

DA 11/06/2006

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000025204

APPLICANT GLENN KEEN PHONE 867.0156

ADDRESS 1534 SW DEKLE ROAD LAKE CITY FL 32024

OWNER JOHN KEEN/A&B MANAGEMNT,LLC PHONE 386.961.8223

ADDRESS 159 SW POPPY GLEN LAKE CITY FL 32024

CONTRACTOR JASON ELIXSON PHONE 386.623.1741

LOCATION OF PROPERTY 90-W TO SR 247-S TO CALLAHAN,TL TO ROLLING MEADOWS S.D. TO MORNING GLORY,TR TO BUTTERCUP,TR TO POPPY GLN,3RD ON R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 85300.00

HEATED FLOOR AREA 1706.00 TOTAL AREA 2382.00 HEIGHT 18.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO. \_\_\_\_\_

PARCEL ID 15-4S-16-03023-530 SUBDIVISION ROLLING MEADOWS

LOT 30 BLOCK \_\_\_\_\_ PHASE \_\_\_\_\_ UNIT \_\_\_\_\_ TOTAL ACRES 0.50

000001251 \_\_\_\_\_ CBC1250331 JTH

Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor

18"X32'MITERED X-06-0321 BLK JTH N

Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: NOC ON FILE. PLAT REQUIRES 1ST. FLOOR ELEVATION OF 107.5'. ELEVATION LETTER REQUIRED.

Check # or Cash 1191

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power \_\_\_\_\_ Foundation \_\_\_\_\_ Monolithic \_\_\_\_\_  
date/app. by date/app. by date/app. by

Under slab rough-in plumbing \_\_\_\_\_ Slab \_\_\_\_\_ Sheathing/Nailing \_\_\_\_\_  
date/app. by date/app. by date/app. by

Framing \_\_\_\_\_ Rough-in plumbing above slab and below wood floor \_\_\_\_\_  
date/app. by date/app. by

Electrical rough-in \_\_\_\_\_ Heat & Air Duct \_\_\_\_\_ Peri. beam (Lintel) \_\_\_\_\_  
date/app. by date/app. by date/app. by

Permanent power \_\_\_\_\_ C.O. Final \_\_\_\_\_ Culvert \_\_\_\_\_  
date/app. by date/app. by date/app. by

M/H tie downs, blocking, electricity and plumbing \_\_\_\_\_ Pool \_\_\_\_\_  
date/app. by date/app. by

Reconnection \_\_\_\_\_ Pump pole \_\_\_\_\_ Utility Pole \_\_\_\_\_  
date/app. by date/app. by date/app. by

M/H Pole \_\_\_\_\_ Travel Trailer \_\_\_\_\_ Re-roof \_\_\_\_\_  
date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 430.00 CERTIFICATION FEE \$ 11.91 SURCHARGE FEE \$ 11.91

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ \_\_\_\_\_

FLOOD DEVELOPMENT FEE \$ \_\_\_\_\_ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 553.82

INSPECTORS OFFICE \_\_\_\_\_ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

# Columbia County Building Permit Application

For Office Use Only Application # 0610-69 Date Received 10/24 By TW Permit # 25204-1251

Application Approved by - Zoning Official DLK Date 0611.06 Plans Examiner OK JTH Date 11-6-06

Flood Zone X new plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Dev.

Comments Plat Requires 1st Floor Elevation of 107.5' Elevation Letter Required

☒ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Permit

Name Authorized Person Signing Permit Glenn Keen / Keith Framing Fax (386) 758-9175 Phone (386) 867-0156

Address 1534 SW DEKLE Rd. LAKE CITY, FL 32024

Owners Name John Keen / AEB Management Phone (386) 961-8223

911 Address 159 SW Poppy Glen Rd. LAKE CITY, FL 32024

Contractors Name JASON ELIXSON Construction LLC Phone (386) 623-1741

Address 4853 West SR 238 LAKE BUTLER, FL 32054

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Jason Elixson / Marty R. Eskridge 14952 Main St Alachua, FL 32015

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 15-45-16-03023-530 Estimated Cost of Construction 88,600.00

Subdivision Name Rolling MEADOWS Lot 30 Block  Unit  Phase

Driving Directions GO 90 WEST to Hwy 247, turn left go south 2 miles to Poppy Glen Rd. Turn left go 1 mile to Rolling Meadows Subdivision, Turn right on Marking Poppy Glen go 2 blocks to Poppy Glen Rd. Turn left go 2 blocks to Poppy Glen Rd. Turn left go 2 blocks to Poppy Glen Rd. Turn left go 2 blocks to Poppy Glen Rd.

Type of Construction Residential / new home Number of Existing Dwellings on Property 2nd lot on right

Total Acreage 1/2 Lot Size  Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 50' Side 20' Side 20' Rear 90'

Total Building Height 18 Number of Stories 1 Heated Floor Area 1706 Roof Pitch 6/12

TOTAL 2382

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

L.K. Keith Framing  
Owner Builder or Authorized Person by Notarized Letter

STATE OF FLORIDA  
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me  
this 17th day of Oct 2006.

Personally known ☒ or Produced Identification



BARRY COLEMAN  
MY COMMISSION # 00000000  
EXPIRES: September 24, 2007  
Bonded Through Notary Seal

[Signature]  
Contractor Signature  
Contractors License Number CBC 12500331  
Competency Card Number   
NOTARY STAMP/SEAL

[Signature]  
Notary Signature (Revised Sept. 2'



## SECTION

SCALE: 1" = 100'

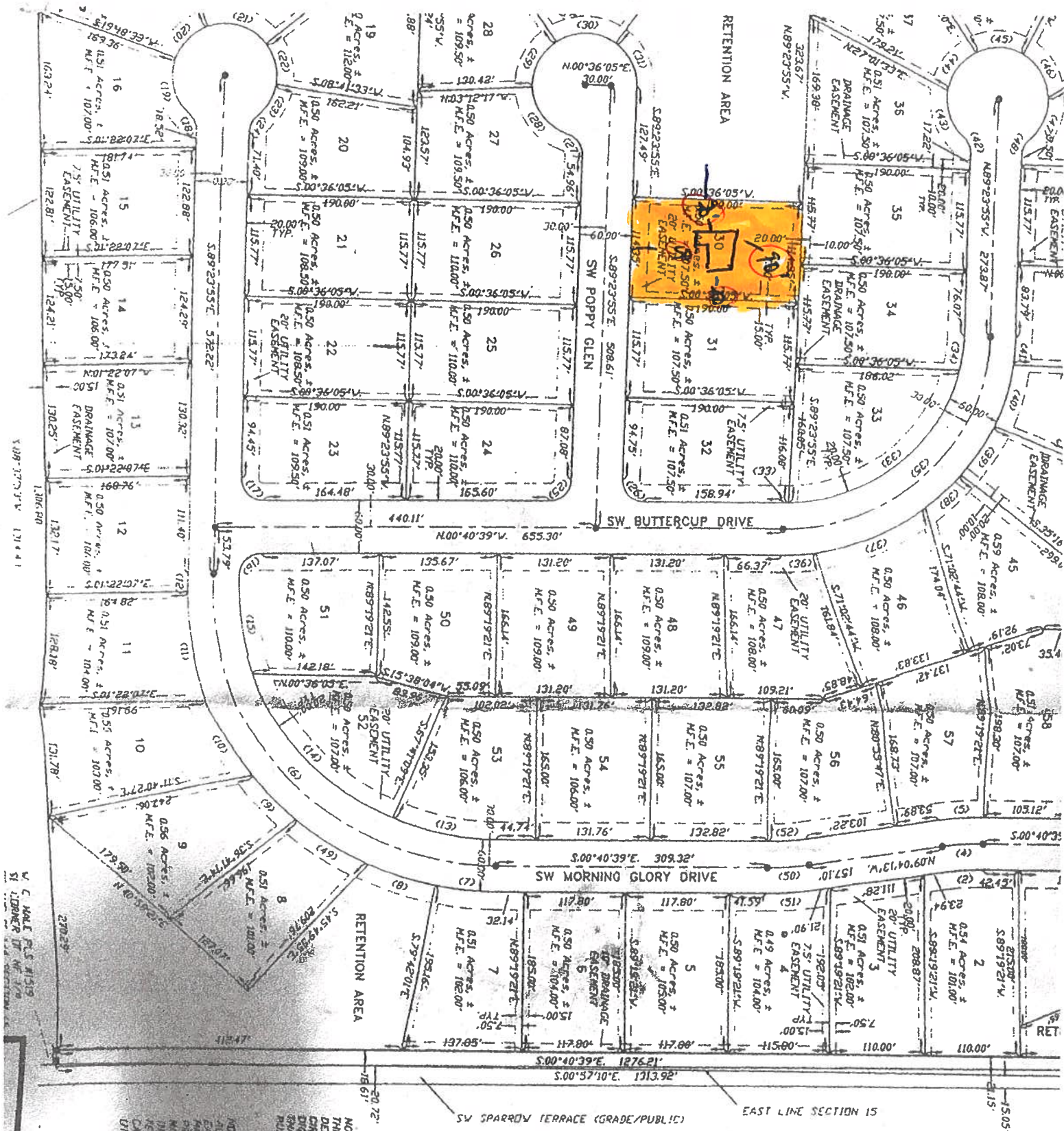
UNPLATTED LANDS  
80 ACRES

SEE SHEET 1 OF 2

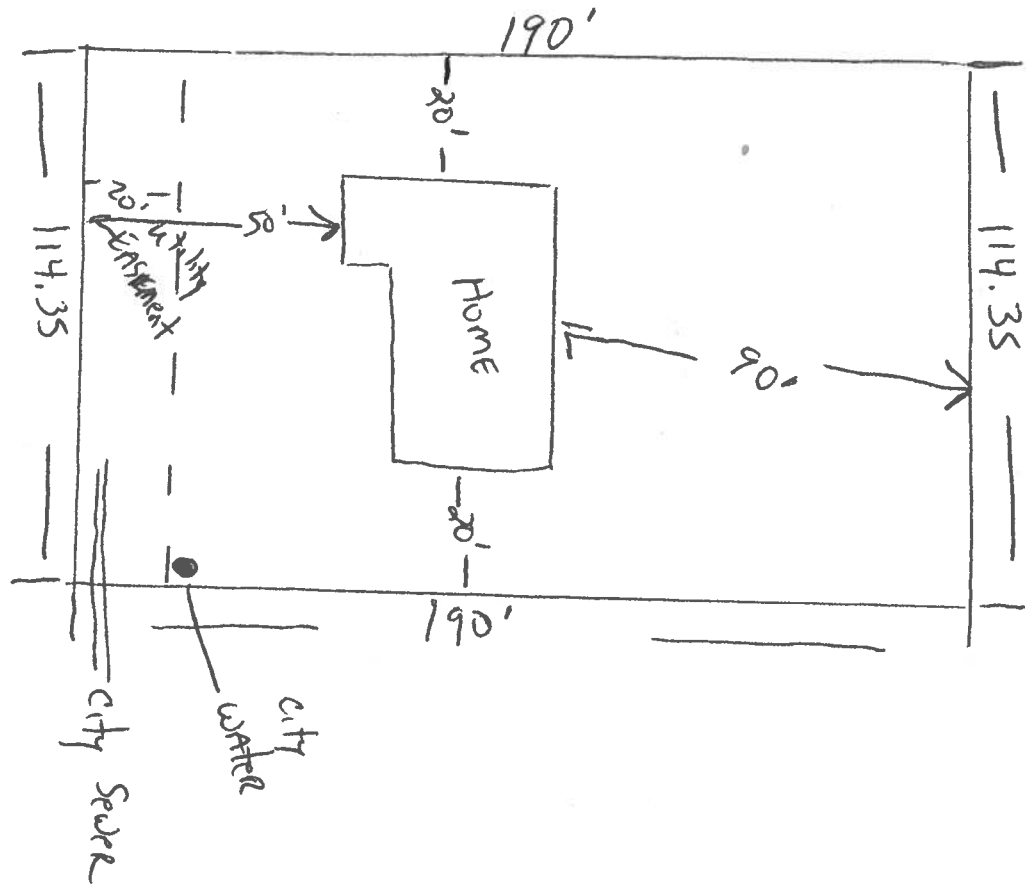
NOTICE: THIS PLAT AS RECORDED IN ITS CAPTIONED RECORD, IS A CORRECTION OF THE SUBMITTED LAND CIRCUMSTANCES BE SUPPLEMENTED IN DIGITAL FORM OF THE PLAT, THERE BEING NO RECORD OF THIS PLAT, THERE ARE NOT RECORDED ON THIS PLAT, RECORDS OF THIS COUNTY.

# ENGINEER

CURTIS KEEN  
9263 COUNTY ROAD  
LIVE OAK, FLORIDA 3  
1-386-362-4787



Retention Pond Area



Sw Poppy Glen Rd.

Sw Buttercup Drive

# **K & H Framing/Vinyl Siding, Inc.**

1534 S.W.Dekle Road  
Lake City, Florida 32024  
(386)961-8223

Direction to  
Rolling Meadows lot #30

October 24, 2006

Go Hwy 90 West to Hwy 247 South. Turn left and go 2 miles to Callahan drive. Turn left on Callahan Drive and go 1 mile to Morning Glory Drive (Rolling Meadows entrance). Turn right into subdivision on Morning Glory and go  $\frac{1}{4}$  mile to SW Buttercup Drive. Turn right on Buttercup and go 2 blocks to Poppy Glenn Rd. Turn left and go to third lot on right.

NOTICE OF COMMENCEMENT FORM  
COLUMBIA COUNTY, FLORIDA

**\*\*\* THIS DOCUMENT MUST BE RECORDED AT THE COUNTY  
CLERKS OFFICE BEFORE YOUR FIRST INSPECTION. \*\*\***

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 15-45-16-03023-530

PERMIT NUMBER \_\_\_\_\_

1. Description of property: (legal description of the property and street address or 911 address)

159 SW Poppy Glen Rd Lake City, FL 32024

2. General description of Improvement:

Residential / new Home

3. Owner Name & Address

John W. Keen / A&B Management, LLC.

Interest in Property \_\_\_\_\_

4. Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name

JASON ELIXSON CONST. LLC

Phone Number (386) 623-1741

Address 4853 WEST SR 238 LAKE BUTLER, FL 32054

6. Surety Holders Name

Phone Number \_\_\_\_\_

Address \_\_\_\_\_

Amount of Bond \_\_\_\_\_

Inst: 2006024247 Date: 10/11/2006 Time: 13:24

7. Lender Name

DC, P. DeWitt Cason, Columbia County B: 1098 P: 1951

Address \_\_\_\_\_

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name \_\_\_\_\_

Phone Number \_\_\_\_\_

Address \_\_\_\_\_

9. In addition to himself/herself the owner designates \_\_\_\_\_ of

\_\_\_\_\_ to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee \_\_\_\_\_

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified) \_\_\_\_\_

**NOTICE AS PER CHAPTER 713, Florida Statutes:**

The owner must sign the notice of commencement and no-one else may be permitted to sign in his/her stead.

John Keen  
Signature of Owner

Sworn to (or affirmed) and subscribed before  
day of 8th Oct., 2006

NOTARY STAMP/SEAL



BARRY COLEMAN  
MY COMMISSION # DD 587907  
EXPIRES: September 24, 2010  
Bonded Thru Budget Notary Services

[Signature]  
Signature of Notary

**Corporate Warranty Deed**

Inst:2006022132 Date:09/15/2006 Time:13:46

Doc Stamp-Deed : 630.00

DC, P. DeWitt Cason, Columbia County B:1036 P:646

This Indenture, made this September <sup>15<sup>th</sup></sup> 2006 A.D.

Between

MILESTONE HS, LLC, a Florida Limited Liability Company, whose post office address is: PO Box 3001, Lake City, FL 32056; Grantor and A & B MANAGEMENT, LLC, a Florida Limited Liability Company and JOHN W. KEEN, EACH AS TO AN UNDIVIDED ONE HALF INTEREST AS JOINT TENANTS WITH RIGHTS OF SURVIVORSHIP AND NOT AS TENANTS IN COMMON whose post office address is: 3624 NW Brown Road, Lake City, Florida 32055, Grantee,

Witnesseth, that the said Grantor, for and in consideration of the sum of Ten and No/100 Dollars (\$10.00), to it in hand paid by the said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee forever, the following described land, situate, lying and being in the County of Columbia, State of Florida, to wit:

Lots 39 and 59, ROLLING MEADOWS, according to the Plat thereof, recorded in Plat Book 8, Pages 45 and 46, of the Public Records of Columbia County, Florida.

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

Parcel Identification Number:

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2005.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

MILESTONE HS, LLC  
a Florida Limited Liability Company

Signed and Sealed in Our Presence:

By:

Robert S. Stewart, Manager

Witness Printed Name:

Matthew D. Rocco

Melinda M. Weaver

(Corporate Seal)

Witness Printed Name:

Melinda M. Weaver

State of  
County of

Florida  
Columbia

The foregoing instrument was acknowledged before me this <sup>15<sup>th</sup></sup> day of September, 2006, by Robert S. Stewart, Manager of Milestone HS, LLC, a Florida Limited Liability Company, on behalf of the company.

He is personally known to me or has produced

*Driver's License*

as identification.

Notary Public

Notary Printed Name:

My Commission Expires:

(Seal)

#06-0274

Prepared by &amp; Return to:

Matt Rocco

Sierra Title, LLC,

619 SW Baya Drive, Suite 102

Lake City, Florida 32025



Matthew Rocco

My Commission 00150700

Expires September 17, 2009

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

## Florida Department of Community Affairs Residential Whole Building Performance Method A

<b>Project Name:</b> ROLLING MEADOWS 30 <b>Address:</b> <b>City, State:</b> , <b>Owner:</b> <b>Climate Zone:</b> North	<b>Builder:</b> K&H FRAMING INC. <b>Permitting Office:</b> Columbia <b>Permit Number:</b> 22206 <b>Jurisdiction Number:</b> 221000
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<ol style="list-style-type: none"> <li>1. New construction or existing <span style="float: right;">New</span> <input type="checkbox"/></li> <li>2. Single family or multi-family <span style="float: right;">Single family</span> <input type="checkbox"/></li> <li>3. Number of units, if multi-family <span style="float: right;">1</span> <input type="checkbox"/></li> <li>4. Number of Bedrooms <span style="float: right;">3</span> <input type="checkbox"/></li> <li>5. Is this a worst case? <span style="float: right;">Yes</span> <input type="checkbox"/></li> <li>6. Conditioned floor area (ft²) <span style="float: right;">1706 ft²</span> <input type="checkbox"/></li> <li>7. Glass type<sup>1</sup> and area: (Label reqd. by 13-104.4.5 if not default)           <table style="width: 100%;"> <tr> <td style="width: 30%;">a. U-factor:</td> <td style="width: 30%;">Description</td> <td style="width: 40%;">Area</td> </tr> <tr> <td>(or Single or Double DEFAULT)</td> <td>7a. 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Glass/Floor Area: 0.06

Total as-built points: 22584

Total base points: 26295

# PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

**PREPARED BY:** SUNCOAST INSULATORS  
825 NW 253rd Terrace  
Mechanicsville, FL 32060  
**DATE:** 10-16-04 (352) 472-0900  
Fax (352) 472-0900

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

**OWNER/AGENT:** \_\_\_\_\_  
**DATE:** \_\_\_\_\_

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

**BUILDING OFFICIAL:** \_\_\_\_\_  
**DATE:** \_\_\_\_\_



<sup>1</sup> Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.



# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Omt Len Hgt			Area X SPM X SOF = Points			
.18	1706.0	20.04	6153.9	Double, Clear	W	2.0	6.0	35.0	38.52	0.85	1145.3
				Double, Clear	N	2.0	6.0	6.0	19.20	0.90	103.7
				Double, Clear	E	2.0	6.0	69.0	42.06	0.85	2461.3
				As-Built Total:							110.0
WALL TYPES Area X BSPM = Points				Type	R-Value			Area X SPM = Points			
Adjacent	262.0	0.70	183.4	Frame, Wood, Exterior	13.0			1179.0	1.50	1768.5	
Exterior	1179.0	1.70	2004.3	Frame, Wood, Adjacent	13.0			262.0	0.60	157.2	
Base Total:		1441.0	2187.7	As-Built Total:			1441.0			1925.7	
DOOR TYPES Area X BSPM = Points				Type				Area X SPM = Points			
Adjacent	18.0	2.40	43.2	Exterior Insulated				36.0	4.10	147.6	
Exterior	36.0	6.10	219.6	Adjacent Insulated				18.0	1.60	28.8	
Base Total:		54.0	262.8	As-Built Total:			54.0			176.4	
CEILING TYPES Area X BSPM = Points				Type	R-Value			Area X SPM X SCM = Points			
Under Attic	1706.0	1.73	2951.4	Under Attic	30.0			1706.0	1.73 X 1.00	2951.4	
				Under Attic	19.0			122.0	2.34 X 1.00	285.5	
Base Total:		1706.0	2951.4	As-Built Total:			1828.0			3236.9	
FLOOR TYPES Area X BSPM = Points				Type	R-Value			Area X SPM = Points			
Slab	169.0(p)	-37.0	-6253.0	Slab-On-Grade Edge Insulation	0.0			169.0(p)	-41.20	-6962.8	
Raised	0.0	0.00	0.0								
Base Total:		-6253.0		As-Built Total:			169.0			-6962.8	
INFILTRATION Area X BSPM = Points							Area X SPM = Points				
		1706.0	10.21			17418.3			1706.0	10.21	17418.3

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
<b>Summer Base Points: 22721.0</b>				<b>Summer As-Built Points: 19504.7</b>						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
22721.0	0.4266		9692.8	<small>(sys 1: Central Unit 29300 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)</small> 19505      1.00    (1.09 x 1.147 x 1.00)    0.263      1.000      6402.1 <b>19504.7    1.00      1.250      0.263      1.000      6402.1</b>						

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT								
GLASS TYPES												
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ormt Len Hgt		Area X WPM X WOF = Points					
.18	1706.0	12.74	3912.2	Double, Clear	W	2.0	6.0	35.0	20.73	1.04	756.5	
				Double, Clear	N	2.0	6.0	6.0	24.58	1.00	148.2	
				Double, Clear	E	2.0	6.0	69.0	18.79	1.06	1375.2	
				As-Built Total:				110.0				2279.9
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points					
Adjacent	262.0	3.60	943.2	Frame, Wood, Exterior	13.0		1179.0	3.40				4008.6
Exterior	1179.0	3.70	4362.3	Frame, Wood, Adjacent	13.0		262.0	3.30				864.6
Base Total: 1441.0 5305.5				As-Built Total:				1441.0				4873.2
DOOR TYPES Area X BWPM = Points				Type	Area X WPM = Points							
Adjacent	18.0	11.50	207.0	Exterior Insulated	36.0 8.40 302.4							
Exterior	36.0	12.30	442.8	Adjacent Insulated	18.0 8.00 144.0							
Base Total: 54.0 649.8				As-Built Total:				54.0				448.4
CEILING TYPESArea X BWPM = Points				Type	R-Value	Area X WPM X WCM = Points						
Under Attic	1706.0	2.05	3497.3	Under Attic	30.0	1706.0	2.05 X 1.00	3497.3				
				Under Attic	19.0	122.0	2.70 X 1.00	329.4				
Base Total: 1706.0 3497.3				As-Built Total:				1828.0				3826.7
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points					
Slab	169.0(p)	8.9	1504.1	Slab-On-Grade Edge Insulation	0.0		169.0(p)	18.80	3177.2			
Raised	0.0	0.00	0.0									
Base Total: 1504.1				As-Built Total:				169.0	3177.2			
INFILTRATION Area X BWPM = Points				Area X WPM = Points								
1706.0 -0.59 -1006.5				1706.0 -0.59 -1006.5								

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
<b>Winter Base Points:</b>		<b>13862.4</b>		<b>Winter As-Built Points:</b>			<b>13596.8</b>		
Total Winter Points	X System Multiplier	=	Heating Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
<b>13862.4</b>	<b>0.6274</b>		<b>8697.2</b>	(sys 1: Electric Heat Pump 28000 btuh ,EFF(7.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0 13596.8	1.000	(1.069 x 1.169 x 1.00)	0.487	1.000	8277.3
<b>13862.4</b>	<b>0.6274</b>		<b>8697.2</b>	<b>13596.8</b>	<b>1.00</b>	<b>1.250</b>	<b>0.487</b>	<b>1.000</b>	<b>8277.3</b>



**WATER HEATING & CODE COMPLIANCE STATUS****Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE					AS-BUILT					
<b>WATER HEATING</b>										
Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit = Total Multiplier
3		2635.00		7905.0	50.0	0.92	3		1.00	2635.00
					<b>As-Built Total:</b>					<b>7905.0</b>

CODE COMPLIANCE STATUS												
BASE							AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
9693		8697		7905		26295	6402		8277		7905	22584

**PASS**

# Code Compliance Checklist

## Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

**6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST**

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

**6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)**

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

**ESTIMATED ENERGY PERFORMANCE SCORE\* = 85.7**

**The higher the score, the more efficient the home.**

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 29.3 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft²)	1706 ft²		
7. Glass type <sup>1</sup> and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 28.0 kBtu/hr
(or Single or Double DEFAULT)	7a. (Dble Default) 110.0 ft²		HSPF: 7.00
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 110.0 ft²	c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 169.0(p) ft	a. Electric Resistance	Cap: 50.0 gallons
b. N/A			EF: 0.92
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Frame, Wood, Exterior	R=13.0, 1179.0 ft²	(HR-Heat recovery, Solar	
b. Frame, Wood, Adjacent	R=13.0, 262.0 ft²	DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits	
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=30.0, 1706.0 ft²	MZ-C-Multizone cooling,	
b. Under Attic	R=19.0, 122.0 ft²	MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 199.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: JK / Jason Elison Date: 10/16/08

Address of New Home: 159 SW poppy Glen Rd. City/FL Zip: LAKE CITY, FL 32029



*\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at [www.fsec.ucf.edu](http://www.fsec.ucf.edu) for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

<sup>1</sup> Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.  
EnergyGauge® (Version: FLRCSB v4.0)

# **ROLLING MEADOWS LOT 30 HVAC LOAD ANALYSIS**

for

**K&H FRAMING**

Prepared By:

DAVID HALL  
DAVID HALL'S INC.  
PO BOX 244  
LAKE CITY FL.  
386-755-9792  
10/9/06



### Miscellaneous Project Data

Project File Name: K & H FRAMING, LOT 30

### System Input Data

—System 1—	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum.	Indoor Dry Bulb	Grains Difference
Winter:	31	N/A	N/A	72	N/A
Summer:	98	83	50%	75	83

### External Overhangs

No.	Projection	Offset	No.	Projection	Offset
1	3	1	6	0	0
2	5	0	7	0	0
3	4	0.5	8	0	0
4	0	0	9	0	0
5	0	0	10	0	0

### Duct Sizing Inputs

	Runouts	Main Trunk
Duct Material:	Flexible Duct	Fiberglass Duct Board
Roughness Factor:	0.010000	0.003000
Pressure Drop:	0.1000 In.wg/100 Ft.	0.1000 In.wg/100 Ft.
Minimum Velocity:	450.0 Ft./Minute	650.0 Ft./Minute
Maximum Velocity:	750.0 Ft./Minute	900.0 Ft./Minute
Minimum Height:	0 Inches	0 Inches
Maximum Height:	0 Inches	0 Inches

### Outside Air Data

	Winter	Summer
Infiltration:	0.900 AC/Hr	0.400 AC/Hr
Volume of Conditioned Space:	X 14348 Cu.Ft.	X 14348 Cu.Ft.
	12,913 Cu.Ft./Hr	5,739 Cu.Ft./Hr
	X 0.0167	X 0.0167
Total Building Infiltration:	215.22 CFM	95.65334 CFM
Total Building Ventilation:	0 CFM	0 CFM
—System 1—		
Infiltration & Ventilation Sensible Gain Multiplier:	25.30 = (1.10 X 23.00 Summer Temp. Difference)	
Infiltration & Ventilation Latent Gain Multiplier:	56.64 = (0.68 X 83.30 Grains Difference)	
Infiltration & Ventilation Sensible Loss Multiplier:	45.10 = (1.10 X 41.00 Winter Temp. Difference)	

### Total Building Summary Loads

Component Description	Area Quan	Sen. Loss	Lat. Gain	Sen. Gain	Total Gain
3C Window Double Pane Clear Glass Metal Frame	86	2,557	0	2,900	2,900
9G French Door Double Clear Glass Wood Frame	42	899	0	1,016	1,016
11C Door Metal Polystyrene Core	63	1,215	0	789	789
12C Wall R-11 + 1/2" Gypsum(R-0.5)	1,179	4,352	0	2,819	2,819
16G Ceiling R-30 Insulation	1,706	2,307	0	2,645	2,645
22A Slab on Grade No Edge Insulation	169	5,613	0	0	0
Subtotals for structure:	3,245	16,943	0	10,169	10,169
Active People:	4	0	920	1,200	2,120
Inactive People:	0	0	0	0	0
Appliances:	0	0	1,200	1,200	2,400
Lighting:	0	0		4,774	
Ductwork:	0	1,439	0	2,046	2,046
Infiltration: Winter CFM: 215.2, Summer CFM: 95.7	191	9,705	5,419	2,419	7,838
Ventilation: Winter CFM: 0.0, Summer CFM: 0.0	0	0	0	0	0
Sensible Gain Total:				21,808	
Temperature Swing Multiplier:				X1.00	
Building Load Totals:		28,087	7,539	21,808	29,347

### Check Figures

Total Building Supply CFM:	991	CFM per square foot:	0.581
Square feet of room area:	1,706	Square feet per ton:	624.56

### Building Loads

Total heating required with outside air:	28,087 Btuh	28.087 MBH
Total sensible gain:	21,808 Btuh	74 %
Total latent gain:	7,539 Btuh	26 %
Total cooling required with outside air:	29,347 Btuh	2.446 Tons (based on sensible + latent)
		2.732 Tons (based on 77% sensible capacity)

### Notes

Calculations are based on 7th edition of ACCA Manual J.  
 All computed results are estimates as building use and weather may vary.  
 Be sure to select a unit that meets both sensible and latent loads.

## System #1 Summary Loads

Component Description	Area Quan	Sen. Loss	Lat. Gain	Sen. Gain	Total Gain
3C Window Double Pane Clear Glass Metal Frame	86	2,557	0	2,900	2,900
9G French Door Double Clear Glass Wood Frame	42	899	0	1,016	1,016
11C Door Metal Polystyrene Core	63	1,215	0	789	789
12C Wall R-11 + 1/2" Gypsum(R-0.5)	1,179	4,352	0	2,819	2,819
16G Ceiling R-30 Insulation	1,706	2,307	0	2,645	2,645
22A Slab on Grade No Edge Insulation	169	5,613	0	0	0
Subtotals for structure:	3,245	16,943	0	10,169	10,169
Active People:	4	0	920	1,200	2,120
Inactive People:	0	0	0	0	0
Appliances:	0	0	1,200	1,200	2,400
Lighting:	0	0		4,774	
Ductwork:	0	1,439	0	2,046	2,046
Infiltration: Winter CFM: 215.2, Summer CFM: 95.7	191	9,705	5,419	2,419	7,838
Ventilation: Winter CFM: 0.0, Summer CFM: 0.0	0	0	0	0	0
Sensible Gain Total:				21,808	
Temperature Swing Multiplier:				X1.00	
System Load Totals:		28,087	7,539	21,808	29,347

## Check Figures

Supply CFM:	991	CFM per square foot:	0.581
Square feet of room area:	1,706	Square feet per ton:	624.56

## System Loads

Total heating required with outside air:	28,087 Btuh	28.087 MBH
Total sensible gain:	21,808 Btuh	74 %
Total latent gain:	7,539 Btuh	26 %
Total cooling required with outside air:	29,347 Btuh	2.446 Tons (based on sensible + latent)
		2.732 Tons (based on 77% sensible capacity)

## Notes

Calculations are based on 7th edition of ACCA Manual J.  
 All computed results are estimates as building use and weather may vary.  
 Be sure to select a unit that meets both sensible and latent loads.

## Room Load Summary Reports

### System #1 Room Load Summary

No	Room Name	Area SF	Htg Sens Btuh	Htg Nom CFM	Run Duct Size	Run Duct Vel	Clg Sens Btuh	Clg Lat Btuh	Clg Nom CFM	Zone Adj Fact	Clg Adj CFM	Air Sys CFM
—Zone 1—												
1	Equipment Room	32	2,157	28	1-4	604	1,159	596	53	1.00	53	53
2	Laundry	75	2,412	31	1-4	673	1,293	596	59	1.00	59	59
3	Master Bedroom	165	3,705	48	1-6	569	2,459	1,081	112	1.00	112	112
4	Master Bath	84	1,468	19	1-4	702	1,348	170	61	1.00	61	61
5	Master Closet	36	846	11	1-3	641	692	0	31	1.00	31	31
6	Entry	87	1,917	25	1-4	602	1,155	596	53	1.00	53	53
7	Kitchen/eating	330	3,070	40	2-6	590	4,367	1,997	199	1.17	232	199
8	Family Room	342	4,907	64	2-5	626	3,755	1,421	171	1.00	171	171
9	Bath #2	120	859	11	1-4	432	830	0	38	1.00	38	38
10	Bedroom#2	193	3,321	43	1-6	506	2,187	656	99	1.00	99	99
11	Bedroom#3	193	3,321	43	1-6	577	2,104	426	96	1.18	113	96
12	Hall	49	104	1	1-3	425	459	0	21	1.00	21	21
System 1 Totals		1706	28,087	365			21,808	7,539	991		1,042	991
Main Trunk Size: 16x12 in.												

### System #1 Cooling System Summary

	Cooling Tons	Sensible/Latent Split	Sensible Btuh	Latent Btuh	Total Btuh
Net Required:	2.446	74%/26%	21,808	7,539	29,347
Recommended:	2.732	77%/23%	25,239	7,539	32,778

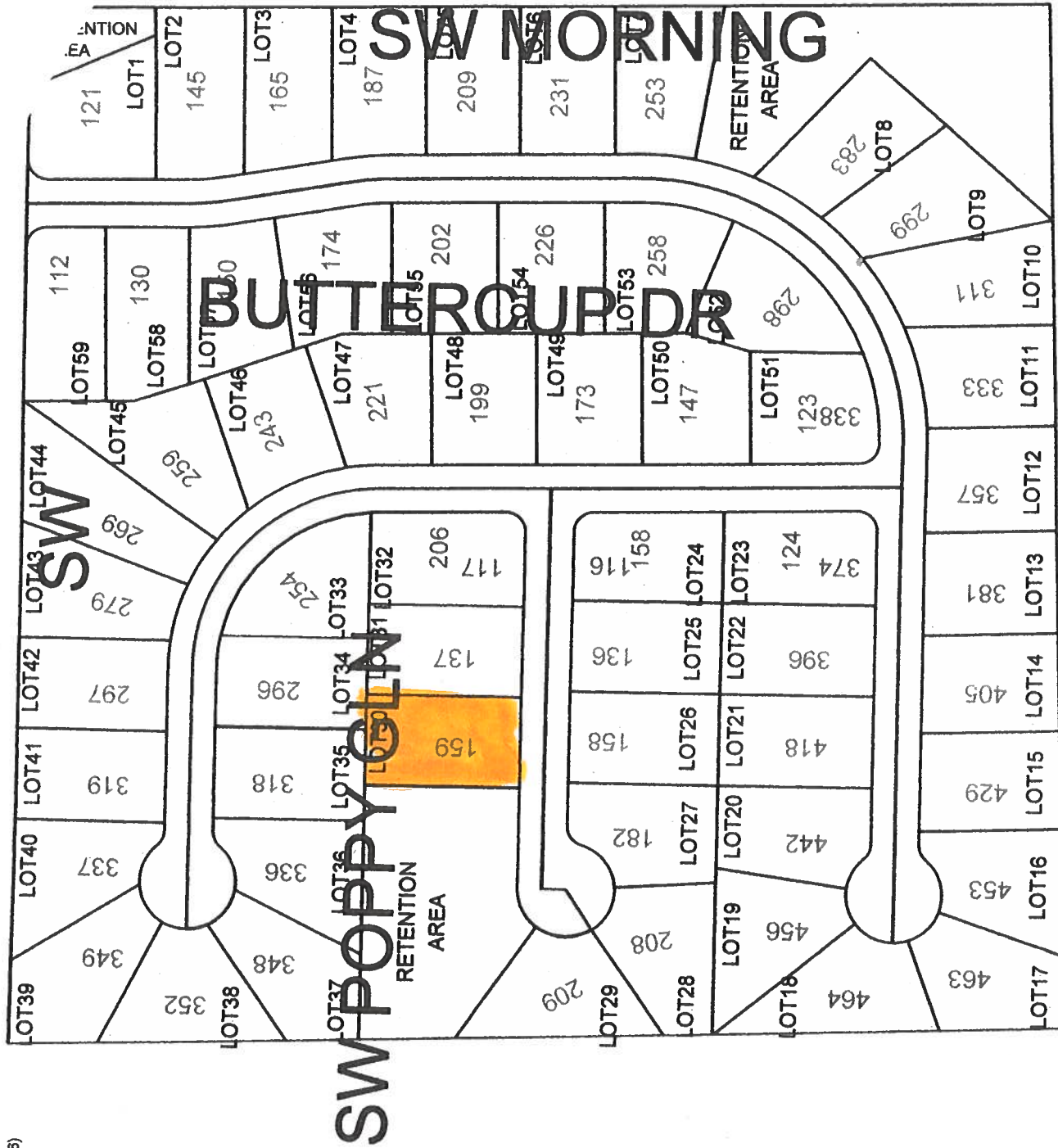


**Columbia County 9-1-1 Addressing / GIS Department  
Address Assignment Data for Rolling Meadows Subdivision  
Section 15, Township 4 South, Range 16 East**

<b>LOT#</b>	<b>ADDRESS</b>	<b>LOT#</b>	<b>ADDRESS</b>
LOT1	121 SW MORNING GLORY DR	LOT31	137 SW POPPY GLN
LOT2	145 SW MORNING GLORY DR	*LOT32	206 SW BUTTERCUP DR
LOT3	165 SW MORNING GLORY DR	*LOT32	117 SW POPPY GLN
LOT4	187 SW MORNING GLORY DR	LOT33	254 SW BUTTERCUP DR
LOT5	209 SW MORNING GLORY DR	LOT34	296 SW BUTTERCUP DR
LOT6	231 SW MORNING GLORY DR	LOT35	318 SW BUTTERCUP DR
LOT7	253 SW MORNING GLORY DR	LOT36	336 SW BUTTERCUP DR
LOT8	283 SW MORNING GLORY DR	LOT37	348 SW BUTTERCUP DR
LOT9	299 SW MORNING GLORY DR	LOT38	352 SW BUTTERCUP DR
LOT10	311 SW MORNING GLORY DR	LOT39	349 SW BUTTERCUP DR
LOT11	333 SW MORNING GLORY DR	LOT40	337 SW BUTTERCUP DR
LOT12	357 SW MORNING GLORY DR	LOT41	319 SW BUTTERCUP DR
LOT13	381 SW MORNING GLORY DR	LOT42	297 SW BUTTERCUP DR
LOT14	405 SW MORNING GLORY DR	LOT43	279 SW BUTTERCUP DR
LOT15	429 SW MORNING GLORY DR	LOT44	269 SW BUTTERCUP DR
LOT16	453 SW MORNING GLORY DR	LOT45	259 SW BUTTERCUP DR
LOT17	463 SW MORNING GLORY DR	LOT46	243 SW BUTTERCUP DR
LOT18	464 SW MORNING GLORY DR	LOT47	221 SW BUTTERCUP DR
LOT19	456 SW MORNING GLORY DR	LOT48	199 SW BUTTERCUP DR
LOT20	442 SW MORNING GLORY DR	LOT49	173 SW BUTTERCUP DR
LOT21	418 SW MORNING GLORY DR	LOT50	147 SW BUTTERCUP DR
LOT22	396 SW MORNING GLORY DR	*LOT51	338 SW MORNING GLORY DR
*LOT23	374 SW MORNING GLORY DR	*LOT51	123 SW BUTTERCUP DR
*LOT23	124 SW BUTTERCUP DR	LOT52	298 SW MORNING GLORY DR
*LOT24	158 SW BUTTERCUP DR	LOT53	258 SW MORNING GLORY DR
*LOT24	116 SW POPPY GLN	LOT54	226 SW MORNING GLORY DR
LOT25	136 SW POPPY GLN	LOT55	202 SW MORNING GLORY DR
LOT26	158 SW POPPY GLN	LOT56	174 SW MORNING GLORY DR
LOT27	182 SW POPPY GLN	LOT57	150 SW MORNING GLORY DR
LOT28	208 SW POPPY GLN	LOT58	130 SW MORNING GLORY DR
LOT29	209 SW POPPY GLN	LOT59	112 SW MORNING GLORY DR
LOT30	159 SW POPPY GLN		

**(NOTE: \* IDENTIFIES CORNER LOTS. CONTACT THE 9-1-1 ADDRESSING DEPARTMENT FOR CORRECT ADDRESS.)**

159 SW Poppy Glen Rd.  
Lake City, FL 32024



GLORY DR

# COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

Telephone: (386) 758-1125 \* FAX (386) 758-1365 \* Email: [ron\\_croft@columbiacountyfla.com](mailto:ron_croft@columbiacountyfla.com)

To: Mr. John Kerce, Building and Zoning Coordinator

Fr: Ronal Croft, 9-1-1 Addressing

Dt: November 23, 2005

Re: 9-1-1 Addressing of "Rolling Meadows" Subdivision.

Please find attached 9-1-1 Addressing data for Rolling Meadows Subdivision in Section 15, Township 4 South, Range 16 East.

**NOTE: Please contact the 9-1-1 Address Department concerning addresses for corner lots 23, 24, 32 and 51. Also, contact the 9-1-1 Address Department if two or more lots are to be combined for one residential location, as this will affect the address number.**

Please contact us at Telephone Number 758-1125 if there are any questions concerning the addressing of this subdivision.

XC: Environmental Health Department  
Lake City Post Office  
George Johnson, Bell South  
Larry Cook, Property Appraiser's Office  
File

# Columbia County Building Department Culvert Permit

Culvert Permit No.  
**000001251**

DATE 11/06/2006 PARCEL ID # 15-4S-16-03023-530  
APPLICANT GLENN KEEN PHONE 961.8223  
ADDRESS 1534 SW DEKLE ROAD LAKE CITY FL 32024  
OWNER JOHN KEEN/A&B MANAGEMENT,LLC PHONE 961.8223  
ADDRESS 159 SW POPPY GLEN LAKE CITY FL 32024  
CONTRACTOR JASON ELIXSON PHONE 386.623.1741  
LOCATION OF PROPERTY 90-W TO SR.247-S TO CALLAHAN,TL TO ROLLING MEADOWS S.D. TO MORNING  
GLORY,TR TO BUTTERCUP,TR TO POPPY GLN, 3RD LOT ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT ROLLING MEADOWS 30

SIGNATURE 

## INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
- b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



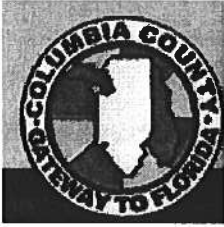
Other \_\_\_\_\_

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED  
DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21  
Lake City, FL 32055  
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00





From: The Columbia County Building & Zoning Department  
Plan Review  
135 NE Hernando Av.  
P.O. Box 1529  
Lake City Florida 32056-1529

Reference to a building permit application Number: **0610-69**  
Glenn Keen/ Jason Elixson Construction Owner A & B Management Property ID  
15-4s-16-03023-530

On the date of October 26, 2006 application 0610-69 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

**Please include application number 0610-69 and when making reference to this application.**

***This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.***

**1.** This is to clarify the 2004 Florida Residential Building Code requirements as the code relates to residential garages.

Section R309 garages and carports:

**A. R309.1 Opening protection:**

Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.

**B. R309.2 Separation required:**

The plans show that the mechanical room door will provide the required the 20-minute fire-rated doors, ***please verify that the egress door from the garage into the residence will also be a 20-minute fire-rated doors.***

The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side.

**D. R309.3 Floor surface:**

Garage floor surfaces shall be of approved noncombustible material.

The area of floor used for parking of automobiles or other vehicles shall be sloped to facilitate the movement of liquids to a drain or toward the main vehicle entry doorway.

**F.** If an attic access opening (pull down ladder type attic egress door) is installed in the garage ceiling it shall have the same protection requirements of sections R309.1 and R309.2 which requires this attic egress door and the door assembly



to have a 20-minute fire-rating. *Please show the size and location of the attic access opening.*

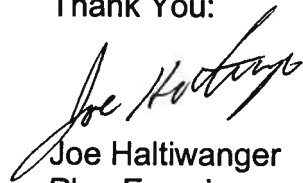
**2.** The electrical plans show the electrical panel being located in the utility room.

At the electrical service entrance point an overcurrent protection device shall be installed on the exterior of structure which will provide overcurrent protection for the total service amperage rating and a means of disconnecting electrical service from the serving utility company. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

**3.** Please have Mr. Eskridge engineer the required header size to span the sixteen foot opening for the garage overhead door opening.

*Please provide documentation which will verify that the above 2004 Florida Residential Building Code requirements will be complied with.*

Thank You:



Joe Haltiwanger  
Plan Examiner  
Columbia County Building  
Department

# **K & H Framing/Vinyl Siding, Inc.**

1534 S.W.Dekle Road  
Lake City, Florida 32024  
(386)961-8223

November 3, 2006

To: The Columbia County Building & Zoning Department  
Joe Haltwinger

RE: 0610-69

Glenn Keen/Jason Elixson Construction  
Owner A & B Management  
Property ID# 15-4s-16-03023-530

Upon receiving the Florida residential Code 2004, I agree to the following:

**A. Opening protection:**

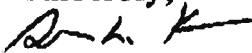
- will not open into a room used for sleeping
- other openings between the garage and the resident will be equipped with a honeycomb steel door, not less than an 1 3/8' thick and a 20 minute fire rating

**B. The mechanical room door will have a 20 minute fire rating and meet the requirement of the above mentioned (A.)**

- C. The attic access area will be made of 1/2' gypsum board
- D. The garage floor will be made of non combustibile materials (concrete) and it will slope to drain towards the main vehicle entrance way
- E. The electric plans show the electric panel in the utility room, at the electric service entrance an over current will be installed on the exterior structure which will provide an over-current protection for the total service amperage
- F. Mr. Jason Elixson is faxing over the garage header specks

All the above mention is agreed to and will be carried out to meet the 2004 Florida Residential Building Code Requirements.

Sincerely,



Glenn L. Keen

K&H Framing/A &B Management

Reference Number: 0610-69



**ELK**

ROOFING PRODUCTS SPECIFICATIONS - TUSCALOOSA, AL

**PRESTIQUE®  
HIGH DEFINITION®****RAISED PROFILE®****Prestique Plus High Definition  
and Prestique Gallery Collection\*\***

Product size \_\_\_\_\_ 13 1/2" x 39 1/2"  
 Exposure \_\_\_\_\_ 5 1/2"  
 Pieces/Bundle \_\_\_\_\_ 16  
 Bundles/Square \_\_\_\_\_ 4/98.5 sq.ft.  
 Squares/Pallet \_\_\_\_\_ 11

50-year limited warranty period:  
 5-7\*\*years non-prorated coverage for  
 shingles and application labor with  
 prorated coverage for remainder of  
 limited warranty period, plus an  
 option for transferability\*. 5-year  
 limited wind warranty\*. Wind  
 Coverage: standard 80 mph, extended  
 110 mph\*\*\*

*See separate High Definition*

Product size \_\_\_\_\_ 13 1/2" x 39 1/2"  
 Exposure \_\_\_\_\_ 5 1/2"  
 Pieces/Bundle \_\_\_\_\_ 16  
 Bundles/Square \_\_\_\_\_ 4/98.5 sq.ft.  
 Squares/Pallet \_\_\_\_\_ 14

40-year limited warranty period:  
 5-7\*\*years non-prorated coverage for  
 shingles and application labor with  
 prorated coverage for remainder of  
 limited warranty period, plus an  
 option for transferability\*. 5-year  
 limited wind warranty\*. Wind  
 Coverage: standard 80 mph, extended  
 90 mph\*\*\*

**Prestique High Definition**

Product size \_\_\_\_\_ 13 1/2" x 38 1/2"  
 Exposure \_\_\_\_\_ 5 1/2"  
 Pieces/Bundle \_\_\_\_\_ 22  
 Bundles/Square \_\_\_\_\_ 3/100 sq.ft.  
 Squares/Pallet \_\_\_\_\_ 16

30-year limited warranty period:  
 5-7\*\*years non-prorated coverage for  
 shingles and application labor with  
 prorated coverage for remainder of  
 limited warranty period, plus an  
 option for transferability\*. 5-year  
 limited wind warranty\*. Wind  
 Coverage: standard 80 mph.

**Raised Profile**

Product size \_\_\_\_\_ 13 1/2" x 38 1/2"  
 Exposure \_\_\_\_\_ 5 1/2"  
 Pieces/Bundle \_\_\_\_\_ 22  
 Bundles/Square \_\_\_\_\_ 3/100 sq.ft.  
 Squares/Pallet \_\_\_\_\_ 16

30-year limited warranty period:  
 5-7\*\*years non-prorated coverage for  
 shingles and application labor with  
 prorated coverage for remainder of  
 limited warranty period, plus an  
 option for transferability\*. 5-year  
 limited wind warranty\*. Wind  
 Coverage: standard 70 mph.

**HIP AND RIDGE SHINGLES****Seal-A-Ridge® w/FLX™**

Size: 12" x 12"  
 Exposure: 6 1/2"  
 Pieces/Bundle: 45  
 Coverage: 4 Bundles =  
 100 linear feet

**Vented RidgeCrest™ w/FLX™**

Size: 13" x 13 1/2"  
 Exposure: 9 1/4"  
 Pieces/Box: 26  
 Coverage: 5 boxes =  
 100 linear feet

**Elk Starter Strip**

52 Bundles/Pallet  
 16 Pallets/Truck  
 936 Bundles/Truck  
 19 Pieces/Bundle  
 1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakeswood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood.  
 Gallery Collection: Balsam Forest®, Weathered Sage®, Sierra Sunset®.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard™ treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBD, and Texas Department of Insurance.

\*See actual limited warranty for conditions and limitations.

\*\* Effective January 1, 2004, the seven year non-prorated Underlayment Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, off in accordance with Elk's application instructions for such products. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all eaves and gable edges, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the eaves and gable edges of the roof in and north of the states of VA, KY, MO, KS, CO, UT, NV, & OR.  
 \*\*\*For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grand, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

**SPECIFICATIONS**

**SCOPE:** Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula FLX.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

**PREPARATION OF ROOF DECK:** Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

**Materials:** Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes (4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)), use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

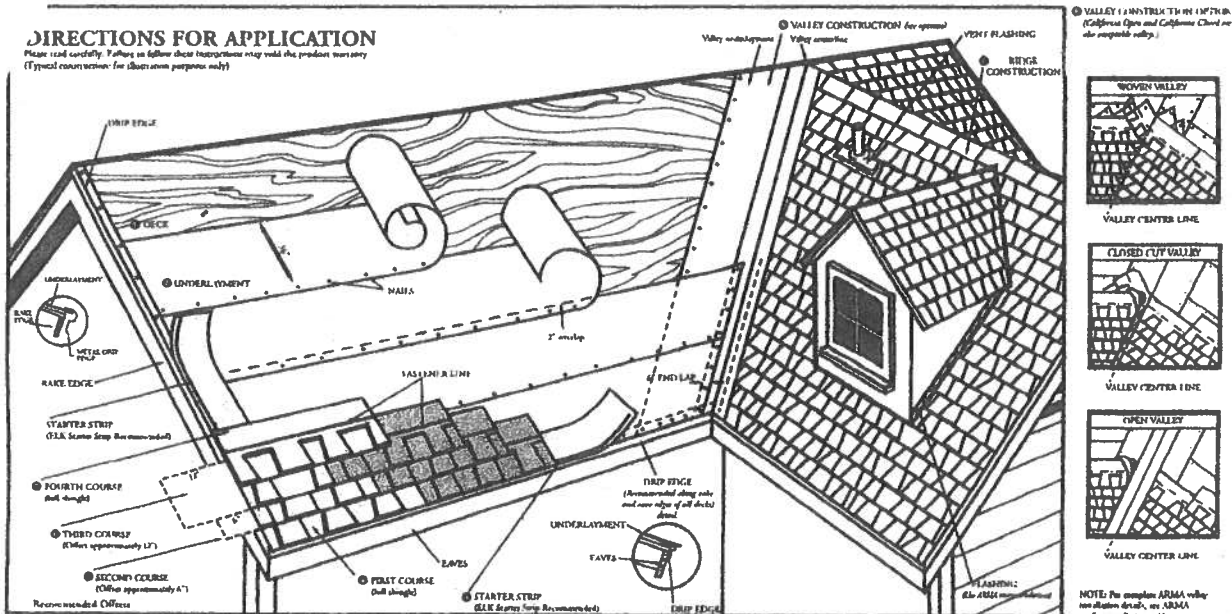
For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula FLX with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

## DIRECTIONS FOR APPLICATION

Please read carefully. Failure to follow these instructions may void the product warranty.  
(Typical construction for shingles purposes only)



## DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

### 1 DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

### 2 UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt, Elk Verasheild® or self-adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 18". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

**EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)**

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

### 3 STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

### 4 FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

### 5 SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

### 6 THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

### 7 FOURTH COURSE

Start at the rake and continue with full shingles across roof.

### FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rock shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

### 8 VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

### 9 RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" 2" Ridge or Seal-A-Ridge® with formula FLX™ or RidgeCrest™ with FLX (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

### FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common bond area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

**NAILS:** Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for re-roofs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

**STAPLES:** Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in related staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

### MANSARD APPLICATIONS

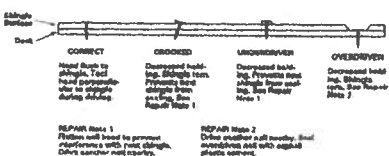
Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

### LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4" of an inch.

### HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the 'fastener line' or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.

All Prestique and Raised Profile shingles have a U.L.C. Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

**CAUTION TO WHOLESALER:** Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. **DO NOT DOUBLE STACK.** Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

**ELK**  
The Premium Choice®  
www.elkcorp.com



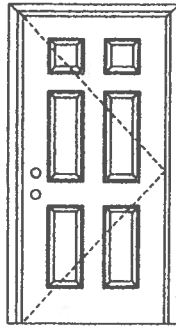
**X**

Opaque Inswing Unit

COP-WL-JH4101-02

## WOOD-EDGE STEEL DOORS

### APPROVED ARRANGEMENT:



**Note:**

Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itssemko.com](http://www.itssemko.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Single Door**

Maximum unit size = 3'0" x 6'8"

**Design Pressure**

**+66.0/-66.0**

limited water unless special threshold design is used.

**Large Missile Impact Resistance**

**Hurricane protective system (shutters) is NOT REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

### MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

### MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

### APPROVED DOOR STYLES:



Flush



Arch Top 3-panel



3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



15-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 5-panel with scroll

X

Opaque Inswing Unit

COP-WL-JH4101-02

## WOOD-EDGE STEEL DOORS

### CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

### PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH  
MIAMI-DADE BCCO  
PA201, PA202 & PA203

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

*Kurt L Balthazor*

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. - License Number 56533



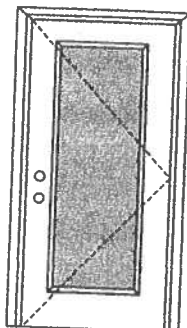
Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.edsemko.com](http://www.edsemko.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**X**  
Glazed Outswing Unit

COP-WL-JH4161-02

## WOOD-EDGE STEEL DOORS

### APPROVED ARRANGEMENT:



**Note:**  
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

Single Door  
Maximum unit size = 3'0" x 6'8"

Design Pressure  
**+40.5/-40.5**

Limited water unless special threshold design is used.

**Large Missile Impact Resistance**

**Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

### MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0011-02 and MAD-WL-MA0041-02.

### MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

### APPROVED DOOR STYLES:

#### 1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

#### 1/2 GLASS:



105 Series\*



106, 160 Series\*



129 Series\*



200 Series\*



12 R/L, 23 R/L, 24 R/L Series\*



107 Series\*



108 Series



304 Series

\* This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

**Johnson**  
EntrySystems

June 17, 2002

PREMDOR Collection



Exclusively from

Masonite

**X**

Glazed Outswing Unit

COP-WL-JH4161-02

**WOOD-EDGE STEEL DOORS****APPROVED DOOR STYLES:****3/4 GLASS:**

404 Series



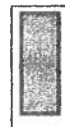
410 Series



450 Series

**FULL GLASS:**

109 Series

114, 120, 122  
Series

152 Series



149 Series



300 Series

**CERTIFIED TEST REPORTS:**

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

**PRODUCT COMPLIANCE LABELING:**

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. - License Number 56533

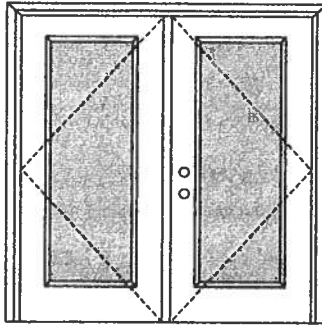


Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.itswh.com](http://www.itswh.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**XX**

Glazed Outswing Unit

COP-WL-JH4162-02

**WOOD-EDGE STEEL DOORS****APPROVED ARRANGEMENT:**

Test Data Review Certificate #3026447A  
and COP/Test Report Validation Matrix  
#3026447A-001 provides additional  
information - available from the ITS/WH  
website ([www.itsmko.com](http://www.itsmko.com)), the  
Masonite website ([www.masonite.com](http://www.masonite.com))  
or the Masonite technical center.

**Note:**

Units of other sizes are covered by this  
report as long as the panels used do not  
exceed 3'0" x 6'8".

**Double Door**

Maximum unit size = 6'0" x 6'8"

**Design Pressure****+40.5/-40.5**

Limited water unless special threshold design is used.

**Large Missile Impact Resistance****Hurricane protective system (shutters) is REQUIRED.**

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national,  
state or local building codes specify the edition required.

**MINIMUM ASSEMBLY DETAIL:**

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and  
MAD-WL-MA0041-02.

**MINIMUM INSTALLATION DETAIL:**

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

**APPROVED DOOR STYLES:****1/4 GLASS:**

100 Series



133, 135 Series



136 Series



680 Series



822 Series

**1/2 GLASS:**

105 Series\*



106, 160 Series\*



129 Series\*



200 Series\*

12 R/L, 23 R/L, 24 R/L  
Series\*

107 Series\*



108 Series



304 Series

\* This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

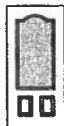
**XX**

Glazed Outswing Unit

COP-WL-JH4162-02

**WOOD-EDGE STEEL DOORS****APPROVED DOOR STYLES:****3/4 GLASS:**

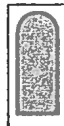
404 Series



410 Series



450 Series

**FULL GLASS:**

109 Series

114, 120, 122  
Series

152 Series



149 Series



300 Series

**CERTIFIED TEST REPORTS:**

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

**PRODUCT COMPLIANCE LABELING:**

TESTED IN  
ACCORDANCE WITH  
MIAMI-DADE BCCO PA202

COMPANY NAME  
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer  
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website ([www.ettsemko.com](http://www.ettsemko.com)), the Masonite website ([www.masonite.com](http://www.masonite.com)) or the Masonite technical center.

**Johnson™**  
**EntrySystems**

June 17, 2002  
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from

**Masonite®**  
Masonite International Corporation





**AAMA/NWWDA 101/I.S.2-97  
TEST REPORT**

**Rendered to:**

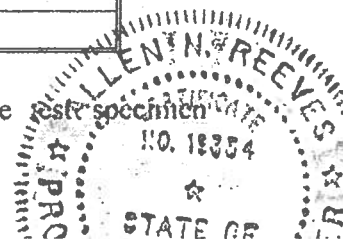
**MI HOME PRODUCTS, INC.**

**SERIES/MODEL: 650**

**TYPE: Aluminum Triple Single Hung Window**

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft <sup>2</sup>
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.



Architectural Testing

**AAMA/NWWDA 101/I.S.2-97 TEST REPORT**

Rendered to

MI HOME PRODUCTS, INC.  
P.O. Box 370  
650 West Market Street  
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01  
Test Date: 05/13/02  
And: 05/16/02  
Report Date: 06/05/02  
Expiration Date: 05/16/06

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

**Test Specimen Description:**

**Series/Model:** 650

**Type:** Aluminum Triple Single Hung Window

**Overall Size:** 9' 3-1/2" wide by 5' 11-11/16" high

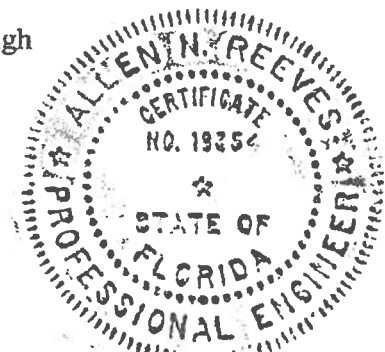
**Active Sash Size (3):** 3' 0-1/4" wide by 2' 10-3/4" high

**Fixed Daylight Opening Size (3):** 2' 8-1/4" wide by 2' 9-1/8" high

**Screen Size (3):** 2' 9-1/8" wide by 2' 11" high

**Finish:** All aluminum was painted white.

130 Derry Court  
York, PA 17402-9405  
phone: 717.764.7700  
fax: 717.764.4129  
www.archtest.com



*Allen M. Reeves*  
7 JUNE 2002

**Test Specimen Description: (Continued)**

**Glazing Details:** The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

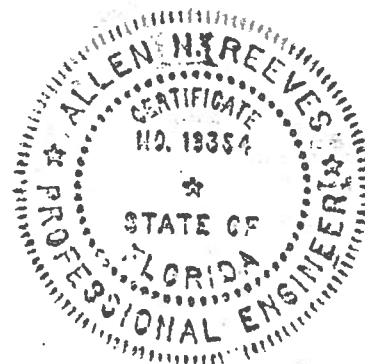
**Weatherstripping:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

**Frame Construction:** The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

**Sash Construction:** The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

**Screen Construction:** The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.



*Allen N. Reeves*  
7 JUNE 2002

## Test Specimen Description: (Continued)

### Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

**Drainage:** Sloped sill

**Reinforcement:** No reinforcement was utilized.

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

### Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft <sup>2</sup>	0.3 cfm/ft <sup>2</sup> max.

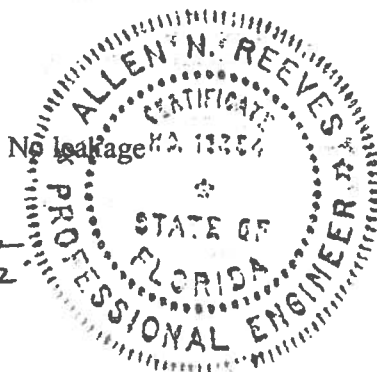
**Note #1:** The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00)  
(with and without screen)  
WTP = 2.86 psf

No leakage

No leakage

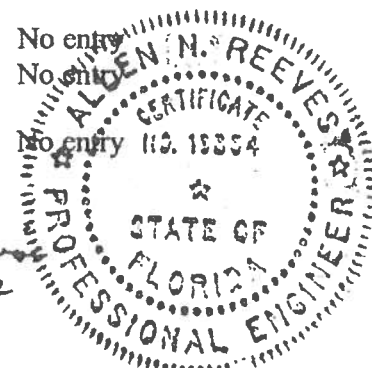
*Allen N. Reeves*  
7 JUNE 2002



**Test Results: (Continued)**

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.15" 0.29"	0.41" max. 0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" 0.01"	0.29" max. 0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs Right sash, meeting rail Right sash, bottom rail Middle sash, meeting rail Middle sash, bottom rail Left sash, meeting rail Left sash, bottom rail  In remaining direction at 50 lbs Right sash, right stile Right sash, left stile Middle sash, right stile Middle sash, left stile Left sash, right stile Left sash, left stile	0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25% 0.12"/25%  0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12% 0.06"/12%	0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%  0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100% 0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)  Type: A Grade: 10  Lock Manipulation Test  Test A1 through A5 Test A7  Lock Manipulation Test	No entry  No entry No entry  No entry	No entry  No entry No entry  No entry

Allen N. Reeves  
7 JUNE 2002

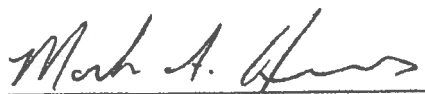


**Test Results: (Continued)**


<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Optional Performance</u>			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
<i>*Exceeds L/175 for deflection, but meets all other test requirements.</i>			
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

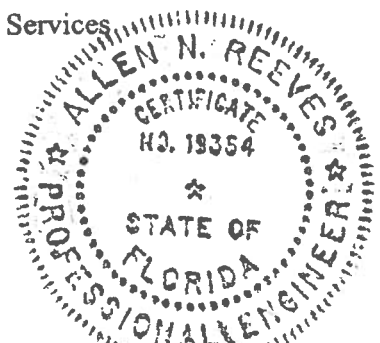
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC

  
Mark A. Hess  
Technician

MAH:nlb  
01-41641.01

  
Allen N. Reeves, P.E.  
Director - Engineering Services  
7 JUNE 2002







**AAMA/NWWDA 101/L.S.2-97  
TEST REPORT SUMMARY**

**Rendered to:**

**MI HOME PRODUCTS, INC.**

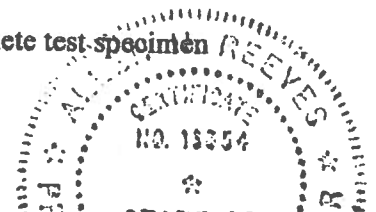
**SERIES/MODEL: 650 Fin  
TYPE: Aluminum Single Hung Window**

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft <sup>2</sup>
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

*Mark A. Lewis*





Architectural Testing

**AAMA/NWWDA 101/I.S.2-97 TEST REPORT**

Rendered to

MI HOME PRODUCTS, INC.  
650 West Market Street  
P.O. Box 370  
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01

Test Date: 03/07/02

Report Date: 03/26/02

Expiration Date: 03/07/06

**Project Summary:** Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

**Test Specification:** The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

**Test Specimen Description**

**Series/Model:** 650 Fin

**Type:** Aluminum Single Hung Window

**Overall Size:** 4' 4-1/4" wide by 6' 0-3/8" high

**Active Sash Size:** 4' 1-3/4" wide by 3' 0-5/8" high

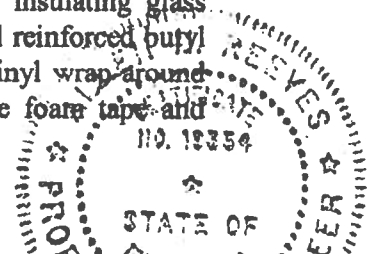
**Daylight Opening Size:** 3' 11-3/8" wide by 2' 9-1/2" high

**Screen Size:** 4' 0-1/4" wide by 2' 11-1/8" high

**Finish:** All aluminum was white.

**Glazing Details:** The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court  
York, PA 17402-9405  
phone: 717.764.7700



**Test Specimen Description: (Continued)**

**Weatherstripping:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

**Frame Construction:** The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

**Sash Construction:** The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

**Screen Construction:** The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

**Hardware:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail



# **Test Specimen Description: (Continued)**

**Drainage:** Sloped sill

**Reinforcement:** No reinforcement was utilized.

**Installation:** The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

## **Test Results:**

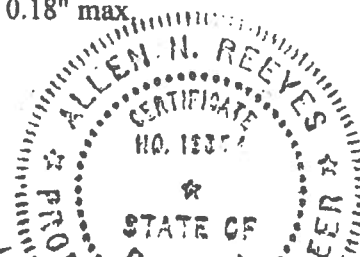
The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft <sup>2</sup>	0.3 cfm/ft <sup>2</sup> max
	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0.42"* 0.43"*	0.26" max. 0.26" max.

*\*Exceeds L/175 for deflection, but passes all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0.02" 0.02"	0.18" max. 0.18" max.
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*Allen N. Reeves*



**Test Specimen Description: (Continued)**

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.2	Deglazing Test (ASTM E 987) In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance

4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

*\*Exceeds L/175 for deflection, but passes all other test requirements.*

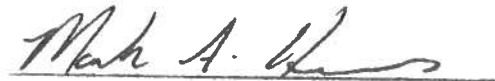
Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)	
@ 67.5 psf (positive)	0.05"
@ 70.8 psf (negative)	0.05"




*Allen N. Reeves*

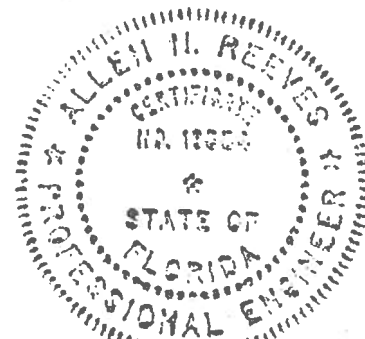
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

  
Mark A. Hess  
Technician

MAH:nlb  
01-41134.01

  
Allen N. Reeves, P.E.  
Director - Engineering Services  
1 APRIL 2002



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## **RESIDENTIAL WIND DESIGN & ANALYSIS**

FBC SECTION 1609 \ ***NO COPIES ARE TO BE PERMITTED***

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**PREPARED FOR:**

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JASON ELIXSON CONSTRUCTION \ THE KEEN RESIDENCE

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**PREPARED BY:**

**MARTY R. ESKRIDGE  
14952 MAIN ST  
ALACHUA FL 32615  
386-462-1340 / 352-375-6329**

# SUMMARY

## OF WIND DESIGN & ANALYSIS

**Trusses:** Lumber type So. Pine Grade #1 #2 #3 Size 2 x 4 Spacing 24 in.

**Roof sheathing:** Type OSB Size 7/16 Fastener type Nails Size 8d/131 6d  
Interior zone spacing: Interior 8 in. Periphery 4 in.  
Edge and end zone spacing: Interior 8 in. Periphery 4 in.

**Top double pl:** Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 12 in.

**Studs:** Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4  
Interior stud spacing 16 in. Composite (yes or no) Y  
End stud spacing 16 in. Composite (yes or no) Y

**Shearwall siding:** Type OSB Thickness 7/16 in.  
70' - Trans: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.  
56' - Long: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.

**Allowable unit shear on shearwalls:** 324 pounds per linear foot

**Wall tension transferred by:** Siding nails 8d/131 @ 4 O.C. edges

**Foundation anchor bolts:** Concrete strength 3000 psi  
Size 1/2 in. Shape L Washer 2" Embedment 7 in.  
Location of first anchor bolt from corner 8 in.

**Anchor Bolts @ 48" O.C.** Model A307 Loc. from corner 8 in.

**Type of foundation:** 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. # 5 at 96 in.  
Monolithic footing: Depth 20 in. Bottom width 12 in.

**Footing:** Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars  
Interior Footings: 16" W X 10" D

**Porch Columns:** 6X6X8' 54" #2 @ 144" o.c.

**Porch Column Fasteners:** 8d/131 - CB66/CC66 OR EARN  
Install 3#5 rebar cont. in all brick sections as per code

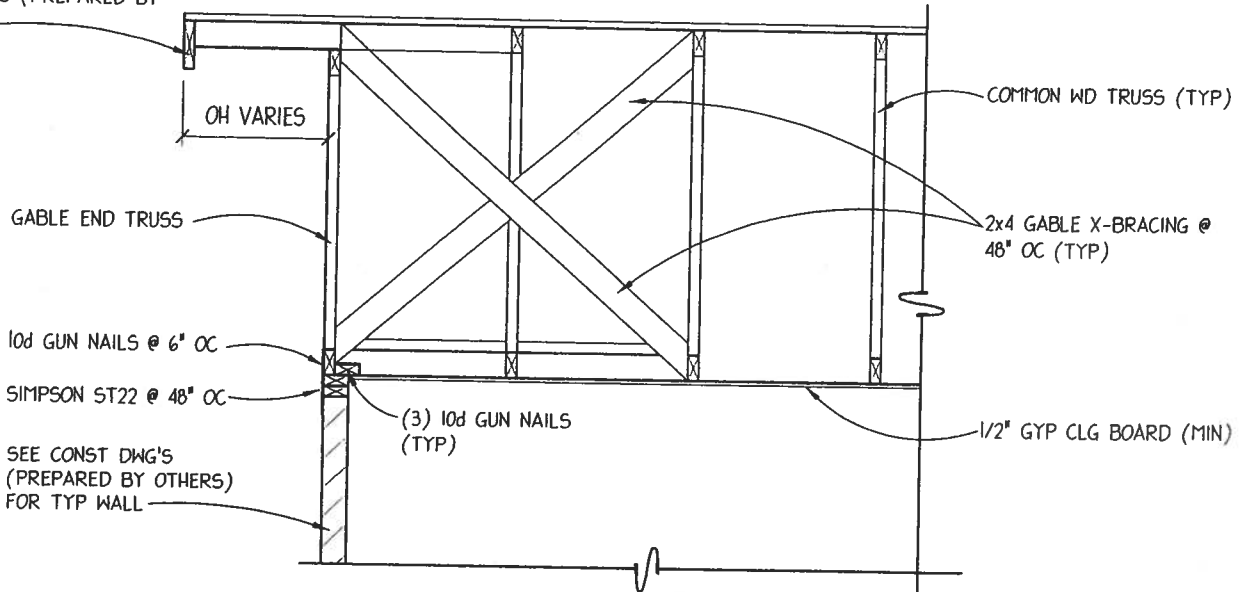
**NOTE:**

1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, NOT a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

Keen  
9/27/06



WOOD FASCIA- REFER TO  
CONST DWG'S (PREPARED BY  
OTHERS)



THIS WIND ANALYSIS ASSUMES IDEAL SOIL  
CONDITIONS W/MIN 2500 PSI SOIL BRG  
CAPACITY & 95% DRY PROCTOR DENSITIES

## GABLE END DETAIL

SCALE: 1/2" = 1'-0"

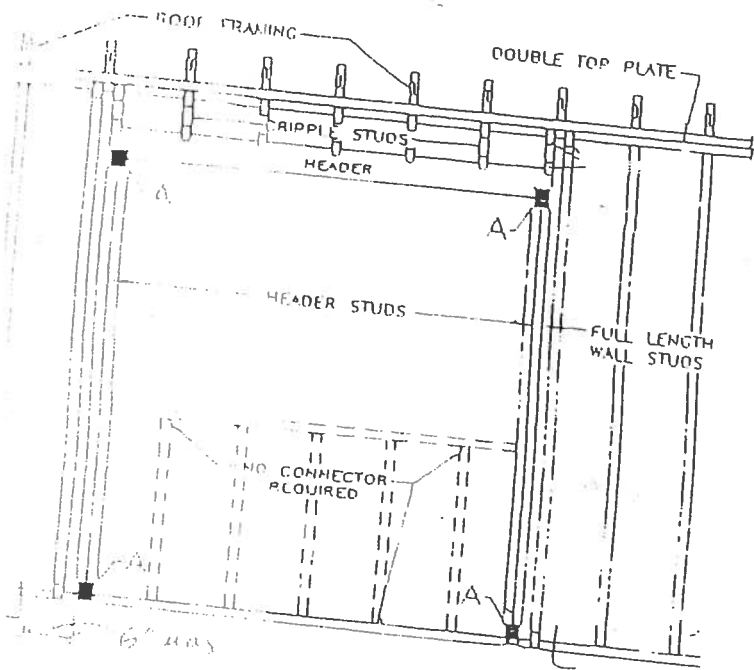
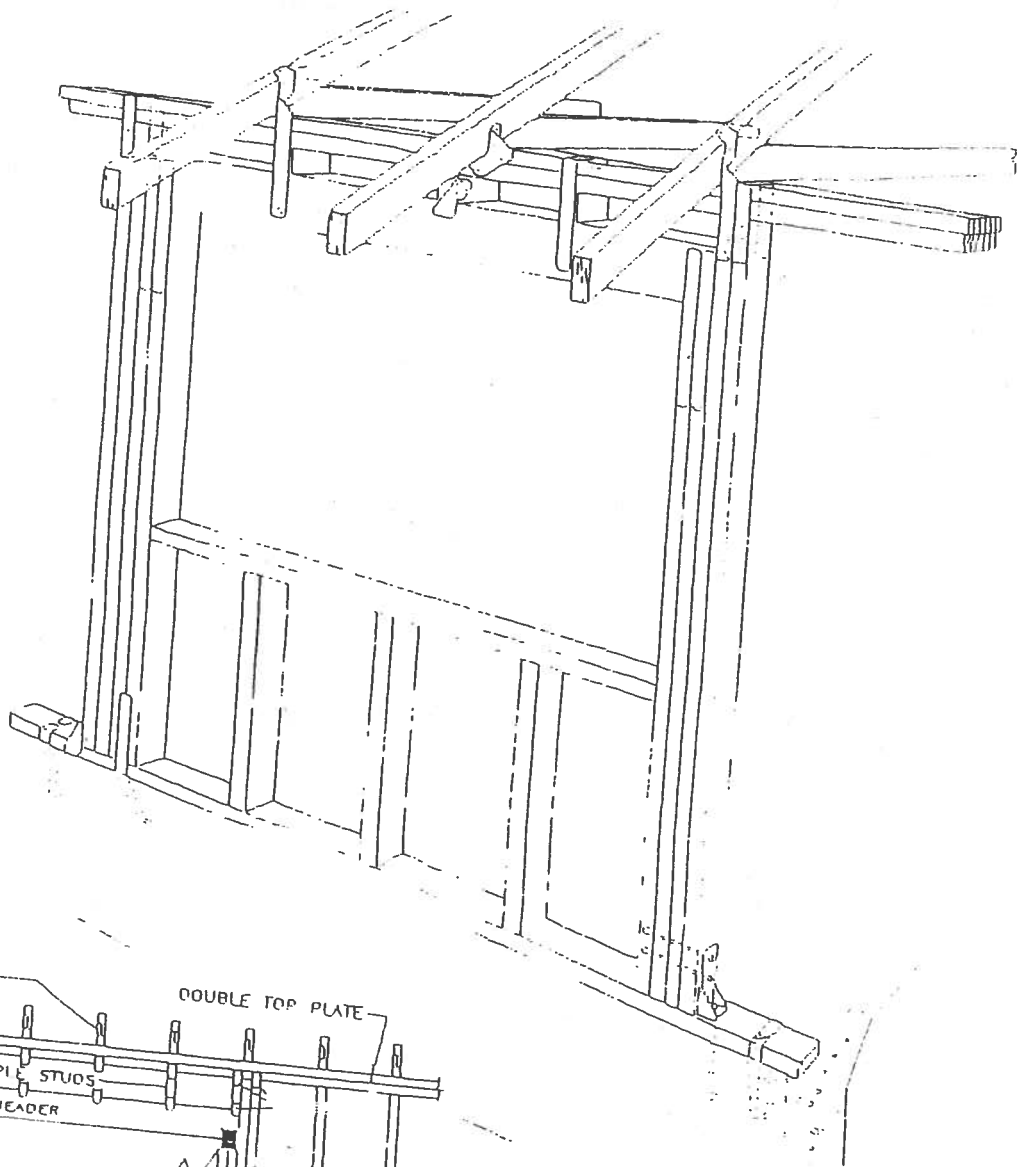
### NOTE:

INSTALL AND ERECT ALL TRUSS MEMBERS IN STRICT CONFORMANCE WITH  
THE PRE-ENGINEERED ROOF TRUSS MANUFACTURERS ERECTION SHEET  
ACCOMPANYING THE TRUSS PACKAGE. IF NOT AVAILABLE, IT IS THE  
CONTRACTOR'S RESPONSIBILITY TO CONTACT THE SUPPLIER TO OBTAIN  
ERECTION & BRACING SHEET.

*Handwritten signature and date:*  
9/27/06  
Kee

O:\SUPPORT\DETAILS\WINDLOADS\02-GABLE END.DWG 06/22/06 15:27

TYPICAL GABLE END DETAILS		<div>PAUL STRESING ASSOCIATES, INC.</div> <div><div><div><div><div></div><div></div></div><div><div></div><div></div></div></div><div><div><div>14517 MAIN STREET</div><div>ALACHUA, FLORIDA 32310</div><div>E-MAIL: <a href="mailto:psa@psa.net">psa@psa.net</a></div><div>TELEPHONE (904) 482-8407 FAX (904) 482-8408</div><div>REGISTRATION NO. ANYWHERE IN CA NO. 1A0003377</div></div><div><div><div></div><div></div></div><div><div></div><div></div></div></div></div><div><div>DRAWN BY C.A.D.</div><div>DATE 2006</div><div>PROJECT FILE WINDLOADS</div></div><div><div>SHEET NO.</div><div>Of</div></div></div></div>	
---------------------------	--	--	--



Total each truss uplift on the header divide by 2 for header anchorage

**TIE-DOWN TABLES**

<b>HEADERS</b>				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700
Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.				

<b>TRUSSES/GIRDERS</b>		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A
Two 12d common toenails are required per truss/rafter per bearing point into plate. Use proper bolt anchors. Strap rafters to truss or at each end with minimum uplift resistance of 450# each end. Strap ridge beam at each end with minimum uplift resistance of 1000#. It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.		

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
<b>BEAM SEATS</b>	LSTA18*	1200	LTT19*	1250
<b>POSTS</b> (max 17' spacing)	2-LSTA18	2400	ABU44	2300
*or per truss engineering Use proper bolt anchors All beams to be sheathed or strapped to Double Top Plate when applicable.				

<b>CRIPPLES</b>	Sheathing nailing alone adequate w/8d nails @ 3" O.C.
-----------------	---

<b>STUDS</b>
Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.
Wall sheathing nailing Adequate exterior walls top w/8d nails @ 3" O.C., as long as sheathing covers top plate, otherwise use SP2 @ 32" O.C. in addition to sheathing nailing.
Use SP2 top and SP1 bottom each stud for all interior load bearing walls and anchor bolts @ 32" O.C.
Interior anchor bolts to be ½" x 8" A307 or ½" x 6" wedge anchor or equivalent.

\*\* Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE: For nailing into SPF members, multiply table values by .86

# ASCE 7-02

9/26/06

## Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	19.44	ft
Mean Roof Height (Ht)	14.07	ft
Width Perp. to Wind (B)	57.00	ft
Width Parallel to Wind (L)	55.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.26
Flexible Structure	No

Calculated Parameters	
Importance Factor	1
Hurricane Prone Region (V>100 mph)	
Table C6-4 Values	
Alpha =	7.000
zg =	1200.000
At =	0.143
Bt =	0.840
Am =	0.250
Bm =	0.450
Cc =	0.300
l =	320.00 ft
Epsilon =	0.333
Zmin =	30.00 ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
lzm	$Cc * (33/z)^{0.167}$	0.3048	
Lzm	$l * (zm/33)^{Epsilon}$	309.99	ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.8948	
Gust2	$0.925 * ((1+1.7 * lzm * 3.4 * Q)/(1+1.7 * 3.4 * lzm))$	0.8629	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	$V * (5280/3600)$	161.33	ft/s
Vzm	$bm * (zm/33)^{Am} * Vhref$	70.89	ft/s
NF1	$NatFreq * Lzm / Vzm$	4.37	Hz
Rn	$(7.47 * NF1) / (1 + 10.302 * NF1)^{1.667}$	0.0552	
Nh	$4.6 * NatFreq * Ht / Vzm$	0.91	
Nb	$4.6 * NatFreq * B / Vzm$	3.70	
Nd	$15.4 * NatFreq * Depth / Vzm$	11.95	
Rh	$1/Nh - (1/(2 * Nh^2) * (1 - Exp(-2 * Nh)))$	0.5921	
Rb	$1/Nb - (1/(2 * Nb^2) * (1 - Exp(-2 * Nb)))$	0.2338	
Rd	$1/Nd - (1/(2 * Nd^2) * (1 - Exp(-2 * Nd)))$	0.0802	
RR	$((1/Beta) * Rn * Rh * Rb * (0.53 + 0.47 * Rd))^{0.5}$	0.6585	
gg	$+(2 * LN(3600 * n1))^{0.5} + 0.577 / (2 * LN(3600 * n1))^{0.5}$	4.19	
Gust3	$0.925 * ((1 + 1.7 * lzm * (3.4^2 * Q^2 + GG^2 * RR^2)^{0.5}) / (1 + 1.7 * 3.4 * lzm))$	1.05	

Gust Factor Summary			
Main Wind-force resisting system:		Components and Cladding:	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

# ASCE 7-02

9/26/06

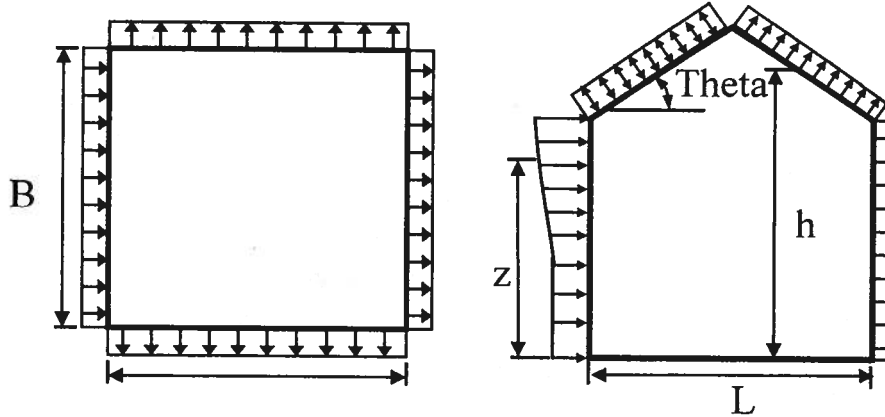
Wind Load Design per ASCE 7-02

## 6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft <sup>2</sup>	Pressure (lb/ft <sup>2</sup> )	
					Windward Wall*	
					+GCpi	-GCpi
19.44	0.70	1.00	1.00	21.70	11.78	18.19
15	0.70	1.00	1.00	21.70	11.78	18.19

**Figure 6-3 - External Pressure Coefficients, Cp**

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (15/z_g)^{2/\alpha}$	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d$	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 57 ft wall)	-0.50	-10.89	-4.48
Leeward Walls (Wind Dir Parallel to 55 ft wall)	-0.49	-10.77	-4.36
Side Walls	-0.70	-13.96	-7.55
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	-0.20	-6.30	0.11
Windward - Max Positive	0.30	1.37	7.78
Leeward Normal to Ridge	-0.60	-12.42	-6.01
Overhang Top	-0.20	-3.10	-3.10
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 7.035 ft	-0.90	-17.03	-10.62
Dist from Windward Edge: 7.035 ft to 14.07 ft	-0.90	-17.03	-10.62
Dist from Windward Edge: 14.07 ft to 28.14 ft	-0.50	-10.89	-4.48

## ASCE 7-02

9/26/06

### Wind Load Design per ASCE 7-02

Dist from Windward Edge: > 28.14 ft      -0.30      -7.81      -1.40

\* Horizontal distance from windward edge

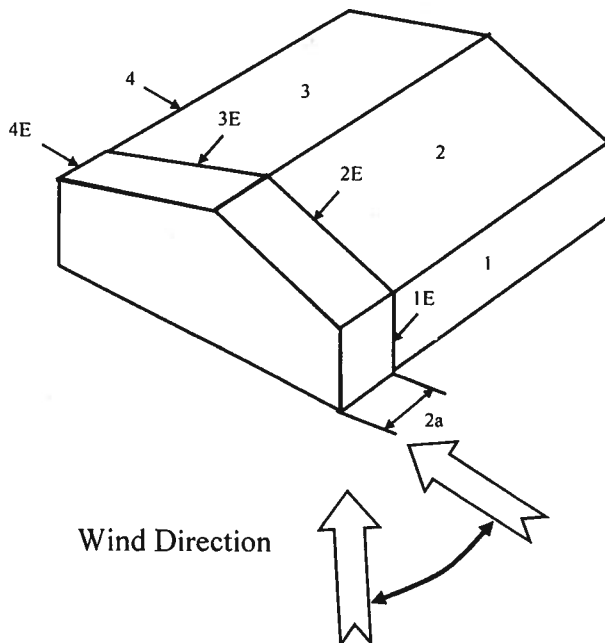
### Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned} K_h &= 2.01 \cdot (15/z_g)^{(2/\alpha)} &= & 0.57 \\ K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\ Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 17.80 \end{aligned}$$

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

$$* p = q_h \cdot (GC_{pf} - GC_{pi})$$



**ASCE 7-02**

9/26/06

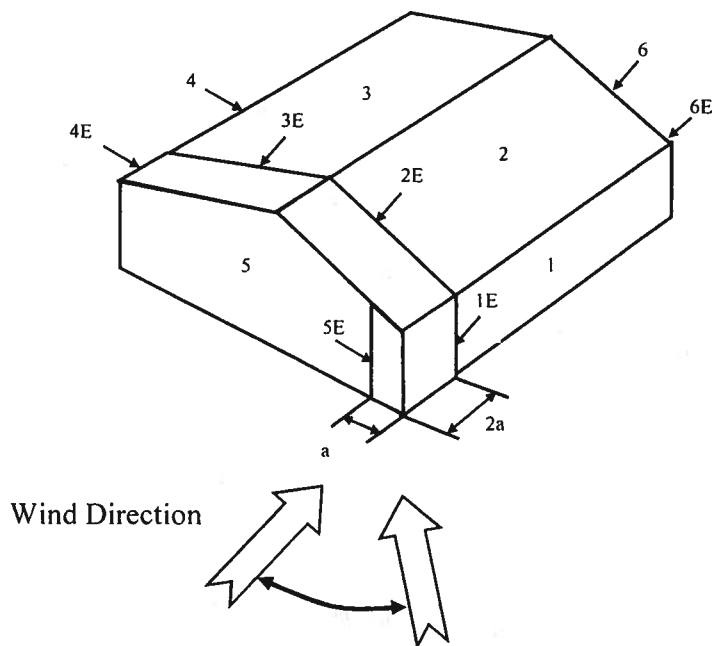
**Wind Load Design per ASCE 7-02****Figure 6-4 - External Pressure Coefficients, GCpf**

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (15/z_g)^{(2/\alpha)} &= & 0.57 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 17.80
 \end{aligned}$$

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

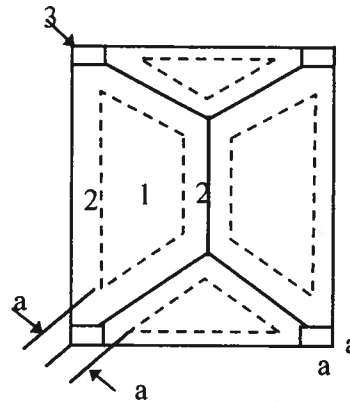
$$* p = q_h * (GC_{pf} - GC_{pi})$$

**Figure 6-5 - External Pressure Coefficients, GCp**

Loads on Components and Cladding for Buildings w/ Ht ≤ 60 ft

## Wind Load Design per ASCE 7-02

A 3D perspective drawing of a rectangular box. The front face is a rectangle with a width of 4 and a height of 5. The depth of the box is labeled as 'a'. The top edge of the box is labeled 'Ht' for height. The side faces are also labeled with dimensions 4 and 5.



Hipped Roof  
 $10 < \text{Theta} \leq 30$

$$a = 5.5 \Rightarrow \boxed{5.50 \text{ ft}}$$
[illegible]

### Table 6-7 Internal Pressure Coefficients for Buildings, $C_{spi}$

Condition	Gcpi	
	Max +	Max -



**ASCE 7-02**

9/26/06

**Wind Load Design per ASCE 7-02**

Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
<b>Enclosed Buildings</b>	<b>0.18</b>	<b>-0.18</b>

**Table 6-8 External Pressure Coefficients for Arched Roofs,  $C_p$** 

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	$C_p$		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	$C_p$	0.13	-1	-0.5
	P (+GCpi) - psf	-1.28	-18.57	-10.89
	P (-GCpi) - psf	5.12	-12.16	-4.48
Roof Springing from Ground	$C_p$	0.42	-1	-0.5
	P (+GCpi) - psf	3.25	-18.57	-10.89
	P (-GCpi) - psf	3.25	-18.57	-10.89

**Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings,  $C_f$** 

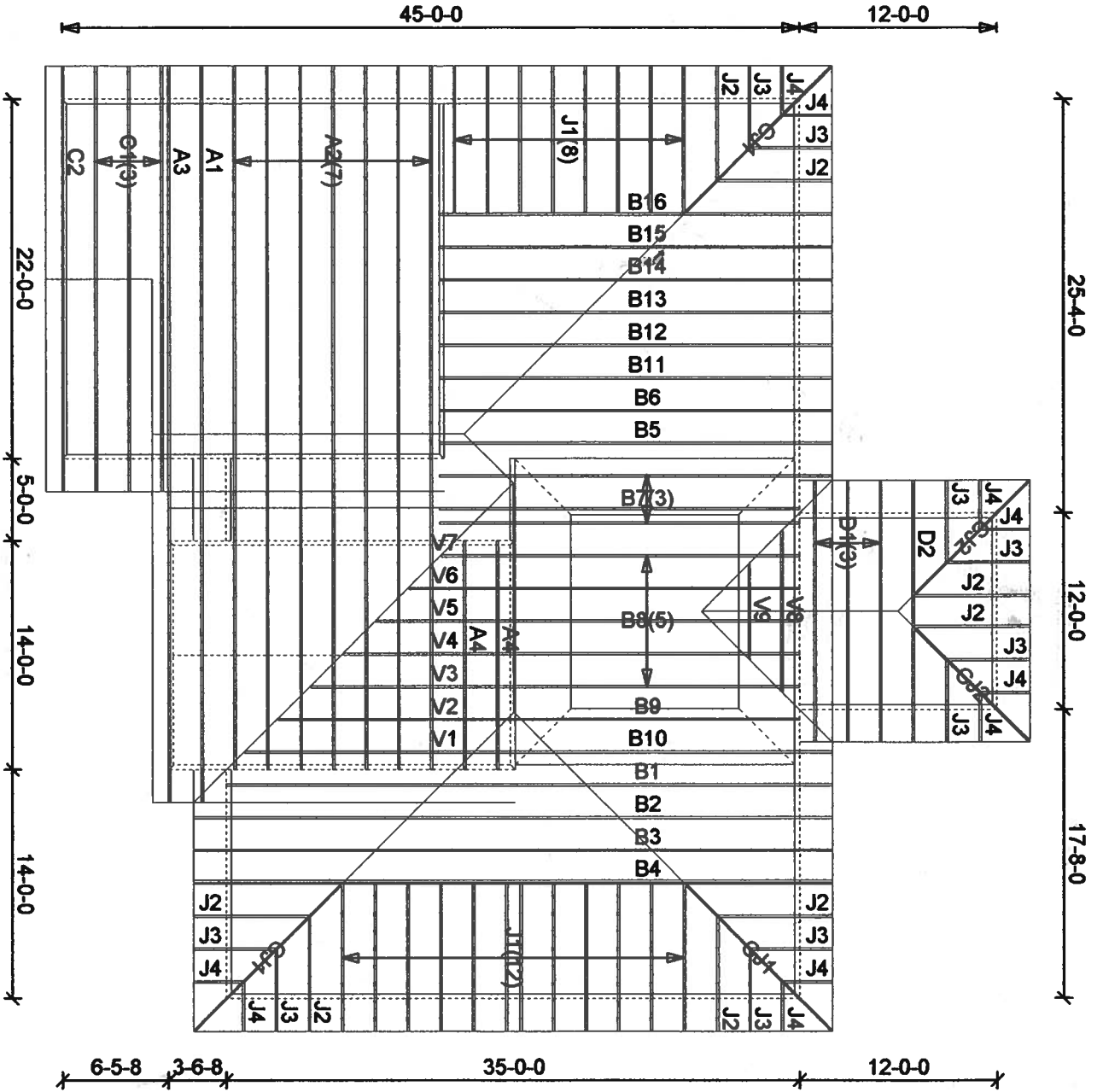
Variable	Description	Value	
L	Roof dimension normal to wind direction	55.00	ft
B	Roof dimension parallel to wind direction	57.00	ft
L/B	Ratio of L to B	0.965	
Theta	Slope of Roof	26.6	Deg
$C_f$	Force Coefficient	1.19	
X	Distance to center of pressure from windward edge	0.42	ft

Mayo Truss Co. Inc.  
 845 East US 27  
 MAYO, FL 32066  
 (386) 294-4988  
 (877) 588-6262

# GLEN KEEN

ROLLING MEADOWS LOT 30

110 MPH ASCE WIND LOAD



Roof Loading  
 TC Live: 20.00 psf  
 TC Dead: 10.00 psf  
 BC Live: 0.00 psf  
 BC Dead: 10.00 psf  
 TC Stress Inc: 25.00  
 BC Stress Inc: 25.00  
 Spacing: 2'-0" O.C.

Account: CONTRACTORS  
 Job: KEEN  
 Designer: CHAD LITTLE  
 Checker: M. MURRAY  
 Date: 10-18-08

Permit Number: \_\_\_\_\_ Lot Number: \_\_\_\_\_

Miscellaneous: \_\_\_\_\_ Address: \_\_\_\_\_

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator: Mayo Truss Company, Inc

Job Reference: KEEN - ROLLING MEADOWS - 30

**Standard Loading:**

T.C. Live	20 psf
T.C. Dead	10 psf
B.C. Live	0 psf
B.C. Dead	10 psf
Total	40 psf

**ROBBINS  
ENGINEERING, INC.**P.O. Box 280055  
Tampa, FL 33682-0055  
Phone: (813) 972-1135**Engineering Index Sheet**

Index Page 1 of 1

ANSI/ASCE 7-02  
Wind Speed - 110 MPH  
Mean Roof Ht. - 15 FT  
Exposure Category - B  
Occupancy Factor - 1.00  
C and C  
Enclosed

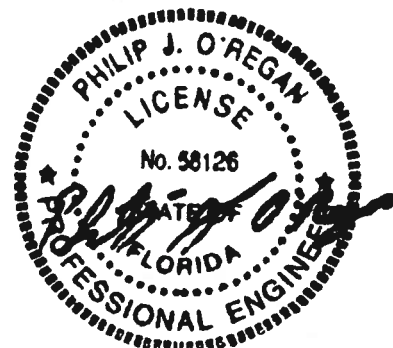
Job Number	Date	FBC - 2004 Chapter 16 and 23	Specification Quantity
T06101774	10/17/2006		39

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-2002, Section 2.2. Permanent files of the original Truss Specification Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Software - Online Plus)

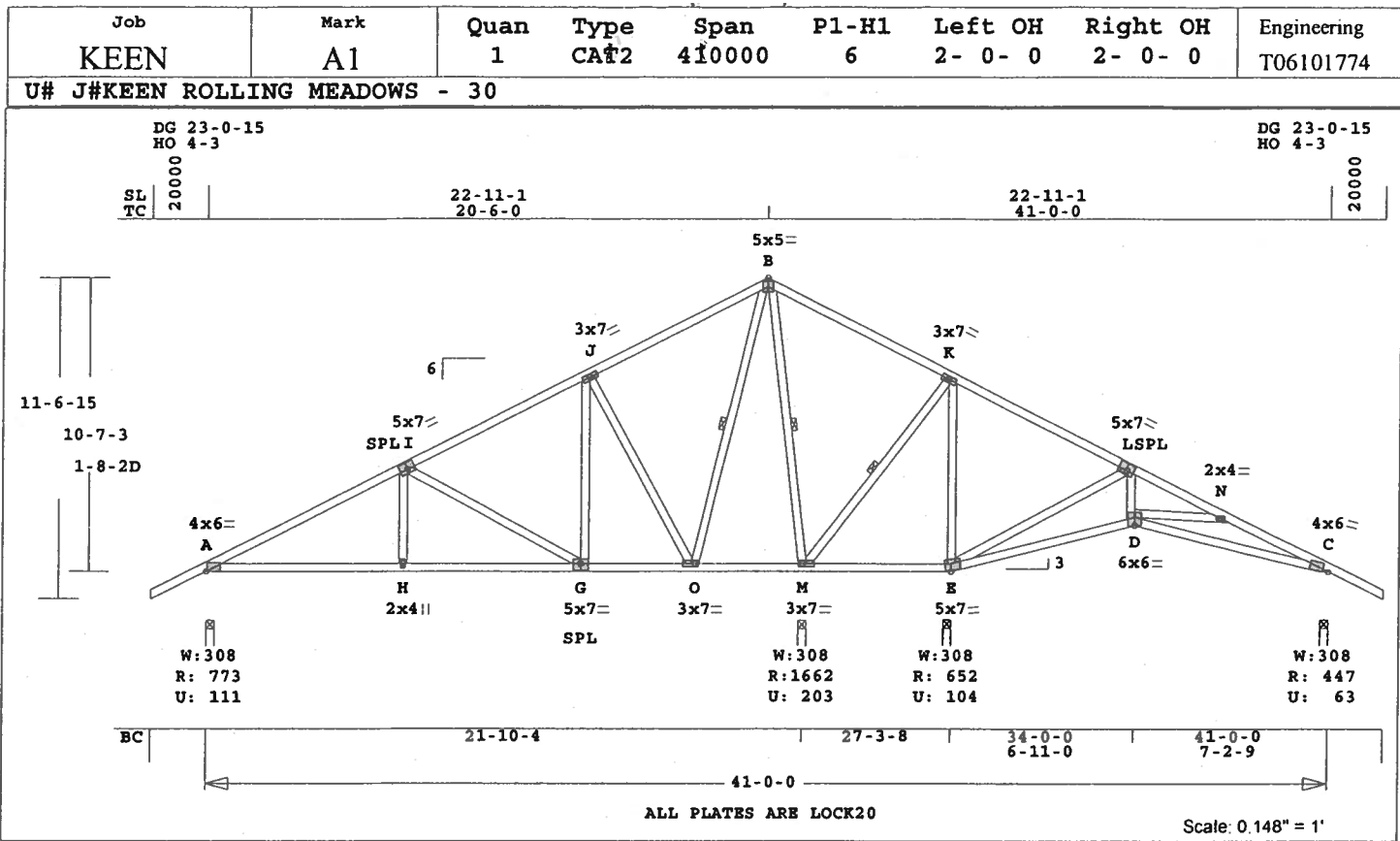
Notes: Refer to individual truss design drawings for special loading conditions.

Date	Mark	Date	Mark	Date	Mark	Date	Mark
1	10/17/06 A1	2	10/17/06 A2	3	10/17/06 A3	4	10/17/06 A4
5	10/17/06 B1	6	10/17/06 B2	7	10/17/06 B3	8	10/17/06 B4
9	10/17/06 B5	10	10/17/06 B6	11	10/17/06 B7	12	10/17/06 B8
13	10/17/06 B9	14	10/17/06 B10	15	10/17/06 B11	16	10/17/06 B12
17	10/17/06 B13	18	10/17/06 B14	19	10/17/06 B15	20	10/17/06 B16
21	10/17/06 C1	22	10/17/06 C2	23	10/17/06 CJ1	24	10/17/06 CJ2
25	10/17/06 D1	26	10/17/06 D2	27	10/17/06 J1	28	10/17/06 J2
29	10/17/06 J3	30	10/17/06 J4	31	10/17/06 V1	32	10/17/06 V2
33	10/17/06 V3	34	10/17/06 V4	35	10/17/06 V5	36	10/17/06 V6
37	10/17/06 V7	38	10/17/06 V8	39	10/17/06 V9		

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 10/17/2006



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 311.8 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----

TC	0.58	2x 4	SP-#2
BC	0.37	2x 4	SP-#2
WB	0.58	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	41- 0- 0
BC Cont.	0- 0- 0	41- 0- 0
WB 1 rows CLB on O -B		
WB 1 rows CLB on B -M		
WB 1 rows CLB on M -K		

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	774	111	3- 8	1- 8
			Hz =	-222
M	1663	203	3- 8	1-12
E	652	104	3- 8	1- 8
C	447	63	3- 8	1- 8
			Hz =	223

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -I	0.46		864 C	0.00	0.46
I -J	0.46		273 T	0.00	0.46
J -B	0.40		224 T	0.01	0.39
B -K	0.57		586 T	0.08	0.49
K -L	0.58		488 T	0.09	0.49
L -N	0.27		207 C	0.00	0.27
N -C	0.11		615 C	0.00	0.11
-----Bottom Chords-----					

A -H	0.37	783 T	0.08	0.29
H -G	0.37	783 T	0.08	0.29
G -O	0.18	239 T	0.02	0.16
O -M	0.15	388 T	0.00	0.15
M -E	0.15	423 C	0.00	0.15
E -D	0.22	244 T	0.02	0.20
D -C	0.23	575 T	0.09	0.14
-----Webs-----				
H -I	0.04	302 T		
I -G	0.46	657 C		
G -J	0.07	458 T		
J -O	0.58	730 C		
O -B	0.13	719 T	1 Br	
B -M	0.51	1429 C	1 Br	
M -K	0.04	209 T	1 Br	
K -E	0.14	247 C		
E -L	0.47	695 C		
L -D	0.06	403 T		
D -N	0.06	357 C		

TL Defl	-0.11"	in E -D	L/999
LL Defl	-0.05"	in A -H	L/999
Hz Disp	LL	DL	TL
Jt C	0.02"	0.02"	0.03"
Shear //	Grain	in A -I	0.26

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	-	RHS	20 Ga, Gross Area
Jt Type	Plt Size	X	Y JSI
A	LOCK	4.0x 6.0	Ctr 0.1 0.74
I	LOCK	5.0x 7.0	0.2 0.5 0.79
J	LOCK	3.0x 7.0	Ctr Ctr 0.52
B	LOCK	5.0x 5.0	Ctr Ctr 0.85
K	LOCK	3.0x 7.0	Ctr Ctr 0.48
L	LOCK	5.0x 7.0	0.2 0.5 0.79
N	LOCK	2.0x 4.0	Ctr Ctr 0.52
C	LOCK	4.0x 6.0	Ctr Ctr 0.92
H	LOCK	2.0x 4.0	Ctr Ctr 0.48
G	LOCK	5.0x 7.0	Ctr-0.5 0.80
O	LOCK	3.0x 7.0	Ctr Ctr 0.47
M	LOCK	3.0x 7.0	Ctr Ctr 0.47
E	LOCK	5.0x 7.0	0.6 3.2 0.86
D	LOCK	6.0x 6.0	Ctr-0.6 0.71

REVIEWED BY:  
Robbins Engineering, Inc.

PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

#### NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-  
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

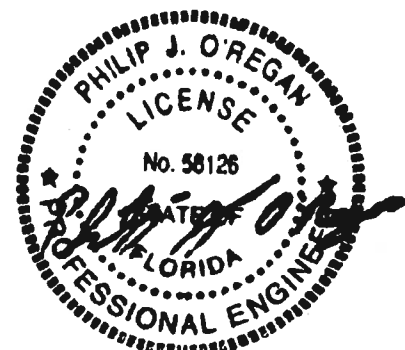
TC Dead Load: 5.0 psf

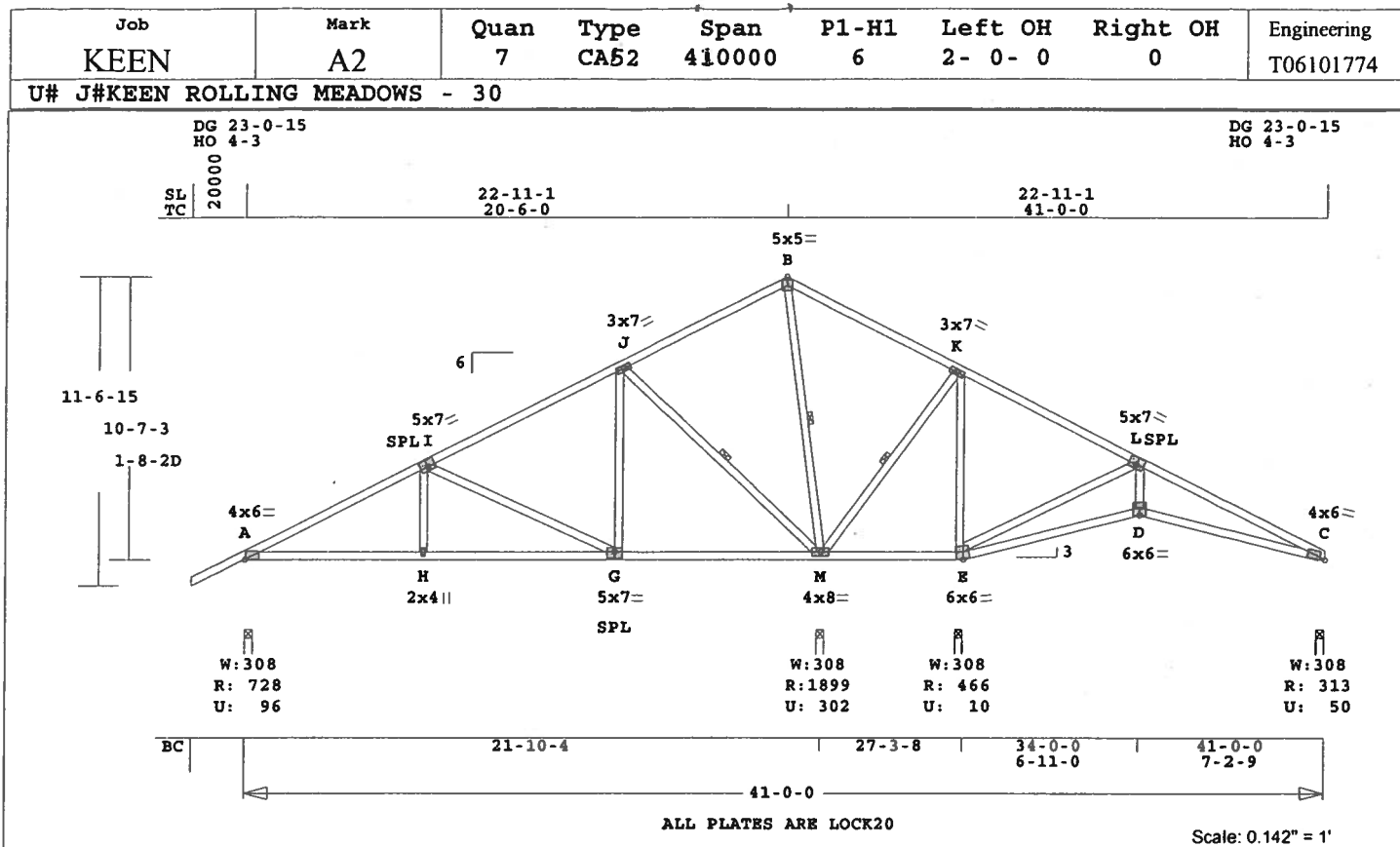
BC Dead Load: 5.0 psf

Max comp. force 1429 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™  
 Online Plus -- Version 20.0.001  
 RUN DATE: 17-OCT-06

CSI -Size- ---Lumber---  
 TC 0.56 2x 4 SP-#2  
 BC 0.37 2x 4 SP-#2  
 WB 0.57 2x 4 SP-#2

Brace truss as follows:  
 O.C. From To  
 TC Cont. 0- 0- 0 41- 0- 0  
 BC Cont. 0- 0- 0 41- 0- 0  
 WB 1 rows CLB on J -M  
 WB 1 rows CLB on B -M  
 WB 1 rows CLB on M -K  
 Attach CLB with (2)-10d nails  
 at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	728	97	3- 8	1- 8
			Hz =	-222
M	1900	303	3- 8	2- 0
E	466	11	3- 8	1- 8
C	314	51	3- 8	1- 8
			Hz =	223

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A -I	0.48	802	C	0.00	0.48
I -J	0.48	188	T	0.00	0.48
J -B	0.53	555	T	0.07	0.46
B -K	0.56	694	T	0.10	0.46
K -L	0.55	499	T	0.09	0.46

APPROX. TRUSS WEIGHT: 287.2 LBS  
 PO Box 280055  
 Tampa, FL 33682

L	-C	0.45	281	C	0.00	0.45
-----Bottom Chords-----						
A -H	0.34	729	T	0.07	0.27	
H -G	0.37	729	T	0.07	0.30	
G -M	0.31	212	T	0.01	0.30	
M -E	0.29	445	C	0.00	0.29	
E -D	0.30	274	T	0.04	0.26	
D -C	0.29	280	T	0.04	0.25	
-----Webs-----						
H -I	0.04	292	T			
I -G	0.56	691	C			
G -J	0.08	522	T			
J -M	0.29	827	C			1 Br
B -M	0.33	928	C			1 Br
M -K	0.07	354	T			1 Br
E -K	0.09	153	C			
E -L	0.57	793	C			
D -L	0.05	365	T			

TL Defl -0.15" in D -C L/999  
 LL Defl -0.07" in D -C L/999  
 Hz Disp LL DL TL  
 Jt C 0.02" 0.02" 0.04"  
 Shear // Grain in L -C 0.26

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.

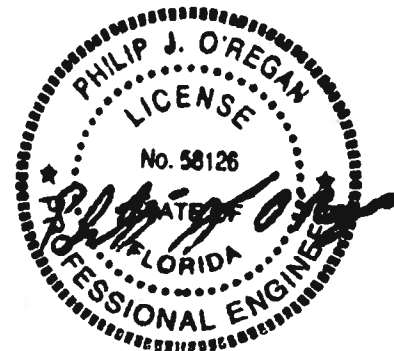
Plate	-	LOCK	20	Ga,	Gross	Area
Plate	-	RHS	20	Ga,	Gross <td>Area</td>	Area
Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	4.0x	6.0	Ctr	0.1	0.74
I	LOCK	5.0x	7.0	-0.2	0.5	0.79
J	LOCK	3.0x	7.0	Ctr	Ctr	0.45
B	LOCK	5.0x	5.0	Ctr	Ctr	0.72
K	LOCK	3.0x	7.0	Ctr	Ctr	0.48
L	LOCK	5.0x	7.0	-0.2	0.5	0.79
C	LOCK	4.0x	6.0	Ctr	Ctr	0.92
H	LOCK	2.0x	4.0	Ctr	Ctr	0.48
G	LOCK	5.0x	7.0	Ctr	-0.5	0.80
M	LOCK	4.0x	8.0	Ctr	Ctr	0.47
E	LOCK	6.0x	6.0	-0.4	3.3	0.71
D	LOCK	6.0x	6.0	Ctr	-0.6	0.69

REVIEWED BY:  
 Robbins Engineering, Inc.

REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

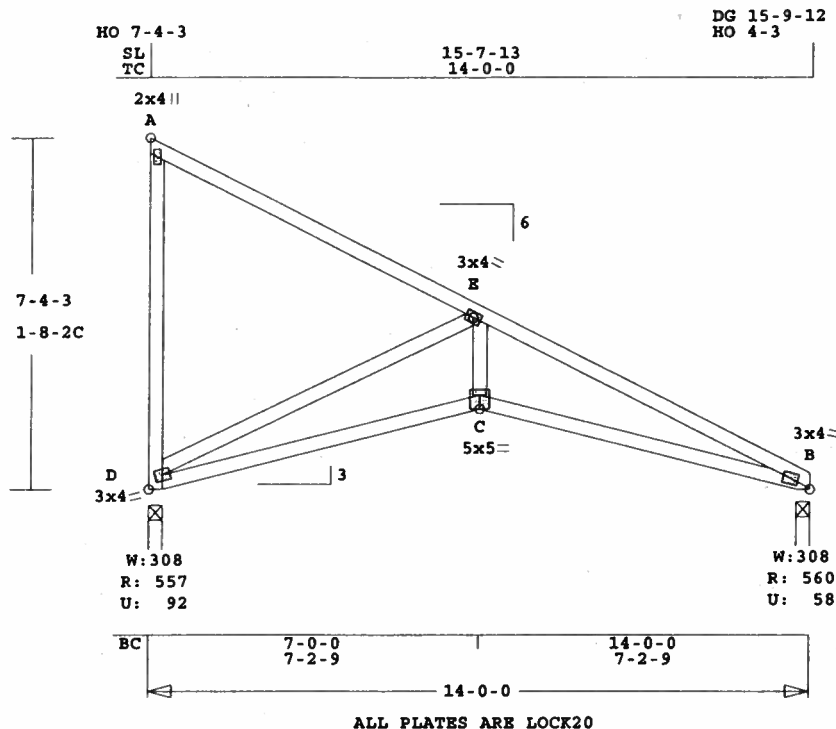
NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 OH Loading  
 Soffit psf 2.0  
 Design checked for 10 psf non-  
 concurrent LL on BC.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 928 Lbs  
 Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
 License #: 58126  
 Address: P.O. Box 280055, Tampa, FL 33682





Job <b>KEEN</b>	Mark <b>A4</b>	Quan <b>2</b>	Type <b>MON3</b>	Span <b>140000</b>	Pl-H1 <b>6</b>	Left OH <b>0</b>	Right OH <b>0</b>	Engineering <b>T06101774</b>
U# J#KEEN ROLLING MEADOWS - 30								



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 87.0 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI	Size	Lumber
TC	0.52 2x 4	SP-#2
BC	0.39 2x 4	SP-#2
WB	0.98 2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 0- 0
BC Cont.	0- 0- 0	14- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
D	557	92	3- 8	1- 8
			Hz =	-248
B	560	59	3- 8	1- 8
			Hz =	100

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -E	0.51		116 C	0.00	0.51
E -B	0.52		1319 C	0.01	0.51

-----Bottom Chords-----					
D -C	0.39	1233 T	0.12	0.27	
C -B	0.35	1234 T	0.20	0.15	
-----Webs-----					
D -A	0.32	176 T	WindLd		
D -E	0.98	1347 C			
C -E	0.13	749 T			

TL Defl	LL Defl	Hz Disp	LL	DL	TL
-0.20"	-0.09"				
		Jt B	0.04"	0.04"	0.08"
		Shear //	Grain	in A -E	0.28

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.38  
E LOCK 3.0x 4.0 Ctr Ctr 0.74  
B LOCK 3.0x 4.0 Ctr Ctr 0.79  
D LOCK 3.0x 4.0 Ctr Ctr 0.95  
C LOCK 5.0x 5.0 Ctr-1.1 0.43

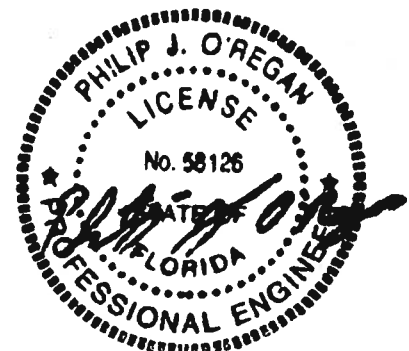
REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

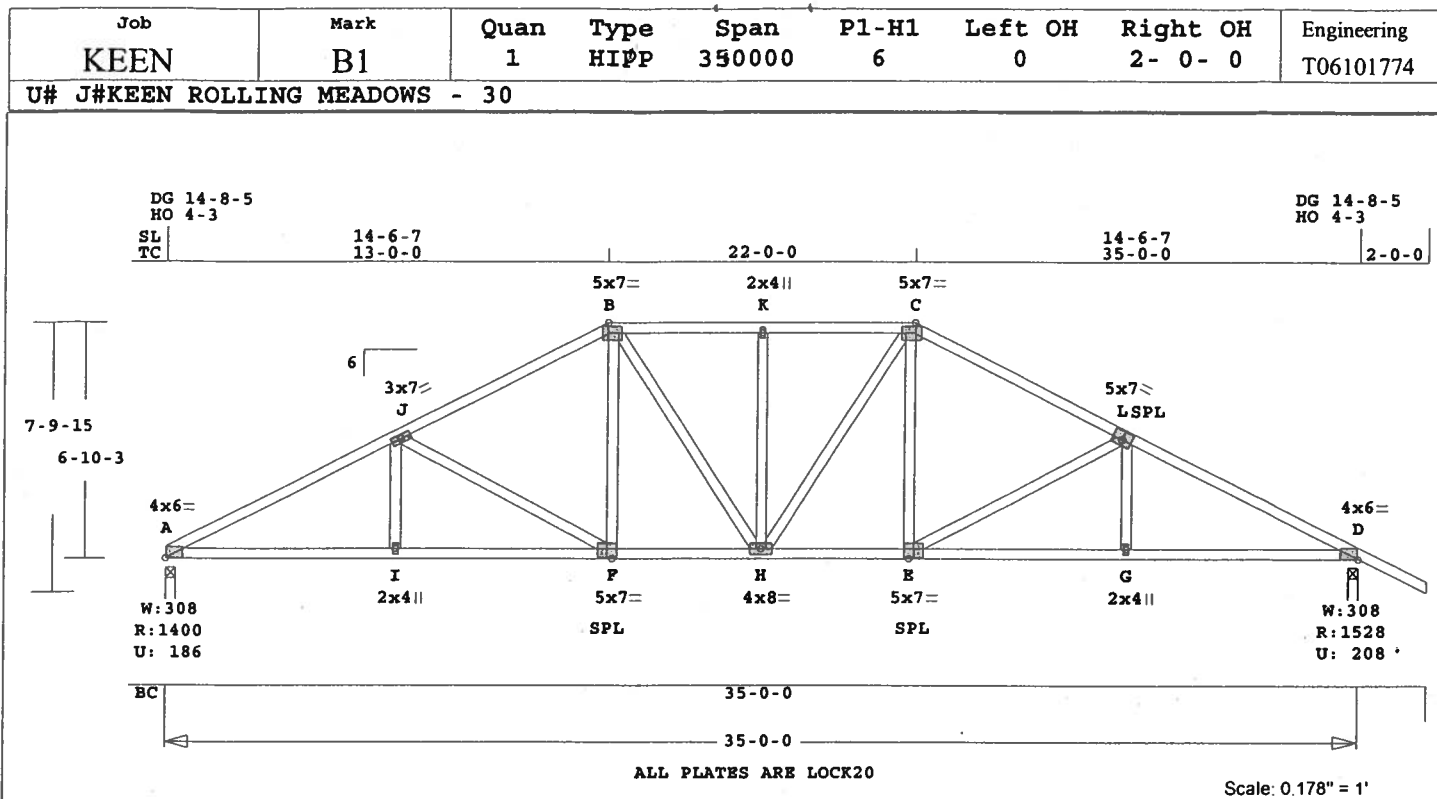
REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1347 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 244.5 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----

TC	0.46	2x 4	SP-#2
BC	0.48	2x 4	SP-#2
WB	0.36	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	35- 0- 0
BC Cont.	0- 0- 0	35- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1400	187	3- 8	1-10
			Hz =	-136
D	1528	209	3- 8	1-13
			Hz =	137

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -J	0.46	2507 C	0.14	0.32
J -B	0.44	1951 C	0.12	0.32
B -K	0.21	1855 C	0.12	0.09
K -C	0.21	1855 C	0.12	0.09
C -L	0.44	1951 C	0.12	0.32
L -D	0.46	2507 C	0.14	0.32
-----Bottom Chords-----				
A -I	0.48	2246 T	0.37	0.11
I -F	0.47	2246 T	0.37	0.10
F -H	0.38	1735 T	0.29	0.09

H -E	0.38	1735 T	0.29	0.09	
E -G	0.47	2246 T	0.37	0.10	
G -D	0.48	2246 T	0.37	0.11	
-----Webs-----					
I -J	0.04	264 T			
J -F	0.36	572 C			
F -B	0.07	424 T			
B -H	0.08	216 T			
H -K	0.15	288 C			
H -C	0.08	216 T			
E -C	0.07	424 T			
E -L	0.36	572 C			
G -L	0.04	264 T			

TL Defl -0.24" in E -G L/999  
LL Defl -0.12" in H -E L/999  
Shear // Grain in A -J 0.22

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga	Gross Area
Plate - RHS	20 Ga <td>Gross Area</td> <td></td>	Gross Area	
Jt	Type	Plt Size	X Y JSI
A	LOCK	4.0x 6.0	Ctr 0.1 0.69
J	LOCK	3.0x 7.0	Ctr Ctr 0.40
B	LOCK	5.0x 7.0	0.5-0.1 0.93
K	LOCK	2.0x 4.0	Ctr Ctr 0.44
C	LOCK	5.0x 7.0	0.5-0.1 0.93
L	LOCK	5.0x 7.0	0.2 0.5 0.73
D	LOCK	4.0x 6.0	Ctr 0.1 0.69
I	LOCK	2.0x 4.0	Ctr Ctr 0.44
F	LOCK	5.0x 7.0	Ctr-0.5 0.74
H	LOCK	4.0x 8.0	Ctr Ctr 0.45
E	LOCK	5.0x 7.0	Ctr-0.5 0.74
G	LOCK	2.0x 4.0	Ctr Ctr 0.44

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*  
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

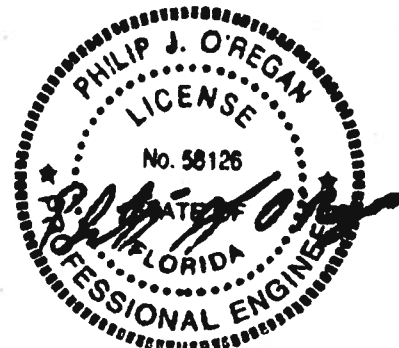
TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

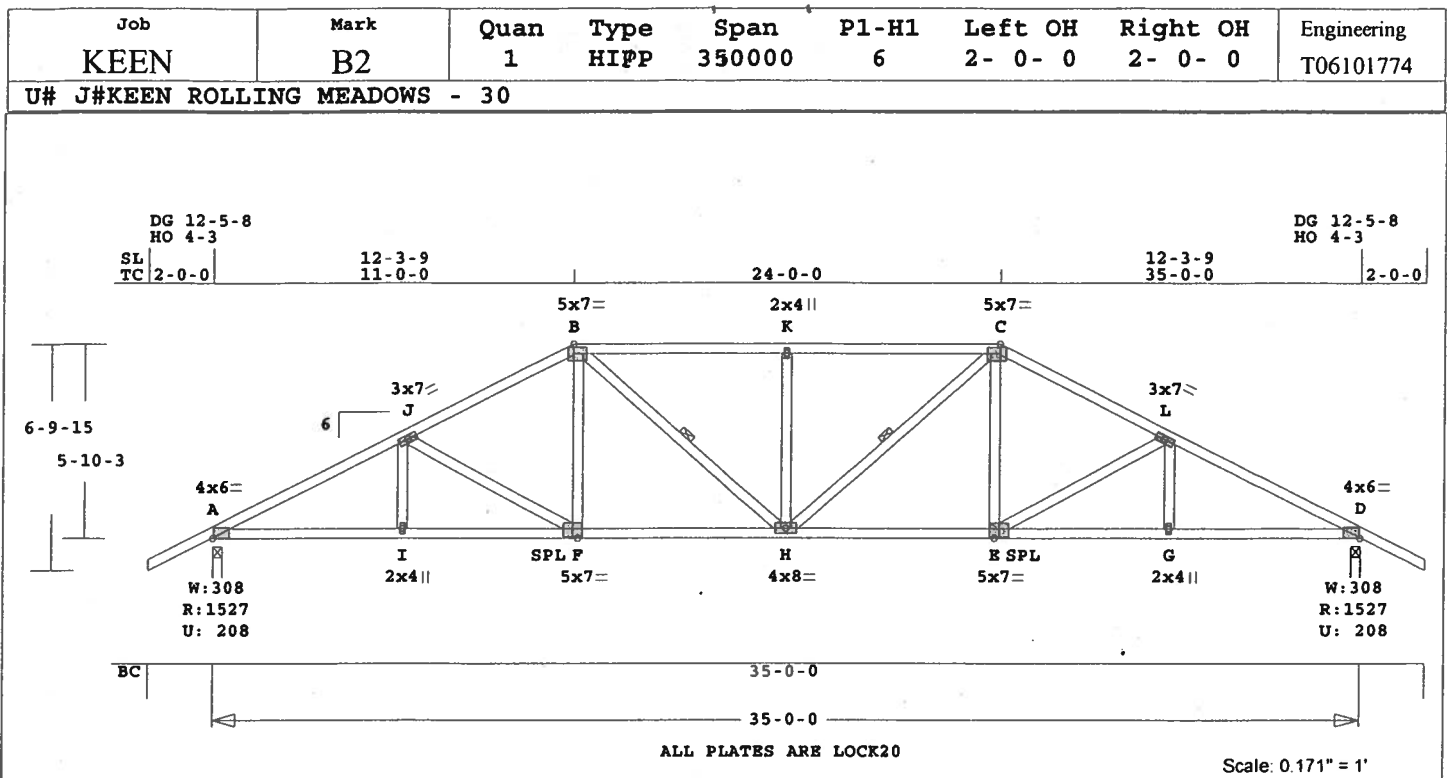
Max comp. force 2507 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682







Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 238.2 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----

TC	0.48	2x 4	SP-#2
BC	0.48	2x 4	SP-#2
WB	0.19	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	35- 0- 0
BC Cont.	0- 0- 0	35- 0- 0
WB 1 rows CLB on B -H		
WB 1 rows CLB on H -C		

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1528	209	3- 8	1-13
			Hx =	-114
D	1528	209	3- 8	1-13
			Hx =	115

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -J	0.36	2548	C	0.14	0.22
J -B	0.34	2122	C	0.12	0.22
B -K	0.48	2224	C	0.03	0.45
K -C	0.48	2224	C	0.03	0.45
C -L	0.34	2122	C	0.12	0.22
L -D	0.36	2548	C	0.14	0.22
-----Bottom Chords-----					

A -I	0.45	2277	T	0.38	0.07	
I -F	0.48	2277	T	0.38	0.10	
F -H	0.43	1895	T	0.31	0.12	
H -E	0.43	1895	T	0.31	0.12	
E -G	0.48	2277	T	0.38	0.10	
G -D	0.45	2277	T	0.38	0.07	
-----Webs-----						
I -J	0.03	201	T			
J -F	0.19	427	C			
F -B	0.06	387	T			
B -H	0.08	433	T			1 Br
H -K	0.16	436	C			
K -C	0.08	433	T			1 Br
C -E	0.06	387	T			
E -L	0.19	427	C			
L -G	0.03	201	T			

TL Defl -0.28" in F -H L/999  
LL Defl -0.13" in F -H L/999  
Shear // Grain in B -K 0.28

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	4.0x	6.0	Ctr	0.1	0.69
J	LOCK	3.0x	7.0	Ctr	Ctr	0.40
B	LOCK	5.0x	7.0-0.5-0.1	0.93		
K	LOCK	2.0x	4.0	Ctr	Ctr	0.44
C	LOCK	5.0x	7.0	0.5-0.1	0.93	
L	LOCK	3.0x	7.0	Ctr	Ctr	0.40
D	LOCK	4.0x	6.0	Ctr	0.1	0.69
I	LOCK	2.0x	4.0	Ctr	Ctr	0.44
F	LOCK	5.0x	7.0	Ctr-0.5	0.74	
H	LOCK	4.0x	8.0	Ctr	Ctr	0.42
E	LOCK	5.0x	7.0	Ctr-0.5	0.74	
G	LOCK	2.0x	4.0	Ctr	Ctr	0.44

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

#### NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.

Analysis Conforms To:  
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-  
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

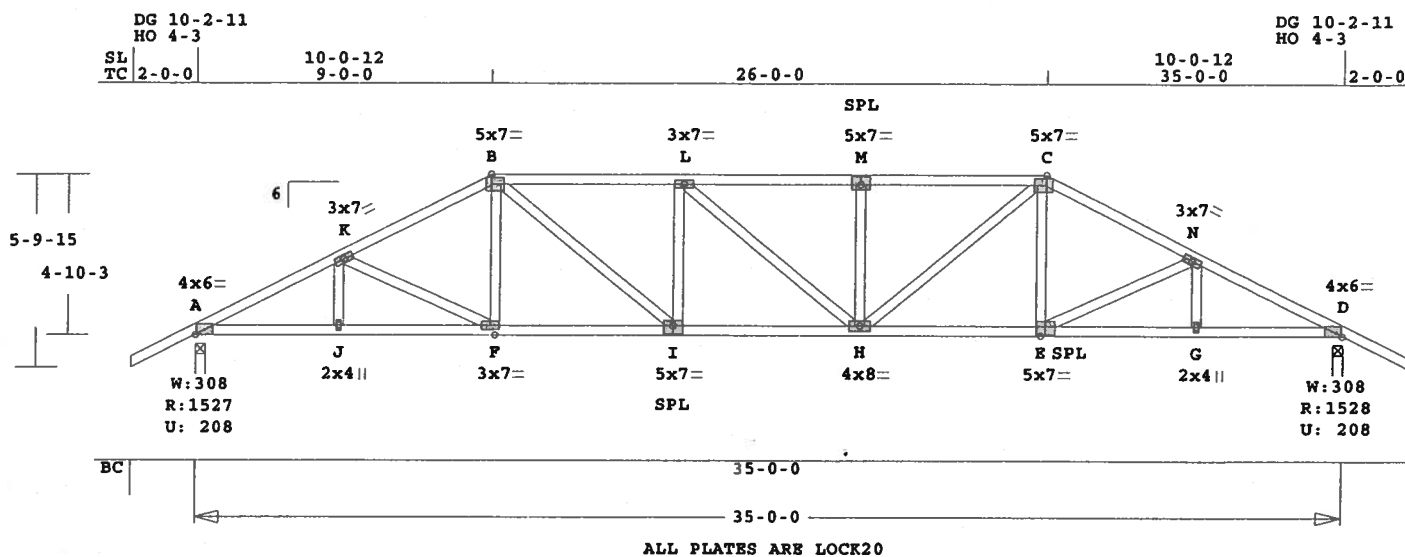
Max comp. force 2548 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job <b>KEEN</b>	Mark <b>B3</b>	Quan <b>1</b>	Type <b>HIP</b>	Span <b>350000</b>	P1-H1 <b>6</b>	Left OH <b>2- 0- 0</b>	Right OH <b>2- 0- 0</b>	Engineering <b>T06101774</b>
U# J#KEEN ROLLING MEADOWS - 30								



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 241.9 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----  
TC 0.32 2x 4 SP-#2  
BC 0.50 2x 4 SP-#2  
WB 0.17 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	35- 0- 0
BC Cont.	0- 0- 0	35- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1528	209	3- 8	1-13
			Hz =	-93
D	1528	209	3- 8	1-13
			Hz =	94

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -K	0.28	2601	C	0.14	0.14
K -B	0.29	2286	C	0.13	0.16
B -L	0.32	2618	C	0.07	0.25
L -M	0.32	2618	C	0.04	0.28
M -C	0.32	2618	C	0.04	0.28
C -N	0.29	2286	C	0.13	0.16
N -D	0.28	2601	C	0.14	0.14
-----Bottom Chords-----					
A -J	0.49	2318	T	0.38	0.11
J -F	0.47	2318	T	0.38	0.09
F -I	0.43	2047	T	0.34	0.09

I -H	0.50	2619	T	0.43	0.07
H -E	0.43	2047	T	0.34	0.09
E -G	0.47	2318	T	0.38	0.09
G -D	0.49	2318	T	0.38	0.11

-----Webs-----

J -K	0.02	143	T		
K -F	0.09	293	C		
F -B	0.04	310	T		
B -I	0.17	733	T		
I -L	0.09	348	C		
L -H	0.02	47	C		
H -M	0.09	348	C		
M -C	0.17	731	T		
E -C	0.04	310	T		
E -N	0.09	293	C		
G -N	0.02	142	T		

TL Defl -0.34" in I -H L/999  
LL Defl -0.16" in I -H L/999  
Shear // Grain in B -L 0.23

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga	Gross Area
Plate - LOCK 20 Ga, Gross Area			
Plate - RHS 20 Ga, Gross Area			
Jt Type	Plt Size	X	Y
			JSI
A LOCK	4.0x 6.0	Ctr	0.1 0.69
K LOCK	3.0x 7.0	Ctr	Ctr 0.40
B LOCK	5.0x 7.0	0.5-0.1	0.93
L LOCK	3.0x 7.0	Ctr	Ctr 0.43
M LOCK	5.0x 7.0	Ctr	0.5 0.74
C LOCK	5.0x 7.0	0.5-0.1	0.93
N LOCK	3.0x 7.0	Ctr	Ctr 0.40
D LOCK	4.0x 6.0	Ctr	0.1 0.69
J LOCK	2.0x 4.0	Ctr	Ctr 0.44
F LOCK	3.0x 7.0	Ctr	Ctr 0.45
I LOCK	5.0x 7.0	Ctr	0.5 0.74
H LOCK	4.0x 8.0	Ctr	Ctr 0.43
E LOCK	5.0x 7.0	Ctr	0.5 0.74
G LOCK	2.0x 4.0	Ctr	Ctr 0.44

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

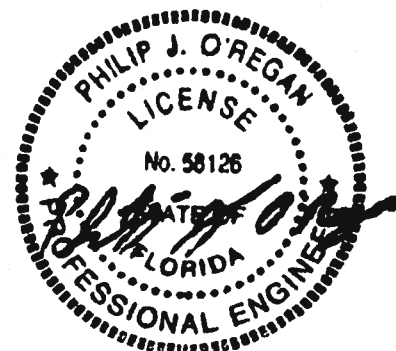
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

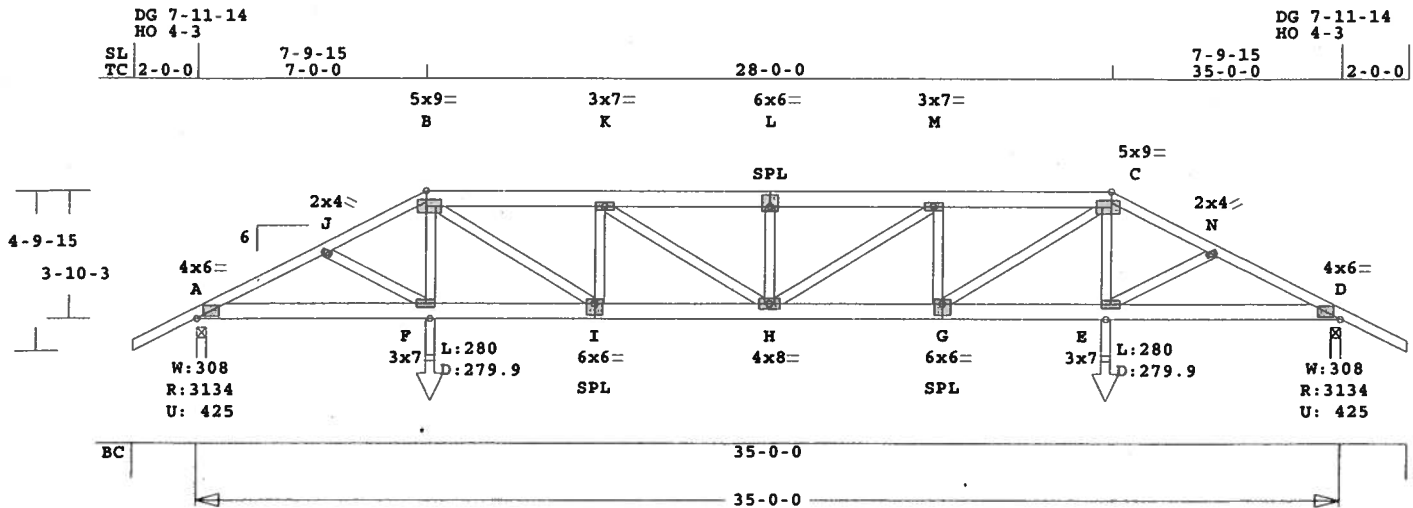
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 2618 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job <b>KEEN</b>	Mark <b>B4</b>	Quan <b>1*2P</b>	Type <b>HIFP</b>	Span <b>35'000</b>	Pl-H1 <b>6</b>	Left OH <b>2- 0- 0</b>	Right OH <b>2- 0- 0</b>	Engineering <b>T06101774</b>
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U# J#KEEN ROLLING MEADOWS - 30



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 281.3 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06  
\*\*\*\*\*  
\* 2-Ply Truss \*  
\*\*\*\*\*

CSI -Size- ----Lumber-----  
TC 0.34 2x 4 SP-#2  
EX B -L 2x 6 SP-#2  
EX L -C 2x 6 SP-#2  
BC 0.59 2x 6 SP-#2  
WB 0.26 2x 4 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 35- 0- 0  
BC Cont. 0- 0- 0 35- 0- 0

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.00 Fc=1.00 Ft=1.00  
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
plf - Live Dead From To  
TC V 40 20 0.0' 35.0'  
BC V 0 20 0.0' 35.0'  
TC V 50 25 7.0' 28.0'  
BC V 0 25 7.1' 27.9'  
BC V 280 280 7.1' CL-LB  
BC V 280 280 27.9' CL-LB

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd  
Lbs Lbs In-Sx In-Sx  
A 3134 425 3- 8 1-14  
Hz = -70  
D 3134 425 3- 8 1-14  
Hz = 71

Membr CSI P Lbs Ax1-CSI-Bnd  
-----Top Chords-----  
A -J 0.23 6121 C 0.18 0.05  
J -B 0.34 6043 C 0.07 0.27  
B -K 0.25 7839 C 0.06 0.19  
K -L 0.26 8480 C 0.18 0.08  
L -M 0.26 8480 C 0.18 0.08  
M -C 0.25 7840 C 0.06 0.19  
C -N 0.34 6043 C 0.07 0.27  
N -D 0.23 6121 C 0.18 0.05

-----Bottom Chords-----  
A -F 0.44 5448 T 0.36 0.08  
F -I 0.44 5429 T 0.36 0.08  
I -H 0.59 7839 T 0.52 0.07  
H -G 0.59 7839 T 0.52 0.07  
G -E 0.44 5429 T 0.36 0.08  
E -D 0.44 5449 T 0.36 0.08

-----Webs-----  
J -F 0.01 159 T  
F -B 0.06 734 T  
B -I 0.26 2834 T  
I -K 0.06 1214 C  
K -H 0.07 765 T  
H -L 0.03 645 C  
H -M 0.07 765 T  
G -M 0.06 1214 C  
G -C 0.26 2834 T  
E -C 0.06 734 T  
E -N 0.01 159 T

TL Defl -0.41" in H -G L/999  
LL Defl -0.20" in H -G L/999  
Shear // Grain in B -K 0.17

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 4.0x 6.0 Ctr Ctr 0.72  
J LOCK 2.0x 4.0 Ctr Ctr 0.44  
B LOCK 5.0x 9.0-0.5 Ctr 1.00  
K LOCK 3.0x 7.0 Ctr Ctr 0.42  
L LOCK 6.0x 6.0 Ctr 1.2 0.59  
M LOCK 3.0x 7.0 Ctr Ctr 0.42  
C LOCK 5.0x 9.0 0.5 Ctr 1.00  
N LOCK 2.0x 4.0 Ctr Ctr 0.44  
D LOCK 4.0x 6.0 Ctr Ctr 0.72  
F LOCK 3.0x 7.0 Ctr Ctr 0.44  
I LOCK 6.0x 6.0 Ctr-1.2 0.68  
H LOCK 4.0x 8.0 Ctr Ctr 0.44  
G LOCK 6.0x 6.0 Ctr-1.2 0.68  
E LOCK 3.0x 7.0 Ctr Ctr 0.44

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

FBC2004  
Girder Step Down Hip  
Framing King Jacks  
Jack Open Faced  
Setback 7- 0- 0  
2 COMPLETE TRUSSES REQUIRED.  
Fasten together in staggered  
pattern. (1/2" bolts -OR-  
SDS3 screws -OR- 10d nails  
as each layer is applied.)  
-----Spacing (In)-----  
Rows Nails Screws Bolts  
TC 1 12 24 0  
BC 2 12 24 0  
WB 1 8 8

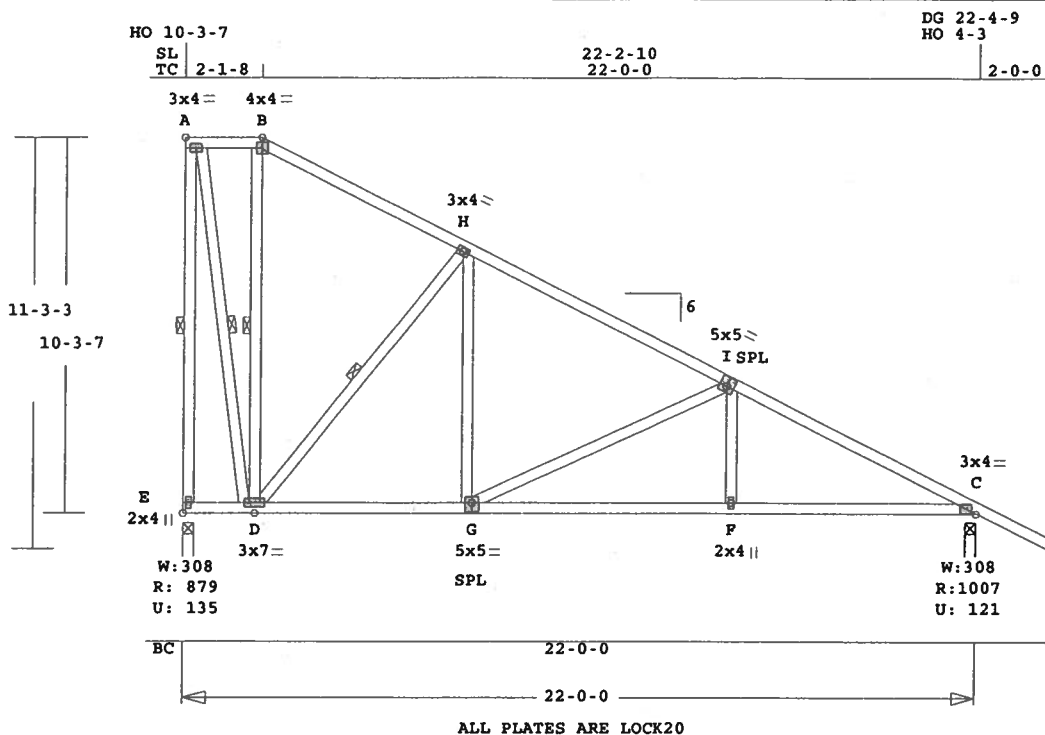
Plus clusters of nails where  
shown.  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Prevent truss rotation at all  
bearing locations.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 8480 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job <b>KEEN</b>	Mark <b>B5</b>	Quan <b>1</b>	Type <b>HHIP</b>	Span <b>22'0000</b>	Pl-H1 <b>6</b>	Left OH <b>0</b>	Right OH <b>2- 0- 0</b>	Engineering <b>T06101774</b>
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U# J#KEEN ROLLING MEADOWS - 30



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 197.7 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

TC	0.47	2x 4	SP-#2
BC	0.40	2x 4	SP-#2
WB	0.58	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	22- 0- 0
BC Cont.	0- 0- 0	22- 0- 0

WB 1 rows CLB on E -A  
WB 1 rows CLB on A -D  
WB 1 rows CLB on D -B  
WB 1 rows CLB on D -H  
Attach CLB with (2)-10d nails  
at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
E	880	135	3- 8	1- 8
			Hz =	-356
C	1008	122	3- 8	1- 8
			Hz =	165

Membr	CSI	P	Lbs	Axl	CSI	Bnd
-----Top Chords-----						
A -B	0.13		288 T	0.03	0.10	
B -H	0.45		245 C	0.00	0.45	
H -I	0.46		730 C	0.00	0.46	

I -C	0.47	1402 C	0.01	0.46
-----Bottom Chords-----				
E -D	0.09	281 T	0.00	0.09
D -G	0.31	653 T	0.06	0.25
G -F	0.40	1264 T	0.13	0.27
F -C	0.40	1264 T	0.13	0.27
-----Webs-----				
E -A	0.58	867 C	WindLd	1 Br
A -D	0.15	845 T		1 Br
D -B	0.05	151 C		1 Br
D -H	0.22	786 C		1 Br
G -H	0.07	496 T		
G -I	0.54	674 C		
F -I	0.04	293 T		

TL Defl -0.14" in G -F L/999  
LL Defl -0.06" in G -F L/999  
Shear // Grain in H -I 0.25

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

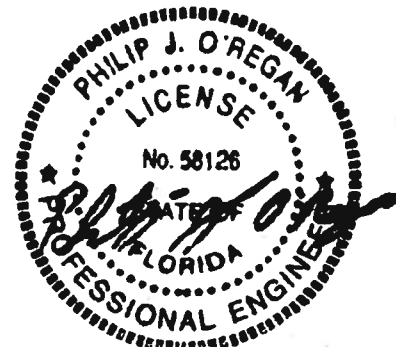
Plate	LOCK	20 Ga	Gross Area
Jt Type	Plt Size	X	Y
A LOCK	3.0x 4.0	Ctr	Ctr
B LOCK	4.0x 4.0	Ctr	Ctr
H LOCK	3.0x 4.0	Ctr	Ctr
I LOCK	5.0x 5.0	0.2	0.5
C LOCK	3.0x 4.0	Ctr	Ctr
E LOCK	2.0x 4.0	Ctr	Ctr
D LOCK	3.0x 7.0	Ctr	Ctr
G LOCK	5.0x 5.0	Ctr	-0.5
F LOCK	2.0x 4.0	Ctr	Ctr

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Provide connection to bearing  
for 356 Lbs Horiz Reaction  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1402 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682

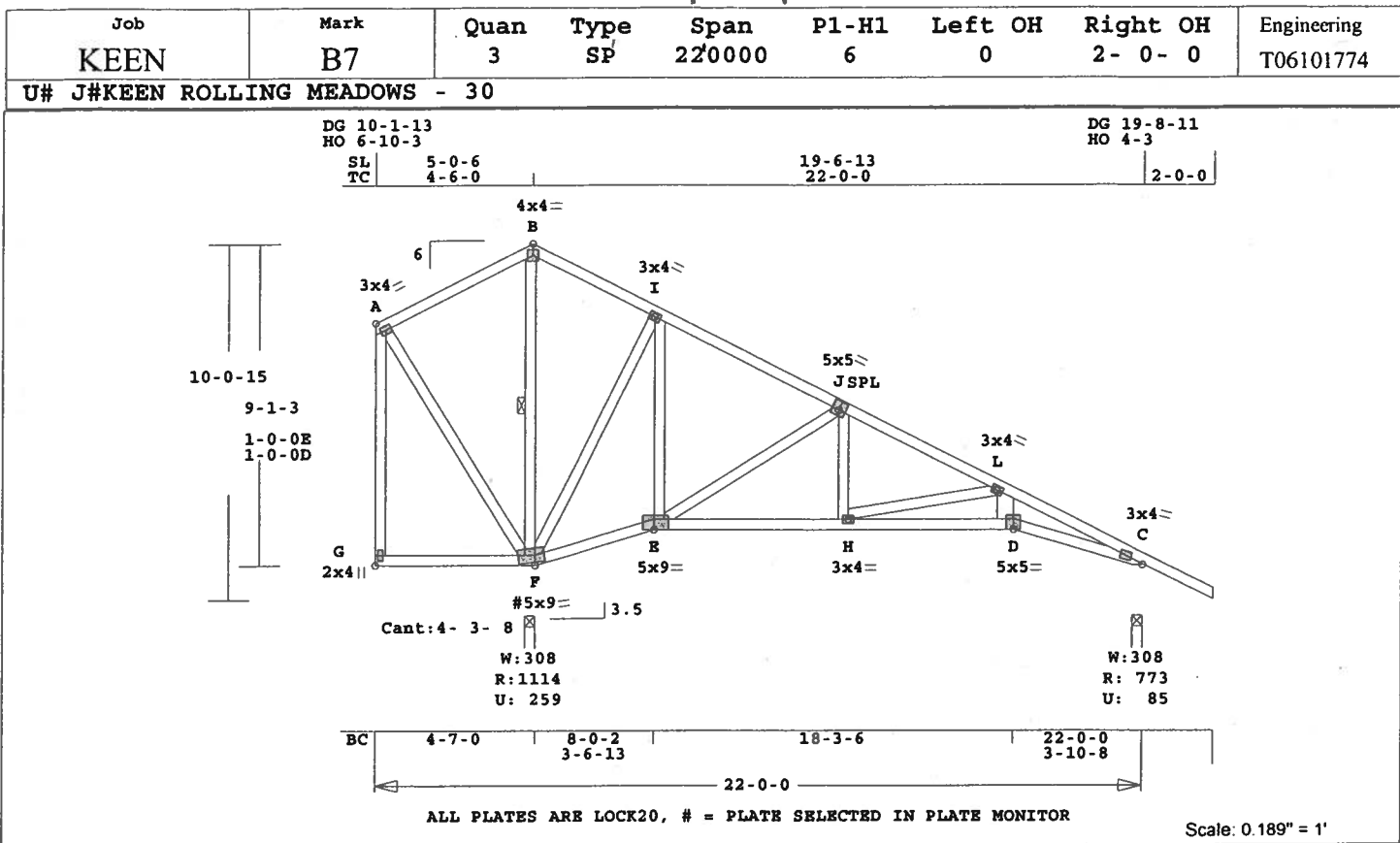


U# J#KEEN ROLLING MEADOWS - 30



Scale: 0.195" = 1'

Date Sealed: 10/17/2006



Online Plus -- Version 20.0.001  
 RUN DATE: 17-OCT-06

CSI -Size- ---Lumber---  
 TC 0.27 2x 4 SP-#2  
 BC 0.47 2x 4 SP-#2  
 WB 0.48 2x 4 SP-#2  
 EX D -L 2x 6 SP-#2

Brace truss as follows:  
 O.C. From To  
 TC Cont. 0- 0- 0 22- 0- 0  
 BC Cont. 0- 0- 0 22- 0- 0  
 WB 1 rows CLB on F -B  
 Attach CLB with (2)-10d nails  
 at each web.

Loading Live Dead (psf)  
 TC 20.0 10.0  
 BC 0.0 10.0  
 Total 20.0 20.0 40.0  
 Spacing 24.0"  
 Lumber Duration Factor 1.25  
 Plate Duration Factor 1.25  
 TC Fb=1.15 Fc=1.10 Ft=1.10  
 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)  
 Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
F	1114	259	3- 8	1- 8
			Hx =	-275
C	774	86	3- 8	1- 8
			Hx =	151

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.22	116	T	0.00	0.22
B -I	0.18	124	T	0.00	0.18
I -J	0.27	244	C	0.00	0.27
J -L	0.27	884	C	0.00	0.27
L -C	0.19	2067	C	0.11	0.08
-----Bottom Chords-----					
G -F	0.24	140	T	0.00	0.24
F -E	0.40	225	T	0.03	0.37

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 188.4 LBS

	E -H	0.21	804	T	0.13	0.08
H -D	0.36	1842	T	0.30	0.06	
D -C	0.47	1903	T	0.32	0.15	

-----Webs-----  
 G -A 0.25 106 T WindLd  
 A -F 0.13 315 T  
 F -B 0.08 342 C  
 F -I 0.48 632 C  
 E -I 0.08 440 T  
 E -J 0.32 693 C  
 H -J 0.05 374 T  
 H -L 0.29 1061 C  
 D -L 0.07 566 T

	TL Defl	-0.15"	in H -D	L/999
LL Defl	-0.07"	in H -D	L/999	
LL Cant	-0.02"	in G -F	L/999	
Hx Disp	LL	DL	TL	
Jt F	0.05"	0.05"	0.10"	
Shear //	Grain	in J -L	0.20	

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 3.0x 4.0 Ctr Ctr 0.71  
 B LOCK 4.0x 4.0 Ctr Ctr 0.66  
 I LOCK 3.0x 4.0 Ctr Ctr 0.60  
 J LOCK 5.0x 5.0 0.2 0.5 0.58  
 L LOCK 3.0x 4.0 Ctr Ctr 0.70  
 C LOCK 3.0x 4.0 Ctr Ctr 0.97  
 G LOCK 2.0x 4.0 Ctr Ctr 0.40  
 F# LOCK 5.0x 9.0-1.2 3.4 0.60  
 E LOCK 5.0x 9.0 0.5-1.0 0.65  
 H LOCK 3.0x 4.0 Ctr Ctr 0.72  
 D LOCK 5.0x 5.0 Ctr-1.0 0.73

# = Plate Monitor used

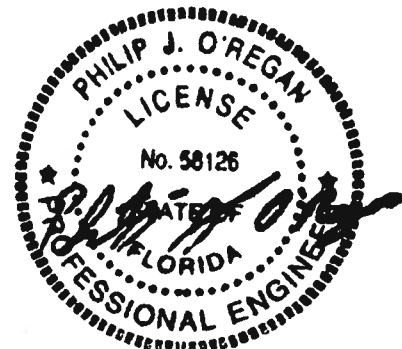
REVIEWED BY:  
 Robbins Engineering, Inc.  
 PO Box 280055  
 Tampa, FL 33682

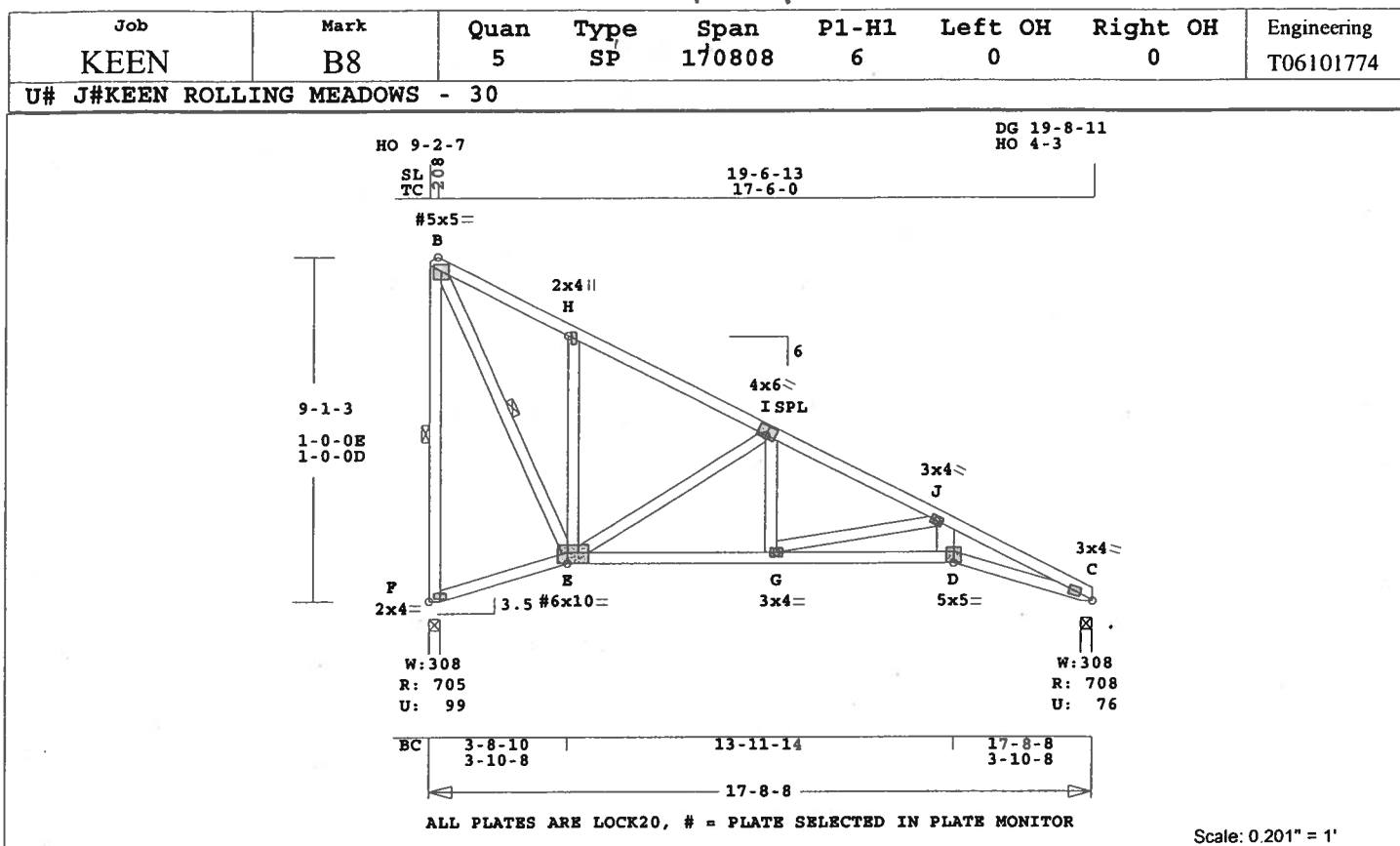
REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 OH Loading  
 Soffit psf 2.0  
 Design checked for 10 psf non-  
 concurrent LL on BC.  
 NOTE: USER MODIFIED PLATES  
 This design may have plates  
 selected through a plate  
 monitor.  
 Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 2067 Lbs  
 Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
 License #: 58126  
 Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 141.7 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----

TC	0.27	2x 4	SP-#2
BC	0.53	2x 4	SP-#2
WB	0.50	2x 4	SP-#2
EX D -J	2x 6	SP-#2	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	17- 8- 8
BC Cont.	0- 0- 0	17- 8- 8
WB 1 rows CLB on F -B		
WB 1 rows CLB on B -E		

Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
F	705	99	3- 8	1- 8
			Hz =	-312
C	708	76	3- 8	1- 8
			Hz =	135

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
B -H	0.22		417 C	0.04	0.18
H -I	0.27		407 C	0.00	0.27
I -J	0.27		1053 C	0.00	0.27
J -C	0.16		2326 C	0.10	0.06
-----Bottom Chords-----					

F -E	0.14	314 T	0.00	0.14
E -G	0.35	956 T	0.16	0.19
G -D	0.41	2070 T	0.34	0.07
D -C	0.53	2139 T	0.36	0.17
-----Webs-----				
F -B	0.50	649 C	WindLd	1 Br
B -E	0.15	824 T		1 Br
E -H	0.13	324 T		
E -I	0.32	694 C		
G -I	0.05	372 T		
G -J	0.31	1140 C		
D -J	0.08	630 T		

TL Defl	-0.16"	in G -D	L/999
LL Defl	-0.08"	in G -D	L/999
Hz Disp	LL	DL	TL
Jt C	0.05"	0.05"	0.11"
Shear // Grain	in I -J		0.20

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
B# LOCK 5.0x 5.0 Ctr-0.2 0.39  
H LOCK 2.0x 4.0 Ctr Ctr 0.38  
I LOCK 4.0x 6.0 0.5 0.9 0.57  
J LOCK 3.0x 4.0 Ctr Ctr 0.64  
C LOCK 3.0x 4.0 Ctr Ctr 0.89  
F LOCK 2.0x 4.0 Ctr Ctr 0.46  
E# LOCK 6.0x10.0 Ctr-0.4 0.48  
G LOCK 3.0x 4.0 Ctr Ctr 0.69  
D LOCK 5.0x 5.0 Ctr-1.0 0.73

# = Plate Monitor used

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

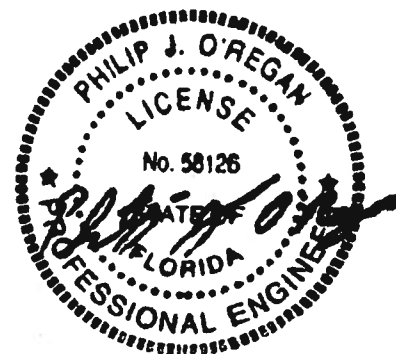
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

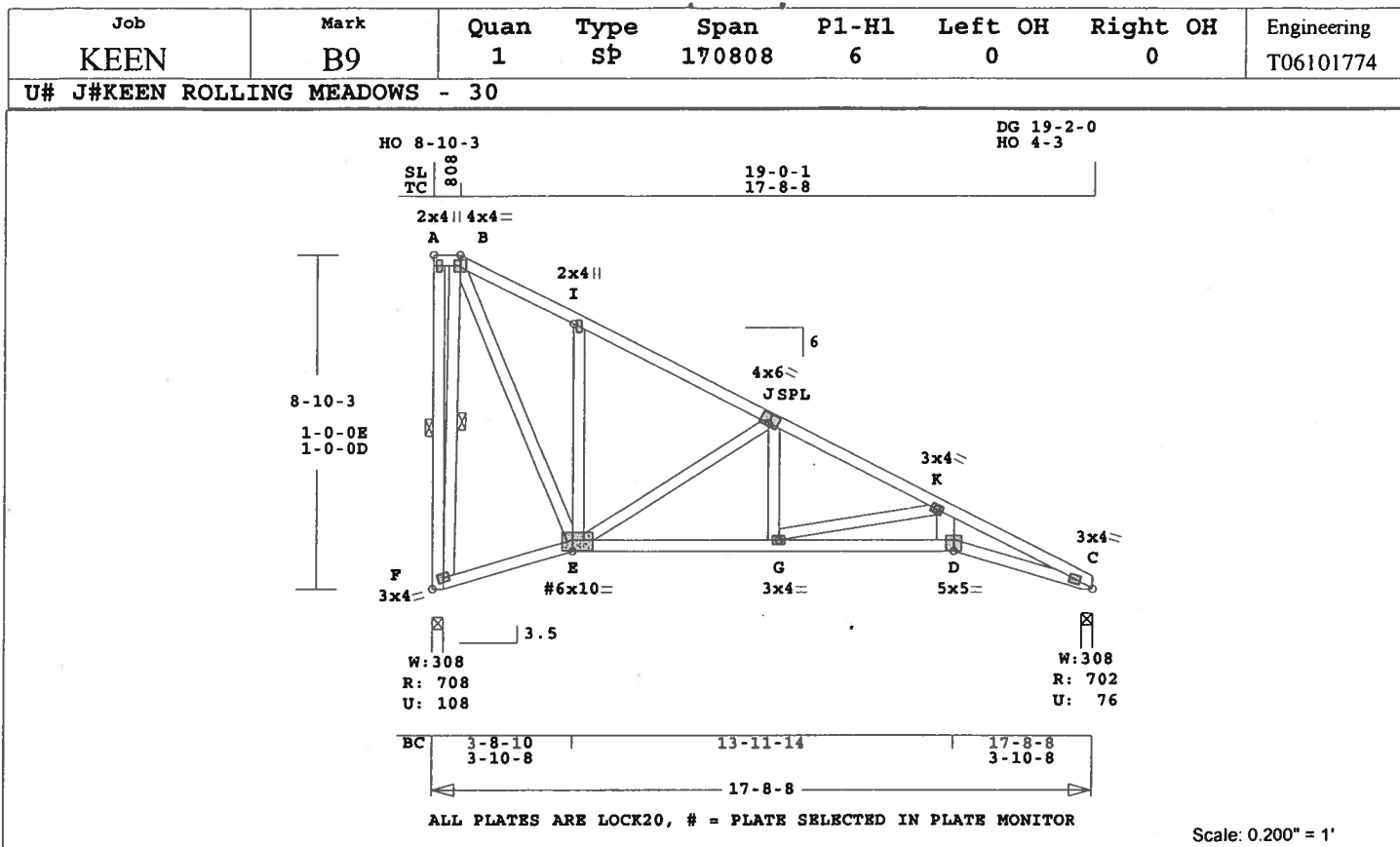
Provide connection to bearing  
for 313 Lbs Horiz Reaction  
Design checked for 10 psf non-  
concurrent LL on BC.

NOTE: USER MODIFIED PLATES  
This design may have plates  
selected through a plate  
monitor.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 2326 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 156.0 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ---Lumber---  
TC 0.28 2x 4 SP-#2  
BC 0.52 2x 4 SP-#2  
WB 0.43 2x 4 SP-#2  
EX D -K 2x 6 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 17- 8- 8  
BC Cont. 0- 0- 0 17- 8- 8  
WB 1 rows CLB on F -A  
WB 1 rows CLB on F -B  
Attach CLB with (2)-10d nails  
at each web.

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
F	708	108	3- 8	1- 8
			Hx =	-304
C	702	77	3- 8	1- 8
			Hx =	131

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.06		185 T	0.00	0.06
B -I	0.19		395 C	0.04	0.15
I -J	0.28		389 C	0.00	0.28
J -K	0.28		1038 C	0.00	0.28
K -C	0.16		2301 C	0.10	0.06
-----Bottom Chords-----					

F -E	0.14	293 T	0.00	0.14
E -G	0.34	943 T	0.15	0.19
G -D	0.41	2048 T	0.34	0.07
D -C	0.52	2116 T	0.35	0.17
-----Webs-----				
F -A	0.43	111 C	WindLd	1 Br
F -B	0.13	549 C		1 Br
B -E	0.31	784 T		
E -I	0.11	298 T		
E -J	0.33	698 C		
G -J	0.05	372 T		
G -K	0.30	1131 C		
D -K	0.08	624 T		

TL Defl	-0.16"	in G -D	L/999
LL Defl	-0.08"	in G -D	L/999
Hx Disp	LL	DL	TL
Jt C	0.05"	0.05"	0.10"
Shear //	Grain	in J -K	0.20

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.40  
B LOCK 4.0x 4.0 Ctr Ctr 0.81  
I LOCK 2.0x 4.0 Ctr Ctr 0.38  
J LOCK 4.0x 6.0 0.5 0.9 0.57  
K LOCK 3.0x 4.0 Ctr Ctr 0.64  
C LOCK 3.0x 4.0 Ctr Ctr 0.89  
F LOCK 3.0x 4.0 Ctr Ctr 0.65  
E# LOCK 6.0x10.0-0.1-0.3 0.49  
G LOCK 3.0x 4.0 Ctr Ctr 0.68  
D LOCK 5.0x 5.0 Ctr-1.0 0.73

# = Plate Monitor used

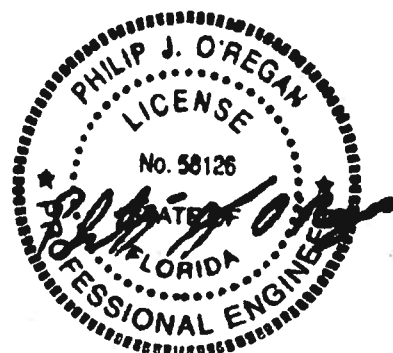
REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR

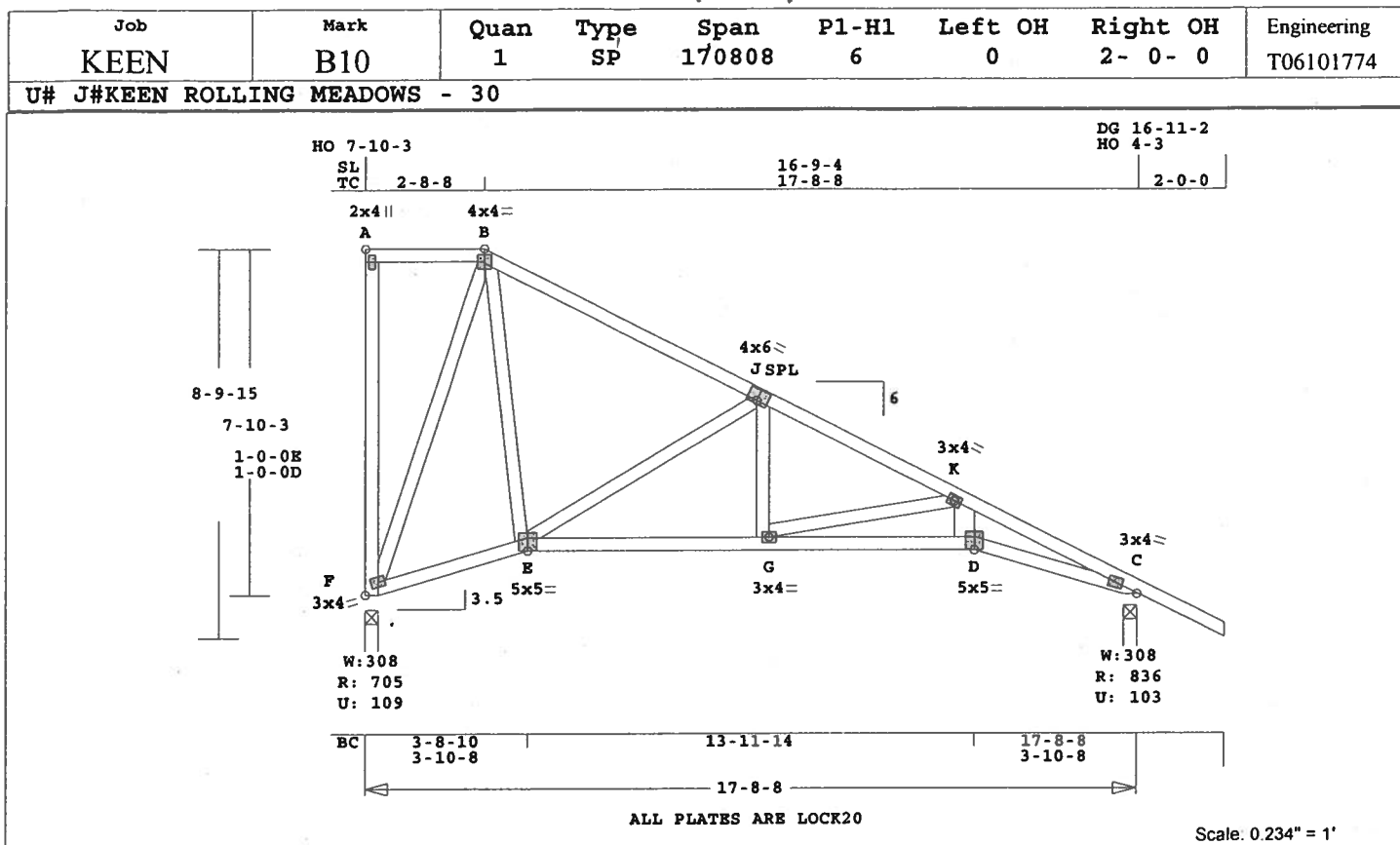
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
Provide connection to bearing  
for 305 Lbs Horiz Reaction  
Design checked for 10 psf non-  
concurrent LL on BC.  
NOTE: USER MODIFIED PLATES  
This design may have plates  
selected through a plate  
monitor.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 2301 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682







Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 143.6 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI	Size	Lumber
TC	0.37	2x 4 SP-#2
BC	0.52	2x 4 SP-#2
WB	0.55	2x 4 SP-#2
EX D	-K	2x 6 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	17- 8- 8
BC Cont.	0- 0- 0	17- 8- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
F	705	109	3- 8	1- 8
			Hz =	-269
C	836	103	3- 8	1- 8
			Hz =	116

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.15		164 T	0.02	0.13
B -J	0.37		363 C	0.00	0.37
J -K	0.37		1069 C	0.00	0.37
K -C	0.15		2311 C	0.04	0.11
-----Bottom Chords-----					
F -E	0.13		244 T	0.04	0.09

E	-G	0.29	976 T	0.16	0.13
G <td>-D</td> <td>0.41</td> <td>2055 T</td> <td>0.34</td> <td>0.07</td>	-D	0.41	2055 T	0.34	0.07
D <td>-C</td> <td>0.52</td> <td>2124 T</td> <td>0.35</td> <td>0.17</td>	-C	0.52	2124 T	0.35	0.17
-----Webs-----					
F <td>-A</td> <td>0.33</td> <td>67 C</td> <td>WindLd</td> <td></td>	-A	0.33	67 C	WindLd	
F <td>-B</td> <td>0.55</td> <td>704 C</td> <td></td> <td></td>	-B	0.55	704 C		
B <td>-E</td> <td>0.10</td> <td>551 T</td> <td></td> <td></td>	-E	0.10	551 T		
E <td>-J</td> <td>0.39</td> <td>763 C</td> <td></td> <td></td>	-J	0.39	763 C		
G <td>-J</td> <td>0.05</td> <td>384 T</td> <td></td> <td></td>	-J	0.05	384 T		
G <td>-K</td> <td>0.30</td> <td>1104 C</td> <td></td> <td></td>	-K	0.30	1104 C		
D <td>-K</td> <td>0.08</td> <td>624 T</td> <td></td> <td></td>	-K	0.08	624 T		

TL Defl	-0.15"	in G -D	L/999
LL Defl	-0.07"	in G -D	L/999
Hz Disp	LL	DL	TL
Jt C	0.05"	0.05"	0.10"
Shear //	Grain	in B -J	0.23

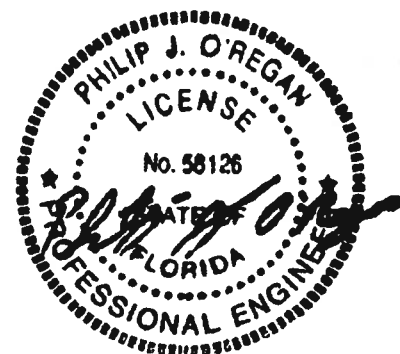
Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.40  
B LOCK 4.0x 4.0 Ctr Ctr 0.81  
J LOCK 4.0x 6.0 0.5 0.9 0.61  
K LOCK 3.0x 4.0 Ctr Ctr 0.64  
C LOCK 3.0x 4.0 Ctr Ctr 0.89  
F LOCK 3.0x 4.0 Ctr Ctr 0.65  
E LOCK 5.0x 5.0 Ctr-1.0 0.48  
G LOCK 3.0x 4.0 Ctr Ctr 0.67  
D LOCK 5.0x 5.0 Ctr-1.0 0.73

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

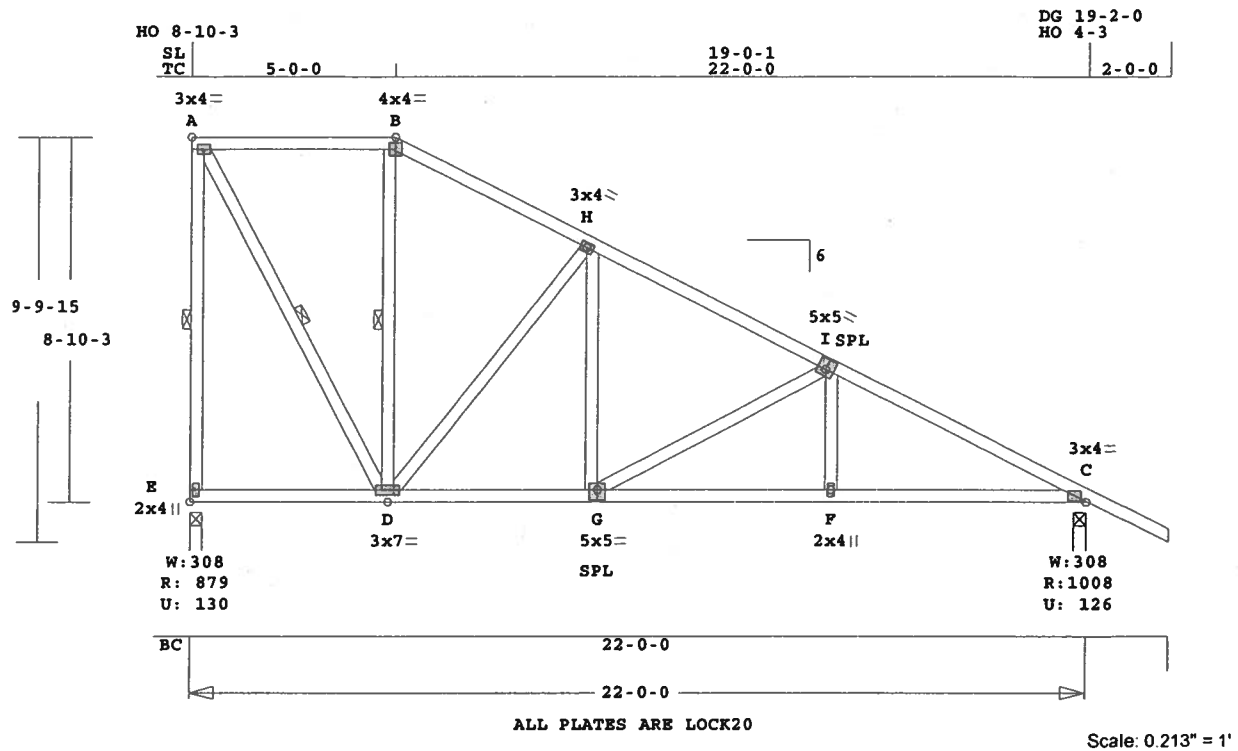
REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.  
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 2311 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job <b>KEEN</b>	Mark <b>B11</b>	Quan <b>1</b>	Type <b>HHIP</b>	Span <b>220000</b>	P1-H1 <b>6</b>	Left OH <b>0</b>	Right OH <b>2- 0- 0</b>	Engineering <b>T06101774</b>
U# J#KEEN ROLLING MEADOWS - 30								



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 183.1 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----  
TC 0.33 2x 4 SP-#2  
BC 0.33 2x 4 SP-#2  
WB 0.49 2x 4 SP-#2

Brace truss as follows:

O.C. From To  
TC Cont. 0- 0- 0 22- 0- 0  
BC Cont. 0- 0- 0 22- 0- 0  
WB 1 rows CLB on E -A  
WB 1 rows CLB on A -D  
WB 1 rows CLB on D -B  
Attach CLB with (2)-10d nails  
at each web.

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
E	880	131	3- 8	1- 8
			H <sub>z</sub> =	-304
C	1008	126	3- 8	1- 8
			H <sub>z</sub> =	141

Membr CSI P Lbs Axl-CSI-Bnd  
-----Top Chords-----  
A -B 0.24 387 C 0.00 0.24  
B -H 0.27 441 C 0.00 0.27  
H -I 0.32 890 C 0.00 0.32

I -C 0.33 1424 C 0.01 0.32 ADDITIONAL SPECIFICATIONS.

-----Bottom Chords-----

E -D 0.14 298 T 0.00 0.14  
D -G 0.22 794 T 0.08 0.14  
G -F 0.32 1280 T 0.21 0.11  
F -C 0.33 1280 T 0.21 0.12

-----Webs-----

E -A 0.43 838 C WindLd 1 Br  
A -D 0.14 803 T 1 Br  
D -B 0.02 108 T 1 Br  
D -H 0.49 642 C  
G -H 0.06 410 T  
G -I 0.30 552 C  
F -I 0.03 250 T

TL Defl -0.09" in G -F L/999  
LL Defl -0.03" in G -F L/999  
Shear // Grain in I -C 0.22

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

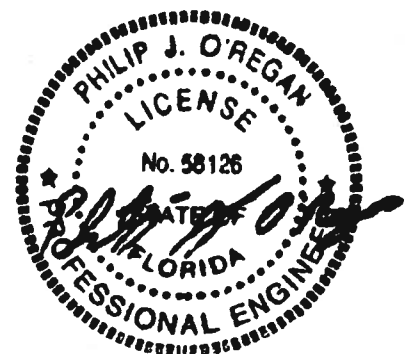
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.71  
B LOCK 4.0x 4.0 Ctr Ctr 0.89  
H LOCK 3.0x 4.0 Ctr Ctr 0.60  
I LOCK 5.0x 5.0 0.2 0.5 0.58  
C LOCK 3.0x 4.0 Ctr Ctr 0.80  
E LOCK 2.0x 4.0 Ctr Ctr 0.47  
D LOCK 3.0x 7.0 Ctr Ctr 0.62  
G LOCK 5.0x 5.0 Ctr-0.5 0.59  
F LOCK 2.0x 4.0 Ctr Ctr 0.40

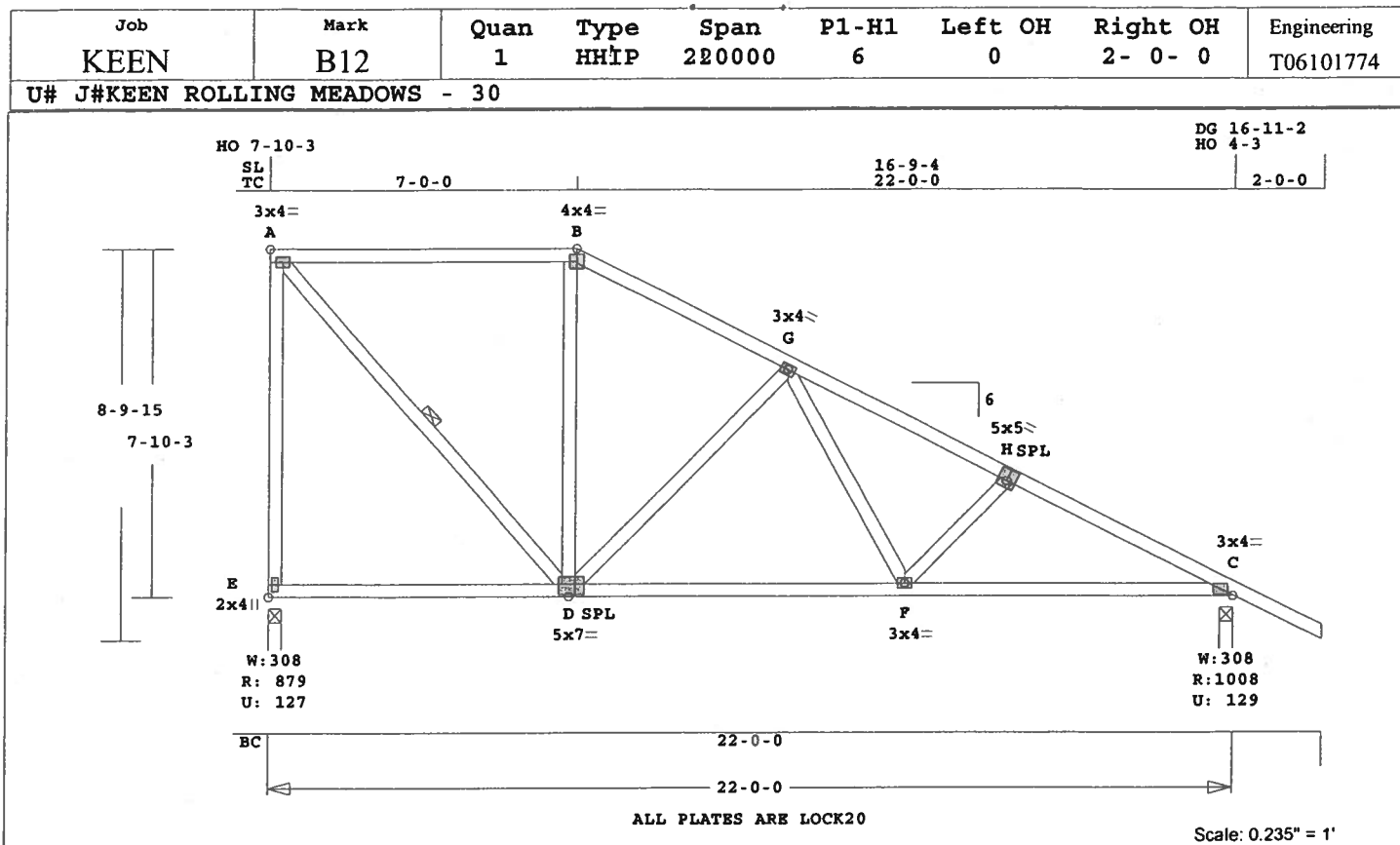
REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Provide connection to bearing  
for 304 Lbs Horiz Reaction  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1424 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus" APPROX. TRUSS WEIGHT: 166.3 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ---Lumber---  
TC 0.52 2x 4 SP-#2  
BC 0.42 2x 4 SP-#2  
WB 0.59 2x 4 SP-#2

Brace truss as follows:

O.C. From To  
TC Cont. 0- 0- 0 22- 0- 0  
BC Cont. 0- 0- 0 22- 0- 0  
WB 1 rows CLB on A -D  
Attach CLB with (2)-10d nails  
at each web.

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
E	880	128	3- 8	1- 8
			Hz =	-268
C	1008	129	3- 8	1- 8
			Hz =	124

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -B	0.52	563 C	0.00	0.52
B -G	0.24	629 C	0.04	0.20
G -H	0.27	1245 C	0.07	0.20
H -C	0.23	1463 C	0.01	0.22

-----Bottom Chords-----					
E -D	0.32	268 T	0.00	0.32	
D -F	0.41	926 T	0.09	0.32	
F -C	0.42	1315 T	0.13	0.29	
-----Webs-----					
E -A	0.59	824 C	WindLd		
A -D	0.15	849 T		1 Br	
D -B	0.07	128 T			
D -G	0.33	522 C			
G -F	0.07	433 T			
F -H	0.05	290 C			

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga	Gross Area
Plate	RHS	20 Ga <td>Gross Area</td>	Gross Area
Jt Type	Plt Size	X	Y
A	LOCK	3.0x 4.0	Ctr Ctr
B	LOCK	4.0x 4.0	Ctr Ctr
G	LOCK	3.0x 4.0	Ctr Ctr
H	LOCK	5.0x 5.0	0.2 0.5
C	LOCK	3.0x 4.0	Ctr Ctr
E	LOCK	2.0x 4.0	Ctr Ctr
D	LOCK	5.0x 7.0	1.0-0.5
F	LOCK	3.0x 4.0	Ctr Ctr

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

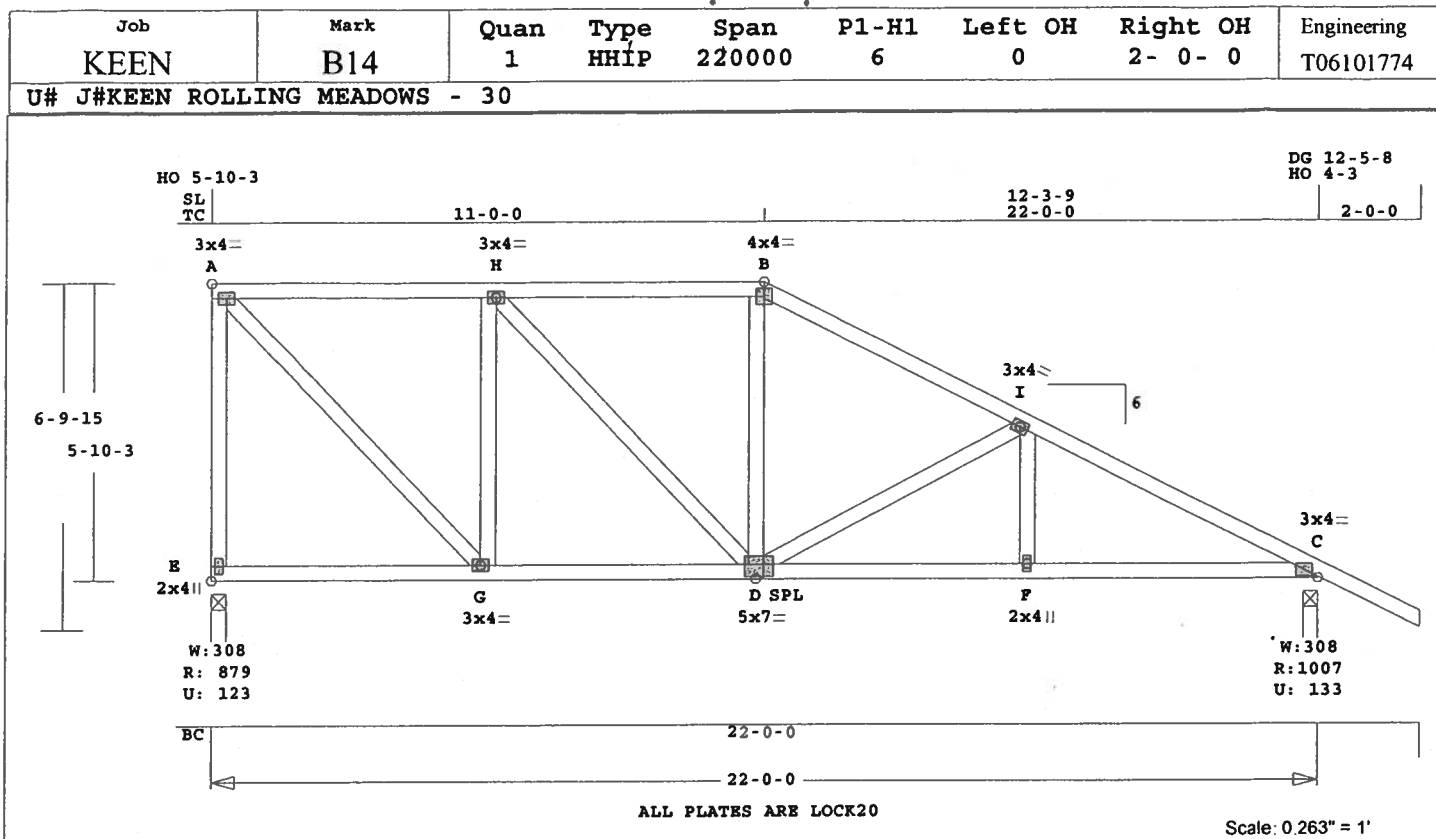
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1463 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682







Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 163.4 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size-	----	Lumber----
TC	0.34	2x 4 SP-#2
BC	0.30	2x 4 SP-#2
WB	0.32	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	22- 0- 0
BC Cont.	0- 0- 0	22- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
E	880	123	3- 8	1- 8
			Hx =	-196
C	1008	134	3- 8	1- 8
			Hx =	91

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -H	0.34		673 C	0.00	0.34
H -B	0.34		867 C	0.00	0.34
B -I	0.30		976 C	0.06	0.24
I -C	0.32		1450 C	0.08	0.24
-----Bottom Chords-----					
E -G	0.18		209 T	0.00	0.18
G -D	0.25		673 T	0.07	0.18
D -F	0.29		1302 T	0.21	0.08
F -C	0.30		1302 T	0.21	0.09

-----Webs-----			
E -A	0.32	833 C	WindLd
A -G	0.30	971 T	
G -H	0.22	579 C	
H -D	0.13	278 T	
D -B	0.03	226 T	
D -I	0.22	495 C	
F -I	0.03	223 T	

TL Defl -0.08" in D -F L/999  
LL Defl -0.04" in D -F L/999  
Shear // Grain in A -H 0.25

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.71  
H LOCK 3.0x 4.0 Ctr Ctr 0.53  
B LOCK 4.0x 4.0 Ctr Ctr 0.89  
I LOCK 3.0x 4.0 Ctr Ctr 0.60  
C LOCK 3.0x 4.0 Ctr Ctr 0.80  
E LOCK 2.0x 4.0 Ctr Ctr 0.42  
G LOCK 3.0x 4.0 Ctr Ctr 0.61  
D LOCK 5.0x 7.0-1.0-0.5 0.59  
F LOCK 2.0x 4.0 Ctr Ctr 0.40

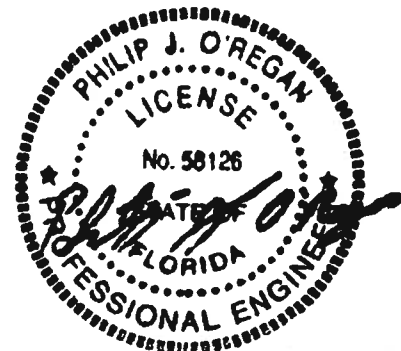
REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

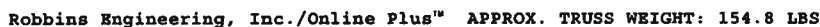
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1450 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682



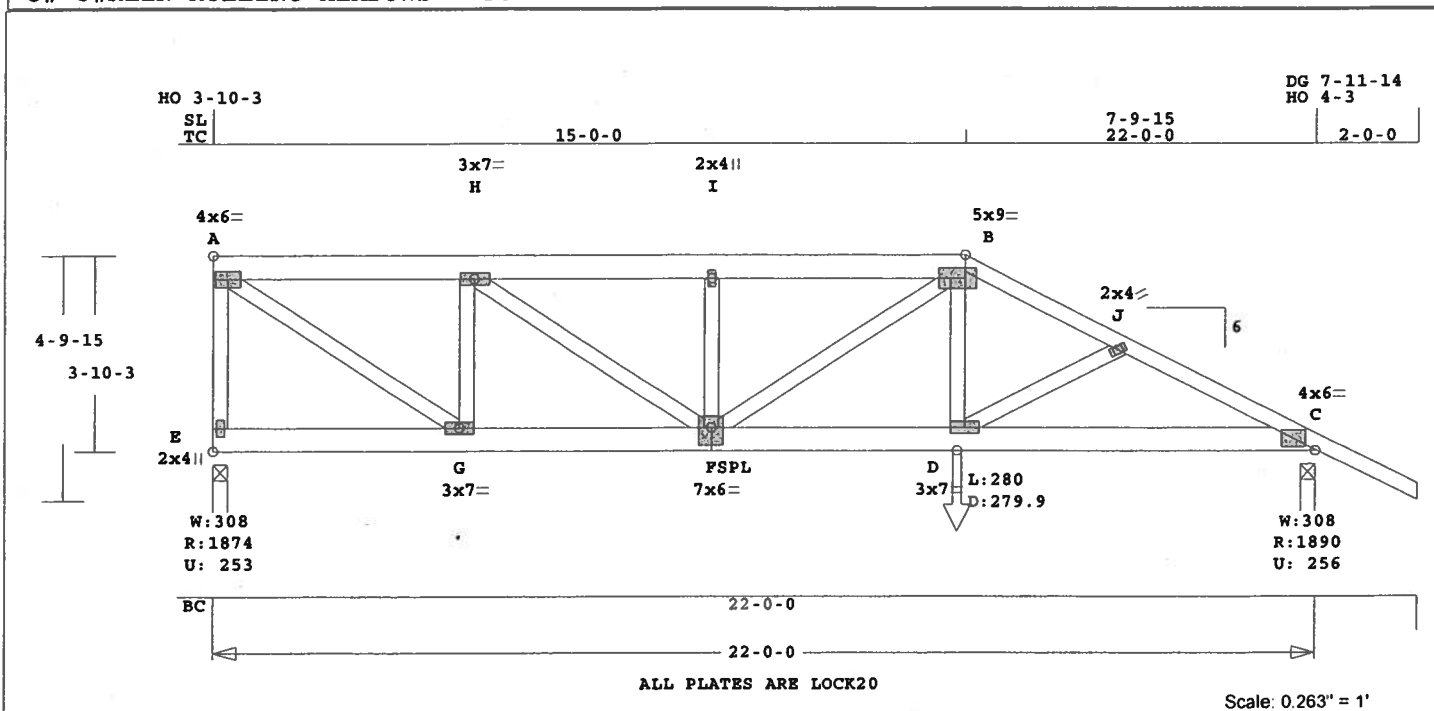
U# J#KEEN ROLLING MEADOWS - 30



Date Sealed: 10/17/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
KEEN	B16	1	HHIP	220000	6	0	2- 0- 0	T06101774

U# J#KEEN ROLLING MEADOWS - 30



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 183.9 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ---Lumber---  
TC 0.41 2x 6 SP-#2  
EX B -C 2x 4 SP-#2  
BC 0.55 2x 6 SP-#2  
WB 0.52 2x 4 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 22- 0- 0  
BC Cont. 0- 0- 0 22- 0- 0

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.00 Fc=1.00 Ft=1.00  
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
plf - Live Dead From To  
TC V 40 20 0.0' 22.0'  
BC V 0 20 0.0' 22.0'  
TC V 50 25 1.0' 15.0'  
TC V -40 -20 0.0' 1.0'  
BC V 0 25 1.0' 14.9'  
BC V 0 -20 0.0' 1.0'  
BC V 280 280 14.9' CL-LB

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt React Uplift Size Req'd  
Lbs Lbs In-Sx In-Sx  
E 1874 253 3- 8 2- 3  
Hz = -117  
C 1890 256 3- 8 2- 4  
Hz = 55

Membr CSI P Lbs Axl-CSt-Bnd  
-----Top Chords-----

A -H 0.29 2359 C 0.02 0.27  
H -I 0.35 3408 C 0.04 0.31  
I -B 0.41 3408 C 0.04 0.37  
B -J 0.56 3295 C 0.09 0.47  
J -C 0.29 3415 C 0.10 0.19  
-----Bottom Chords-----  
E -G 0.14 150 T 0.00 0.14  
G -F 0.38 2359 T 0.31 0.07  
F -D 0.51 2958 T 0.39 0.12  
D -C 0.55 3043 T 0.40 0.15  
-----Webs-----  
E -A 0.27 1807 C WindLd  
A -G 0.52 2869 T  
G -H 0.21 1412 C  
H -F 0.23 1274 T  
F -I 0.11 768 C  
I -B 0.09 537 T  
D -B 0.13 755 T  
D -J 0.02 191 T

TL Defl -0.21" in F -D L/999  
LL Defl -0.10" in F -D L/999  
Shear // Grain in I -B 0.35

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 4.0x 6.0 Ctr Ctr 0.86  
H LOCK 3.0x 7.0 Ctr Ctr 0.44  
I LOCK 2.0x 4.0 Ctr Ctr 0.53  
B LOCK 5.0x 9.0 Ctr Ctr 0.87  
J LOCK 2.0x 4.0 Ctr Ctr 0.37  
C LOCK 4.0x 6.0 Ctr Ctr 0.77  
E LOCK 2.0x 4.0 Ctr Ctr 0.82  
G LOCK 3.0x 7.0 Ctr Ctr 0.98  
F LOCK 7.0x 6.0 Ctr-0.8 0.74  
D LOCK 3.0x 7.0 Ctr Ctr 0.37

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:

Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

Girder Half Hip  
Framing King Jacks  
Jack Open Faced  
Setback 7- 0- 0

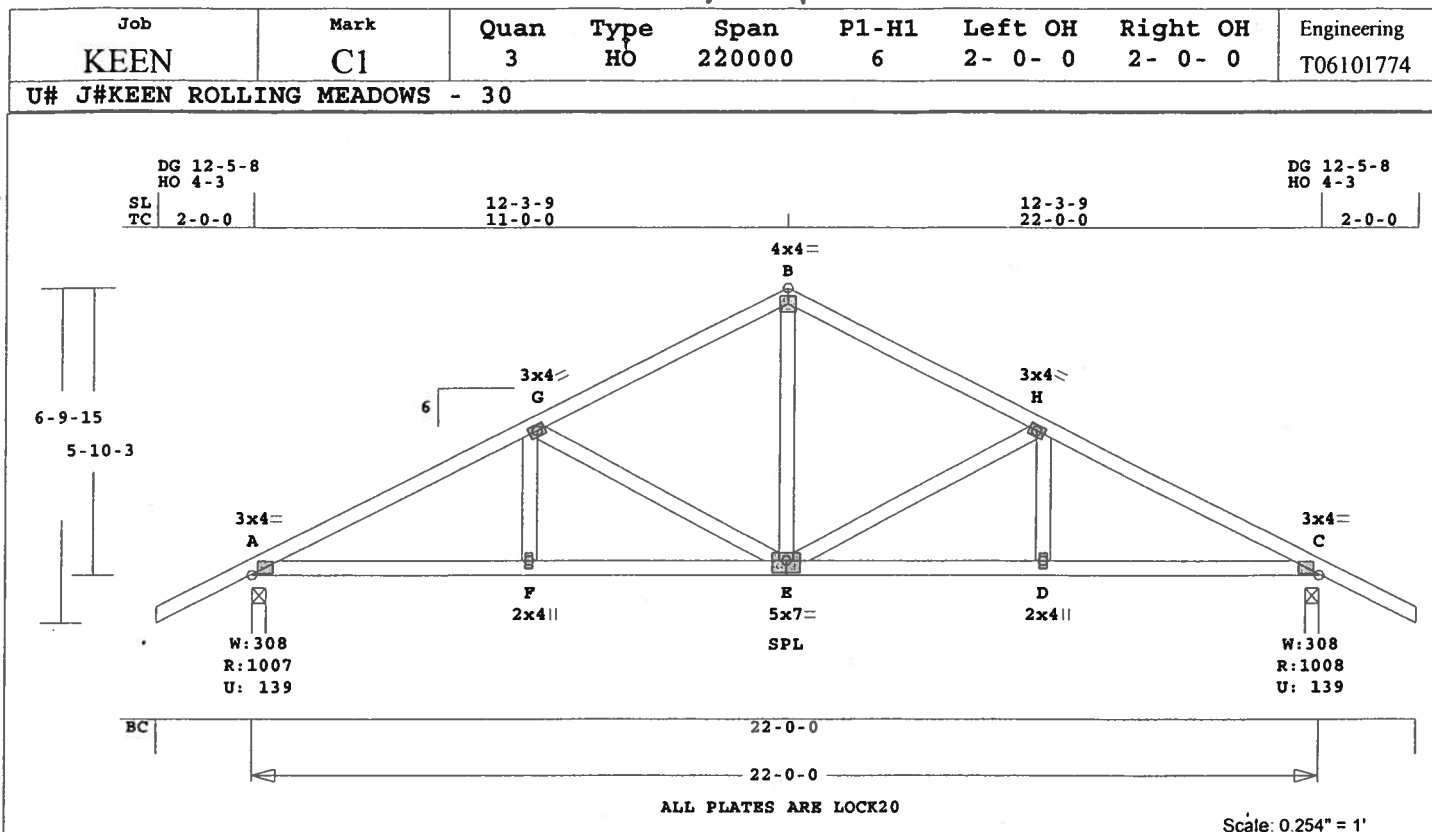
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as

Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 3415 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 138.3 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----

TC	0.32	2x 4	SP-#2
BC	0.30	2x 4	SP-#2
WB	0.21	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	22- 0- 0
BC Cont.	0- 0- 0	22- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing		24.0"	
Lumber Duration Factor		1.25	
Plate Duration Factor		1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	1008	140	3- 8	1- 8
			Hz =	-103
C	1008	140	3- 8	1- 8
			Hz =	104

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -G	0.32		1454 C	0.09	0.23
G -B	0.30		986 C	0.07	0.23
B -H	0.30		986 C	0.07	0.23
H -C	0.32		1454 C	0.09	0.23
-----Bottom Chords-----					
A -F	0.30		1306 T	0.21	0.09
F -E	0.29		1306 T	0.21	0.08
E -D	0.29		1306 T	0.21	0.08
D -C	0.30		1306 T	0.21	0.09

-----Webs-----

F -G	0.03	217 T
G -E	0.21	488 C
E -B	0.10	575 T
E -H	0.21	488 C
D -H	0.03	217 T

TL Defl -0.09" in E -D L/999  
LL Defl -0.04" in E -D L/999  
Shear // Grain in A -G 0.19

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.80  
G LOCK 3.0x 4.0 Ctr Ctr 0.60  
B LOCK 4.0x 4.0 Ctr Ctr 0.66  
H LOCK 3.0x 4.0 Ctr Ctr 0.60  
C LOCK 3.0x 4.0 Ctr Ctr 0.80  
F LOCK 2.0x 4.0 Ctr Ctr 0.40  
E LOCK 5.0x 7.0 Ctr-0.5 0.59  
D LOCK 2.0x 4.0 Ctr Ctr 0.40

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-

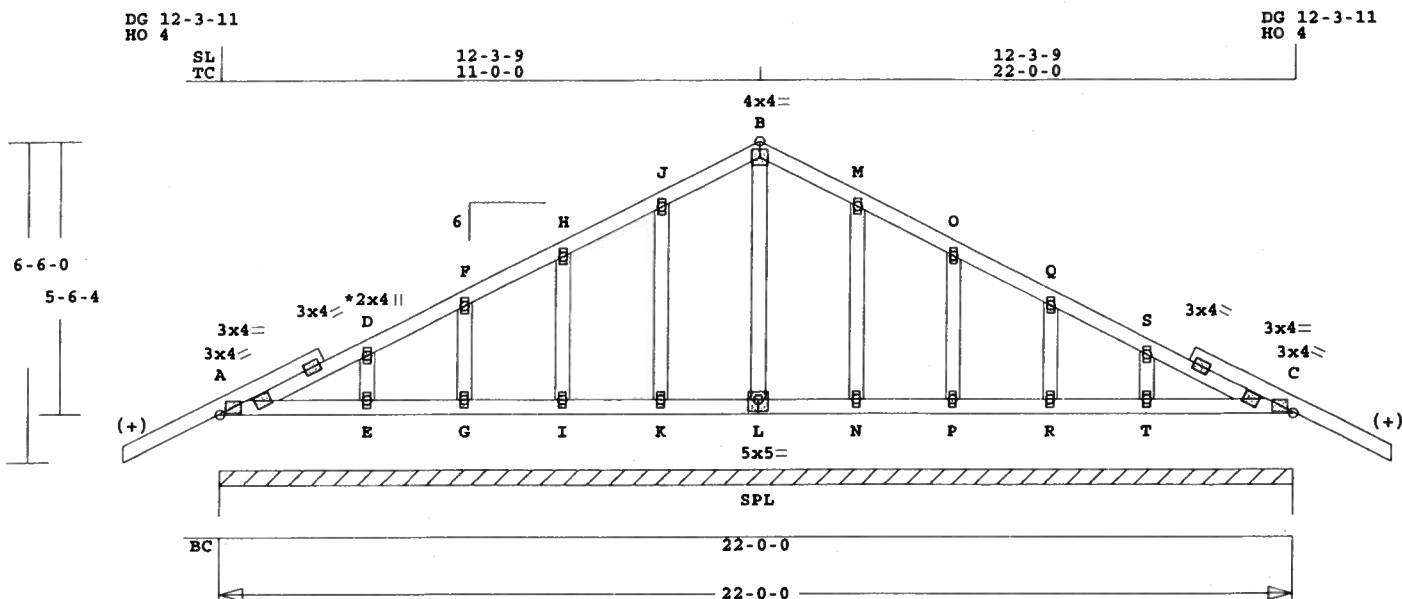
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 1454 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Job <b>KEEN</b>	Mark <b>C2</b>	Quan <b>1</b>	Type <b>HO</b>	Span <b>220000</b>	P1-H1 <b>6</b>	Left OH <b>0</b>	Right OH <b>0</b>	Engineering <b>T06101774</b>
U# J#KEEN ROLLING MEADOWS - 30								



ALL PLATES ARE LOCK20  
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.255" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 149.8 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ---Lumber---  
TC 0.05 2x 4 SP-#1+)  
BC 0.06 2x 4 SP-#2  
GW 0.02 2x 4 SP-#2

Brace truss as follows:  
O.C. From To  
TC Cont. 0- 0- 0 22- 0- 0  
BC Cont. 0- 0- 0 22- 0- 0

Loading Live Dead (psf)  
TC 20.0 10.0  
BC 0.0 10.0  
Total 20.0 20.0 40.0  
Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd  
Lbs Lbs In-Sx In-Sx  
Cont. Brg 0- 0- 0 to 22- 0- 0  
1760 234 Hz = 98

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -D	0.05	77	C	0.00	0.05
D -F	0.05	68	C	0.00	0.05
F -H	0.03	73	T	0.00	0.03
H -J	0.04	132	T	0.01	0.03
J -B	0.05	186	T	0.02	0.03
B -M	0.05	186	T	0.02	0.03
M -O	0.04	132	T	0.01	0.03
O -Q	0.03	73	T	0.00	0.03
Q -S	0.05	68	C	0.00	0.05
S -C	0.05	77	C	0.00	0.05
-----Bottom Chords-----					
A -E	0.06	15	T	0.00	0.06
E -G	0.03	0	T	0.00	0.03
G -I	0.02	0	T	0.00	0.02
I -K	0.02	0	T	0.00	0.02
K -L	0.02	0	T	0.00	0.02
L -N	0.02	0	T	0.00	0.02
N -P	0.02	0	T	0.00	0.02

	P	-R	0.02	0	T	0.00	0.02
R	-T	0.03	0	T	0.00	0.03	
T	-C	0.06	15	T	0.00	0.06	

-----Gable Webs-----

E	-D	0.02	164	T
G	-F	0.01	114	C
I	-H	0.01	121	C
K	-J	0.02	178	T
L	-B	0.01	54	C
N	-M	0.02	178	T
P	-O	0.01	121	C
R	-Q	0.01	114	C
T	-S	0.02	164	T

TL Defl 0.00" in T -C L/999  
LL Defl 0.00" in T -C L/999  
Shear // Grain in A -D 0.09

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	3.0x	4.0	Ctr	Ctr	0.80
D	LOCK	2.0x	4.0	Ctr	Ctr	0.00
F	LOCK	2.0x	4.0	Ctr	Ctr	0.00
H	LOCK	2.0x	4.0	Ctr	Ctr	0.00
J	LOCK	2.0x	4.0	Ctr	Ctr	0.00
B	LOCK	4.0x	4.0	Ctr	Ctr	0.66
M	LOCK	2.0x	4.0	Ctr	Ctr	0.00
O	LOCK	2.0x	4.0	Ctr	Ctr	0.00
Q	LOCK	2.0x	4.0	Ctr	Ctr	0.00
S	LOCK	2.0x	4.0	Ctr	Ctr	0.00
C	LOCK	3.0x	4.0	Ctr	Ctr	0.80
E	LOCK	2.0x	4.0	Ctr	Ctr	0.00
G	LOCK	2.0x	4.0	Ctr	Ctr	0.00
I	LOCK	2.0x	4.0	Ctr	Ctr	0.00
K	LOCK	2.0x	4.0	Ctr	Ctr	0.00
L	LOCK	5.0x	5.0	Ctr	-0.5	0.59
N	LOCK	2.0x	4.0	Ctr	Ctr	0.00
P	LOCK	2.0x	4.0	Ctr	Ctr	0.00
R	LOCK	2.0x	4.0	Ctr	Ctr	0.00
T	LOCK	2.0x	4.0	Ctr	Ctr	0.00

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.

Analysis Conforms To:  
FBC2004

WARNING Do Not Cut overframe  
member between outside of  
truss and first tie-plate  
to inside of heel plate.  
Design checked for 10 psf non-  
concurrent LL on BC.

Refer to Gen Det 3 series for  
web bracing and plating.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as

Components and Claddings\*  
for Exterior zone location.

Wind Speed: 110 mph  
Mean Roof Height: 15'-0

Exposure Category: B  
Occupancy Factor : 1.00

Building Type: Enclosed  
TC Dead Load: 5.0 psf

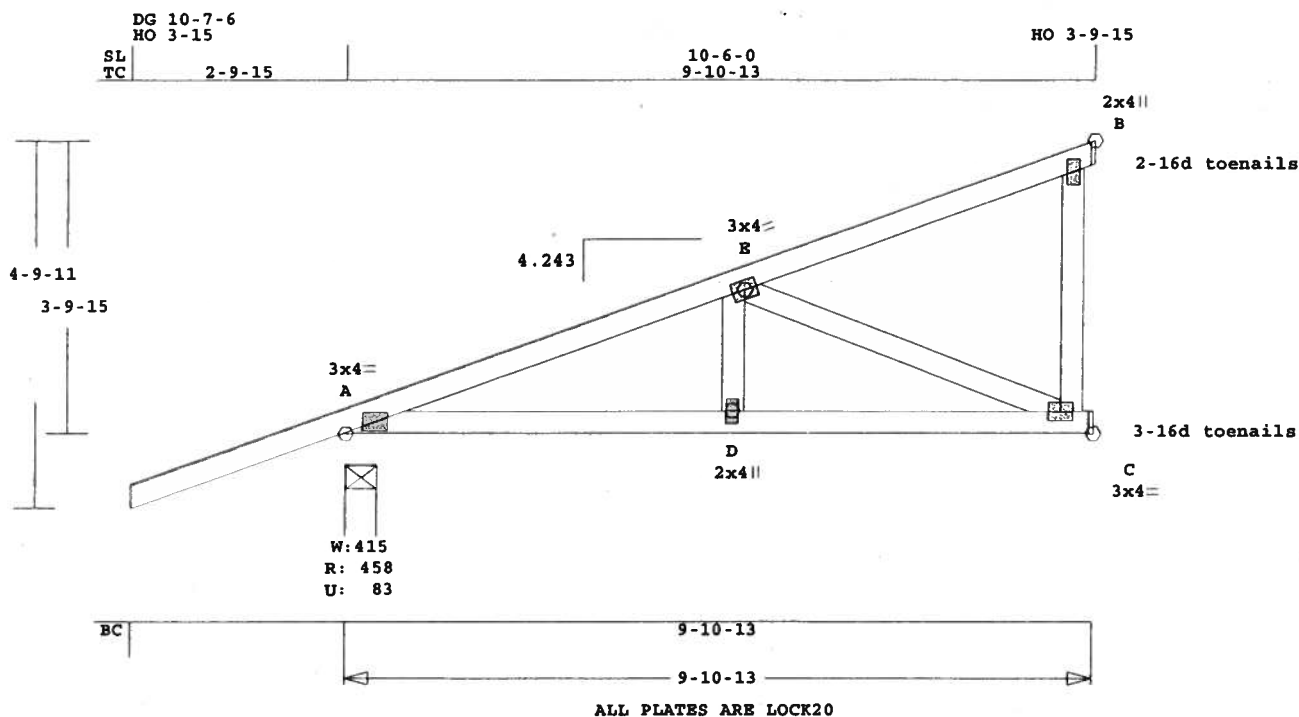
BC Dead Load: 5.0 psf  
Max comp. force 153 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job <b>KEEN</b>	Mark <b>CJ1</b>	Quan <b>3</b>	Type <b>MONO.DD</b>	Span <b>91013</b>	P1-H1 <b>4.243</b>	Left OH <b>2- 9-15</b>	Right OH <b>0</b>	Engineering <b>T06101774</b>
U# J#KEEN ROLLING MEADOWS - 30								



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 61.9 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ---Lumber---  
TC 0.38 2x 4 SP-#2  
BC 0.27 2x 4 SP-#2  
WB 0.19 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9-10-13
BC Cont.	0- 0- 0	9-10-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Girder Loading	Lumber Duration Factor	Plate Duration Factor	plf - Live	Dead	From	To
TC V	40	20	0.0'	9.9'		
BC V	0	20	0.0'	9.9'		
TC V	-40	-20	0.0'	9.9'		
BC V	0	-20	0.0'	9.9'		

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	458	83	4-15	1- 8
			Hx =	-43
C	348	8	1- 8	1- 8
B	240	84	1- 8	1- 8
			Hx =	127

Membr CSI P Lbs Axl-Csi-Bnd  
-----Top Chords-----  
A -E 0.31 632 C 0.03 0.28  
E -B 0.38 77 T 0.00 0.38  
-----Bottom Chords-----  
A -D 0.23 612 T 0.07 0.16  
D -C 0.27 612 T 0.07 0.20  
-----Webs-----  
D -E 0.03 234 T  
E -C 0.19 661 C  
C -B 0.05 0 T WindLd

TL Defl -0.05" in D -C L/999  
LL Defl -0.02" in D -C L/999  
Shear // Grain in E -B 0.27

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.64  
E LOCK 3.0x 4.0 Ctr Ctr 0.43  
B LOCK 2.0x 4.0 Ctr Ctr 0.38  
D LOCK 2.0x 4.0 Ctr Ctr 0.38  
C LOCK 3.0x 4.0 Ctr Ctr 0.54

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

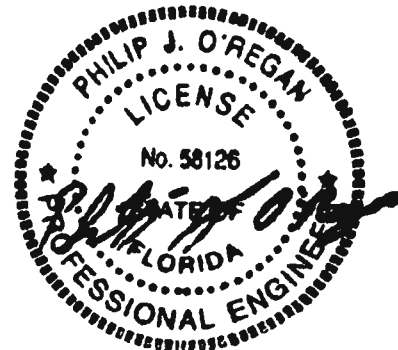
For proper installation of  
toe-nails, refer to the 2001  
National Design Specification  
(NDS) for Wood Construction

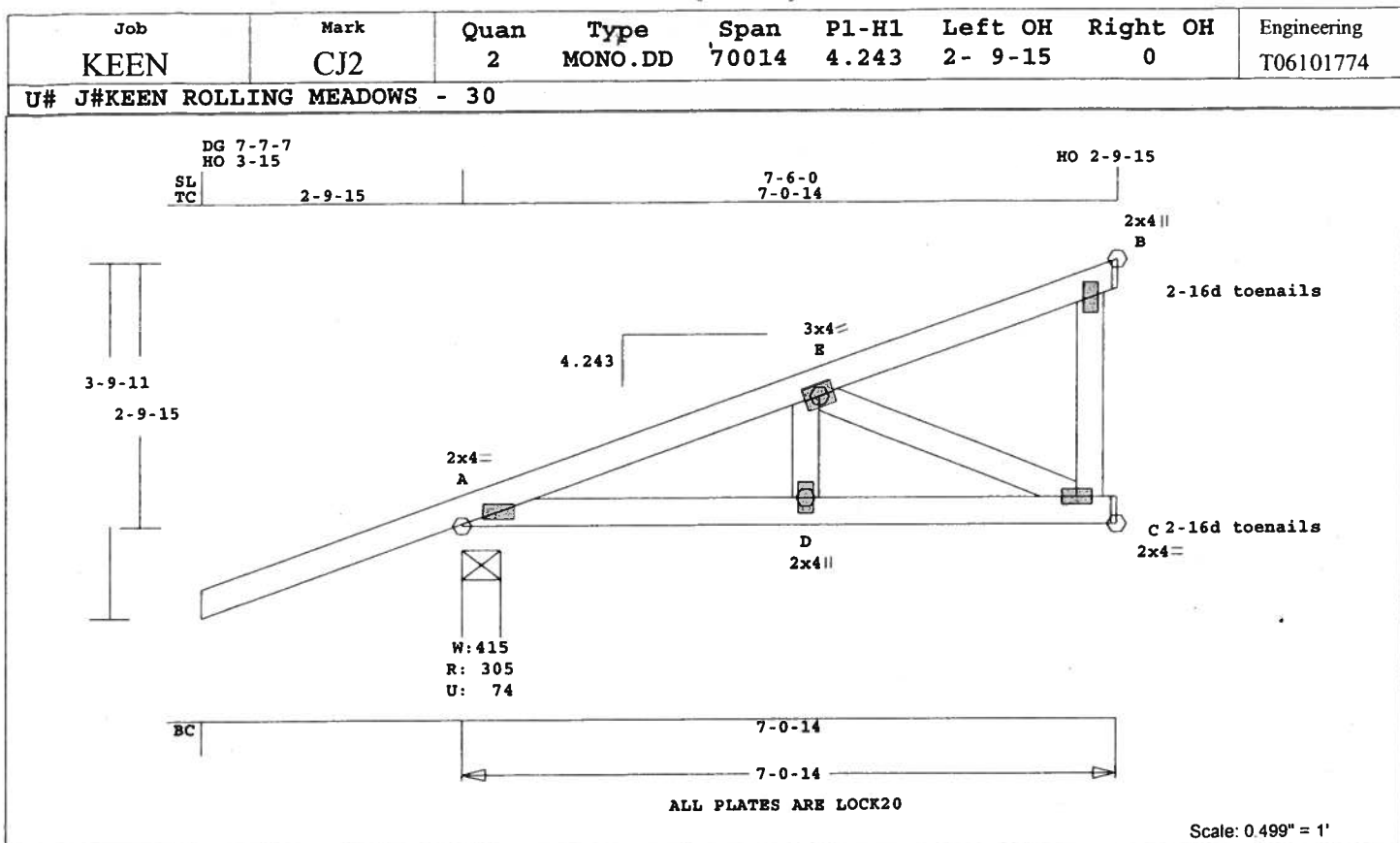
NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

Girder King Jack  
Loading TC and BC  
Setback 7- 0- 0  
OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Use properly rated hangers for  
loads framing into girder  
truss.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 661 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.2 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size-	Lumber
TC 0.16 2x 4 SP-#2	
BC 0.09 2x 4 SP-#2	
WB 0.04 2x 4 SP-#2	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0-14
BC Cont.	0- 0- 0	7- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00 Fc=1.00 Ft=1.00			
BC Fb=1.00 Fc=1.00 Ft=1.00			

Load Case # 1 Girder Loading	Lumber Duration Factor	Plate Duration Factor
plf - Live Dead From To		
TC V 40 20 0.0' 7.1'		
BC V 0 20 0.0' 7.1'		
TC V -40 -20 0.0' 7.1'		
BC V 0 -20 0.0' 7.1'		

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
A	Lbs	Lbs	In-Sx	In-Sx
A	306	75	4-15	1- 8
C	166	3	1- 8	1- 8
B	119	39	1- 8	1- 8

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -E	0.12	274	C	0.02	0.10
E -B	0.16	42	T	0.00	0.16
-----Bottom Chords-----					
A -D	0.07	265	T	0.03	0.04
D -C	0.09	265	T	0.03	0.06
-----Webs-----					
D -E	0.01	105	T		
E -C	0.04	289	C		
C -B	0.02	0	T	WindLd	

TL Defl -0.01" in D -C L/999  
LL Defl 0.00" in D -C L/999  
Shear // Grain in E -B 0.16

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.77  
E LOCK 3.0x 4.0 Ctr Ctr 0.40  
B LOCK 2.0x 4.0 Ctr Ctr 0.38  
D LOCK 2.0x 4.0 Ctr Ctr 0.38  
C LOCK 2.0x 4.0 Ctr Ctr 0.75

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

For proper installation of  
toe-nails, refer to the 2001  
National Design Specification  
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

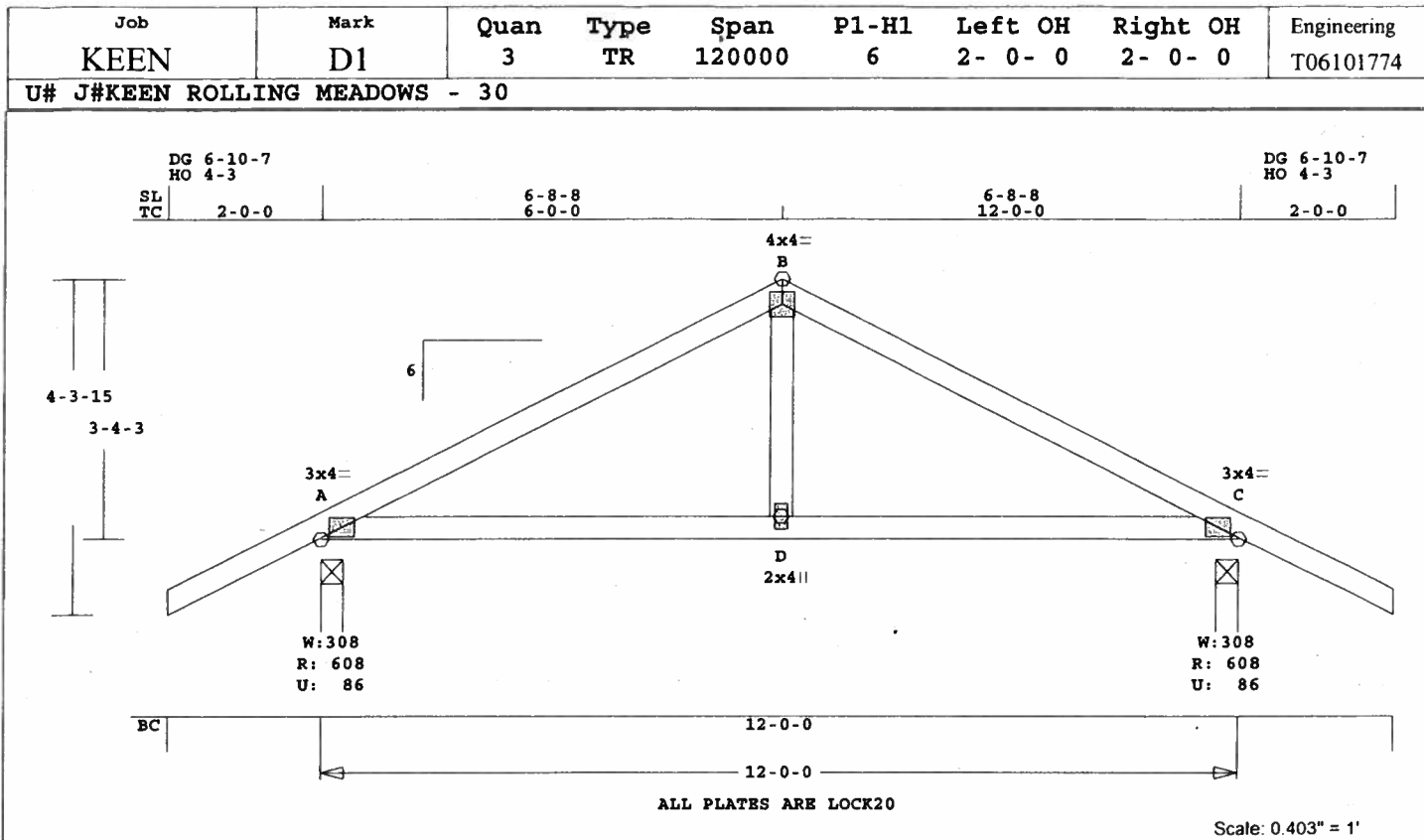
Girder King Jack  
Loading TC and BC  
Setback 5- 0- 0

OH Loading  
Soffit psf 2.0  
Design checked for 10 psf non-  
concurrent LL on BC.  
Use properly rated hangers for  
loads framing into girder  
truss.

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 289 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 62.2 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size-	----	Lumber----
TC	0.29	2x 4 SP-#2
BC	0.28	2x 4 SP-#2
WB	0.04	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 0- 0
BC Cont.	0- 0- 0	12- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 9 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	608	86	3- 8	1- 8
			Hz =	-48
C	608	86	3- 8	1- 8
			Hz =	49

Membr CSI P Lbs Axl-CSt-Bnd

-----Top Chords-----

A -B	0.29	589	C	0.05	0.24
B -C	0.29	589	C	0.05	0.24

-----Bottom Chords-----

A -D	0.28	530	T	0.08	0.20
D -C	0.28	530	T	0.08	0.20
-----Webs-----					
D -B	0.04	267	T		

TL Defl -0.05" in D -C L/999  
LL Defl -0.02" in D -C L/999  
Shear // Grain in A -B 0.23

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 3.0x 4.0 Ctr Ctr 0.63  
B LOCK 4.0x 4.0 Ctr Ctr 0.53  
C LOCK 3.0x 4.0 Ctr Ctr 0.63  
D LOCK 2.0x 4.0 Ctr Ctr 0.38

REVIEWED BY:

Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:  
Mayo Truss Co. Inc.

Analysis Conforms To:  
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*  
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

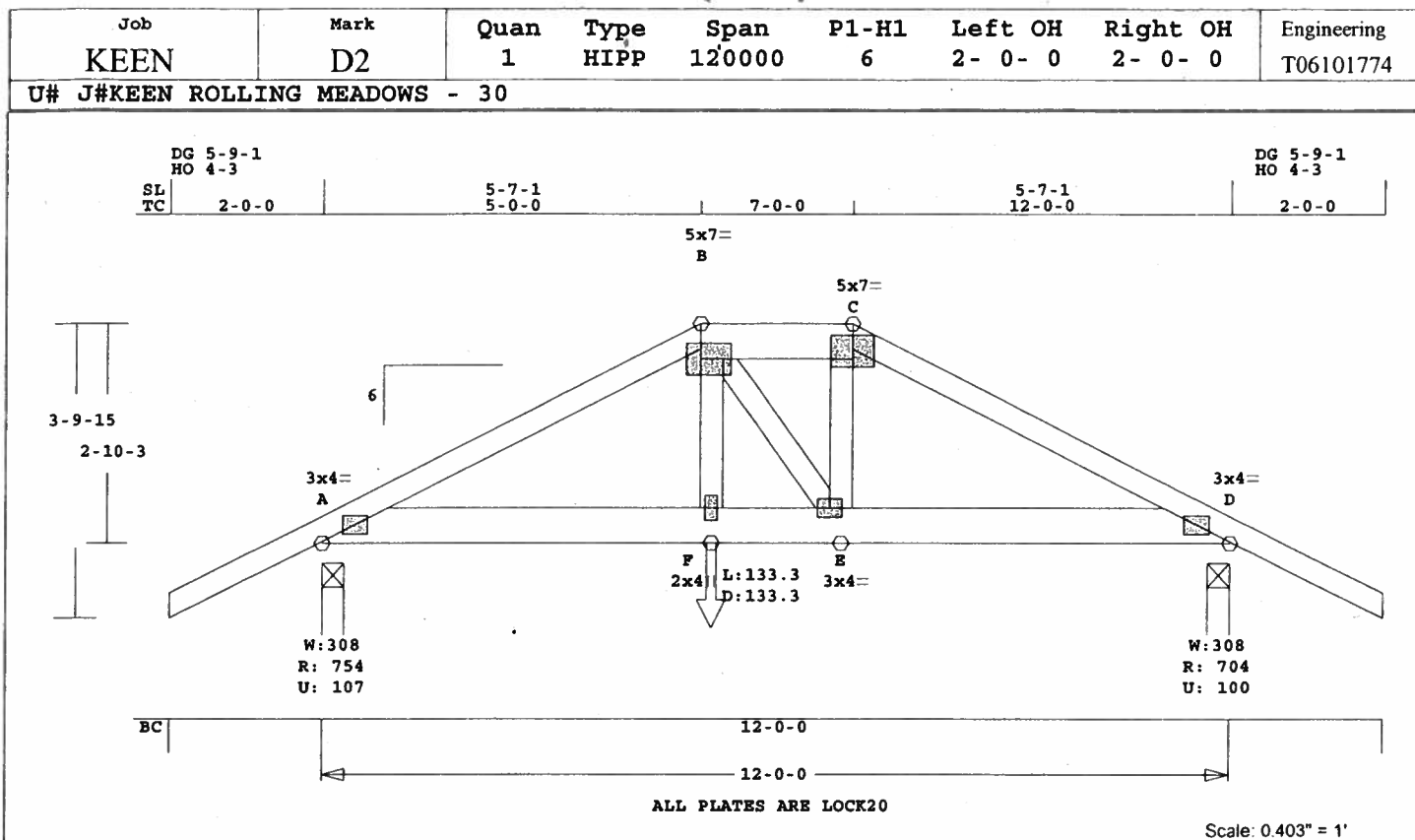
BC Dead Load: 5.0 psf

Max comp. force 589 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 81.6 LBS

D 704 100 3- 8 1- 8  
Hz = 40

**NOTES:**

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

Girder Half Hip

Framing King Jacks

Jack Open Faced

Setback 5- 0- 0

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15'-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 959 Lbs

Quality Control Factor 1.25

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size-	----	Lumber----
TC 0.28	2x 4	SP-#2
EX B -C	2x 6	SP-#2
BC 0.22	2x 6	SP-#2
WB 0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 0- 0
BC Cont.	0- 0- 0	12- 0- 0

Loading Live Dead (psf)

TC 20.0 10.0

BC 0.0 10.0

Total 20.0 20.0 40.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.00 Fc=1.00 Ft=1.00

BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

plf - Live Dead From To

TC V 40 20 0.0' 12.0'

BC V 0 20 0.0' 12.0'

TC V 30 15 5.0' 6.0'

TC V -40 -20 6.0' 7.0'

BC V 0 15 5.1' 5.9'

BC V 0 -20 5.9' 6.9'

BC V 133 133 5.1' CL-LB

Membr CSI P Lbs Axl-CST-Bnd

-----Top Chords-----

A -B 0.28 959 C 0.10 0.18

B -C 0.07 780 C 0.06 0.01

C -D 0.28 870 C 0.09 0.19

-----Bottom Chords-----

A -F 0.22 862 T 0.11 0.11

F -E 0.16 848 T 0.11 0.05

E -D 0.21 780 T 0.10 0.11

-----Webs-----

F -B 0.05 319 T

B -E 0.01 129 T

E -C 0.03 210 T

TL Defl -0.03" in A -F L/999

LL Defl -0.02" in A -F L/999

Shear // Grain in C -D 0.18

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr Ctr 0.73

B LOCK 5.0x 7.0-0.5 Ctr 0.78

C LOCK 5.0x 7.0 Ctr-0.3 0.48

D LOCK 3.0x 4.0 Ctr Ctr 0.73

F LOCK 2.0x 4.0 Ctr Ctr 0.38

E LOCK 3.0x 4.0 Ctr Ctr 0.43

REVIEWED BY:

Robbins Engineering, Inc.

PO Box 280055

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

Jt React Uplft Size Req'd

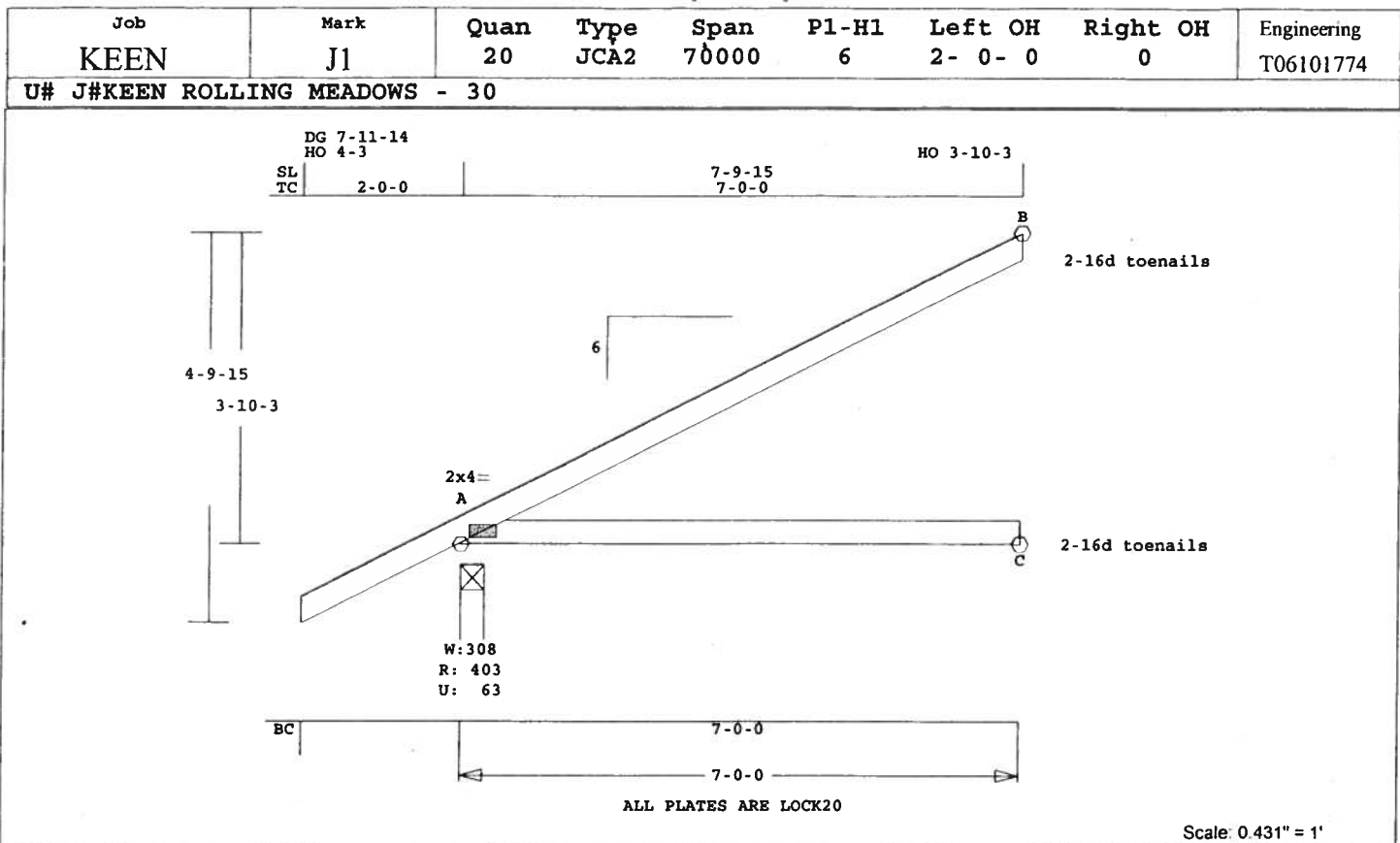
Lbs Lbs In-Sx In-Sx

A 754 108 3- 8 1- 8

Hz = -39

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 32.5 LBS

A -C 0.41 0 T 0.00 0.41

concurrent LL on BC.

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

TL Defl -0.18" in A -C L/427  
LL Defl -0.07" in A -C L/999  
Shear // Grain in A -B 0.27

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*  
for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 166 lbs

Quality Control Factor 1.25

CSI -Size- ----Lumber----

TC 0.52 2x 4 SP-#2

BC 0.41 2x 4 SP-#2

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.69

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading Live Dead (psf)

TC 20.0 10.0

BC 0.0 10.0

Total 20.0 20.0 40.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

REVIEWED BY:

Robbins Engineering, Inc.

PO Box 280055

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

For proper installation of  
toe-nails, refer to the 2001  
National Design Specification  
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

Plus 8 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd

Lbs Lbs In-Sx In-Sx

A 404 64 3- 8 1- 8

Hz = 106

C 132 0 3- 8 1- 8

B 196 85 3- 8 1- 8

Hz = 72

Membr CSI P Lbs Axl-CSt-Bnd

-----Top Chords-----

A -B 0.52 166 C 0.00 0.52

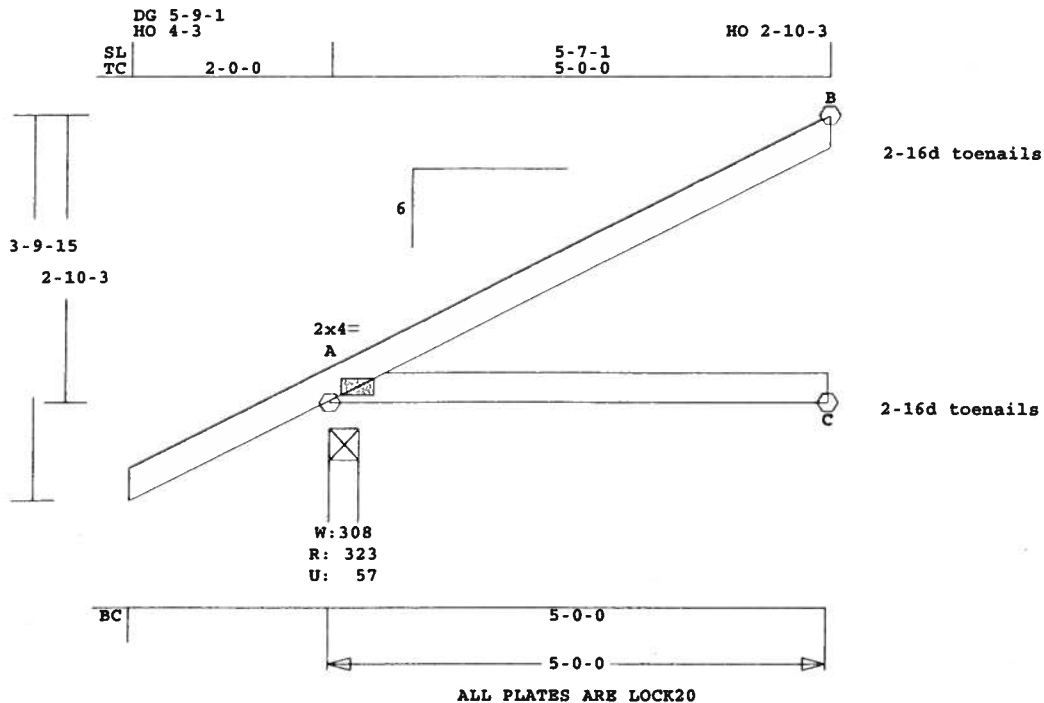
-----Bottom Chords-----

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
KEEN	J2	8	JCA2	5'0000	6	2- 0- 0	0	T06101774

U# J#KEEN ROLLING MEADOWS - 30



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.5 LBS

A -C 0.24 0 T 0.00 0.24 concurrent LL on BC.

Online Plus -- Version 20.0.001 TL Defl -0.04" in A -C L/999  
 RUN DATE: 17-OCT-06 LL Defl -0.02" in A -C L/999  
 Shear // Grain in A -B 0.22

CSI -Size- ----Lumber----  
 TC 0.31 2x 4 SP-#2  
 BC 0.24 2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 0	
BC Cont.	0- 0- 0	5- 0- 0	

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15 Fc=1.10 Ft=1.10			
BC Fb=1.10 Fc=1.10 Ft=1.10			

Plates for each ply each face.  
 PLATING CONFORMS TO TPI.  
 REPORTS: SBCCI 9761  
 ROBBINS ENGINEERING, INC.  
 BASED ON SP LUMBER  
 USING GROSS AREA TEST.  
 Plate - LOCK 20 Ga, Gross Area  
 Plate - RHS 20 Ga, Gross Area  
 Jt Type Plt Size X Y JSI  
 A LOCK 2.0x 4.0 Ctr Ctr 0.65

REVIEWED BY:  
 Robbins Engineering, Inc.  
 PO Box 280055  
 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
 NOTES AND SYMBOLS SHEET FOR  
 ADDITIONAL SPECIFICATIONS.

For proper installation of  
 toe-nails, refer to the 2001  
 National Design Specification  
 (NDS) for Wood Construction

NOTES:  
 Trusses Manufactured by:  
 Mayo Truss Co. Inc.  
 Analysis Conforms To:  
 FBC2004  
 OH Loading  
 Soffit psf 2.0  
 Design checked for 10 psf non-

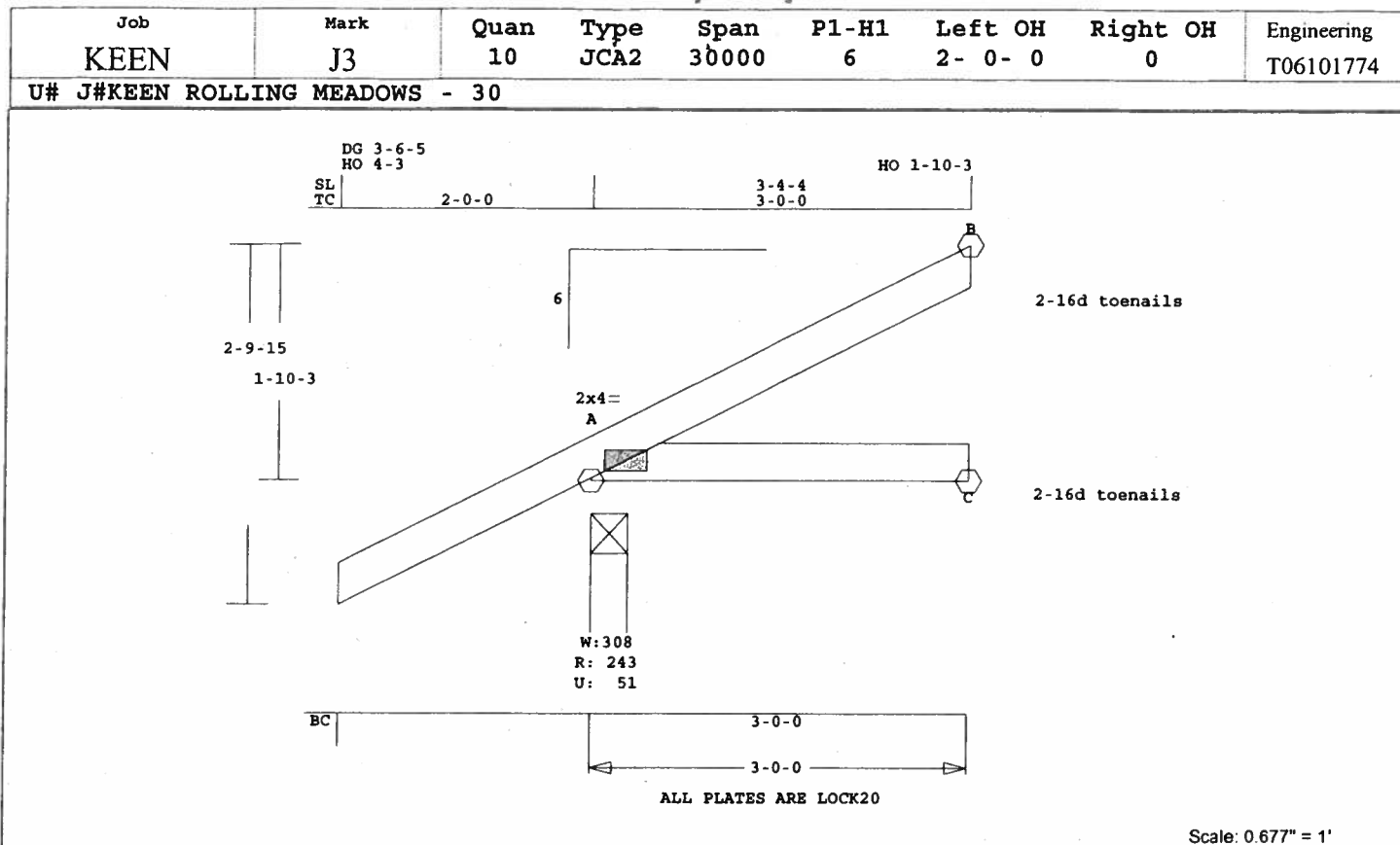
Wind Loads - ANSI / ASCE 7-02  
 Truss is designed as  
 Components and Claddings\*  
 for Exterior zone location.  
 Wind Speed: 110 mph  
 Mean Roof Height: 15-0  
 Exposure Category: B  
 Occupancy Factor : 1.00  
 Building Type: Enclosed  
 TC Dead Load: 5.0 psf  
 BC Dead Load: 5.0 psf  
 Max comp. force 137 Lbs  
 Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
 License #: 58126  
 Address: P.O. Box 280055, Tampa, FL 33682



Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
A	324	58	3- 8	1- 8
			Hz =	75
C	94	0	3- 8	1- 8
B	142	62	3- 8	1- 8
			Hz =	51

Membr CSI P Lbs Axl-Csi-Bnd  
 -----Top Chords-----  
 A -B 0.31 137 C 0.00 0.31  
 -----Bottom Chords-----



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 16.5 LBS

A -C 0.09 0 T 0.00 0.09

concurrent LL on BC.

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

TL Defl 0.00" in A -C L/999  
LL Defl 0.00" in A -C L/999  
Shear // Grain in A -B 0.13

Wind Loads - ANSI / ASCE 7-02  
Truss is designed as

CSI -Size- ---Lumber---  
TC 0.10 2x 4 SP-#2  
BC 0.09 2x 4 SP-#2

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

Components and Claddings\*  
for Exterior zone location.

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 0
BC Cont.	0- 0- 0	3- 0- 0

REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 89 Lbs

Quality Control Factor 1.25

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0

BASED ON SP LUMBER  
USING GROSS AREA TEST.  
Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.65

Spacing 24.0"  
Lumber Duration Factor 1.25  
Plate Duration Factor 1.25  
TC Fb=1.15 Fc=1.10 Ft=1.10  
BC Fb=1.10 Fc=1.10 Ft=1.10

REVIEWED BY:

Robbins Engineering, Inc.

PO Box 280055

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd
	Lbs	Lbs	In-Sx In-Sx
A	243	52	3- 8 1- 8
			Hx = 45
C	56	0	3- 8 1- 8
B	88	39	3- 8 1- 8
			Hx = 30

For proper installation of  
toe-nails, refer to the 2001  
National Design Specification  
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-

Membr CSI P Lbs Ax1-CSI-Bnd  
-----Top Chords-----  
A -B 0.10 89 C 0.00 0.10  
-----Bottom Chords-----

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





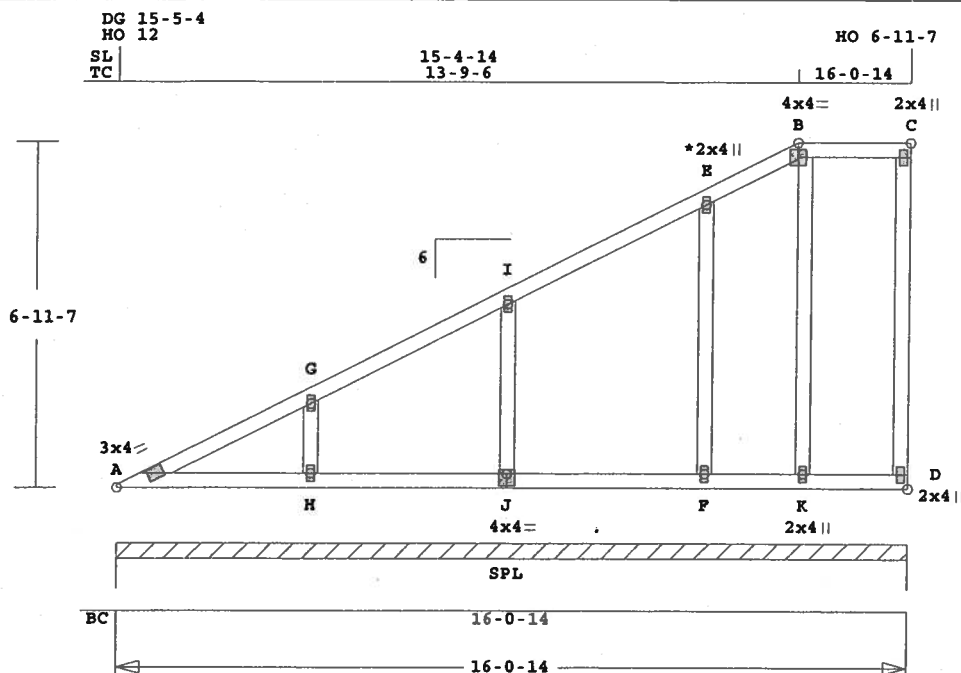
U# J#KEEN ROLLING MEADOWS - 30



Date Sealed: 10/17/2006

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
KEEN	V1	1	VLMHP.SB	160014	6	0	0	T06101774

U# J#KEEN ROLLING MEADOWS - 30



See Joint G For Typical Gable Plate Size and Placement

Scale: 0.257" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 106.6 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI	-Size-	----	Lumber----
TC	0.21	2x 4	SP-#2
BC	0.21	2x 4	SP-#2
WB	0.24	2x 4	SP-#2
GW	0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	16- 0-14
BC Cont.	0- 0- 0	16- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0

Spacing 24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 9 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 16- 0-14		
	1355	127	Hz =	418

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -G	0.13		212 C	0.00	0.13
G -I	0.15		143 C	0.00	0.15
I -E	0.15		112 C	0.00	0.15
E -B	0.08		144 T	0.00	0.08
B -C	0.21		146 T	0.01	0.20
-----Bottom Chords-----					
A -H	0.08		2 T	0.00	0.08
H -J	0.08		0 T	0.00	0.08

J	-F	0.09	0 T	0.00	0.09
F	-K	0.05	0 T	0.00	0.05
K	-D	0.21	0 T	0.00	0.21
-----Webs-----					
K	-B	0.01	102 C	0.01	0.00
D	-C	0.24	119 T	0.01	0.23
-----Gable Webs-----					
H	-G	0.03	234 C	0.02	0.01
J	-I	0.03	247 C	0.03	0.00
F	-E	0.05	256 T	0.03	0.02

TL Defl -0.01" in J -F L/999

LL Defl 0.00" in J -F L/999

Shear // Grain in G -I 0.14

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	3.0x	4.0	Ctr	Ctr	0.70
G	LOCK	2.0x	4.0	Ctr	Ctr	0.00
I	LOCK	2.0x	4.0	Ctr	Ctr	0.00
E	LOCK	2.0x	4.0	Ctr	Ctr	0.00
B	LOCK	4.0x	4.0	Ctr	Ctr	0.78
C	LOCK	2.0x	4.0	Ctr	Ctr	0.40
H	LOCK	2.0x	4.0	Ctr	Ctr	0.00
J	LOCK	4.0x	4.0	Ctr	Ctr	0.52
F	LOCK	2.0x	4.0	Ctr	Ctr	0.00
K	LOCK	2.0x	4.0	Ctr	Ctr	0.40
D	LOCK	2.0x	4.0	Ctr	Ctr	0.40

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

Provide connection to bearing

for 418 Lbs Horiz Reaction

Design checked for 10 psf non-

concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

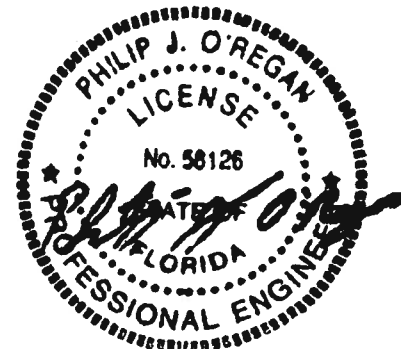
Max comp. force 247 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan

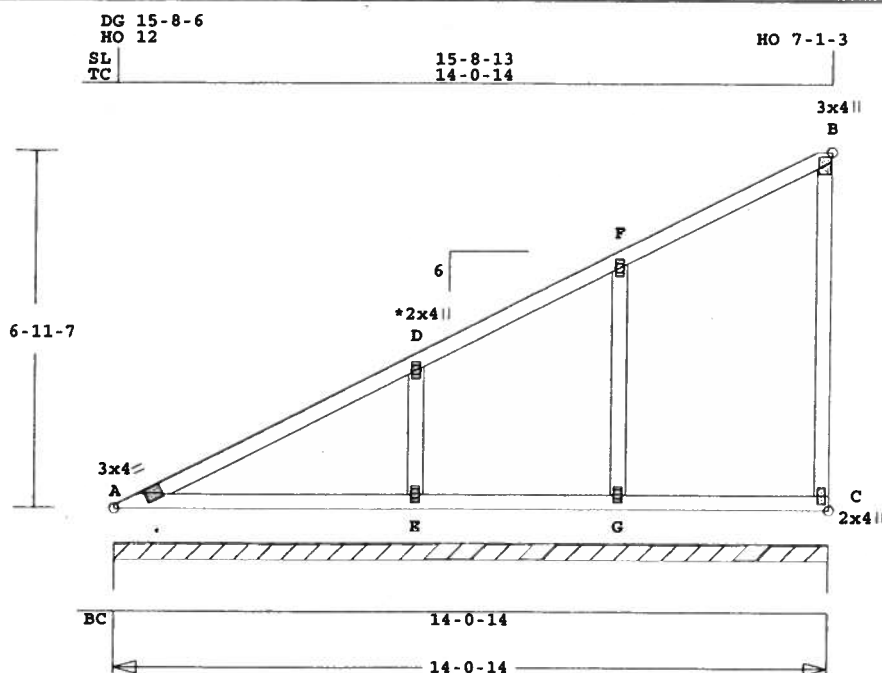
License # 58126

Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
KEEN	V2	1	VLM.SB	140014	6	0	0	T06101774

U# J#KEEN ROLLING MEADOWS - 30



Scale: 0.266" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 80.6 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI	Size	Lumber
TC	0.30	2x 4 SP-#2
BC	0.19	2x 4 SP-#2
WB	0.31	2x 4 SP-#2
GW	0.13	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 0-14
BC Cont.	0- 0- 0	14- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 14- 0-14		
	1278	123	Hz =	423

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A	-D	0.30	178	C	0.00 0.30
D	-F	0.19	124	C	0.00 0.19
F	-B	0.26	107	C	0.01 0.25

-----Bottom Chords-----					
A	-E	0.18	2	C	0.00 0.18
E	-G	0.11	0	T	0.00 0.11
G	-C	0.19	0	T	0.00 0.19

-----Webs-----					
C	-B	0.31	148	T	0.02 0.29
-----Gable Webs-----					
E	-D	0.13	289	C	0.00 0.13
G	-F	0.05	247	C	0.03 0.02

TL Defl -0.04" in A -E L/999  
LL Defl -0.02" in A -E L/999  
Shear // Grain in A -D 0.21

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	-	LOCK	20	Ga,	Gross	Area
Plate	-	RHS	20	Ga,	Gross	Area
Jt Type		Plt	Size	X	Y	JSI
A	LOCK	3.0x	4.0	Ctr	Ctr	0.67
D	LOCK	2.0x	4.0	Ctr	Ctr	0.00
F	LOCK	2.0x	4.0	Ctr	Ctr	0.00
B	LOCK	3.0x	4.0	Ctr	Ctr	0.30
E	LOCK	2.0x	4.0	Ctr	Ctr	0.00
G	LOCK	2.0x	4.0	Ctr	Ctr	0.00
C	LOCK	2.0x	4.0	Ctr	Ctr	0.38

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

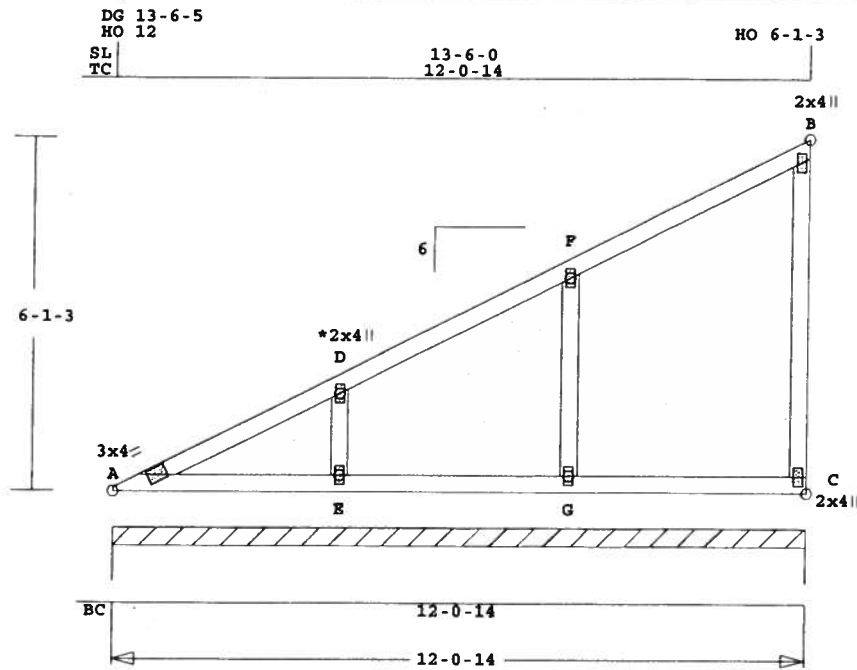
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
Provide connection to bearing  
for 423 Lbs Horiz Reaction  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor: 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 289 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
KEEN	V3	1	VLM.SB	120014	6	0	0	T06101774

U# J#KEEN ROLLING MEADOWS - 30



ALL PLATES ARE LOCK20  
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.300" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 67.0 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI	Size	Lumber
TC	0.21	2x 4 SP-#2
BC	0.13	2x 4 SP-#2
WB	0.26	2x 4 SP-#2
GW	0.06	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 0-14
BC Cont.	0- 0- 0	12- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 12- 0-14		
	1037	92	Hz	= 360

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -D	0.13	203	C	0.00 0.13
D -F	0.15	116	C	0.00 0.15
F -B	0.21	93	C	0.00 0.21

Bottom Chords					
A -E	0.08	2	T	0.00	0.08
E -G	0.09	0	T	0.00	0.09
G -C	0.13	0	T	0.00	0.13
Webs					
C -B	0.26	154	T	0.02	0.24
Gable Webs					
E -D	0.05	232	C	0.03	0.02
G -F	0.06	278	T	0.04	0.02

TL Defl -0.01" in G -C L/999  
LL Defl 0.00" in G -C L/999  
Shear // Grain in F -B 0.19

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr Ctr 0.64

D LOCK 2.0x 4.0 Ctr Ctr 0.00

F LOCK 2.0x 4.0 Ctr Ctr 0.00

B LOCK 2.0x 4.0 Ctr Ctr 0.38

E LOCK 2.0x 4.0 Ctr Ctr 0.00

G LOCK 2.0x 4.0 Ctr Ctr 0.00

C LOCK 2.0x 4.0 Ctr Ctr 0.38

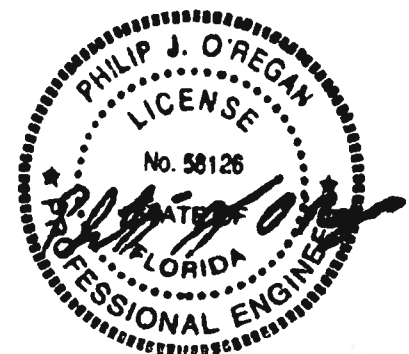
REVIEWED BY:

Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

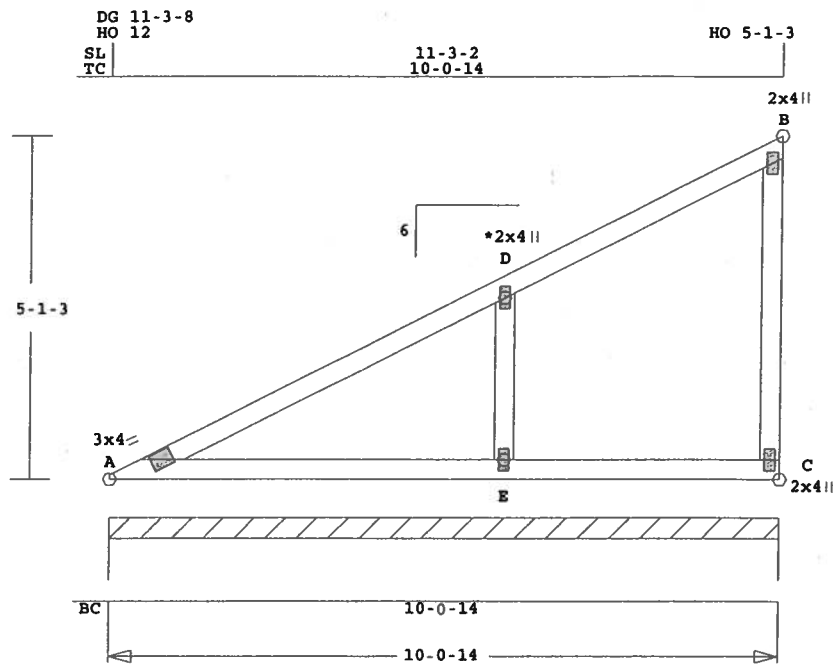
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004  
Provide connection to bearing  
for 360 Lbs Horiz Reaction  
Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 257 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
KEEN	V4	1	VLM.SB	100014	6	0	0	T06101774

U# J#KEEN ROLLING MEADOWS - 30



ALL PLATES ARE LOCK20  
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.348" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 52.5 LBS

A -E 0.19 2 C 0.00 0.19 FBC2004

E -C 0.13 0 T 0.00 0.13

C -B 0.22 160 T 0.02 0.20

E -D 0.12 350 T 0.04 0.08

TL Defl -0.04" in A -E L/999

LL Defl -0.01" in A -E L/999

Shear // Grain in A -D 0.21

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 3.0x 4.0 Ctr Ctr 0.60

D LOCK 2.0x 4.0 Ctr Ctr 0.00

B LOCK 2.0x 4.0 Ctr Ctr 0.38

E LOCK 2.0x 4.0 Ctr Ctr 0.00

C LOCK 2.0x 4.0 Ctr Ctr 0.38

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI	Size	Lumber
TC	0.31	2x 4 SP-#2
BC	0.19	2x 4 SP-#2
WB	0.22	2x 4 SP-#2
GW	0.12	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 0-14
BC Cont.	0- 0- 0	10- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	0 to 10- 0-14		
	958	99	Hz	= 296

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -D	0.31	131	C	0.00	0.31
D -B	0.21	79	C	0.00	0.21
-----Bottom Chords-----					

REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

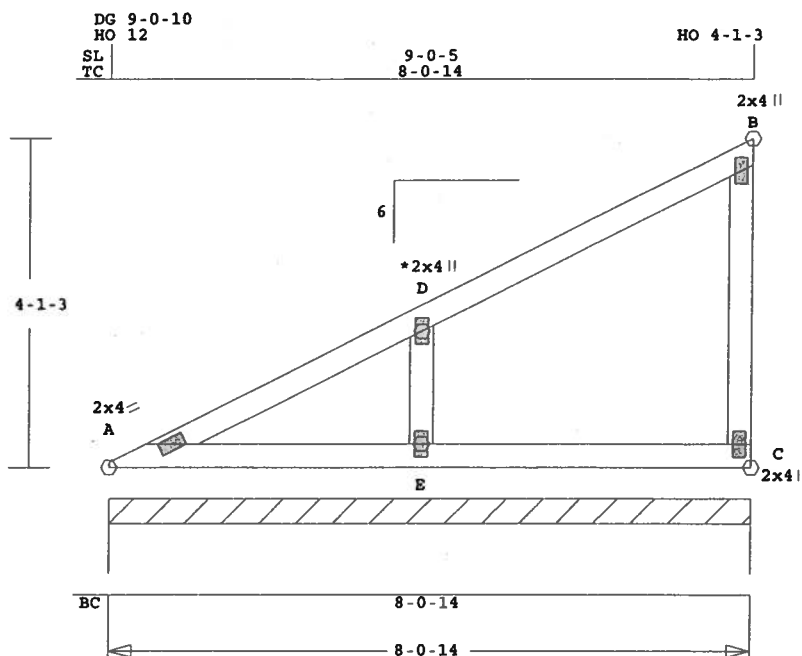
NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 306 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682



Job <b>KEEN</b>	Mark <b>V5</b>	Quan <b>1</b>	Type <b>VLM.SB</b>	Span <b>80014</b>	P1-H1 <b>6</b>	Left OH <b>0</b>	Right OH <b>0</b>	Engineering <b>T06101774</b>
U# J#KEEN ROLLING MEADOWS - 30								



ALL PLATES ARE LOCK20  
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.415" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.7 LBS

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI	Size	Lumber
TC	0.18	2x 4 SP-#2
BC	0.09	2x 4 SP-#2
WB	0.21	2x 4 SP-#2
GW	0.09	2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 0-14
BC Cont.	0- 0- 0	8- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

A - E	0.08	2 T	0.00	0.08
E - C	0.09	0 T	0.00	0.09
C - B	0.21	183 T	0.02	0.19
E - D	0.09	325 T	0.04	0.05
TL Defl	-0.01"	in E - C	L/999	
LL Defl	0.00"	in E - C	L/999	
Shear // Grain		in D - B	0.20	

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.  
BASED ON SP LUMBER  
USING GROSS AREA TEST.

Plate	LOCK	20 Ga	Gross Area	
Plate - LOCK	20 Ga <td>Gross Area</td> <td></td>	Gross Area		
Jt Type	Plt Size	X	Y	JSI
A LOCK	2.0x 4.0	Ctr	Ctr	0.71
D LOCK	2.0x 4.0	Ctr	Ctr	0.00
B LOCK	2.0x 4.0	Ctr	Ctr	0.38
E LOCK	2.0x 4.0	Ctr	Ctr	0.00
C LOCK	2.0x 4.0	Ctr	Ctr	0.38

Design checked for 10 psf non-concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.  
Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 251 Lbs  
Quality Control Factor 1.25

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	8- 0-14	
	717	68	Hz	233

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -D	0.14	165	C	0.00	0.14
D -B	0.18	63	C	0.00	0.18
-----Bottom Chords-----					

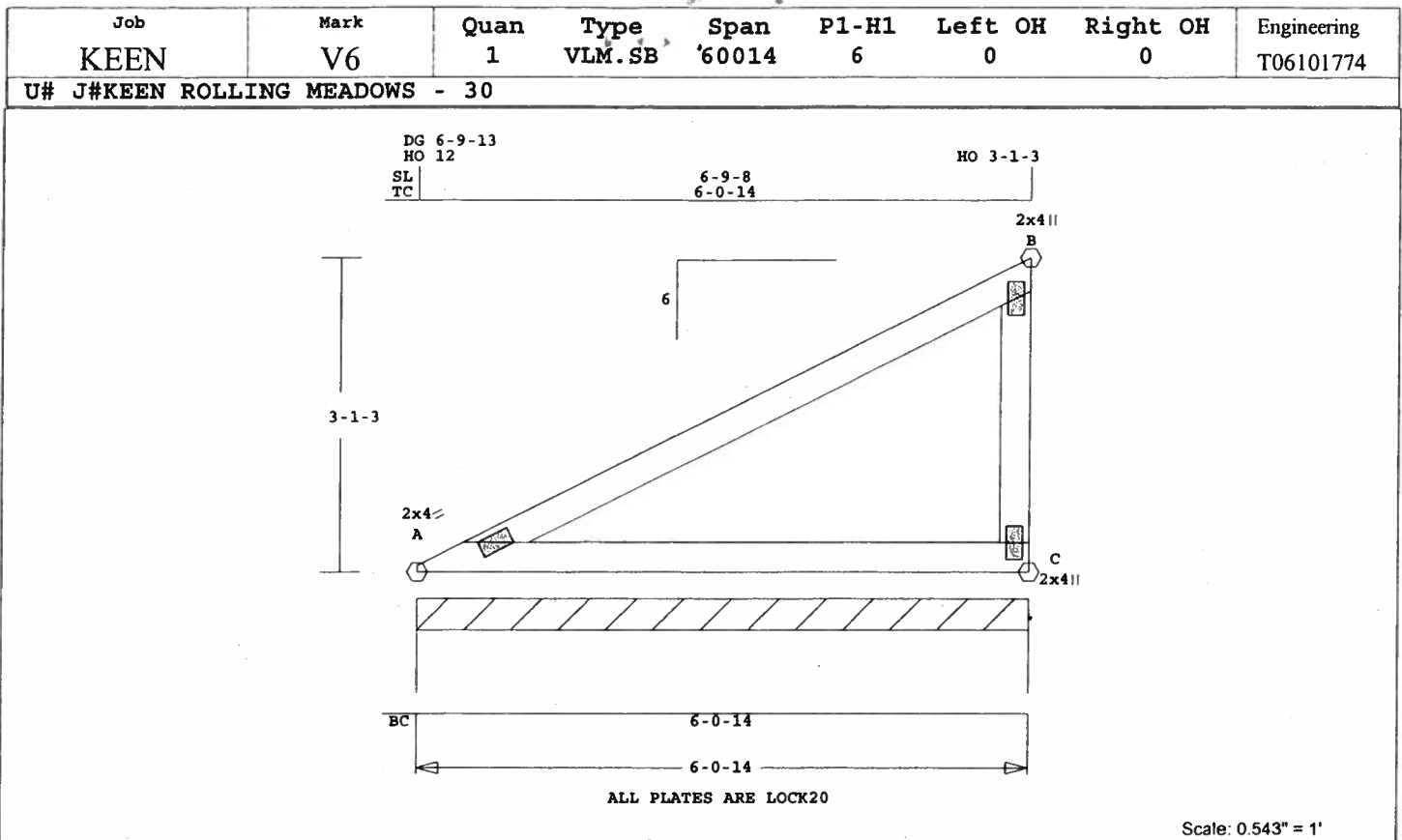
REVIEWED BY:  
Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:  
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus" APPROX. TRUSS WEIGHT: 28.0 LBS  
C -B 0.03 255 T WindLd

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----  
TC 0.40 2x 4 SP-#2  
BC 0.29 2x 4 SP-#2  
WB 0.03 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	6- 0-14
BC Cont.	0- 0- 0	6- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

TL Defl -0.08" in A -C L/923  
LL Defl -0.03" in A -C L/999  
Shear // Grain in A -B 0.24

Plates for each ply each face.  
PLATING CONFORMS TO TPI.  
REPORTS: SBCCI 9761  
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area  
Plate - RHS 20 Ga, Gross Area  
Jt Type Plt Size X Y JSI  
A LOCK 2.0x 4.0 Ctr Ctr 0.67  
B LOCK 2.0x 4.0 Ctr Ctr 0.38  
C LOCK 2.0x 4.0 Ctr Ctr 0.38

REVIEWED BY:

Robbins Engineering, Inc.  
PO Box 280055  
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	6- 0-14	
	664	81	Hz =	173

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.40	64	C	0.00	0.40
-----Bottom Chords-----					
A -C	0.29	3	T	0.00	0.29
-----Webs-----					

NOTES:

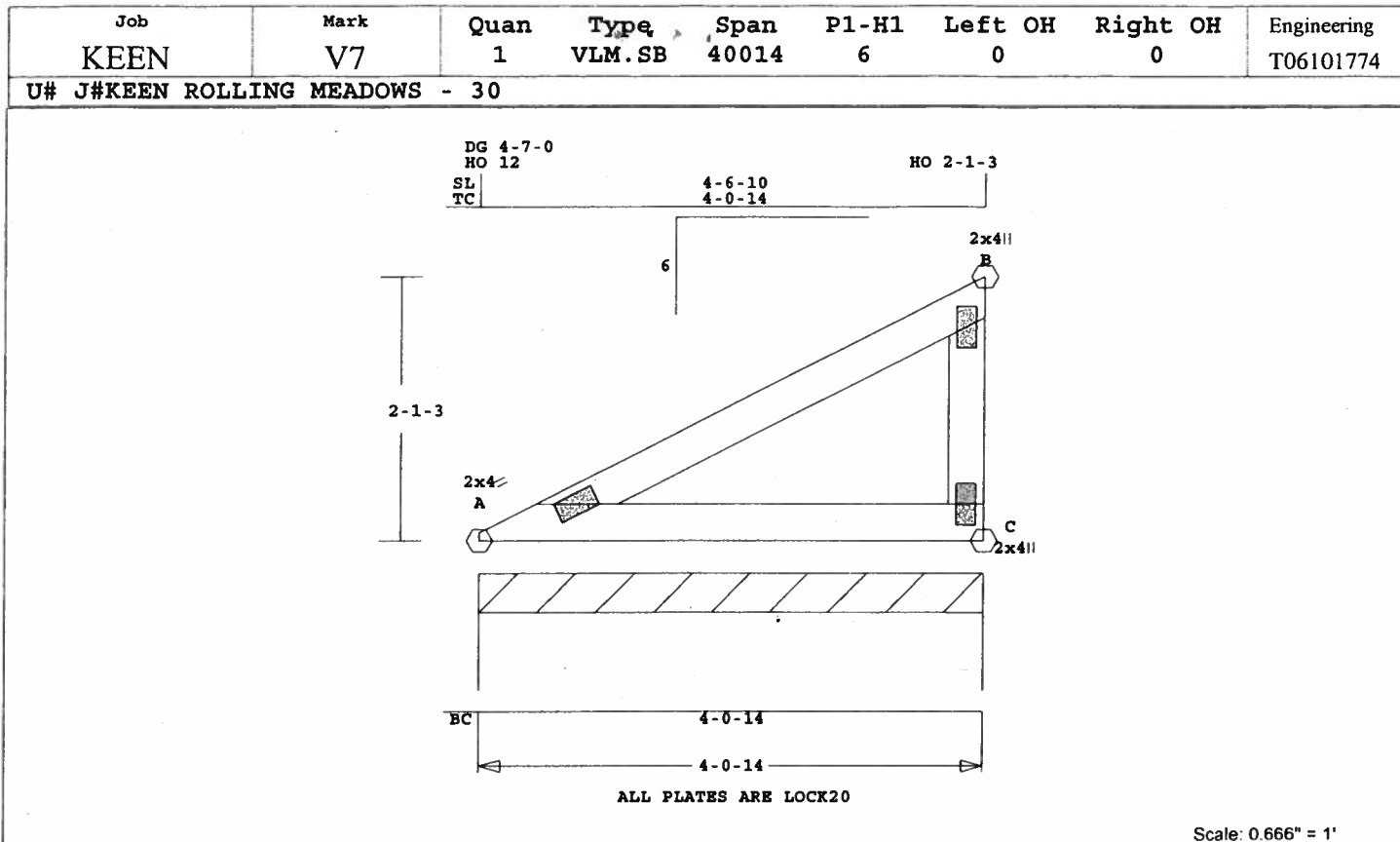
Trusses Manufactured by:  
Mayo Truss Co. Inc.  
Analysis Conforms To:  
FBC2004

Design checked for 10 psf non-  
concurrent LL on BC.  
Wind Loads - ANSI / ASCE 7-02  
Truss is designed as  
Components and Claddings\*  
for Exterior zone location.

Wind Speed: 110 mph  
Mean Roof Height: 15-0  
Exposure Category: B  
Occupancy Factor : 1.00  
Building Type: Enclosed  
TC Dead Load: 5.0 psf  
BC Dead Load: 5.0 psf  
Max comp. force 155 Lbs  
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License #: 58126  
Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 18.1 LBS

C -B 0.02 186 T WindLd

Online Plus -- Version 20.0.001

RUN DATE: 17-OCT-06

CSI -Size- ----Lumber----

TC 0.18 2x 4 SP-#2

BC 0.14 2x 4 SP-#2

WB 0.02 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 0-14
BC Cont.	0- 0- 0	4- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 8 Wind Load Case(s)  
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	4- 0-14	
	424	50	Hz	108

Membr	CSI	P	Lbs	Axl	CSI-Bnd
-----Top Chords-----					
A -B	0.18		54 C	0.00	0.18
-----Bottom Chords-----					
A -C	0.14		4 T	0.00	0.14
-----Webs-----					

TL Defl -0.01" in A -C L/999

LL Defl -0.01" in A -C L/999

Shear // Grain in A -B 0.18

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.65

B LOCK 2.0x 4.0 Ctr Ctr 0.38

C LOCK 2.0x 4.0 Ctr Ctr 0.38

REVIEWED BY:

Robbins Engineering, Inc.

PO Box 280055

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL  
NOTES AND SYMBOLS SHEET FOR  
ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

FBC2004

Design checked for 10 psf non-  
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings\*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

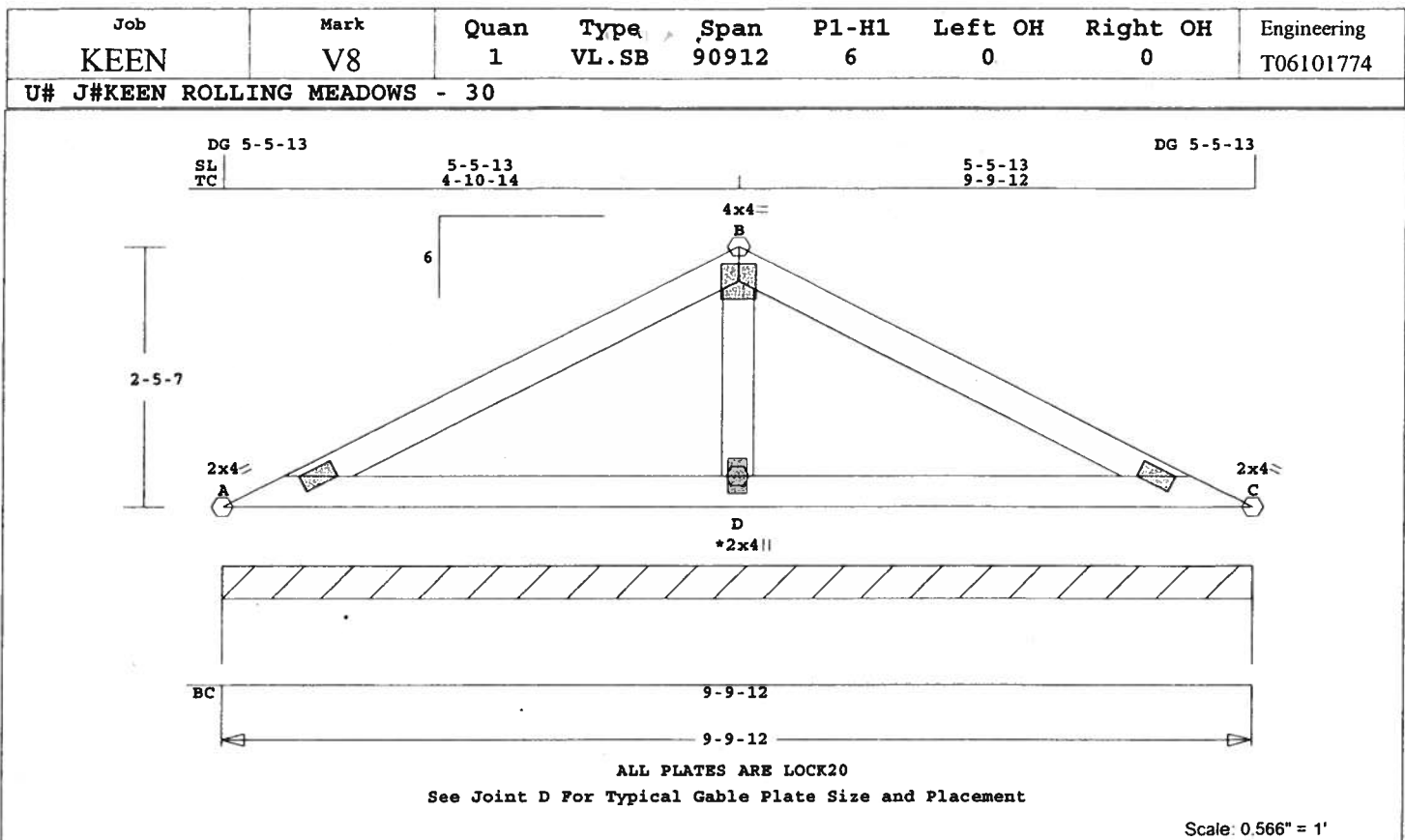
Max comp. force 100 Lbs

Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682







Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.3 LBS

D -C 0.14 2 T 0.00 0.14 Truss is designed as

-----Gable Webs-----

D -B 0.03 208 T

Components and Claddings\*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 15-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 5.0 psf

BC Dead Load: 5.0 psf

Max comp. force 191 Lbs

Quality Control Factor 1.25

Online Plus -- Version 20.0.001  
RUN DATE: 17-OCT-06

CSI -Size-	----	Lumber----
TC	0.18	2x 4 SP-#2
BC	0.14	2x 4 SP-#2
GW	0.03	2x 4 SP-#2

TL Defl -0.01" in D -C L/999  
LL Defl -0.01" in D -C L/999  
Shear // Grain in A -B 0.18

Plates for each ply each face.  
PLATING CONFORMS TO TPI.

REPORTS: SBCCI 9761

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.75

B LOCK 4.0x 4.0 Ctr Ctr 0.50

C LOCK 2.0x 4.0 Ctr Ctr 0.75

D LOCK 2.0x 4.0 Ctr Ctr 0.00

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 9-12
BC Cont.	0- 0- 0	9- 9-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

REVIEWED BY:

Robbins Engineering, Inc.

PO Box 280055

Tampa, FL 33682

Plus 9 Wind Load Case(s)

Plus 1 UBC LL Load Case(s)

Jt	React	Uplift	Size	Req'd
	Lbs	Lbs	In-Sx	In-Sx
Cont. Brg	0- 0- 0	to	9- 9-12	
	1036	149	Hz =	70

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

A -B 0.18 110 T 0.01 0.17

B -C 0.18 110 T 0.01 0.17

-----Bottom Chords-----

A -D 0.14 2 T 0.00 0.14

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by:

Mayo Truss Co. Inc.

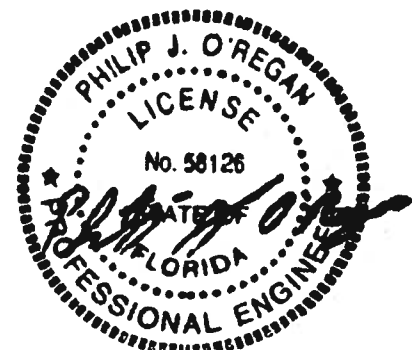
Analysis Conforms To:

FBC2004

Design checked for 10 psf non-concurrent LL on BC.

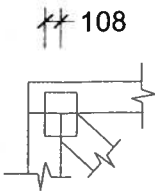
Wind Loads - ANSI / ASCE 7-02

Truss Design Engineer: Philip J. O'Regan  
License # 58126  
Address: P.O. Box 280055, Tampa, FL 33682



# ROBBINS ENG. GENERAL NOTES & SYMBOLS

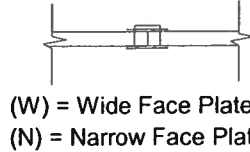
## PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5" ) or IN-16ths (i.e. 108)

## FLOOR TRUSS SPLICE

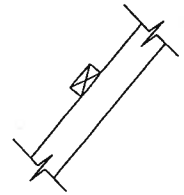
( 3X2, 4X2, 6X2 )



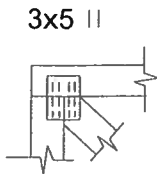
(W) = Wide Face Plate  
(N) = Narrow Face Plate

## LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.



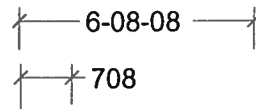
## PLATE SIZE AND ORIENTATION



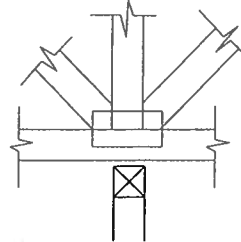
The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

## DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08 ). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



708



W = Actual Bearing Width (IN-SX)  
R = Reaction (lbs.)  
U = Uplift (lbs.)

## BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA ), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

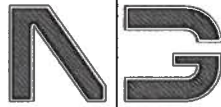
Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.



6904 Parke East Blvd.  
Tampa, FL 33610-4115  
Tel: 813-972-1135 Fax: 813-971-6117

[www.robbseng.com](http://www.robbseng.com)



NICHOLAS  
PAUL  
GEISLER

ARCHITECT  
N.C.A.R.B. Certified

1758 NW Brown Road  
Lake City, FL 32055  
386/755-9021

Permit # 25204

21 NOVEMBER 2006

JOE HALTIWANGER, BUILDING OFFICIAL  
COLUMBIA COUNTY, BUILDING DEPT.  
COLUMBIA COUNTY COURTHOUSE ANNEX  
LAKE CITY, FLORIDA 32055

RE: GLEN KEEN RESIDENCE  
PLAN REVIEW Nr.: \_\_\_\_\_

/ JASON ELIXON, LLC.

DEAR SIR:

PLEASE BE ADVISED OF THE FOLLOWING CLARIFICATION FOR THE  
FOUNDATION STEMWALL OF THE ABOVE REFERENCED PROJECT:

1. DUE TO A SLOPING SITE, PORTIONS OF THE PROPOSED STEMWALL SHALL BE PERMITTED TO BE CONSTRUCTED WITH UP TO SIX BLOCK COURSES W/ 1 #5 REBAR, VERTICAL AT 48" O.C., HOOKED TO THE FOOTING BELOW AND THE BOND BEAM, ABOVE, AT SLAB LEVEL. THIS CHANGE PROVIDES UP TO AN ADDITIONAL 24" OF STEMWALL HEIGHT TO COMPENSATE FOR THE NATURAL SLOPE OF THE SITE.
2. NOTE: STEMWALL HEIGHTS IN EXCESS OF SIX BLOCK COURSES WILL REQUIRE PLACEMENT OF THE VERTICAL REINFORCING AT 32" O.C., UP TO EIGHT BLOCK COURSES AND AT 24" O.C. FOR STEMWALL HEIGHTS OF NINE BLOCK COURSES UP TO TWELVE BLOCK COURSES.
3. ALL STEMWALLS IN EXCESS OF SIX BLOCK COURSES SHALL REQUIRE HORIZONTAL JOINT REINFORCEMENT: #3 LADDER TYPE.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY,  
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005



**Donald F. Lee & Associates, Inc.**

**Surveyors & Engineers**

140 NW Ridgewood Avenue  
Lake City, Florida 32055  
(386) 755-6166  
Fax (386) 755-6167  
donald@dlfa.com

Permit # 25204

**Monday, November 27, 2006**

**FROM: Tim Delbene, P.L.S.**

**TO: Columbia County Building & Zoning Dept.**

**CC: K & H Framing**

**RE: Foundation Elevation Check – Lot 30, Rolling Meadows**

We have obtained elevations on a foundation under construction on the above referenced lot. The elevations are based on Local Benchmark Datum. The results are as follows:

**Floor Elevation (at Stemwall): 107.94'**

The record subdivision plat for Rolling Meadows indicates a minimum floor elevation of 107.5' for the subject Lot 30.

SIGNED:

  
Timothy A. Delbene, P.L.S.  
Florida Reg. Cert. No. 5594

DATE: 11/27/2006.

# COLUMBIA COUNTY OFFICE ALLEN

## OCCUPANCY

COLUMBIA COUNTY, FLORIDA

### Department of Building and Zoning Inspection

*This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.*

Parcel Number 15-4S-16-03023-530

Building permit No. 000025204

Use Classification SFD, UTILITY

Fire: 27.90

Permit Holder JASON ELIXSON

Waste: 83.75

Owner of Building JOHN KEEN/A&B MANAGEMENT, LLC

Total: 111.65

Location: 159 SW POPPY GLEN, LAKE CITY, FL

Date: 05/07/2007

*John V. Force*

Building Inspector



POST IN A CONSPICUOUS PLACE  
(Business Places Only)

# Notice of Treatment

12335

**Applicator:** Florida Pest Control & Chemical Co. (www.flapest.com)

Address: Bay Area

City: LAKE CITY

Phone: 52.1703

**Site Location:** Subdivision Rolling Meadows

Lot # 30

Block#

Permit # 25204

Address 159 SW Poppy St

**Product used**

**Active Ingredient**

**% Concentration**

☐ Premise

Imidacloprid

0.1%

☐ Termidor

Fipronil

0.12%

☒ Bora-Care

Disodium Octaborate Tetrahydrate

23.0%

**Type treatment:**

☐ Soil

☒ Wood

**Area Treated**

**Square feet**

**Linear feet**

**Gallons Applied**

Dwelling

2386

4

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line \_\_\_\_\_.

2/2/07

Date

1200

Time

F254

Print Technician's Name

Remarks: \_\_\_\_\_

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05

©