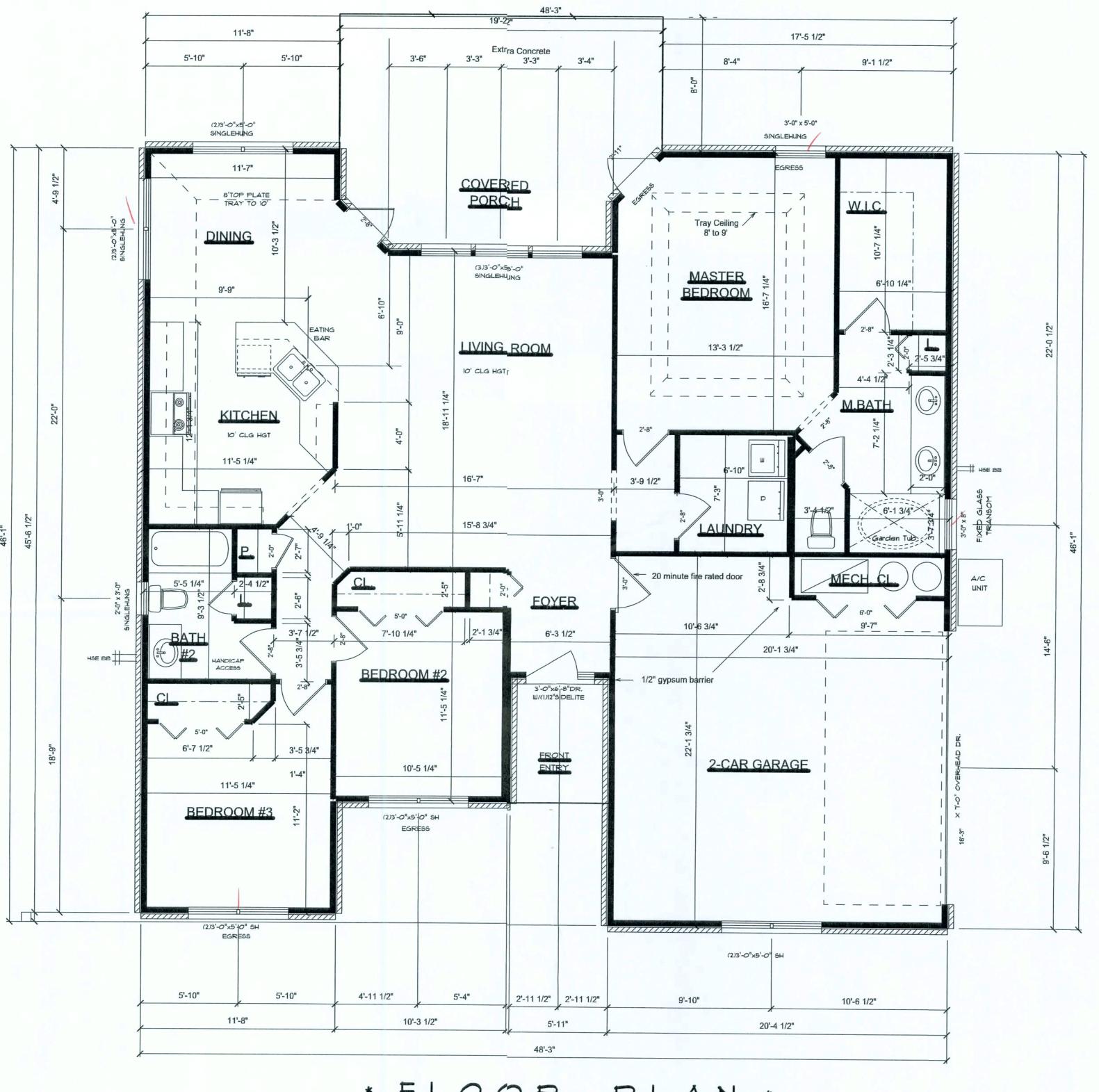
REVISIONS
03/08/06

SOFTPIAN ARCHITECTURAL DESIGN SOTWARE



## \* FLOOR PLAN \*

A/C LIVING AREA = 1511 S.F.
FRONT LENTRY = 35 S.F.
COVERED PORCH = 88 S.F.
2-CAR GARAGE & MECH.CL = 455 S.F.
TOTAL AAREA UNDER ROOF = 2089 S.F.

"THE SAMUEL"

LOT \*98

EMERALD COVE

LAKE CITY FLORIDA

Chris W. Cox 252N.W. Ivy Glen Lake City, Florida 32C55 386-T52 ITII Office 386-86T-0633 Cell

PRINTED DATE:
September 07, 2406

DRAWN BY: CHECKED BY:
Chris W. Cox

Plan On Us LLC.

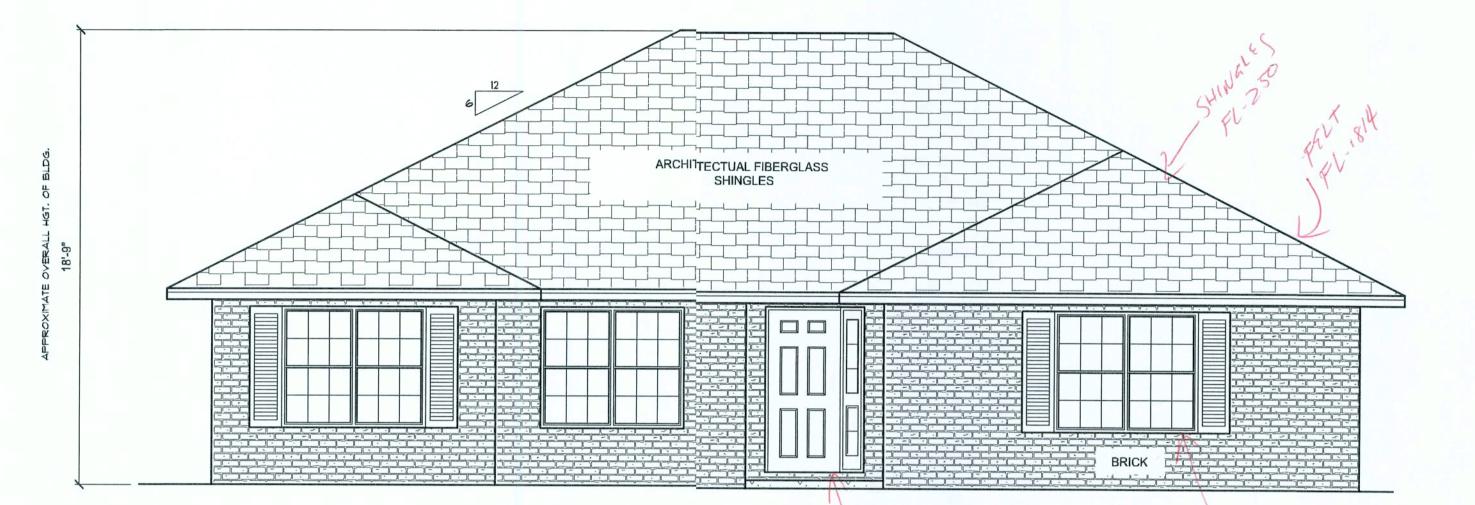
JOB NUMBER:

DRAWING NUMBER

1 OF 3 SHEETS

REVISIONS
03/08/06

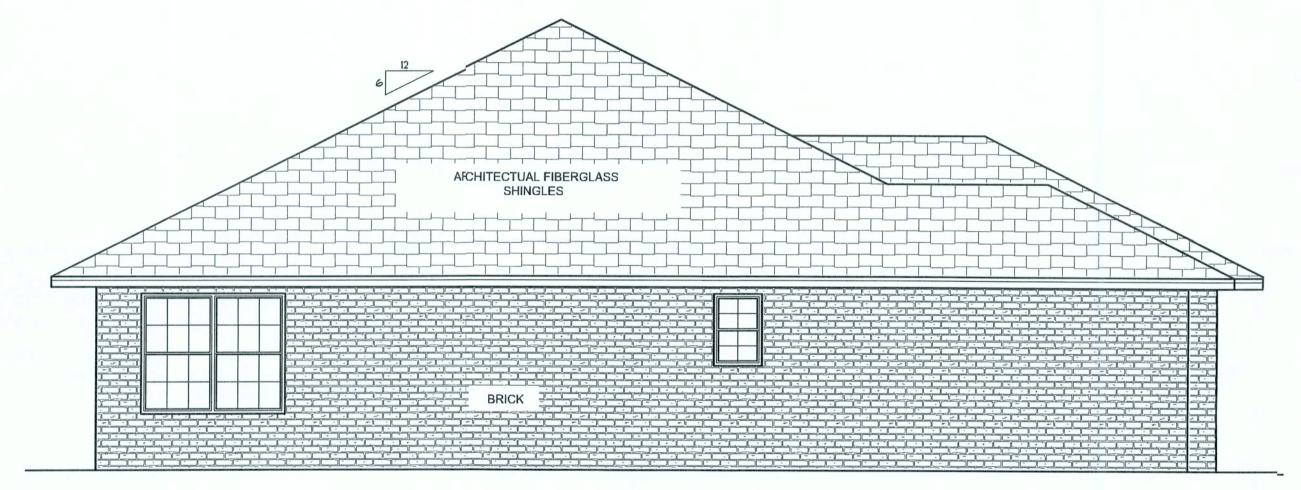
SOFTPIN ARCHITECTURAL DESIGN SOFTWARE



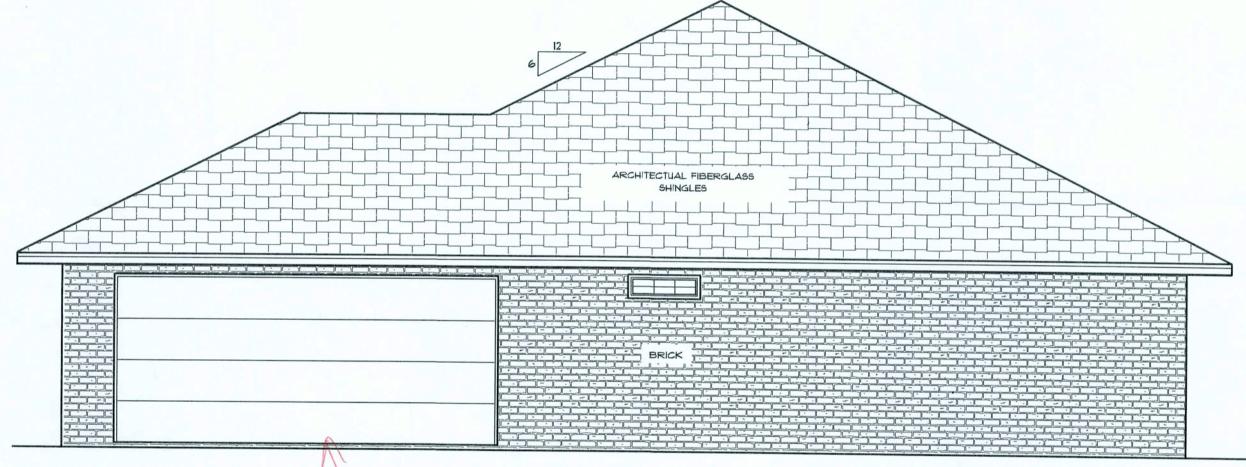
## Roof Notes:

- All overhangs to be 24" unless otherwise noted
- 2) Attic insulation to be R-19
- 3) Roof ventilation to meet the 1'/300' code
- Shingles to be 30 yr. Architectural installed to manufacturers specs.

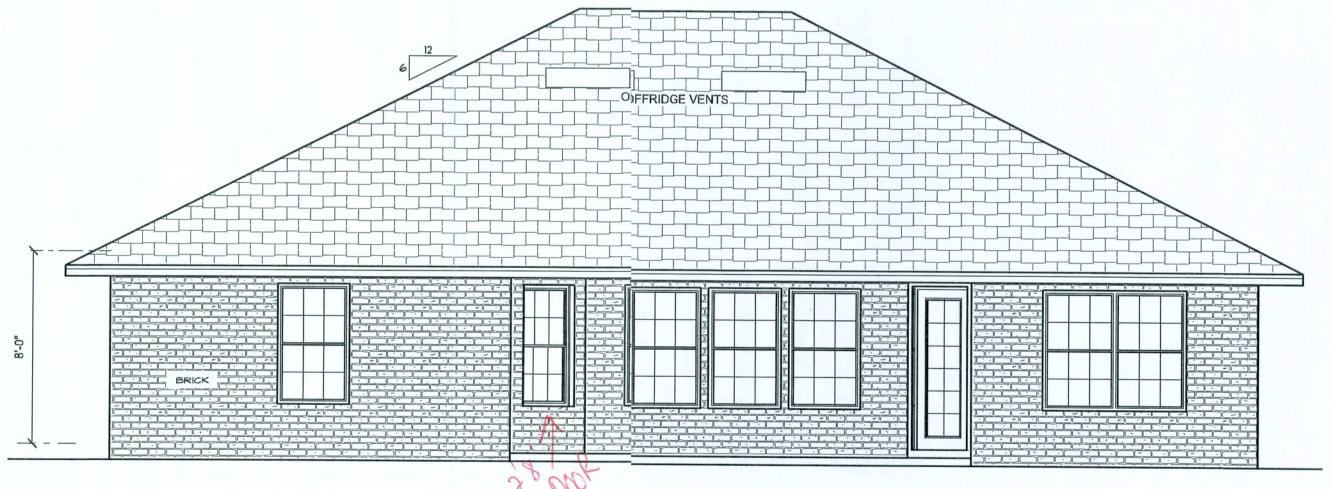
# \* FRONT ELEVATION \* SCHE: 1/4" = 1'-0"



\*LEFT SIDE ELEVATION \*



\*RIGHT SIDE ELEVATION \*



\* REAR ELEVATION \*

"THE SAMUEL"

LOT \*98

EMERALD COVE

LAKE CITY FLORIDA

Chris W. Cox 252 N.W. Ivy Glen Lake City, Florida 3205; 386-752-1711 Office 386-867-0633 Cell

PRINTED DATE:
September 07, 2003

DRAWN BY: CHECKED BY:
Chris W. Cox

Plan On Us L\_C.

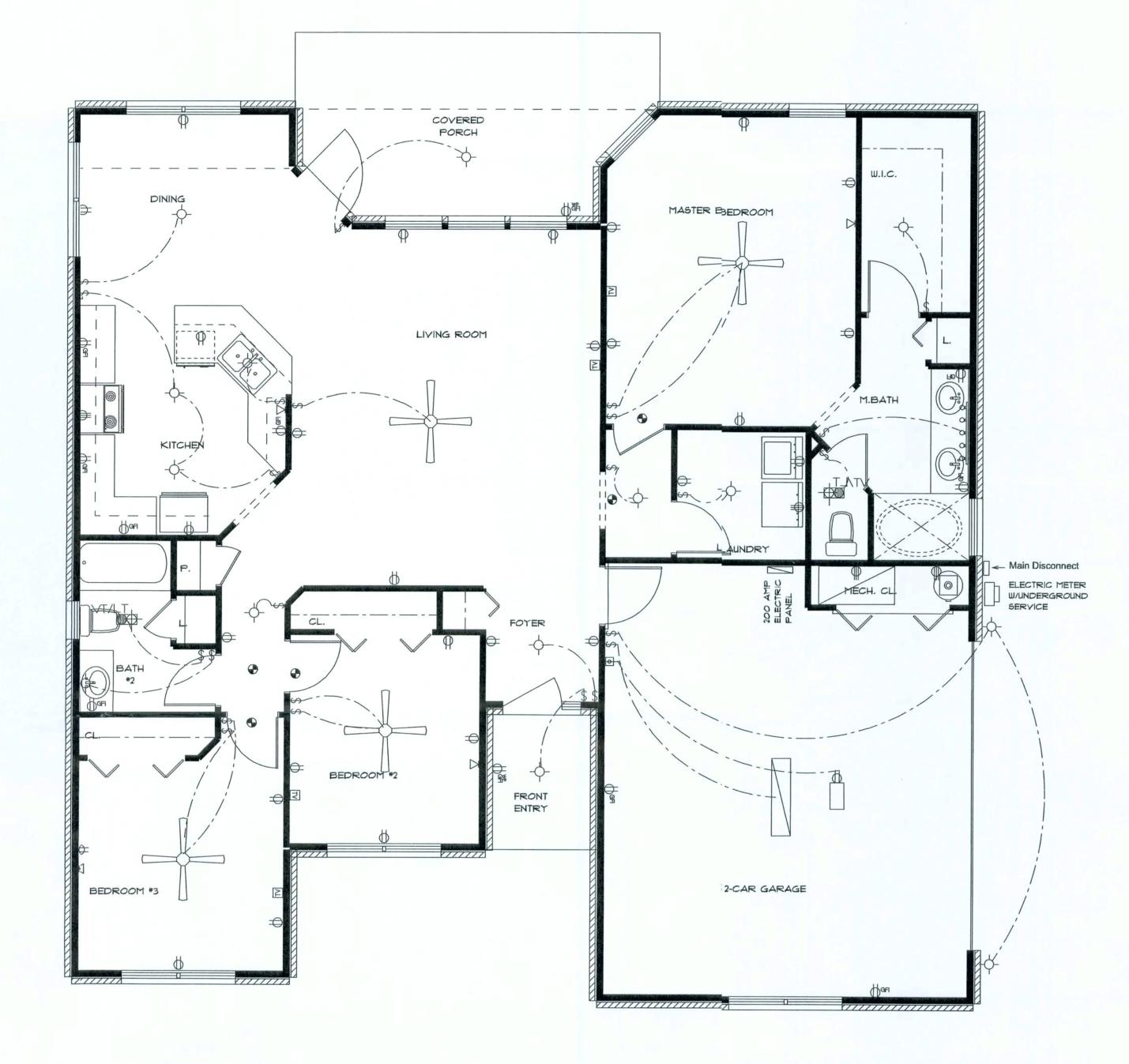
JOB NUMBER:

DRAWING NUMBER

2 OF 3 SHEETS

REVISIONS 03/08/06

SOFTPIAN ARCHITECTURAL ESTON SOFTWARE



| ELECTRICAL                 | COUNT | SYMBO             |
|----------------------------|-------|-------------------|
| ceiling fan                | 2     |                   |
| 200 amp service panel      | 1     | ELECTRIC<br>PANEL |
| Light                      | 5     | <b></b>           |
| automatic garage dr opener | 2     |                   |
| cable tv outlet            | 4     | M                 |
| ceiling fan                | 2     |                   |
| electric meter             | ı     | WAINDERGROU       |
| fluorescent It fixture     | 1     |                   |
| garage dr opener           | 1     | ф                 |
| gfi waterproof outlet      | 2     | ⊕ <sub>sk</sub>   |
| light                      | 10    | <b></b>           |
| outlet                     | 28    | ф                 |
| outlet 220v                | 3     | 4                 |
| outlet gfi                 | 9     | ⊕ <sup>on</sup>   |
| smoke detector             | 5     | 9                 |
| switch                     | 22    | \$                |
| switch 3 way               | 2     | \$.               |
| telephone                  | 4     |                   |
| vent light combo           | 2     | VIV.T             |

## Electrical Plan Notes

- E-1 Wire all appliances, HVAC units and other equipment per manufactors specifications.
  E-2 Consult the owner for the number of seperate telephone lines to be installed.
  E-3 All installations shall be per national electric code.
  E-4 All smoke detectors shall be 120V w/battery backup of the photoelectric type, and shall be interlocked together Install inside and near all bedrooms.
- E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners direction and in accordance with applicable sections or national electrical code latest edition.
- E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.
  E-7 Entry of service underground or overhead is to be determined by the power company.
  E-8 All bedroom recepticals are to be AFCI (Arc Fault Circuit Interrupt).
  E-9 All outside recepticals are to be weatherproofed GFIs.

\* ELECTRICAL PLAN \*

"THE SAMUEL" LOT 198 EMERALD COVE

> LAKE CITY FLORIDA

252 N.W. Ivy Glen Lake City, Floida 32055 386-752-1711 Office 386-867-0633 Cell

PRINTED DATE: September 07, 2006 DRAWN BY:

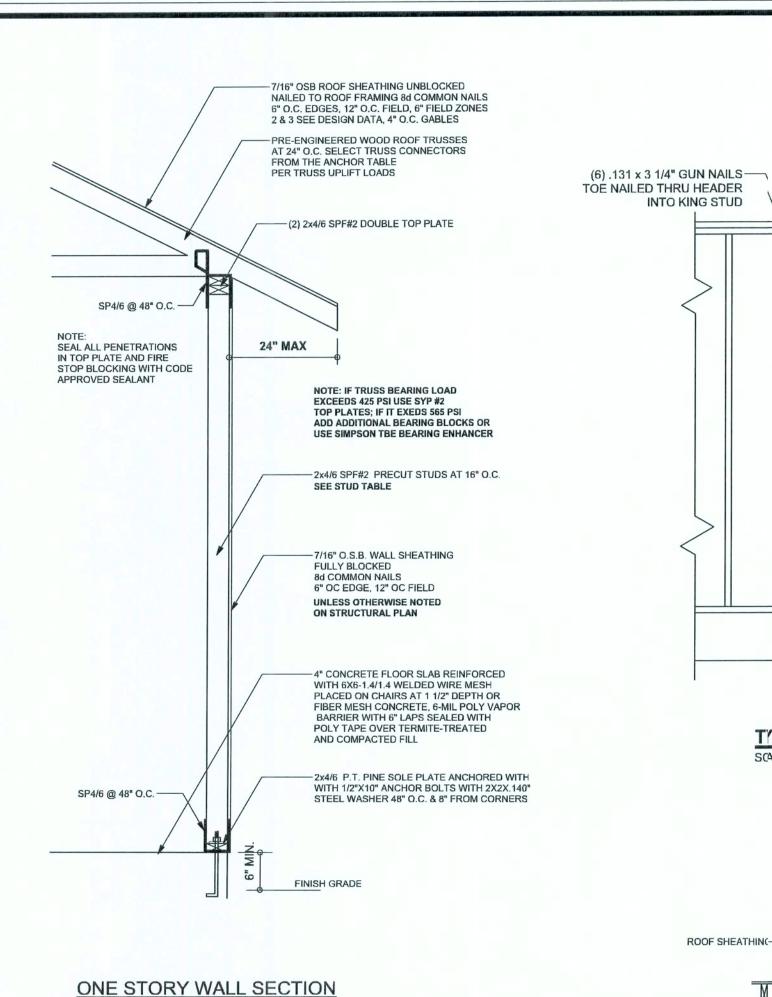
Plan On Us LLC.

FINIALS DATE:

JOB NUMBER:

DRAWING NUMBER

3 OF 3 SHEETS



SCALE: 3/4" = 1'-0"

(2) 2X10 SYP #2 MIN.

(2)-2x4 SPF #2 JACKS STUDS -

BEAM MID-WALL CONNECTION DETAIL

LSTA24

NAIL THRU 2x4 INTO

BEAM MAY BE ATTACHED IN

**BEAM CORNER CONNECTION. DETAIL** 

BEAM W/4-16d

SIMPSON HUS412 MIN. SEE STRUCTURAL PLAN

LSTA18

W/ 2-10d NAILS AT 16" O.C. MIN.

W/ DOUBLE TOP PLATE w/ SP4 @ 48" OC

ALL OPENINGS (U.N.O.) (1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.) TYPICAL HEADER STRAPING DETAIL ROOF SHEATHIN(-\_\_8d @ 6" O.C. ROOF SHEATHING -ROOF TRUSS -

-LSTA18 (U.N.O. --

CRIPPLES IF REQUIRED

(4) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU SILL-

INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

-SP4 OR (2) H2.5A OR (2) SSP-

(SEE STRUCTURAL PLAN)

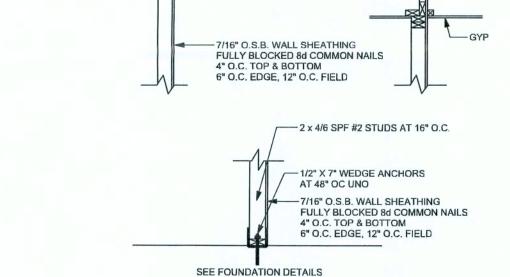
-NAIL SHEATHING TO HEADER AND TOP

—(6) .131 x 3 1/4" GUN NAILS

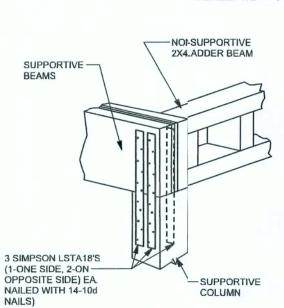
INTO KING STUD

TOE NAILED THRU HEADER

PLATE WITH 8d AT 4" O.C. FOR UPLIFT



INTERIOR SHEAR WALL DETAIL



(10) .131"x3.25" GUN-NAILS

-2x4 SPF #2 KING STUD

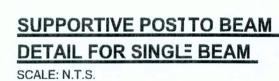
2-10d NAILS AT 16" O.C. MIN.

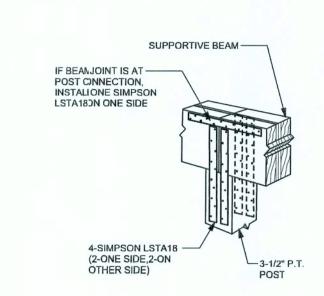
AND SPH4 @ BOTTOM PLATE

NAILED TO JACKS W/

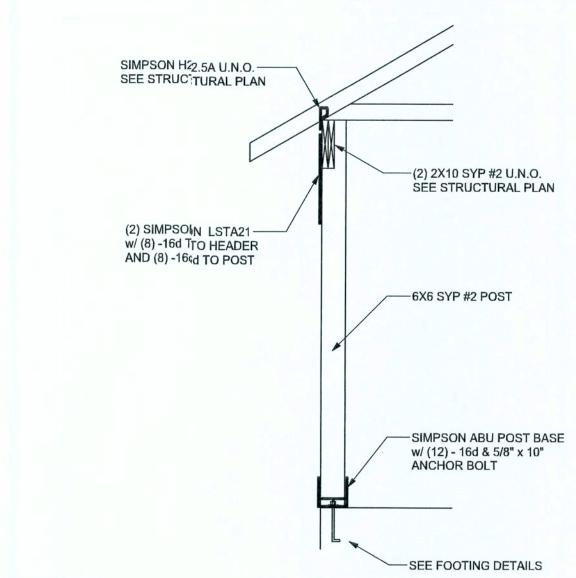
SEE STRUCTURAL PLAN

INTO KING STUD





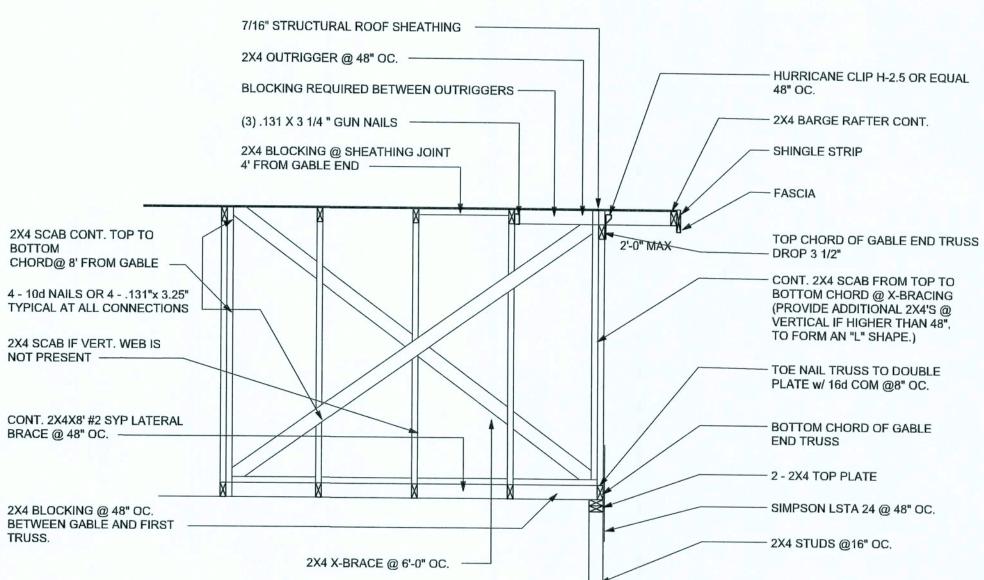
SUPPORTIVE CENTER POST TO BEAM DETAIL



SEE STRUCTURAL PLAN STRAP STUDS --2 x 4/6 STUDS AT 16" O.C. SP4/6 TOP & BOTTOM OR (2) H2.5A TOP & BOTTOM @ 32" O.C. --- 1/2" X 7" WEDGE ANCHORS AT 48" OC U.N.O. SEE FOUNDATION DETAILS

TRUSS.

TYPICAL PORCH POST DETAIL



ALL MEMBERS SHALL BE SYP

**GRADE & SPECIES TABLE** TYPICAL GABLE END (X-BRACING)

|      |              | Fb (psi) | E (10 <sup>i</sup> psi) |
|------|--------------|----------|-------------------------|
| 2x8  | SYP #2       | 1200     | 13                      |
| 2x10 | SYP #2       | 1050     | 13                      |
| 2x12 | SYP #2       | 975      | 13                      |
| GLB  | 24F-V3 SP    | 2400     | 13                      |
| LSL  | TIMBERSTRAND | 1700     | 17                      |
| LVL  | MICROLAM     | 2900     | 2.)                     |
| PSL  | PARALAM      | 2900     | 2.)                     |

INDLOAD ENGINEER: Mar. Disosway,

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these instruments of service. his document

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permission and consent of Mak Disosway.

CERTIFICATION: I hereby cetify that I have

examined this plan, and that the applicable portions of the plan, relating towind enginee

code residential 2004, to the best of my

LIMITATION: This design is valid for one

building, at specified location.

comply with section R301.2.1, florida building

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Do not proceed without clarification.

32056, 386-754-5419

IMENSIONS:

REVISIONS

SOFTPLAN

INTERIOR BEARING WALL SCALE: 1/2" = 1'-0"

**ANCHOR TABLE** OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| < 420   | < 245  | H5A                    | 3-8d           | 3-8d          |                                      |
|---------|--------|------------------------|----------------|---------------|--------------------------------------|
| < 455   | < 265  | H5                     | 4-8d           | 4-8d          |                                      |
| < 360   | < 235  | H4                     | 4-8d           | 4-8d          |                                      |
| < 455   | < 320  | H3                     | 4-8d           | 4-8d          |                                      |
| < 415   | < 365  | H2.5                   | 5-8d           | 5-8d          |                                      |
| < 600   | < 535  | H2.5A                  | 5-8d           | 5-8d          |                                      |
| < 950   | < 820  | H6                     | 8-8d           | 8-8d          |                                      |
| < 745   | < 565  | H8                     | 5-10d, 1 1/2"  | 5-10d, 1 1/2" |                                      |
| < 1465  | < 1050 | H14-1                  | 13-8d          | 12-8d, 1 1/2" |                                      |
| < 1465  | < 1050 | H14-2                  | 15-8d          | 12-8d, 1 1/2" |                                      |
| < 990   | < 850  | H10-1                  | 8-8d, 1 1/2"   | 8-8d, 1 1/2"  |                                      |
| < 760   | < 655  | H10-2                  | 6-10d          | 6-10d         |                                      |
| < 1470  | < 1265 | H16-1                  | 10-10d, 1 1/2" | 2-10d, 1 1/2" |                                      |
| < 1470  | < 1265 | H16-2                  | 10-10d, 1 1/2" | 2-10d, 1 1/2" |                                      |
| < 1000  | < 860  | MTS24C                 | 7-10d 1 1/2"   | 7-10d 1 1/2"  | :                                    |
| < 1450  | < 1245 | HTS24                  | 12-10d 1 1/2"  | 12-10d 1 1/2" |                                      |
| < 2900  | < 2490 | 2 - HTS24              |                |               |                                      |
| < 2050  | < 1785 | LGT2                   | 14 -16d        | 14 -16d       |                                      |
|         |        | HEAVY GIRDER TIEDOWNS* |                | 700 900       | TO FOUNDATION                        |
| < 3965  | < 3330 | MGT                    |                | 22 -10d       | 1-5/8" THREADED ROD<br>12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2                  |                | 16 -10d       | 2-5/8" THREADED ROD<br>12" EMBEDMENT |
| < 10530 | < 9035 | HGT-3                  |                | 16 -10d       | 2-5/8" THREADED ROD<br>12" EMBEDMENT |
| < 9250  | < 9250 | HGT-4                  |                | 16 -10d       | 2-5/8" THREADED ROD<br>12" EMBEDMENT |
|         |        | STUD STRAP CONNECTOR*  |                |               | TO STUDS                             |
| < 435   | < 435  | SSP DOUBLE TOP PLATE   | 3 -10d         |               | 4 -10d                               |
| < 455   | < 420  | SSP SINGLE SILL PLATE  | 1 -10d         |               | 4 -10d                               |
| < 825   | < 825  | DSP DOUBLE TOP PLATE   | 6 -10d         |               | 8 -10d                               |
| < 825   | < 600  | DSP SINGLE SILL PLATE  | 2 -10d         |               | 8 -10d                               |
| < 885   | < 760  | SP4                    |                |               | 6-10d, 1 1/2"                        |
| < 1240  | < 1065 | SPH4                   |                |               | 10-10d, 1 1/2"                       |
| < 885   | < 760  | SP6                    |                | 6-10d, 1 1/2" |                                      |
| < 1240  | < 1065 | SPH6                   |                |               | 10-10d, 1 1/2"                       |
| < 1235  | < 1165 | LSTA18                 | 14-10d         |               |                                      |
| < 1235  | < 1235 | LSTA21                 | 16-10d         |               |                                      |
| < 1030  | < 1030 | CS20                   | 18-8d          |               |                                      |
| < 1705  | < 1705 | CS16                   | 28-8d          |               |                                      |
|         |        | STUD ANCHORS*          | TO STUDS       |               | TO FOUNDATION                        |
| < 1350  | < 1305 | LTT19                  | 8-16d          |               | 1/2" AB                              |
| < 2310  | < 2310 | LTTI31                 | 18-10d, 1 1/2" |               | 1/2" AB                              |
| < 2775  | < 2570 | HD2A                   | 2-5/8" BOLTS   |               | 5/8" AB                              |
| < 4175  | < 3695 | HTT16                  | 18 - 16d       |               | 5/8" AB                              |
| < 1400  | < 1400 | PAHD42                 | 16-16d         |               |                                      |
| < 3335  | < 3335 | HPAHD22                | 16-16d         |               |                                      |
| < 2200  | < 2200 | ABU44                  | 12-16d         |               | 1/2" AB                              |
| < 2300  | < 2300 | ABU66                  | 12-16d         |               | 1/2" AB                              |
| < 2320  | < 2320 | ABU88                  | 18 - 16d       |               | 2-5/8" AB                            |

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR\* TO PLATES TO RAFTER/TRUSS TO STUDS

## **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'G = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" X 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH A WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 \* DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU. WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

| THE BUILDER<br>SPECIFICALLY       | AND OWNER ARE R<br>NOT PART OF THE                                 | ESPONSIBLE FOR THE FOLLOWING, WHICH ARE WIND LOAD ENGINEER'S SCOPE OF WORK.                           |
|-----------------------------------|--|---|
| CONFIRM SITE CO<br>BACKFILL HEIGH | NDITIONS, FOUNDATION<br>, WIND SPEED AND DEBI                      | I BEARING CAPACITY, GRADE AND<br>RIS ZONE, AND FLOOD ZONE.  |
| PROVIDE MATER<br>REQUIREMENTS     | ALS AND CONSTRUCTIO<br>FOR THE STATED WIND \                       | N TECHNIQUES, WHICH COMPLY WITH FBCR 2004<br>/ELOCITY AND DESIGN PRESSURES.                           |
| BELIEVE THE PLA                   | NUOUS LOAD PATH FRO<br>N OMITS A CONTINUOUS<br>NGINEER IMMEDIATELY | M TRUSSES TO FOUNDATION. IF YOU<br>LOAD PATH CONNECTION, CALL   |
| DESIGN, PLACEM                    | ENT PLANS, TEMPORARY<br>CONNECTIONS, AND UP                        | ALED ENGINEERING INCLUDES TRUSS<br>AND PERMANENT BRACING DETAILS,<br>PLIFT AND REACTION LOADS FOR ALL |

## ROOF SYSTEM DESIGN

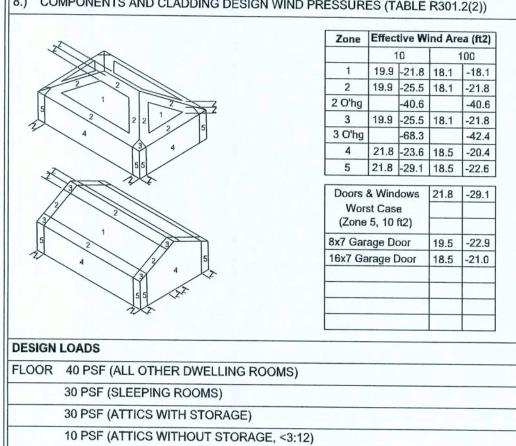
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R302.1.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## **EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

| (1) 2x4 @ 16" OC | TO 11'-9" STUD HEIGHT  |  |
|------------------|------------------------|--|
| (1) 2x4 @ 12" OC | TO 13'-0" STUD HEIGHT  |  |
| (1) 2x6 @ 16" OC | TO 18'-10' STUD HEIGHT |  |
| (1) 2x6 @ 12" OC | TO 20.0' STUD HEIGHT   |  |

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.

| WIND LOADS BED EL SEIN A THU THE THE   |                 |                           |                |       |          |  |
|--|-----------------|---------------------------|----------------|-------|----------|--|
| WIND LOADS PER FLORIDA BUILDING CODE 2004 RE   |                 |                           |                |       |          |  |
| (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH I<br>MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZ<br>ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN<br>SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIG   | ZONTAL D        | IMEN                      | SION           | OR 6  | 0 FT; NC |  |
| BUILDING IS NOT IN THE HIGH VELOCITY HURRICAN  | IE ZONE         |                           |                |       |          |  |
| BUILDING IS NOT IN THE WIND-BORNE DEBRIS REG   | ION             |                           |                |       |          |  |
| 1.) BASIC WIND SPEED = 110 MPH   |                 |                           |                |       |          |  |
| 2.) WIND EXPOSURE = B  |                 |                           |                |       |          |  |
| 3.) WIND IMPORTANCE FACTOR = 1.0   |                 |                           |                |       |          |  |
| 4.) BUILDING CATEGORY = II   |                 |                           |                |       |          |  |
| 5.) ROOF ANGLE = 10-45 DEGREES   |                 |                           |                |       |          |  |
| 6.) MEAN ROOF HEIGHT = <30 FT  |                 |                           |                |       |          |  |
|  | N OOFD D        |                           |                |       |          |  |
| , TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL TOTAL CONTROL TO TAKE THE TOTAL TO |                 |                           |                |       |          |  |
| 8.) COMPONENTS AND CLADDING DESIGN WIND P  | RESSURI         | ES (T                     | ABLE           | R301. | .2(2))   |  |
|  | Zone            | Effective Wind Area (ft2) |                |       |          |  |
| ****   |                 | 1                         | 0              | 100   |          |  |
|  | 1               | 19.9                      | -21.8          | 18.1  | -18.1    |  |
| 2 2  | 2               | 19.9                      | -25.5          | 18.1  | -21.8    |  |
|  | 2 O'hg          |                           | -40.6          |       | -40.6    |  |
|  | 3               | 19.9                      | -25.5          | 18.1  | -21.8    |  |
| 2 2 2 5  | 1 0 011         |                           | -68.3          |       | -42.4    |  |
| 2 2 2 2 5 5  | 3 O'hg          |                           |                |       | -20.4    |  |
| 2 2 2 1 2 5 5  | 4               |                           | -23.6          | -     |          |  |
| 3 2 2 2 5 5  |                 |                           | -23.6<br>-29.1 | 18.5  | -22.6    |  |
| 3 3 4 5  | 4               | 21.8                      | -29.1          | -     |          |  |
| 3<br>2<br>2<br>2<br>2<br>2<br>3<br>4<br>5<br>5<br>5<br>5   | 4<br>5<br>Doors | 21.8                      | -29.1<br>lows  | 18.5  | -22.6    |  |



ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

P.E. 53915 Cornerstone Developme Zecher Bryan The Samuel Model **ADDRESS** Lot #98 Emerald Cove S/D Lake City, Floida FINALS DATE: Sept. 7, 2006

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269-4871 PRINTED DATE: September 07,2006 STRUCTURAL BY EVAN BEAMSLEY

> JOB NUMBER: 608234 DRAWING NUMBER

> > OF 2 SHEETS

