



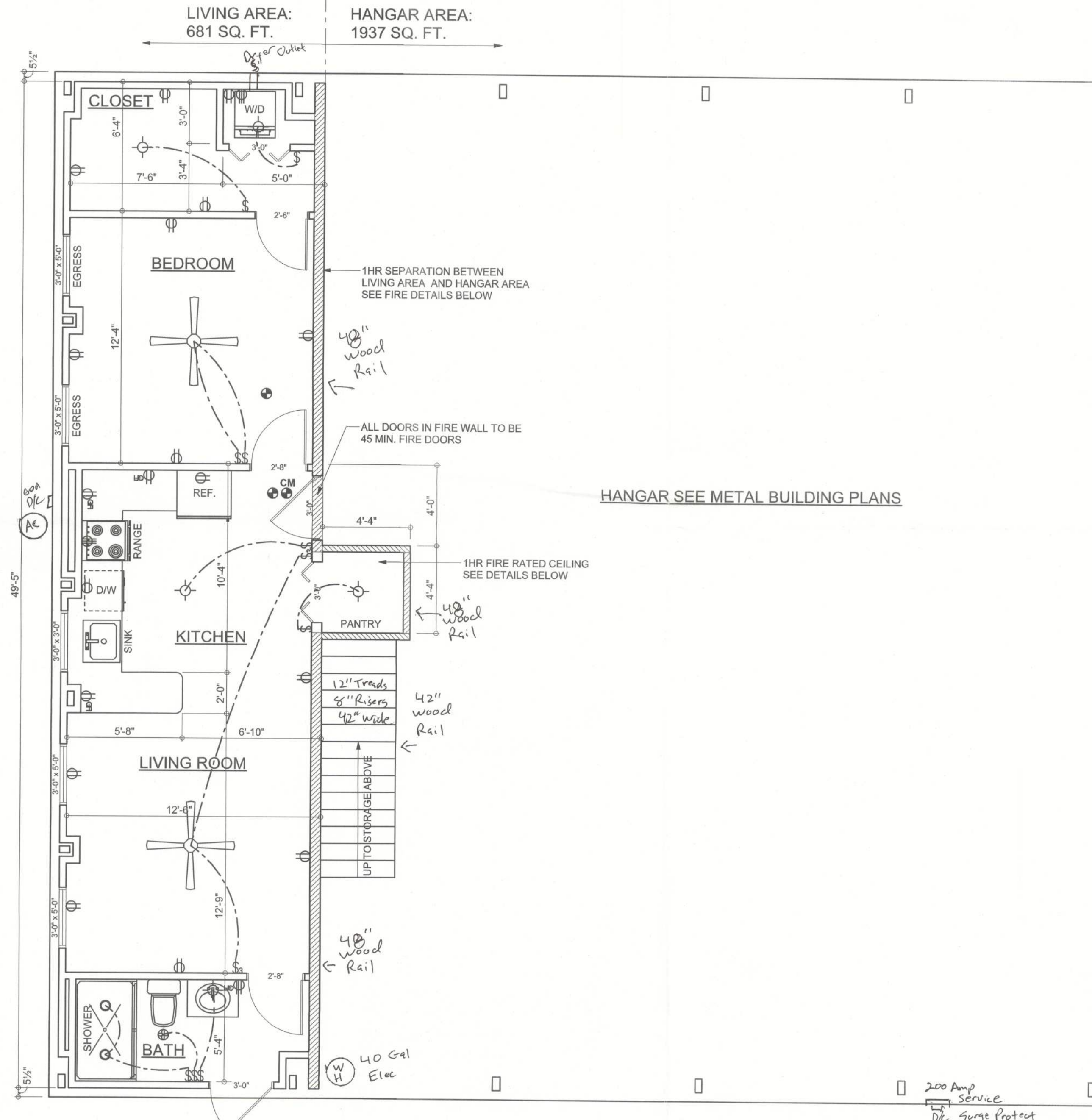
CEILING FRAMING PLAN
SCALE: 1/4" = 1'-0"

FLOOR DESIGN LOAD:
40 PSF LIVE
10 PSF DEAD

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	CARBON MONOXIDE ALARM

ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DEN, BEDROOMS, SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING.
SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.
- E -11 CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.
- E -12 ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.



BUILD-OUT FLOOR & ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

FIRE PARTITION WALL DETAILS:

WALLS AND INTERIOR PARTITIONS, WOOD-FRAMED

GA FILE NO. WP 3605	GENERIC	1 HOUR FIRE	30 to 34 STC SOUND
GYPSUM WALLBOARD, WOOD STUDS One layer 5/8" type X plain or predecorated gypsum wallboard, water-resistant gypsum backing board, or gypsum veneer base applied parallel or at right angles to each side of 2 x 6 wood studs 16" o.c. with 6d coated nails, 1 7/8" long, 0.0915" shank, 1/4" heads, 7" o.c. Joints of square edge, bevel edge or predecorated wallboard may be left exposed. Joints staggered 16" on opposite sides. (LOAD-BEARING)			
Thickness: 4 7/8" Approx. Weight: 7 psf Fire Test: UL R1319-4, -6, 6-17-52; UL R2717-39, 1-20-66; UL R3501-52, 3-15-66; UL Design U305; ULC Design W301 OR 64-8, 2-4-64 Sound Test:			

FLOOR-CEILING SYSTEMS, WOOD-FRAMED

GA FILE NO. FC 5406	GENERIC	1 HOUR FIRE	35 to 39 STC SOUND
WOOD JOISTS, GYPSUM WALLBOARD Base layer 5/8" type X gypsum wallboard applied at right angles to 2 x 10 wood joists 24" o.c. with 1 1/4" Type W or S drywall screws 24" o.c. Face layer 5/8" type X gypsum wallboard or gypsum veneer base applied at right angles to joists with 1 7/8" Type W or S drywall screws 12" o.c. at joints and intermediate joists and 1 1/2" Type G drywall screws 12" o.c. placed 2" back on either side of end joints. Joints offset 24" from base layer joints. Wood joists supporting 1/2" plywood with exterior glue applied at right angles to joists with 8d nails. Ceiling provides one hour fire resistance protection for framing, including trusses.			
Approx. Ceiling Weight: 5 psf Fire Test: ITS, 8-6-98 Sound Test: Estimated			

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

SCOPE OF WORK

THIS ENGINEERING IS ONLY FOR THE FOUNDATION DESIGN TO MEET THE DESIGN LOADS OF THE METAL BUILDING DESIGNER. I AM NOT ENGINEER OF RECORD AND DID NOT DESIGN THE BUILDING OR REVIEW THE DESIGN OF THE BUILDING. SINCE THIS IS A SINGLE FAMILY RESIDENTIAL PROJECT THE OWNER AND BUILDER ARE RESPONSIBLE TO RESOLVE ANY GAPS IN ENGINEERING. THE METAL BUILDING PLANS CALL OUT A DOOR WHICH DOES NOT HAVE FLORIDA PRODUCT APPROVAL. THERE APPEARS TO BE NO SPECIFICATION FOR THE DOOR ATTACHMENT TO THE BUILDING OR BRACING TO WITHSTAND WIND PRESSURE AT THE TOP OF THE DOOR, APPROX. +/- 350 PLF X 40' = 14000 LB. ALSO, I'M NOT SURE IF THE METAL BUILDING ROOF NEEDS TO ACT AS A DIAPHRAGM OR THE WALLS NEED TO ACT AS SHEAR WALLS AND THE BUILDING ENGINEER DID NOT SPECIFY ROOFING OR WALL MATERIAL BUT THE ROOF AND WALLS MAY NEED OSB SHEATHING SINCE LIGHT GAGE SHEET METAL, 29GA OR 26GA MAY NOT HAVE ADEQUATE SHEAR CAPACITY. CHECK PRODUCT APPROVAL DATA FOR SHEAR CAPACITY, NOT WIND PRESSURE OR SUCTION.



WIND LOAD ENGINEER:
Mark Disoway, P.E.
No. 53915, P.O. Box 868, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions approximate scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location and is limited to just the design of the foundation based on the loads provided by the builder.



Doroshenko Hangar

(Metal Building Foundation)

ADDRESS:
252 SW Voyager Ct.
Lake City, Florida

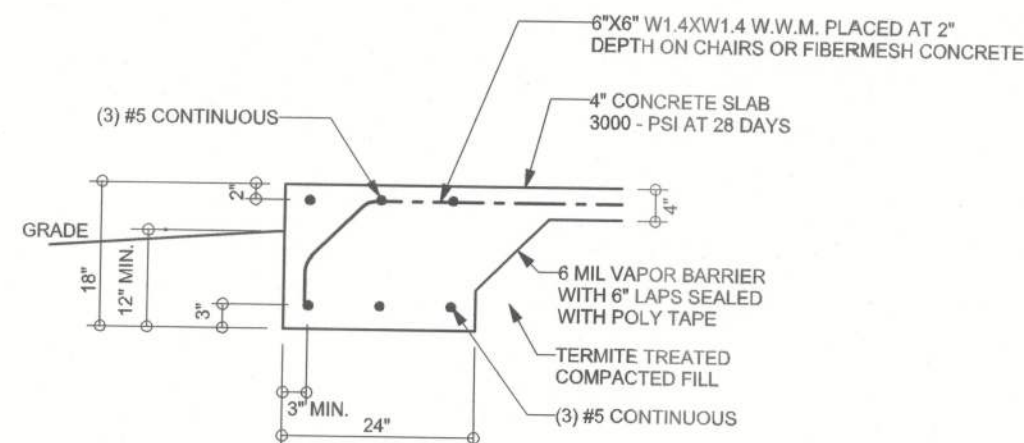
Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
Friday, January 24, 2014

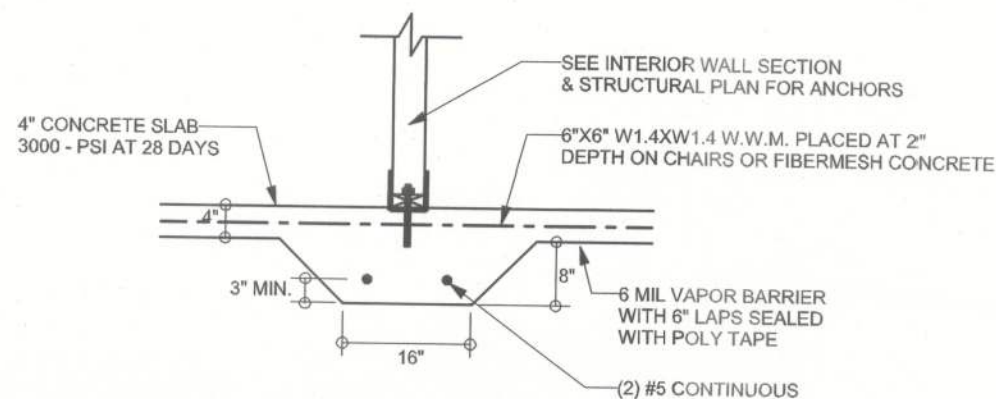
DRAWN BY: STRUCTURAL BY:

FINALS DATE:
21Jan14

JOB NUMBER:
1312045a
DRAWING NUMBER



SECTION A-A
SCALE: 1/2" = 1'-0"



SECTION B-B
SCALE: 1/2" = 1'-0"

DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CODE, SECTION 1609			
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT)			
BUILDING IS NOT IN THE HIGH-VELOCITY HURRICANE ZONE			
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION			
1.) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 35 FT)			
2.) WIND EXPOSURE = C; BUILDER MUST FIELD VERIFY			
3.) TOPOGRAPHIC FACTOR = 1.0; BUILDER MUST FIELD VERIFY			
4.) BUILDING CATEGORY = II, (MRI = 700 YR)			
5.) ROOF ANGLE = 7.46 DEGREES			
6.) MEAN ROOF HEIGHT = 30 FT			
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)			
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))			
Zone	Effective Wind Area (ft ²)		
1	39	43	
2	39	-68	
3	39	-100	
4	43	-46	
5	43	-57	
Garage Door			
2010 FBCR, Table R301.2.4(4)			
6x7 Garage Door	37	-42	
18x7 Garage Door	36	-40	
SOIL BEARING CAPACITY 1500PSF			
NOT IN FLOOD ZONE (BUILDER TO VERIFY)			

NOTE: THIS FOUNDATION DESIGN MEETS ALL REQUIREMENTS FOR WIND LOADS PER FBC2010, SECTION 1609, 130 MPH BASIC WIND SPEED, EXPOSURE C, EXACT ANCHOR BOLT LOCATIONS AND SIZES ARE PER METAL BUILDING SEALED ENGINEERING ANCHOR BOLT PLAN.

- ANCHOR BOLTS AND REINFORCEMENT -
(ANCHOR BOLTS ARE TO BE HEADED ANCHORS WITH HEADS 3" FROM BOTTOM OF FOUNDATION. DO NOT USE "L" OR "J" BOLTS)
ANCHOR BOLTS, BOLT DIAMETER, AND LOCATION PER
METAL BUILDING SEALED ENGINEERING DESIGN DRAWINGS. TIE ANCHOR BOLTS TO BOTTOM REINFORCING STEEL.
REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, WELDED WIRE FABRIC SHALL CONFORM TO
ASTM A185, DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI
DETAILING MANUAL, SP-66, AND ACI318. REINFORCING SHALL NOT BE HEATED OR WELDED. REINFORCING SHALL
BE APPROVED BY ENGINEER OR HIS REPRESENTATIVE BEFORE CONCRETE IS PLACED. PROVIDE 3" COVER FOR
EXPOSED FOOTING SURFACES, 2" COVER FOR FORMED EXPOSED SURFACES, 3/4" COVER FOR NOT EXPOSED
SURFACES. LAP SPLICES SHALL BE 48 BAR DIAMETERS.

SITE PREPARATION:
SITE ANALYSIS AND PREPARATION ARE NOT PART OF THIS PLAN AND ARE RESPONSIBILITY OF THE OWNER. SITE INSPECTION BY
BUILDER OR BUILDING OFFICIAL SHALL DETERMINE IF THERE IS ANY EVIDENCE OF UNSUITABLE BEARING MATERIALS. IF THERE IS ANY QUESTION,
CALL A GEOTECHNICAL ENGINEER TO ASSURE THAT EXPANDING CLAYS AND OTHER PROBLEMATIC SOIL CONDITIONS DO NOT EXIST OR TO ALLOW
MITIGATION SHOULD THEY EXIST. ALL FILL UNDER STRUCTURAL ELEMENTS SHALL BE CLEAN SAND/SOIL FILL, FREE FROM DEBRIS AND ORGANIC
MATERIALS COMPACTED TO 95% OF MAXIMUM DRY BEARING CAPACITY. IN LIFTS OF NOT MORE THAN 6 INCHES. IT IS THE OWNER'S/BUILDER'S
RESPONSIBILITY TO VERIFY EXISTING SOIL AND CLEAN FILL ARE COMPACTED STABLE SOIL CONDITIONS WITH 1500 PSF BEARING CAPACITY OR TO
REQUEST FOUNDATION DESIGN BASED ON ACTUAL SITE CONDITIONS.

FOUNDATION:
THE OWNER HAS NOT YET PROVIDED A GEOTECHNICAL REPORT TO THE ENGINEER. ASSUMED SAFE BEARING CAPACITY OF 1500 PSF
SHALL BE APPROVED BY THE OWNER. FOOTINGS AND SLAB ARE TO BEAR ON FIRM UNDISTURBED EARTH OR CLEAN SAND/SOIL FILL, FREE FROM
DEBRIS AND ORGANIC MATERIALS COMPACTED IN LIFTS OF NOT MORE THAN 6 INCHES. WHERE UNACCEPTABLE MATERIAL
OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL. FOUNDATION WORK MUST BE COORDINATED WITH UNDERGROUND UTILITIES. FOOTINGS
SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. TO MINIMIZE WEATHERING, THE LAST 6 INCHES OF EXCAVATION FOR ALL FOOTINGS
SHALL BE MADE IMMEDIATELY PRIOR TO PLACEMENT OF FOOTINGS.

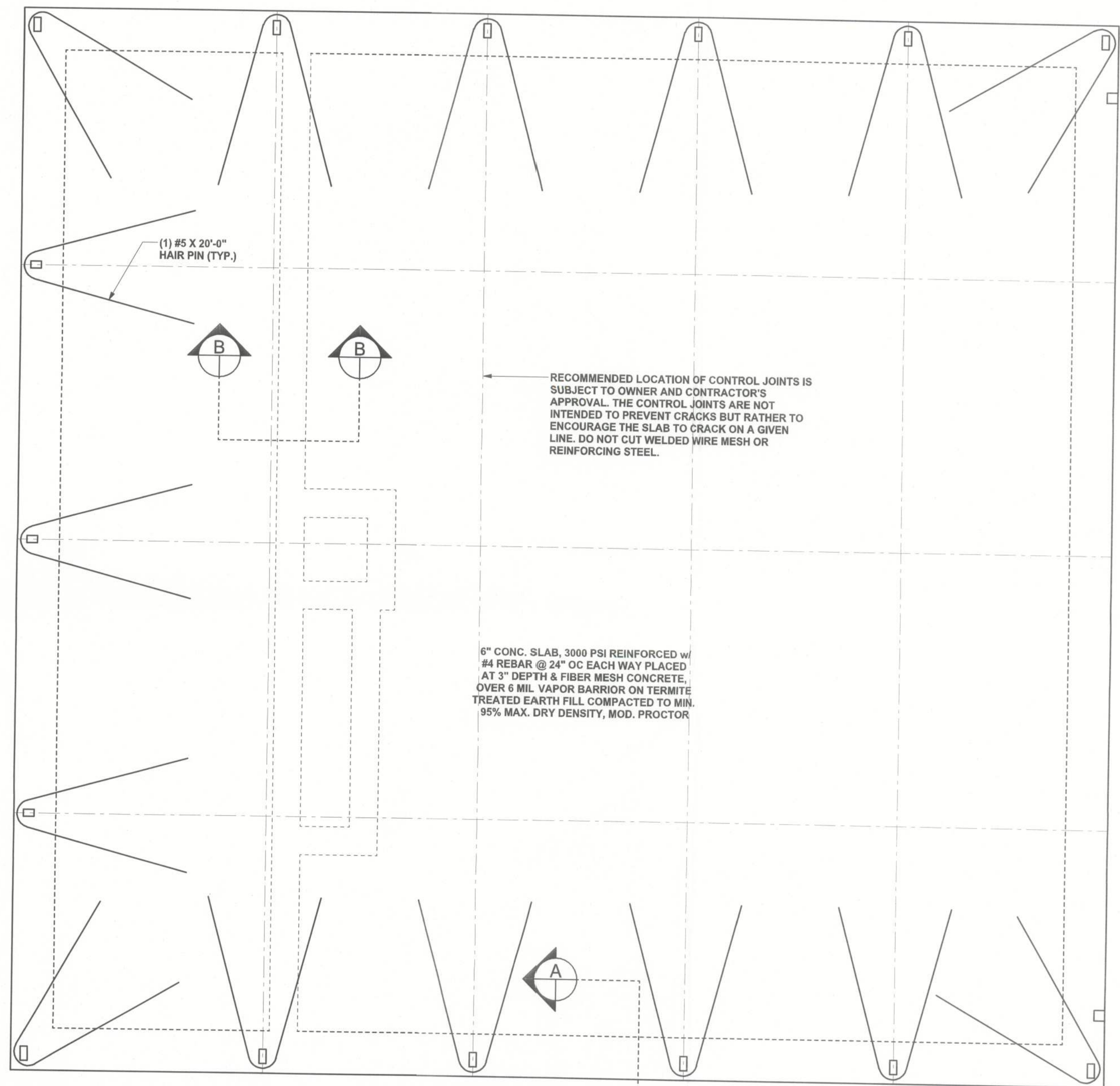
CONCRETE:
MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE $f'_c = 3000$ PSI. WHERE EXCESS WATER IS ADDED TO THE
CONCRETE SO THAT ITS SERVICEABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM
PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE
SHALL BE VIBRATED, NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE
ENGINEER, OWNER, OR HIS REPRESENTATIVE.

FIBER CONCRETE SLAB:
CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/4 INCH TO
2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S
RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION
OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

WELDED WIRE REINFORCED SLAB:
6"x6" W1.4XW1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185;
LOCATED IN THE MIDDLE OF THE SLAB: SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACING NOT TO EXCEED 3'.

REBAR:
ASTM A 615, GRADE 60, REINFORCED BARS, FY = 60 KSI. ALL LAP SPLICES 40" DB (30" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE
DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-95 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 30 INCHES.

CONCRETE CONTROL JOINTS:
WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS
SHALL BE CUT WITH IN 12 HOURS OF SLAB PLACEMENT. LENGTH/WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING
OF CUTS TO BE 12FT. DO NOT CUT W.W.M. OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER
AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO
CRACK ON A GIVEN LINE.)



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

NOTE: SEE ANCHOR BOLT PLAN FOR DIMENSIONS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

SCOPE OF WORK

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FOUNDATION DESIGN

BASED ON COLUMN REACTIONS BY:
STEPHEN P. MASLAN & CO.
ENGINEERS / DESIGNERS
8011 PASEO SUITE 201
KANSAS CITY, MO 64131
816.444.6260
DRAWING # E1
DATED 11/11/13

WINDLOAD ENGINEER:

Mark Disosway, P.E.
No. 59915, P.O. Box 868, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location and is limited to just the design of the foundation based on the loads provided by the builder.

MARK DISOSWAY
P.E. 59915
PROFESSIONAL ENGINEER
FLORIDA
Friday, January 24, 2014
SEAL

Doroshenko Hangar

(Metal Building Foundation)

ADDRESS:
252 SW Voyager Ct.
Lake City, Florida

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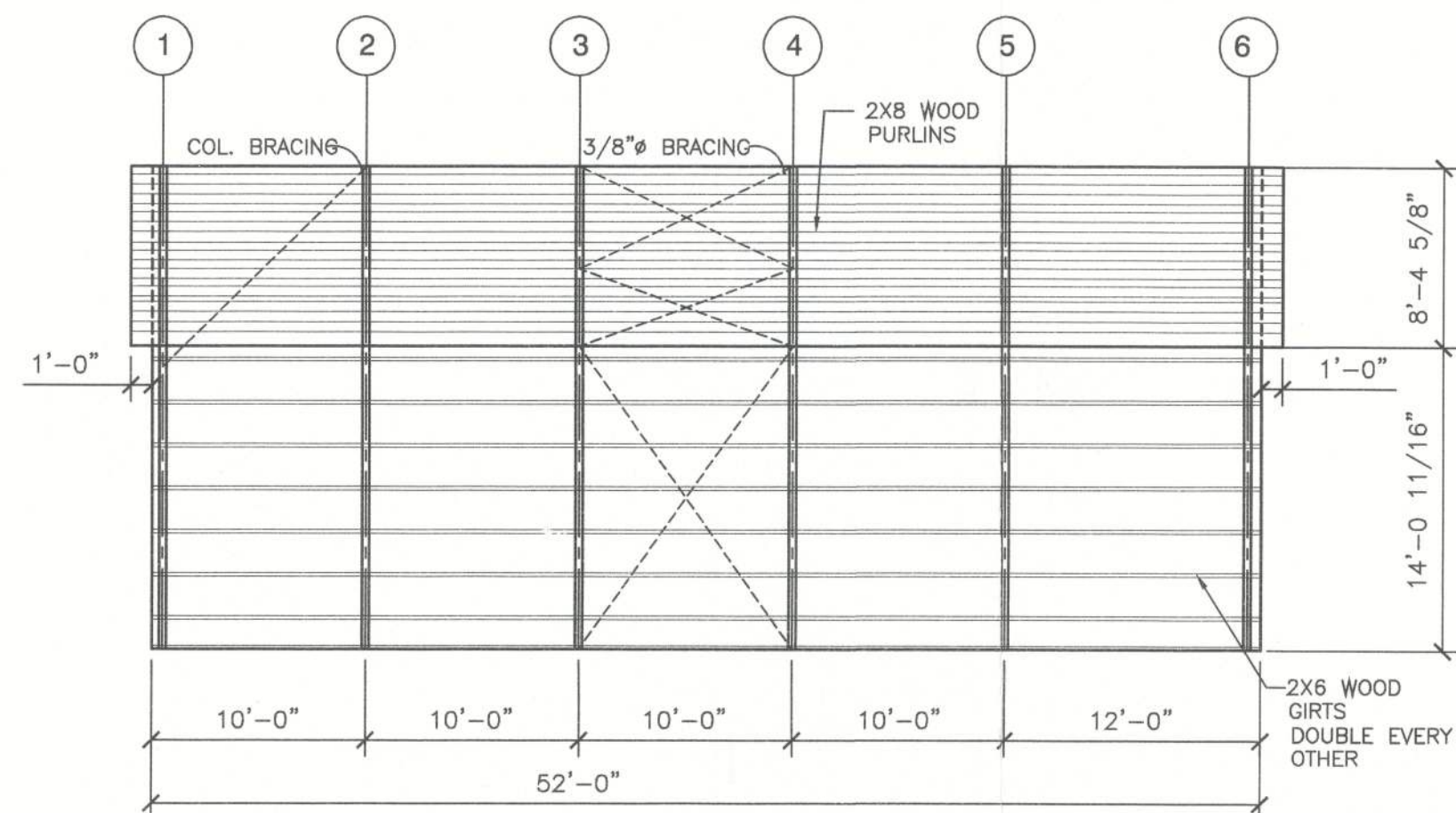
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Friday, January 24, 2014
DRAWN BY: STRUCTURAL BY:

FINALS DATE:
21Jan14

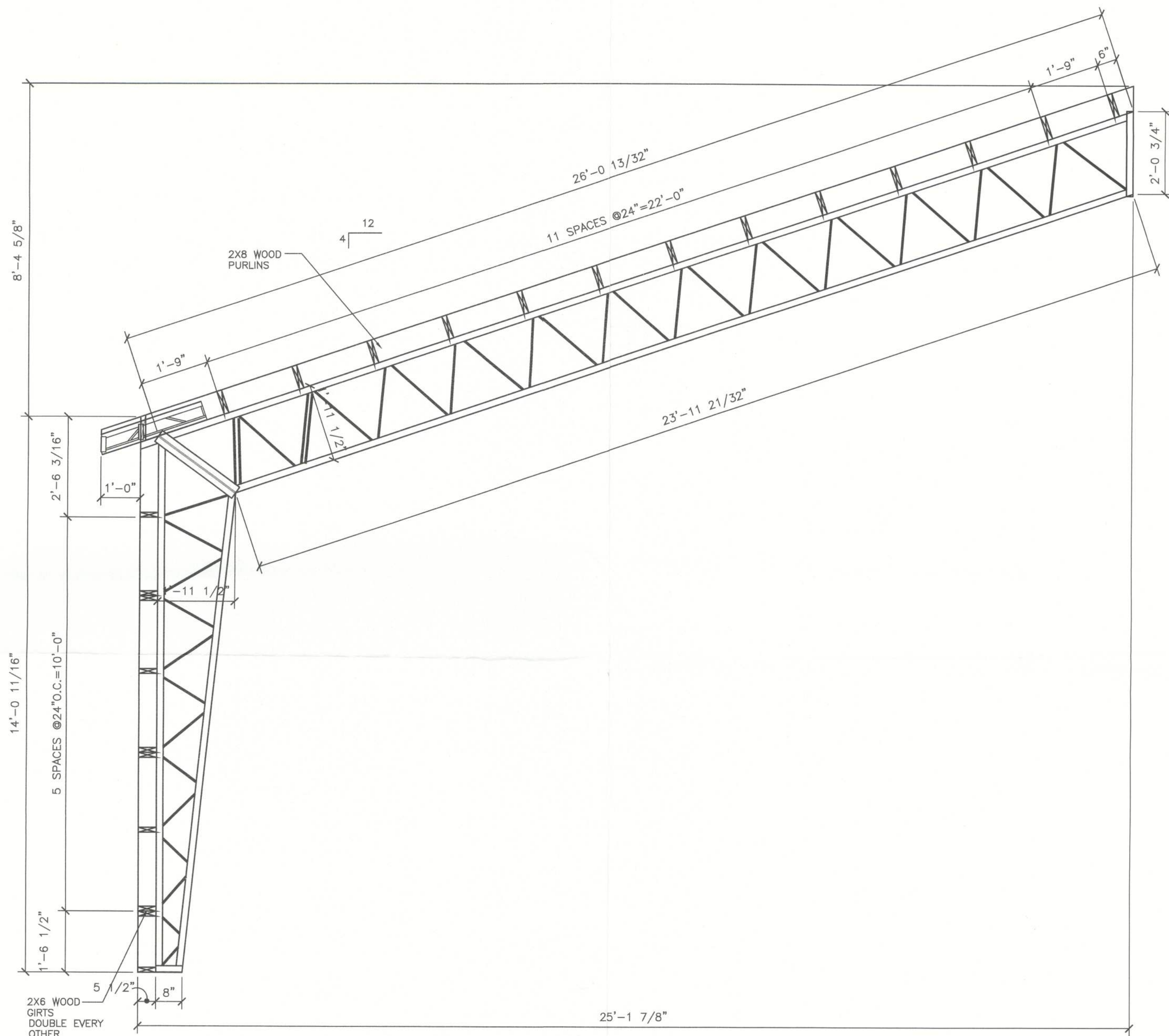
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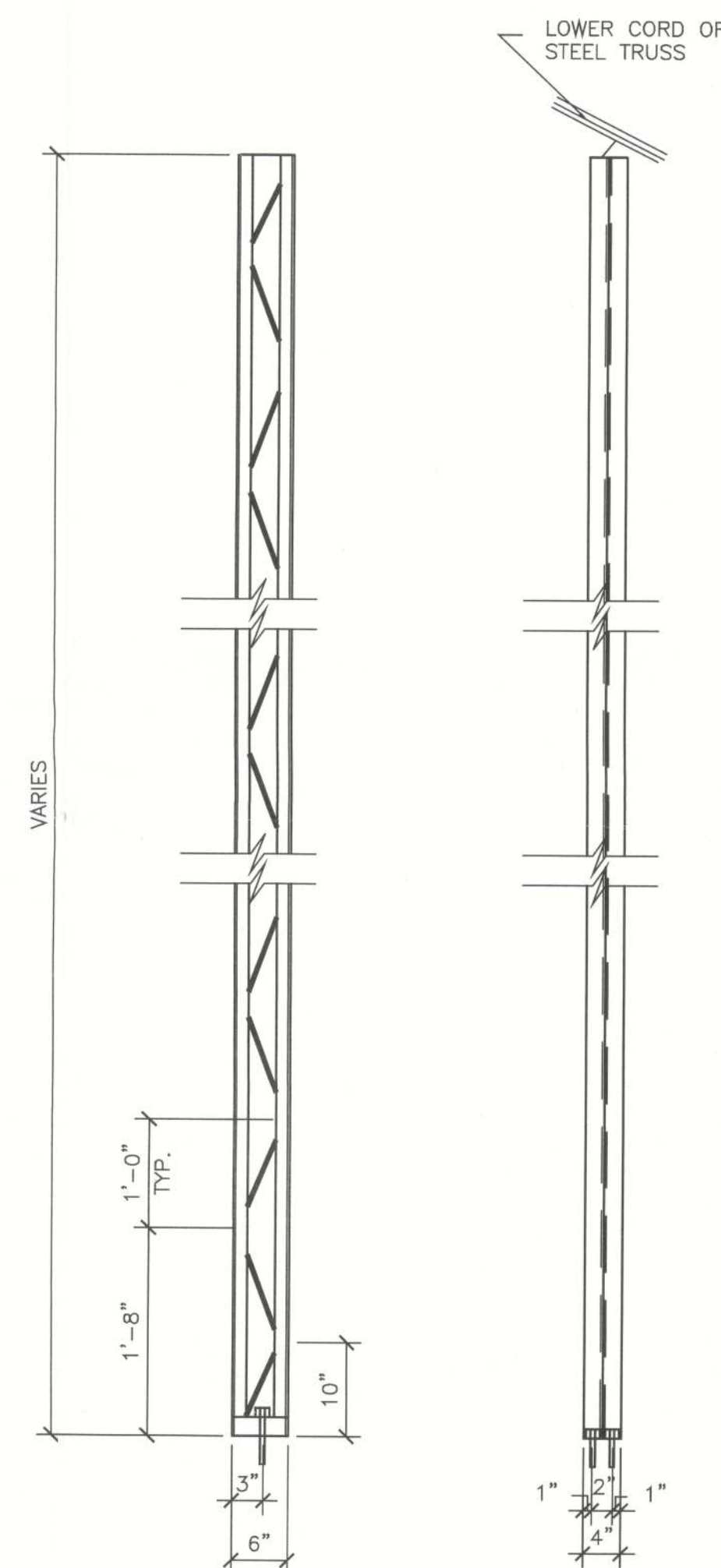
OF 2 SHEETS



RIGHT SIDEWALL ELEVATION
SCALE 1/8"=1'-0"



TRUSS DETAIL
SCALE 1/2"=1'-0"



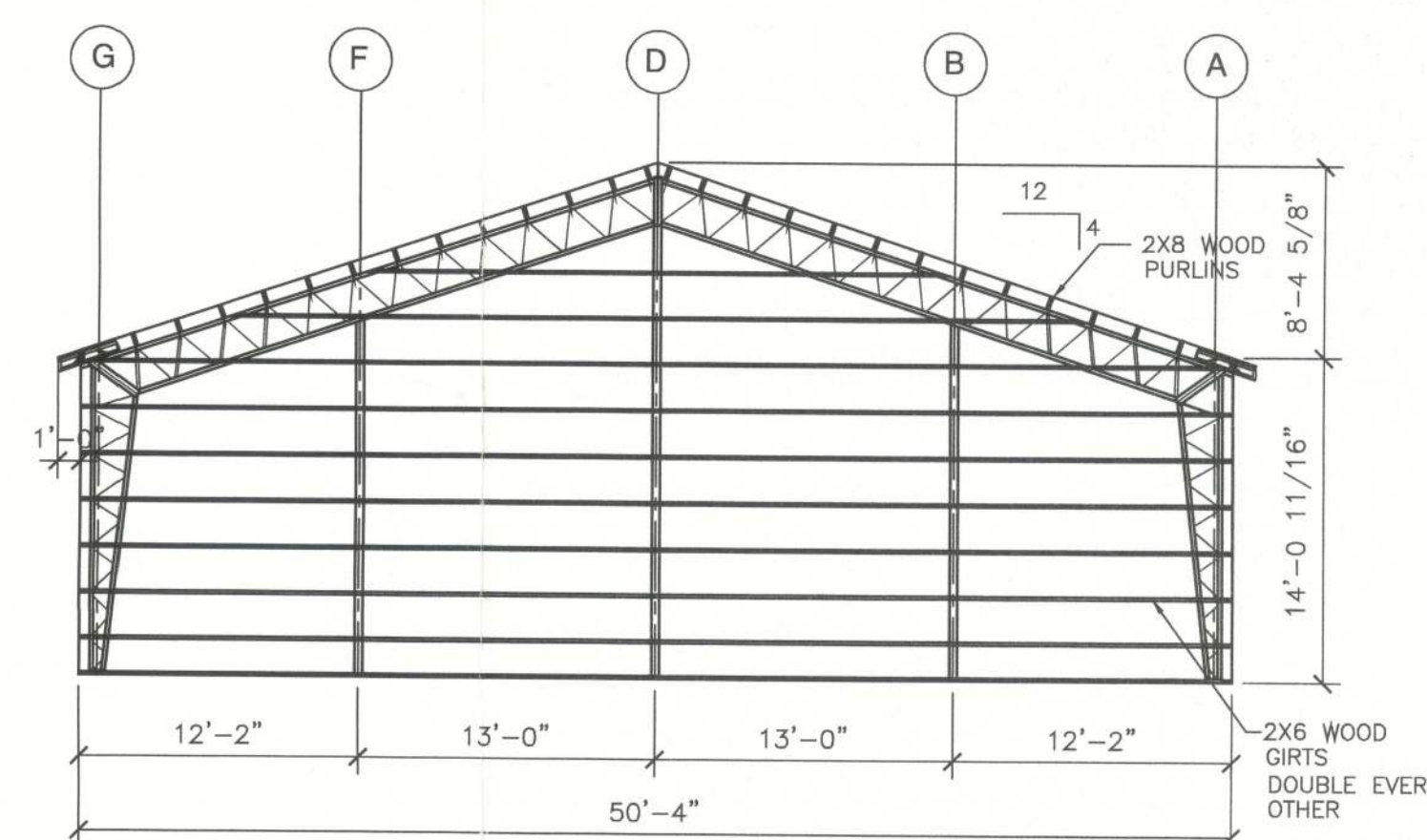
ENDWALL COLUMN
SCALE 3/4"=1'-0"

GENERAL STRUCTURAL NOTES

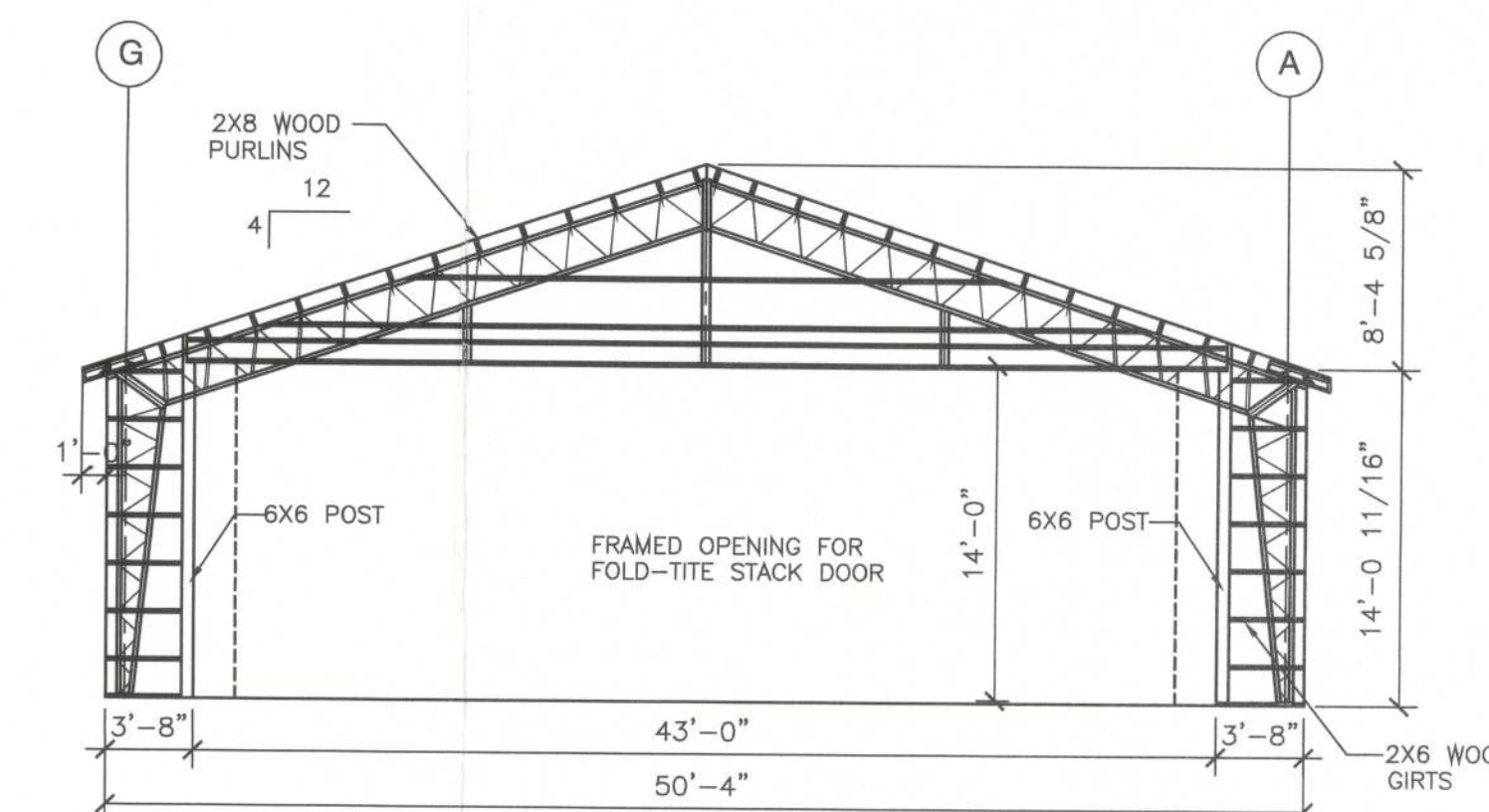
- DESIGN SPECIFICATIONS
2010 FBC
 - MATERIAL SPECIFICATIONS
STRUCTURAL STEEL
WELDING
BOLTING
- F_y = 36 KSI
AWS - E70XX ELECTRODES
AISC 5/8" DIA. GRADE 5A
- LIVE LOAD: 20 PSF
WIND LOAD: V ULT=120 MPH EXP. C
SEISMIC DESIGN CATEGORY: B
- PURLINS DOUGLAS FIR-LARCH #1
GIRTS DOUGLAS FIR-LARCH #1

Exact location of windows, overhead doors and man doors shall be determined during building erection. Trusses and diagonal bracing shall not be moved to install windows, overhead doors and man doors.

REACTIONS	KIPS	DEAD LOAD	LIVE LOAD	WIND LOAD
VERTICAL	1.761	5.421	-6.261	
HORIZONTAL	0.751	2.524	-5.155	



REAR ENDWALL ELEVATION
SCALE 1/8"=1'-0"



FRONT ENDWALL ELEVATION
SCALE 1/8"=1'-0"

STEPHEN P. MASLAN & CO.
ENGINEERS/DESIGNERS
8011 PASEO SUITE 201
KANSAS CITY, MO. 64131
(816) 444-6260

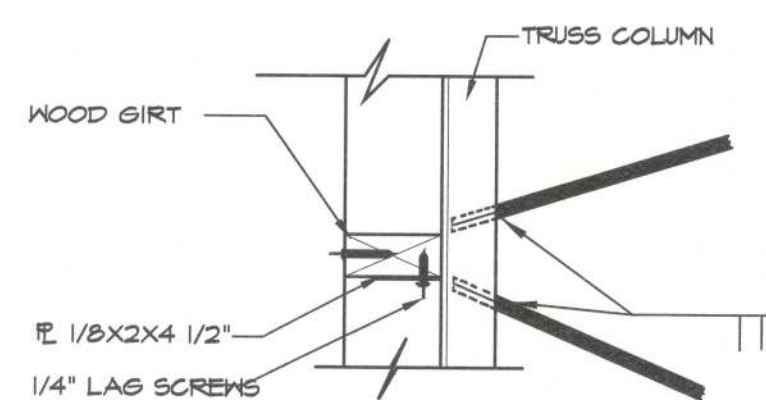


JOEL DOROSCHENKO
2409 SW SISTERS WELCOME LAKE CITY, FL

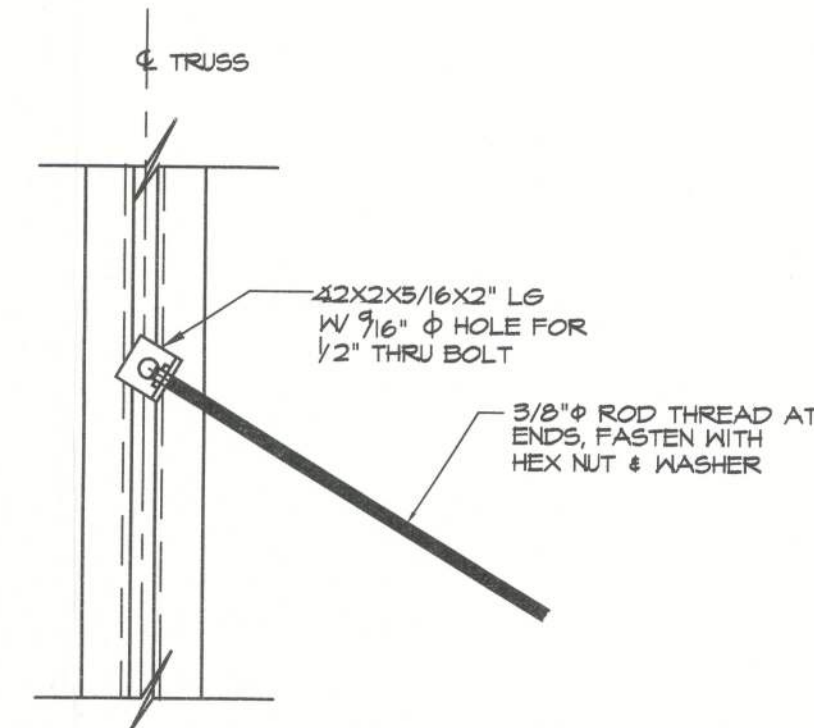
date 11-9-13
drawn by R.E.S.
checked by S.P.M.
revised

sheet no.

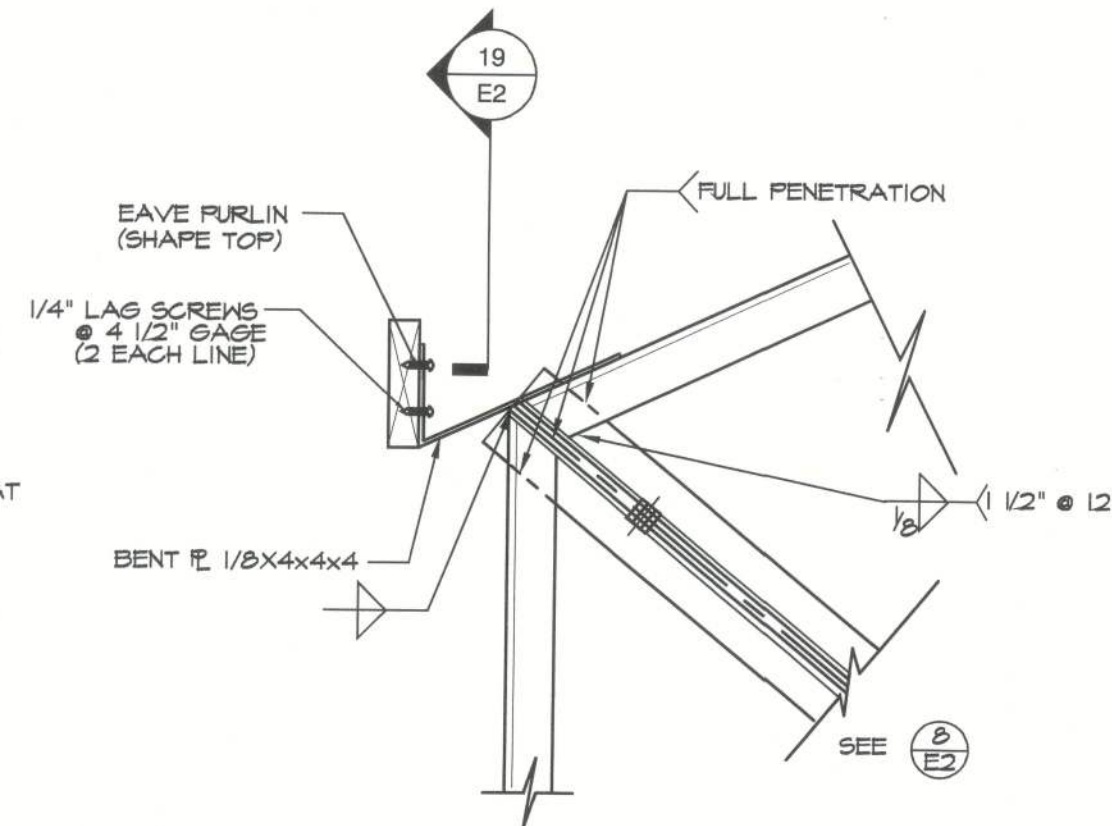
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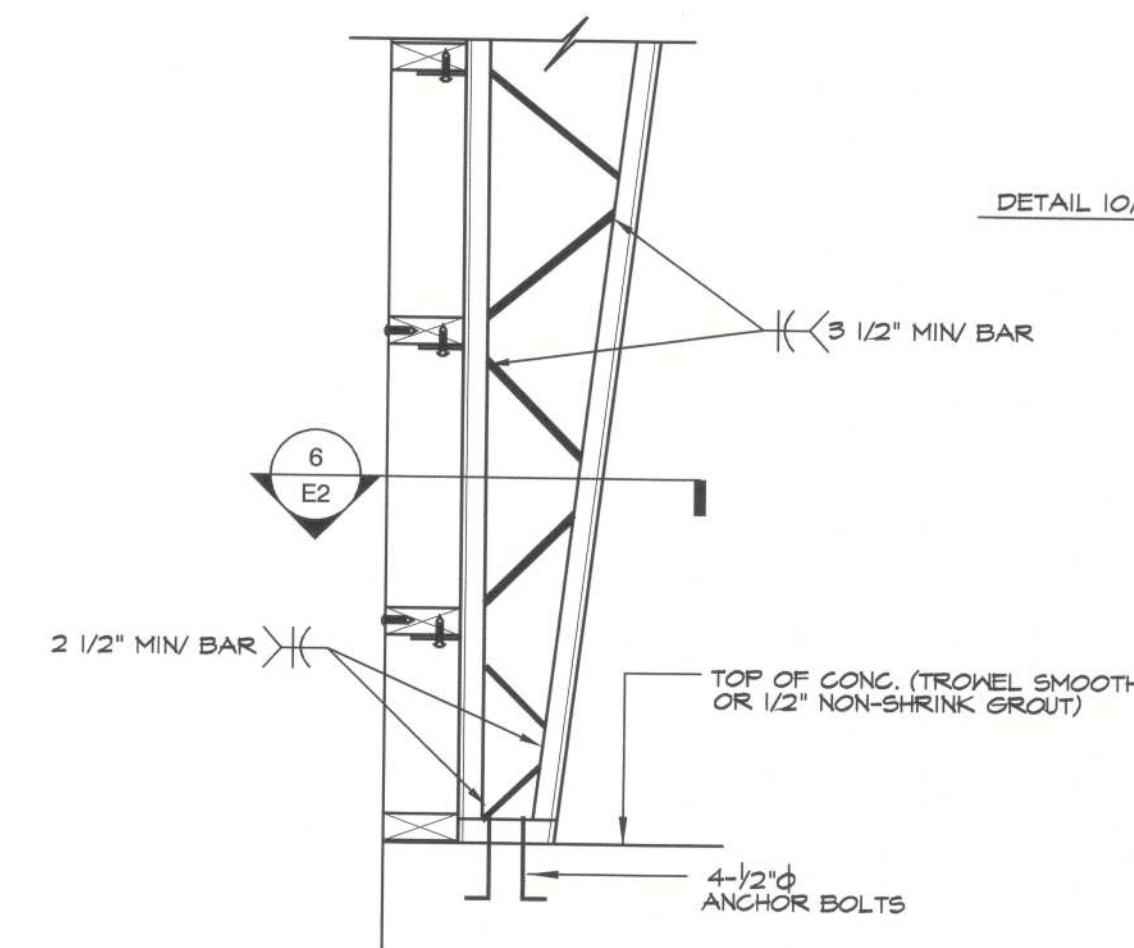
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E2 GIRT CONNECTION



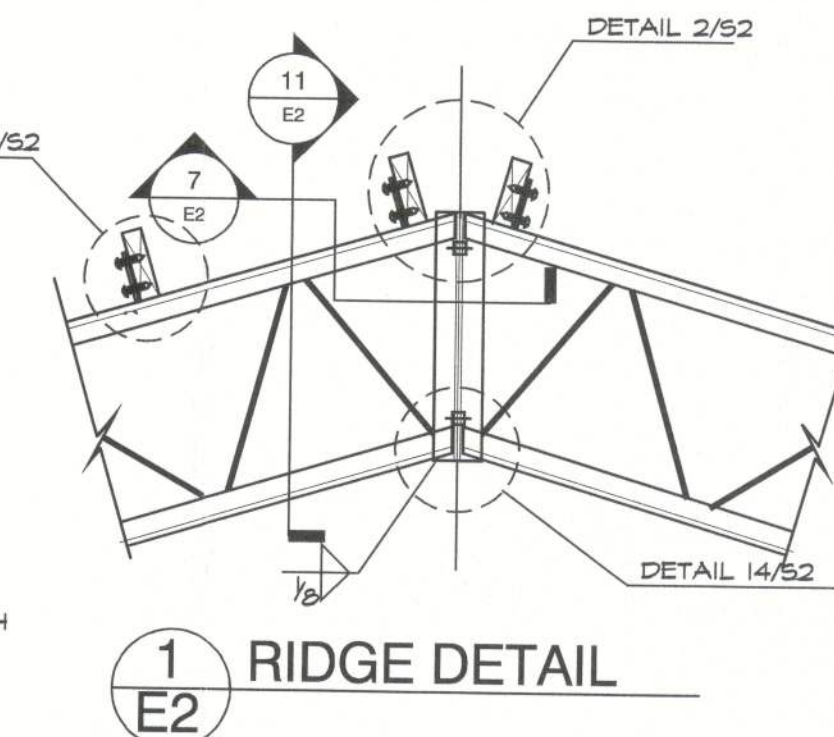
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E2 WIND BRACING DETAIL



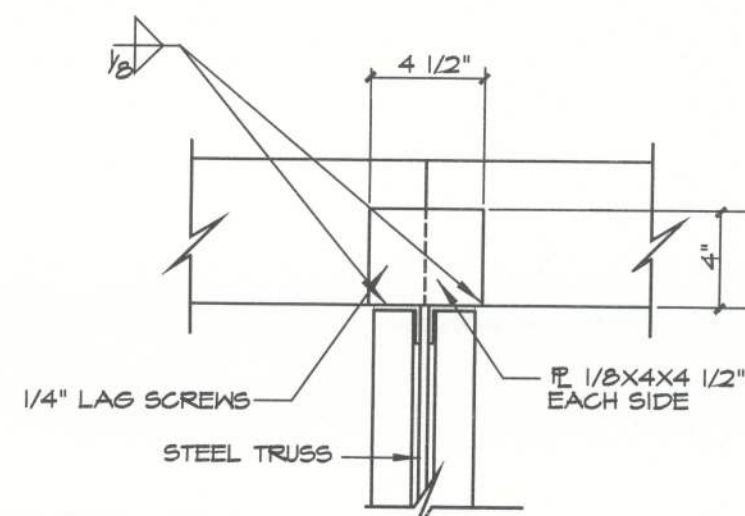
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E2 EAVE PURLIN CONNECTION



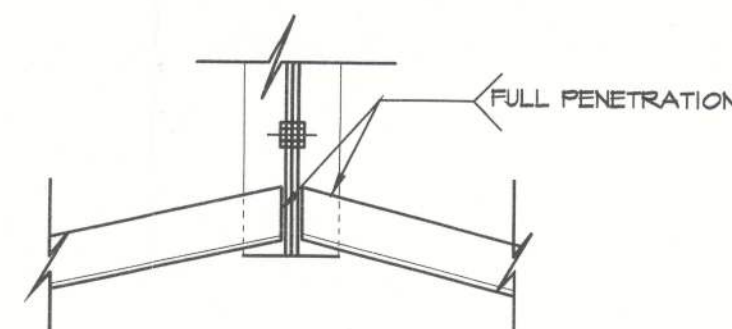
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E2 COLUMN BASE DETAIL



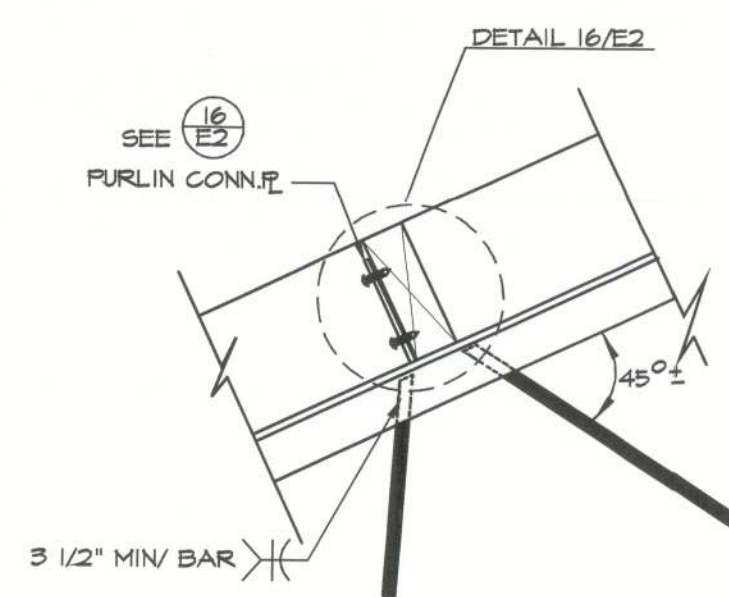
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E2 RIDGE DETAIL



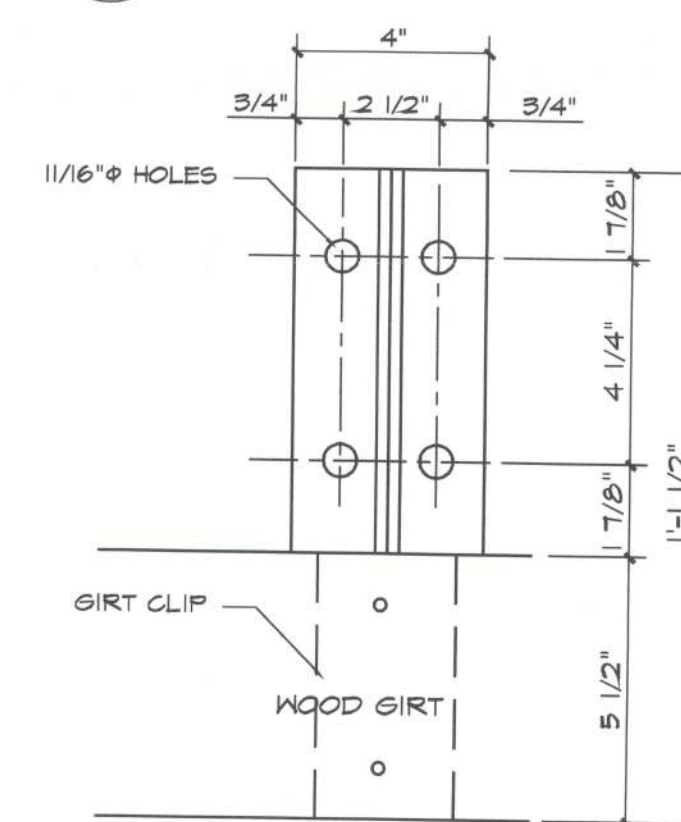
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E2 RIDGE PURLIN CONNECTION



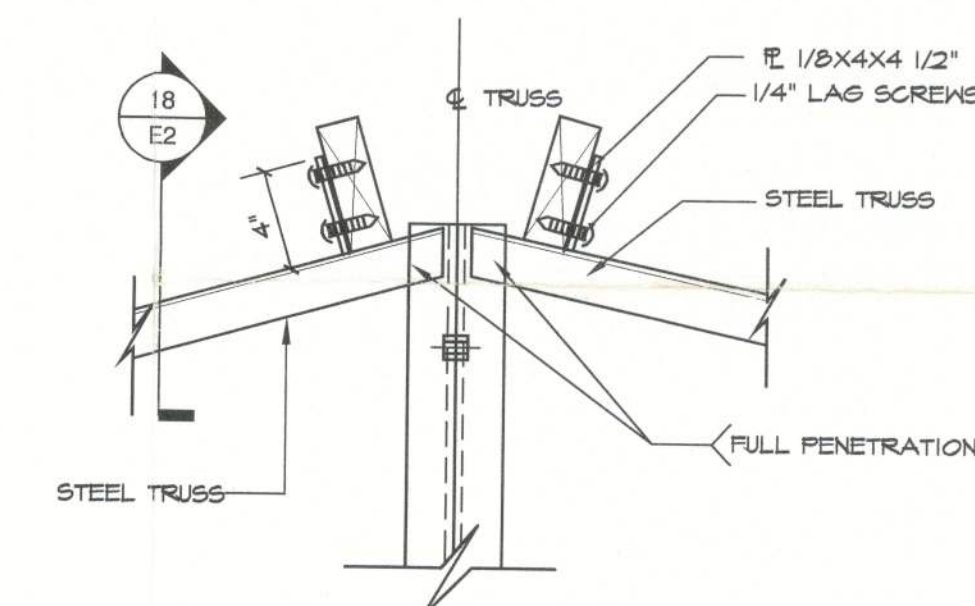
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E2 BOTTOM CORD



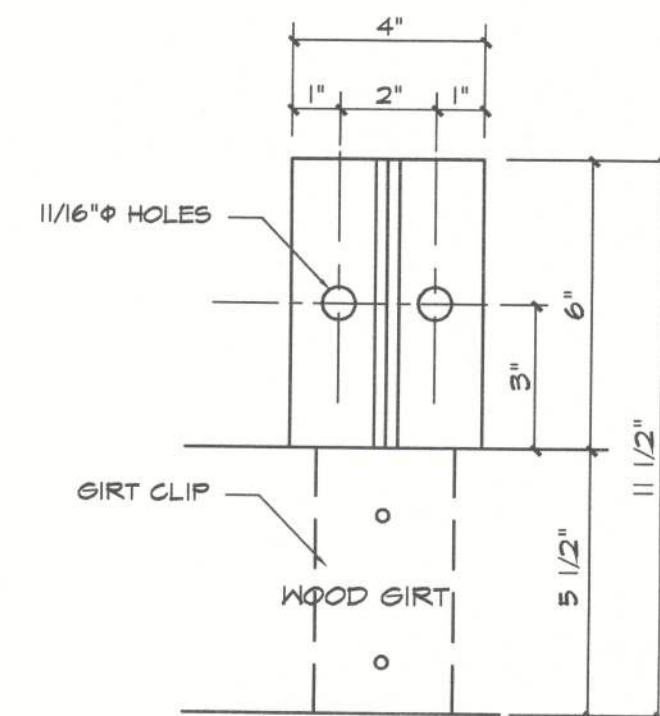
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E2 WEB CONNECTION



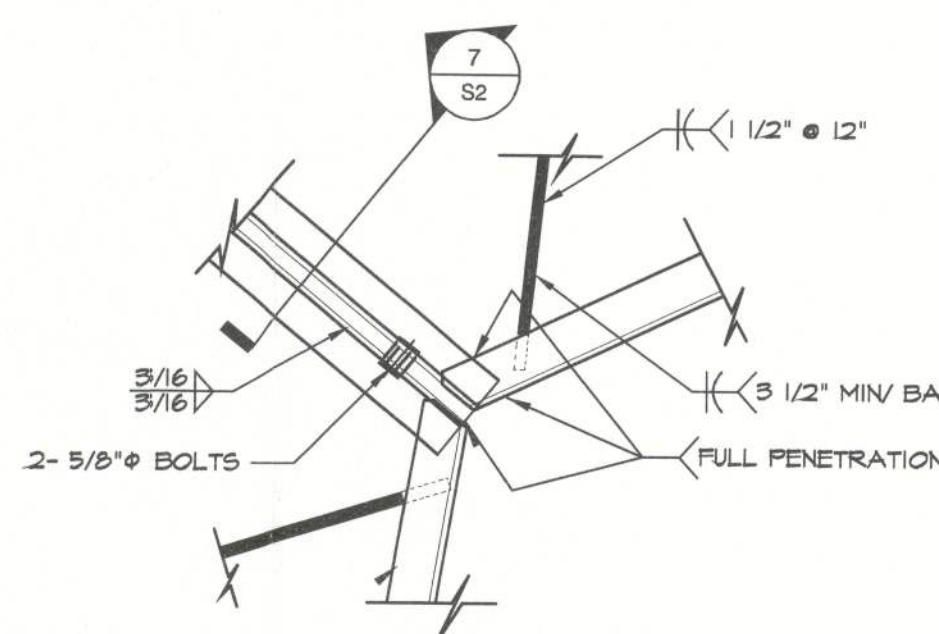
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E2 COLUMN BASE PLATE DETAIL



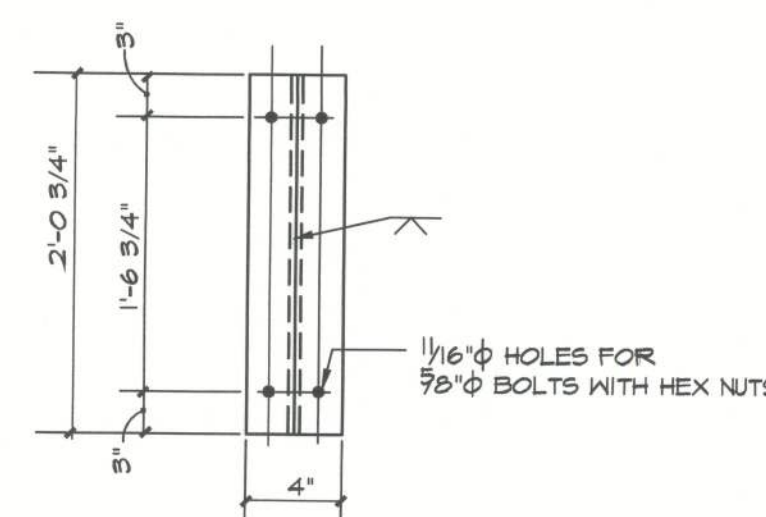
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E2 RIDGE CONNECTION



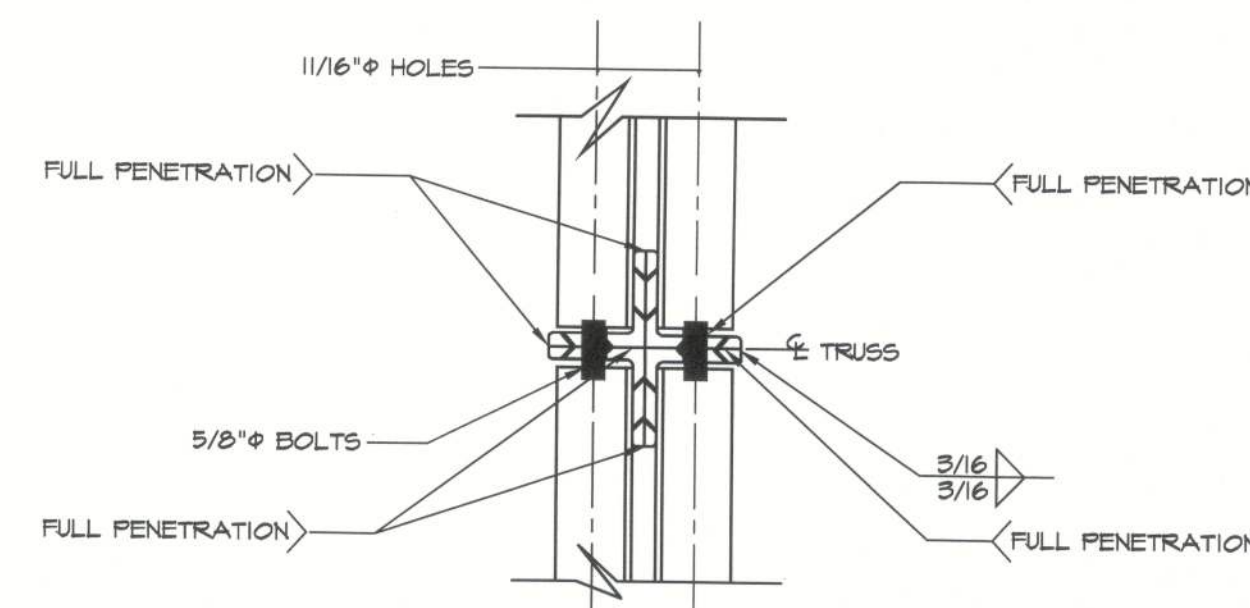
19
E2 ENDWALL COLUMN BASE PLATE
FOR ENDWALL COLUMNS OVER 20' USE 9" COLUMN, BOLT HOLES ARE THE SAME



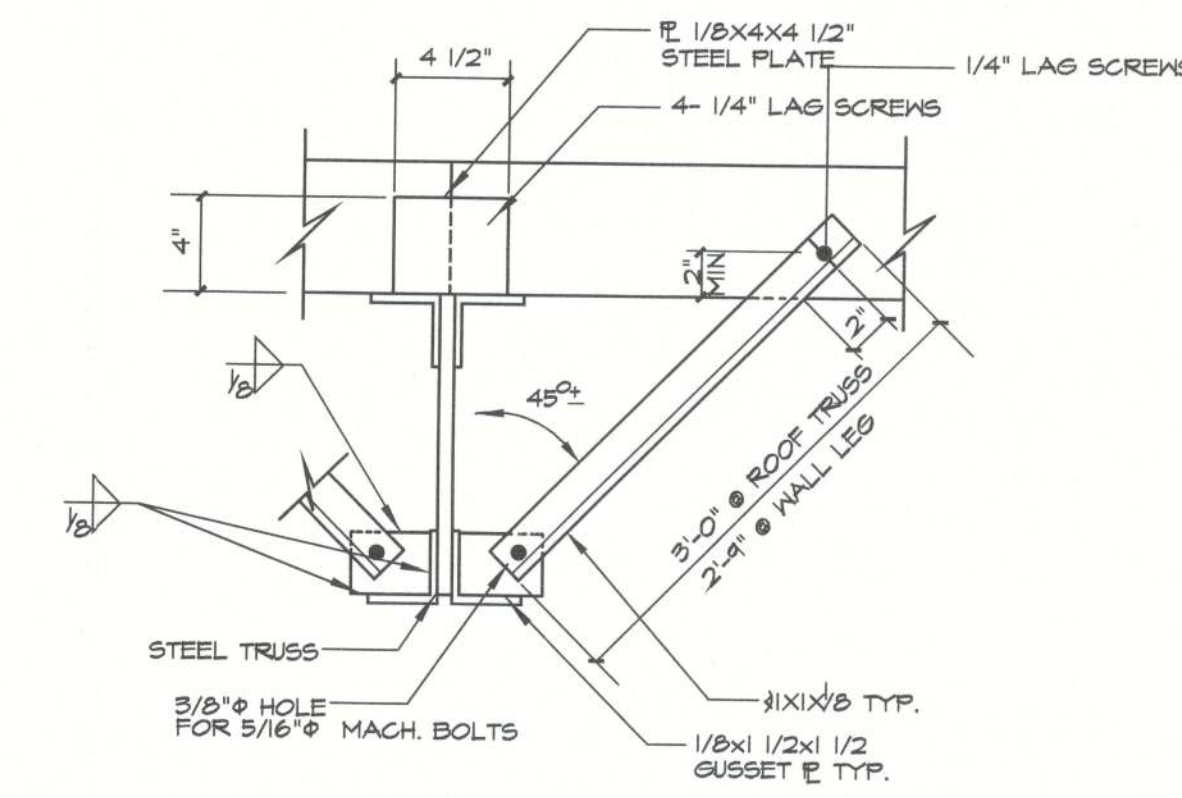
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E2 KNEE CONNECTION



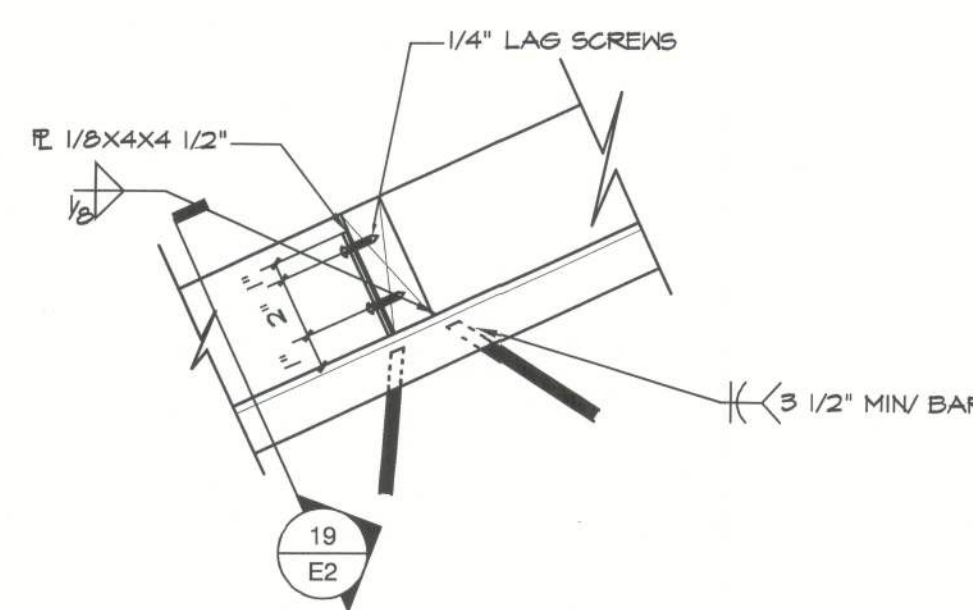
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E2 RIDGE ANGLES



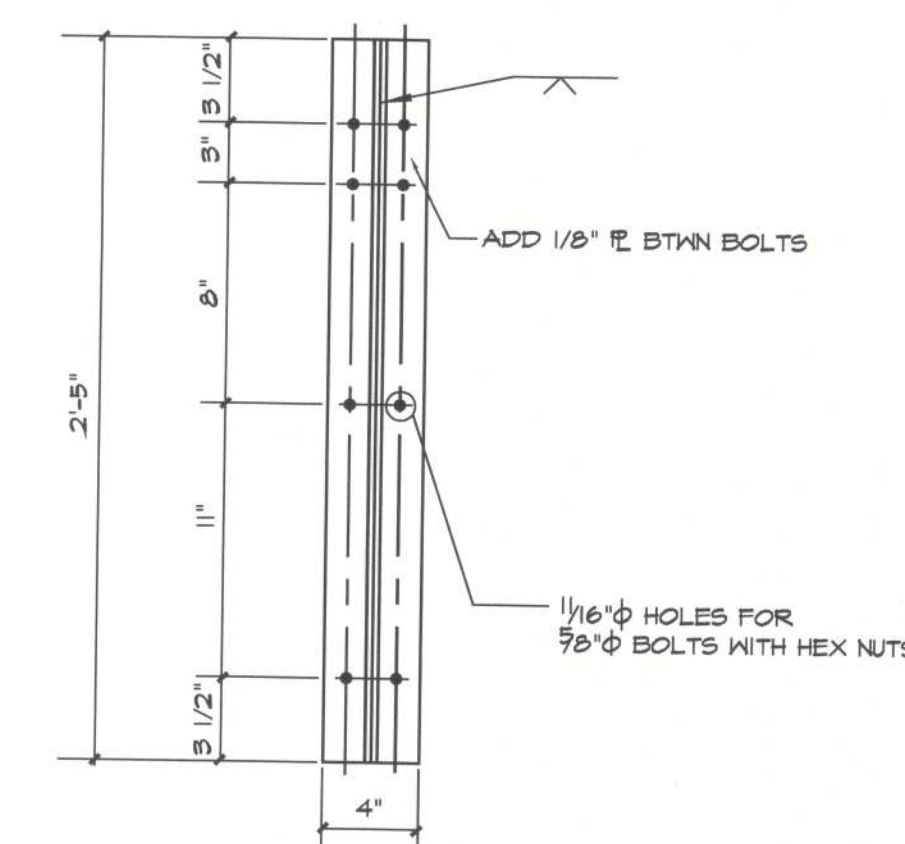
7
E2 SECTION



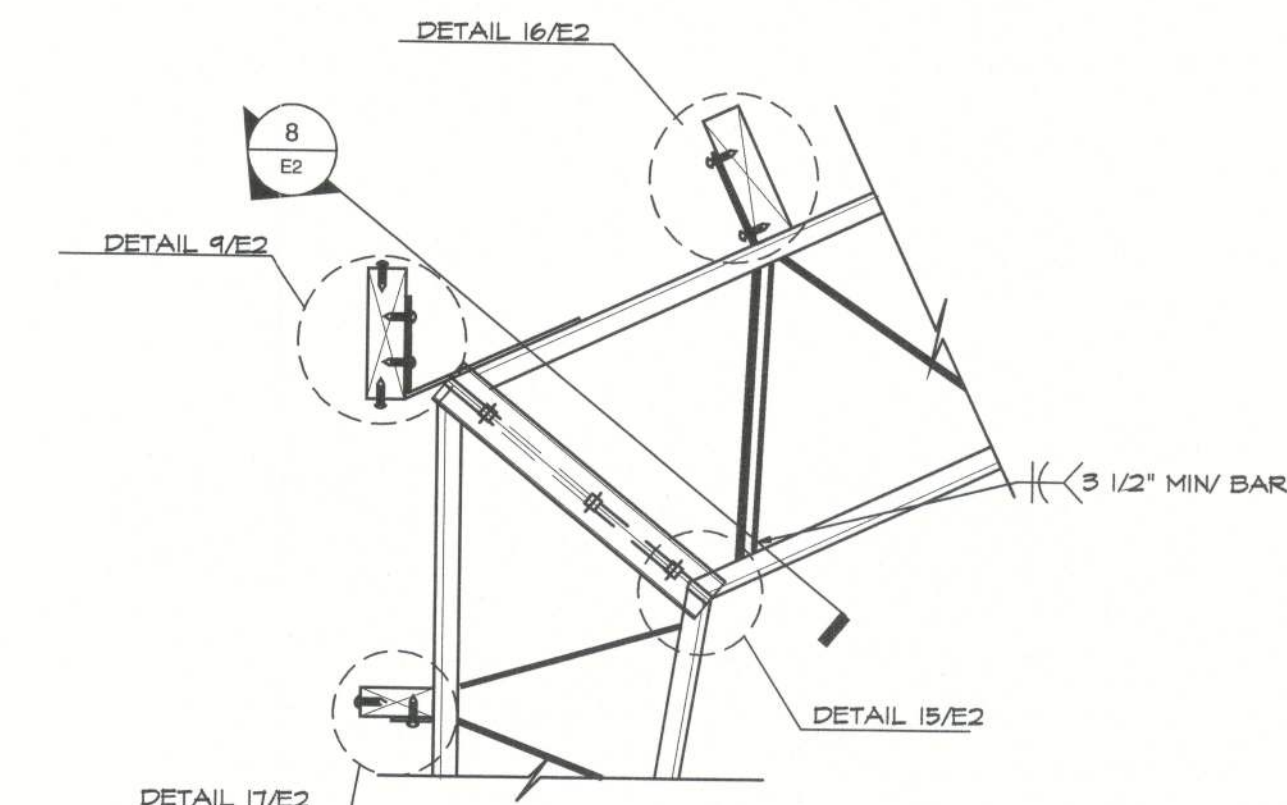
3
E2 SWAY BRACE DETAIL



16
E2 PURLIN CONNECTION



8
E2 KNEE DIAGONAL



4
E2 KNEE DETAIL

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drawn by R.F.S.
checked by S.P.M.
revised

sheet no.
E2