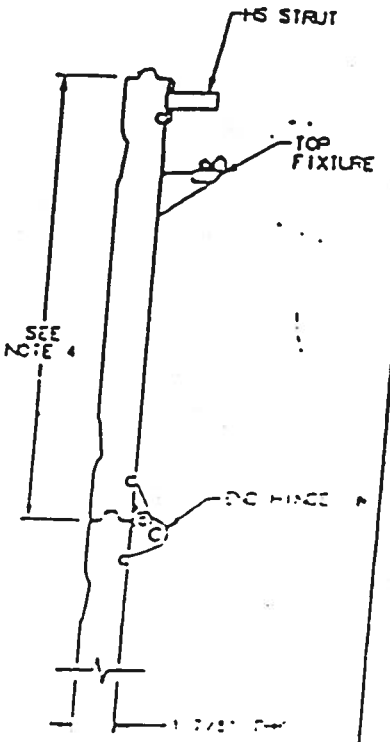
2.



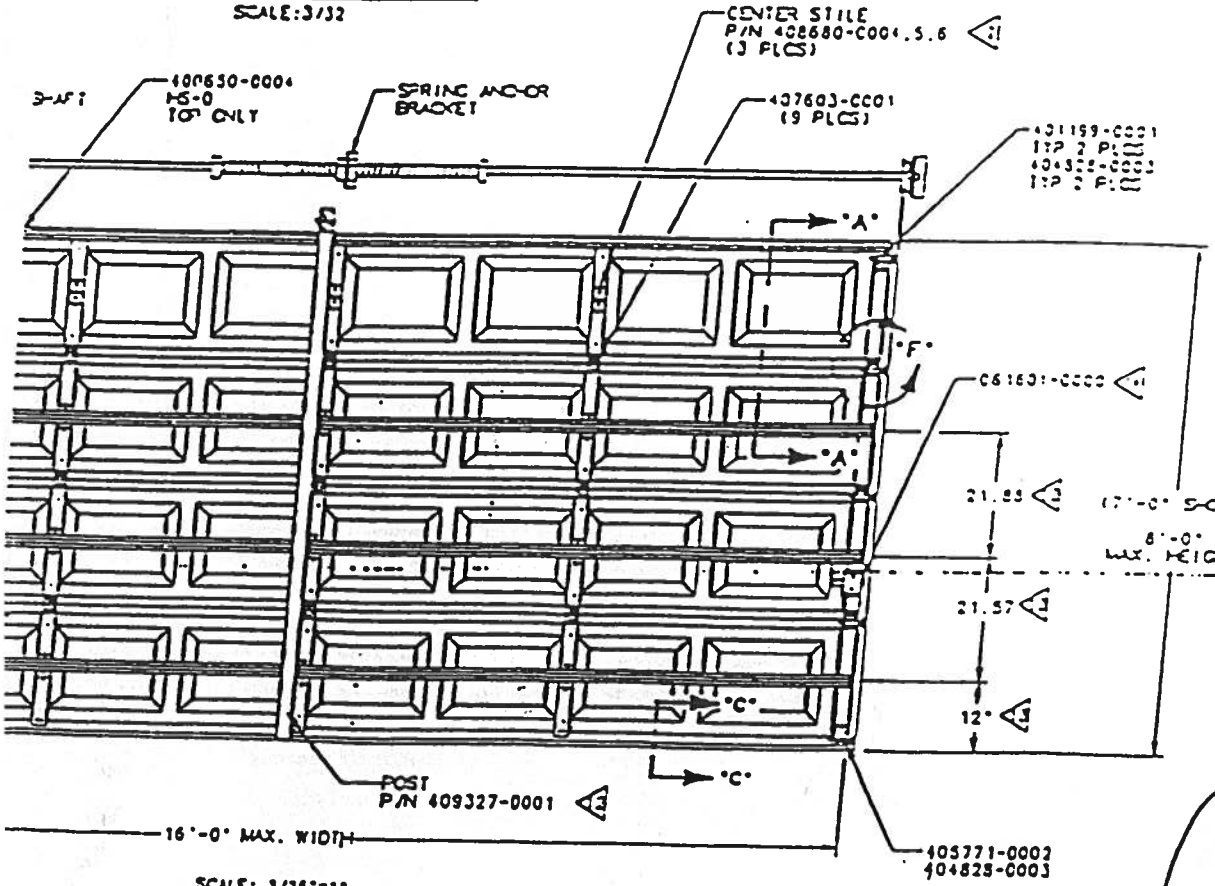
OPTIONAL WINDOW DETAIL 4
SCALE: 3/32



DETAIL "F"
SCALE: 1/4
(16" WIDE)



SECTION "A-A"
SCALE 1/4

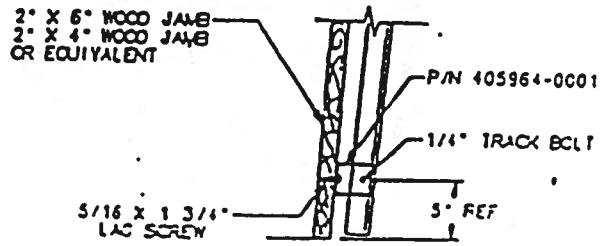


SCALE: 1/16"=1"
INTERIOR ELEVATION

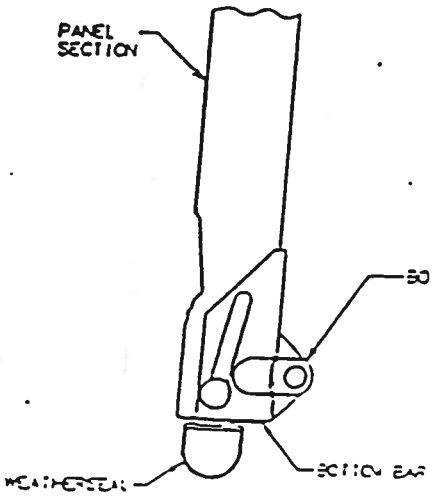
DRAWN BY: M.TOLNIS	01/13/98	SERIES 280 & 180 RESI STL DR, 16'-0" MAX WIDTH, WINDLOAD	
CHECKED BY: DAVID FAX	02/19/98	DRAWING NUMBER: D-409335	
APPROVED BY: DAVID FAX	02/19/98		

NOTES:

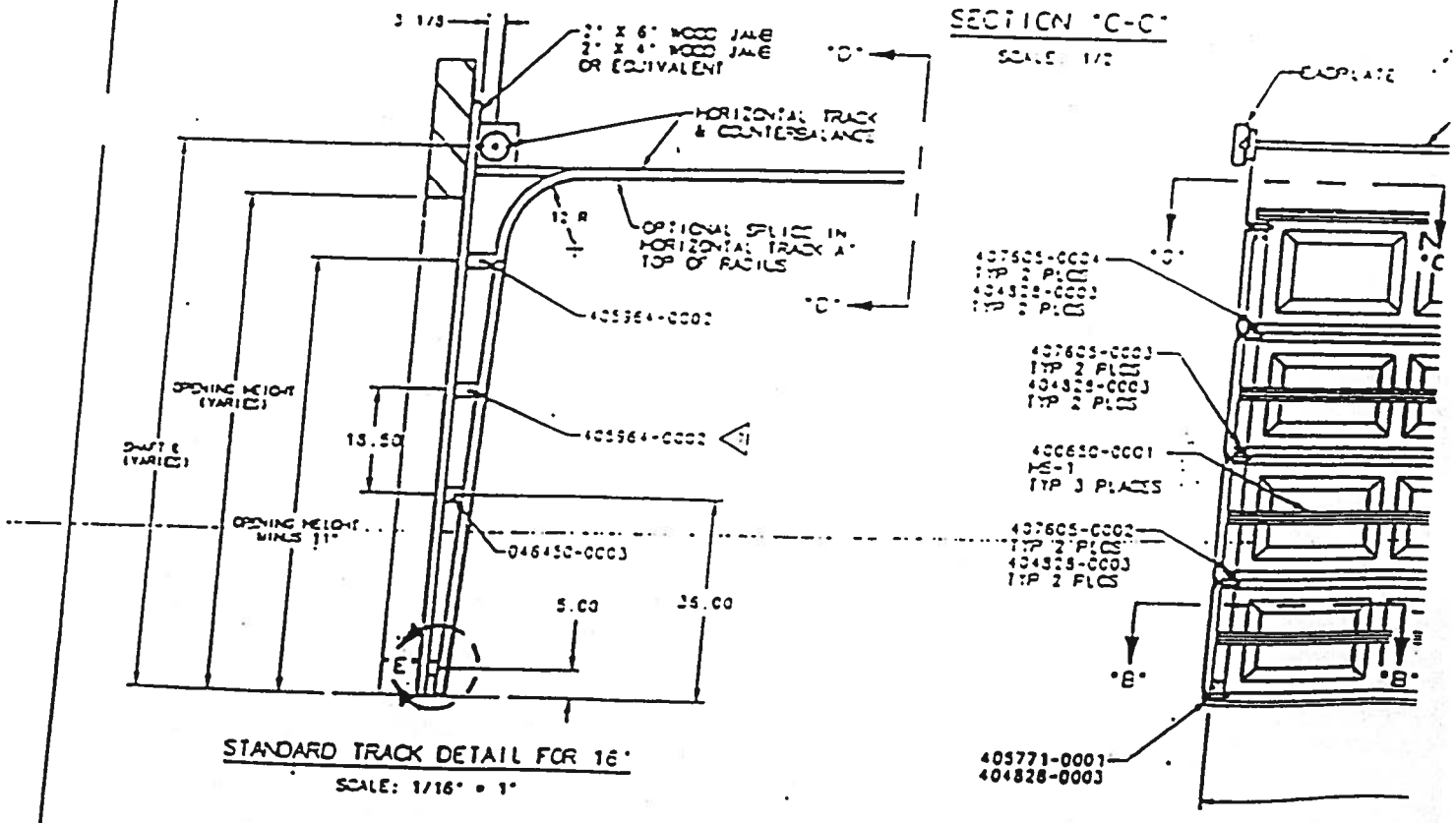
1. TESTED IN ACCORDANCE WITH STANDARD BUILDING CODE, CHAPTER 17, TO A POSITIVE AND NEGATIVE 37.5 PSF.
2. DASH NUMBERS REPRESENT VARIOUS SECTION HEIGHTS.
3. FOUR SECTION 7' HIGH DOOR SHOWN. 8' HIGH DOORS HAVE FIVE SECTIONS.
4. SECTION HEIGHT OF 20.812, 19.00 & 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS HEIGHT DOORS.
5. EMBOSSMENT PATTERN OF 14.50 X 20.375 SHOWN. ALTERNATE PATTERNS OF 12.50 X 43.375 AND 12.50 X 20.375 MAY BE USED.
6. TORSION SPRINGS SHOWN. EXTENSION SPRINGS AVAILABLE.
7. USE THIS BRACKET, REF. P/N 405964-0002, ON 8' HIGH DOORS ONLY.
8. WINDOW MAY BE INSTALLED IN THE TOP SECTION OR THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
9. THE STRUT PLACEMENT ON D CONSISTENT WITH THE DOOR SECTIONS ARE TO BE PLACE
10. THE STRUT PLACEMENT DIMEN REDUCED BY THE DIFFERENCE
11. SCREW P/N 605911-0001 IS
12. POST TO BE INSTALLED ONLY
13. STRUT PLACEMENTS CAN VARY
14. QUANTITY FOR LOCKS CAN BE



DETAIL "E"
SCALE: 1/3



VIEW "D-
2" TRACK
.049 THK (VERT



STANDARD TRACK DETAIL FOR 16'
SCALE: 1/16" = 1"

SECTION "C-C"
SCALE: 1/2

SERIES 280 THRU 289 ARE EQUIVALENT CONSTRUCTION 25GA STEEL
SERIES 180 ARE SAME CONSTRUCTION AS SERIES 280 ONLY 24GA STEEL AND END CAPS

DOOR TESTED WAS 281 SERIES.

DOOR WIDTH	CENTER STILE	END STILE	ROLLER SHAFT BRACKET	STRUTS/SECT.	ROLLER	VERTICAL TRACK GAGE	JAMB LOAD (# PER FT-HT)	HARDWARE
16'	3	SINGLE		HS1 HS0	2" X 7/16"	.049"	100	STD.

Anthony Power Header®

GARAGE HEADER (84) 26F$_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

	810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
	990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-3/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
	610 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-1/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"
	765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"
	750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"
	900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	—

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

WOODFORD PLYWOOD, INC.
"Structural Wood Products"

11960 West Beaver Street
Jacksonville, Florida 32220

(904) 695-9080
(800) 447-6568
FAX (904) 695-9160

Anthony POWER HEADER®

26F$_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1373							
161	207	254	330	390	510	552	669	752	824			
114	145	180	231	277	359	391	510	534	653	707	789	

844	975		1322									
161	207	254	330	390	510	552	724	732	897			
114	145	180	231	277	359	391	510	534	699	693		

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	268	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

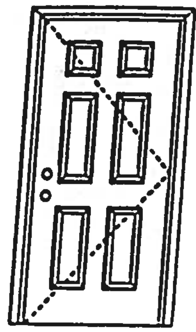
To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
limited water unless special threshold design is used

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required

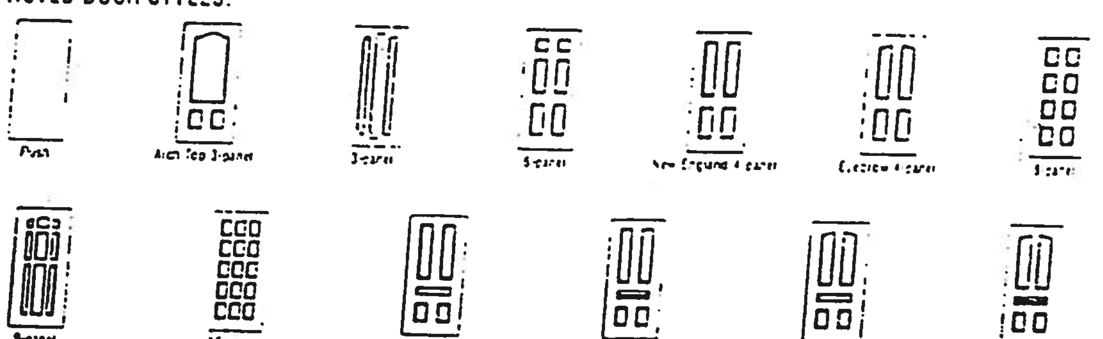
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02

APPROVED DOOR STYLES:



Flush | Arch Top 3-panel | 3-panel | 6-panel | New England 4-panel | Eyebrow 4-panel | 8-panel | 9-panel | 15-panel | 5-panel | 5-panel with scroll | Eyebrow 5-panel | Eyebrow 5-panel with scroll

Johnson EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOOR Collection Premium Quality Doors

Exclusively from Masonite
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson EntrySystems

June 17, 2002
[illegible]

PREMDOOR
Exclusively from Masonite
Masonite International Corporation

WINDOWS

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:mlb

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
No. 15054
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.architest.com

Allen G. Reeves
1 APRIL 2002

REEVES ☆ PROFESSIONAL ENGINEER ☆ NO. 18354 ☆ STATE OF FLORIDA

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen H. Reeves
1 APRIL 2002

ALLEN H. REEVES
CERTIFICATE
NO. 1[illegible]
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES CERTIFICATE NO. 15244 STATE OF FLORIDA PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft²	0.3 cfm/ft² max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Allen H. Reeves
1 APRIL 2002

ALLEN H. REEVES
CERTIFICATE
NO. 18377
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:mb
01-41134.01

ALLEN N. REEVES
CERTIFICATE NO. 1[illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	**Deglazing Test (ASTM E 987)**		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test:	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test:	No entry	No entry
	Optional Performance		
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.15" max.
	@ 70.8 psf (negative)	0.05"	0.15" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
No. 18334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SHINGLES

ELK Premium Roofing

March 6, 2002

Subject: Elk Product Approval Information

All Prestique® and Capstone® products manufactured in Tuscaloosa, AL are certified under the Miami – Dade County Building Code Office (BCCO). These products also meet the requirements for the Florida Building Code since they are MD approved. The following test protocols must be passed by each of the products in order for MD product certification:

ASTM D3462
PA 100 (110 mph uplift and wind driven rain resistance)
PA 107 (Modified ASTM D3161 - 110 mph wind uplift resistance)

The nailing patterns that were used during the PA 100 and PA 107 wind test protocols for the Prestique and Capstone products are listed below. Also listed below are the Miami – Dade Notice of Acceptance Numbers (NOA)

Raised Profile, Prestique High Definition, Prestique 25, or Prestique 30 –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.04

Prestique I 55 or Prestique I* –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.05

Prestique Plus or Prestique Gallery Collection* –
PA 100 = 4 nails
PA 107 = 4 nails
MD NOA# = 01-1226.03

Capstone*
PA 100 = 4 Nails
PA 107 = 4 Nails
MD NOA# = 01-0523.01

* As per the Elk Limited Warranty, six nails are required for the Elk high wind warranty

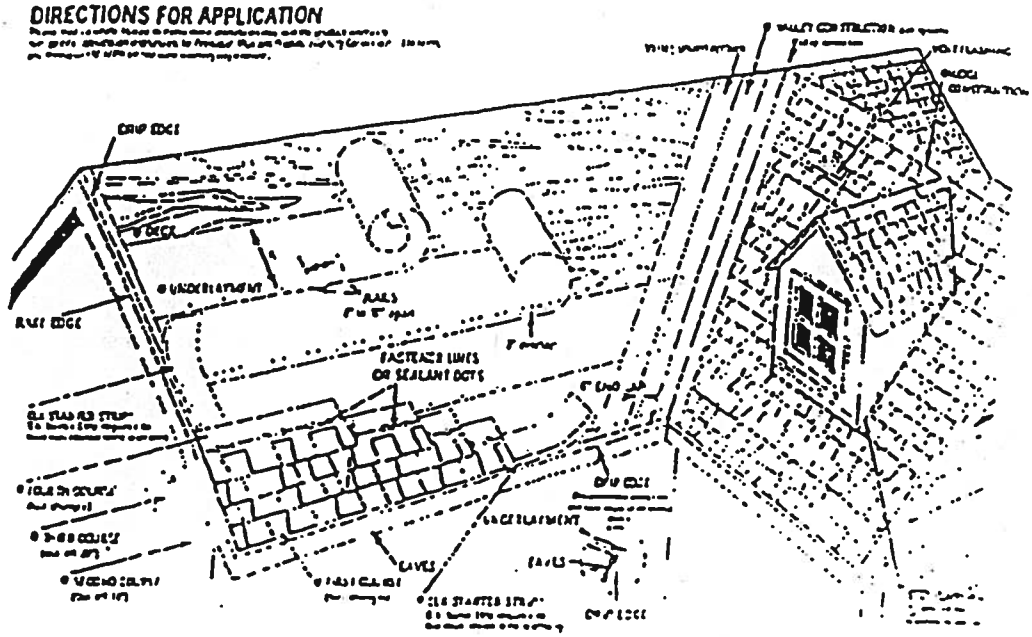
If there are any questions please contact:

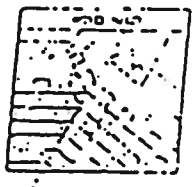
Mike Reed – Technical Manager (205) 342-0287

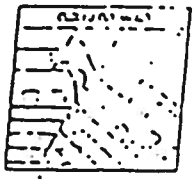
or

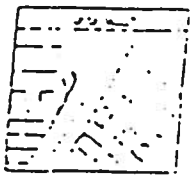
Daniel Delamette – QA Engineer (205) 342-0293

DIRECTIONS FOR APPLICATION









DIRECTIONS FOR APPLICATION

STARTER COURSE

FIELD COURSE

VALLEY CONSTRUCTION

EAVE FLASHING FOR ICE DAMS (AS A SECOND CONTRACTOR, REFER TO APPLICABLE MANUAL OR CHECK LOCAL CODES)

STARTER SHINGLE COURSE

FIRST COURSE

SECOND COURSE

THIRD COURSE

HIP AND RIDGE APPLICATIONS

LIMITED WIND WARRANTY

CAUTION TO WHOLESALER

ELK

www.elkcorp.com

ROOFING PRODUCTS SPECIFICATIONS – TUSCALOOSA, AL

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

[illegible]

HIP AND RIDGE SHINGLES

[illegible]

All Prestique and Raised Profile shingles meet UL Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790) and ASTM Specifications D 3018, Type I, D 3161, Type I, E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Miami Dade building code requirements.

[illegible]

SOUTHEAST & ATLANTIC OFFICE:
800.945.5551

CORPORATE HEADQUARTERS:
800.354.7732

PLANT LOCATION:
800.945.5545

ELK
www.elkcorp.com

Residential System Sizing Calculation

Summary

The Jane Model

Lake City, FL

Project Title:
605112CornerstoneDevelopmentTheJaneModel

Class 3 Rating
Registration No. 0
Climate: North

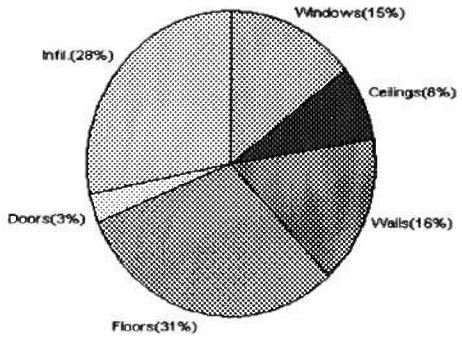
5/16/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**24944**	**Btuh**	**Total cooling load calculation**	**19713**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	112.3	28000	Sensible (SHR = 0.75)	138.0	21000
Heat Pump + Auxiliary(0.0kW)	112.3	28000	Latent	155.9	7000
			Total (Electric Heat Pump)	142.0	28000

WINTER CALCULATIONS

Winter Heating Load (for 1608 sqft)

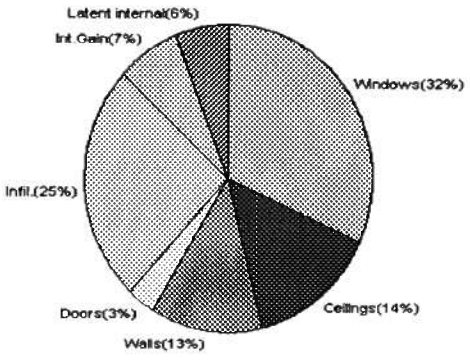
Load component			Load	
Window total	113	sqft	3628	Btuh
Wall total	1206	sqft	3962	Btuh
Door total	60	sqft	777	Btuh
Ceiling total	1688	sqft	1989	Btuh
Floor total	175	sqft	7641	Btuh
Infiltration	172	cfm	6948	Btuh
Duct loss			0	Btuh
Subtotal			**24944**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**24944**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1608 sqft)

Load component			Load	
Window total	113	sqft	6267	Btuh
Wall total	1206	sqft	2516	Btuh
Door total	60	sqft	588	Btuh
Ceiling total	1688	sqft	2795	Btuh
Floor total			0	Btuh
Infiltration	90	cfm	1676	Btuh
Internal gain			1380	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**15223**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			3291	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			**4491**	**Btuh**
TOTAL HEAT GAIN			**19713**	**Btuh**



For Florida residences only

EnergyGauge® System Sizing
PREPARED BY:
DATE: 5-16-06

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

The Jane Model | Project Title: 605112CornerstoneDevelopmentTheJaneModel | Class 3 Rating

Lake City, FL | Registration No. 0 | Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F | 5/16/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	NE	2.7	32.2	87 Btuh
4	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
5	2, Clear, Metal, 0.87	SW	15.0	32.2	483 Btuh
	Window Total		113(sqft)		3628 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1046	3.3	3436 Btuh
2	Frame - Wood - Ext(0.09)	13.0	160	3.3	525 Btuh
	Wall Total		1206		3962 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1688	1.2	1989 Btuh
	Ceiling Total		1688		1989Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	175.0 ft(p)	43.7	7641 Btuh
	Floor Total		175		7641 Btuh
			Zone Envelope Subtotal:		17996 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	12864	171.5	6948 Btuh
Ductload	Unsealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**24944 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	24944 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	24944 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

The Jane Model	Project Title: 605112CornerstoneDevelopmentTheJaneModel	Class 3 Rating
		Registration No. 0
Lake City, FL		Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

The Jane Model | Project Title: 605112CornerstoneDevelopmentTheJaneModel | Class 3 Rating
Lake City, FL | Registration No. 0 | Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F | 5/16/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	NE	2.7	32.2	87 Btuh
4	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
5	2, Clear, Metal, 0.87	SW	15.0	32.2	483 Btuh
	Window Total		113(sqft)		3628 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1046	3.3	3436 Btuh
2	Frame - Wood - Ext(0.09)	13.0	160	3.3	525 Btuh
	Wall Total		1206		3962 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1688	1.2	1989 Btuh
	Ceiling Total		1688		1989Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	175.0 ft(p)	43.7	7641 Btuh
	Floor Total		175		7641 Btuh
			Zone Envelope Subtotal:		17996 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	12864	171.5	6948 Btuh
Ductload	Unsealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**24944 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	24944 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	24944 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

The Jane Model	Project Title: 605112CornerstoneDevelopmentTheJaneModel	Class 3 Rating
		Registration No. 0
Lake City, FL		Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

The Jane Model | Project Title: 605112CornerstoneDevelopmentTheJaneModel | Class 3 Rating
Lake City, FL | Registration No. 0 | Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 5/16/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

	Type*		Overhang		Window Area(sqft)			HTM		Load
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
3	2, Clear, 0.87, None,N,N	NE	1.5ft.	1.16	2.7	0.0	2.7	29	60	162 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	30.0	12.1	17.9	29	63	1468 Btuh
5	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	15.0	6.1	8.9	29	63	734 Btuh
	Window Total				113 (sqft)					6267 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1046.3	2.1	2182 Btuh
2	Frame - Wood - Ext	13.0/0.09	160.0	2.1	334 Btuh
	Wall Total		1206 (sqft)		2516 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
3	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1688.0	1.7	2795 Btuh
	Ceiling Total		1688 (sqft)		2795 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	175 (ft(p))	0.0	0 Btuh
	Floor Total		175.0 (sqft)		0 Btuh

	Zone Envelope Subtotal:	12167 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.42	12864	90.0	1676 Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load
	6	X 230 +	0	1380 Btuh

Duct load	Unsealed, R6.0, Supply(Attic), Return(Attic) DGM = 0.00	0.0 Btuh
	Sensible Zone Load	**15223 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

The Jane Model
Lake City, FL

Project Title:
605112CornerstoneDevelopmentTheJaneModel

Class 3 Rating
Registration No. 0
Climate: North

5/16/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**15223 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**15223 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**15223 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	3291 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**4491 Btuh**
	TOTAL GAIN	**19713 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

The Jane Model | Project Title: 605112CornerstoneDevelopmentTheJaneModel | Class 3 Rating

Lake City, FL | Registration No. 0 | Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 5/16/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
3	2, Clear, 0.87, None,N,N	NE	1.5ft.	1.16	2.7	0.0	2.7	29	60	162 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	30.0	12.1	17.9	29	63	1468 Btuh
5	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	15.0	6.1	8.9	29	63	734 Btuh
	Window Total				113 (sqft)					6267 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1046.3	2.1	2182 Btuh
2	Frame - Wood - Ext	13.0/0.09	160.0	2.1	334 Btuh
	Wall Total		1206 (sqft)		2516 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
3	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1688.0	1.7	2795 Btuh
	Ceiling Total		1688 (sqft)		2795 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	175 (ft(p))	0.0	0 Btuh
	Floor Total		175.0 (sqft)		0 Btuh

				Zone Envelope Subtotal:	12167 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.42	12864	90.0	1676 Btuh
Internal gain		Occupants 6	Btuh/occupant X 230 +	Appliance 0	Load 1380 Btuh
Duct load	Unsealed, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
				Sensible Zone Load	**15223 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

The Jane Model
Lake City, FL

Project Title:
605112CornerstoneDevelopmentTheJaneModel

Class 3 Rating
Registration No. 0
Climate: North

5/16/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**15223**	**Btuh**
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	**15223**	**Btuh**
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
	Total sensible gain	**15223**	**Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	3291	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	**4491**	**Btuh**
	TOTAL GAIN	**19713**	**Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

The Jane Model

Lake City, FL

Project Title:
605112CornerstoneDevelopmentTheJaneModel

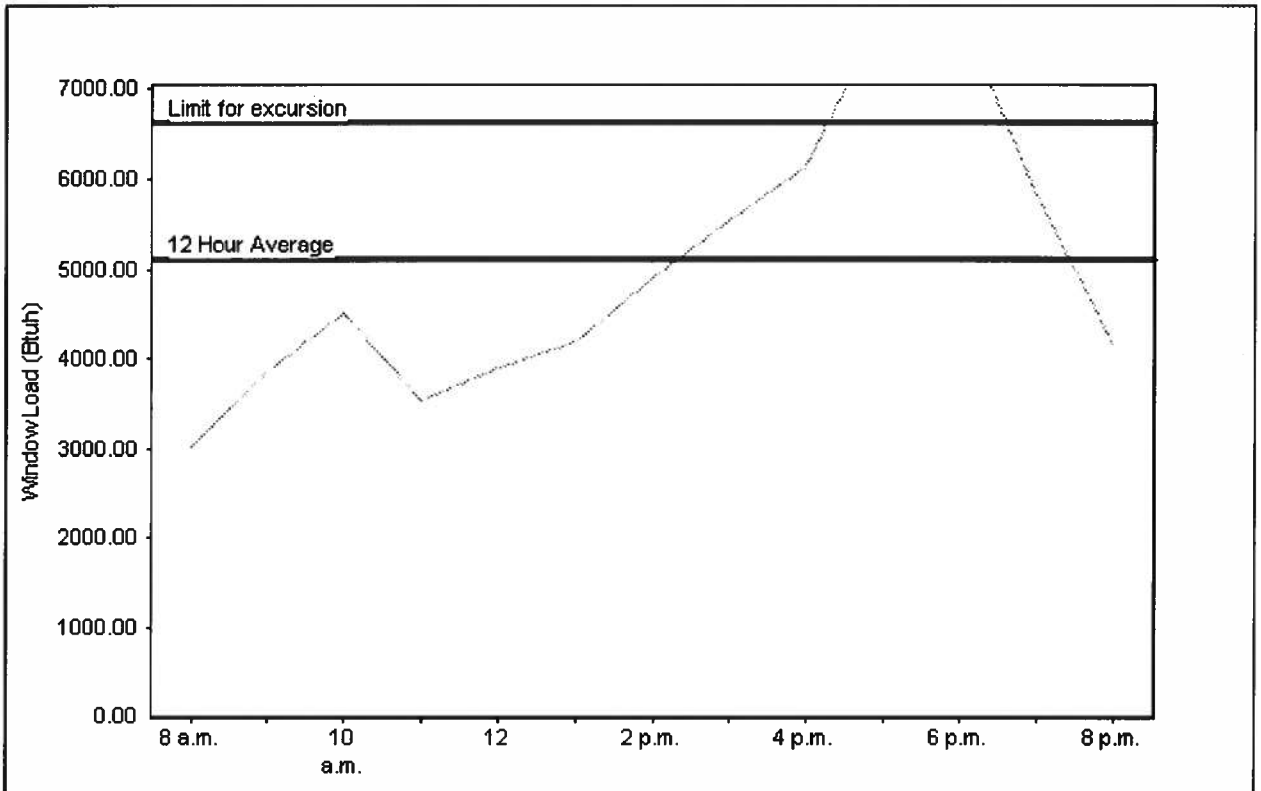
Class 3 Rating
Registration No. 0
Climate: North

5/16/2006

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	5093 Btuh
Summer setpoint	75 F	Peak window load for July	7942 Btuh
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	6620 Btuh
Latitude	29 North	Window excursion (July)	1322 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY: Ben Lynch
DATE: 5-16-06

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS	TLC	EXT DOORS	FL-5507
A. SWINGING			
B. SLIDING	TLC	SLIDERS	FL-5483
C. SECTIONAL/ROLL UP		GARAGE DOOR	FL-4606
D. OTHER			
2. WINDOWS	AWT	VYNAL WINDOWS	FL-1782
A. SINGLE/DOUBLE HUNG			
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	✓ ELK	SHINGLES	FL-250
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE

6-16-06
DATE

R-1305 01-04

Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

LABORATORIES

P.O. Box 1625 · Lake City, FL 32056 Tel. (386) 755-3633 · Fax (386) 752-5456
4784 Rosselle Street · Jacksonville, FL 32254 Tel. (904) 381-8901 · Fax (904) 381-8902
2230 Greensboro Highway · Quincy, FL 32351 Tel (850) 442-3495 · Fax (850) 442-4008

September 1, 2006

Cornerstone Developers, LLC
180 NW Amenity Court
Lake City, Florida 32055

Attention: Chris Cox

#24681

Reference: Proposed Residence
Emerald Cove, Phase I, Lot 65
Columbia County, Florida
Cal-Tech Project No. 06-521

Dear Mr. Cox,

Cal-Tech Testing, Inc. has completed an investigation and evaluation of lot 65 of Emerald Cove, Phase I in Columbia County, Florida. The purposes of our work were to evaluate the potential for flooding of a home to be constructed at the site and to provide recommendations for selecting a finished floor elevation.

Based upon the U. S. Coast and Geodetic Survey marker "BP19" located near the intersection of U. S. 90 and Brown Road, the elevation of the roadway centerline and the proposed finished floor elevation are 130.83 feet and 128.79 feet, respectively. Thus the finished floor elevation is to be approximately 2.0 feet below the centerline elevation of the adjacent roadway.

Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon the FEMA flood map for Columbia County, the drainage basin in which the proposed home site is located is not a flood area; therefore, flooding of the home should not be expected if the floor is constructed at the proposed elevation. If for some reason however flooding did occur within this drainage basin, flooding to an elevation of 129 feet would produce flood depths on the order of 40 feet within portions of the basin. Flooding to this depth is highly unlikely.

It should be noted a relatively large, topographically isolated flood area is located approximately one-half mile south southeast of the building site. The flood elevation for this area has not been determined by FEMA; however, based upon the area delineated by the flood map, this flood elevation is estimated to be about 112.0 feet. The proposed

finished floor elevation is roughly 17 feet above this flood elevation; therefore, flooding should not be expected.

Elevating the floor of the residence to 12 inches above the adjacent roadway should not be required; however, we recommend the finished floor be a minimum of 12 inches above the finished surface grade at the perimeter of the residence.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / CEO

John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer

5/1/06
52612

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 33-3S-16-02438-165 **Building permit No.** 000024681

Use Classification SFD,UTILITY **Fire:** 61.38

Permit Holder BRYAN ZECHER **Waste:** 184.25

Owner of Building CORNERSTONE DEVELOPERS **Total:** 245.63

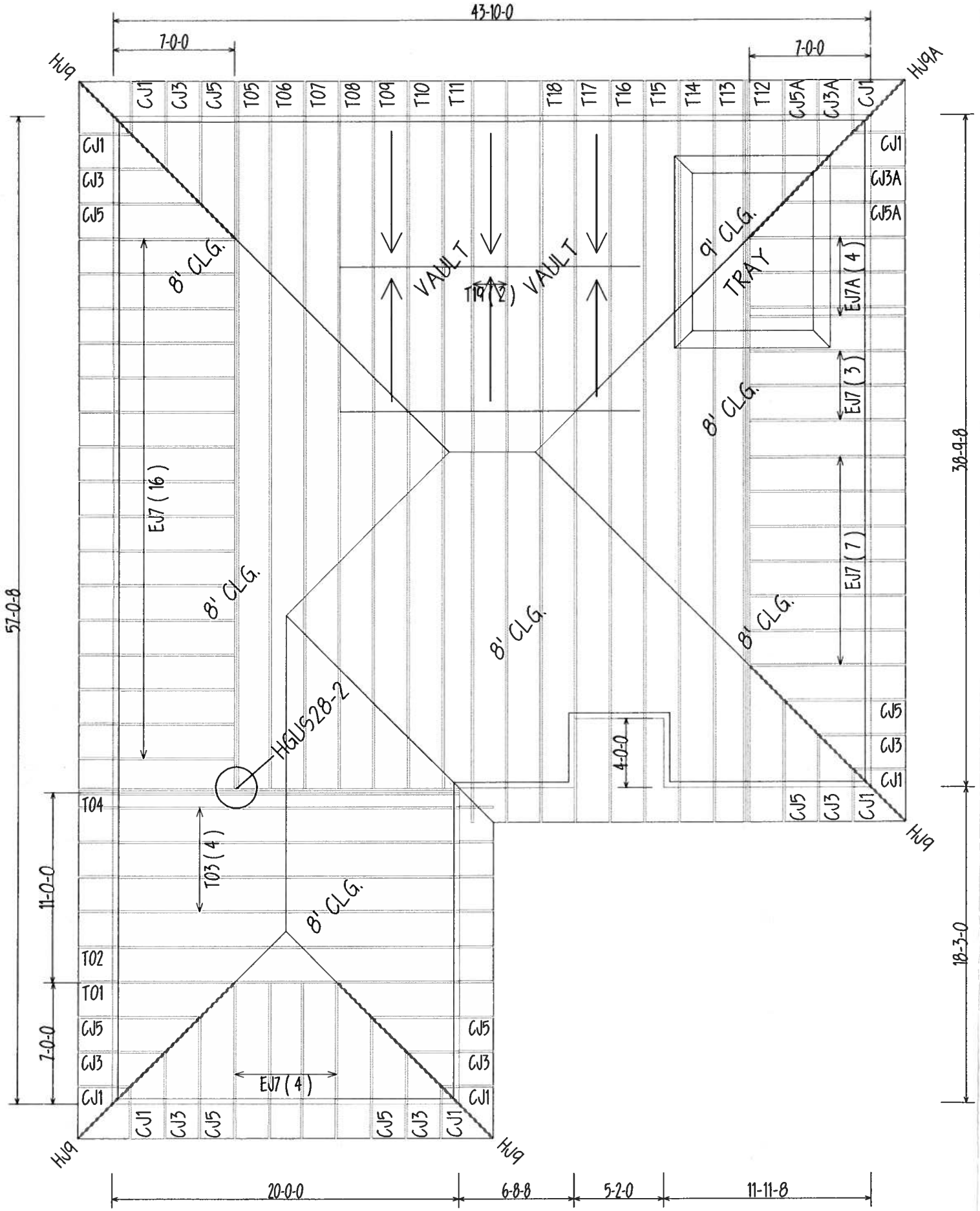
Location: 196 SW FIELDSTONE CRT, LAKE CITY, FL 32055

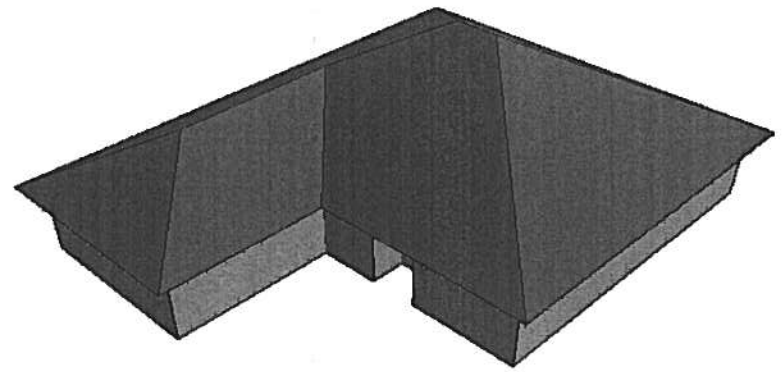
Date: 11/17/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)





BEARING HEIGHT SCHEDULE

	8'-0"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date :_____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: CORNERSTONE			
LEGAL ADDRESS: LOT 65 EMERALD COVE			
MODEL: THE JANE		REVISION:	
		SCALE: NTS	
DATE: 05/15/06	DRAWN BY: BL	JOB #: L164073	