DATE 09/21/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024993

APPLICANT LINDA RODER PHONE 752-2281

ADDRESS 387 SW KEMP COURT LAKE CITY FL 32024

OWNER DARBY ROGERS CO PHONE 752-6575

ADDRESS COUNTRY LAKE DRIVE LAKE CITY FL 32055

CONTRACTOR BLAKE CONSTRUCTION CO PHONE 754-5810

LOCATION OF PROPERTY 90W, TR ON LAKE JEFFREY, TL ON SCENIC LAKE DR,
TR ON COUNTRY LAKE DRIVE, 9TH LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 140900.00

HEATED FLOOR AREA 2818.00 TOTAL AREA 3354.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 10/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 26

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 22-3S-16-02267-118 SUBDIVISION COUNTRY LAKES AT WOODBOROUGH

LOT 18 BLOCK PHASE UNIT TOTAL ACRES

000001209		CBC1253408	Linda Roder
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor

CULVERT	06-0791-N	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD,NOC ON FILE

Check # or Cash 4341

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 705.00 CERTIFICATION FEE $ 16.77 SURCHARGE FEE $ 16.77

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 838.54

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

CK#4341 838-54 Revised 9-23-04

For Office Use Only Application # 0609-29 Date Received 9/12/06 By [illegible] Permit # 1209/24993

Application Approved by - Zoning Official BLK Date 9.09.06 Plans Examiner OKJTH Date 9-20-06

Flood Zone X per plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. L.D. DEN.

Comments

Applicants Name Linda Roder Phone 752-2281

Address 387 S.W. Kempt Lake City FL 32024

Owners Name The Darby Rogers Co. Phone 752-6575

911 Address 911 PENDING PER RC 9/20/06

Contractors Name Blake Construction Co. Phone 754-5810

Address 291 SW Sisters Welcome Lake City FL 32025

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address Donald Gardner/ Mark Disosway

Mortgage Lenders Name & Address People's

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 22-3S-16-02267-118 Estimated Cost of Construction $200,000.00

Subdivision Name Country Lakes at Woodborough Lot 18 Block ____ Unit ____ Phase ____

Driving Directions Hwy 90 West, Ron Lake Jeffrey, Lon Scenic Lake Dr, Ron Country Lake Dr, Lot on Left (4th lot down) See Sign

Type of Construction SFD Number of Existing Dwellings on Property 0

Total Acreage ____ Lot Size .91 Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 45' Side 39' Side 39' Rear 160'

Total Building Height 26' Number of Stories 1 Heated Floor Area 2818 TOTAL 3354 Roof Pitch 10-12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor) Contractor Signature

Contractors License Number CBC1253408

Competency Card Number

NOTARY STAMP/SEAL

NORA L. TERRY
Notary Public - State of Florida
My Commission Expires Mar 24, 2009
Commission # DD 410803
Bonded By National Notary Assn.

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 30th day of May 2006.

Personally known ✓ or Produced Identification ____

Notary Signature

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

File No. 06-413

Inst:200617157 Date:07/18/2006 Time:11:46
DC,P.DeWitt Cason,Columbia County B:1090 P:472

PERMIT NO._____

TAX FOLIO NO.:R02252-000, R02268-004 & R02253-000

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property:

Lot 18, Country Lake In Woodborough, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 97-99, public records, Columbia County, Florida.

2. General description of improvement: Construction of dwelling.

3. Owner information:

a. Name and address: The Darby Rogers Company, 3101 W US Hwy 90, Suite 101, Lake City, FL 32055

b. Interest in property: Fee Simple

c. Name and address of fee simple title holder (if other than Owner):

4. Contractor: Blake Construction Company of North Florida, 291 SW Sisters Welcome Road, Suite 102, Lake City, FL 32025,

5. Surety

a. Name and address: None

b. Amount of bond:

6. Lender: Peoples State Bank, 350 SW Main Blvd., Lake City, Florida 32025

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

8. In addition to himself, Owner designates Lonnie Haltiwanger of Peoples State Bank, 350 SW Main Blvd., Lake City, Florida 32025, to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified). July 18, 2006

CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By: Shawn Feagle
Deputy Clerk
Date 07-19-2006

THE DARBY ROGERS COMPANY

By: _____
Blake N. Lunde, II
President

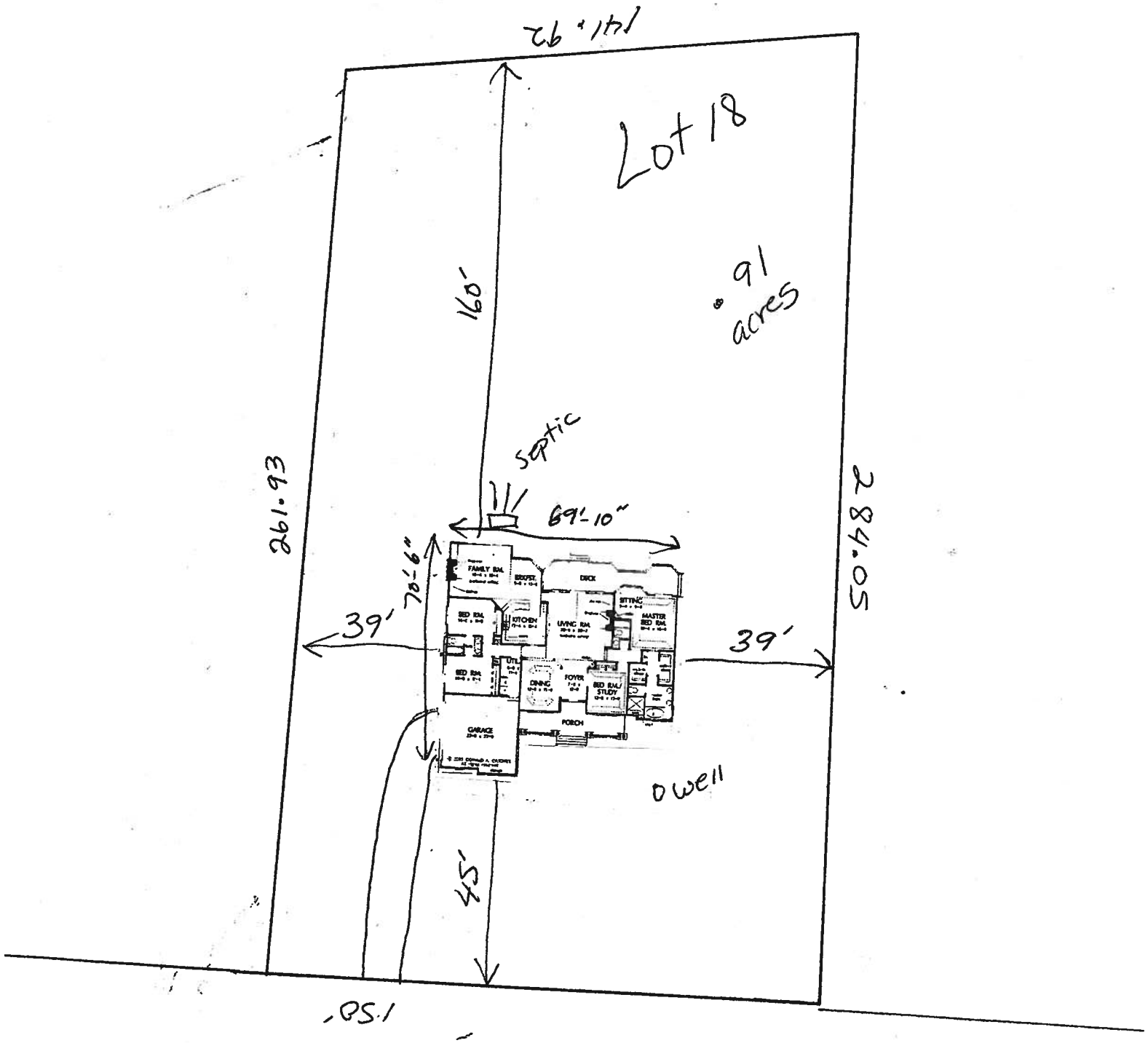
STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 18th day of July, 2006 by Blake N. Lunde, II, President of The Darby Rogers Company, on behalf of the corporation. He is personally known to me and did not take an oath.

TERRY McDAVID
MY COMMISSION # DD 500169
EXPIRES: January 16, 2010
Bonded Thru Notary Public Underwriters

_____
Notary Public
My commission expires:_____

Lot 18 Country Lakes in Woodborough



(Proposed) N. W. Country Lake Glen

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Inst:200617155 Date:07/19/2006 Time:11:15
Doc Stamp-Deed : 280.90
DC,P.DeWitt Cason,Columbia County B:1090 P:489

Property Appraiser's
Identification Number R02252-000 & R02268-004

TM File No: 06-413

WARRANTY DEED

This Warranty Deed, made this 18th day of July, 2006, BETWEEN MS, DM & BL, LLC, a Florida limited liability company, whose post office address is 3101 US Highway 90 West, Suite 101, Lake City, FL 32055, of the County of Columbia, State of Florida, grantor*, and THE DARBY ROGERS COMPANY, whose post office address is 3101 W US Highway 90, Suite 101, Lake City, FL 32055, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lot 18, Country Lake In Woodborough, Phase 1, a subdivision according to the plat thereof as recorded in Plat Book 8, Pages 97-99, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(Signature of First Witness)
Terry Maddox
(Typed Name of First Witness)

(Signature of Second Witness)
Karen N. Wright
(Typed Name of Second Witness)

NS, DM & BL, LLC

BY: Deborah S. Myles (SEAL)
Deborah S. Myles, Managing Member

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 18th day of July, 2006, by Deborah S. Myles, Managing Member of NS, DM & BL, LLC, a Florida limited liability company, who is/are personally known to me or who has/have produced _____ as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

TERRY MADDOX
MY COMMISSION # DD 000704
EXPIRES: January 19, 2010
Bonded Thru Notary Public Underwriters

Inst:200617155 Date:07/18/2006 Time:11:48
Doc Stamp-Mtmd : 280.90
DC,P.DeWitt Cason,Columbia County B:1090 P:482

Notice of Intent for Preventative Treatment for Termites

(As required by Florida Building Code 104.2.6)

Date: 9-7-06

Lot 18 Country Lakes at Woodborough — (Address of Treatment or Lot/Block of Treatment)

Lake City — City

Florida Pest Control & Chemical Co.

www.flapest.com

Product to be used: Bora-Care Termiticide (Wood Treatment)

Chemical to be used: 23% Disodium Octaborate Tetrahydrate

Application will be performed onto structural wood at dried-in stage of construction. Bora-Care Termiticide application shall be applied according to EPA registered label directions as stated in the Florida Building Code Section 1861.1.8

(Information to be provided to local building code offices prior to concrete foundation installation.)

6/04 ©

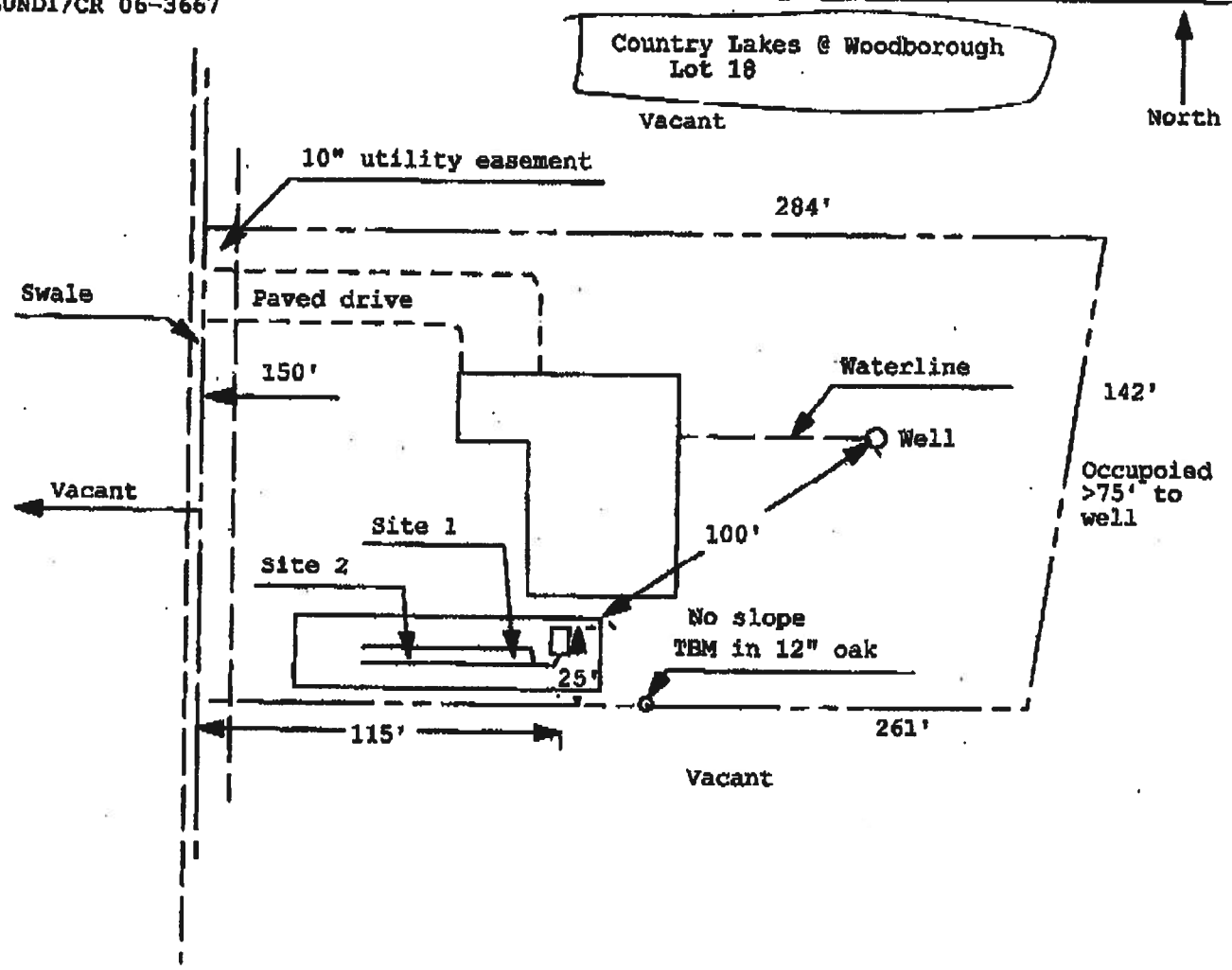
0609-29 Blake Lunde

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0791N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

LUNDI/CR 06-3667



1 inch = 50 feet

Site Plan Submitted By Paul Lyle Date 8/29/06

Plan Approved X Not Approved ___ Date ___

By Sallie Graddy EST 9-7-06 CPHU

Notes: Columbia CHD

FORM 600A-2004 EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	608155BlakeConstruction\Lot18CountryLakeinWBuilderS/DPhaseI		
Address:	Lot: 18, Sub: Country Lake in, Plat:	Permitting Office:	Columbia
City, State:	, FL	Permit Number:	24993
Owner:	Spec House	Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 52.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 11.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft²)	2818 ft²	13. Heating systems	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		a. Electric Heat Pump	Cap: 52.0 kBtu/hr
a. U-factor: (or Single or Double DEFAULT) 7a.	Description Area (Dble Default) 452.0 ft²		HSPF: 7.30
b. SHGC: (or Clear or Tint DEFAULT) 7b.	(Clear) 452.0 ft²	b. N/A	
8. Floor types		c. N/A	
a. Slab-On-Grade Edge Insulation	R=0.0, 222.0(p) ft	14. Hot water systems	
b. N/A		a. Electric Resistance	Cap: 40.0 gallons
c. N/A			EF: 0.93
9. Wall types		b. N/A	
a. Frame, Wood, Exterior	R=13.0, 1275.0 ft²	c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
b. Frame, Wood, Exterior	R=13.0, 190.0 ft²	15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic	R=30.0, 2948.0 ft²		
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 210.0 ft		
b. N/A			

Glass/Floor Area: 0.16 Total as-built points: 38398 Total base points: 38600 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [signature]

DATE: 8-21-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: [signature]

DATE: 9-8-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 18, Sub: Country Lake in, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2818.0	20.04	10165.1	Double, Clear	S	1.5	0.0	24.7	35.87	0.43	382.6	
				Double, Clear	S	1.5	0.0	32.9	35.87	0.43	509.7	
				Double, Clear	S	1.5	7.5	24.7	35.87	0.91	806.1	
				Double, Clear	SW	1.5	7.5	24.7	40.16	0.93	926.0	
				Double, Clear	S	7.0	7.5	57.6	35.87	0.53	1095.2	
				Double, Clear	S	7.0	6.5	20.0	35.87	0.51	365.5	
				Double, Clear	SE	13.0	6.5	11.7	42.75	0.40	202.1	
				Double, Clear	S	6.0	6.5	27.2	35.87	0.53	518.6	
				Double, Clear	SW	13.0	6.5	13.6	40.16	0.39	215.4	
				Double, Clear	W	1.5	5.5	9.7	38.52	0.90	335.2	
				Double, Clear	N	1.5	5.5	19.4	19.20	0.93	345.8	
				Double, Clear	N	7.0	7.5	24.7	19.20	0.72	342.1	
				Double, Clear	N	7.0	7.5	45.2	19.20	0.72	626.0	
				Double, Clear	N	7.0	7.5	24.7	19.20	0.72	342.1	
				Double, Clear	N	10.0	8.0	20.0	19.20	0.68	259.5	
				Double, Clear	E	1.5	7.3	62.2	42.06	0.94	2468.9	
				Double, Clear	E	1.5	6.0	9.0	42.06	0.91	345.5	
				As-Built Total:				**452.0**			**10086.2**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1275.0	1.50		1912.5	
Exterior	1465.0	1.70	2490.5	Frame, Wood, Exterior			13.0	190.0	1.50		285.0	
Base Total:	**1465.0**		**2490.5**	**As-Built Total:**				**1465.0**			**2197.5**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	1.60	32.0	Exterior Insulated				40.0	4.10		164.0	
Exterior	60.0	4.10	246.0	Exterior Insulated				20.0	4.10		82.0	
				Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**80.0**		**278.0**	**As-Built Total:**				**80.0**			**278.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	2818.0	1.73	4875.1	Under Attic			30.0	2948.0	1.73 X 1.00		5100.0	
Base Total:	**2818.0**		**4875.1**	**As-Built Total:**				**2948.0**			**5100.0**	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 18, Sub: Country Lake in, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT					
FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	=	Points
Slab	222.0(p)	-37.0	-8214.0	Slab-On-Grade Edge Insulation	0.0	222.0(p	-41.20		-9146.4
Raised	0.0	0.00	0.0						
Base Total:			**-8214.0**	**As-Built Total:**		**222.0**			**-9146.4**
INFILTRATION	Area X	BSPM	= Points			Area X	SPM	=	Points
	2818.0	10.21	28771.8			2818.0	10.21		28771.8
Summer Base Points: 38366.5				**Summer As-Built Points:**					**37287.1**
Total Summer Points	X System Multiplier	= Cooling Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
				(sys 1: Central Unit 52000 btuh ,SEER/EFF(11.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)					
				37287	1.00	(1.09 x 1.147 x 0.91)	0.310	1.000	13162.4
38366.5	**0.4266**	**16367.2**		**37287.1**	**1.00**	**1.138**	**0.310**	**1.000**	**13162.4**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 18, Sub: Country Lake in, Plat: , , FL,** PERMIT #:

BASE

GLASS TYPES

.18 X	Conditioned Floor Area	X BWPM	= Points
.18	2818.0	12.74	6462.2

WALL TYPES

	Area	X BWPM	= Points
Adjacent	0.0	0.00	0.0
Exterior	1465.0	3.70	5420.5
Base Total:	**1465.0**		**5420.5**

DOOR TYPES

	Area	X BWPM	= Points
Adjacent	20.0	8.00	160.0
Exterior	60.0	8.40	504.0
Base Total:	**80.0**		**664.0**

CEILING TYPES

	Area	X BWPM	= Points
Under Attic	2818.0	2.05	5776.9
Base Total:	**2818.0**		**5776.9**

AS-BUILT

GLASS TYPES

Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF =	Points
Double, Clear	S	1.5	0.0	24.7	13.30	3.66	1202.1
Double, Clear	S	1.5	0.0	32.9	13.30	3.66	1601.2
Double, Clear	S	1.5	7.5	24.7	13.30	1.06	347.0
Double, Clear	SW	1.5	7.5	24.7	16.74	1.04	427.9
Double, Clear	S	7.0	7.5	57.6	13.30	2.62	2008.9
Double, Clear	S	7.0	6.5	20.0	13.30	2.86	759.8
Double, Clear	SE	13.0	6.5	11.7	14.71	2.48	426.0
Double, Clear	S	6.0	6.5	27.2	13.30	2.61	942.6
Double, Clear	SW	13.0	6.5	13.6	16.74	1.93	438.4
Double, Clear	W	1.5	5.5	9.7	20.73	1.03	206.7
Double, Clear	N	1.5	5.5	19.4	24.58	1.00	478.2
Double, Clear	N	7.0	7.5	24.7	24.58	1.02	617.7
Double, Clear	N	7.0	7.5	45.2	24.58	1.02	1130.3
Double, Clear	N	7.0	7.5	24.7	24.58	1.02	617.7
Double, Clear	N	10.0	8.0	20.0	24.58	1.02	501.9
Double, Clear	E	1.5	7.3	62.2	18.79	1.02	1197.8
Double, Clear	E	1.5	6.0	9.0	18.79	1.04	175.1
As-Built Total:				**452.0**			**13079.3**

WALL TYPES

Type	R-Value	Area X	WPM =	Points
Frame, Wood, Exterior	13.0	1275.0	3.40	4335.0
Frame, Wood, Exterior	13.0	190.0	3.40	646.0
As-Built Total:		**1465.0**		**4981.0**

DOOR TYPES

Type	Area X	WPM =	Points
Exterior Insulated	40.0	8.40	336.0
Exterior Insulated	20.0	8.40	168.0
Adjacent Insulated	20.0	8.00	160.0
As-Built Total:	**80.0**		**664.0**

CEILING TYPES

Type	R-Value	Area X	WPM X	WCM =	Points
Under Attic	30.0	2948.0	2.05 X	1.00	6043.4
As-Built Total:		**2948.0**			**6043.4**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 18, Sub: Country Lake in, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT				
FLOOR TYPES	Area X	BWPM =	Points	Type	R-Value	Area X	WPM =	Points
Slab	222.0(p)	8.9	1975.8	Slab-On-Grade Edge Insulation	0.0	222.0(p	18.80	4173.6
Raised	0.0	0.00	0.0					
Base Total:			**1975.8**	**As-Built Total:**		**222.0**		**4173.6**
INFILTRATION	Area X	BWPM =	Points			Area X	WPM =	Points
	2818.0	-0.59	-1662.6			2818.0	-0.59	-1662.6
Winter Base Points:			**18636.8**	**Winter As-Built Points:**				**27278.7**
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier = Heating Points
				(sys 1: Electric Heat Pump 52000 btuh ,EFF(7.3) Ducts:Unc(S),Unc(R),Int(AH),R6.0				
				27278.7	1.000	(1.069 x 1.169 x 0.93)	0.467	1.000 14809.1
18636.8	**0.6274**	**11692.7**		**27278.7**	**1.00**	**1.162**	**0.467**	**1.000 14809.1**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 18, Sub: Country Lake in, Plat: , , FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
4		2635.00	10540.0	40.0	0.93	4		1.00	2606.67	1.00	10426.7
				As-Built Total:							**10426.7**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
16367	**11693**	**10540**	**38600**	**13162**	**14809**	**10427**	**38398**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 18, Sub: Country Lake in, Plat: , , FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.2

The higher the score, the more efficient the home.

Spec House, Lot: 18, Sub: Country Lake in, Plat: , , FL,

1.	New construction or existing	New		12.	Cooling systems	
2.	Single family or multi-family	Single family			a. Central Unit	Cap: 52.0 kBtu/hr
3.	Number of units, if multi-family	1				SEER: 11.00
4.	Number of Bedrooms	4			b. N/A	
5.	Is this a worst case?	Yes				
6.	Conditioned floor area (ft²)	2818 ft²			c. N/A	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)					
	a. U-factor:	Description Area		13.	Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 452.0 ft²			a. Electric Heat Pump	Cap: 52.0 kBtu/hr
	b. SHGC:					HSPF: 7.30
	(or Clear or Tint DEFAULT) 7b.	(Clear) 452.0 ft²			b. N/A	
8.	Floor types					
	a. Slab-On-Grade Edge Insulation	R=0.0, 222.0(p) ft			c. N/A	
	b. N/A					
	c. N/A			14.	Hot water systems	
9.	Wall types				a. Electric Resistance	Cap: 40.0 gallons
	a. Frame, Wood, Exterior	R=13.0, 1275.0 ft²				EF: 0.93
	b. Frame, Wood, Exterior	R=13.0, 190.0 ft²			b. N/A	
	c. N/A					
	d. N/A				c. Conservation credits	
	e. N/A				(HR-Heat recovery, Solar	
10.	Ceiling types				DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 2948.0 ft²		15.	HVAC credits	
	b. N/A				(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A				HF-Whole house fan,	
11.	Ducts				PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 210.0 ft			MZ-C-Multizone cooling,	
	b. N/A				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-[illegible]4
FAX (904) 755-70[illegible]
[illegible]X
LAKE CITY, FLORIDA [illegible]
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001209

DATE 09/21/2006 PARCEL ID # 22-3S-16-02267-118

APPLICANT LINDA RODER PHONE 752-2281

ADDRESS 387 SW KEMP COURT LAKE CITY FL 32024

OWNER THE DARBY ROGERS CO PHONE 752-6575

ADDRESS COUNTRY LAKE DRIVE LAKE CITY FL 32055

CONTRACTOR BLAKE LUNDE PHONE 754-5810

LOCATION OF PROPERTY 90W, TR ONLAE JEFFREY, TL ON SCENIC LAKE DRIVE, TR ON COUNTRY LAKE DR., 9TH LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT COUNTRY LAKES AT WB 18

SIGNATURE Linda Rodr

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

Blake Construction Company of North Florida
License# CBC1253408
291 S. W. Sisters Welcome Rd., #102
Lake City, Florida 32025
Phone: (386) 754-5810
Fax: (386) 719-6708

September 20, 2006

Joe Haltiwanger
Columbia County Building & Zoning
Plan Review
135 NE Hernando Ave.
Lake City, FL 32055

Re: Permit Application Number 0609-29

Joe,

In reference to the above mentioned permit application, I am providing the additional information you requested.

1. Windows N & G, according to plan specification schedule, sheet #3, will be tempered to comply with Section R308.4 of the Florida Building Code.

2. The door going to the guest bath will be changed to #5 (2^8x 6^8) according to the door schedule shown on sheet #3 of the plans.

3. The door schedule on sheet #3 shows the garage walk through door as #3 at 2^8x6^8 exterior one hour fire rated door, which meets Florida Building Code, Section R309.1.

4. Exterior wall chimney has an elevation of 25.5' and extends 13.5' from closest ridge. The highest ridge elevation is 29'. The second interior chimney will be eliminated.

5. Truss area involving trusses #T05 are for storage only and will not be erected as habitable space.

6. All bedroom outlets shall be wired with arc-fault circuit interrupter protection. Also, the main meter can will have an exterior disconnect supplied with 4-wire conductor.

If you have any questions, please give me a call at 754-5810.

Sincerely,

Blake N. Lunde, II
President

BNL/nlt

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0609-29

Contractor Blake Construction Owner: The Darby Rogers Company Property ID#22-3s-16-02268-004

On the date of September 15, 2006 application 0609-29 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0609-29 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

1. The windows in the bathrooms will be required to comply with section R308.4 of the Florida Residential Building Code: Hazardous locations: Glazing in doors and enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers. Glazing in any part of a building wall enclosing these compartments where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface. Each pane of glazing installed in hazardous locations as defined in Section R308.4 shall be provided with a manufacturer's or installer's label, designating the type and thickness of glass and the safety glazing standard with which it complies, which is visible in the final installation. The label shall be acid etched, sandblasted, ceramic-fired, embossed mark, or shall be of a type which once applied cannot be removed without being destroyed. Please verify that these windows comply with this building code requirement.
2. Please provide for compliance with the Florida Residential Building Code section R322.1.1 All new single-family houses, duplexes, triplexes, condominiums and townhouses shall provide at least one bathroom, located with maximum possible privacy, where bathrooms are provided on habitable grade levels, with a door that has a 29-inch (737 mm) clear opening. However, if only a toilet room is provided at grade level, such toilet rooms shall have a clear opening of not less than 29 inches (737 mm).

3. In the garage area the following code requirements need to be corrected or verified.

A. Please verify that section R309.1 of the Florida Residential Building Code will be complied with as this section relates to the garage entry door in to the residence. Opening protection: Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.

4. On the dwelling elevation drawing show the height of the chimney flue above the roof peak and the distance from the nearest roof line intersection. Sections R1001.17 of the Florida Residential Building Code requires, Chimney crickets: Chimneys shall be provided with crickets when the dimension parallel to the ridgeline is greater than 30 inches (762 mm) and does not intersect the ridgeline. The intersection of the cricket and the chimney shall be flashed and counter flashed in the same manner as normal roof-chimney intersections. Crickets shall be constructed in compliance with Figure R1001.17 and Table R1001.17.

4. On the truss drawing Truss T05 is designed for attic storage and is not considered to be a habitable room by the Florida Residential Building Code sections R305.1 minimum height. Habitable rooms, hallways,

corridors, bathrooms, toilet rooms, laundry rooms and basements shall have a ceiling height of not less than 7 feet (2134 mm). The required height shall be measured from the finish floor to the lowest projection from the ceiling. Therefore the ceiling covering in the garage area will not be required to meet the requirements of section R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent. Please verify that this attic space will remain as an uninhabitable room.

5. Please verify that compliance with the requirement of the 2005 National Electrical Code section 210.12 Arc-Fault Circuit-Interrupter Protection and the Florida Residential Building code 2004 section R313.1 will be complied with. Also that an overcurrent protection device will be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

6. Any structural designs within the plans which were designed by Donald A. Gardner Inc. which relate to design requirements of the Florida Residential

Building Code sections R301 design criteria shall not be used. Mr. Mark Disosway of Windload Engineering structural engineering design shall be used. Any and all question which relate to the requirements of sections R301 should be address to Mr. Disosway, as the engineer of record for this structure.

Joe Haltiwanger

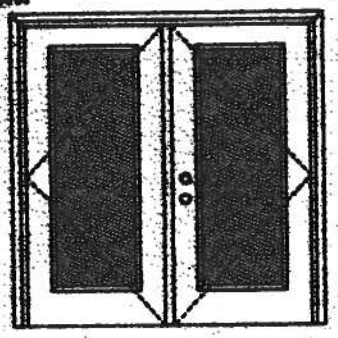
Plan Examiner
Columbia County Building
Department

XX
Glazed Outswing Unit

[redacted]

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'6".

Double Door
Maximum unit size - 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:


100 Series


135, 136 Series


138 Series


600 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


120 Series*


200 Series*


12 Rl., 22 Rl., 34 Rl. Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Johnson Entry Systems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

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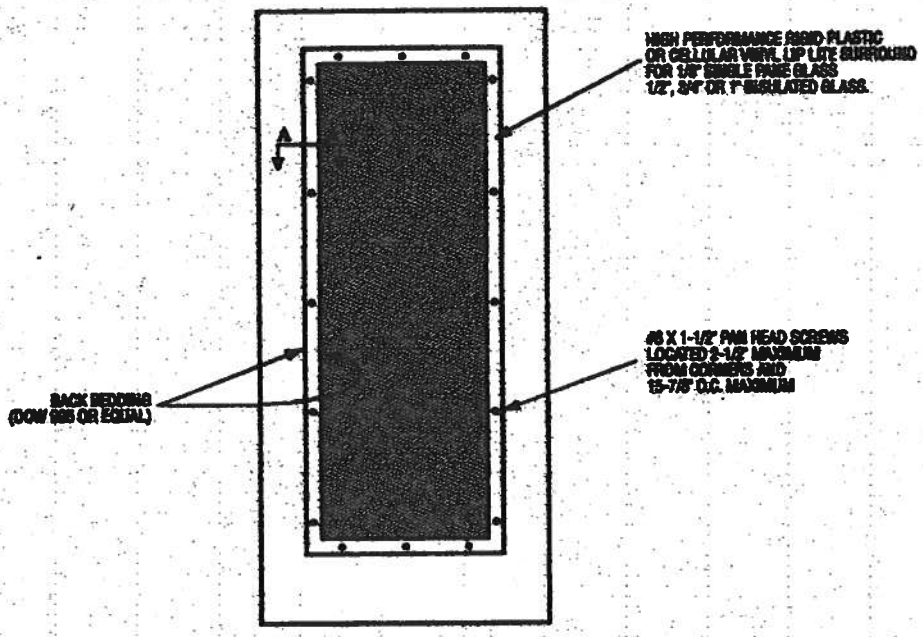
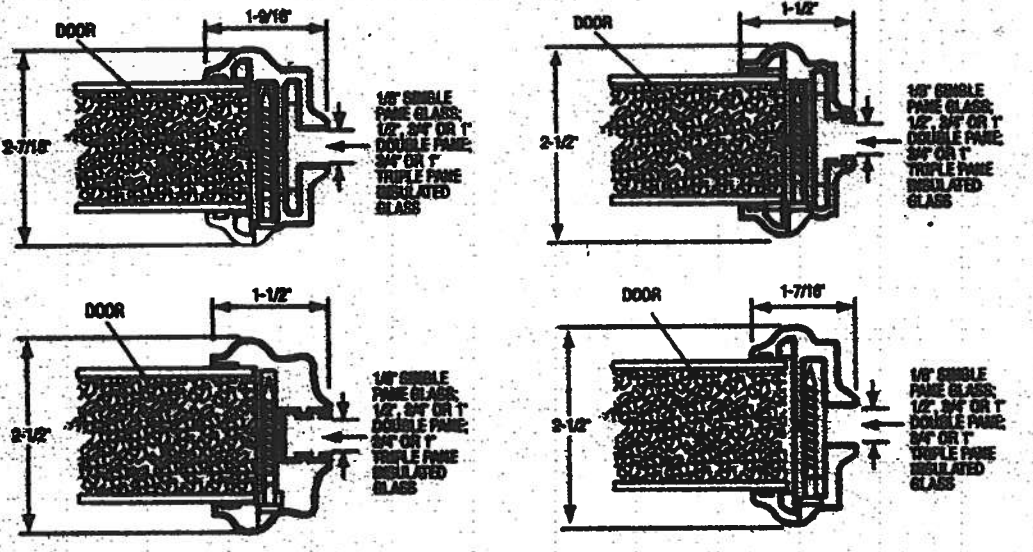
Masonite International Corporation

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



XX
Glazed Outswing Unit

COP-WL-[illegible]-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


140 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 18258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip tite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Johnson Entry Systems

March 29, 2002
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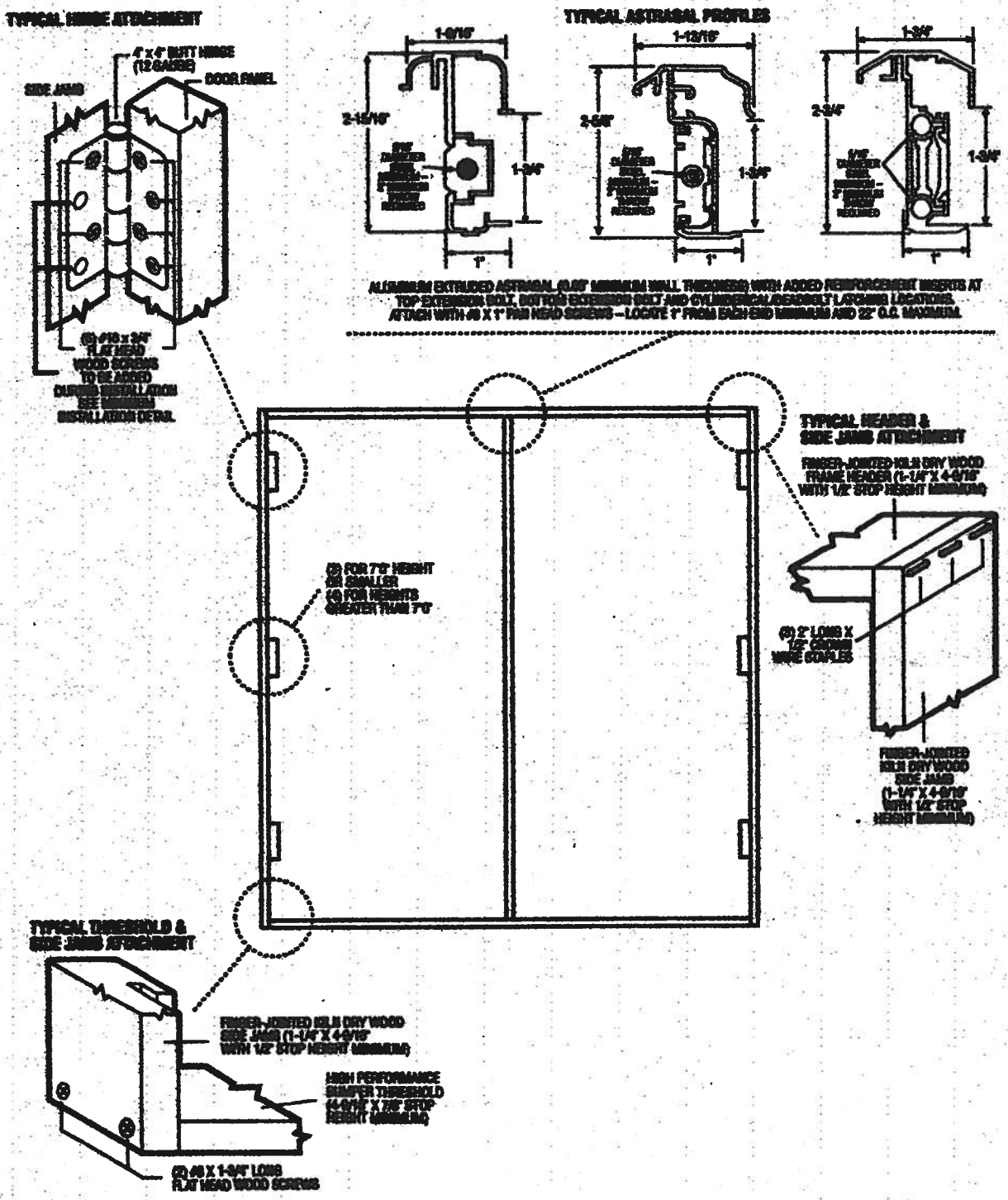
Masonite International Corporation

XX
Unit

MAD-WL-MAD612-02

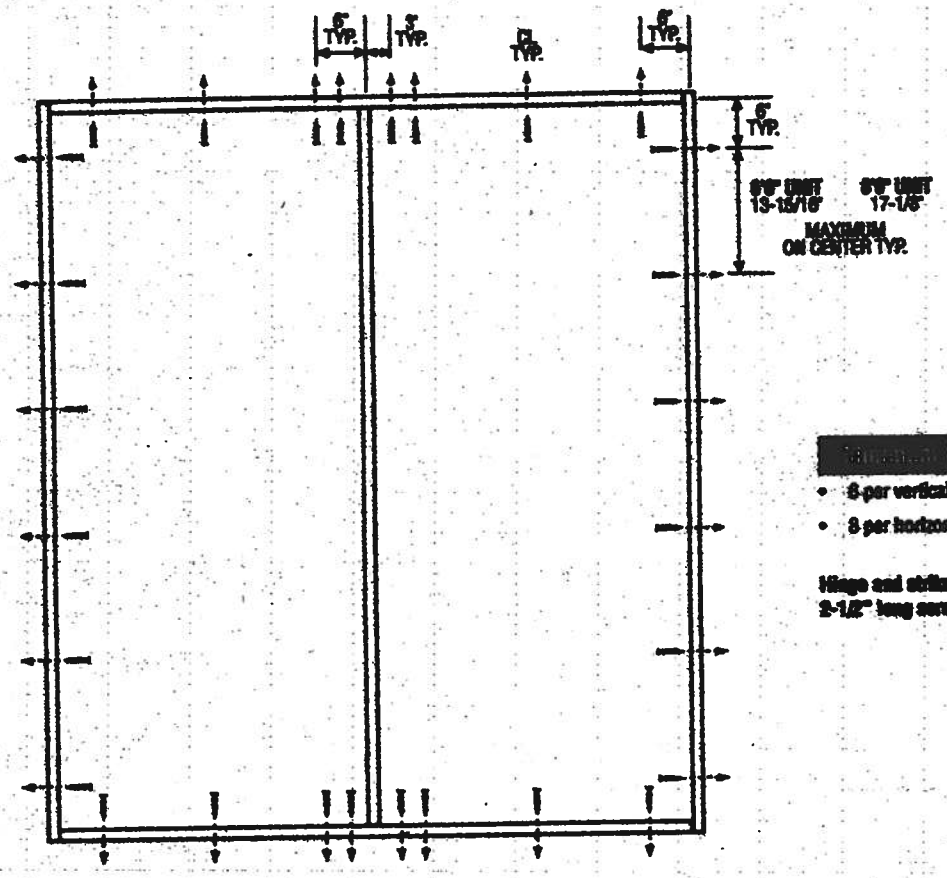
OUTSWING UNITS WITH DOUBLE DOOR



XX
Unit

[redacted]

DOUBLE DOOR



[redacted]

- 8 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

[redacted]

FIBREDOR Premium Quality Doors

Exclusively from Masonite®

Masonite International Corporation

I

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:mlb

ALLEN R. REEVES
LICENSE NO. 11254
STATE OF FLORIDA
PROFESSIONAL ENGINEER

1 APRIL 2002

II

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced vinyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allan G. Reeves
1 APRIL 2002

ALLAN G. REEVES
No. 12384
STATE OF FLORIDA
PROFESSIONAL ENGINEER

III

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES
CERTIFICATE NO. 19354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

IV

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max
	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES ★ CERTIFICATE NO. 18[illegible] ★ STATE OF FLORIDA ★ PROFESSIONAL ENGINEER ★

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00)		
	(with and without screen)		
	WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the meeting rail)		
	(Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Uniform Load Structural (ASTM E 330-97)		
(Measurements reported were taken on the meeting rail)		
(Loads were held for 10 seconds)		
@ 67.5 psf (positive)	0.05"	0.18" max.
@ 70.8 psf (negative)	0.05"	0.18" max.

Allen N. Reeves
1 APRIL 2002

LEII N. REEVES
CERTIFICATE
NO. 18334
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

VI

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

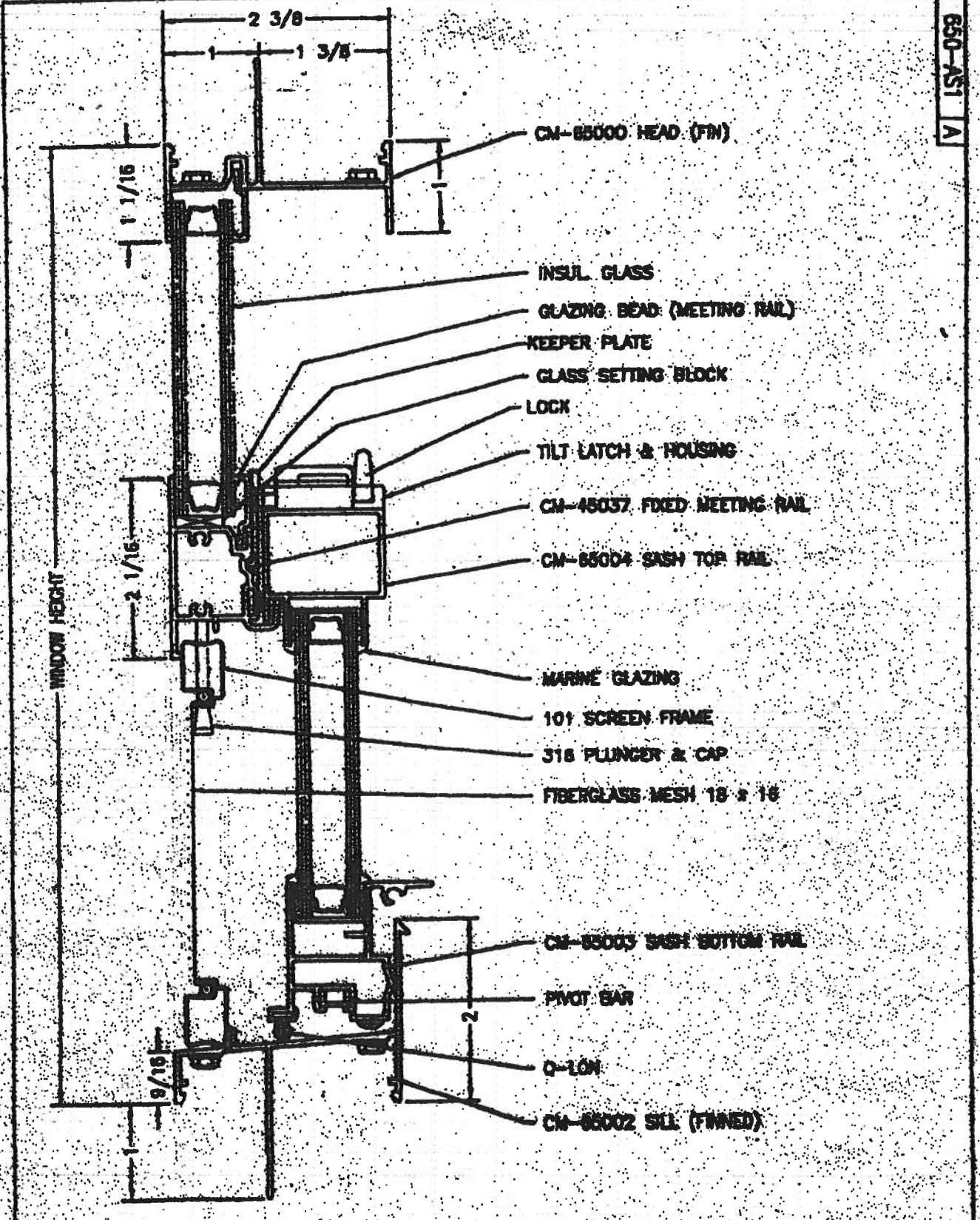
Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:nlb
01-41134.01

ALLEN N. REEVES
CERTIFICATE
NO. 19[illegible]
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

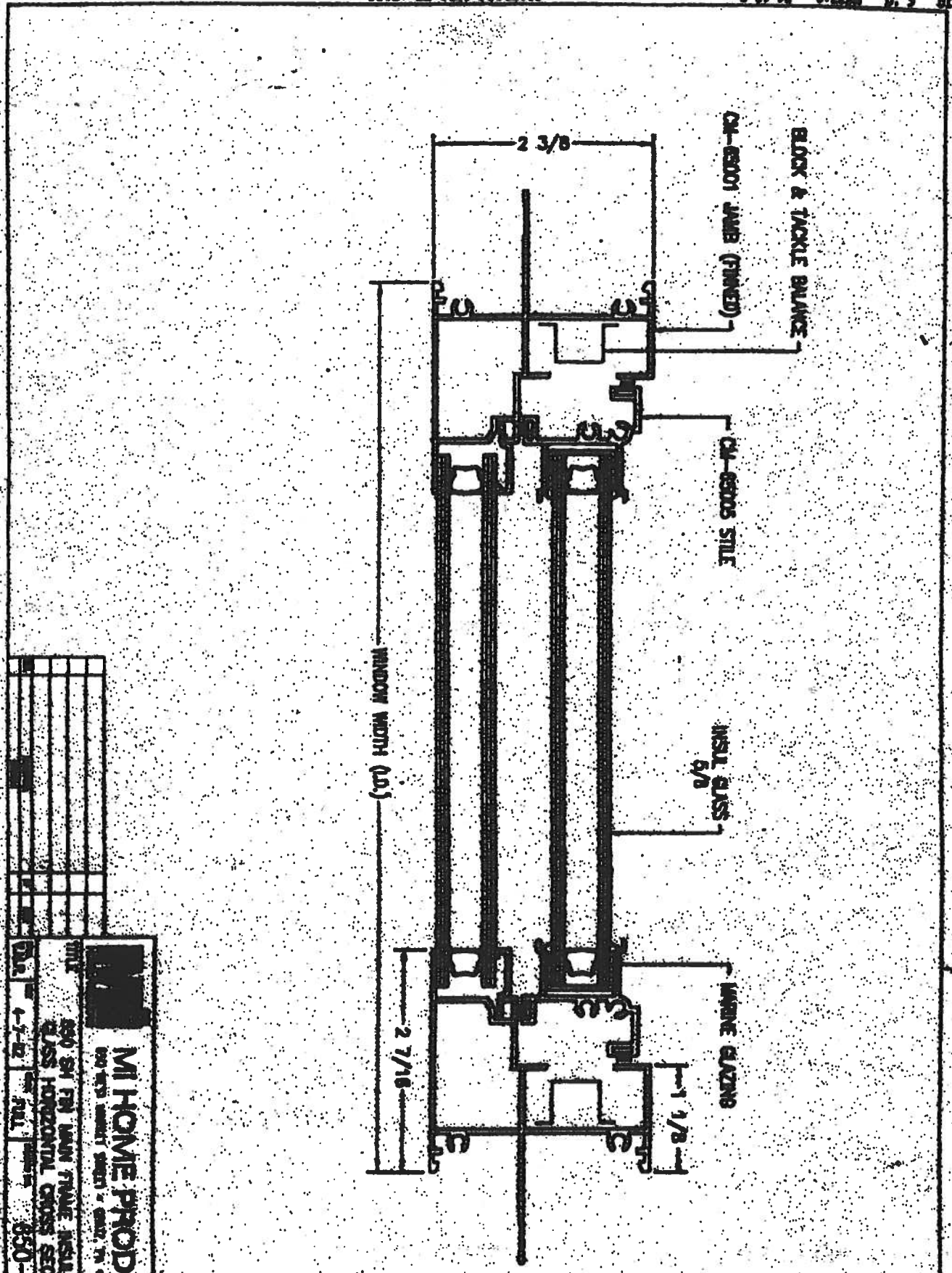
650-AS1 A



MI HOME PRODUCTS
650 NEW MARKET STREET • CHUTE, PA. • 17604-0370

TITLE: 650 SN FIN MAIN FRAME
VERTICAL CROSS SECTION

DRN.	DATE	APP.	DWG. NO.	REV.
V.M.R.	4-7-98	PHIL	650-AS1	A


2 3/8
BLOCK & TACKLE BALANCE
CM-6500 JAMB (FINNED)
CM-6500S STILE
INSUL GLASS 6/8
MARINE GLAZING
1 1/8
2 7/16
WINDOW WIDTH (U.I.)
MI HOME PRODUCTS
850-AS2
B

TAMKO ROOFING PRODUCTS

• Glass-Seal • Glass-Seal AR • Elite Glass-Seal® • Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a 3.25 piece and applied to shingles with a 5 in. exposure, use 6 fasteners per shingle. See Section 3 for the Mansard Fastening Pattern.

5. RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

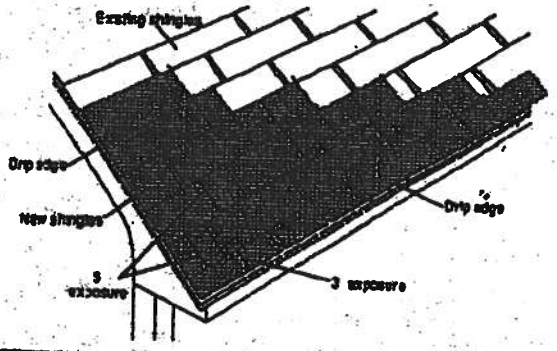
If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nesting procedure described below is the preferred method for re-roofing over square tab strip shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tabs from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 3.

Second and Succeeding Courses: According to the off-set application method you choose to use, remove the appropriate length from the rake end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingle used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.



6. VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Fast® or a minimum 50 lb. roll roofing in the valley. Nail the felt only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 6 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

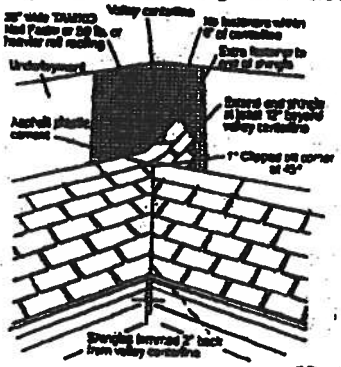
Note: For a neater installation, snap a chalkline over the shingles for guidance.

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

- ***CAUTION:*** *Adhesive must be applied in smooth, thin, even layers.*

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.



(Continued)

TAMKO
ROOFING PRODUCTS

FEB - 4 RECD

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

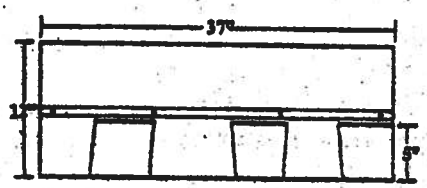
TAMKO Roofing Products, Inc.

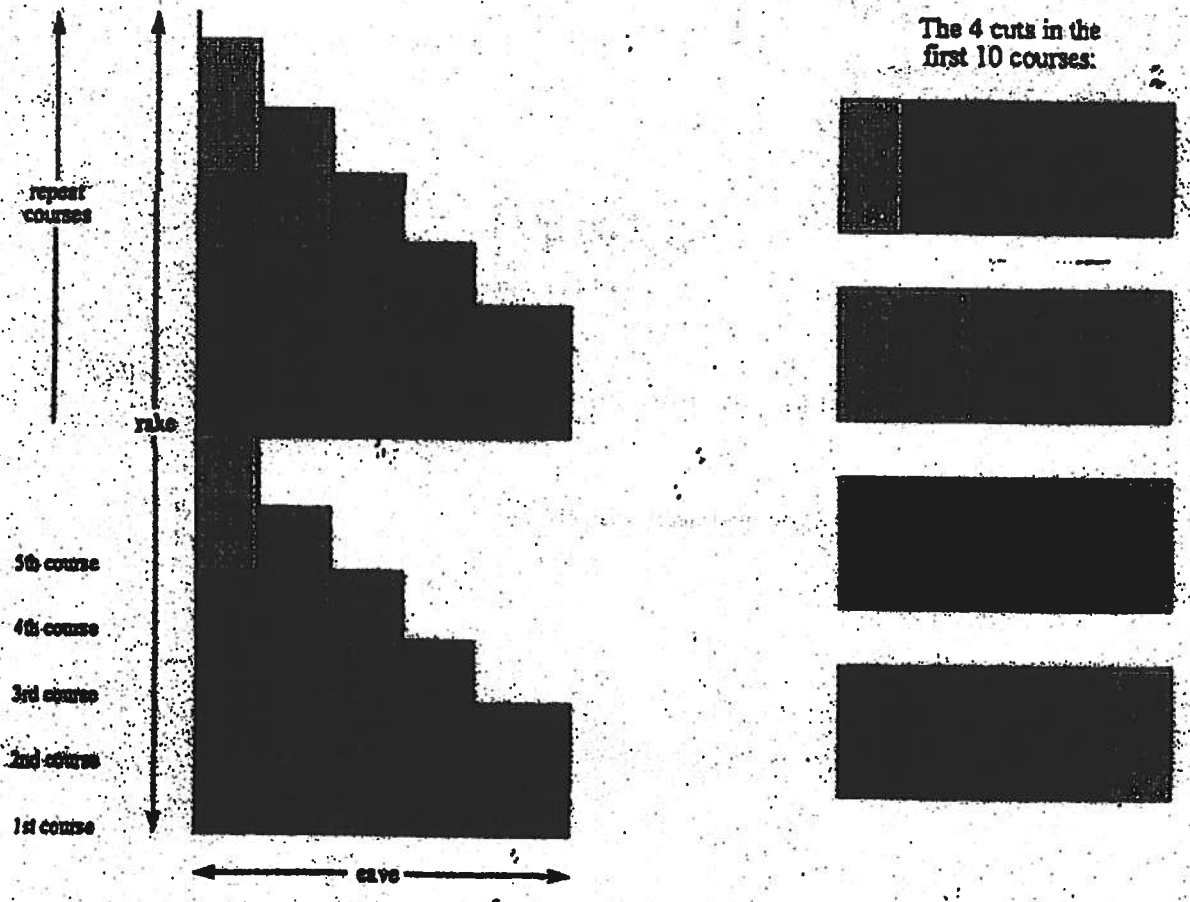
CORPORATE HEADQUARTERS
220 W. FOURTH STREET P.O. BOX 1404 JOPLIN, MO 64802-1404 800-641-4691 FAX 800-641-1925

TAMKO
ROOFING PRODUCTS

Application Instructions For Heritage® 25 Series Shingles

SPECIFICATIONS (APPROX.)	
Length	37"
Width	12"
Bundles per Sq.	3
Shingles per Sq.	78
Shingles per Bundle	26
Coverage per Sq. (Sq. Ft.)	100
Exposure	5"





In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

NOTE: These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.

TAMKO ROOFING PRODUCTS

Application Instructions for

• Glass-Seal
• Glass-Seal AR
• Elite Glass-Seal®
• Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.
THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.
IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thick-nels and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.
FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

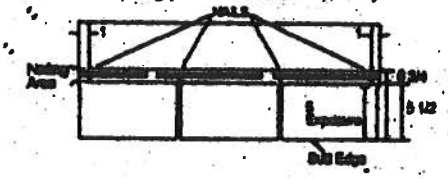
3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

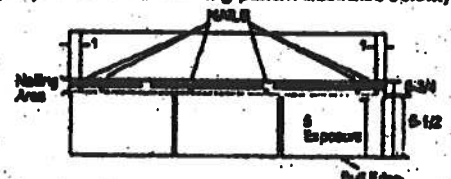
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) **Standard Fastening Pattern.** (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below).



2) **Mansard or High Wind Fastening Pattern.** (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fastener per shingle. (See Mansard fastening pattern illustrated below.)



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

FLORIDA BUILDING CODE

Overview | User Registration | Organization Registration | User Authentication | Organization Search | Organization Accreditation

Select the organization type, status, or name to find an organization

Organization Type: Product Manufacturer

Manufact. Buildings

Approval Status: (ALL)

Organization Name: General American Door - Product Manufacturer

Cancel Search

Result List for Organizations

Displaying 1-1 of 1

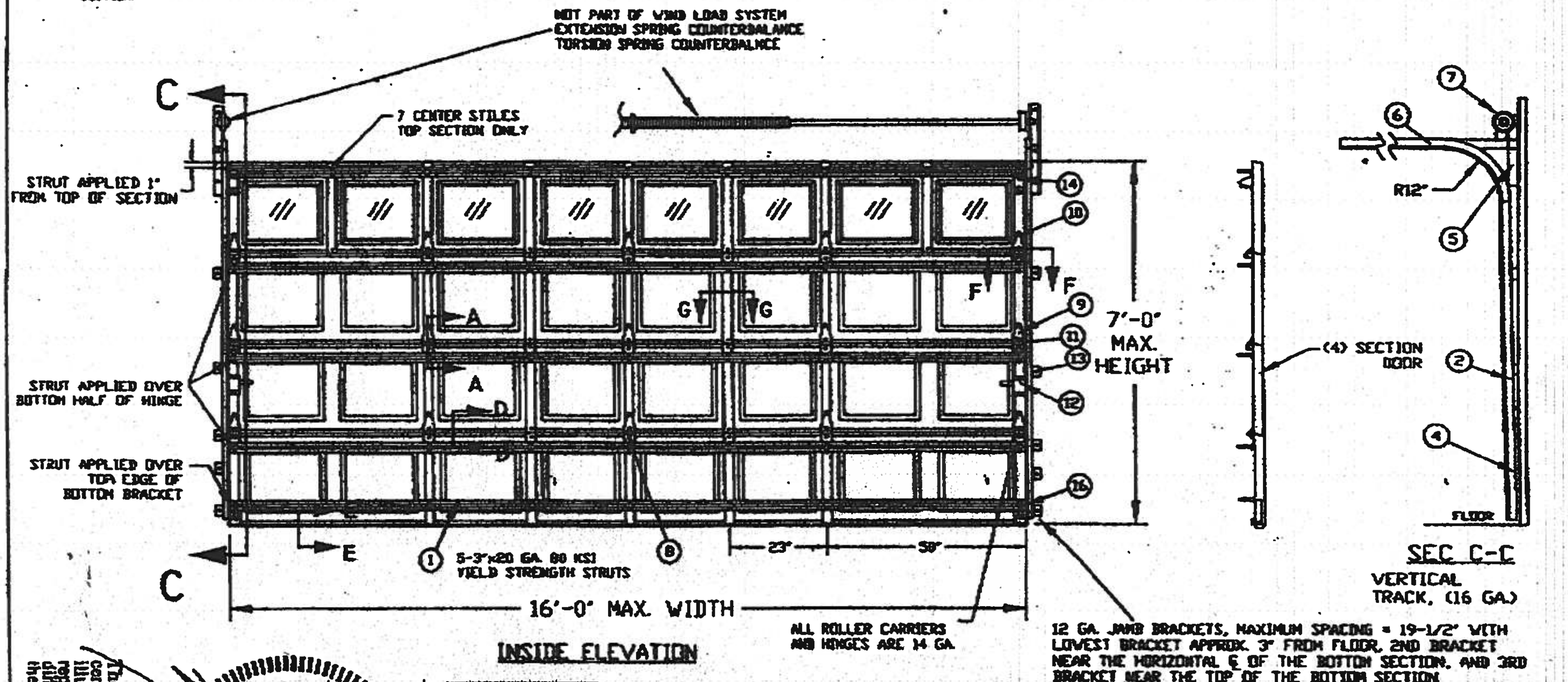
Name	City	Contact	Phone	Type	Expire	Status
General American Door	Montgomery	James Campbell	6308593000	Product Manufacturer	01/01/2099	Approved
Org Codes PDM System IDs: 3585			Site Links: www.gadoo.com			

Displaying 1-1 of 1

REV.	DATE	BY	DESCRIPTION
A	11-10-00	DW	SEE E.C.N. 831

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5½" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED.
9. ⚠ DROP IN TYPE OF INSULATION IS OPTIONAL.



INSIDE ELEVATION

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER YASER R. REYVAN

LISTED

REPORT No. 2202

TEST REPORTS ON FILE VIDEO 10/19/00 (00293)

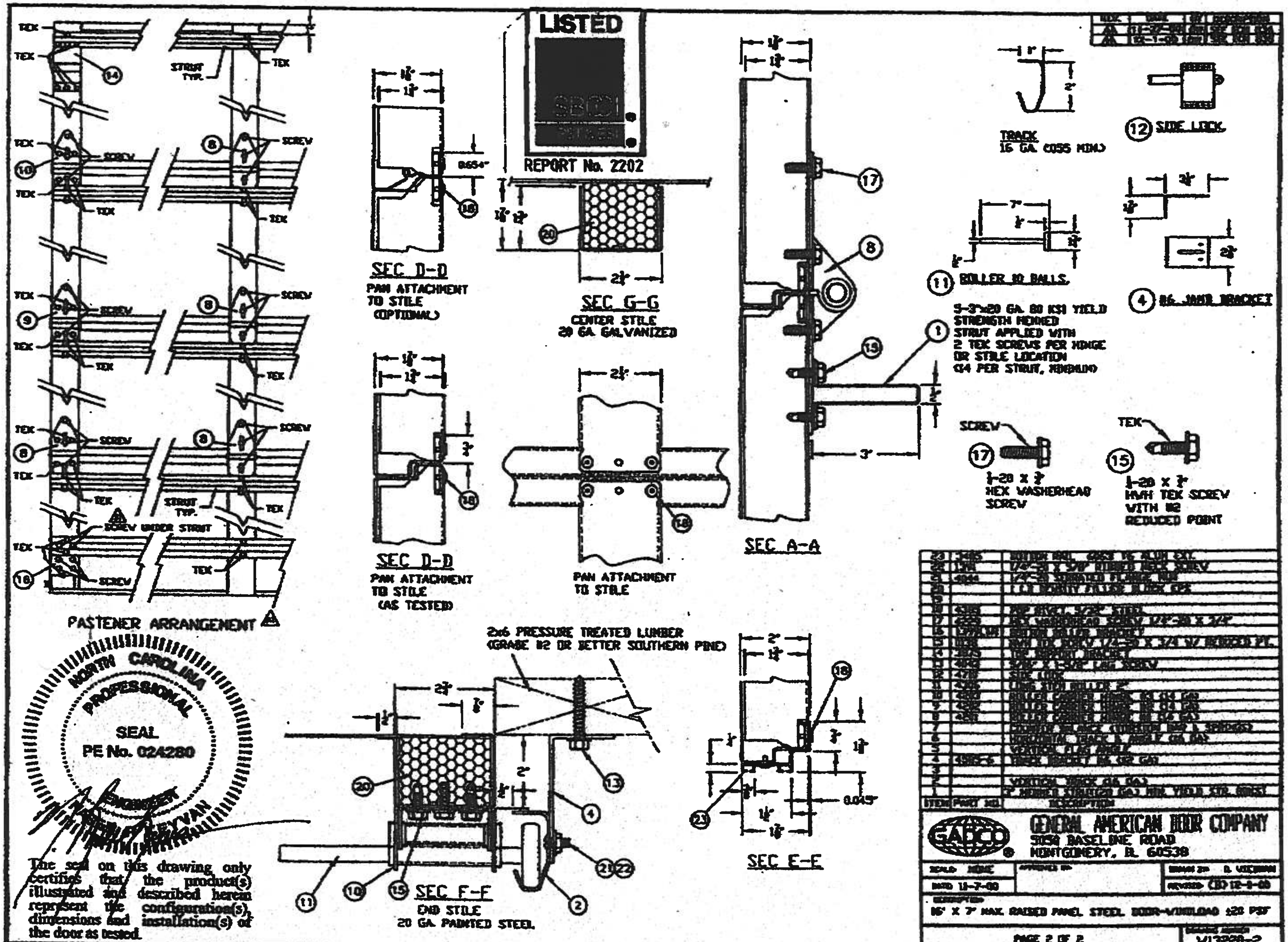
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

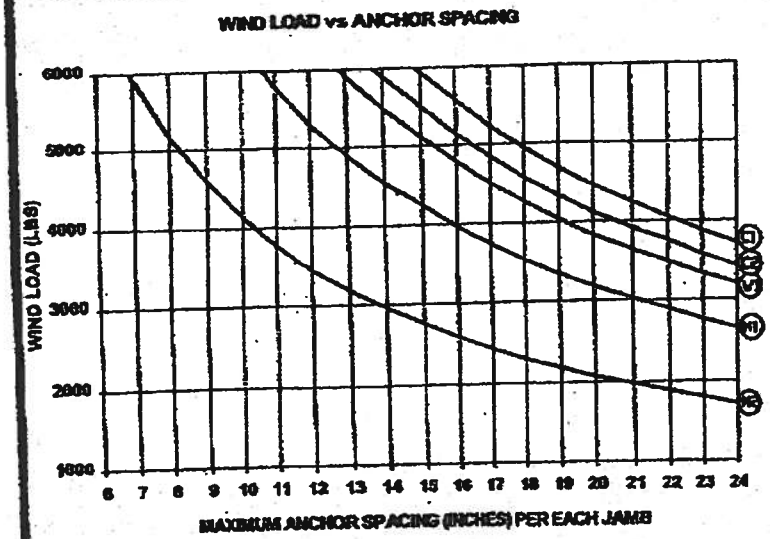
GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN ⚠
SERIES 7524, EXTERIOR STEEL =.024" MIN ⚠
(TESTED WITH WINDOWS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

GADCO ® GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NTS | APPROVED BY | DRAWN BY: D. WIEMANN
DATE 10-20-00 | REVISED (A) 11-10-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR -WINDLOAD ±20 PSF
PART NUMBER | PAGE 1 OF 2 | DRAWING NUMBER W13220-1


FASTENER ARRANGEMENT
TEX
SCREW
STRUT TYP.
SCREW UNDER STRUT
SEC D-D
PAN ATTACHMENT TO STILE (OPTIONAL)
PAN ATTACHMENT TO STILE (AS TESTED)
LISTED
REPORT No. 2202
SEC G-G
CENTER STILE
20 GA. GALVANIZED
PAN ATTACHMENT TO STILE
SEC A-A
TRACK
15 GA. (.055 MIN.)
SIDE LOCK
ROLLER 10 BALLS
#6 JAMB BRACKET
3-3"x20 GA. 80 KSI YIELD STRENGTH HEMMED STRUT APPLIED WITH 2 TEK SCREWS PER HINGE OR STILE LOCATION (34 PER STRUT, MINIMUM)
1/4-20 x 3/4" HEX WASHERHEAD SCREW
1/4-20 x 3/4" HWH TEK SCREW WITH #2 REDUCED POINT
2x6 PRESSURE TREATED LUMBER (GRADE #2 OR BETTER SOUTHERN PINE)
SEC F-F
END STILE
20 GA. PAINTED STEEL
SEC E-E
NORTH CAROLINA PROFESSIONAL SEAL PE No. 024280 ENGINEER
The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.
GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538
16' X 7' MAX. RAISED PANEL STEEL DOOR-WINDLOAD ±20 PSF
PAGE 2 OF 2
W13228-2



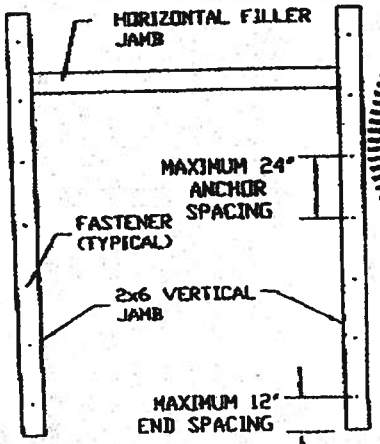
DESIGN LOAD (LBS)/FT² X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

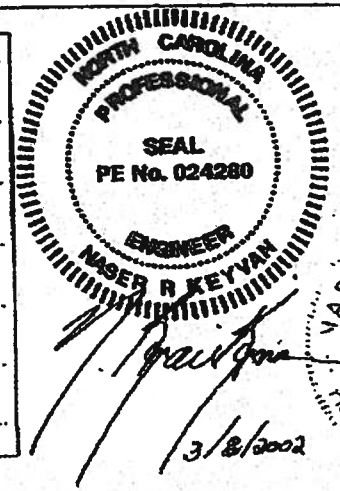
EXAMPLE

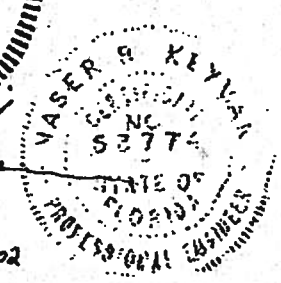
30 LBS/FT² X (16 FT WIDE X 8 FT HIGH) = 3840 LBS

- (C1) USE 22" SPACING
- (C2) USE 21" SPACING
- (W1) USE 19" SPACING
- (M1) USE 16" SPACING
- (M2) USE 10" SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS







2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI, GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI, REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4".

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GADCO

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE	APPROVED BY:	DRAWN BY: DV
DATE: 8-30-99		REVISED

DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS

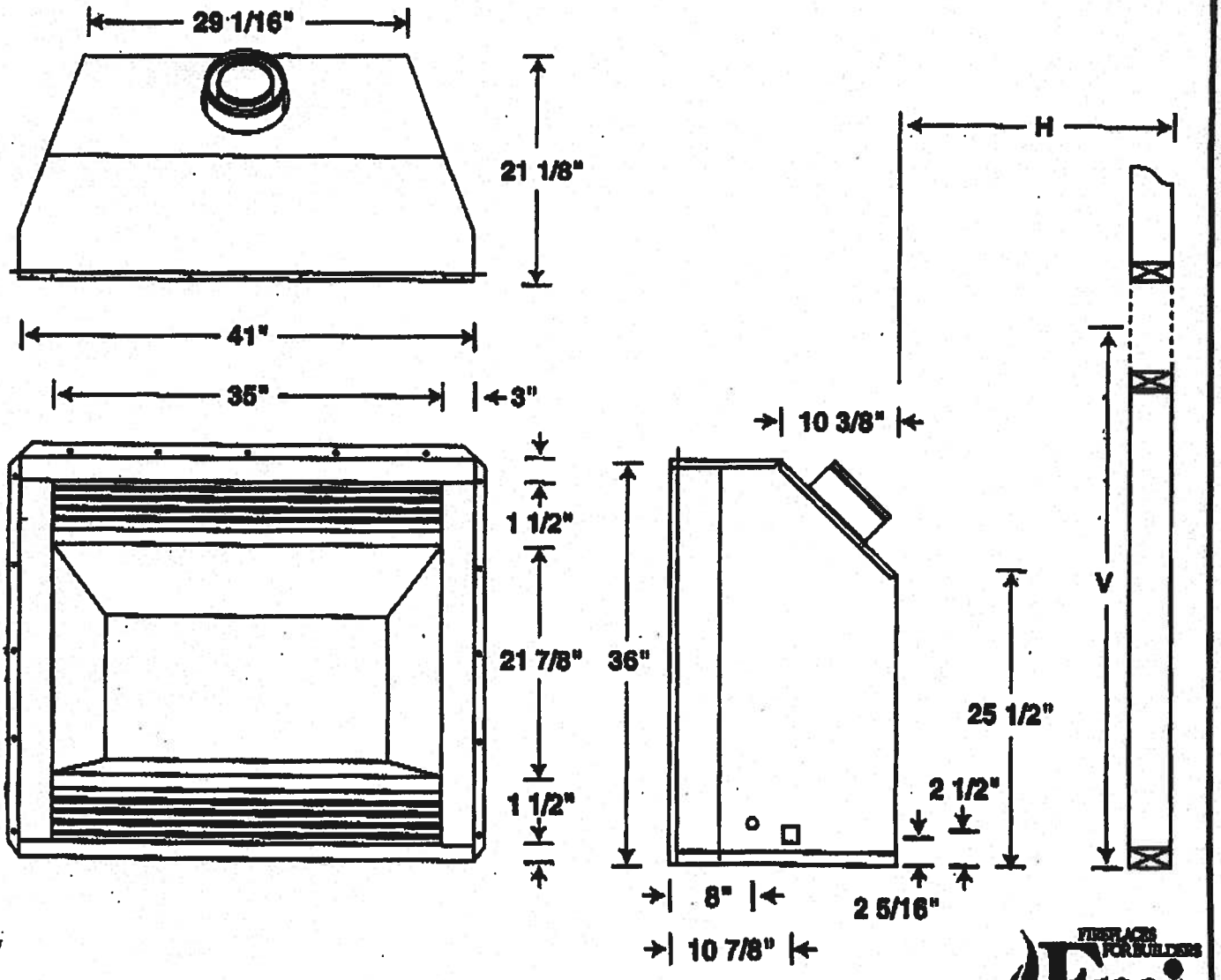
MASTER DRAWING NUMBER: A10560 DRAWING NUMBER

Victorian

36" Direct Vent Fireplace
(5" - 8" Vent Pipe)

Typ. Ground Floor Installation (1 - 45° Elbow)		
Horiz. Run (H)	Min. Height (V)	Required Horiz. Pipe
17" max.	36"	12" max.

Installations requiring a 45° and 90° Elbow		
Horiz. Run (H)	Min. Height (V)	Required Vert. Pipe
30" max.	47 1/4"	none
48" max.	57 1/4"	12"
60" max.	69 1/4"	24"
84" max.	81 1/4"	36"
144" max.	93 1/4"	48"



Fmi FIREPLACES FOR BUILDERS

THE 'RENAISSANCE SERIES

Victorian

36" AND 42" DIRECT VENT GAS FIREPLACES

Model V36 and V42

Timeless Beauty— And The Latest Technologies

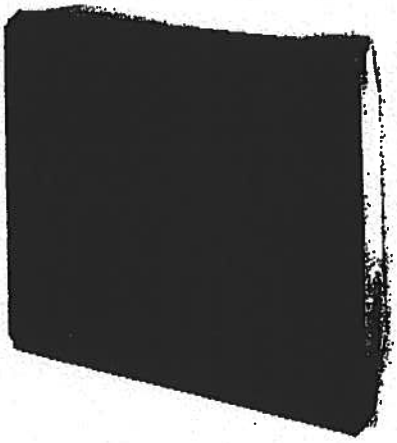
FMI's Victorian direct vent gas fireplaces are the ideal match for today's energy-efficient homes. The Victorian is the centerpiece of our exciting new Renaissance Series, which offers a consistent look, sizing, and construction across the entire line... plus beautiful new features homeowners will love!

Homeowner Highlights:

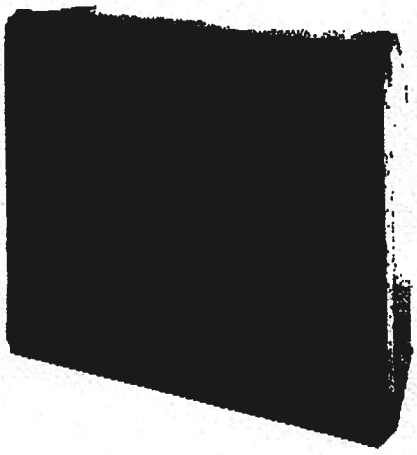
- **Distinctive looks**—Features random flame pattern and realistic glowing ember bed burner... plus exquisite new split oak ceramic fiber logs.
- **Operation and maintenance are a breeze**—Operates from wall switch or remote control. Hinged glass door swings open for easy maintenance and never needs adjustment.
- **Attractive accessories**—You have an array of eye-catching extras, including brass or platinum louvers and trim, realistic textured brick liner kits, and much more.

Builder Benefits:

- **Secure, straight installation**—We've added full-length nailing flanges, and drywall stops.
- **Venting options**—Our 45° slant back design lets you choose between horizontal and vertical venting for painless installation. Your choice of hard or flexible venting.
- **More standard features**—Flex gas connector, shut-off valve and pre-wired "J" box are all standard.



V36N features black rolled louvers.



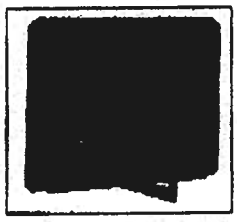
V42NH features black rolled louvers and textured herringbone brick-lined interior.

Victorian Direct Vent Fireplace Product Offering Summary

36" & 42" Direct Vent Fireplace Models Available With The Following:

- Millivolt Or Electronic Ignition
- Natural Or Propane Fuel
- Black, Standard Brick, And Herringbone Pattern Refractory Brick Interiors
- All fireplaces use 5" - 8" pipe. 36" models @ 32,000 Btu/42" models @ 35,000Btu.

Victorian models offer random, tiered flame patterns and gorgeous glowing ember bed burners.

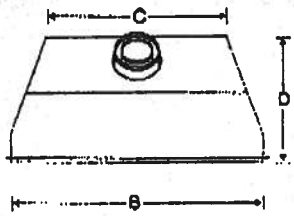


Hinged, tool-less entry door swings open for easy maintenance.

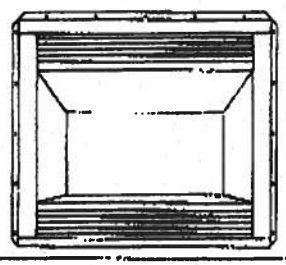
Accessory Offering Summary

- Smooth Face, Stamped Steel and Rolled Black Louver Panels
- Louver Trim (Brushed Brass & Platinum)
- Perimeter Trim Kits (Black, Brushed Brass & Platinum)
- Standard & Herringbone Refractory Brick Liners
- Remote Control Kits
- Fan Kits
- Deflection Hoods

Dimensions



	36"	42"
A	35	42
B	41	48
C	29	36
D	21 1/8	23 1/8
E	10 1/3	10 1/3
X	21 3/4	29 3/4
Y	38	40
Z	25 1/2	29 1/3



FIREPLACES FOR BUILDERS Fmi

DESA International
www.desaint.com
For more information, call (800) 888-2050

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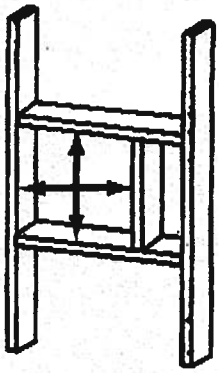
FA2913-0601

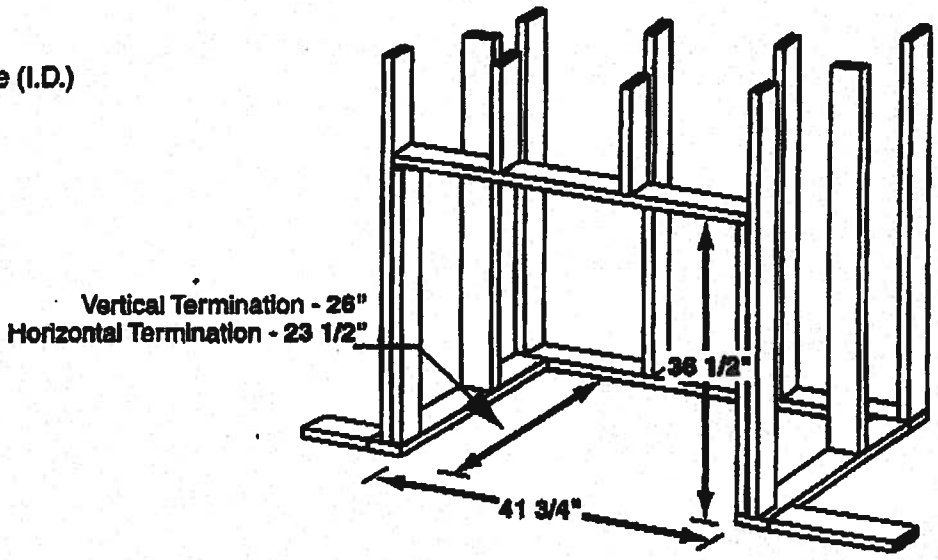
Printed in U.S.A.

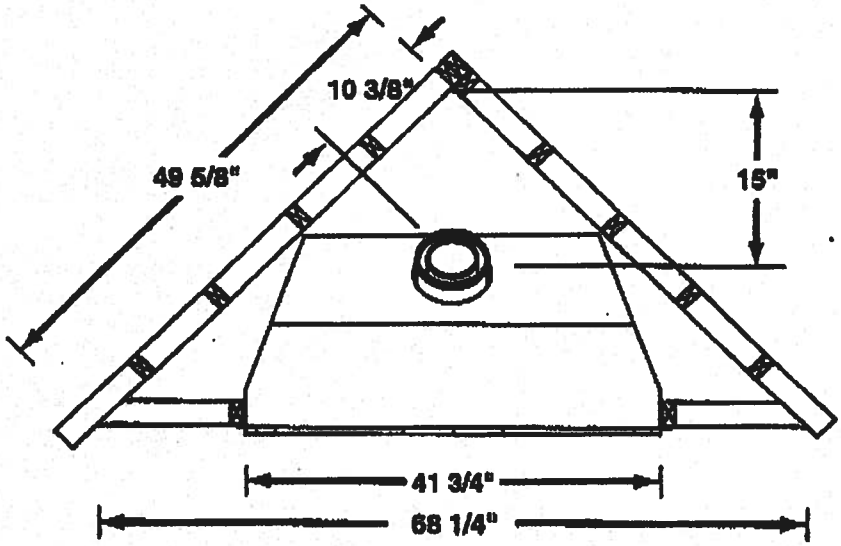
Victorian
36" Direct Vent Fireplace

Framing Dimensions

Vent Opening - 10 3/4" Square (I.D.)







NOTE:
Built-in Features Such as Mantels, Bookshelfs, etc. Made of Combustible Materials Must Maintain Minimum Clearances from the Fireplace. See Installation Instructions for Complete Information

FIREPLACES FOR HOME
Fmi

Residential System Sizing Calculation

Summary

Spec House
, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

8/21/2006

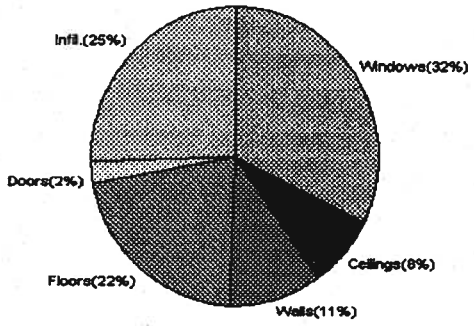
Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)

Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**44864**	**Btuh**	**Total cooling load calculation**	**41686**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	115.9	52000	Sensible (SHR = 0.75)	112.0	39000
Heat Pump + Auxiliary(0.0kW)	115.9	52000	Latent	189.7	13000
			Total (Electric Heat Pump)	124.7	52000

WINTER CALCULATIONS

Winter Heating Load (for 2818 sqft)

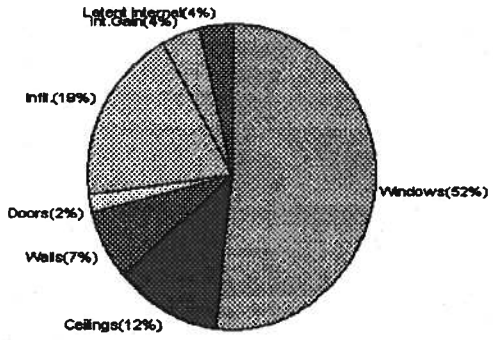
Load component			Load	
Window total	452	sqft	14550	Btuh
Wall total	1465	sqft	4811	Btuh
Door total	80	sqft	1036	Btuh
Ceiling total	2948	sqft	3474	Btuh
Floor total	222	sqft	9693	Btuh
Infiltration	279	cfm	11301	Btuh
Duct loss			0	Btuh
Subtotal			**44864**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**44864**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 2818 sqft)

Load component			Load	
Window total	452	sqft	21597	Btuh
Wall total	1465	sqft	3056	Btuh
Door total	80	sqft	784	Btuh
Ceiling total	2948	sqft	4882	Btuh
Floor total			0	Btuh
Infiltration	144	cfm	2675	Btuh
Internal gain			1840	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**34834**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			5252	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1600	Btuh
Total latent gain			**6852**	**Btuh**
TOTAL HEAT GAIN			**41686**	**Btuh**



For Florida residences only

EnergyGauge® System Sizing
PREPARED BY:
DATE: 6-21-06

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House
, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

8/21/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	24.7	32.2	795 Btuh
2	2, Clear, Metal, 0.87	NW	32.9	32.2	1059 Btuh
3	2, Clear, Metal, 0.87	NW	24.7	32.2	795 Btuh
4	2, Clear, Metal, 0.87	N	24.7	32.2	795 Btuh
5	2, Clear, Metal, 0.87	NW	57.6	32.2	1854 Btuh
6	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
7	2, Clear, Metal, 0.87	W	11.7	32.2	377 Btuh
8	2, Clear, Metal, 0.87	NW	27.2	32.2	876 Btuh
9	2, Clear, Metal, 0.87	N	13.6	32.2	438 Btuh
10	2, Clear, Metal, 0.87	NE	9.7	32.2	312 Btuh
11	2, Clear, Metal, 0.87	SE	19.4	32.2	624 Btuh
12	2, Clear, Metal, 0.87	SE	24.7	32.2	795 Btuh
13	2, Clear, Metal, 0.87	SE	45.2	32.2	1455 Btuh
14	2, Clear, Metal, 0.87	SE	24.7	32.2	795 Btuh
15	2, Clear, Metal, 0.87	SE	20.0	32.2	644 Btuh
16	2, Clear, Metal, 0.87	SW	62.2	32.2	2002 Btuh
17	2, Clear, Metal, 0.87	SW	9.0	32.2	290 Btuh
	Window Total		452(sqft)		14550 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1275	3.3	4187 Btuh
2	Frame - Wood - Ext(0.09)	13.0	190	3.3	624 Btuh
	Wall Total		1465		4811 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		40	12.9	518 Btuh
	Door Total		80		1036Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	2948	1.2	3474 Btuh
	Ceiling Total		2948		3474Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	222.0 ft(p)	43.7	9693 Btuh
	Floor Total		222		9693 Btuh
			Zone Envelope Subtotal:		33563 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.66	25362	279.0	11301 Btuh
Ductload	Unsealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**44864 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House
, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

8/21/2006

WHOLE HOUSE TOTALS

	Subtotal Sensible	44864 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	44864 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Spec House
, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

8/21/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	24.7	32.2	795 Btuh
2	2, Clear, Metal, 0.87	NW	32.9	32.2	1059 Btuh
3	2, Clear, Metal, 0.87	NW	24.7	32.2	795 Btuh
4	2, Clear, Metal, 0.87	N	24.7	32.2	795 Btuh
5	2, Clear, Metal, 0.87	NW	57.6	32.2	1854 Btuh
6	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
7	2, Clear, Metal, 0.87	W	11.7	32.2	377 Btuh
8	2, Clear, Metal, 0.87	NW	27.2	32.2	876 Btuh
9	2, Clear, Metal, 0.87	N	13.6	32.2	438 Btuh
10	2, Clear, Metal, 0.87	NE	9.7	32.2	312 Btuh
11	2, Clear, Metal, 0.87	SE	19.4	32.2	624 Btuh
12	2, Clear, Metal, 0.87	SE	24.7	32.2	795 Btuh
13	2, Clear, Metal, 0.87	SE	45.2	32.2	1455 Btuh
14	2, Clear, Metal, 0.87	SE	24.7	32.2	795 Btuh
15	2, Clear, Metal, 0.87	SE	20.0	32.2	644 Btuh
16	2, Clear, Metal, 0.87	SW	62.2	32.2	2002 Btuh
17	2, Clear, Metal, 0.87	SW	9.0	32.2	290 Btuh
	Window Total		452(sqft)		14550 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1275	3.3	4187 Btuh
2	Frame - Wood - Ext(0.09)	13.0	190	3.3	624 Btuh
	Wall Total		1465		4811 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		40	12.9	518 Btuh
	Door Total		80		1036Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	2948	1.2	3474 Btuh
	Ceiling Total		2948		3474Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	222.0 ft(p)	43.7	9693 Btuh
	Floor Total		222		9693 Btuh
			Zone Envelope Subtotal:		33563 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.66	25362	279.0	11301 Btuh
Ductload	Unsealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**44864 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

8/21/2006

WHOLE HOUSE TOTALS

	Subtotal Sensible	44864 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	44864 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House
, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 8/21/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	0ft.	24.7	0.0	24.7	29	60	1483 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	0ft.	32.9	0.0	32.9	29	60	1975 Btuh
3	2, Clear, 0.87, None,N,N	NW	1.5ft.	7.5ft.	24.7	0.0	24.7	29	60	1483 Btuh
4	2, Clear, 0.87, None,N,N	N	1.5ft.	7.5ft.	24.7	0.0	24.7	29	29	715 Btuh
5	2, Clear, 0.87, None,N,N	NW	7ft.	7.5ft.	57.6	0.0	57.6	29	60	3458 Btuh
6	2, Clear, 0.87, None,N,N	NW	7ft.	6.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
7	2, Clear, 0.87, None,N,N	W	13ft.	6.5ft.	11.7	11.7	0.0	29	80	339 Btuh
8	2, Clear, 0.87, None,N,N	NW	6ft.	6.5ft.	27.2	0.0	27.2	29	60	1633 Btuh
9	2, Clear, 0.87, None,N,N	N	13ft.	6.5ft.	13.6	0.0	13.6	29	29	394 Btuh
10	2, Clear, 0.87, None,N,N	NE	1.5ft.	5.5ft.	9.7	0.0	9.7	29	60	582 Btuh
11	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	19.4	5.5	13.9	29	63	1027 Btuh
12	2, Clear, 0.87, None,N,N	SE	7ft.	7.5ft.	24.7	24.7	0.0	29	63	715 Btuh
13	2, Clear, 0.87, None,N,N	SE	7ft.	7.5ft.	45.2	45.2	0.0	29	63	1309 Btuh
14	2, Clear, 0.87, None,N,N	SE	7ft.	7.5ft.	24.7	24.7	0.0	29	63	715 Btuh
15	2, Clear, 0.87, None,N,N	SE	10ft.	8ft.	20.0	20.0	0.0	29	63	579 Btuh
16	2, Clear, 0.87, None,N,N	SW	1.5ft.	7.25f	62.2	11.8	50.4	29	63	3493 Btuh
17	2, Clear, 0.87, None,N,N	SW	1.5ft.	6ft.	9.0	2.0	7.0	29	63	494 Btuh
	Window Total				452 (sqft)					21597 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1275.0	2.1	2659 Btuh
2	Frame - Wood - Ext	13.0/0.09	190.0	2.1	396 Btuh
	Wall Total		1465 (sqft)		3056 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
3	Insulated - Exterior	40.0	9.8	392 Btuh
	Door Total	80 (sqft)		784 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	2948.0	1.7	4882 Btuh
	Ceiling Total		2948 (sqft)		4882 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	222 (ft(p))	0.0	0 Btuh
	Floor Total		222.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		30319 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.34	25362	143.7	2675 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		8	X 230 +	0	1840 Btuh
Duct load	Unsealed, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		**34834 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

8/21/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**34834 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**34834 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**34834 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	5252 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6852 Btuh**
	TOTAL GAIN	**41686 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Spec House
, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 8/21/2006
This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	0ft.	24.7	0.0	24.7	29	60	1483 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	0ft.	32.9	0.0	32.9	29	60	1975 Btuh
3	2, Clear, 0.87, None,N,N	NW	1.5ft.	7.5ft.	24.7	0.0	24.7	29	60	1483 Btuh
4	2, Clear, 0.87, None,N,N	N	1.5ft.	7.5ft.	24.7	0.0	24.7	29	29	715 Btuh
5	2, Clear, 0.87, None,N,N	NW	7ft.	7.5ft.	57.6	0.0	57.6	29	60	3458 Btuh
6	2, Clear, 0.87, None,N,N	NW	7ft.	6.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
7	2, Clear, 0.87, None,N,N	W	13ft.	6.5ft.	11.7	11.7	0.0	29	80	339 Btuh
8	2, Clear, 0.87, None,N,N	NW	6ft.	6.5ft.	27.2	0.0	27.2	29	60	1633 Btuh
9	2, Clear, 0.87, None,N,N	N	13ft.	6.5ft.	13.6	0.0	13.6	29	29	394 Btuh
10	2, Clear, 0.87, None,N,N	NE	1.5ft.	5.5ft.	9.7	0.0	9.7	29	60	582 Btuh
11	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	19.4	5.5	13.9	29	63	1027 Btuh
12	2, Clear, 0.87, None,N,N	SE	7ft.	7.5ft.	24.7	24.7	0.0	29	63	715 Btuh
13	2, Clear, 0.87, None,N,N	SE	7ft.	7.5ft.	45.2	45.2	0.0	29	63	1309 Btuh
14	2, Clear, 0.87, None,N,N	SE	7ft.	7.5ft.	24.7	24.7	0.0	29	63	715 Btuh
15	2, Clear, 0.87, None,N,N	SE	10ft.	8ft.	20.0	20.0	0.0	29	63	579 Btuh
16	2, Clear, 0.87, None,N,N	SW	1.5ft.	7.25f	62.2	11.8	50.4	29	63	3493 Btuh
17	2, Clear, 0.87, None,N,N	SW	1.5ft.	6ft.	9.0	2.0	7.0	29	63	494 Btuh
	Window Total				452 (sqft)					21597 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1275.0	2.1	2659 Btuh
2	Frame - Wood - Ext	13.0/0.09	190.0	2.1	396 Btuh
	Wall Total		1465 (sqft)		3056 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
3	Insulated - Exterior	40.0	9.8	392 Btuh
	Door Total	80 (sqft)		784 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	2948.0	1.7	4882 Btuh
	Ceiling Total		2948 (sqft)		4882 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	222 (ft(p))	0.0	0 Btuh
	Floor Total		222.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		30319 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.34	25362	143.7	2675 Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load
	8	X 230 +	0	1840 Btuh

Duct load	Unsealed, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh
		Sensible Zone Load	**34834 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

Class 3 Rating
Registration No. 0
Climate: North

8/21/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**34834 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**34834 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**34834 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	5252 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6852 Btuh**
	TOTAL GAIN	**41686 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Spec House

, FL

Project Title:
608155BlakeConstruction\Lot18CountryLakeinWoo

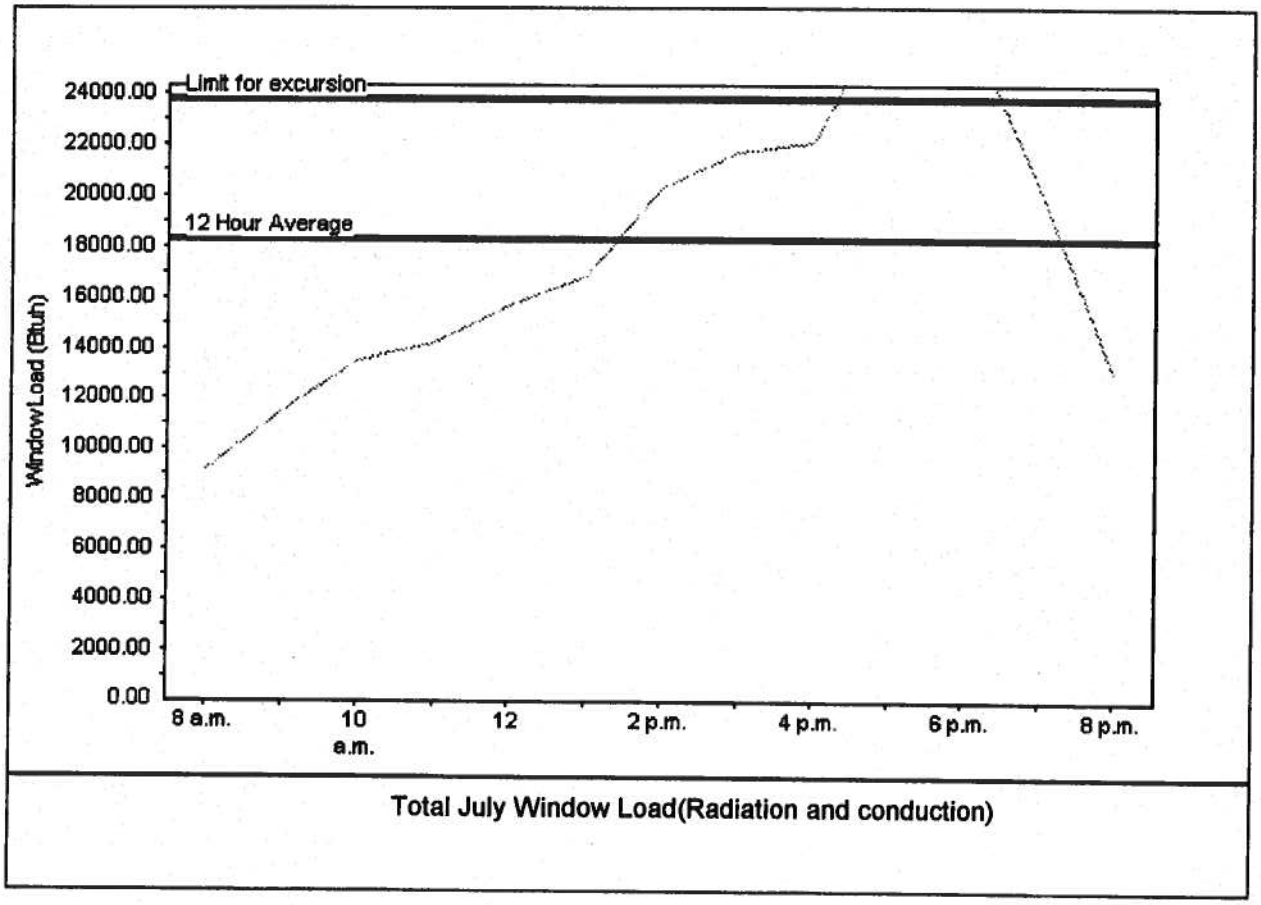
Class 3 Rating
Registration No. 0
Climate: North

8/21/2006

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	18324 Btu
Summer setpoint	75 F	Peak window load for July	27658 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	23822 Btu
Latitude	29 North	Window excursion (July)	3836 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY:
DATE:

Notice of Treatment 12367

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: Bava Ave
City Lake City Phone 752 1703

Site Location: Subdivision Country Lakes / Woodborough
Lot # 18 Block# _____ Permit # 24993 Pns II
Address _____

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	3354	1101	8

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

3/20/07 Date | 1200 Time | FJ54 Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 22-3S-16-02267-118 **Building permit No.** 000024993

Use Classification SFD,UTILITY **Fire:** 11.16

Permit Holder BLAKE CONSTRUCTION CO **Waste:** 33.50

Owner of Building DARBY ROGERS CO **Total:** 44.66

Location: 669 NW COUNTRY LAKE DRIVE, LAKE CITY, FL

Date: 08/09/2007

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L207624		
Builder:	BLAKE CONSTRUCTION	Date:	8/29/2006
Lot:	LOT 18 COUNTRY LAKE	Start Number:	**1071**
Subdivision:	N/A	SEI Ref:	L207624
County or City:	COLUMBIA COUNTY		
Truss Page Count:	97		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

LUNDE, BLAKE N II RR 0067618
Address: 2250 SW JAGUAR DR
LAKE CITY, FL. 32025 Designer: **88**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1B	0829061071	8/29/2006	41	PB12D	0829061112	8/29/2006
2	CJ1D	0829061072	8/29/2006	42	PB13	0829061113	8/29/2006
3	CJ1E	0829061073	8/29/2006	43	PB14	0829061114	8/29/2006
4	CJ1F	0829061074	8/29/2006	44	PB14A	0829061115	8/29/2006
5	CJ2A	0829061075	8/29/2006	45	PB14B	0829061116	8/29/2006
6	CJ2B	0829061076	8/29/2006	46	PB14C	0829061117	8/29/2006
7	CJ2C	0829061077	8/29/2006	47	PB18	0829061118	8/29/2006
8	CJ2D	0829061078	8/29/2006	48	PB18A	0829061119	8/29/2006
9	CJ2E	0829061079	8/29/2006	49	PB18B	0829061120	8/29/2006
10	CJ2F	0829061080	8/29/2006	50	PB18C	0829061121	8/29/2006
11	CJ3A	0829061081	8/29/2006	51	PB19	0829061122	8/29/2006
12	CJ3B	0829061082	8/29/2006	52	PB19A	0829061123	8/29/2006
13	CJ3C	0829061083	8/29/2006	53	PB23	0829061124	8/29/2006
14	CJ3D	0829061084	8/29/2006	54	PB23A	0829061125	8/29/2006
15	CJ3E	0829061085	8/29/2006	55	PB23B	0829061126	8/29/2006
16	CJ4A	0829061086	8/29/2006	56	PB23C	0829061127	8/29/2006
17	CJ4B	0829061087	8/29/2006	57	PB33	0829061128	8/29/2006
18	CJ4C	0829061088	8/29/2006	58	T01	0829061129	8/29/2006
19	EJ4D	0829061089	8/29/2006	59	T02	0829061130	8/29/2006
20	EJ4DA	0829061090	8/29/2006	60	T02G	0829061131	8/29/2006
21	EJ5D	0829061091	8/29/2006	61	T03	0829061132	8/29/2006
22	EJ7E	0829061092	8/29/2006	62	T04	0829061133	8/29/2006
23	EJ9B	0829061093	8/29/2006	63	T05	0829061134	8/29/2006
24	EJ10B	0829061094	8/29/2006	64	T06	0829061135	8/29/2006
25	HJ8E	0829061095	8/29/2006	65	T07	0829061136	8/29/2006
26	HJ11C	0829061096	8/29/2006	66	T08	0829061137	8/29/2006
27	HJ14C	0829061097	8/29/2006	67	T09	0829061138	8/29/2006
28	PB05	0829061099	8/29/2006	68	T10	0829061139	8/29/2006
29	PB05A	0829061100	8/29/2006	69	T11	0829061140	8/29/2006
30	PB05B	0829061101	8/29/2006	70	T12	0829061141	8/29/2006
31	PB05C	0829061102	8/29/2006	71	T13	0829061142	8/29/2006
32	PB09	0829061103	8/29/2006	72	T14	0829061143	8/29/2006
33	PB10A	0829061104	8/29/2006	73	T15	0829061144	8/29/2006
34	PB10B	0829061105	8/29/2006	74	T16	0829061145	8/29/2006
35	PB10C	0829061106	8/29/2006	75	T26G	0829061146	8/29/2006
36	PB10D	0829061107	8/29/2006	76	T17	0829061147	8/29/2006
37	PB12	0829061108	8/29/2006	77	T18	0829061148	8/29/2006
38	PB12A	0829061109	8/29/2006	78	T19	0829061149	8/29/2006
39	PB12B	0829061110	8/29/2006	79	T20	0829061150	8/29/2006
40	PB12C	0829061111	8/29/2006	80	T21	0829061151	8/29/2006

AUG 29 2006

Project Information for:	L207624		
Builder:	BLAKE CONSTRUCTION	Date:	8/29/2006
Lot:	LOT 18 COUNTRY LAKE	Start Number:	1071
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	97		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		**Wind**		**Building Code:**	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)
LUNDE, BLAKE N II RR 0067618
Address: 2250 SW JAGUAR DR
LAKE CITY, FL. 32025

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
81	T22	0829061152	8/29/2006				
82	T23	0829061153	8/29/2006				
83	T24	0829061154	8/29/2006				
84	T25	0829061155	8/29/2006				
85	T26	0829061156	8/29/2006				
86	T27	0829061157	8/29/2006				
87	T28	0829061158	8/29/2006				
88	T29	0829061159	8/29/2006				
89	T30	0829061160	8/29/2006				
90	T31	0829061161	8/29/2006				
91	T32	0829061162	8/29/2006				
92	T33	0829061163	8/29/2006				
93	T34	0829061164	8/29/2006				
94	T35	0829061165	8/29/2006				
95	T36	0829061166	8/29/2006				
96	T37	0829061167	8/29/2006				
97	T38	0829061168	8/29/2006				

AUG 29 2006

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- View Continuing Ed

- Term Glossary
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Licensee Details

Licensee Information

Name:	**LUNDE, BLAKE N II** (Primary Name) **BLAKE CONSTRUCTION** (DBA Name)
Main Address:	**872 SW JAGUAR DR** **LAKE CITY Florida 32025**
License Mailing:	
LicenseLocation:	**2250 SW JAGUAR DR** **LAKE CITY FL 32025**

License Information

License Type:	**Registered Residential Contractor**
Rank:	**Reg Residential**
License Number:	**RR0067618**
Status:	**Current,Active**
Licensure Date:	**03/13/2001**
Expires:	**08/31/2005**

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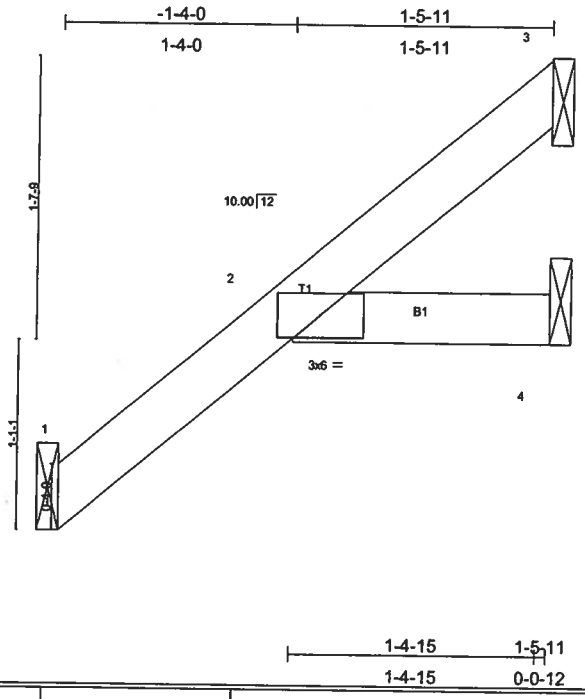
AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061071

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ1B	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 23 14:43:45 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.02	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.02	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 8 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-5-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=98/Mechanical, 4=22/Mechanical, 3=88/Mechanical
Max Horz 1=105(load case 5)
Max Uplift3=-91(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-90/5, 2-3=-75/53
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.01

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 3.

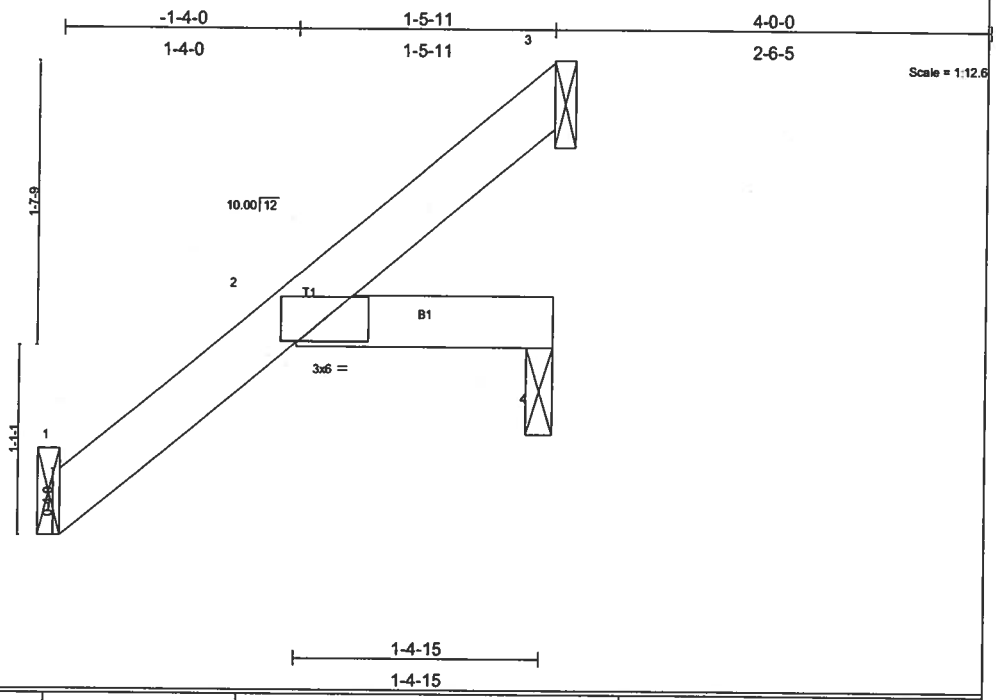
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061072

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ1D	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 23 14:57:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.02	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	0.02	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 8 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-5-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=98/Mechanical, 4=22/0-1-13, 3=88/Mechanical
Max Horz 1=105(load case 5)
Max Uplift1=-8(load case 5), 4=-17(load case 3), 3=-100(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-77/11, 2-3=-90/53
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.02

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 1, 17 lb uplift at joint 4 and 100 lb uplift at joint 3.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061073

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ1E	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 23 14:58:14 2006 Page 1

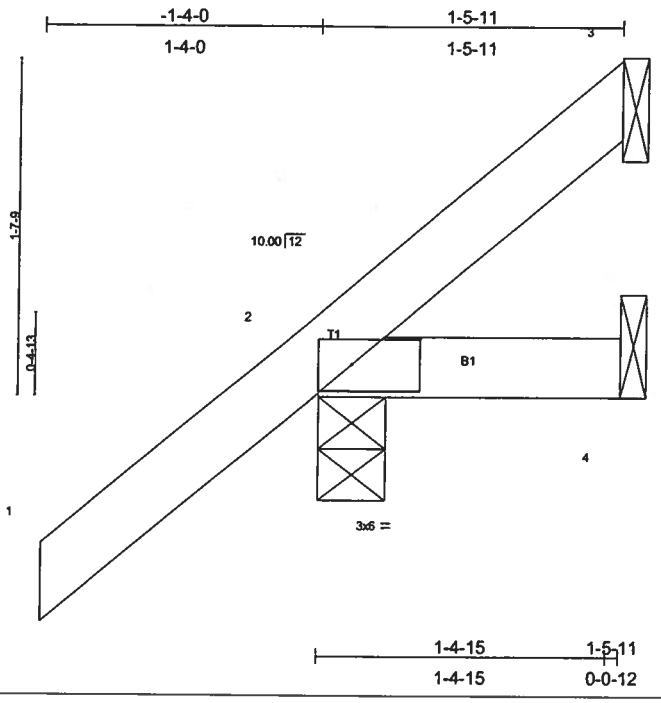


Plate Offsets (X,Y): [2:0-4-1,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 8 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-5-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=169/0-4-0, 4=21/Mechanical, 3=3/Mechanical
Max Horz 2=127(load case 5)
Max Uplift2=-134(load case 5), 3=-15(load case 6)
Max Grav 2=169(load case 1), 4=21(load case 1), 3=24(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-49/15
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 2 and 15 lb uplift at joint 3.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061074

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ1F	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 23 14:58:43 2006 Page 1

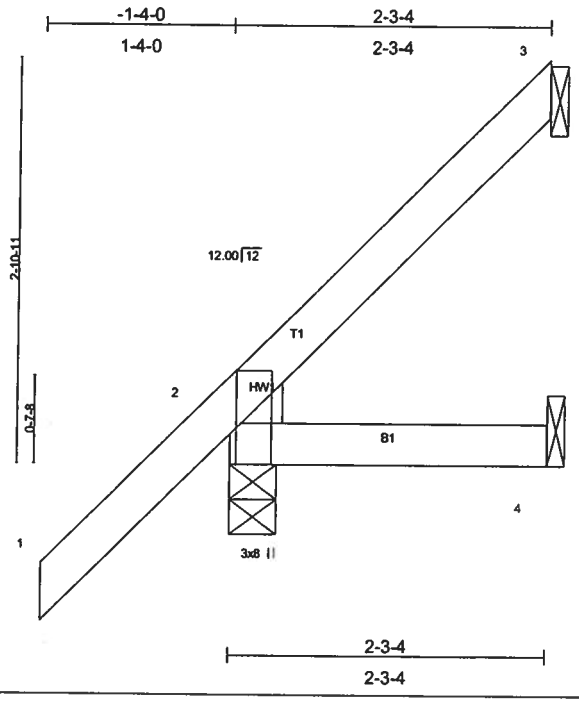


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=25/Mechanical, 2=196/0-4-0, 4=31/Mechanical
Max Horz 2=183(load case 5)
Max Uplift3=-59(load case 5), 2=-96(load case 5)
Max Grav 3=29(load case 3), 2=196(load case 1), 4=31(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-66/21
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.09 and 2 = 0.00

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 3 and 96 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061075

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ2A	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 23 15:06:25 2006 Page 1

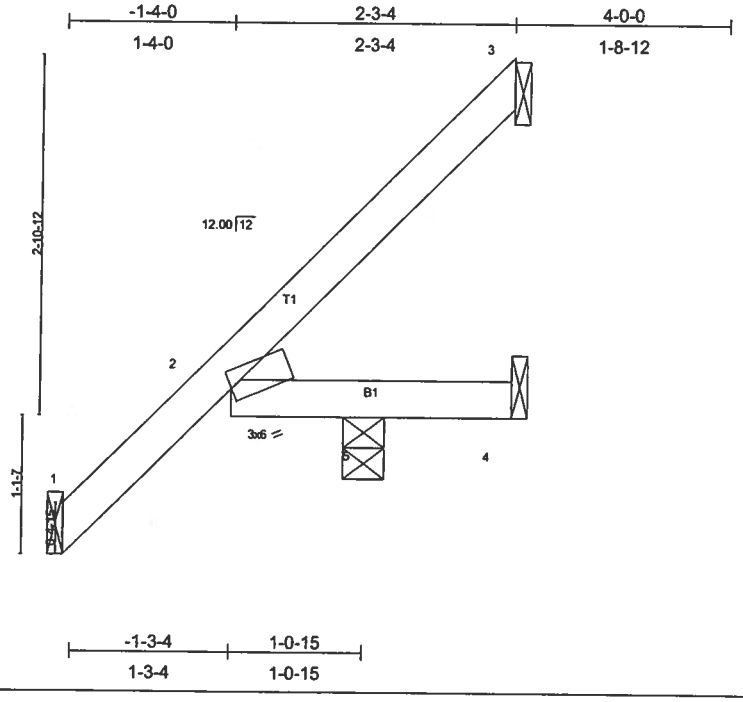


Plate Offsets (X,Y): [2:0-0-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	0.01	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.02	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.02	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=60/Mechanical, 3=70/Mechanical, 4=-69/Mechanical, 5=209/0-4-0
Max Horz 5=160(load case 5)
Max Uplift1=-11(load case 5), 3=-91(load case 5), 4=-69(load case 1), 5=-26(load case 5)
Max Grav 1=60(load case 1), 3=70(load case 1), 4=19(load case 5), 5=209(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-32/21, 2-3=-90/43
BOT CHORD 2-5=-14/177, 4-5=0/0

JOINT STRESS INDEX
2 = 0.18

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1, 91 lb uplift at joint 3, 69 lb uplift at joint 4 and 26 lb uplift at joint 5.

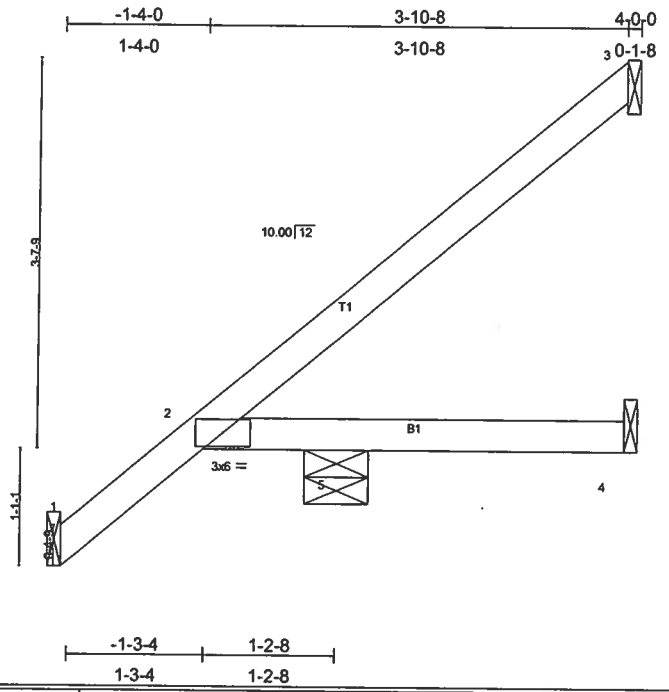
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061076

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ2B	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 23 15:07:23 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	0.03	2	>898	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.05	2	>584	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.04	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=64/Mechanical, 3=110/Mechanical, 4=-23/Mechanical, 5=254/0-7-3
Max Horz 5=195(load case 5)
Max Uplift3=-123(load case 5), 4=-23(load case 1), 5=-71(load case 5)
Max Grav 1=64(load case 1), 3=110(load case 1), 4=34(load case 5), 5=254(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-34/13, 2-3=-110/58
BOT CHORD 2-5=-17/216, 4-5=-0/0

JOINT STRESS INDEX
2 = 0.08

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 123 lb uplift at joint 3, 23 lb uplift at joint 4 and 71 lb uplift at joint 5.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061077

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ2C	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 23 15:27:07 2006 Page 1

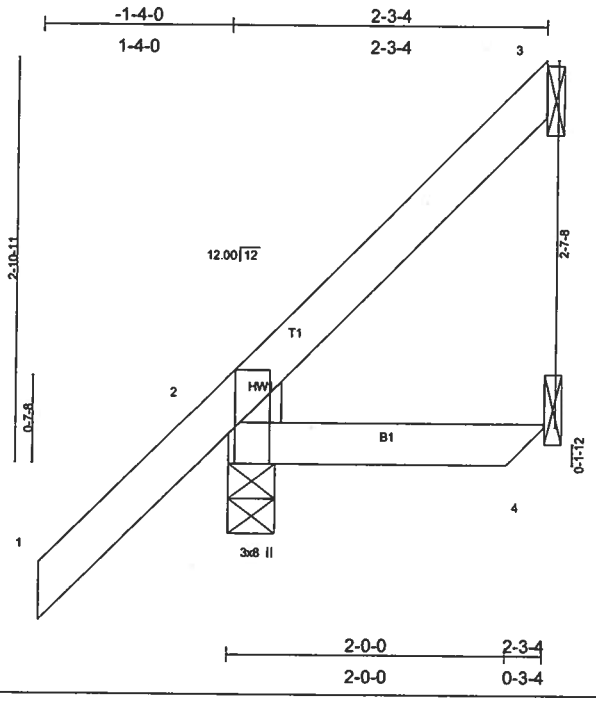


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-3-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=25/Mechanical, 2=196/0-4-0, 4=31/Mechanical
Max Horz 2=183(load case 5)
Max Uplift3=-59(load case 5), 2=-96(load case 5)
Max Grav 3=29(load case 3), 2=196(load case 1), 4=31(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-66/21
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.09 and 2 = 0.00

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 3 and 96 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061078

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ2D	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Aug 23 15:27:53 2006 Page 1

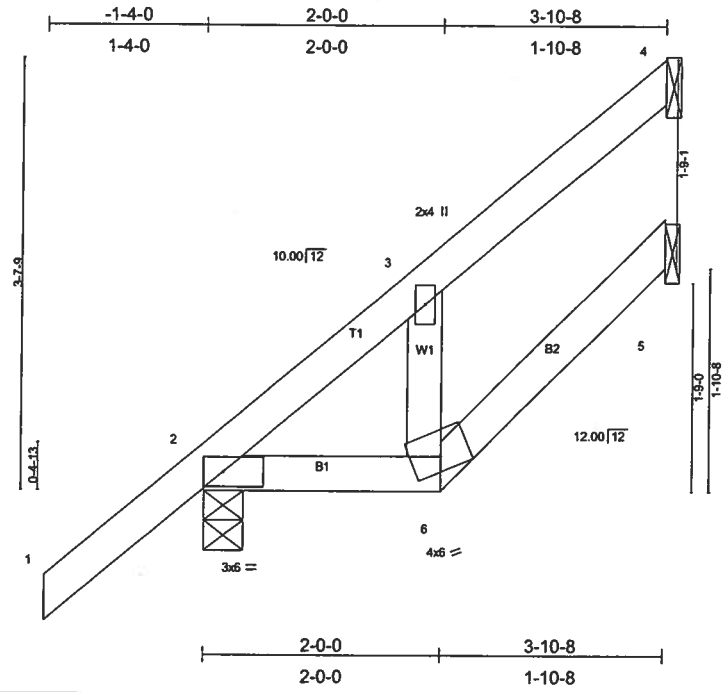


Plate Offsets (X,Y): [2:0-4-1,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	0.03	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.04	6	>990	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=109/Mechanical, 2=251/0-4-0, 5=27/Mechanical
Max Horz 2=217(load case 5)
Max Uplift4=-103(load case 5), 2=-103(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-98/8, 3-4=-83/63
BOT CHORD 2-6=-5/0, 5-6=-16/16
WEBS 3-6=0/55

JOINT STRESS INDEX
2 = 0.18, 3 = 0.04 and 6 = 0.02

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 4 and 103 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061079

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ2E	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:24:38 2006 Page 1

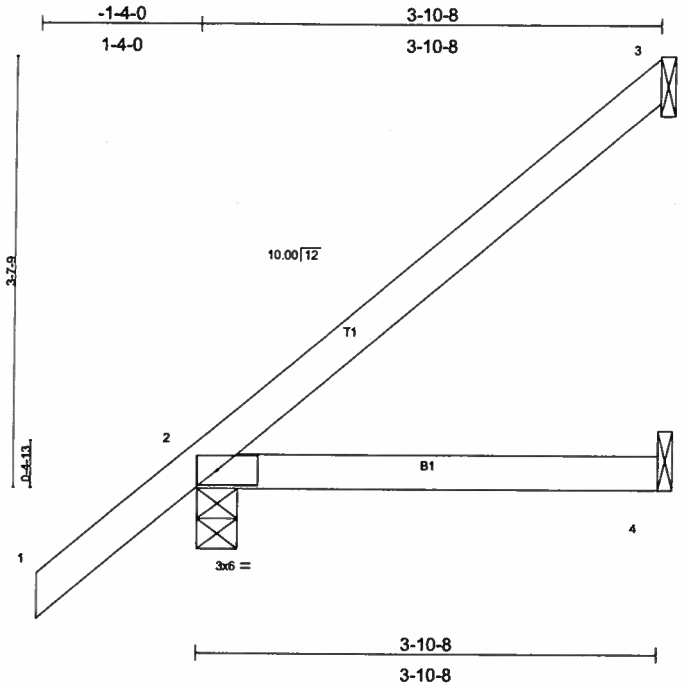


Plate Offsets (X,Y): [2:0-4-1,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=82/Mechanical, 2=251/0-4-0, 4=55/Mechanical
Max Horz 2=217(load case 5)
Max Uplift3=-108(load case 5), 2=-103(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-100/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.18

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 3 and 103 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061080

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ2F	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:25:08 2006 Page 1

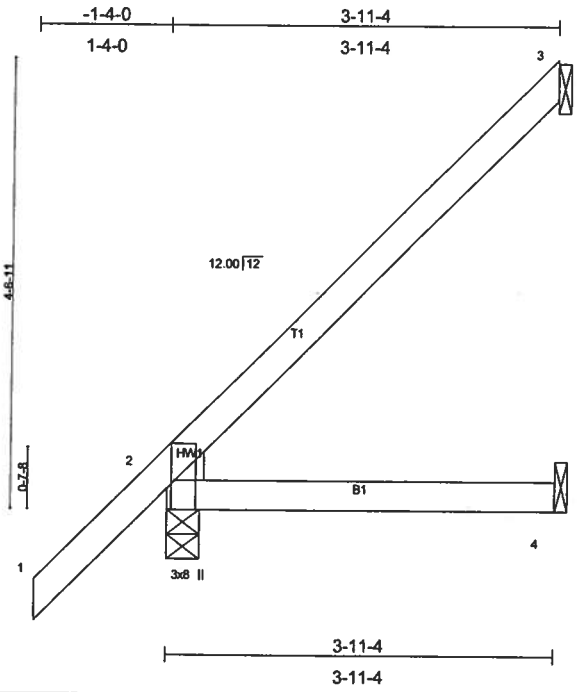


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=84/Mechanical, 2=253/0-4-0, 4=56/Mechanical
Max Horz 2=260(load case 5)
Max Uplift3=-150(load case 5), 2=-63(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-147/47
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13 and 2 = 0.00

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 3 and 63 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061081

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ3A	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:25:49 2006 Page 1

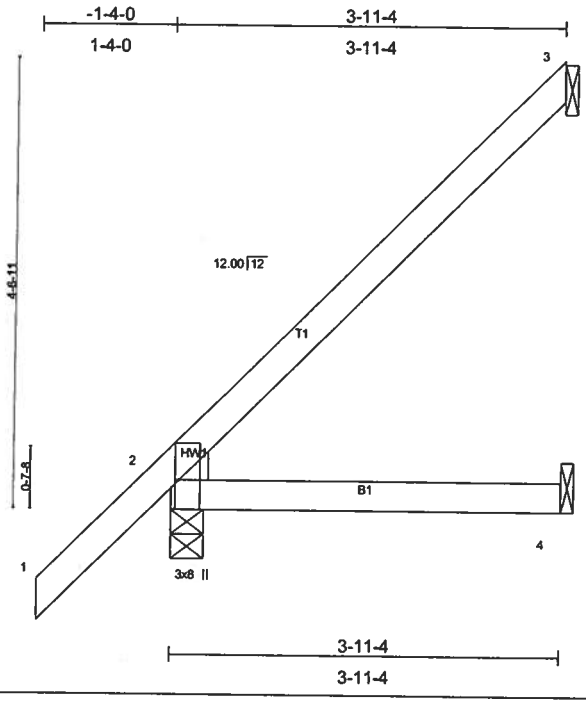


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=84/Mechanical, 2=253/0-4-0, 4=56/Mechanical
Max Horz 2=260(load case 5)
Max Uplift3=-150(load case 5), 2=-63(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-147/47
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13 and 2 = 0.00

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 3 and 63 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061082

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ3B	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:26:22 2006 Page 1

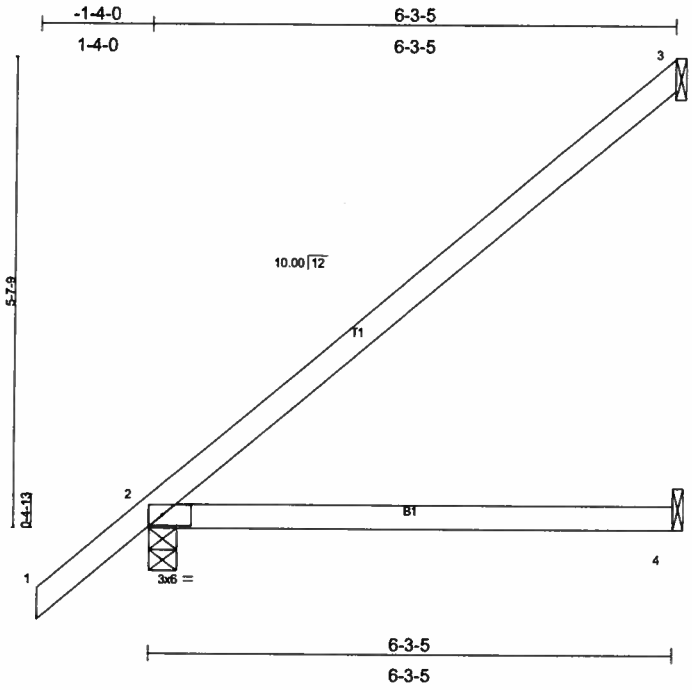


Plate Offsets (X,Y): [2:0-4-1,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.07	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.12	2-4	>591	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=153/Mechanical, 2=345/0-4-0, 4=91/Mechanical
Max Horz 2=309(load case 5)
Max Uplift3=-203(load case 5), 2=-92(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-178/79
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.23

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 203 lb uplift at joint 3 and 92 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061083

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ3C	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:27:10 2006 Page 1

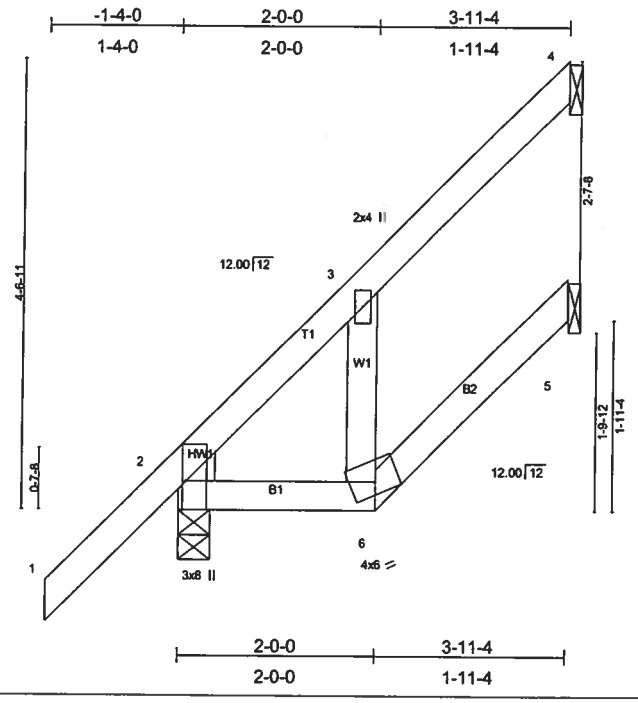


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.06	6	>690	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.05	6	>836	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.06	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=111/Mechanical, 2=253/0-4-0, 5=28/Mechanical
Max Horz 2=260(load case 5)
Max Uplift4=-144(load case 5), 2=-63(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-141/11, 3-4=-124/71
BOT CHORD 2-6=-4/0, 5-6=-16/16
WEBS 3-6=0/56

JOINT STRESS INDEX
2 = 0.13, 2 = 0.00, 3 = 0.03 and 6 = 0.02

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 4 and 63 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061084

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ3D	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:27:47 2006 Page 1

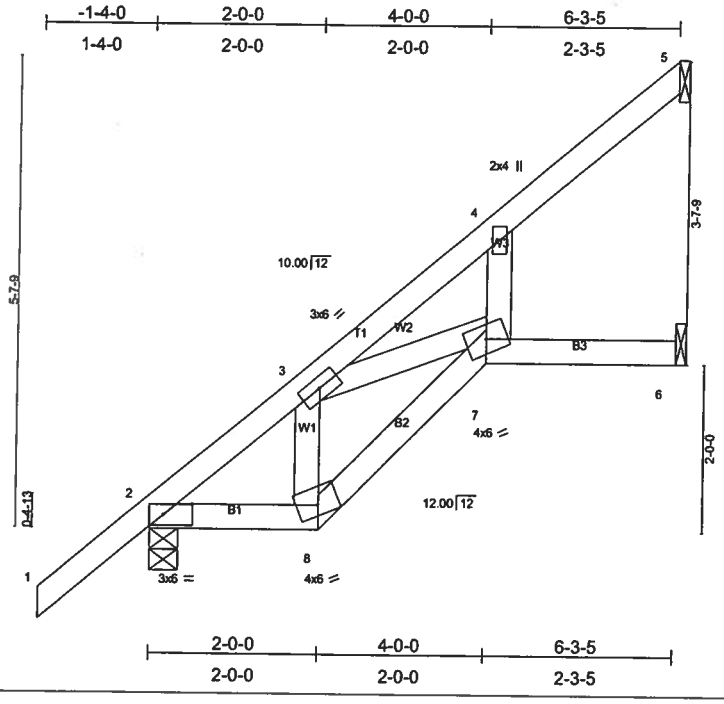


Plate Offsets (X,Y): [2:0-4-1,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.18	7	>413	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.19	7	>382	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=211/Mechanical, 2=345/0-4-0, 6=33/Mechanical
Max Horz 2=309(load case 5)
Max Uplift5=-191(load case 5), 2=-92(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-330/0, 3-4=-186/0, 4-5=-145/128
BOT CHORD 2-8=-192/227, 7-8=-251/317, 6-7=-0/0
WEBS 3-8=-152/184, 3-7=-193/166, 4-7=-131/240

JOINT STRESS INDEX
2 = 0.26, 3 = 0.11, 4 = 0.15, 7 = 0.12 and 8 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 5 and 92 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061085

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ3E	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:28:12 2006 Page 1

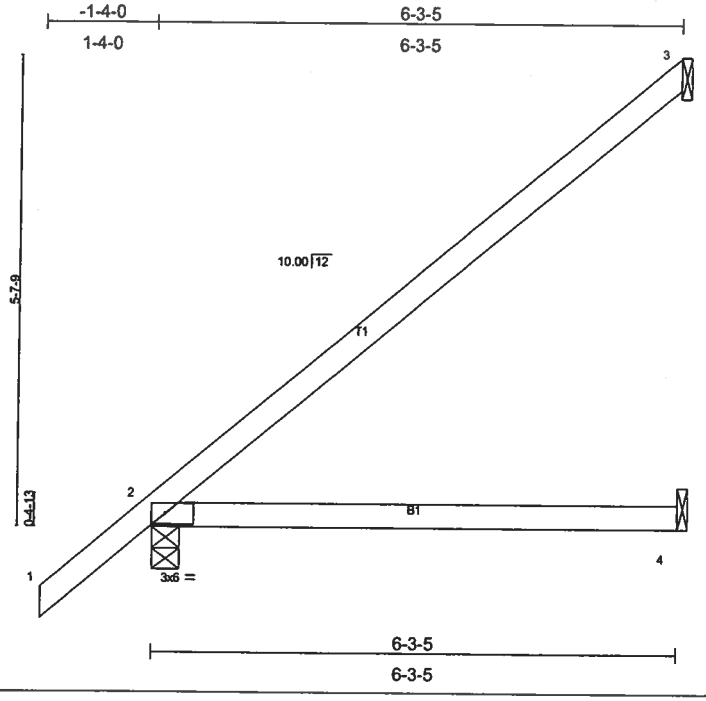


Plate Offsets (X,Y): [2:0-4-1,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.07	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.12	2-4	>591	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=153/Mechanical, 2=345/0-4-0, 4=91/Mechanical
Max Horz 2=309(load case 5)
Max Uplift3=-203(load case 5), 2=-92(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-178/79
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.23

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 203 lb uplift at joint 3 and 92 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061086

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ4A	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:28:49 2006 Page 1

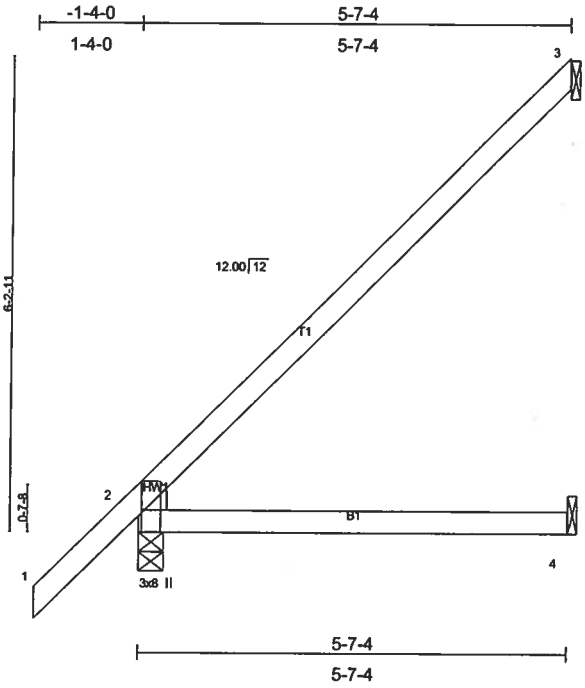


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.04	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.08	2-4	>842	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=134/Mechanical, 2=318/0-4-0, 4=81/Mechanical
Max Horz 2=337(load case 5)
Max Uplift3=-228(load case 5), 2=-43(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-217/77
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.16 and 2 = 0.00

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 3 and 43 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061087

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ4B	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:29:40 2006 Page 1

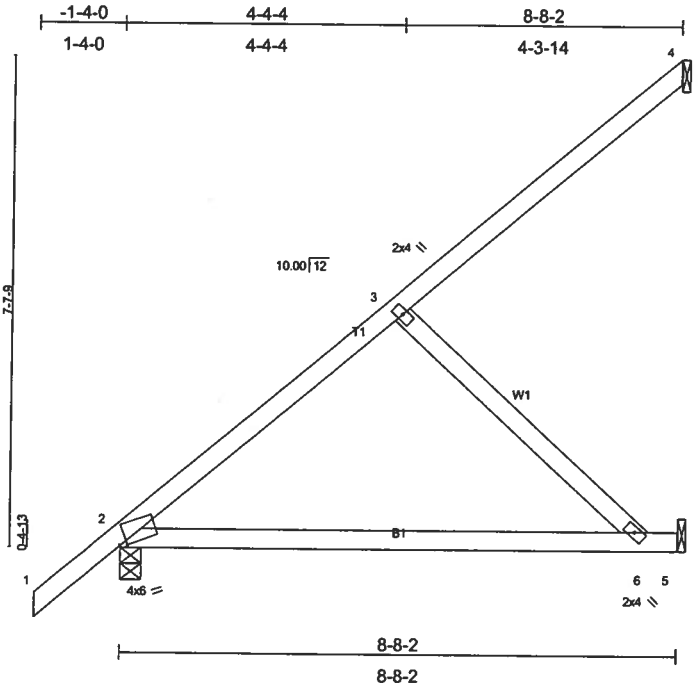


Plate Offsets (X,Y): [2:0-1-6,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.21	2-6	>492	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.35	2-6	>286	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.12	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 40 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=88/Mechanical, 2=443/0-4-0, 5=259/Mechanical
Max Horz 2=402(load case 5)
Max Uplift4=-122(load case 5), 2=-87(load case 5), 5=-143(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-254/0, 3-4=-105/41
BOT CHORD 2-6=-201/177, 5-6=0/0
WEBS 3-6=-240/273

JOINT STRESS INDEX
2 = 0.91, 3 = 0.15 and 6 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 4, 87 lb uplift at joint 2 and 143 lb uplift at joint 5.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061088

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	CJ4C	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:30:27 2006 Page 1

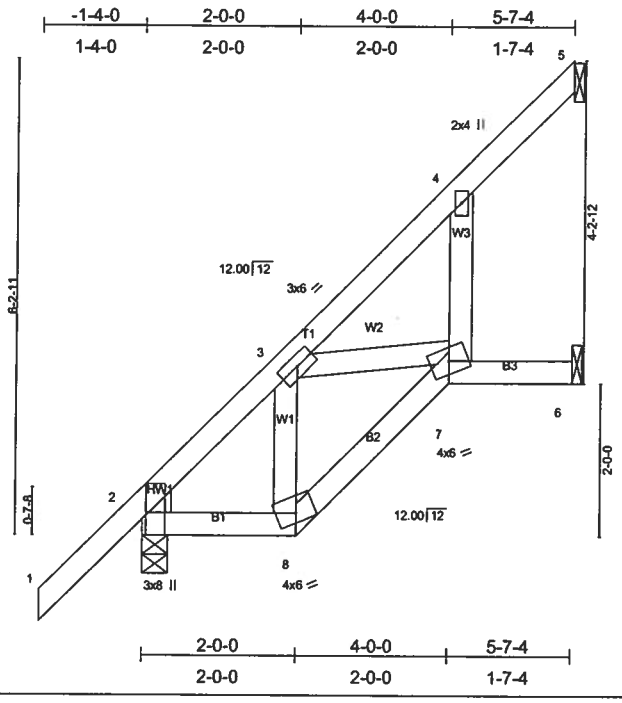


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.11	7	>608	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.09	7	>717	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.06	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=191/Mechanical, 2=318/0-4-0, 6=23/Mechanical
Max Horz 2=337(load case 5)
Max Uplift5=-216(load case 5), 2=-43(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-250/0, 3-4=-147/0, 4-5=-177/130
BOT CHORD 2-8=-175/136, 7-8=-234/202, 6-7=-0/0
WEBS 3-8=-71/172, 3-7=-119/153, 4-7=-159/189

JOINT STRESS INDEX
2 = 0.14, 2 = 0.00, 3 = 0.11, 4 = 0.12, 7 = 0.08 and 8 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 5 and 43 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061089

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	EJ4D	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Sep 07 16:59:57 2006 Page 1

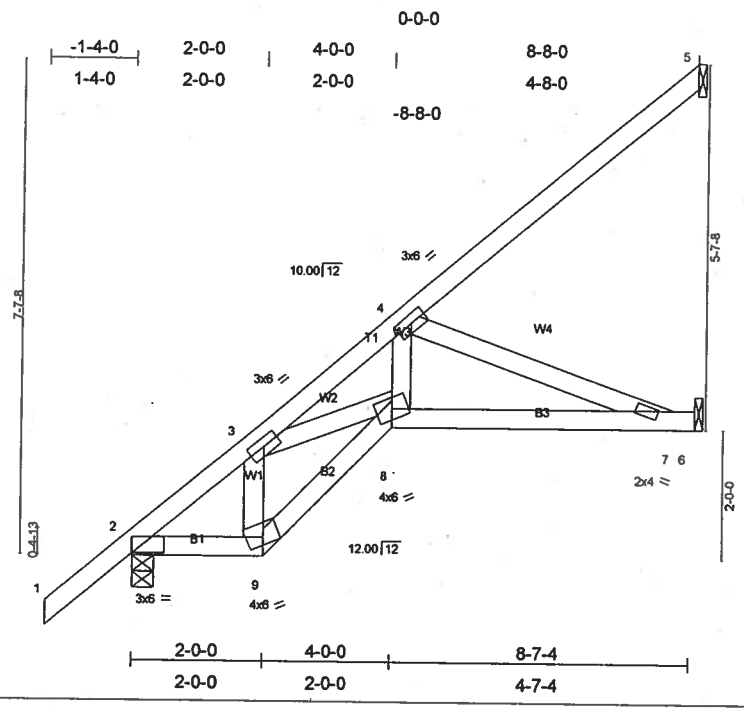


Plate Offsets (X,Y): [2:0-4-1,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.03	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.06	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 48 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-2-13 oc bracing.

REACTIONS (lb/size) 5=101/Mechanical, 2=443/0-4-0, 6=246/Mechanical
Max Horz 2=401(load case 5)
Max Uplift5=-128(load case 5), 2=-87(load case 5), 6=-137(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-406/0, 3-4=-789/305, 4-5=-110/49
BOT CHORD 2-9=-210/258, 8-9=-270/376, 7-8=-462/559, 6-7=0/0
WEBS 3-9=-224/219, 3-8=-300/373, 4-8=-235/462, 4-7=-593/491

JOINT STRESS INDEX
2 = 0.55, 3 = 0.17, 4 = 0.36, 7 = 0.31, 8 = 0.59 and 9 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 5, 87 lb uplift at joint 2 and 137 lb uplift at joint 6.

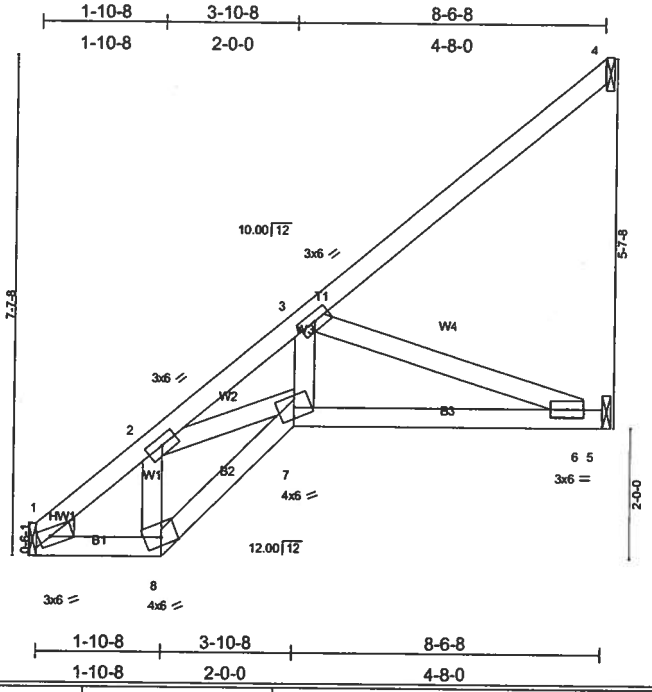
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061090

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	EJ4DA	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 13:41:49 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.03	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.06	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
8-10-14 oc bracing: 6-7.

REACTIONS (lb/size) 4=101/Mechanical, 5=252/Mechanical, 1=353/Mechanical
Max Horz 1=334(load case 5)
Max Uplift4=-129(load case 5), 5=-148(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-440/0, 2-3=-816/361, 3-4=-110/50
BOT CHORD 1-8=-276/290, 7-8=-355/418, 6-7=-497/575, 5-6=0/0
WEBS 2-8=-229/234, 2-7=-276/360, 3-7=-285/486, 3-6=-611/528

JOINT STRESS INDEX
1 = 0.33, 1 = 0.00, 2 = 0.16, 3 = 0.38, 6 = 0.17, 7 = 0.57 and 8 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 4 and 148 lb uplift at joint 5.

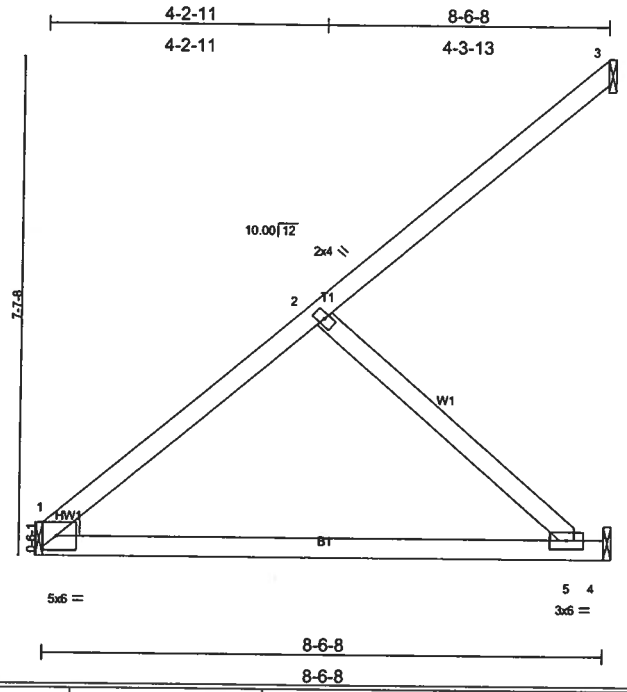
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061091

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	EJ5D	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 13:42:28 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.21	1-5	>474	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.36	1-5	>278	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=86/Mechanical, 4=268/Mechanical, 1=353/Mechanical
Max Horz 1=334(load case 5)
Max Uplift3=-119(load case 5), 4=-158(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-258/0, 2-3=-103/40
BOT CHORD 1-5=-216/186, 4-5=0/0
WEBS 2-5=-250/291

JOINT STRESS INDEX
1 = 0.66, 1 = 0.00, 2 = 0.16 and 5 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 3 and 158 lb uplift at joint 4.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061092

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	EJ7E	MONO TRUSS	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:35:35 2006 Page 1

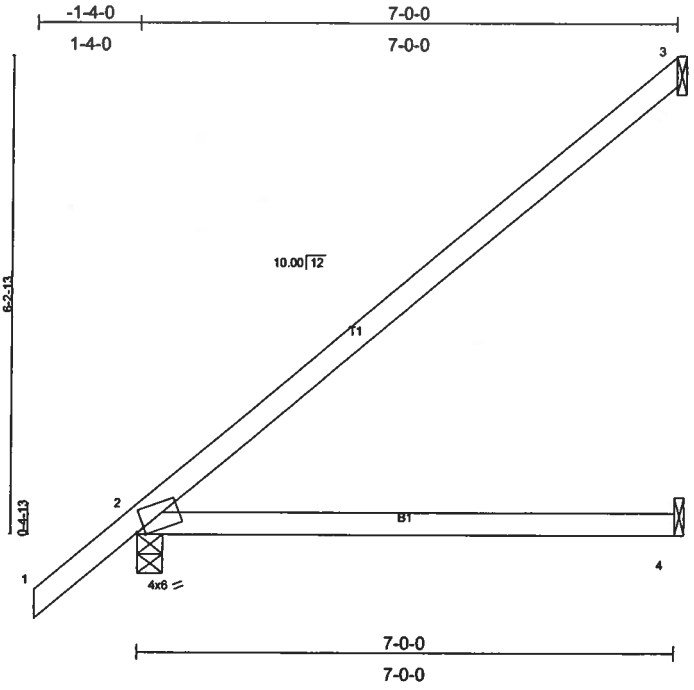


Plate Offsets (X,Y): [2:0-1-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.14	2-4	>570	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.24	2-4	>345	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=164/Mechanical, 2=374/0-4-0, 4=112/Mechanical
Max Horz 2=337(load case 5)
Max Uplift3=-201(load case 5), 2=-90(load case 5), 4=-8(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-179/83
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.73

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 201 lb uplift at joint 3, 90 lb uplift at joint 2 and 8 lb uplift at joint 4.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061093

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	EJ9B	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:35:23 2006 Page 1

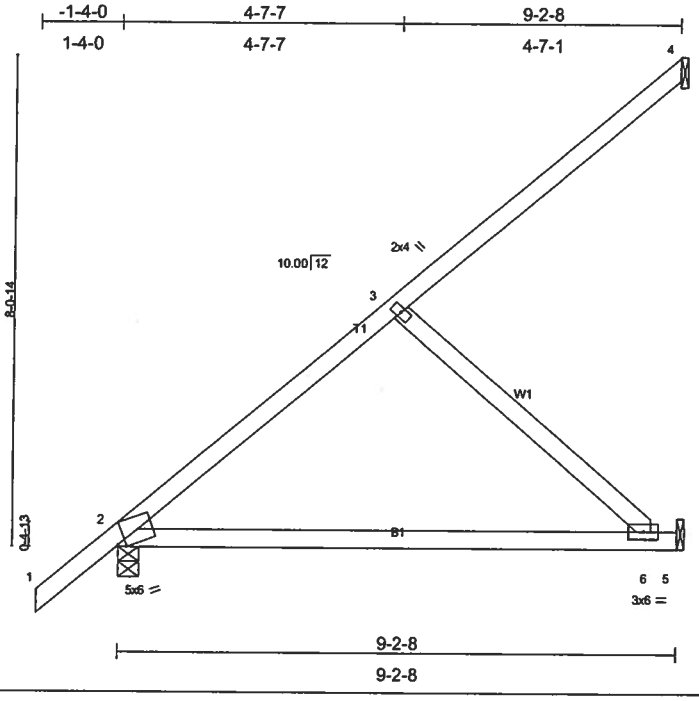


Plate Offsets (X,Y): [2:0-1-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.26	2-6	>421	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.44	2-6	>245	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=93/Mechanical, 2=465/0-4-0, 5=278/Mechanical
Max Horz 2=422(load case 5)
Max Uplift4=-129(load case 5), 2=-86(load case 5), 5=-154(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-274/0, 3-4=-111/43
BOT CHORD 2-6=-215/192, 5-6=0/0
WEBS 3-6=-258/289

JOINT STRESS INDEX
2 = 0.69, 3 = 0.16 and 6 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 4, 86 lb uplift at joint 2 and 154 lb uplift at joint 5.

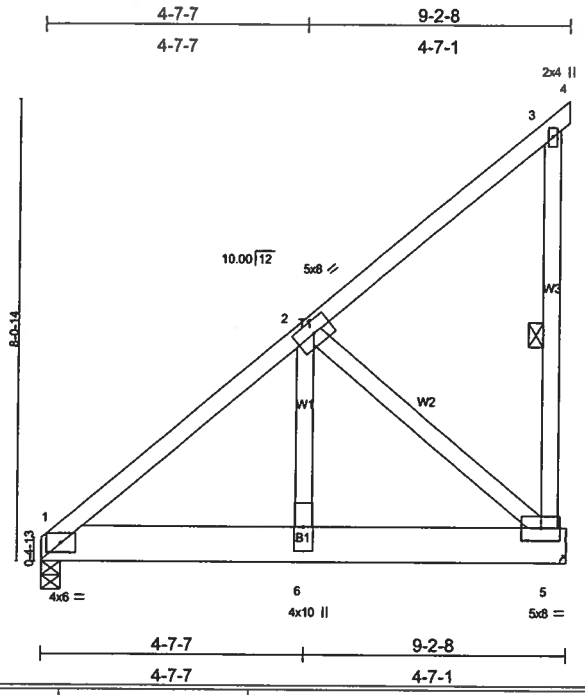
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061094

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	EJ10B	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:31:47 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.03	1-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.05	1-6	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.93	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 70 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-5

REACTIONS (lb/size) 1=1899/0-4-0, 5=1903/Mechanical
Max Horz 1=348(load case 4)
Max Uplift1=-588(load case 4), 5=-898(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1746/489, 2-3=-105/48, 3-4=-3/0, 3-5=-97/139
BOT CHORD 1-6=-601/1283, 5-6=-601/1283
WEBS 2-6=-677/1916, 2-5=-1684/783

JOINT STRESS INDEX
1 = 0.58, 2 = 0.68, 3 = 0.59, 5 = 0.28 and 6 = 0.47

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 588 lb uplift at joint 1 and 898 lb uplift at joint 5.
5) Girder carries tie-in span(s): 19-0-0 from 0-0-0 to 9-2-8
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-14, 1-5=-380(F=-350)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061095

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	HJ8E	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:42:45 2006 Page 1

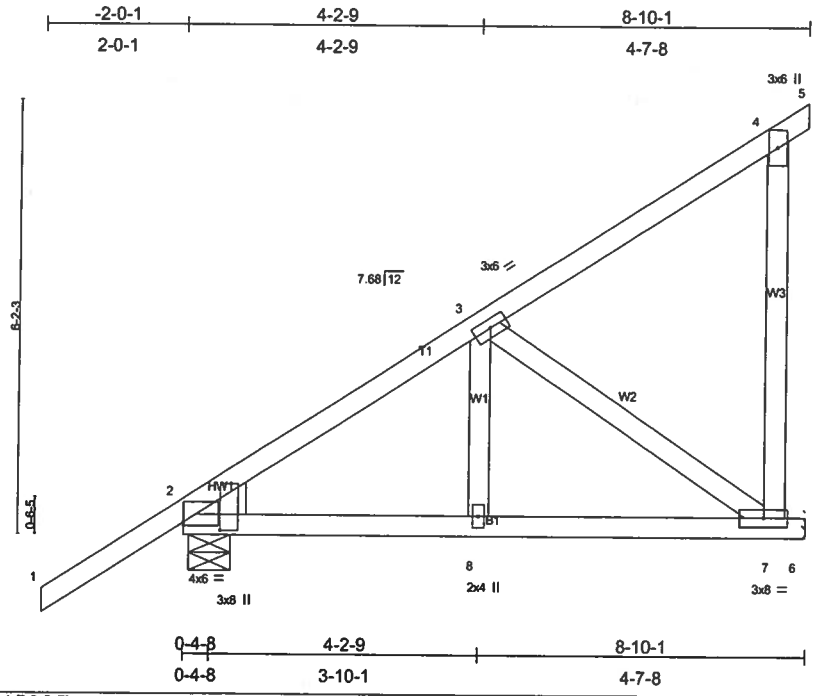


Plate Offsets (X,Y): [2:0-0-1,0-0-9], [2:0-1-7,0-6-5]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.04	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.07	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.16	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 53 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=446/0-7-2, 6=583/Mechanical
Max Horz 2=430(load case 4)
Max Uplift2=-81(load case 4), 6=-443(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-453/0, 3-4=-192/68, 4-5=-31/0, 4-7=-259/346
BOT CHORD 2-8=-237/342, 7-8=-237/342, 6-7=0/0
WEBS 3-8=0/172, 3-7=-358/241

JOINT STRESS INDEX
2 = 0.59, 2 = 0.27, 3 = 0.13, 4 = 0.43, 7 = 0.70 and 8 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 2 and 443 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-5(F=25, B=25)-to-4=-127(F=-36, B=-36), 4=-127(F=-36, B=-36)-to-5=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-6=-74(F=-22, B=-22)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061096

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	HJ11C	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 12:52:37 2006 Page 1

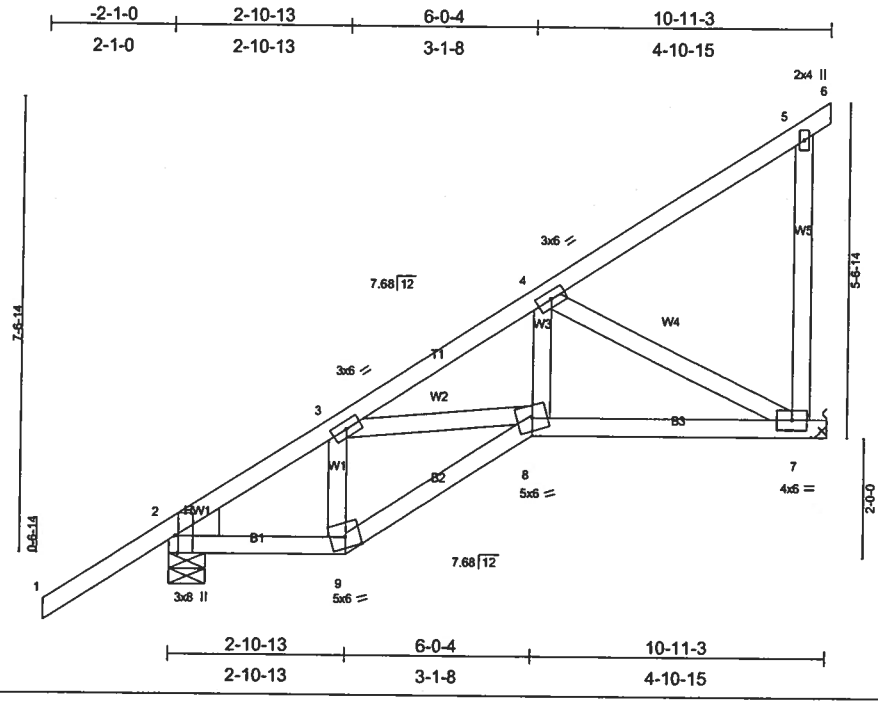


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.04	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.07	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-3 oc bracing.

REACTIONS (lb/size) 2=545/0-7-2, 7=778/Mechanical
Max Horz 2=627(load case 4)
Max Uplift2=-36(load case 4), 7=-730(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-535/0, 3-4=-942/171, 4-5=-283/71, 5-6=-38/0
BOT CHORD 2-9=-330/389, 8-9=-373/497, 7-8=-647/739
WEBS 3-9=-243/248, 3-8=-345/401, 4-8=-147/511, 4-7=-829/726, 5-7=-253/430

JOINT STRESS INDEX
2 = 0.53, 2 = 0.00, 3 = 0.19, 4 = 0.39, 5 = 0.22, 7 = 0.23, 8 = 0.67 and 9 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 2 and 730 lb uplift at joint 7.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-5(F=25, B=25)-to-4=-93(F=-20, B=-20), 4=-53(F=-20, B=-20)-to-5=-119(F=-52, B=-52), 5=-159(F=-52, B=-52)-to-6=-165(F=-56, B=-56), 2=0(F=15, B=15)-to-9=-23(F=3, B=3), 9=-23(F=3, B=3)-to-8=-51(F=-11, B=-11), 8=-51(F=-11, B=-11)-to-7=-91(F=-30, B=-30)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061097

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	HJ14C	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:19:02 2006 Page 1

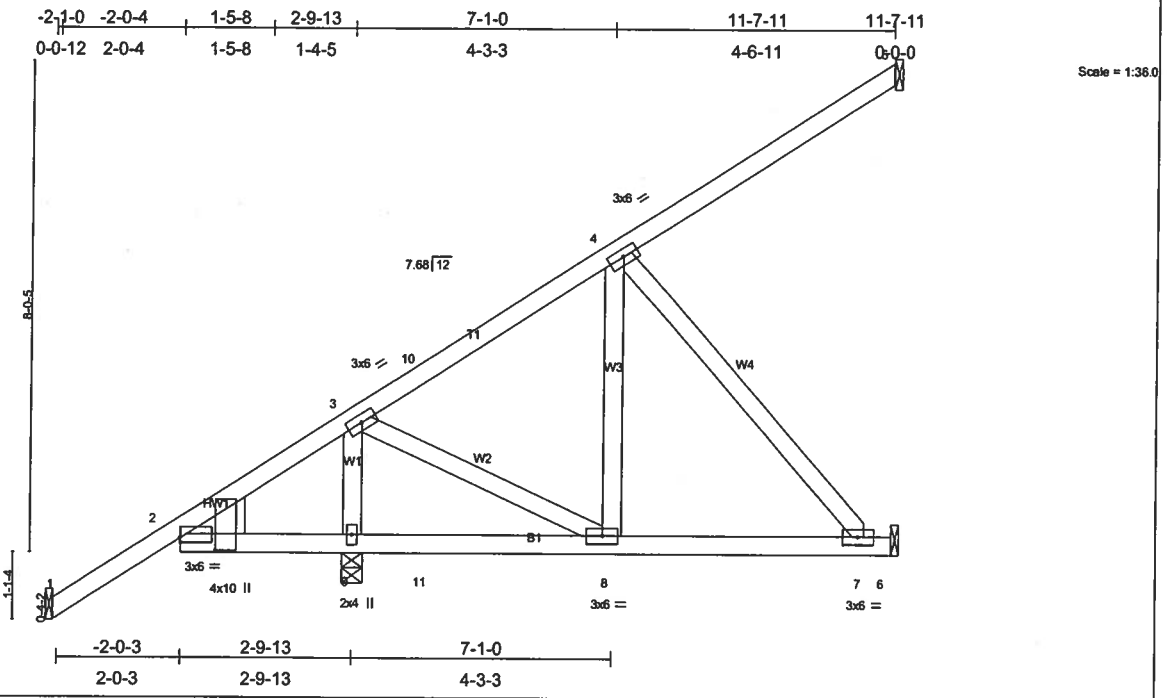


Plate Offsets (X,Y): [2:0-0-2,0-0-14], [2:0-2-8,0-6-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.06	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.09	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.31	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 70 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 8 SYP 2400F 2.0E

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=377/Mechanical, 5=252/Mechanical, 6=440/Mechanical, 9=1207/0-4-2
Max Horz 9=401(load case 4)
Max Uplift1=-146(load case 4), 5=-159(load case 4), 6=-250(load case 4), 9=-317(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-22/46, 2-3=-201/512, 3-10=-433/24, 4-10=-252/15, 4-5=-162/100
BOT CHORD 2-9=-334/246, 9-11=-334/0, 8-11=-334/0, 7-8=-218/286, 6-7=0/0
WEBS 3-9=-1067/336, 3-8=-70/688, 4-8=-18/13, 4-7=-438/335

JOINT STRESS INDEX
2 = 0.48, 2 = 0.04, 3 = 0.42, 4 = 0.20, 7 = 0.22, 8 = 0.39 and 9 = 0.39

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 1, 159 lb uplift at joint 5, 250 lb uplift at joint 6 and 317 lb uplift at joint 9.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 315 lb down and 119 lb up at -2-0-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

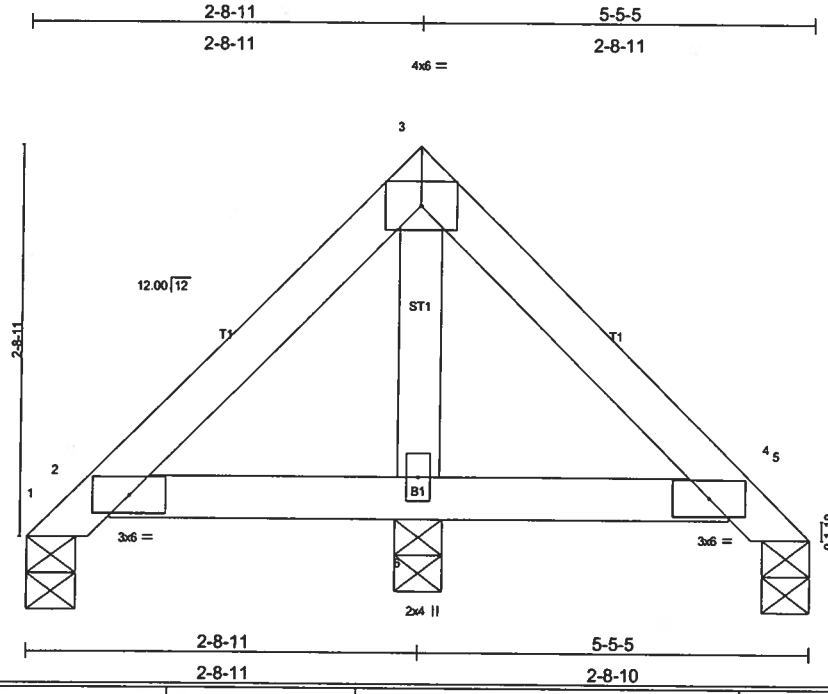
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Concentrated Loads (lb)
Vert: 1=-315(F)
Trapezoidal Loads (plf)
Vert: 1=-65-to-2=-95(F=-30), 2=-79(F=-25)-to-10=-133(F=-79), 10=-54-to-5=-152(F=-98), 2=-35(F=-5)-to-11=-45(F=-15), 11=-30-to-6=-84(F=-54)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061099

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB05	VALLEY	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:44:45 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.06	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.00	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 21 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=49/0-4-0, 5=49/0-4-0, 6=318/0-4-0
Max Horz 1=89(load case 4)
Max Uplift1=-20(load case 6), 5=-20(load case 6), 6=-119(load case 5)
Max Grav 1=60(load case 9), 5=60(load case 10), 6=318(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-83/85, 2-3=-53/98, 3-4=-25/98, 4-5=-36/16
BOT CHORD 2-6=-33/81, 4-6=-33/81
WEBS 3-6=-208/128

JOINT STRESS INDEX
2 = 0.16, 3 = 0.10, 4 = 0.16 and 6 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 1, 20 lb uplift at joint 5 and 119 lb uplift at joint 6.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061100

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB05A	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:45:36 2006 Page 1

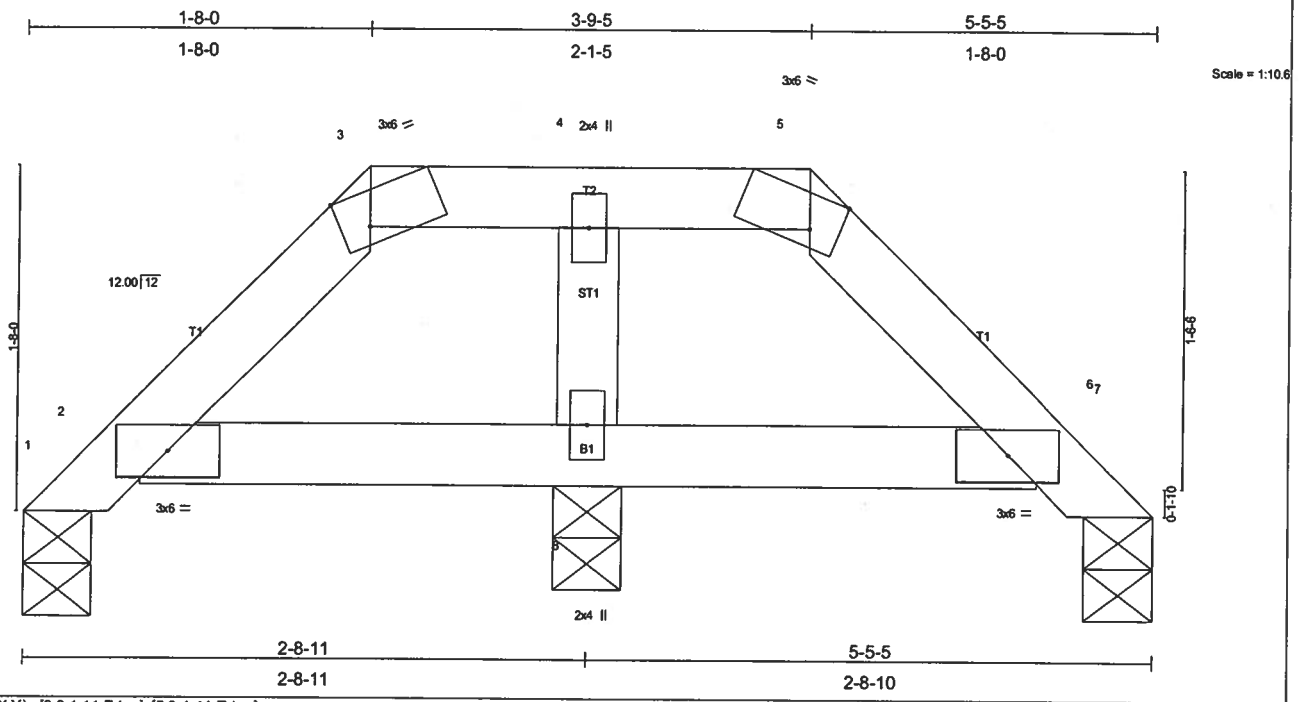


Plate Offsets (X,Y): [3:0-1-11,Edge], [5:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.06	Vert(LL)	-0.00	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.00	6-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=92/0-4-0, 7=92/0-4-0, 8=231/0-4-0
Max Horz 1=-54(load case 3)
Max Uplift1=-35(load case 5), 7=-42(load case 6), 8=-70(load case 4)
Max Grav 1=93(load case 9), 7=93(load case 10), 8=231(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-56/53, 2-3=-59/60, 3-4=-31/69, 4-5=-31/69, 5-6=-59/60, 6-7=-56/41
BOT CHORD 2-8=-28/31, 6-8=-28/31
WEBS 4-8=-118/99

JOINT STRESS INDEX
2 = 0.16, 3 = 0.08, 4 = 0.06, 5 = 0.08, 6 = 0.16 and 8 = 0.06

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1, 42 lb uplift at joint 7 and 70 lb uplift at joint 8.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061101

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB05B	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:46:46 2006 Page 1

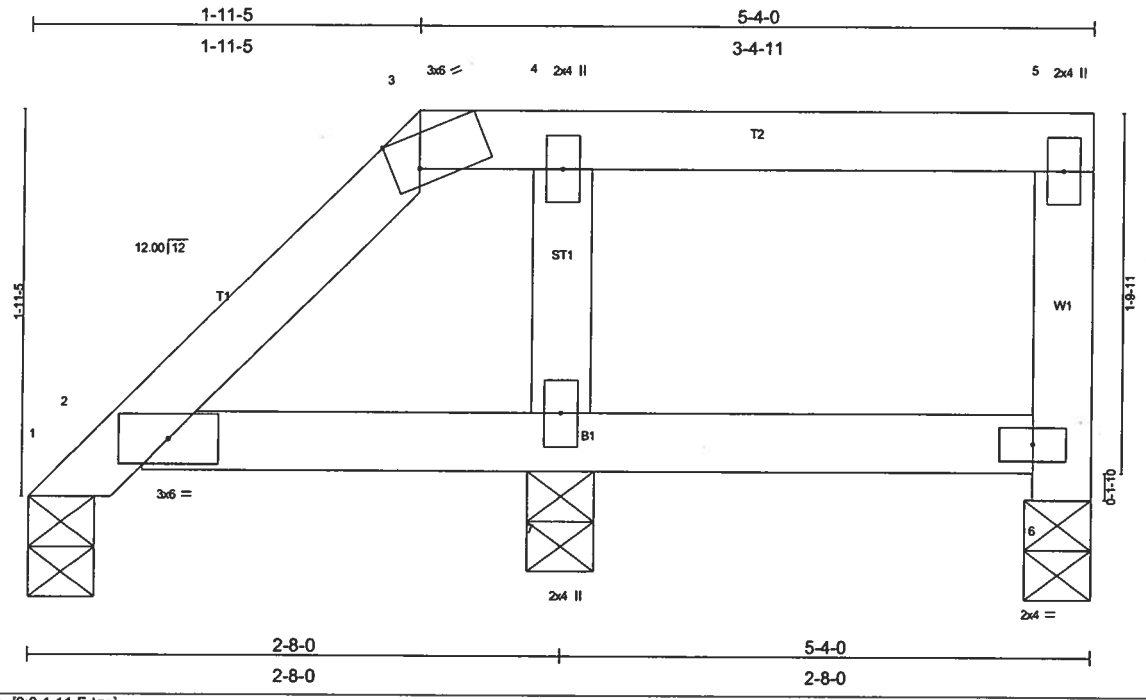


Plate Offsets (X,Y): [3:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.07	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=85/0-4-0, 6=93/0-4-0, 7=237/0-4-0
Max Horz 1=85(load case 5)
Max Uplift6=-54(load case 3), 7=-106(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-72/3, 2-3=-42/16, 3-4=-17/13, 4-5=-17/13, 5-6=-61/71
BOT CHORD 2-7=-13/17, 6-7=-13/17
WEBS 4-7=-140/185

JOINT STRESS INDEX
2 = 0.18, 3 = 0.06, 4 = 0.11, 5 = 0.20, 6 = 0.13 and 7 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 6 and 106 lb uplift at joint 7.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

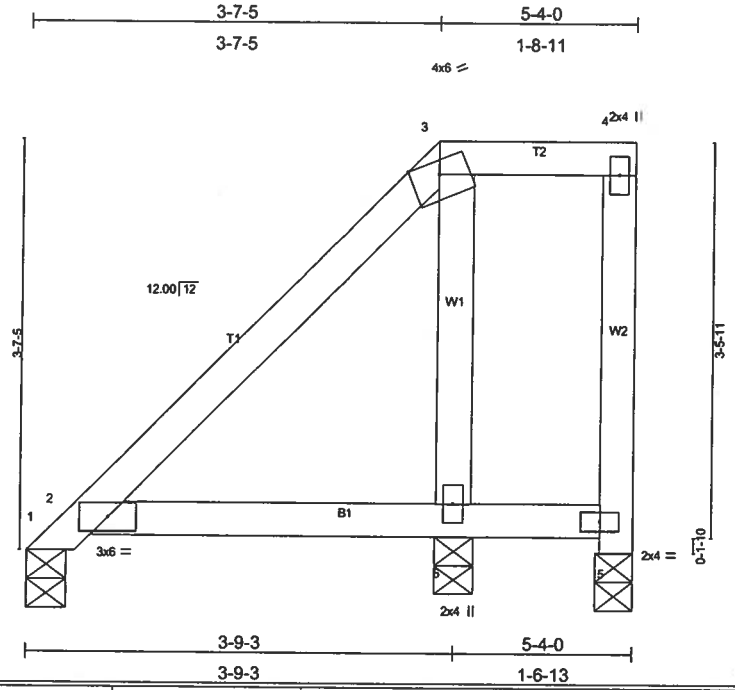
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061102

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB05C	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:48:13 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	0.01	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	0.01	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=114/0-4-0, 5=-16/0-4-0, 6=317/0-4-0
Max Horz 1=161(load case 5)
Max Uplift5=-55(load case 3), 6=-238(load case 5)
Max Grav 1=114(load case 1), 5=53(load case 5), 6=317(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-171/2, 2-3=-126/69, 3-4=-6/6, 4-5=-28/48
BOT CHORD 2-6=-7/2, 5-6=-6/6
WEBS 3-6=-179/257

JOINT STRESS INDEX
2 = 0.38, 3 = 0.38, 4 = 0.09, 5 = 0.13 and 6 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 5 and 238 lb uplift at joint 6.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

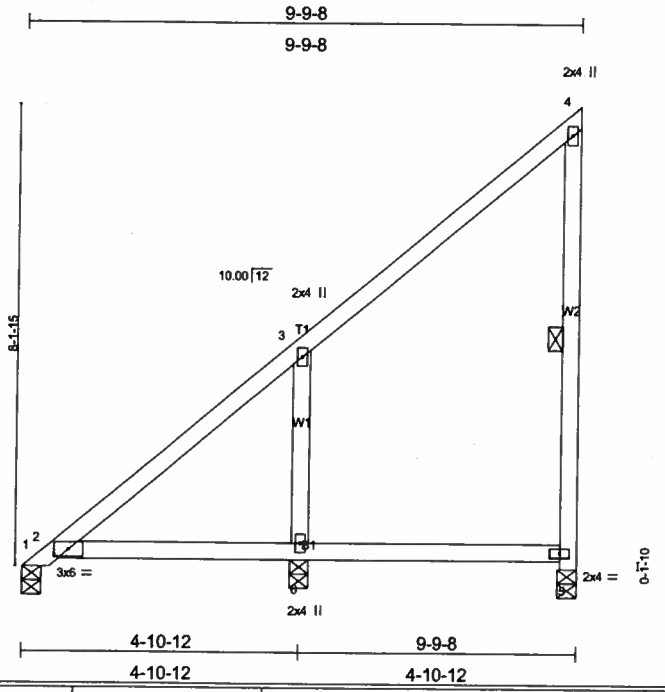
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061103

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB09	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:50:34 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.02	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.03	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-5

REACTIONS (lb/size) 1=152/0-4-0, 5=159/0-4-0, 6=476/0-4-0
Max Horz 1=364(load case 5)
Max Uplift5=-124(load case 5), 6=-356(load case 5)
Max Grav 1=180(load case 5), 5=159(load case 1), 6=476(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-414/21, 2-3=-378/83, 3-4=-109/58, 4-5=-101/141
BOT CHORD 2-6=-9/9, 5-6=-9/9
WEBS 3-6=-293/382

JOINT STRESS INDEX
2 = 0.41, 3 = 0.19, 4 = 0.62, 5 = 0.34 and 6 = 0.22

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 5 and 356 lb uplift at joint 6.
5) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

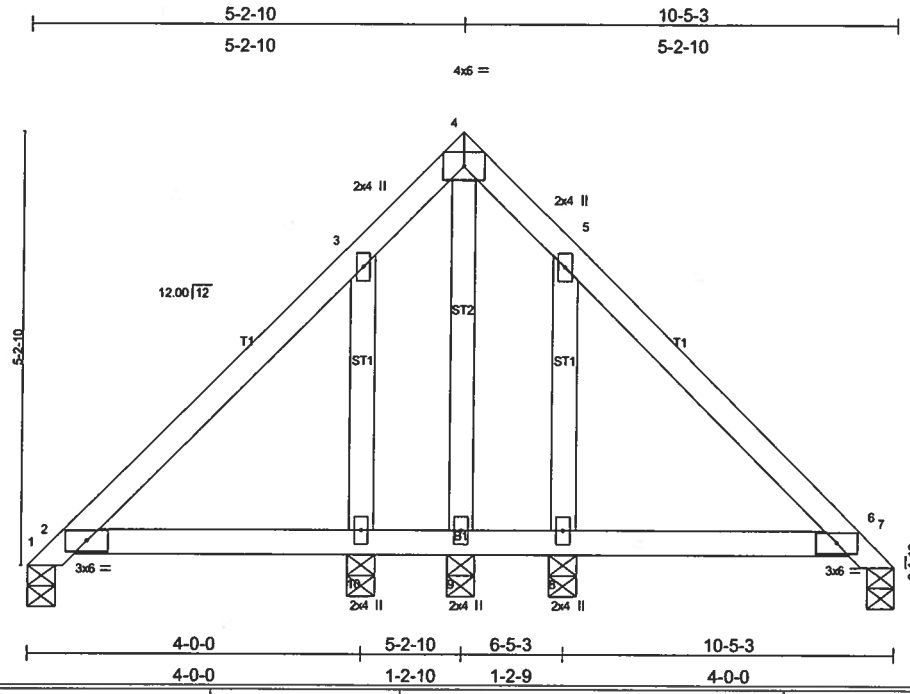
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061104

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB10A	VALLEY	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:52:12 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=27/0-4-0, 9=76/0-4-0, 8=352/0-4-0, 10=352/0-4-0, 7=27/0-4-0
Max Horz 1=175(load case 4)
Max Uplift1=-50(load case 3), 8=-272(load case 6), 10=-287(load case 5), 7=-11(load case 9)
Max Grav 1=92(load case 4), 9=114(load case 6), 8=352(load case 1), 10=352(load case 1), 7=52(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-194/172, 2-3=-185/229, 3-4=-13/131, 4-5=-21/131, 5-6=-168/229, 6-7=-37/14
BOT CHORD 2-10=-108/210, 9-10=-108/210, 8-9=-108/210, 6-8=-108/210
WEBS 4-9=-153/9, 5-8=-204/263, 3-10=-204/263

JOINT STRESS INDEX
2 = 0.30, 3 = 0.13, 4 = 0.07, 5 = 0.13, 6 = 0.30, 8 = 0.15, 9 = 0.06 and 10 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 272 lb uplift at joint 8, 287 lb uplift at joint 10 and 11 lb uplift at joint 7.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

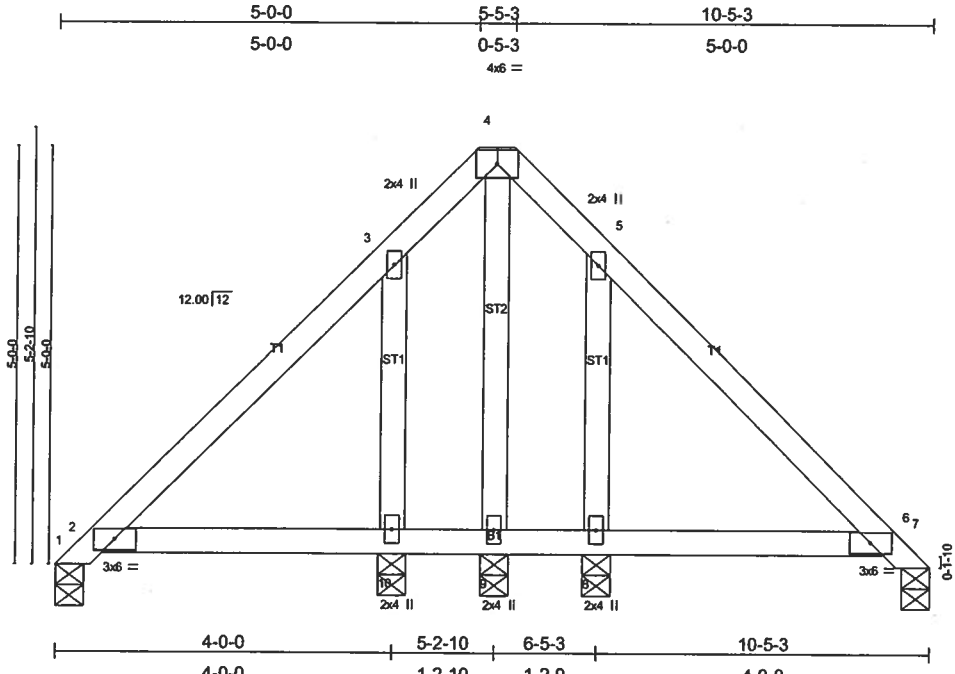
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061105

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB10B	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:54:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=27/0-4-0, 9=76/0-4-0, 8=352/0-4-0, 10=352/0-4-0, 7=27/0-4-0
Max Horz 1=-175(load case 3)
Max Uplift1=-50(load case 3), 8=-272(load case 6), 10=-287(load case 5), 7=-11(load case 9)
Max Grav 1=92(load case 4), 9=114(load case 6), 8=352(load case 1), 10=352(load case 1), 7=52(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-194/172, 2-3=-185/229, 3-4=-13/131, 4-5=-21/131, 5-6=-168/229, 6-7=-37/14
BOT CHORD 2-10=-108/210, 9-10=-108/210, 8-9=-108/210, 6-8=-108/210
WEBS 4-9=-153/9, 5-8=-204/263, 3-10=-204/263

JOINT STRESS INDEX
2 = 0.30, 3 = 0.13, 4 = 0.10, 5 = 0.13, 6 = 0.30, 8 = 0.15, 9 = 0.06 and 10 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 272 lb uplift at joint 8, 287 lb uplift at joint 10 and 11 lb uplift at joint 7.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061106

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB10C	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:55:37 2006 Page 1

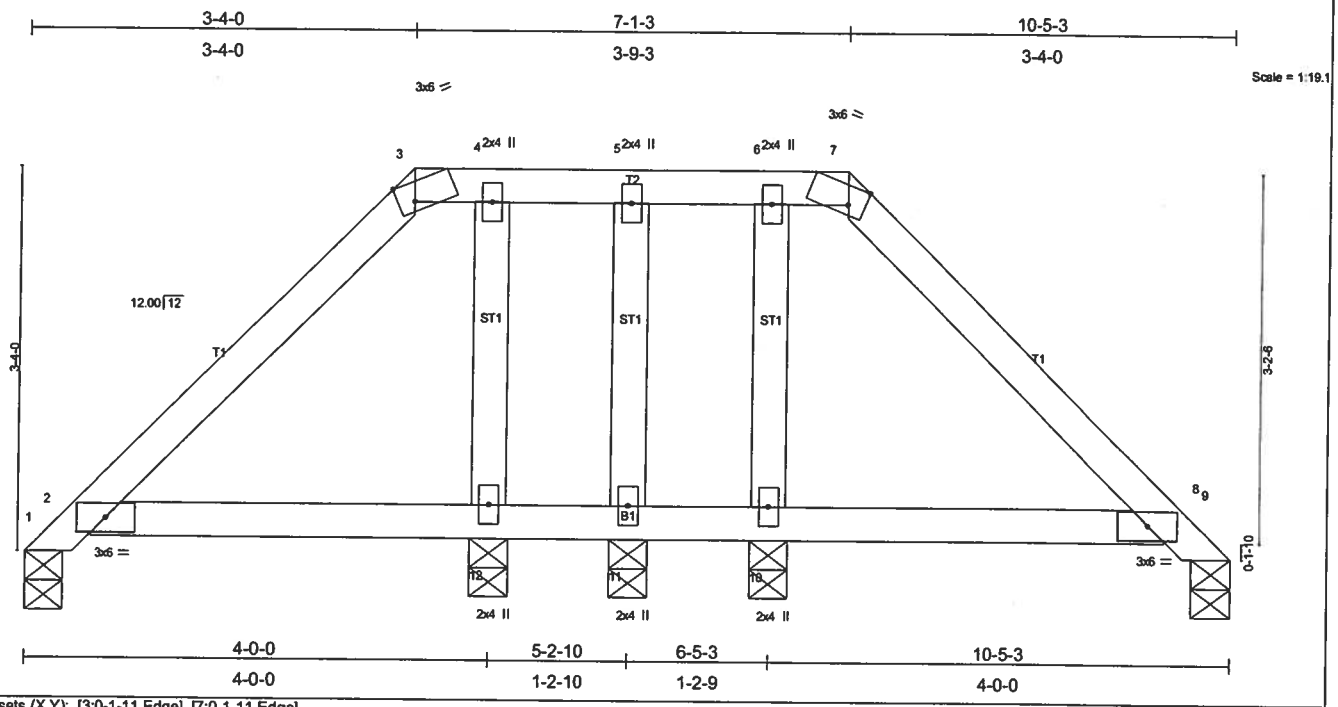


Plate Offsets (X,Y): [3:0-1-11,Edge], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=139/0-4-0, 11=-162/0-4-0, 10=359/0-4-0, 12=359/0-4-0, 9=139/0-4-0
Max Horz 1=-112(load case 3)
Max Uplift1=-70(load case 6), 11=-162(load case 1), 10=-124(load case 6), 12=-168(load case 4), 9=-67(load case 6)
Max Grav 1=139(load case 1), 11=103(load case 5), 10=362(load case 10), 12=362(load case 9), 9=139(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-93/112, 2-3=-72/85, 3-4=-23/119, 4-5=-23/119, 5-6=-23/119, 6-7=-23/119, 7-8=-72/80, 8-9=-85/56
BOT CHORD 2-12=-37/49, 11-12=-37/49, 10-11=-37/49, 8-10=-37/49
WEBS 5-11=-50/59, 6-10=-196/116, 4-12=-196/147

JOINT STRESS INDEX
2 = 0.36, 3 = 0.24, 4 = 0.08, 5 = 0.04, 6 = 0.08, 7 = 0.24, 8 = 0.36, 10 = 0.08, 11 = 0.04 and 12 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 1, 162 lb uplift at joint 11, 124 lb uplift at joint 10, 168 lb uplift at joint 12 and 67 lb uplift at joint 9.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061107

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB10D	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:57:09 2006 Page 1

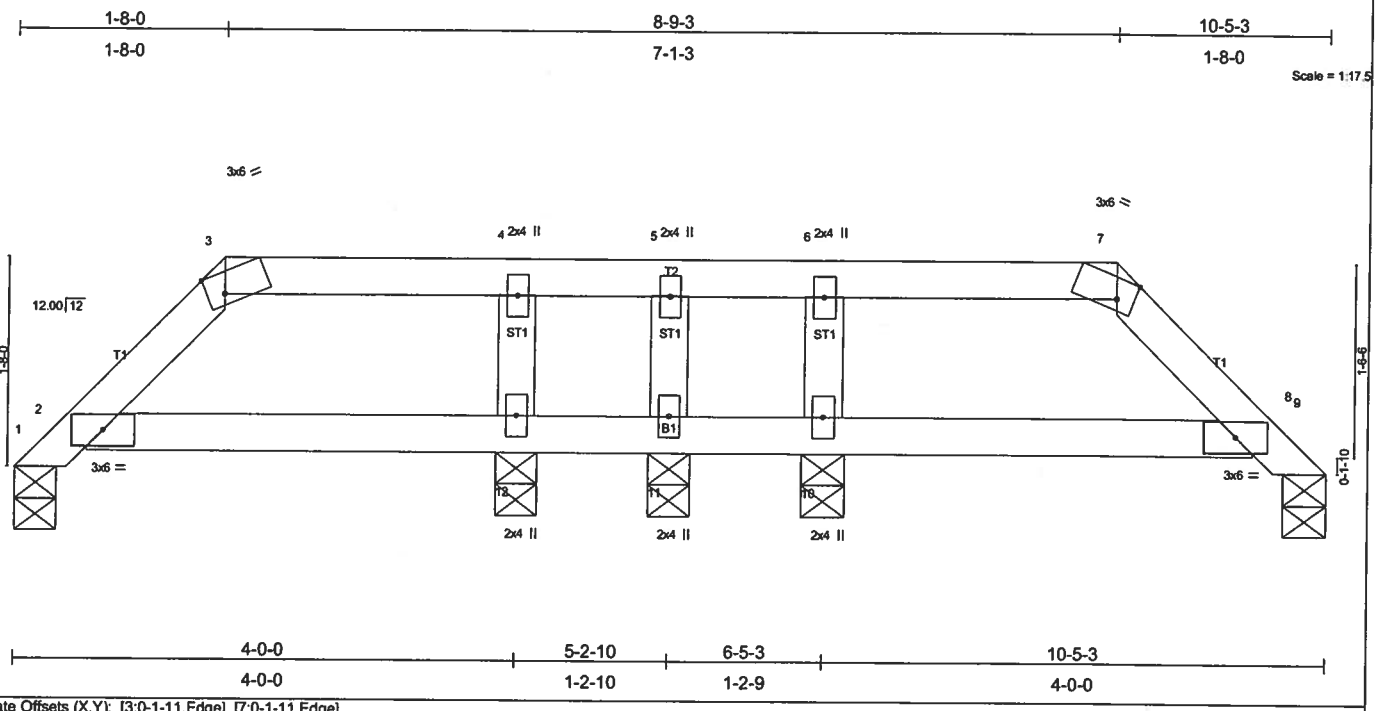


Plate Offsets (X,Y): [3:0-1-11,Edge], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.01	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=174/0-4-0, 9=174/0-4-0, 11=-61/0-4-0, 10=273/0-4-0, 12=273/0-4-0
Max Horz 1=54(load case 4)
Max Uplift1=-57(load case 5), 9=-60(load case 6), 11=-61(load case 1), 10=-112(load case 3), 12=-126(load case 4)
Max Grav 1=174(load case 1), 9=174(load case 1), 11=4(load case 5), 10=280(load case 10), 12=280(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-107/66, 2-3=-167/120, 3-4=-124/117, 4-5=-124/117, 5-6=-124/117, 6-7=-124/117, 7-8=-167/120, 8-9=-107/62
BOT CHORD 2-12=-72/124, 11-12=-72/124, 10-11=-72/124, 8-10=-72/124
WEBS 5-11=-8/22, 6-10=-141/121, 4-12=-141/122

JOINT STRESS INDEX
2 = 0.17, 3 = 0.14, 4 = 0.07, 5 = 0.01, 6 = 0.07, 7 = 0.14, 8 = 0.17, 10 = 0.07, 11 = 0.01 and 12 = 0.07

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1, 60 lb uplift at joint 9, 61 lb uplift at joint 11, 112 lb uplift at joint 10 and 126 lb uplift at joint 12.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061108

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB12	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:59:28 2006 Page 1

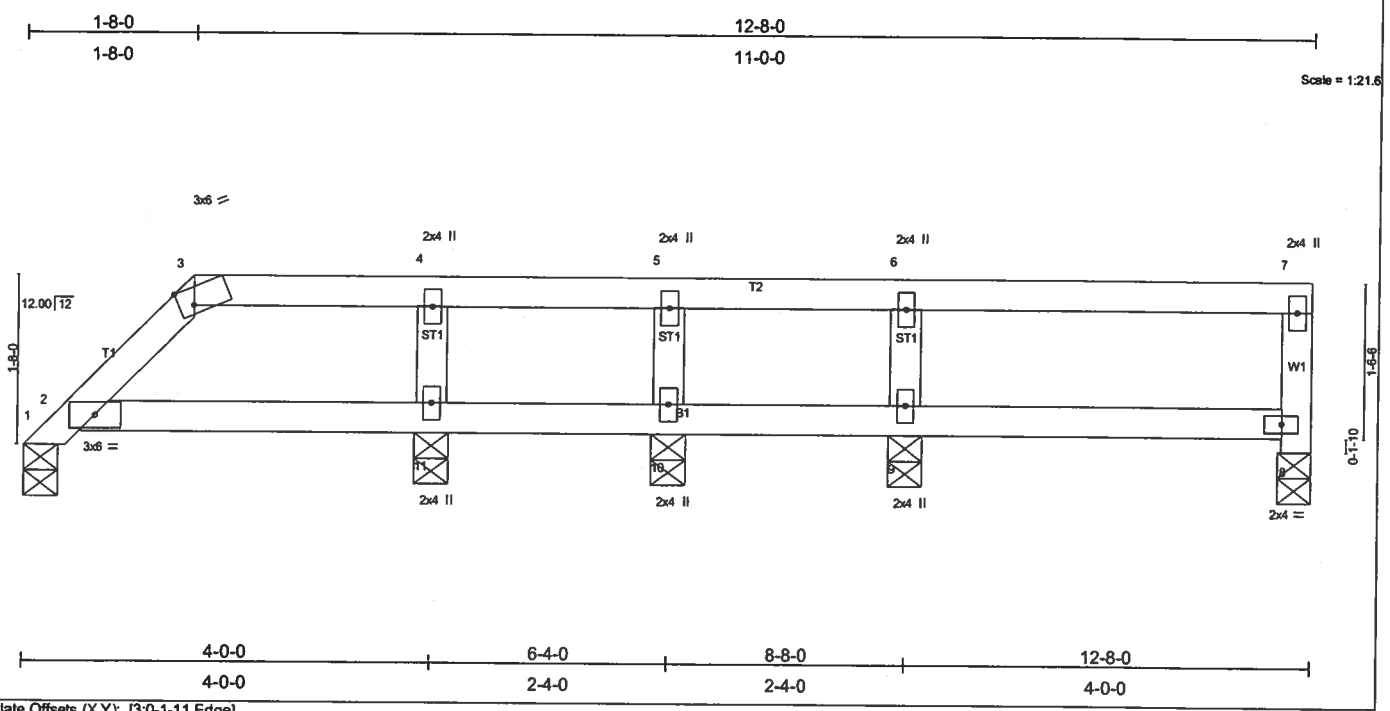


Plate Offsets (X,Y): [3:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=158/0-4-0, 8=167/0-4-0, 10=133/0-4-0, 9=284/0-4-0, 11=288/0-4-0
Max Horz 1=72(load case 5)
Max Uplift1=-37(load case 5), 8=-78(load case 3), 10=-76(load case 3), 9=-135(load case 3), 11=-109(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-97/16, 2-3=-127/57, 3-4=-85/54, 4-5=-85/54, 5-6=-85/54, 6-7=-85/54, 7-8=-106/93
BOT CHORD 2-11=-54/85, 10-11=-54/85, 9-10=-54/85, 8-9=-54/85
WEBS 5-10=-93/85, 6-9=-183/159, 4-11=-164/150

JOINT STRESS INDEX
2 = 0.24, 3 = 0.11, 4 = 0.09, 5 = 0.05, 6 = 0.09, 7 = 0.63, 8 = 0.53, 9 = 0.09, 10 = 0.05 and 11 = 0.09

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 1, 78 lb uplift at joint 8, 76 lb uplift at joint 10, 135 lb uplift at joint 9 and 109 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061109

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB12A	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:00:34 2006 Page 1

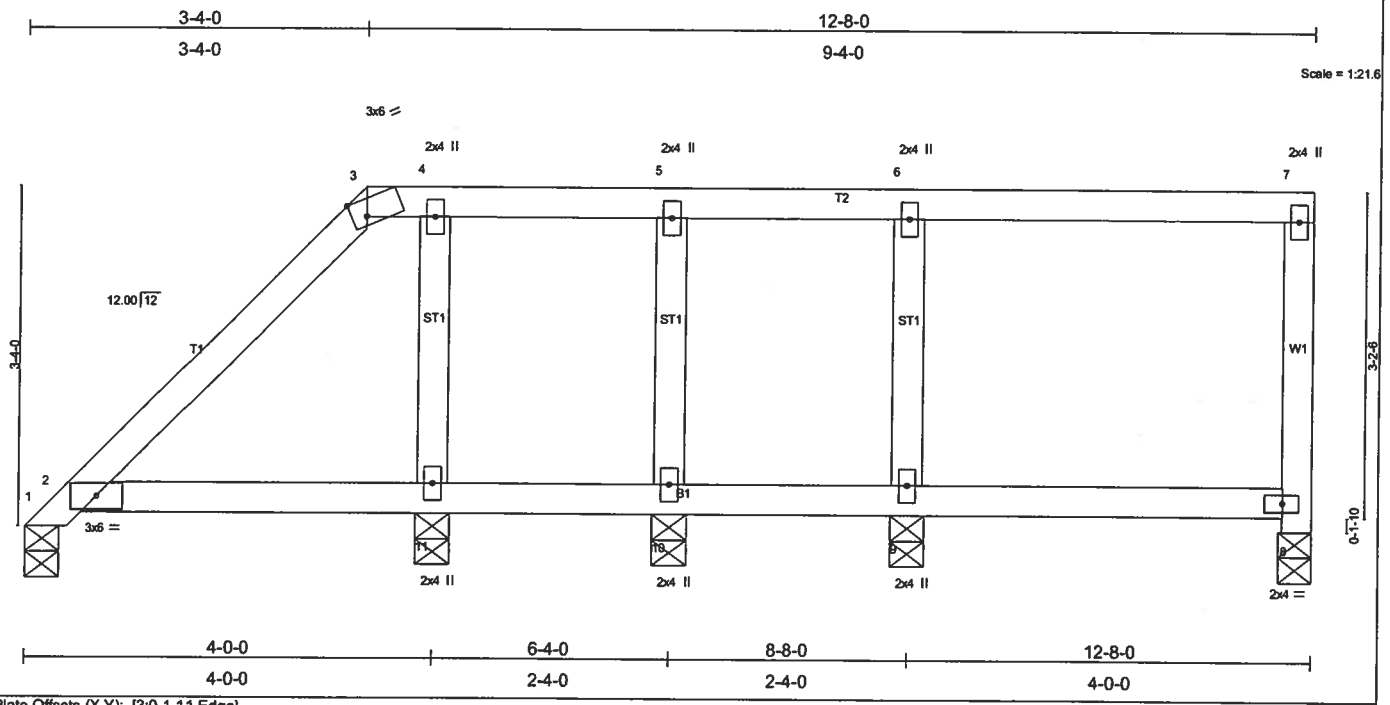


Plate Offsets (X,Y): [3:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=143/0-4-0, 8=157/0-4-0, 10=110/0-4-0, 9=302/0-4-0, 11=318/0-4-0
Max Horz 1=149(load case 5)
Max Uplift8=-75(load case 3), 10=-101(load case 3), 9=-134(load case 3), 11=-203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-130/5, 2-3=-83/29, 3-4=-26/15, 4-5=-26/15, 5-6=-26/15, 6-7=-26/15, 7-8=-100/86
BOT CHORD 2-11=-15/26, 10-11=-15/26, 9-10=-15/26, 8-9=-15/26
WEBS 5-10=-78/104, 6-9=-194/172, 4-11=-179/213

JOINT STRESS INDEX
2 = 0.41, 3 = 0.14, 4 = 0.12, 5 = 0.06, 6 = 0.10, 7 = 0.45, 8 = 0.39, 9 = 0.10, 10 = 0.06 and 11 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 8, 101 lb uplift at joint 10, 134 lb uplift at joint 9 and 203 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061110

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB12B	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:01:27 2006 Page 1

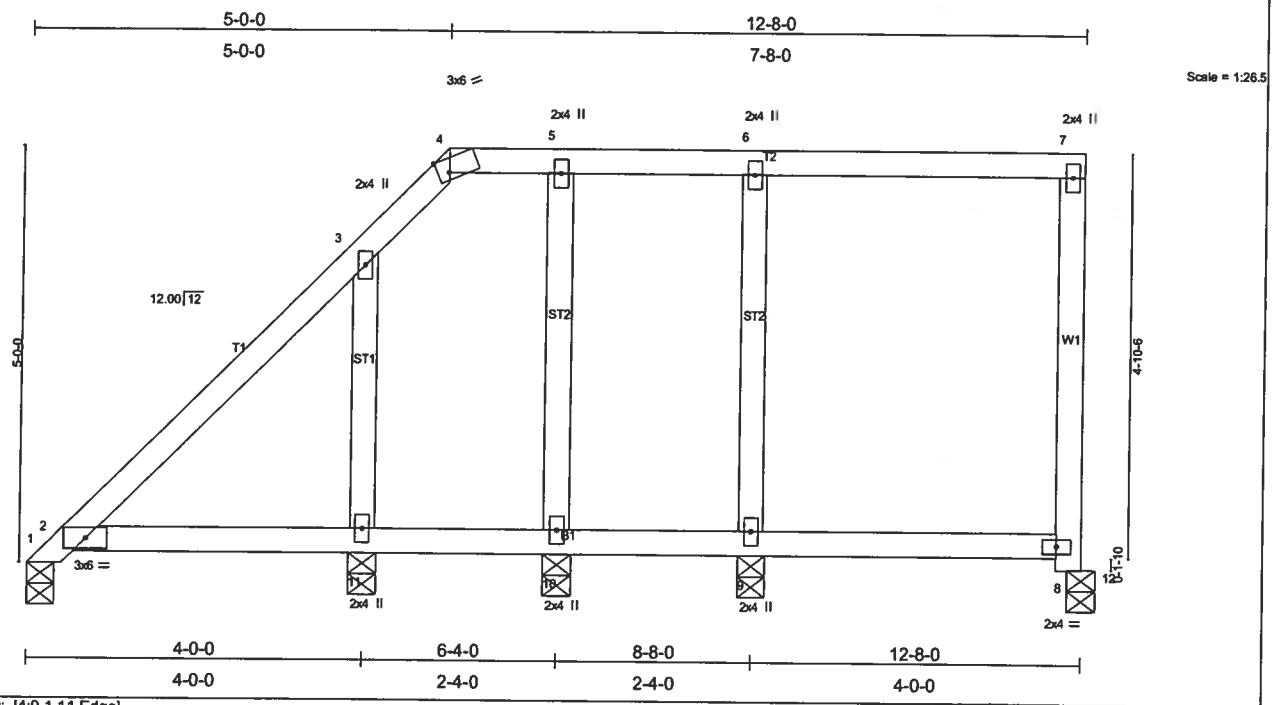


Plate Offsets (X,Y): [4:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 64 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=136/0-4-0, 12=151/0-4-0, 10=100/0-4-0, 9=313/0-4-0, 11=332/0-4-0
Max Horz 1=226(load case 5)
Max Uplift12=-75(load case 3), 10=-81(load case 3), 9=-137(load case 3), 11=-289(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-239/12, 2-3=-199/61, 3-4=-76/27, 4-5=-12/8, 5-6=-12/8, 6-7=-12/8, 8-12=-151/75, 7-8=-96/82
BOT CHORD 2-11=-8/12, 10-11=-8/12, 9-10=-8/12, 8-9=-8/12
WEBS 5-10=-71/84, 6-9=-201/176, 3-11=-190/269

JOINT STRESS INDEX
2 = 0.41, 3 = 0.13, 4 = 0.06, 5 = 0.05, 6 = 0.10, 7 = 0.34, 8 = 0.29, 9 = 0.10, 10 = 0.05 and 11 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 12, 81 lb uplift at joint 10, 137 lb uplift at joint 9 and 289 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061111

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB12C	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:02:18 2006 Page 1

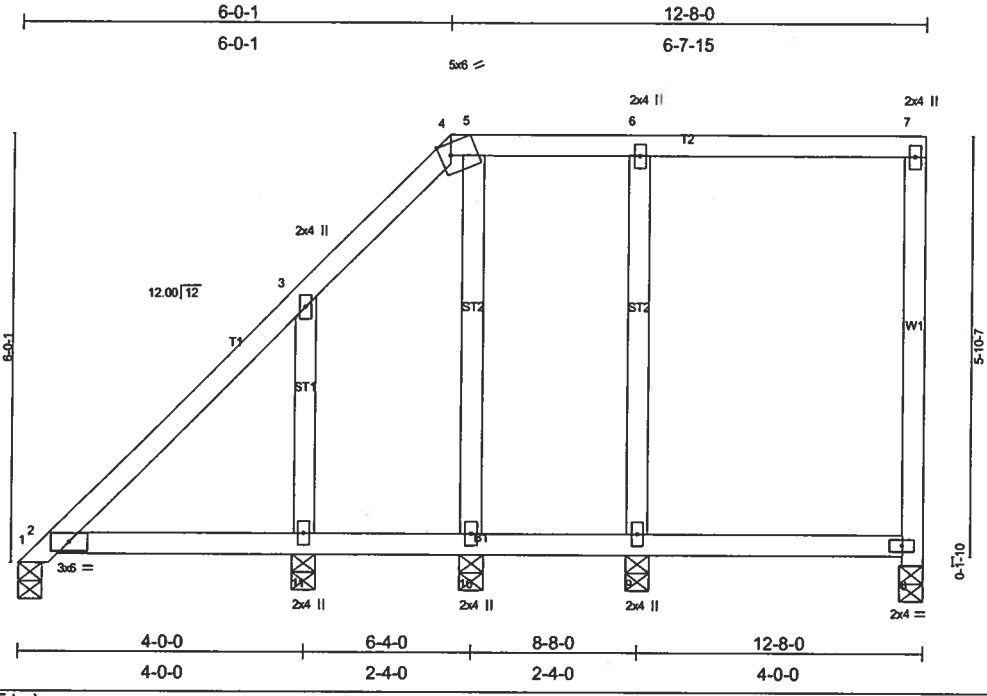


Plate Offsets (X,Y): [4:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 69 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=133/0-4-0, 8=150/0-4-0, 10=98/0-4-0, 9=313/0-4-0, 11=337/0-4-0
Max Horz 1=272(load case 5)
Max Uplift8=-73(load case 3), 10=-45(load case 5), 9=-145(load case 3), 11=-307(load case 5)
Max Grav 1=148(load case 5), 8=150(load case 1), 10=98(load case 1), 9=313(load case 1), 11=337(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-304/17, 2-3=-266/65, 3-4=-75/11, 4-5=-10/5, 5-6=-10/5, 6-7=-10/5, 7-8=-96/82
BOT CHORD 2-11=-5/10, 10-11=-5/10, 9-10=-5/10, 8-9=-5/10
WEBS 5-10=-69/91, 6-9=-200/173, 3-11=-195/285

JOINT STRESS INDEX
2 = 0.39, 3 = 0.14, 4 = 0.04, 5 = 0.00, 6 = 0.10, 7 = 0.32, 8 = 0.29, 9 = 0.10, 10 = 0.05 and 11 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 73 lb uplift at joint 8, 45 lb uplift at joint 10, 145 lb uplift at joint 9 and 307 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061112

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB12D	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:03:09 2006 Page 1

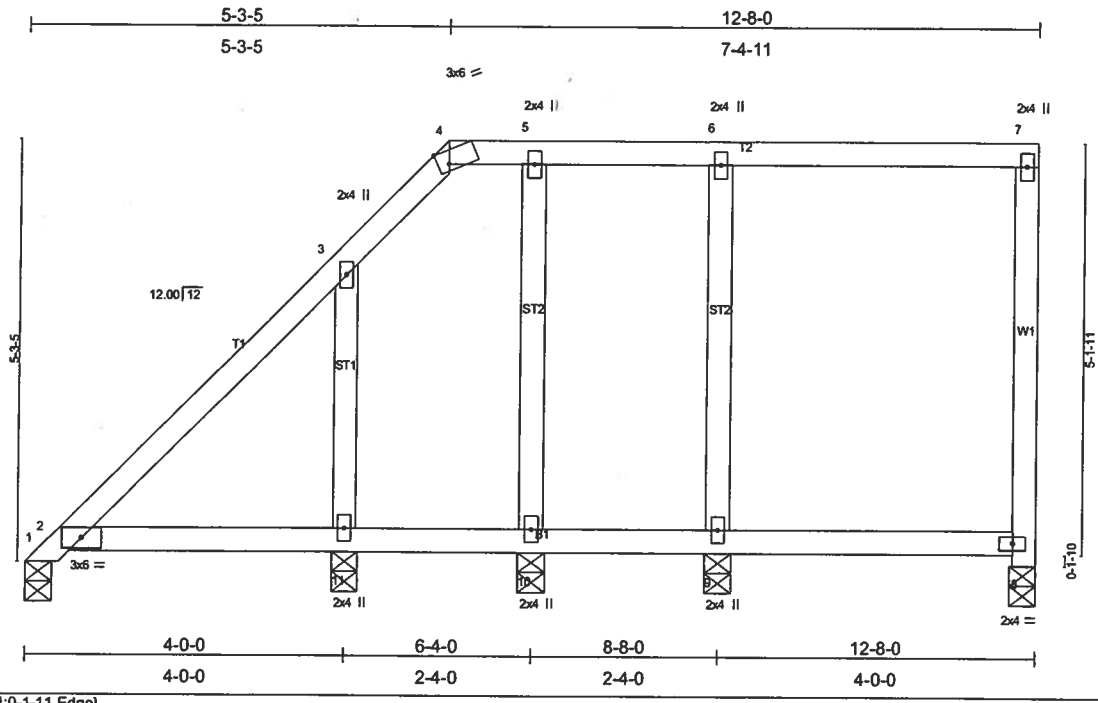


Plate Offsets (X,Y): [4:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=135/0-4-0, 8=150/0-4-0, 10=99/0-4-0, 9=313/0-4-0, 11=334/0-4-0
Max Horz 1=238(load case 5)
Max Uplift8=-74(load case 3), 10=-70(load case 3), 9=-139(load case 3), 11=-297(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-257/14, 2-3=-217/62, 3-4=-76/15, 4-5=-11/7, 5-6=-11/7, 6-7=-11/7, 7-8=-96/82
BOT CHORD 2-11=-7/11, 10-11=-7/11, 9-10=-7/11, 8-9=-7/11
WEBS 5-10=-69/72, 6-9=-201/175, 3-11=-192/275

JOINT STRESS INDEX
2 = 0.41, 3 = 0.14, 4 = 0.03, 5 = 0.04, 6 = 0.10, 7 = 0.33, 8 = 0.30, 9 = 0.10, 10 = 0.04 and 11 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 8, 70 lb uplift at joint 10, 139 lb uplift at joint 9 and 297 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

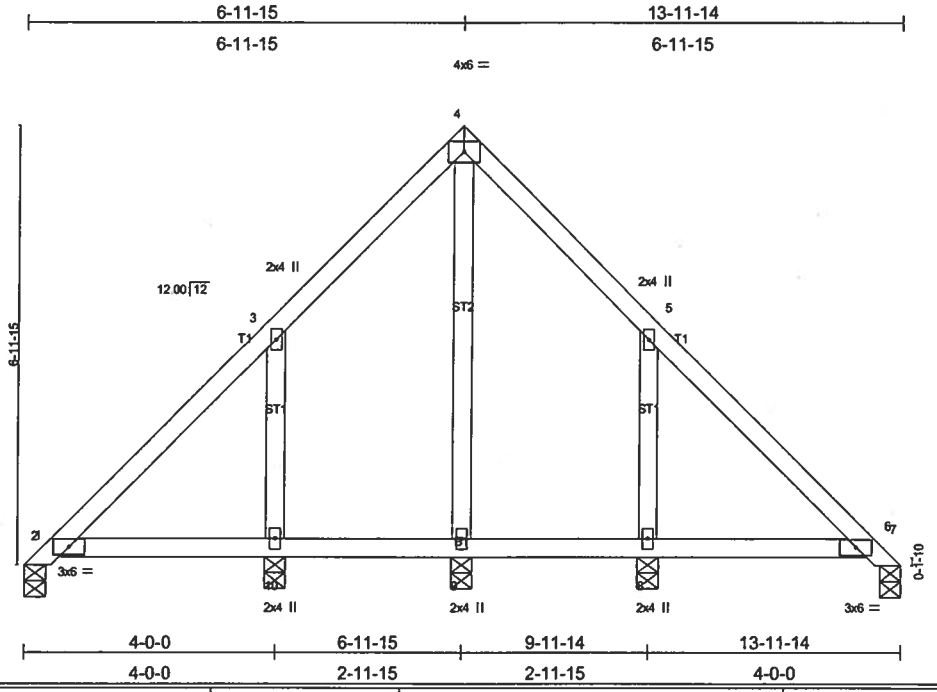
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061113

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB13	VALLEY	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:03:49 2006 Page 1



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.12	Vert(LL)	0.01	2-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.09	Vert(TL)	-0.01	2-10	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.19	Horz(TL)	0.01	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=70/0-4-0, 7=70/0-4-0, 9=318/0-4-0, 8=337/0-4-0, 10=337/0-4-0
Max Horz 1=237(load case 4)
Max Uplift1=-87(load case 3), 9=-14(load case 4), 8=-288(load case 6), 10=-299(load case 5)
Max Grav 1=135(load case 4), 7=80(load case 10), 9=318(load case 1), 8=343(load case 10), 10=343(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-269/244, 2-3=-217/162, 3-4=-53/132, 4-5=-2/132, 5-6=-151/162, 6-7=-48/1
BOT CHORD 2-10=-59/192, 9-10=-59/192, 8-9=-59/192, 6-8=-59/192
WEBS 4-9=-255/36, 5-8=-209/288, 3-10=-209/293

JOINT STRESS INDEX
2 = 0.29, 3 = 0.14, 4 = 0.15, 5 = 0.14, 6 = 0.29, 8 = 0.17, 9 = 0.09 and 10 = 0.17

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 1, 14 lb uplift at joint 9, 288 lb uplift at joint 8 and 299 lb uplift at joint 10.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061114

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB14	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:05:13 2006 Page 1

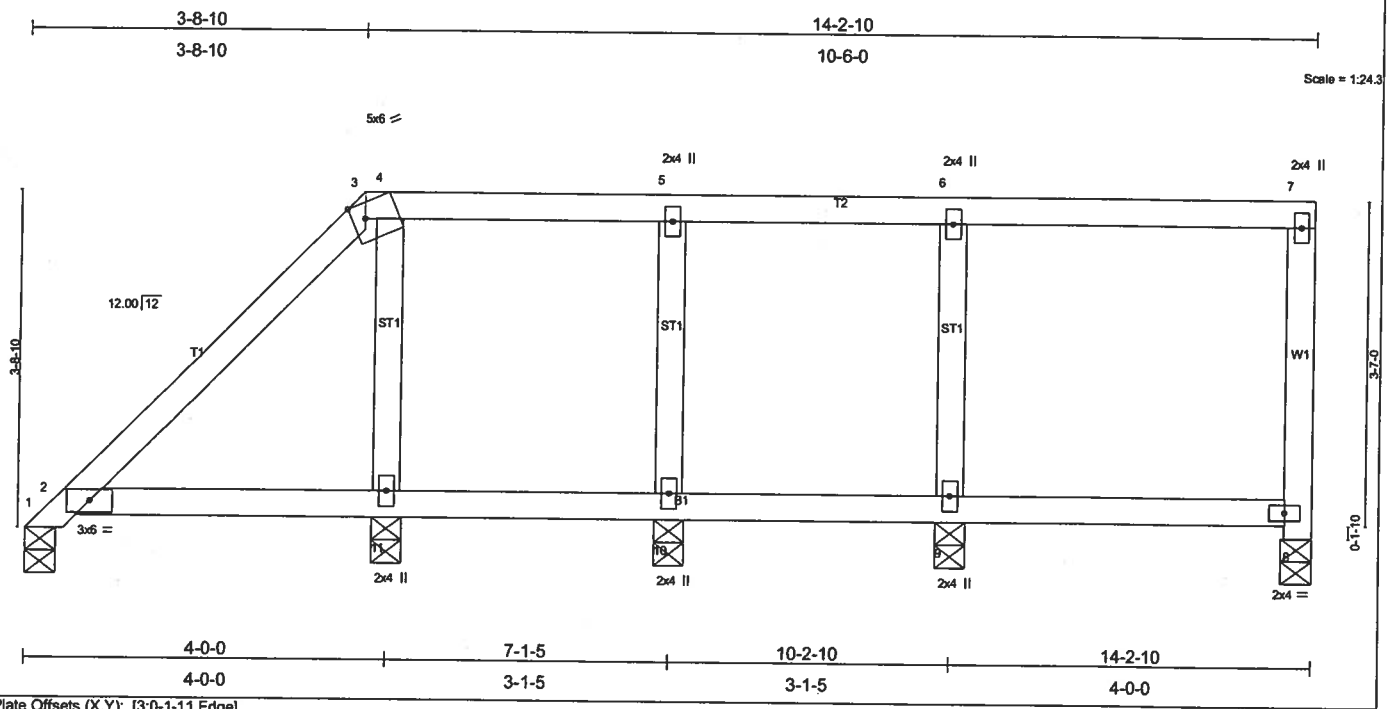


Plate Offsets (X,Y): [3:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.01	1	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 62 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=139/0-4-0, 8=152/0-4-0, 10=217/0-4-0, 9=321/0-4-0, 11=333/0-4-0
Max Horz 8=166(load case 5)
Max Uplift8=-74(load case 3), 10=-139(load case 3), 9=-143(load case 3), 11=-190(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-85/0, 2-3=-102/45, 3-4=-19/12, 4-5=-19/12, 5-6=-19/12, 6-7=-19/12, 7-8=-97/82
BOT CHORD 2-11=-24/160, 10-11=-24/160, 9-10=-24/160, 8-9=-24/160
WEBS 5-10=-144/151, 6-9=-207/170, 4-11=-192/214

JOINT STRESS INDEX
2 = 0.33, 3 = 0.29, 4 = 0.00, 5 = 0.09, 6 = 0.10, 7 = 0.39, 8 = 0.33, 9 = 0.10, 10 = 0.09 and 11 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 8, 139 lb uplift at joint 10, 143 lb uplift at joint 9 and 190 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061115

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB14A	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:06:06 2006 Page 1

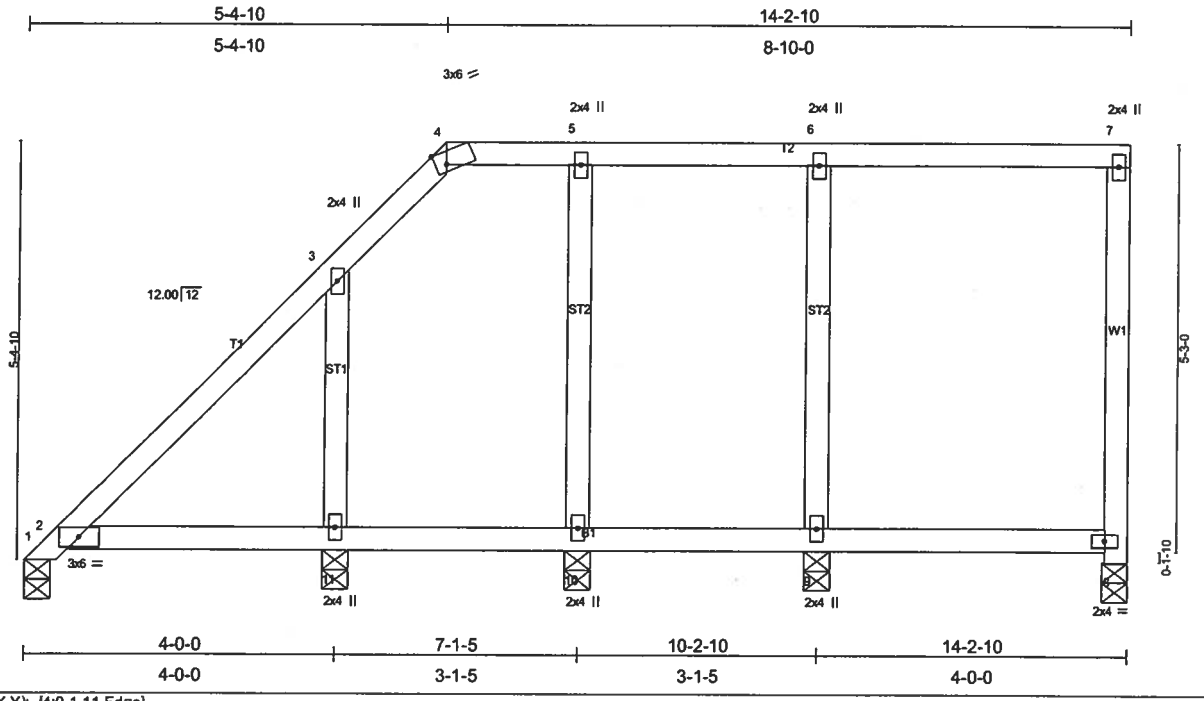


Plate Offsets (X,Y): [4:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 71 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=134/0-4-0, 8=149/0-4-0, 10=211/0-4-0, 9=326/0-4-0, 11=341/0-4-0
Max Horz 1=243(load case 5)
Max Uplift8=-73(load case 3), 10=-112(load case 3), 9=-149(load case 3), 11=-297(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-264/14, 2-3=-225/63, 3-4=-80/17, 4-5=-11/7, 5-6=-11/7, 6-7=-11/7, 7-8=-95/80
BOT CHORD 2-11=-7/11, 10-11=-7/11, 9-10=-7/11, 8-9=-7/11
WEBS 5-10=-139/123, 6-9=-210/177, 3-11=-200/282

JOINT STRESS INDEX
2 = 0.39, 3 = 0.14, 4 = 0.08, 5 = 0.07, 6 = 0.10, 7 = 0.33, 8 = 0.30, 9 = 0.10, 10 = 0.07 and 11 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 73 lb uplift at joint 8, 112 lb uplift at joint 10, 149 lb uplift at joint 9 and 297 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

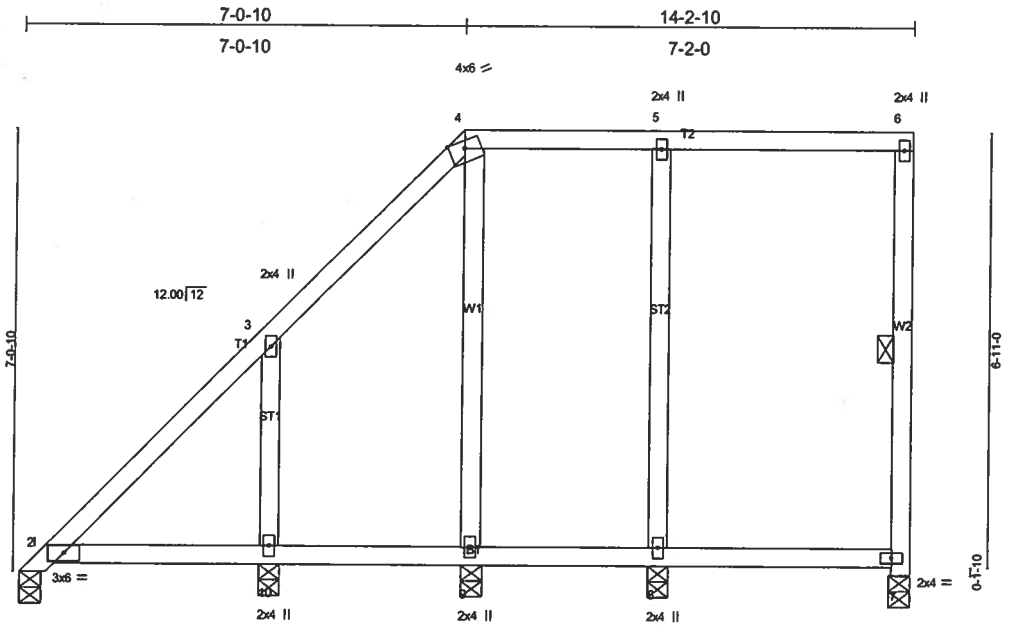
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061116

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB14B	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:07:41 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.01	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 79 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7

REACTIONS (lb/size) 1=132/0-4-0, 7=147/0-4-0, 8=328/0-4-0, 10=348/0-4-0, 9=206/0-4-0
Max Horz 1=320(load case 5)
Max Uplift7=-69(load case 3), 8=-163(load case 3), 10=-321(load case 5), 9=-121(load case 5)
Max Grav 1=194(load case 5), 7=147(load case 1), 8=328(load case 1), 10=348(load case 1), 9=206(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-371/21, 2-3=-333/65, 3-4=-91/36, 4-5=-8/3, 5-6=-7/4, 6-7=-93/77
BOT CHORD 2-10=-8/10, 9-10=-8/10, 8-9=-4/7, 7-8=-4/7
WEBS 5-8=-212/190, 3-10=-205/310, 4-9=-134/170

JOINT STRESS INDEX
2 = 0.36, 3 = 0.15, 4 = 0.23, 5 = 0.11, 6 = 0.29, 7 = 0.28, 8 = 0.11, 9 = 0.10 and 10 = 0.18

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 7, 163 lb uplift at joint 8, 321 lb uplift at joint 10 and 121 lb uplift at joint 9.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061117

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB14C	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 06:58:21 2006 Page 1

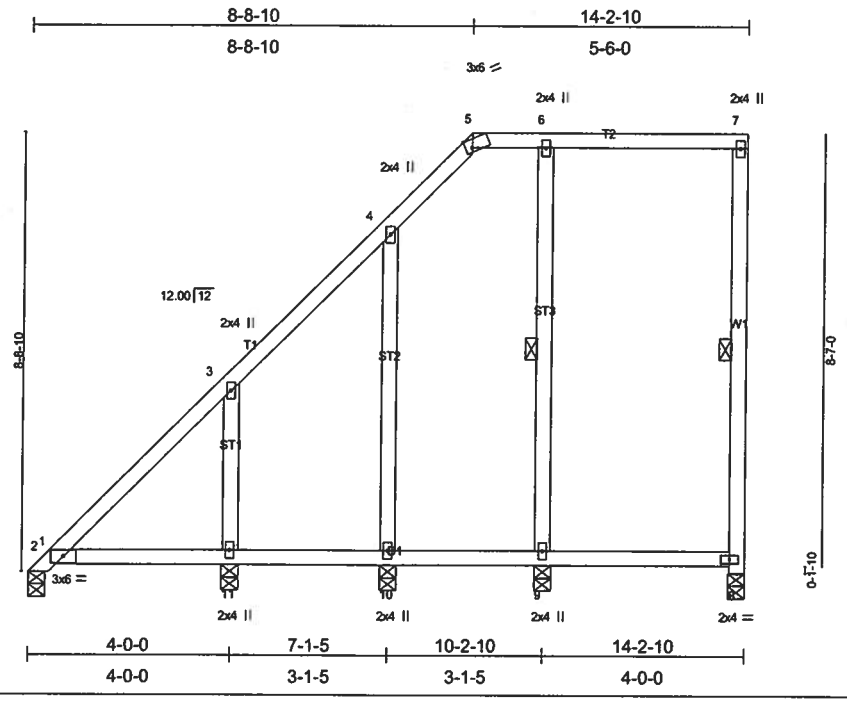


Plate Offsets (X,Y): [5:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 7-8, 6-9

REACTIONS (lb/size) 1=128/0-4-0, 8=146/0-4-0, 10=213/0-4-0, 9=326/0-4-0, 11=348/0-4-0
Max Horz 1=397(load case 5)
Max Uplift1=-1(load case 3), 8=-73(load case 3), 10=-206(load case 5), 9=-132(load case 3), 11=-316(load case 5)
Max Grav 1=274(load case 5), 8=146(load case 1), 10=213(load case 1), 9=326(load case 1), 11=348(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-481/31, 2-3=-447/72, 3-4=-209/43, 4-5=-58/5, 5-6=-5/3, 6-7=-5/3, 7-8=-93/81
BOT CHORD 2-11=-3/5, 10-11=-3/5, 9-10=-3/5, 8-9=-3/5
WEBS 4-10=-141/232, 6-9=-208/184, 3-11=-208/306

JOINT STRESS INDEX
2 = 0.34, 3 = 0.15, 4 = 0.11, 5 = 0.09, 6 = 0.11, 7 = 0.27, 8 = 0.26, 9 = 0.11, 10 = 0.13 and 11 = 0.17

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 1, 73 lb uplift at joint 8, 206 lb uplift at joint 10, 132 lb uplift at joint 9 and 316 lb uplift at joint 11.
6) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061118

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB18	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:18:07 2006 Page 1

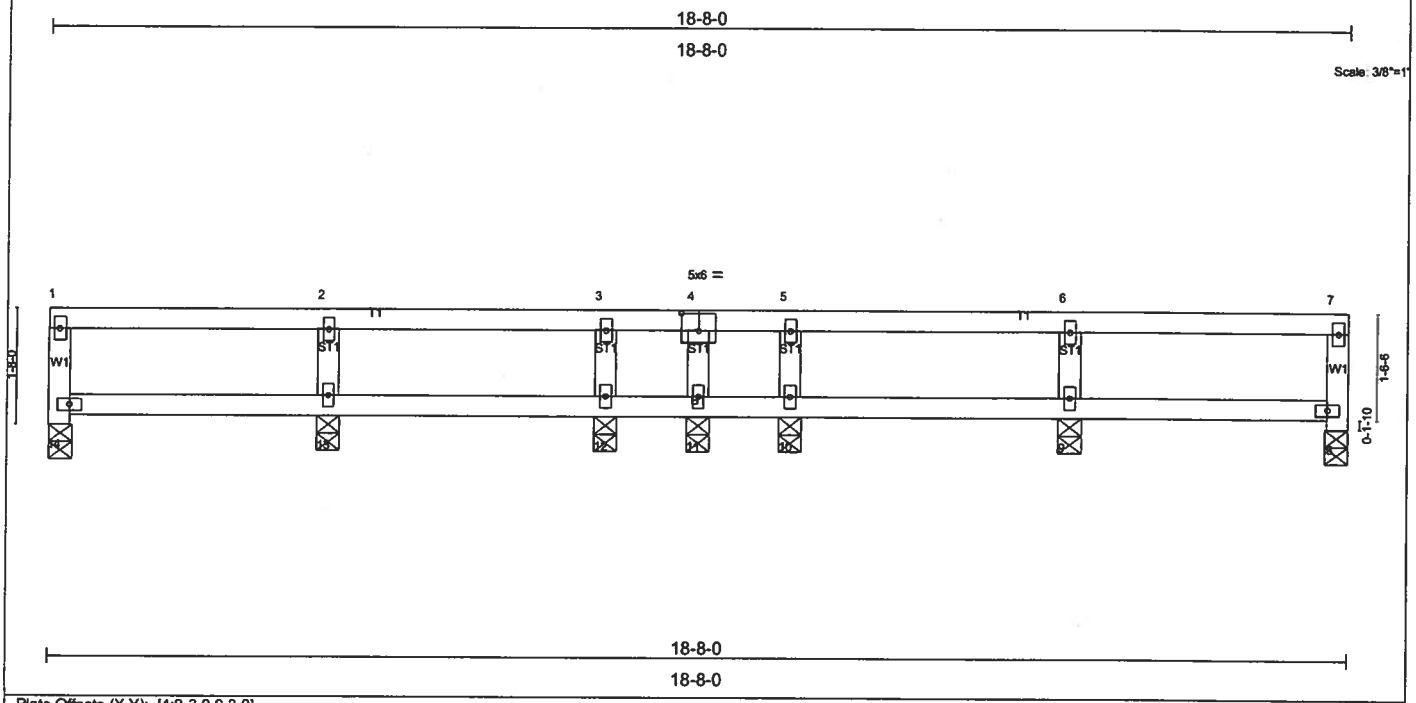


Plate Offsets (X,Y): [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 11=-49/0-4-0, 9=352/0-4-0, 10=296/0-4-0, 13=352/0-4-0, 12=296/0-4-0, 14=149/0-4-0, 8=149/0-4-0
Max Uplift11=-49(load case 1), 9=-133(load case 3), 10=-110(load case 3), 13=-133(load case 3), 12=-110(load case 3), 14=-56(load case 3), 8=-56(load case 3)
Max Grav 11=16(load case 3), 9=352(load case 1), 10=296(load case 1), 13=352(load case 1), 12=296(load case 1), 14=149(load case 1), 8=149(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-14=-95/68, 1-2=-58/23, 2-3=-58/23, 3-4=-58/23, 4-5=-58/23, 5-6=-58/23, 6-7=-58/23, 7-8=-95/68
BOT CHORD 13-14=-23/58, 12-13=-23/58, 11-12=-23/58, 10-11=-23/58, 9-10=-23/58, 8-9=-23/58
WEBS 4-11=-20/28, 6-9=-227/164, 5-10=-188/135, 2-13=-227/164, 3-12=-188/135

JOINT STRESS INDEX
1 = 0.45, 2 = 0.34, 3 = 0.34, 4 = 0.20, 5 = 0.34, 6 = 0.34, 7 = 0.45, 8 = 0.36, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34 and 14 = 0.36

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 14, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 11, 133 lb uplift at joint 9, 110 lb uplift at joint 10, 133 lb uplift at joint 13, 110 lb uplift at joint 12, 56 lb uplift at joint 14 and 56 lb uplift at joint 8.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061119

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB18A	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:16:49 2006 Page 1

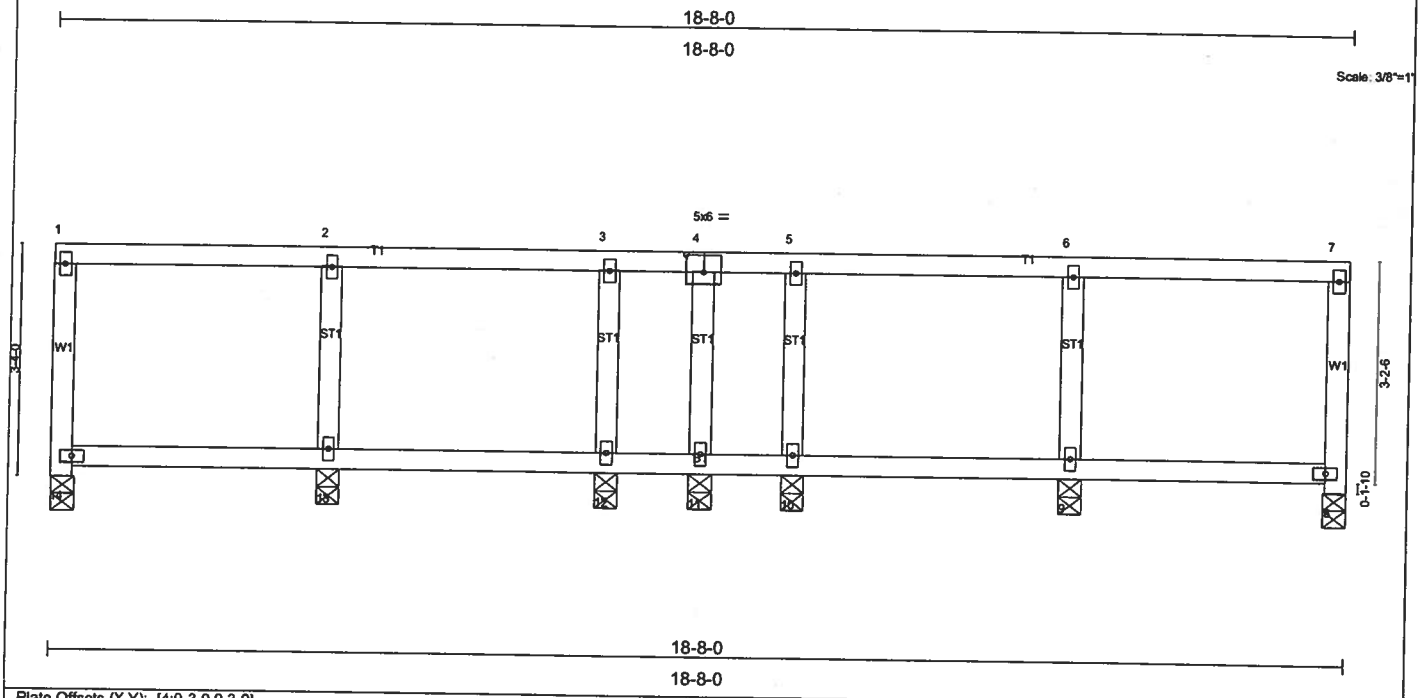


Plate Offsets (X,Y): [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.00	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 83 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 11=-38/0-4-0, 9=355/0-4-0, 10=289/0-4-0, 13=355/0-4-0, 12=289/0-4-0, 14=146/0-4-0, 8=146/0-4-0
Max Uplift11=-38(load case 1), 9=-134(load case 3), 10=-106(load case 3), 13=-134(load case 3), 12=-106(load case 3), 14=-55(load case 3), 8=-55(load case 3)
Max Grav 11=8(load case 3), 9=355(load case 1), 10=289(load case 1), 13=355(load case 1), 12=289(load case 1), 14=146(load case 1), 8=146(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-14=-93/65, 1-2=-22/9, 2-3=-22/9, 3-4=-22/9, 4-5=-22/9, 5-6=-22/9, 6-7=-22/9, 7-8=-93/65
BOT CHORD 13-14=-9/22, 12-13=-9/22, 11-12=-9/22, 10-11=-9/22, 9-10=-9/22, 8-9=-9/22
WEBS 4-11=-11/17, 6-9=-230/167, 5-10=-181/130, 2-13=-230/167, 3-12=-181/130

JOINT STRESS INDEX
1 = 0.39, 2 = 0.34, 3 = 0.34, 4 = 0.20, 5 = 0.34, 6 = 0.34, 7 = 0.39, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34 and 14 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 14, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 11, 134 lb uplift at joint 9, 106 lb uplift at joint 10, 134 lb uplift at joint 13, 106 lb uplift at joint 12, 55 lb uplift at joint 14 and 55 lb uplift at joint 8.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061120

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB18B	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:15:18 2006 Page 1

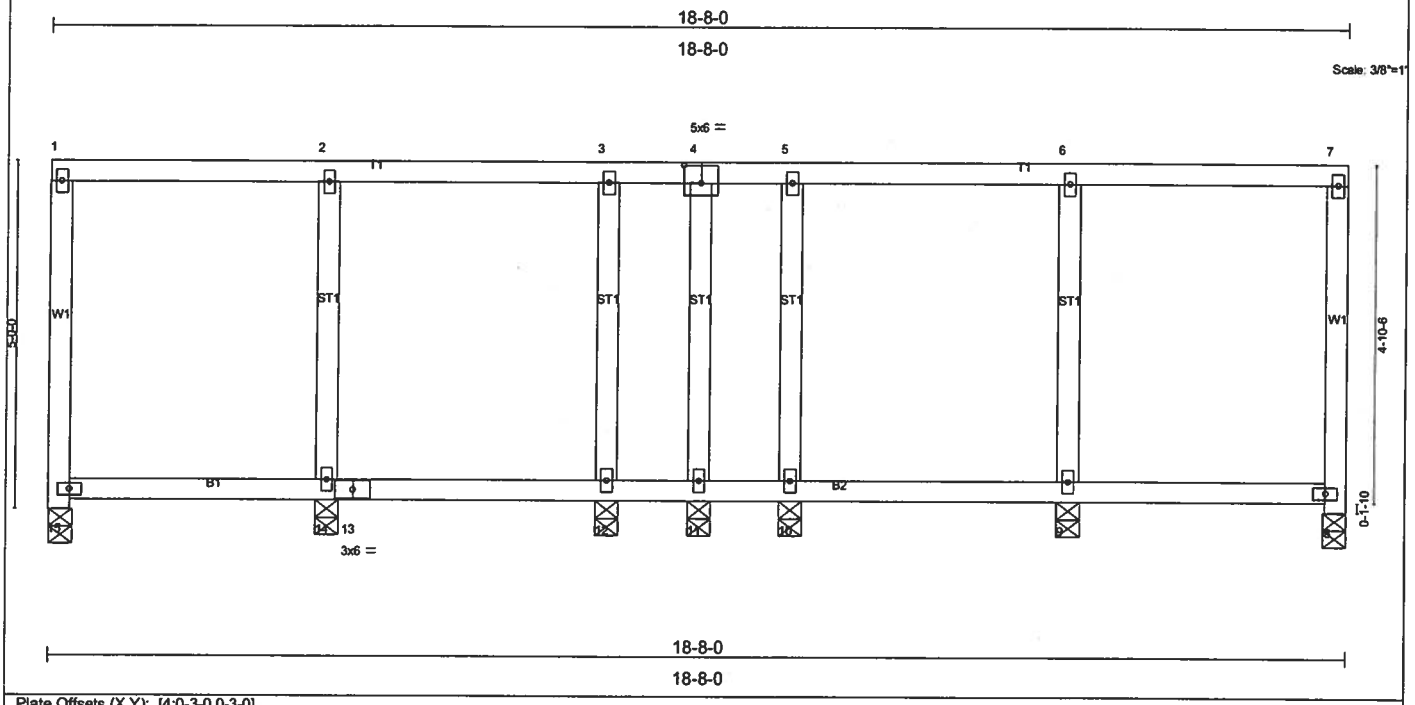


Plate Offsets (X,Y): [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.00	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 15=143/0-4-0, 11=-27/0-4-0, 9=360/0-4-0, 10=282/0-4-0, 14=360/0-4-0, 12=282/0-4-0, 8=143/0-4-0
Max Uplift15=-54(load case 3), 11=-27(load case 1), 9=-136(load case 3), 10=-102(load case 3), 14=-136(load case 3), 12=-102(load case 3), 8=-54(load case 3)
Max Grav 15=143(load case 1), 11=2(load case 3), 9=360(load case 1), 10=282(load case 1), 14=360(load case 1), 12=282(load case 1), 8=143(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-15=-91/64, 1-2=-12/5, 2-3=-12/5, 3-4=-12/5, 4-5=-12/5, 5-6=-12/5, 6-7=-12/5, 7-8=-91/64
BOT CHORD 14-15=-5/12, 13-14=-5/12, 12-13=-5/12, 11-12=-5/12, 10-11=-5/12, 9-10=-5/12, 8-9=-5/12
WEBS 4-11=-4/7, 6-9=-233/169, 5-10=-175/125, 2-14=-233/169, 3-12=-175/125

JOINT STRESS INDEX
1 = 0.34, 2 = 0.34, 3 = 0.34, 4 = 0.20, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.15, 14 = 0.34 and 15 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 15, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 15, 27 lb uplift at joint 11, 136 lb uplift at joint 9, 102 lb uplift at joint 10, 136 lb uplift at joint 14, 102 lb uplift at joint 12 and 54 lb uplift at joint 8.

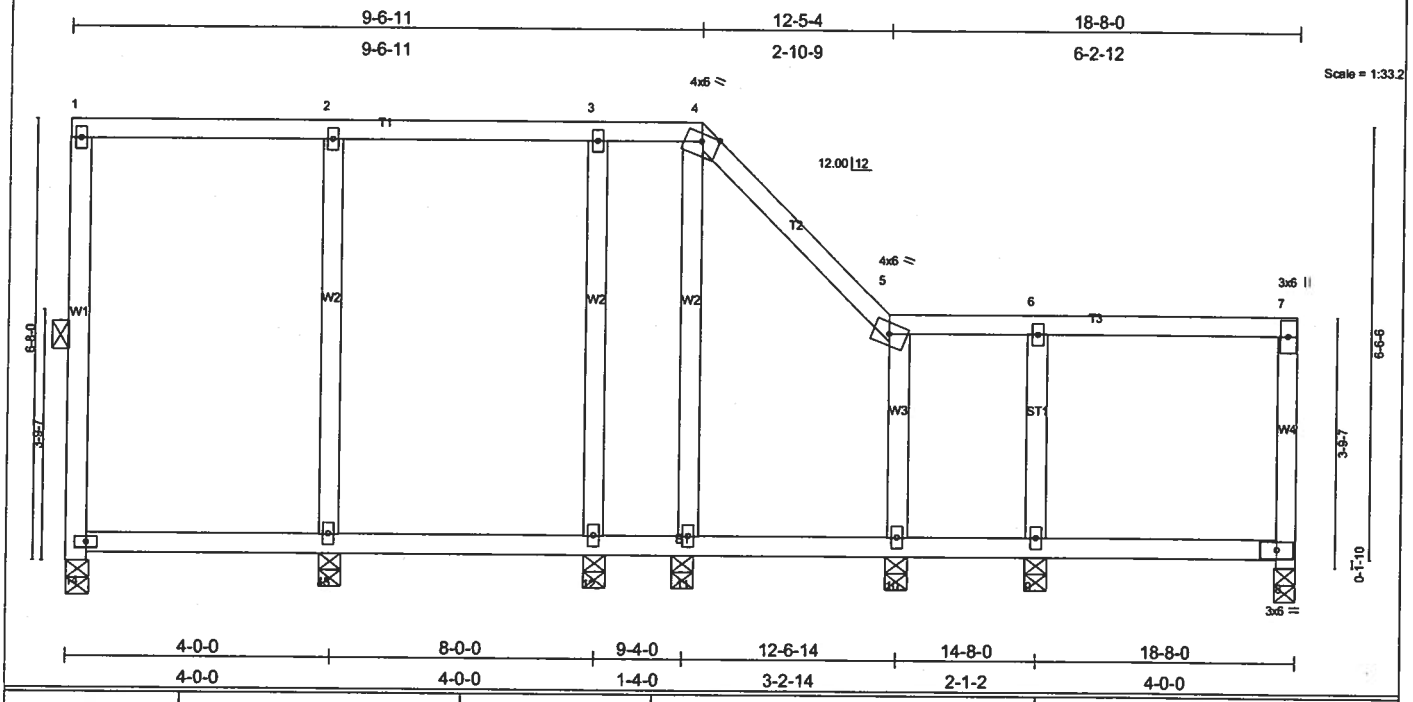
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061121

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB18C	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 08:49:37 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.02	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	0.02	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.16	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 1-14

REACTIONS (lb/size) 9=303/0-4-0, 14=146/0-4-0, 8=141/0-4-0, 11=166/0-4-0, 10=196/0-4-0, 13=357/0-4-0, 12=234/0-4-0
Max Horz 14=-92(load case 5)
Max Uplift9=-202(load case 5), 14=-78(load case 3), 8=-77(load case 4), 11=-112(load case 5), 10=-4(load case 4), 13=-173(load case 4), 12=-138(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-14=-92/84, 1-2=-11/24, 2-3=-11/24, 3-4=-11/24, 4-5=-75/39, 5-6=-98/12, 6-7=-96/13, 7-8=-92/84
BOT CHORD 13-14=-14/101, 12-13=-14/101, 11-12=-14/101, 10-11=-14/97, 9-10=-13/96, 8-9=-13/96
WEBS 6-9=-196/236, 4-11=-103/174, 5-10=-128/15, 2-13=-232/201, 3-12=-150/153

JOINT STRESS INDEX
1 = 0.79, 2 = 0.34, 3 = 0.34, 4 = 0.42, 5 = 0.31, 6 = 0.34, 7 = 0.41, 8 = 0.49, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34 and 14 = 0.59

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 14, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 9, 78 lb uplift at joint 14, 77 lb uplift at joint 8, 112 lb uplift at joint 11, 4 lb uplift at joint 10, 173 lb uplift at joint 13 and 138 lb uplift at joint 12.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061122

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB19	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:21:27 2006 Page 1

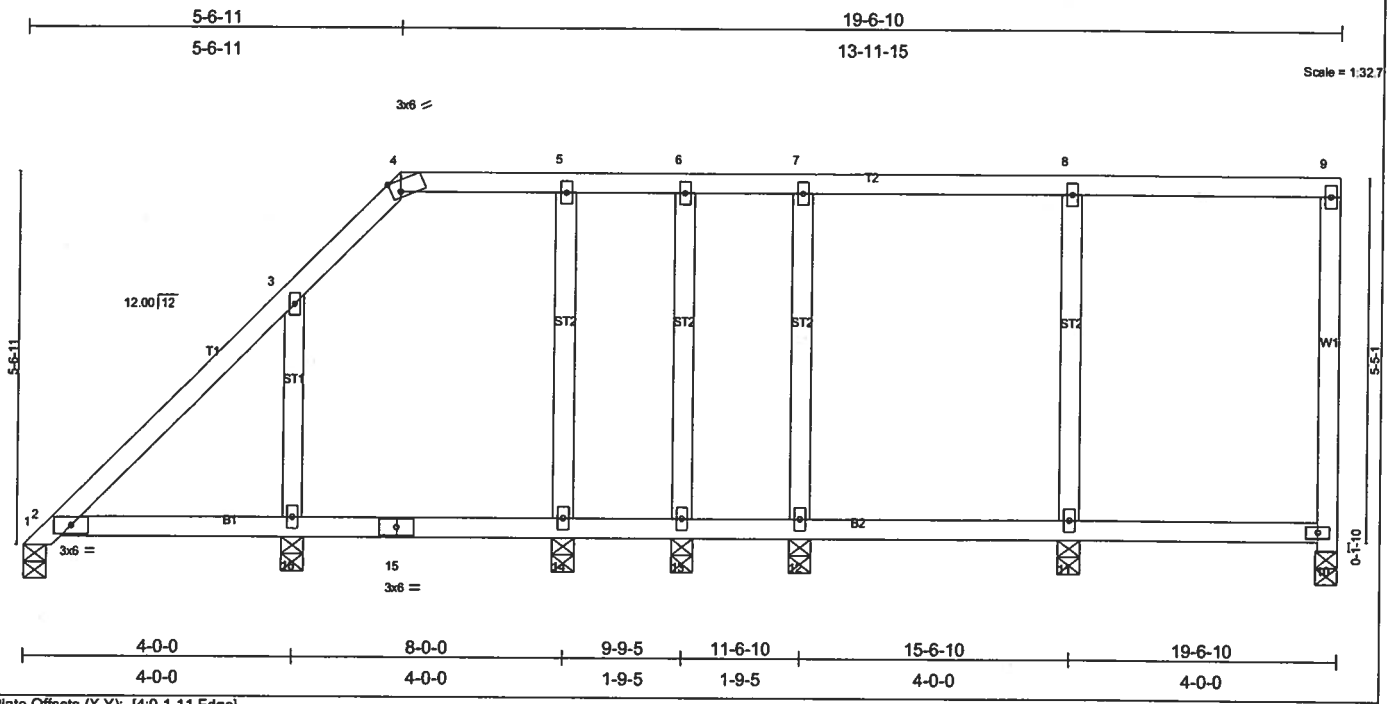


Plate Offsets (X,Y): [4:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	0.01	2-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 102 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=129/0-4-0, 13=62/0-4-0, 11=360/0-4-0, 12=277/0-4-0, 16=375/0-4-0, 14=263/0-4-0, 10=143/0-4-0
Max Horz 1=251(load case 5)
Max Uplift13=-29(load case 3), 11=-168(load case 3), 12=-131(load case 3), 16=-309(load case 5), 14=-127(load case 3), 10=-71(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-275/14, 2-3=-238/67, 3-4=-94/27, 4-5=-11/7, 5-6=-11/7, 6-7=-11/7, 7-8=-11/7, 8-9=-11/7, 9-10=-91/77
BOT CHORD 2-16=-7/11, 15-16=-7/11, 14-15=-7/11, 13-14=-7/11, 12-13=-7/11, 11-12=-7/11, 10-11=-7/11
WEBS 6-13=-42/36, 8-11=-233/199, 7-12=-176/149, 3-16=-223/301, 5-14=-171/144

JOINT STRESS INDEX
2 = 0.37, 3 = 0.34, 4 = 0.25, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.15 and 16 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 13, 168 lb uplift at joint 11, 131 lb uplift at joint 12, 309 lb uplift at joint 16, 127 lb uplift at joint 14 and 71 lb uplift at joint 10.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061123

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB19A	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:22:42 2006 Page 1

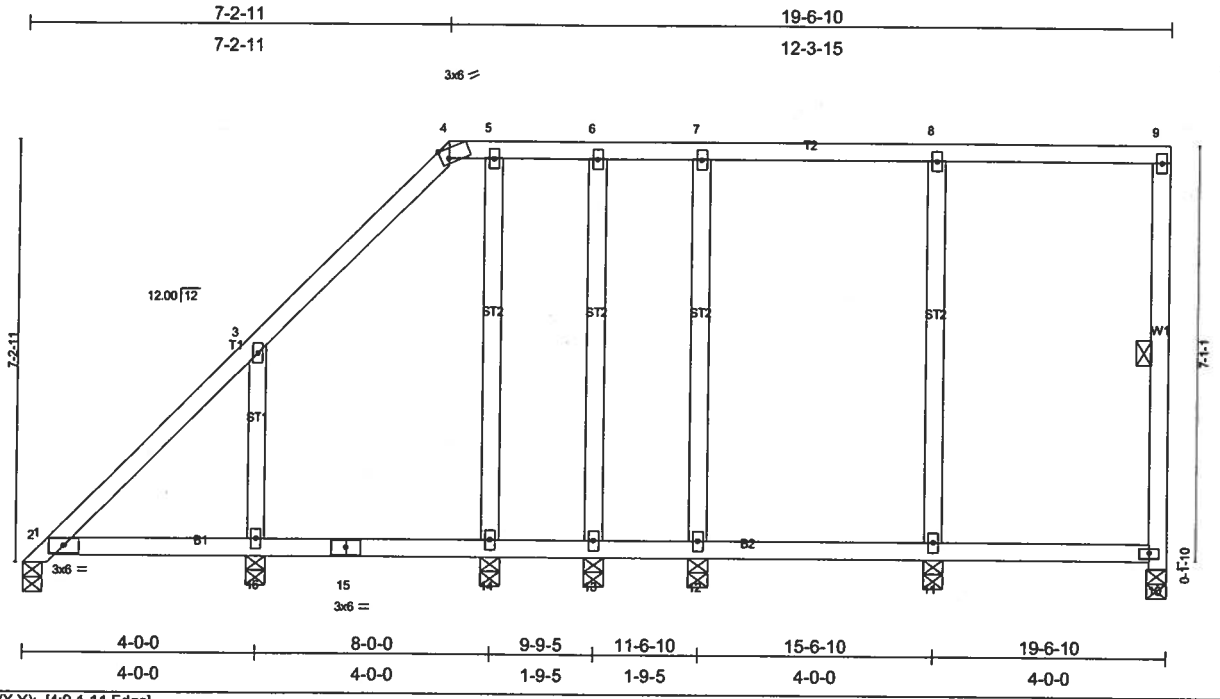


Plate Offsets (X,Y): [4:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	2-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	0.01	2-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.19	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 9-10

REACTIONS (lb/size) 1=124/0-4-0, 13=60/0-4-0, 11=363/0-4-0, 12=275/0-4-0, 16=380/0-4-0, 14=266/0-4-0, 10=141/0-4-0
Max Horz 1=328(load case 5)
Max Uplift13=-66(load case 3), 11=-173(load case 3), 12=-124(load case 3), 16=-347(load case 5), 14=-156(load case 5), 10=-67(load case 3)
Max Grav 1=209(load case 5), 13=60(load case 1), 11=363(load case 1), 12=275(load case 1), 16=380(load case 1), 14=266(load case 1), 10=141(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-387/24, 2-3=-355/73, 3-4=-91/21, 4-5=-7/4, 5-6=-7/4, 6-7=-7/4, 7-8=-7/4, 8-9=-7/4, 9-10=-89/74
BOT CHORD 2-16=-4/7, 15-16=-4/7, 14-15=-4/7, 13-14=-4/7, 12-13=-4/7, 11-12=-4/7, 10-11=-4/7
WEBS 6-13=-39/71, 8-11=-235/202, 7-12=-175/143, 3-16=-230/345, 5-14=-173/202

JOINT STRESS INDEX
2 = 0.31, 3 = 0.34, 4 = 0.25, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.15 and 16 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 13, 173 lb uplift at joint 11, 124 lb uplift at joint 12, 347 lb uplift at joint 16, 156 lb uplift at joint 14 and 67 lb uplift at joint 10.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061124

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB23	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:26:33 2006 Page 1

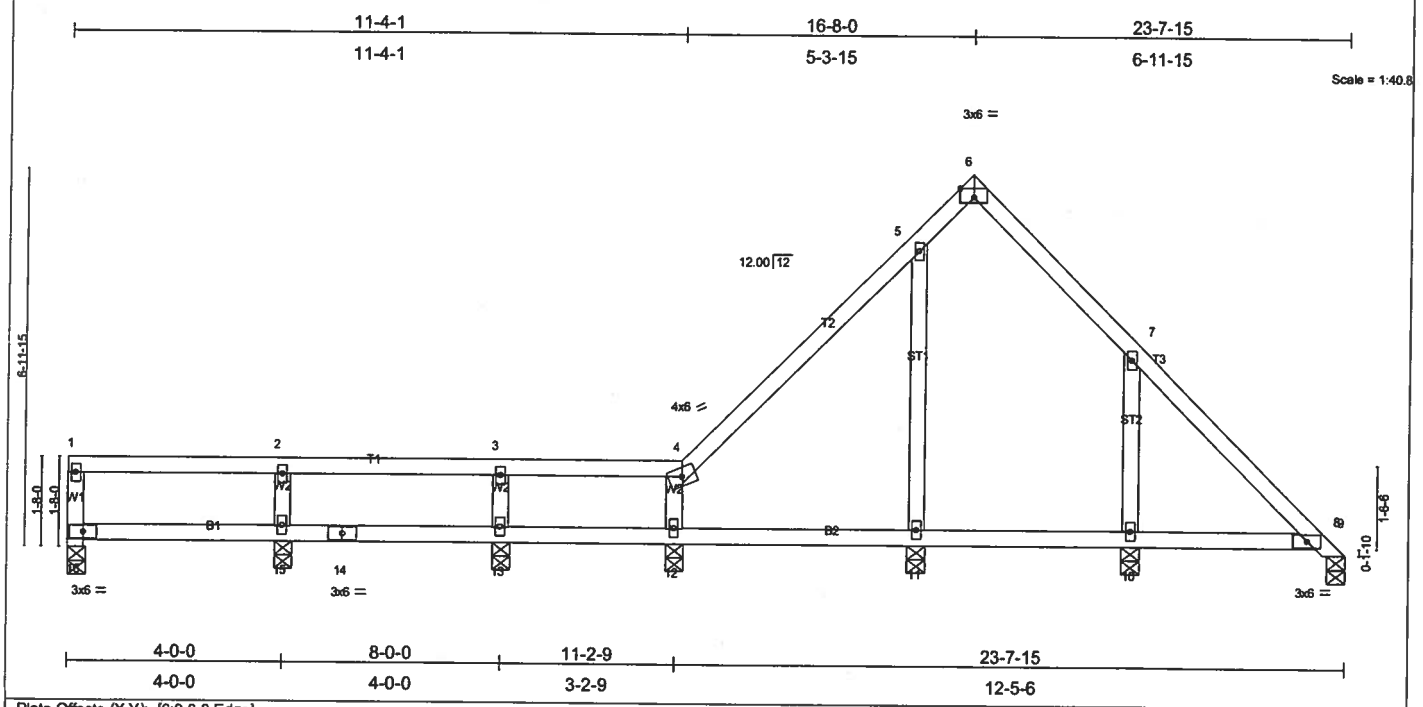


Plate Offsets (X,Y): [6:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.01	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.02	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 9=149/0-4-0, 10=352/0-4-0, 16=124/0-4-0, 11=302/0-4-0, 15=381/0-4-0, 13=280/0-4-0, 12=366/0-4-0
Max Horz 16=-233(load case 3)
Max Uplift9=-16(load case 4), 10=-260(load case 6), 16=-86(load case 3), 11=-89(load case 4), 15=-151(load case 4), 13=-173(load case 3), 12=-100(load case 6)
Max Grav 9=149(load case 1), 10=361(load case 10), 16=124(load case 1), 11=302(load case 1), 15=390(load case 9), 13=280(load case 1), 12=366(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-16=-83/88, 1-2=-143/65, 2-3=-143/65, 3-4=-144/64, 4-5=-125/111, 5-6=-86/97, 6-7=-110/63, 7-8=-174/79, 8-9=-94/14
BOT CHORD 15-16=-46/226, 14-15=-46/226, 13-14=-46/226, 12-13=-46/226, 11-12=-51/212, 10-11=-51/212, 8-10=-51/212
WEBS 7-10=-216/263, 5-11=-178/148, 2-15=-245/208, 3-13=-183/190, 4-12=-248/155

JOINT STRESS INDEX
1 = 0.76, 2 = 0.34, 3 = 0.34, 4 = 0.90, 5 = 0.34, 6 = 0.37, 7 = 0.34, 8 = 0.36, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.15, 15 = 0.34 and 16 = 0.29

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 9, 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 9, 260 lb uplift at joint 10, 86 lb uplift at joint 16, 89 lb uplift at joint 11, 151 lb uplift at joint 15, 173 lb uplift at joint 13 and 100 lb uplift at joint 12.

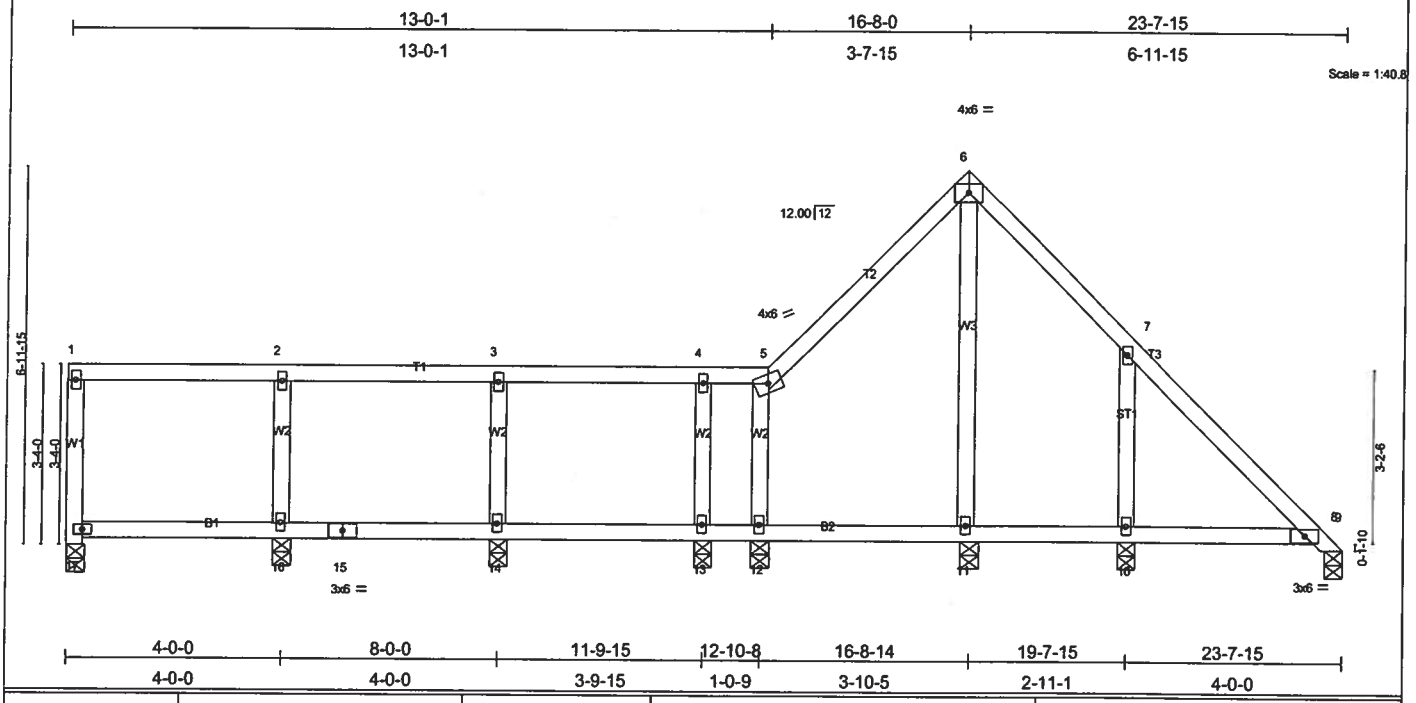
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061125

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB23A	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:29:45 2006 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.13	Vert(LL) 0.01 8-10 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.13	Vert(TL) -0.02 8-10 >999 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.10	Horz(TL) 0.01 9 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 110 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=126/0-4-0, 10=330/0-4-0, 17=144/0-4-0, 16=347/0-4-0, 14=339/0-4-0, 13=188/0-4-0, 12=242/0-4-0, 11=214/0-4-0
Max Horz 17=-216(load case 3)
Max Uplift9=-99(load case 4), 10=-283(load case 6), 17=-78(load case 3), 16=-153(load case 3), 14=-162(load case 3), 13=-150(load case 3), 12=-135(load case 5), 11=-20(load case 3)
Max Grav 9=126(load case 1), 10=339(load case 10), 17=145(load case 9), 16=347(load case 1), 14=339(load case 9), 13=188(load case 1), 12=249(load case 9), 11=214(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-17=-93/82, 1-2=-20/19, 2-3=-20/19, 3-4=-20/19, 4-5=-21/17, 5-6=-112/141, 6-7=-98/162, 7-8=-218/199, 8-9=-81/63
BOT CHORD 16-17=-131/198, 15-16=-131/198, 14-15=-131/198, 13-14=-131/198, 12-13=-131/198, 11-12=-139/200, 10-11=-139/200, 8-10=-139/200
WEBS 7-10=-207/280, 2-16=-224/186, 3-14=-218/185, 4-13=-124/164, 5-12=-172/210, 6-11=-140/42

JOINT STRESS INDEX
1 = 0.40, 2 = 0.34, 3 = 0.34, 4 = 0.34, 5 = 0.61, 6 = 0.35, 7 = 0.34, 8 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.15, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 9, 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 9, 283 lb uplift at joint 10, 78 lb uplift at joint 17, 153 lb uplift at joint 16, 162 lb uplift at joint 14, 150 lb uplift at joint 13, 135 lb uplift at joint 12 and 20 lb uplift at joint 11.

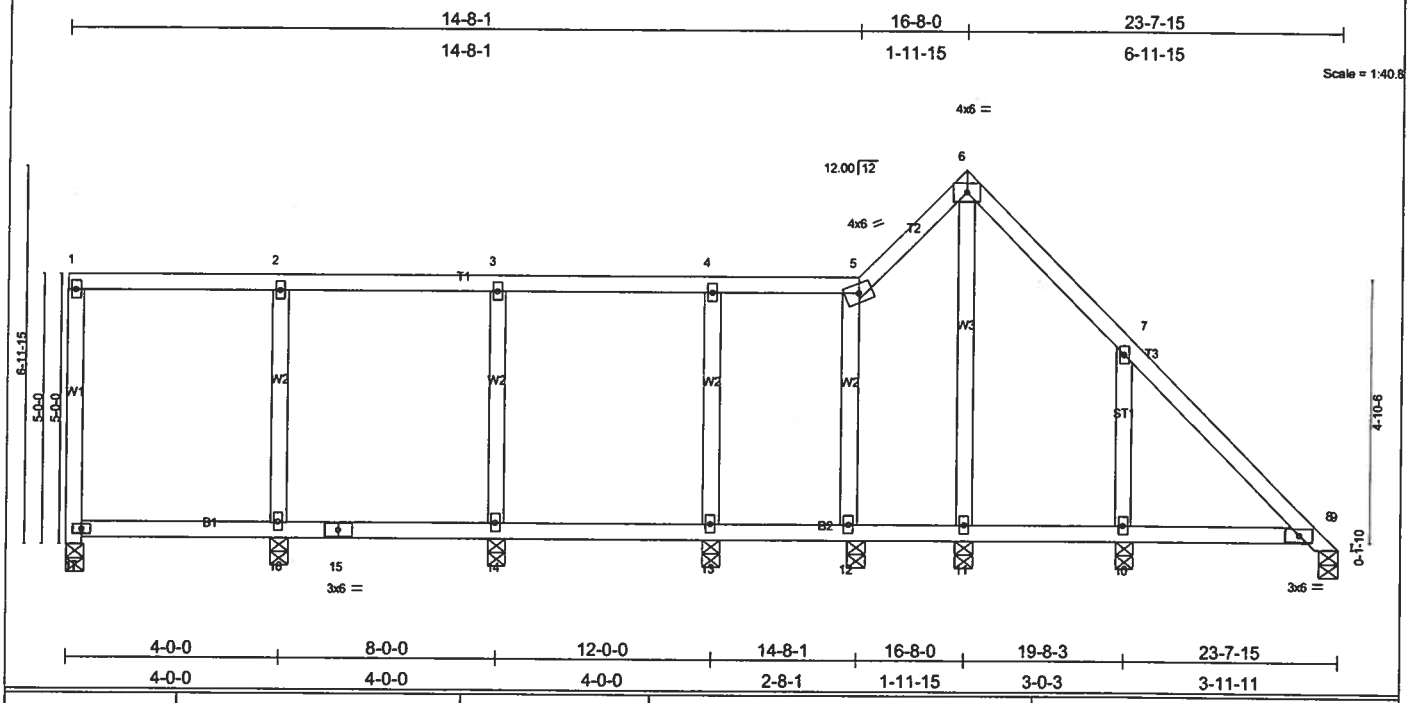
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061126

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB23B	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:32:16 2006 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.12	Vert(LL) 0.01 8-10 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.12	Vert(TL) -0.02 8-10 >999 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.09	Horz(TL) 0.01 9 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 122 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=113/0-4-0, 10=347/0-4-0, 17=143/0-4-0, 11=155/0-4-0, 12=186/0-4-0, 16=347/0-4-0, 14=342/0-4-0, 13=296/0-4-0
Max Horz 17=-238(load case 6)
Max Uplift9=-53(load case 4), 10=-283(load case 6), 17=-73(load case 3), 11=-43(load case 3), 12=-73(load case 4), 16=-159(load case 3), 14=-163(load case 3), 13=-148(load case 3)
Max Grav 9=121(load case 3), 10=348(load case 10), 17=144(load case 9), 11=155(load case 1), 12=193(load case 9), 16=347(load case 1), 14=342(load case 9), 13=296(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-17=-92/79, 1-2=-11/9, 2-3=-11/9, 3-4=-11/9, 4-5=-12/8, 5-6=-63/67, 6-7=-89/99, 7-8=-226/127, 8-9=-78/34
BOT CHORD 16-17=-86/233, 15-16=-86/233, 14-15=-86/233, 13-14=-86/233, 12-13=-86/233, 11-12=-88/231, 10-11=-88/231, 8-10=-88/231
WEBS 7-10=-209/289, 6-11=-94/59, 5-12=-133/112, 2-16=-224/189, 3-14=-219/186, 4-13=-192/169

JOINT STRESS INDEX
1 = 0.34, 2 = 0.34, 3 = 0.34, 4 = 0.34, 5 = 0.31, 6 = 0.35, 7 = 0.34, 8 = 0.32, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.15, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 9, 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 9, 283 lb uplift at joint 10, 73 lb uplift at joint 17, 43 lb uplift at joint 11, 73 lb uplift at joint 12, 159 lb uplift at joint 16, 163 lb uplift at joint 14 and 148 lb uplift at joint 13.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

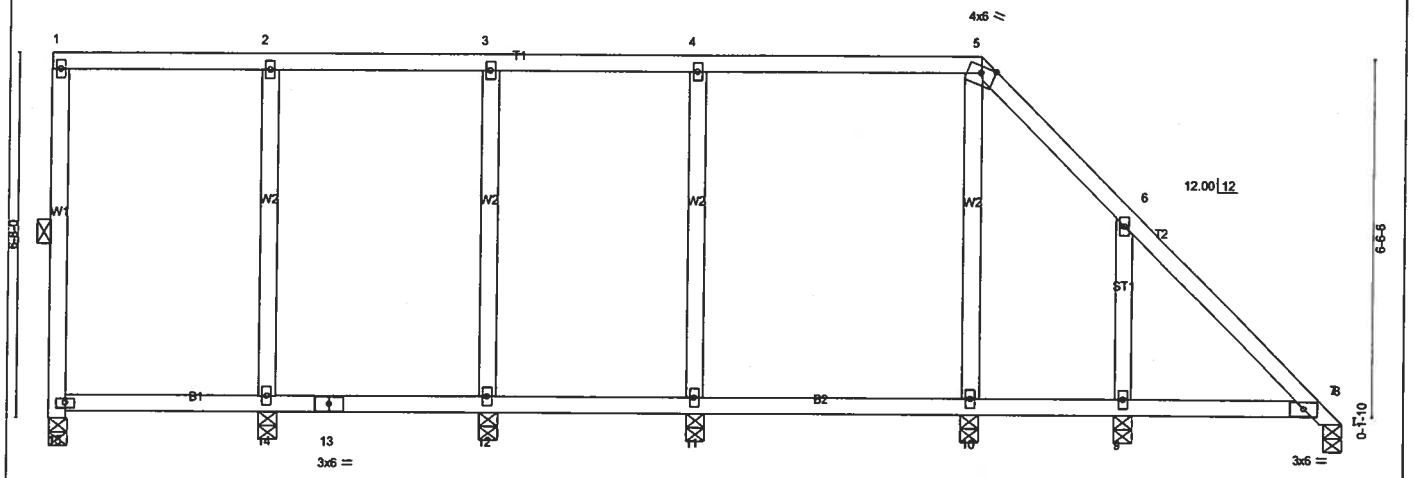
Dwg.#0829061127

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB23C	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:34:59 2006 Page 1

16-11-15 / 16-11-15 — 23-7-15 / 6-8-0

Scale = 1:40.2



4-0-0 / 4-0-0 — 8-0-0 / 4-0-0 — 11-9-15 / 3-9-15 — 16-9-15 / 5-0-0 — 19-7-15 / 2-10-0 — 23-7-15 / 4-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.01	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.02	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 123 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 1-15

REACTIONS (lb/size) 8=116/0-4-0, 9=288/0-4-0, 15=140/0-4-0, 14=360/0-4-0, 12=298/0-4-0, 11=406/0-4-0, 10=321/0-4-0
Max Horz 15=-287(load case 6)
Max Uplift8=-2(load case 4), 9=-259(load case 6), 15=-67(load case 4), 14=-170(load case 4), 12=-140(load case 4), 11=-198(load case 4), 10=-134(load case 3)
Max Grav 8=152(load case 6), 9=288(load case 1), 15=140(load case 1), 14=360(load case 1), 12=298(load case 1), 11=406(load case 1), 10=321(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-15=-89/74, 1-2=-7/5, 2-3=-7/5, 3-4=-7/5, 4-5=-8/4, 5-6=-83/43, 6-7=-292/59, 7-8=-98/1
BOT CHORD 14-15=-26/284, 13-14=-26/284, 12-13=-26/284, 11-12=-26/284, 10-11=-26/284, 9-10=-29/280, 7-9=-29/280
WEBS 6-9=-166/264, 2-14=-233/199, 3-12=-191/161, 4-11=-263/226, 5-10=-206/202

JOINT STRESS INDEX
1 = 0.34, 2 = 0.34, 3 = 0.34, 4 = 0.34, 5 = 0.39, 6 = 0.34, 7 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.15, 14 = 0.34 and 15 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 8, 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 8, 259 lb uplift at joint 9, 67 lb uplift at joint 15, 170 lb uplift at joint 14, 140 lb uplift at joint 12, 198 lb uplift at joint 11 and 134 lb uplift at joint 10.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061128

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	PB33	VALLEY	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 08:52:50 2006 Page 1

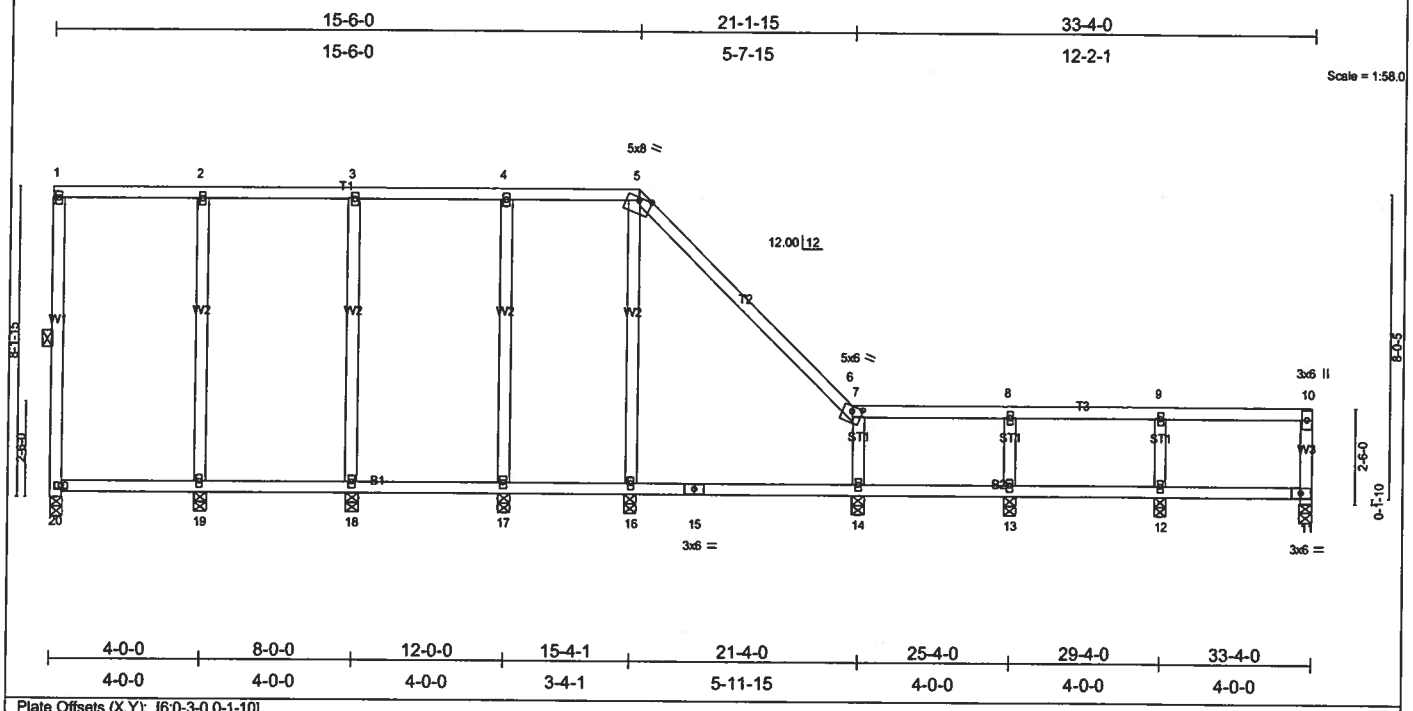


Plate Offsets (X,Y): [6:0-3-0,0-1-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.04	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.04	14-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 1-20

REACTIONS (lb/size) 12=373/0-4-0, 13=287/0-4-0, 14=464/0-4-0, 11=133/0-4-0, 16=417/0-4-0, 19=345/0-4-0, 18=350/0-4-0, 17=263/0-4-0, 20=144/0-4-0
Max Horz 20=-182(load case 5)
Max Uplift12=-229(load case 5), 13=-194(load case 4), 14=-42(load case 5), 11=-77(load case 4), 16=-191(load case 5), 19=-168(load case 4), 18=-152(load case 4), 17=-187(load case 4), 20=-70(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-20=-91/78, 1-2=-7/13, 2-3=-7/13, 3-4=-7/13, 4-5=-8/13, 5-6=-166/93, 6-7=-227/23, 7-8=-227/23, 8-9=-227/23, 9-10=-227/23, 10-11=-87/84
BOT CHORD 19-20=-23/233, 18-19=-23/233, 17-18=-23/233, 16-17=-23/233, 15-16=-23/227, 14-15=-23/227, 13-14=-23/227, 12-13=-23/227, 11-12=-23/227
WEBS 9-12=-238/274, 8-13=-185/214, 7-14=-301/119, 5-16=-263/321, 2-19=-224/196, 3-18=-224/177, 4-17=-172/205

JOINT STRESS INDEX
1 = 0.56, 2 = 0.34, 3 = 0.34, 4 = 0.34, 5 = 0.69, 6 = 0.35, 7 = 0.00, 8 = 0.34, 9 = 0.34, 10 = 0.52, 11 = 0.71, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.15, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34 and 20 = 0.39

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 11, 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 12, 194 lb uplift at joint 13, 42 lb uplift at joint 14, 77 lb uplift at joint 11, 191 lb uplift at joint 16, 168 lb uplift at joint 19, 152 lb uplift at joint 18, 187 lb uplift at joint 17 and 70 lb uplift at joint 20.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061129

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T01	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 07:44:28 2006 Page 1

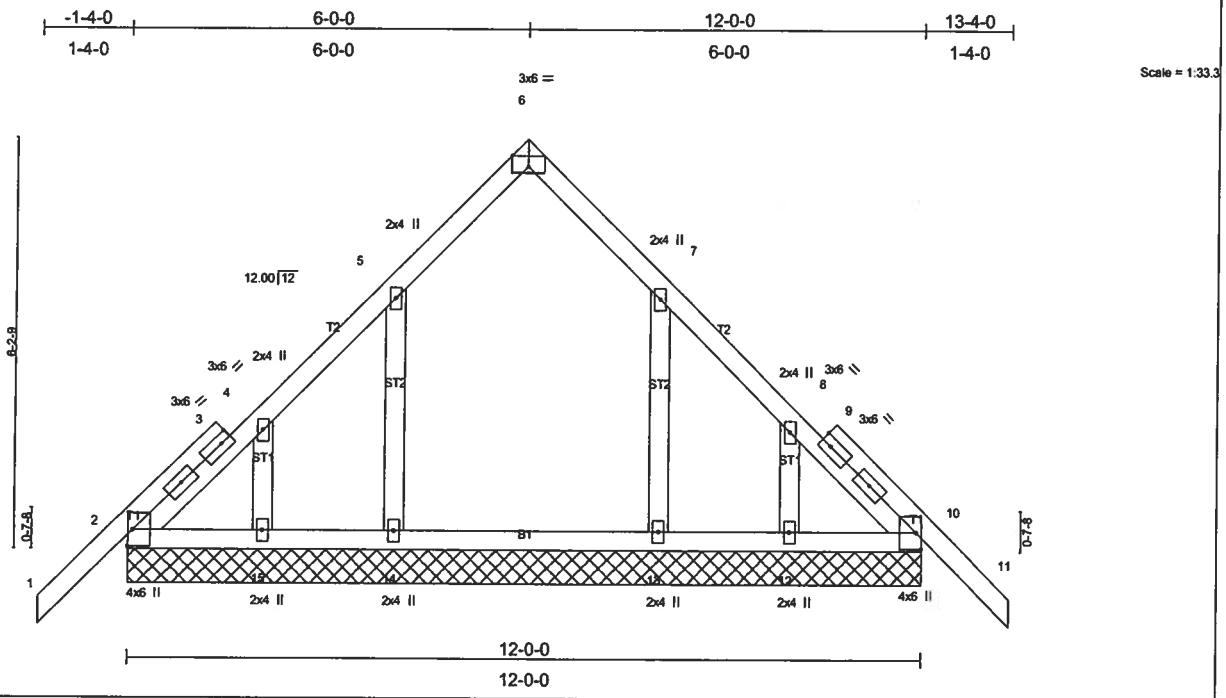

Plate Offsets (X,Y): [6:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.01	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	11	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 70 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=309/12-0-0, 10=309/12-0-0, 14=272/12-0-0, 15=179/12-0-0, 13=272/12-0-0, 12=179/12-0-0
Max Horz 2=209(load case 4)
Max Uplift2=-65(load case 3), 10=-61(load case 4), 14=-166(load case 5), 15=-122(load case 5), 13=-162(load case 6), 12=-124(load case 6)
Max Grav 2=309(load case 1), 10=309(load case 1), 14=276(load case 9), 15=179(load case 1), 13=276(load case 10), 12=179(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1/66, 2-3=-172/71, 3-4=-174/76, 4-5=-159/63, 5-6=-165/74, 6-7=-165/74, 7-8=-159/54, 8-9=-170/70, 9-10=-169/65, 10-11=-1/66
BOT CHORD 2-15=-28/249, 14-15=-28/249, 13-14=-28/249, 12-13=-28/249, 10-12=-28/249
WEBS 5-14=-170/185, 4-15=-143/142, 7-13=-170/180, 8-12=-143/143

JOINT STRESS INDEX
2 = 0.52, 3 = 0.00, 3 = 0.14, 3 = 0.14, 4 = 0.08, 5 = 0.10, 6 = 0.16, 7 = 0.10, 8 = 0.08, 9 = 0.00, 9 = 0.14, 9 = 0.14, 10 = 0.52, 12 = 0.08, 13 = 0.11, 14 = 0.11 and 15 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 2, 61 lb uplift at joint 10, 166 lb uplift at joint 14, 122 lb uplift at joint 15, 162 lb uplift at joint 13 and 124 lb uplift at joint 12.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-79(F=-25), 6-11=-79(F=-25), 2-10=-30

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
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Dwg.#0829061130

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T02	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:56:58 2006 Page 1

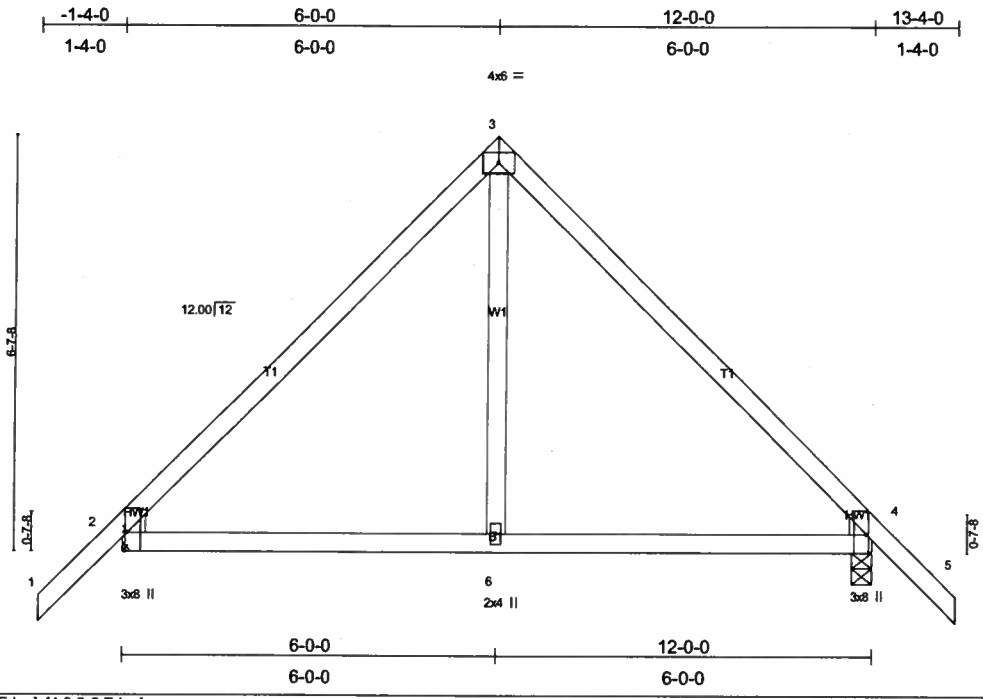

Plate Offsets (X,Y): [2:0-3-8,Edge], [4:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.05	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.07	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 59 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=569/Mechanical, 4=576/0-4-0
Max Horz 2=223(load case 4)
Max Uplift2=-226(load case 5), 4=-232(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-503/182, 3-4=-505/184, 4-5=0/47
BOT CHORD 2-6=-50/275, 4-6=-50/275
WEBS 3-6=-6/230

JOINT STRESS INDEX
2 = 0.57, 2 = 0.00, 3 = 0.71, 4 = 0.57, 4 = 0.00 and 6 = 0.17

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 2 and 232 lb uplift at joint 4.

LOAD CASE(S) Standard

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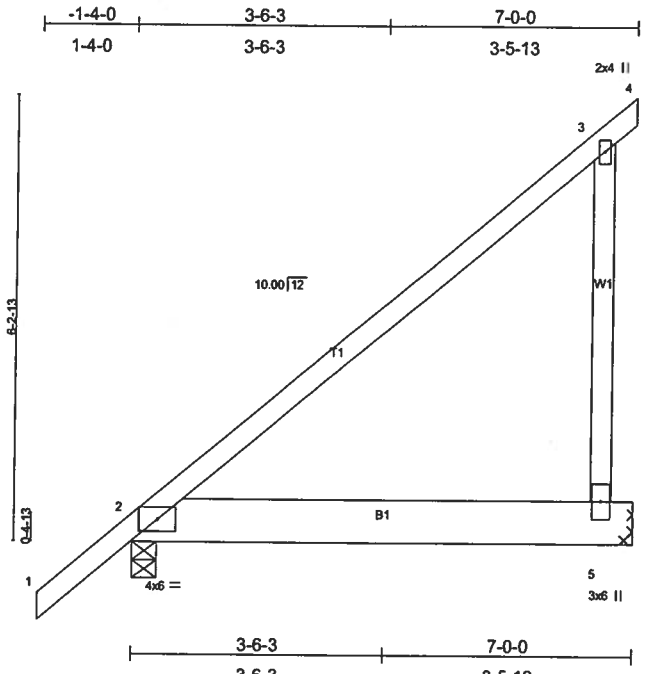
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061131

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T02G	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 08:09:04 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.07	2-5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.11	2-5	>684	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.08	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1010/0-4-0, 5=936/Mechanical
Max Horz 2=340(load case 4)
Max Uplift2=-341(load case 4), 5=-470(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-167/71, 3-4=-15/0
BOT CHORD 2-5=0/0
WEBS 3-5=-168/202

JOINT STRESS INDEX
2 = 0.72, 3 = 0.10 and 5 = 0.07

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 341 lb uplift at joint 2 and 470 lb uplift at joint 5.
5) Girder carries tie-in span(s): 12-0-0 from 0-0-0 to 7-0-0
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 2-5=-234(F=-204)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

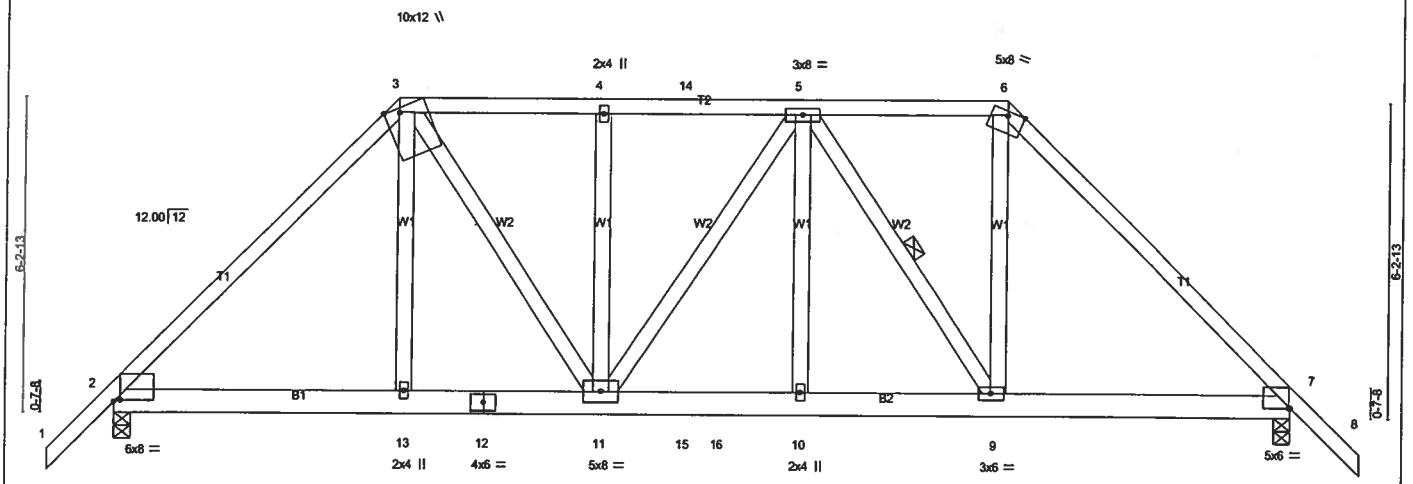
Dwg.#0829061132

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T03	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 08:54:19 2006 Page 1

-1-4-0	2-10-2	5-7-5	9-8-6	13-7-10	17-8-11	20-5-14	23-4-0	24-8-0
1-4-0	2-10-2	2-9-3	4-1-0	3-11-4	4-1-0	2-9-3	2-10-2	1-4-0

Scale = 1:43.6



2-10-2	5-7-5	9-8-6	12-0-0	13-7-10	17-8-11	20-5-14	23-4-0
2-10-2	2-9-3	4-1-0	2-3-10	1-7-10	4-1-0	2-9-3	2-10-2

Plate Offsets (X,Y): [2:0-1-8,0-0-8], [3:0-1-3,Edge], [7:0-0-4,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.11	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.17	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-5 oc bracing.
WEBS 1 Row at midpt 5-9

REACTIONS (lb/size) 2=2183/0-4-0, 7=1835/0-4-0
Max Horz 2=212(load case 3)
Max Uplift2=-1288(load case 3), 7=-879(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-2686/1701, 3-4=-2406/1548, 4-14=-2404/1550, 5-14=-2404/1550, 5-6=-1471/878, 6-7=-2211/1147, 7-8=0/49
BOT CHORD 2-13=-1254/1767, 12-13=-1264/1781, 11-12=-1264/1781, 11-15=-1320/2244, 15-16=-1320/2244, 10-16=-1320/2244, 9-10=-1320/2244, 7-9=-735/1439
WEBS 3-13=-425/577, 3-11=-682/1144, 4-11=-446/648, 5-11=-413/308, 5-10=-289/731, 5-9=-1446/1015, 6-9=-739/1277

JOINT STRESS INDEX
2 = 0.80, 3 = 0.88, 4 = 0.38, 5 = 0.83, 6 = 1.00, 7 = 0.72, 9 = 0.83, 10 = 0.53, 11 = 0.57, 12 = 0.44 and 13 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1288 lb uplift at joint 2 and 879 lb uplift at joint 7.
6) Girder carries hip end with 12-0-0 right side setback, 5-7-5 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 432 lb down and 484 lb up at 5-7-5, and 936 lb down and 444 lb up at 12-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-14=-117(F=-63), 6-14=-54, 6-8=-54, 2-13=-30, 13-15=-65(F=-35), 7-15=-30
Concentrated Loads (lb)
Vert: 13=-432(F) 16=-936(F)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061133

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T04	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 08:54:37 2006 Page 1

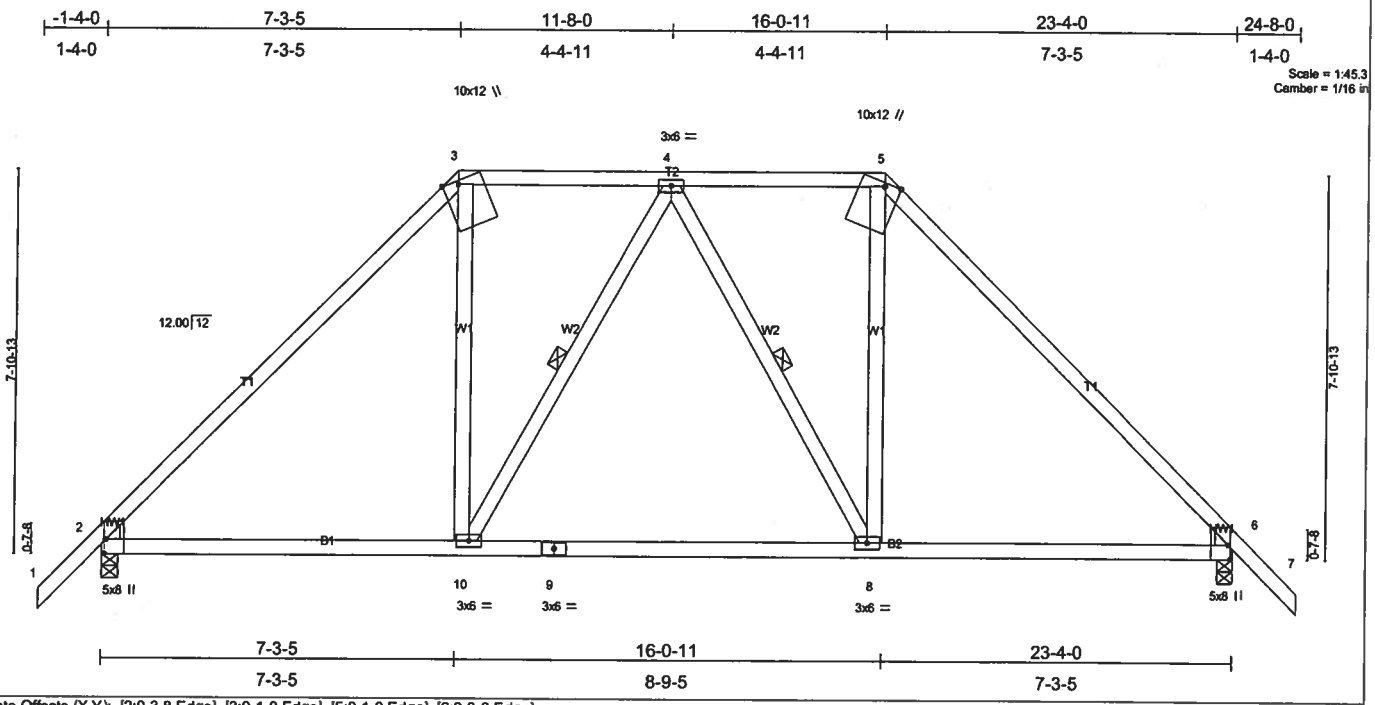


Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-1-0,Edge], [5:0-1-0,Edge], [6:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.12	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.17	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 4-8

REACTIONS (lb/size) 2=1047/0-4-0, 6=1047/0-4-0
Max Horz 2=269(load case 4)
Max Uplift2=-357(load case 5), 6=-357(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1109/397, 3-4=-680/425, 4-5=-680/425, 5-6=-1109/397, 6-7=0/47
BOT CHORD 2-10=-295/672, 9-10=-280/713, 8-9=-280/713, 6-8=-160/672
WEBS 3-10=-139/398, 4-10=-167/242, 4-8=-167/242, 5-8=-139/398

JOINT STRESS INDEX
2 = 0.65, 2 = 0.00, 3 = 0.84, 4 = 0.45, 5 = 0.84, 6 = 0.65, 6 = 0.00, 8 = 0.45, 9 = 0.24 and 10 = 0.45

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 357 lb uplift at joint 2 and 357 lb uplift at joint 6.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061134

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T05	HIP	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:27:55 2006 Page 1

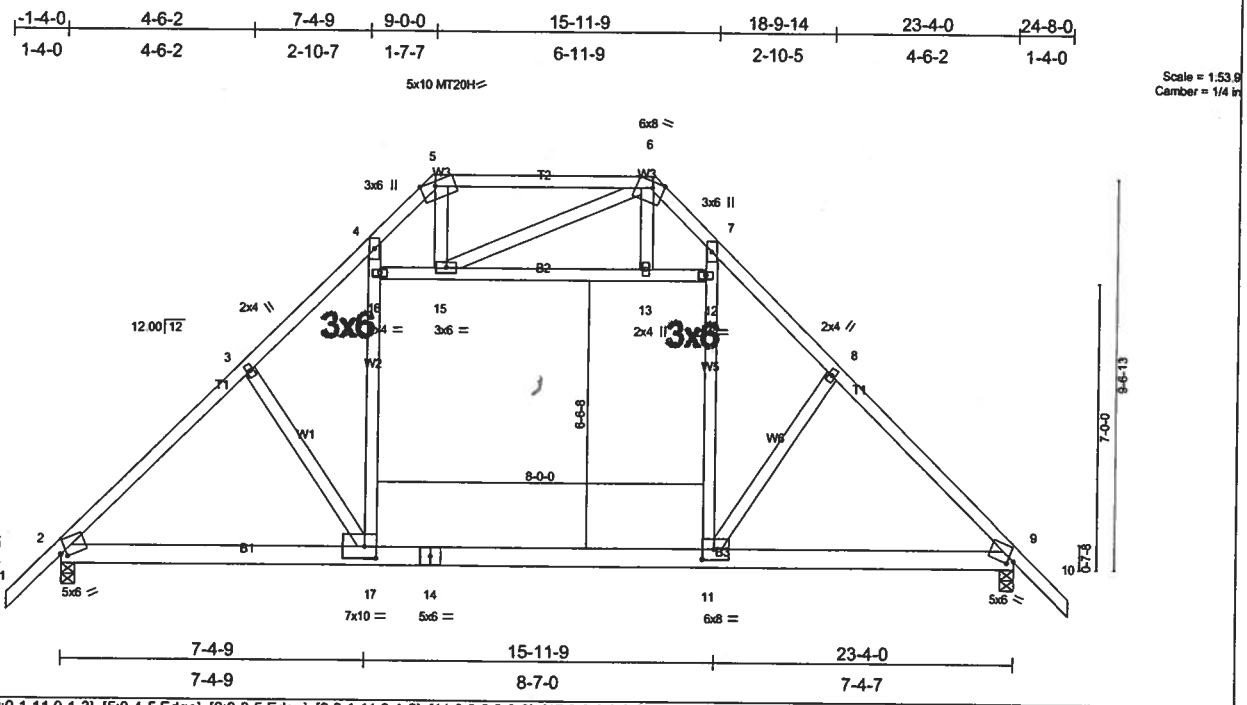


Plate Offsets (X,Y): [2:0-1-11,0-1-3], [5:0-4-5,Edge], [6:0-3-5,Edge], [9:0-1-11,0-1-3], [11:0-3-8,0-3-0], [17:0-3-8,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	-0.42	11-17	>658	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.67	11-17	>409	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.96	Horz(TL)	0.03	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 170 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T2 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.1D, W5 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-10 oc purlins, except 2-0-0 oc purlins (5-1-3 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 9-2-13 oc bracing.
JOINTS 1 Brace at Jt(s): 15, 13

REACTIONS (lb/size) 2=1977/0-4-0, 9=1877/0-4-0
Max Horz 2=325(load case 4)
Max Uplift2=-653(load case 5), 9=-653(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-2508/861, 3-4=-2378/931, 4-5=-990/397, 6-7=-977/401, 7-8=-2347/932, 8-9=-2485/862, 9-10=0/49, 5-6=-965/436
BOT CHORD 2-17=-683/1650, 14-17=-573/1611, 11-14=-573/1611, 9-11=-482/1624, 15-16=-666/357, 13-15=-697/352, 12-13=-712/354
WEBS 3-17=-218/289, 16-17=-519/1320, 4-16=-469/1202, 11-12=-521/1250, 7-12=-469/1134, 8-11=-180/290, 5-15=-75/252, 6-13=-32/241, 6-15=-210/216

JOINT STRESS INDEX
2 = 0.82, 3 = 0.34, 4 = 0.58, 5 = 0.93, 6 = 0.87, 7 = 0.57, 8 = 0.34, 9 = 0.81, 11 = 0.34, 12 = 0.52, 13 = 0.34, 14 = 0.87, 15 = 0.35, 16 = 0.49 and 17 = 0.32

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 653 lb uplift at joint 2 and 653 lb uplift at joint 9.
7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 6-10=-54, 2-17=-30, 11-17=-230(F=-200), 9-11=-10, 12-16=-30, 5-6=-54

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061135

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T06	MONO HIP	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:29:32 2006 Page 1

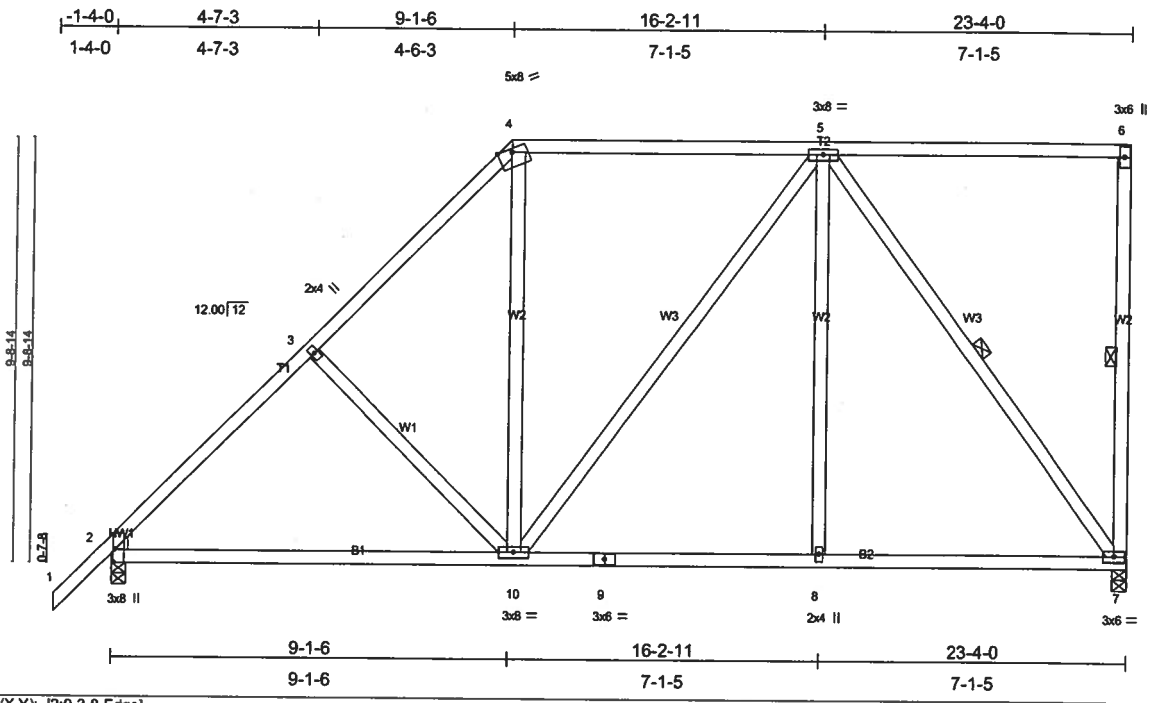


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.16	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.27	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-8-5 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 8-10-6 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7
JOINTS 1 Brace at Jt(s): 6

REACTIONS (lb/size) 7=964/0-4-0, 2=1051/0-4-0
Max Horz 2=504(load case 5)
Max Uplift7=-380(load case 4), 2=-304(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1071/313, 3-4=-919/347, 4-5=-596/339, 5-6=-20/8, 6-7=-165/134
BOT CHORD 2-10=-508/692, 9-10=-265/535, 8-9=-265/535, 7-8=-265/535
WEBS 3-10=-143/269, 4-10=-19/281, 5-10=-161/116, 5-8=0/200, 5-7=-867/433

JOINT STRESS INDEX
2 = 0.77, 2 = 0.00, 3 = 0.34, 4 = 0.56, 5 = 0.58, 6 = 0.31, 7 = 0.43, 8 = 0.34, 9 = 0.21 and 10 = 0.58

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 380 lb uplift at joint 7 and 304 lb uplift at joint 2.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061136

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T07	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:29:57 2006 Page 1

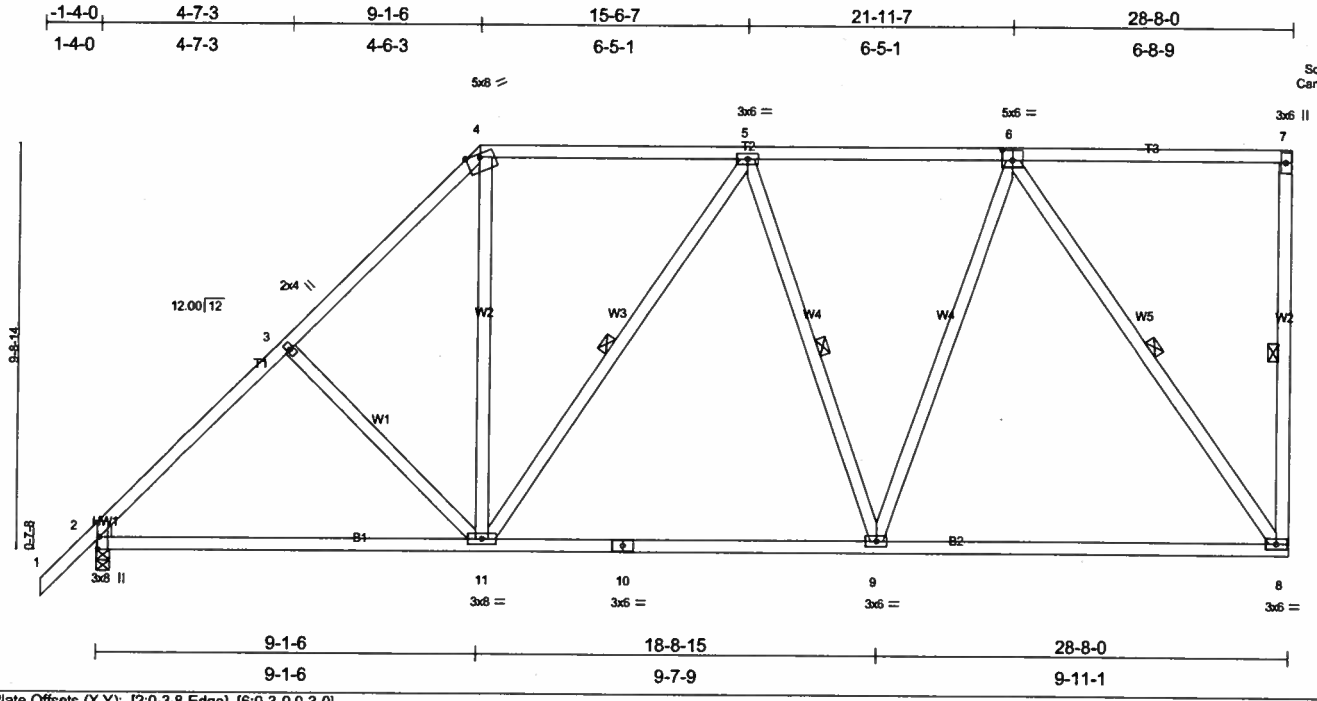


Plate Offsets (X,Y): [2:0-3-8,Edge], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.18	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.32	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-2-2 oc bracing.
WEBS 1 Row at midpt 7-8, 5-11, 5-9, 6-8

REACTIONS (lb/size) 8=1189/Mechanical, 2=1274/0-4-0
Max Horz 2=504(load case 5)
Max Uplift8=-477(load case 3), 2=-372(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1378/449, 3-4=-1226/483, 4-5=-814/436, 5-6=-823/365, 6-7=-28/6, 7-8=-164/132
BOT CHORD 2-11=-599/897, 10-11=-442/898, 9-10=-442/898, 8-9=-309/646
WEBS 3-11=-129/265, 4-11=-120/508, 5-11=-152/231, 5-9=-232/241, 6-9=-172/551, 6-8=-1083/534

JOINT STRESS INDEX
2 = 0.81, 2 = 0.00, 3 = 0.34, 4 = 0.55, 5 = 0.50, 6 = 0.56, 7 = 0.35, 8 = 0.71, 9 = 0.51, 10 = 0.46 and 11 = 0.58

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 477 lb uplift at joint 8 and 372 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061137

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T08	HIP	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:56:39 2006 Page 1

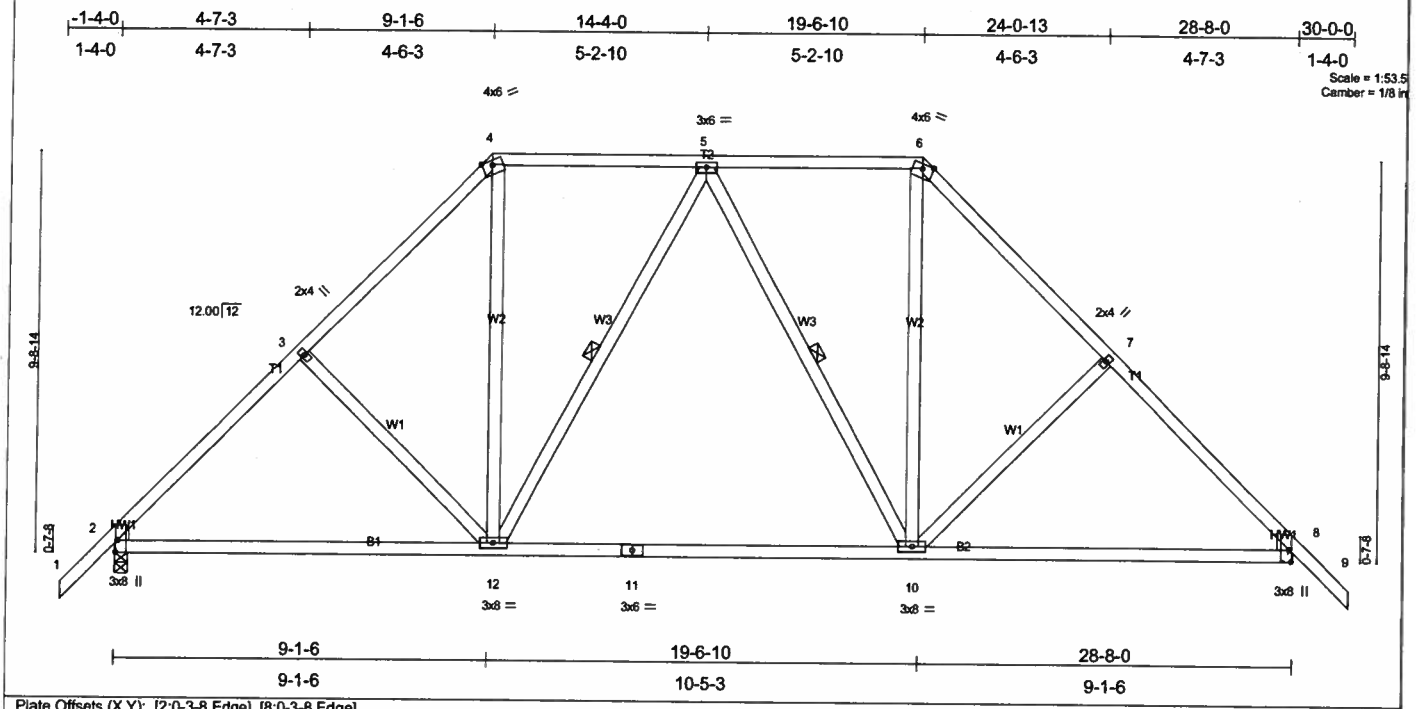


Plate Offsets (X,Y): [2:0-3-8,Edge], [8:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.16	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.27	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.31	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-5-0 oc bracing.
WEBS 1 Row at midpt 5-12, 5-10

REACTIONS (lb/size) 2=1272/0-4-0, 8=1271/Mechanical
Max Horz 2=-333(load case 3)
Max Uplift2=-421(load case 5), 8=-420(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1379/530, 3-4=-1227/563, 4-5=-812/491, 5-6=-813/491, 6-7=-1229/564, 7-8=-1381/530, 8-9=0/47
BOT CHORD 2-12=-448/898, 11-12=-344/886, 10-11=-344/886, 8-10=-174/902
WEBS 3-12=-139/265, 4-12=-195/540, 5-12=-233/272, 5-10=-231/271, 6-10=-196/542, 7-10=-143/267

JOINT STRESS INDEX
2 = 0.79, 2 = 0.00, 3 = 0.34, 4 = 0.67, 5 = 0.45, 6 = 0.67, 7 = 0.34, 8 = 0.79, 8 = 0.00, 10 = 0.61, 11 = 0.49 and 12 = 0.61

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 421 lb uplift at joint 2 and 420 lb uplift at joint 8.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061138

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:35:40 2006 Page 1

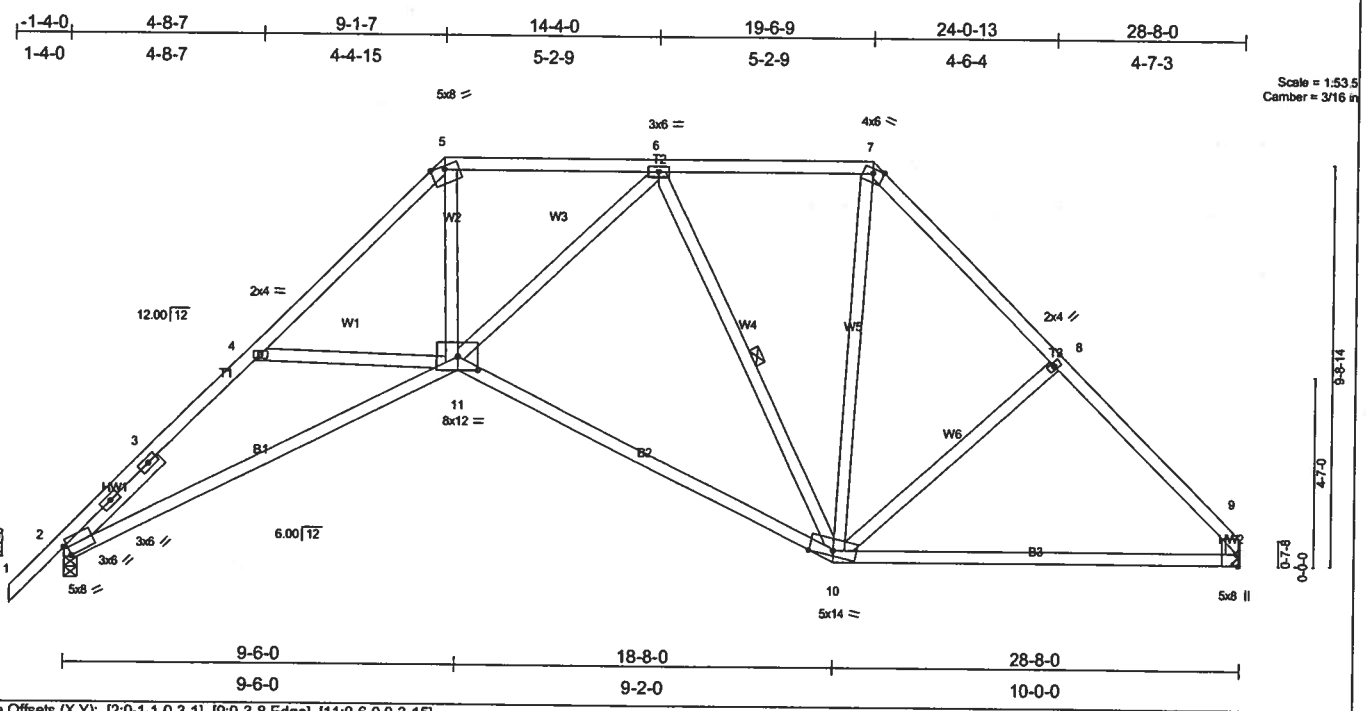


Plate Offsets (X,Y): [2:0-1-1,0-3-1], [9:0-3-8,Edge], [11:0-6-0,0-3-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.23	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.39	9-10	>864	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.21	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 3-3-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-6-6 oc bracing.
WEBS 1 Row at midpt 6-10

REACTIONS (lb/size) 2=1273/0-4-0, 9=1190/Mechanical
Max Horz 2=352(load case 4)
Max Uplift2=-420(load case 5), 9=-334(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-2422/866, 3-4=-2338/879, 4-5=-2180/798, 5-6=-1600/663, 6-7=-841/505, 7-8=-1214/564, 8-9=-1377/548
BOT CHORD 2-11=-938/1826, 10-11=-541/1357, 9-10=-265/909
WEBS 4-11=-116/320, 5-11=-375/1202, 6-11=-319/619, 6-10=-786/522, 7-10=-198/532, 8-10=-161/279

JOINT STRESS INDEX
2 = 0.72, 2 = 0.45, 2 = 0.45, 3 = 0.00, 4 = 0.34, 5 = 1.00, 6 = 0.47, 7 = 0.95, 8 = 0.34, 9 = 0.64, 9 = 0.00, 10 = 0.38 and 11 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 420 lb uplift at joint 2 and 334 lb uplift at joint 9.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061139

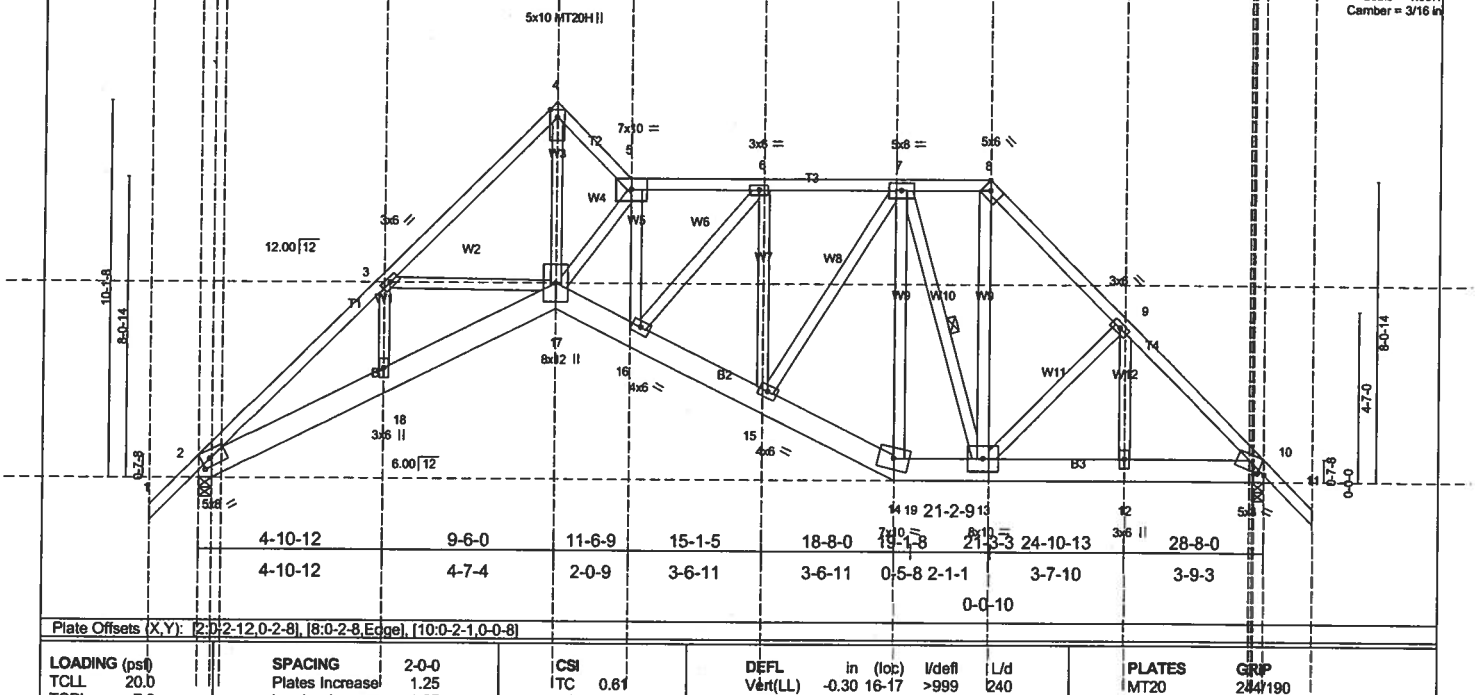
Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:23:40 2006 Page 1

-1-4-0 | 4-10-12 | 9-6-0 | 11-6-9 | 15-1-5 | 18-8-0 | 21-2-9 | 24-10-13 | 28-8-0 | 30-0-0

1-4-0 | 4-10-12 | 4-7-4 | 2-0-9 | 3-6-11 | 3-6-11 | 2-6-9 | 3-8-4 | 3-9-3 | 1-4-0

Scale = 1:59.1
Camber = 3/16 in



4-10-12 | 9-6-0 | 11-6-9 | 15-1-5 | 18-8-0 | 19-1-8 | 21-3-3 | 24-10-13 | 28-8-0

4-10-12 | 4-7-4 | 2-0-9 | 3-6-11 | 3-6-11 | 0-5-8 | 2-1-1 | 3-7-10 | 3-9-3

0-0-10

Plate Offsets (X,Y): [2:0-2-12,0-2-8], [8:0-2-8,Edge], [10:0-2-1,0-0-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.30	16-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.49	16-17	>697	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.97	Horz(TL)	0.37	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 254 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-8-7 oc bracing.
WEBS 1 Row at midpt 7-13

REACTIONS (lb/size) 2=2016/0-4-0, 10=2869/0-4-0
Max Horz 2=-341(load case 2)
Max Uplift2=-705(load case 4), 10=-1095(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-4397/1584, 3-4=-4041/1543, 4-5=-3900/1587, 5-6=-3995/1630, 6-7=-3372/1395, 7-8=-2457/1055, 8-9=-3482/1365, 9-10=-3555/1261, 10-11=0/51
BOT CHORD 2-18=-1459/3282, 17-18=-1459/3290, 16-17=-1833/4486, 15-16=-1554/3753, 14-15=-1301/3151, 14-19=-1162/2805, 13-19=-1155/2791, 12-13=-833/2365, 10-12=-833/2365
WEBS 3-18=0/129, 3-17=-167/276, 4-17=-2124/5264, 5-17=-2164/925, 5-16=-1006/426, 6-16=-370/973, 6-15=-994/483, 7-15=-450/1069, 7-14=-163/353, 7-13=-1194/619, 8-13=-911/2164, 9-13=-187/137, 9-12=-98/124

JOINT STRESS INDEX
2 = 0.89, 3 = 0.47, 4 = 0.88, 5 = 0.90, 6 = 0.67, 7 = 0.54, 8 = 0.90, 9 = 0.47, 10 = 0.83, 12 = 0.16, 13 = 0.41, 14 = 0.55, 15 = 0.61, 16 = 0.52, 17 = 0.90 and 18 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 705 lb uplift at joint 2 and 1095 lb uplift at joint 10.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1903 lb down and 903 lb up at 19-1-8, and 440 lb down and 209 lb up at 21-3-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 5-8=-54, 8-11=-54, 2-17=-30, 14-17=-30, 10-14=-30
Concentrated Loads (lb)
Vert: 13=-440(F) 19=-1903(F)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061140

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T11	SCISSORS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:56:15 2006 Page 1

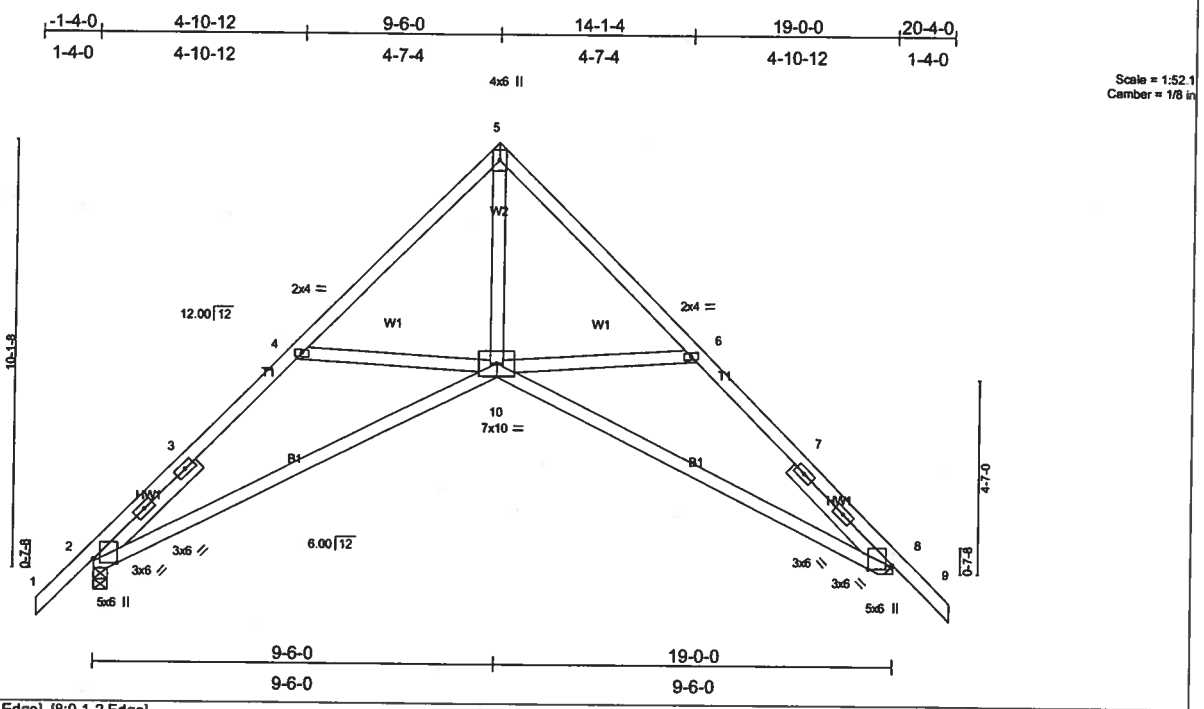


Plate Offsets (X,Y): [2:0-1-2,Edge], [8:0-1-2,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.15	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.25	8-10	>905	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.15	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 109 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 3-4-13, Right 2 X 4 SYP No.3 3-4-13

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-4-4 oc bracing.

REACTIONS (lb/size) 2=866/0-4-0, 8=866/Mechanical
Max Horz 2=347(load case 4)
Max Uplift2=-313(load case 5), 8=-313(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1451/417, 3-4=-1364/443, 4-5=-1164/246, 5-6=-1164/285, 6-7=-1364/394, 7-8=-1451/368, 8-9=0/44
BOT CHORD 2-10=-454/1105, 8-10=-175/1105
WEBS 4-10=-198/346, 5-10=-200/1239, 6-10=-198/366

JOINT STRESS INDEX
2 = 0.90, 2 = 0.27, 2 = 0.27, 3 = 0.00, 4 = 0.34, 5 = 0.57, 6 = 0.34, 7 = 0.00, 8 = 0.90, 8 = 0.27, 8 = 0.27 and 10 = 0.83

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2 and 313 lb uplift at joint 8.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061141

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T12	SCISSOR	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:36:46 2006 Page 1

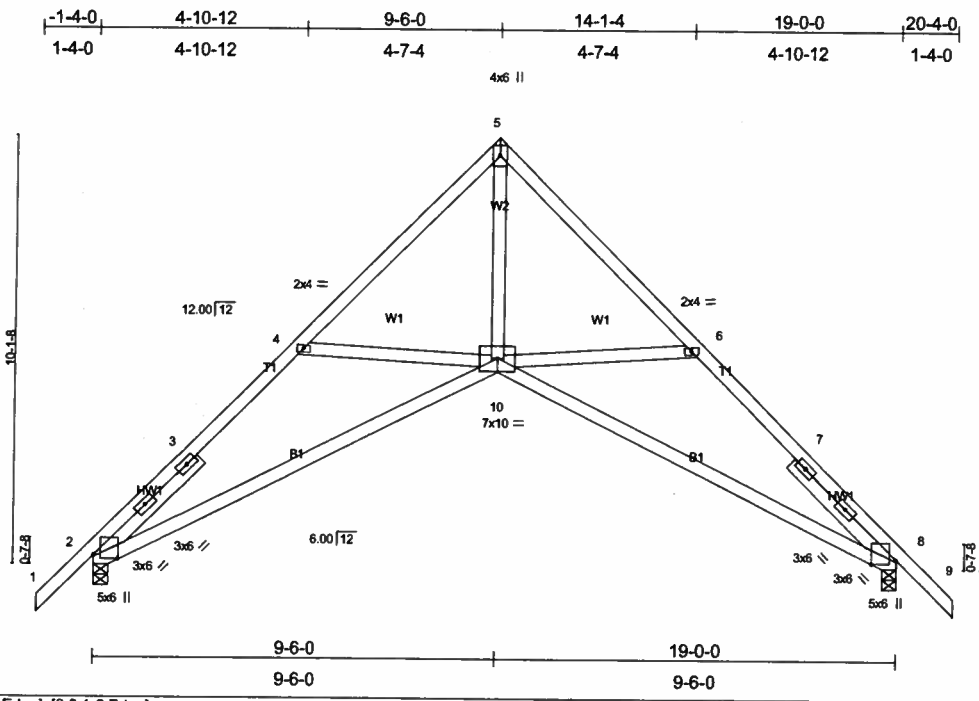


Plate Offsets (X,Y): [2:0-1-2,Edge], [8:0-1-2,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.15	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.25	2-10	>905	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.15	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 109 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 3-4-13, Right 2 X 4 SYP No.3 3-4-13

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-4-4 oc bracing.

REACTIONS (lb/size) 2=866/0-4-0, 8=866/0-4-0
Max Horz 2=-347(load case 3)
Max Uplift2=-313(load case 5), 8=-313(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-1452/417, 3-4=-1365/443, 4-5=-1164/246, 5-6=-1164/285, 6-7=-1365/394, 7-8=-1452/368, 8-9=0/44
BOT CHORD 2-10=-454/1106, 8-10=-175/1106
WEBS 4-10=-199/346, 5-10=-200/1240, 6-10=-199/366

JOINT STRESS INDEX
2 = 0.91, 2 = 0.27, 2 = 0.27, 3 = 0.00, 4 = 0.34, 5 = 0.57, 6 = 0.34, 7 = 0.00, 8 = 0.91, 8 = 0.27, 8 = 0.27 and 10 = 0.83

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2 and 313 lb uplift at joint 8.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061142

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T13	SCISSOR	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:38:49 2006 Page 1

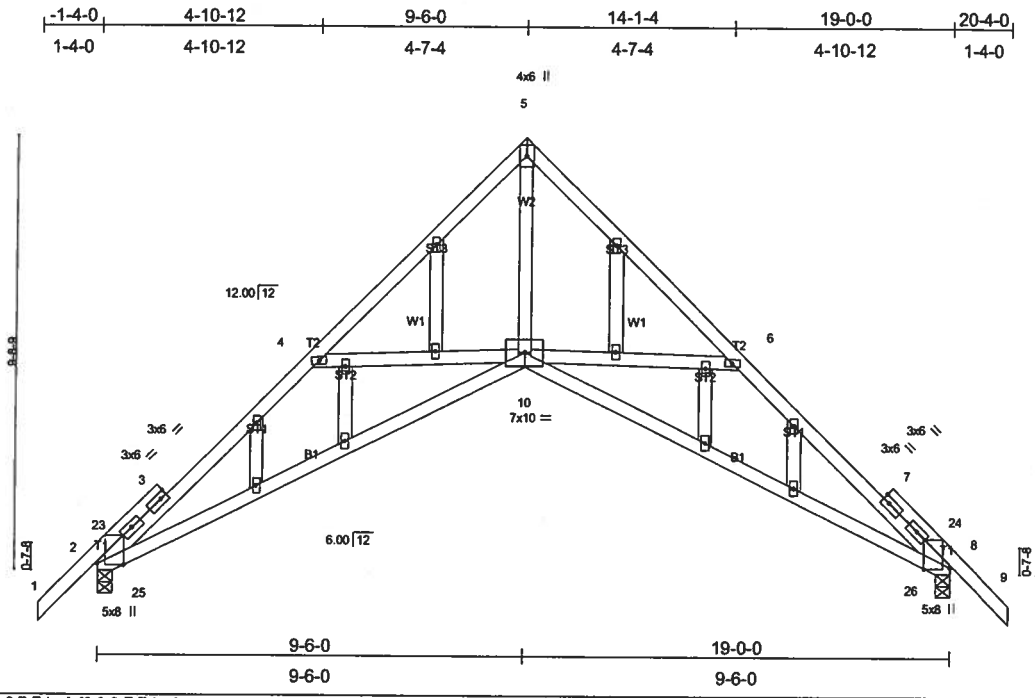

Plate Offsets (X,Y): [2:0-0-7,Edge], [8:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.20	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.33	8-10	>678	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.56	Horz(TL)	0.24	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 121 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-10 oc bracing.

REACTIONS (lb/size) 2=1108/0-4-0, 8=1108/0-4-0
Max Horz 2=-333(load case 3)
Max Uplift2=-415(load case 5), 8=-415(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1/64, 2-23=-2148/686, 3-23=-2113/685, 3-4=-2059/681, 4-5=-1639/370, 5-6=-1639/409, 6-7=-2059/617, 7-24=-2113/620, 8-24=-2148/620, 8-9=-1/64
BOT CHORD 2-25=-566/1573, 10-25=-565/1694, 10-26=-388/1694, 8-26=-389/1573
WEBS 4-10=-410/434, 5-10=-348/1742, 6-10=-410/441

JOINT STRESS INDEX
2 = 0.91, 3 = 0.00, 3 = 0.23, 3 = 0.43, 4 = 0.34, 5 = 0.62, 6 = 0.34, 7 = 0.00, 7 = 0.43, 7 = 0.23, 8 = 0.91, 10 = 0.87, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34 and 22 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 415 lb uplift at joint 2 and 415 lb uplift at joint 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-79(F=-25), 2-23=-39(F=-25), 5-23=-79(F=-25), 5-24=-79(F=-25), 8-24=-39(F=-25), 8-9=-79(F=-25), 2-25=-10, 10-25=-30, 10-26=-30, 8-26=-10

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061143

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T14	MONO HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Sep 07 17:01:15 2006 Page 1

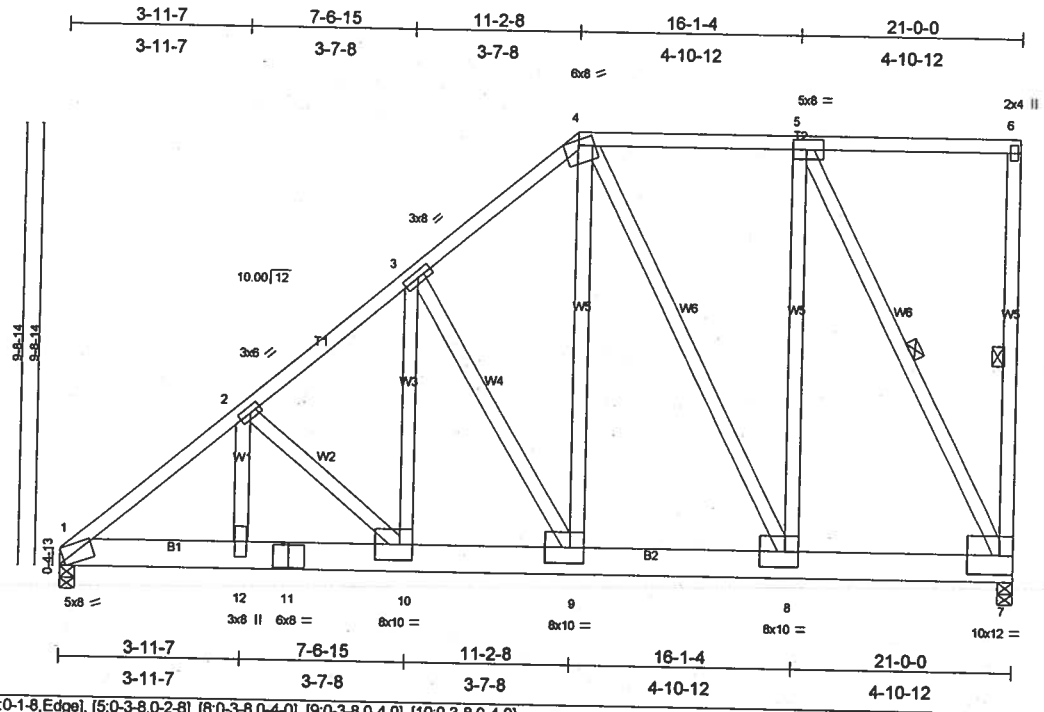

Plate Offsets (X,Y): [1:0-1-8,Edge], [5:0-3-8,0-2-8], [8:0-3-8,0-4-0], [9:0-3-8,0-4-0], [10:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.12	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.20	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 400 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2, W6 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-12 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7
JOINTS 1 Brace at Jt(s): 6

REACTIONS (lb/size) 1=6589/0-4-0, 7=6589/0-4-0
Max Horz 1=428(load case 4)
Max Uplift1=-2914(load case 4), 7=-3038(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8376/3675, 2-3=-6667/2963, 3-4=-4843/2215, 4-5=-2733/1271, 5-6=-25/13, 6-7=-126/98
BOT CHORD 1-12=-3155/6361, 11-12=-3155/6361, 10-11=-3155/6361, 9-10=-2479/5071, 8-9=-1796/3757, 7-8=-1271/2732
WEBS 2-12=-930/2075, 2-10=-1721/902, 3-10=-1523/3260, 3-9=-2681/1386, 4-9=-2274/4770, 4-8=-2194/1124, 5-8=-2360/5039, 5-7=-5946/2767

JOINT STRESS INDEX
1 = 0.80, 2 = 0.81, 3 = 0.99, 4 = 0.95, 5 = 0.86, 6 = 0.50, 7 = 0.48, 8 = 0.53, 9 = 0.50, 10 = 0.34, 11 = 0.70 and 12 = 0.34

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2914 lb uplift at joint 1 and 3038 lb uplift at joint 7.
7) Girder carries tie-in span(s): 26-8-0 from 0-0-0 to 21-0-0
8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 1-7=-583(F=-553)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16106 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061144

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T15	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 09:55:58 2006 Page 1

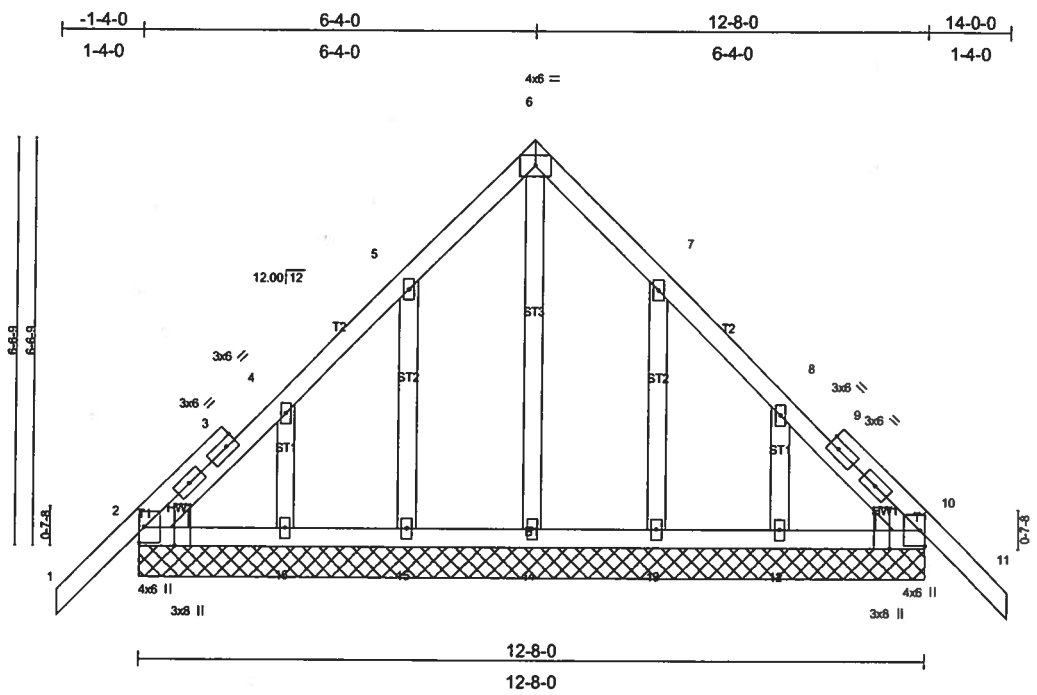

Plate Offsets (X,Y): [2:0-3-8,Edge], [7:0-0-0,0-0-0], [8:0-0-0,0-0-0], [10:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.01	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	11	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 84 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=131/12-8-0, 10=285/12-8-0, 14=161/12-8-0, 15=209/12-8-0, 16=259/12-8-0, 13=219/12-8-0, 12=210/12-8-0
Max Horz 2=-238(load case 3)
Max Uplift2=-52(load case 3), 10=-88(load case 6), 15=-167(load case 5), 16=-195(load case 5), 13=-178(load case 6), 12=-136(load case 6)
Max Grav 2=131(load case 1), 10=285(load case 1), 14=161(load case 1), 15=214(load case 9), 16=259(load case 1), 13=224(load case 10), 12=210(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-0/0, 2-3=-182/113, 3-4=-178/117, 4-5=-100/112, 5-6=-97/150, 6-7=-98/150, 7-8=-97/55, 8-9=-116/72, 9-10=-120/66, 10-11=-1/73
BOT CHORD 2-16=-27/216, 15-16=-27/215, 14-15=-27/215, 13-14=-27/215, 12-13=-27/215, 10-12=-28/215
WEBS 6-14=-112/0, 5-15=-159/181, 4-16=-186/196, 7-13=-164/186, 8-12=-152/148

JOINT STRESS INDEX
2 = 0.62, 2 = 0.09, 3 = 0.00, 3 = 0.16, 3 = 0.16, 4 = 0.11, 5 = 0.10, 6 = 0.11, 7 = 0.10, 8 = 0.11, 9 = 0.00, 9 = 0.16, 9 = 0.16, 10 = 0.62, 10 = 0.14, 12 = 0.12, 13 = 0.11, 14 = 0.04, 15 = 0.11 and 16 = 0.12

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 2, 88 lb uplift at joint 10, 167 lb uplift at joint 15, 195 lb uplift at joint 16, 178 lb uplift at joint 13 and 136 lb uplift at joint 12.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-6=-79(F=-25), 6-11=-79(F=-25), 2-10=-30

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061145

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T16	HIP	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Sep 07 17:01:22 2006 Page 1

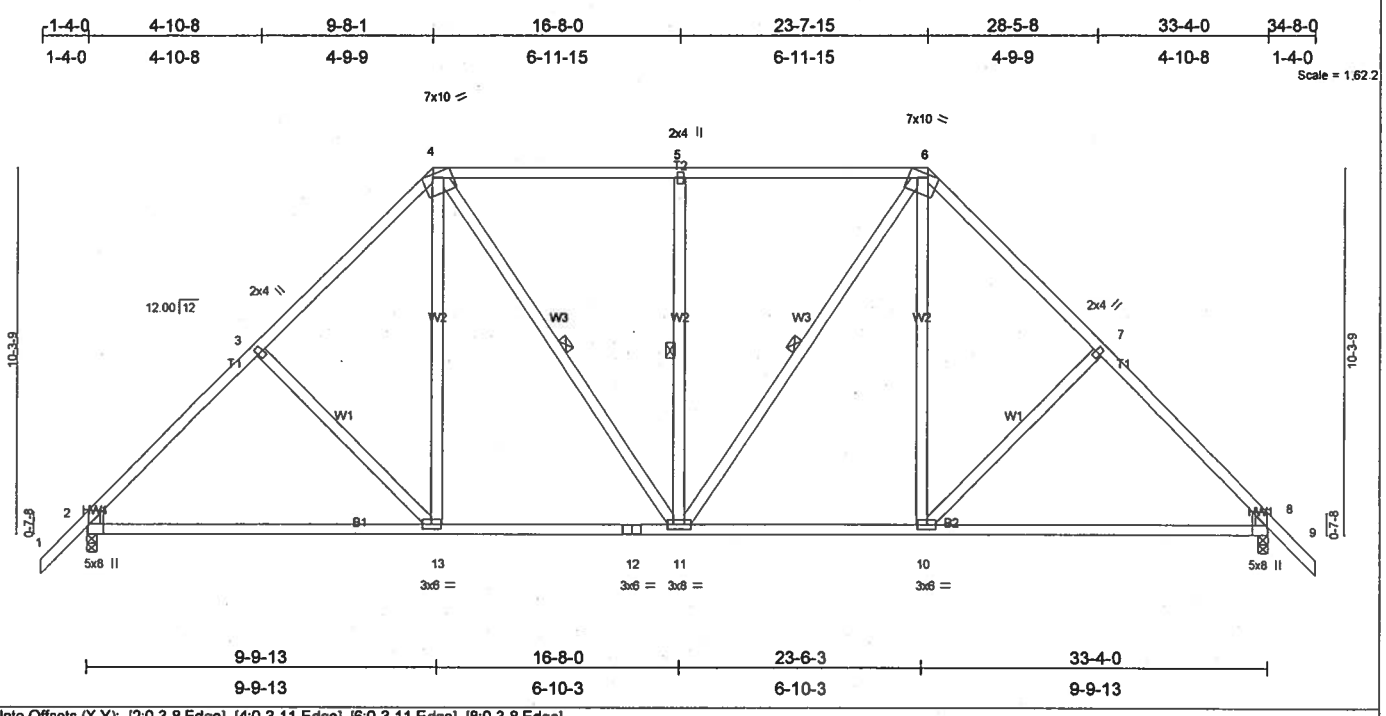


Plate Offsets (X,Y): [2:0-3-8,Edge], [4:0-3-11,Edge], [6:0-3-11,Edge], [8:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	0.45	2-13	>886	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.39	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 217 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-1 oc purlins, except 2-0-0 oc purlins (5-1-3 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 5-10-2 oc bracing.
WEBS 1 Row at midpt 5-11, 4-11, 6-11

REACTIONS (lb/size) 2=1880/0-3-8, 8=1918/0-3-8
Max Horz 2=-352(load case 3)
Max Uplift2=-1041(load case 5), 8=-1059(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2150/1560, 3-4=-1913/1563, 4-5=-1522/1400, 5-6=-1522/1400, 6-7=-1911/1562, 7-8=-2148/1559, 8-9=-3/68
BOT CHORD 2-13=-1057/1405, 12-13=-917/1265, 11-12=-917/1265, 10-11=-770/1264, 8-10=-903/1401
WEBS 4-13=-561/432, 6-10=-559/427, 5-11=-585/434, 3-13=-211/316, 4-11=-468/521, 6-11=-470/524, 7-10=-207/314

JOINT STRESS INDEX
2 = 0.76, 2 = 0.00, 3 = 0.34, 4 = 0.73, 5 = 0.34, 6 = 0.73, 7 = 0.34, 8 = 0.76, 8 = 0.00, 10 = 0.37, 11 = 0.58, 12 = 0.45 and 13 = 0.37

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1041 lb uplift at joint 2 and 1059 lb uplift at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-79(F=-25), 4-6=-79(F=-25), 6-9=-79(F=-25), 2-8=-30

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061146

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T16G	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:30:59 2006 Page 1

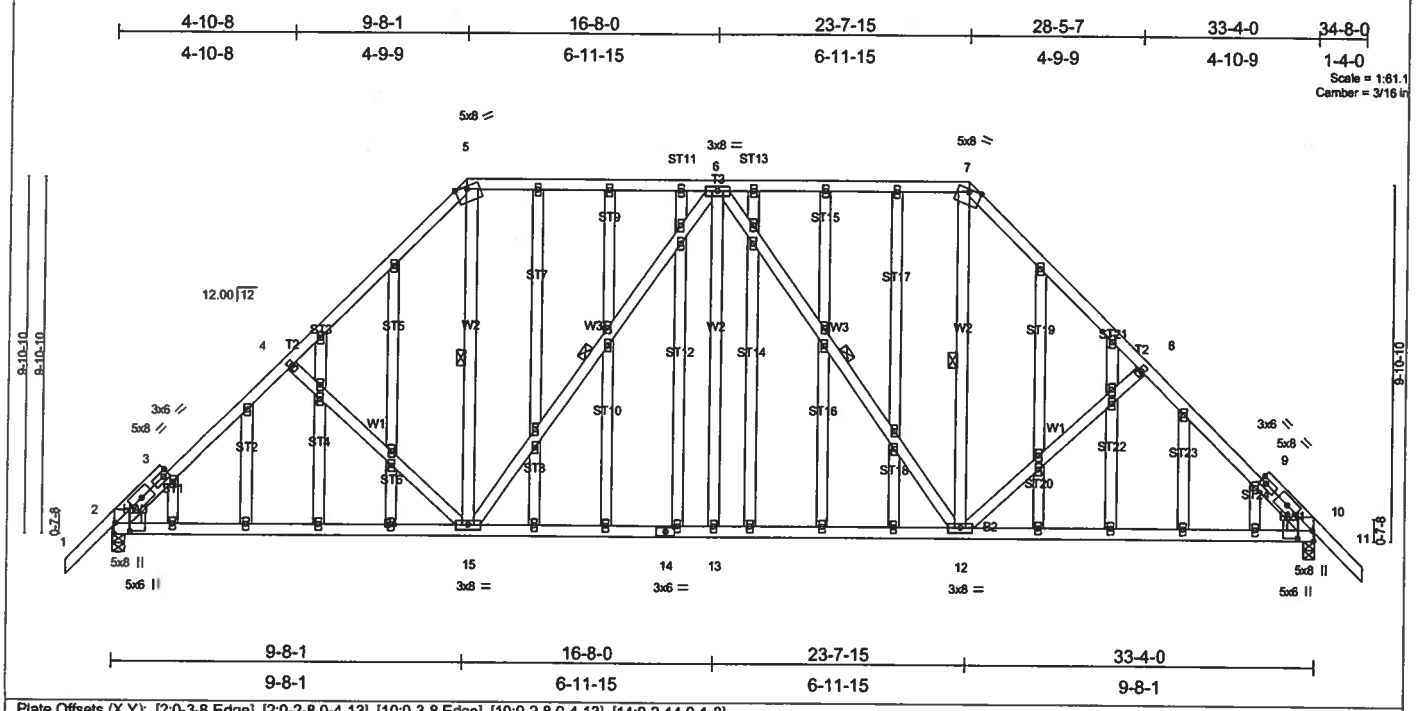


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-2-8,0-4-13], [10:0-3-8,Edge], [10:0-2-8,0-4-13], [14:0-2-14,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.45	2-15	>875	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.42	2-15	>945	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.38	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 351 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-4 oc bracing.
WEBS 1 Row at midpt 5-15, 6-15, 6-12, 7-12

REACTIONS (lb/size) 2=1883/0-4-0, 10=1920/0-4-0
Max Horz 2=333(load case 4)
Max Uplift2=-1048(load case 4), 10=-1066(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-2255/1644, 3-4=-2205/1645, 4-5=-2013/1625, 5-6=-1337/1258, 6-7=-1336/1258, 7-8=-2010/1624, 8-9=-2202/1644, 9-10=-2252/1643, 10-11=-3/73
BOT CHORD 2-15=-1152/1593, 14-15=-1080/1597, 13-14=-1080/1597, 12-13=-1080/1597, 10-12=-1051/1588
WEBS 4-15=-364/401, 5-15=-876/874, 6-15=-528/467, 6-13=-203/156, 6-12=-530/467, 7-12=-876/871, 8-12=-359/399

JOINT STRESS INDEX
2 = 0.57, 2 = 0.60, 3 = 0.59, 3 = 0.34, 3 = 0.00, 3 = 0.34, 4 = 0.34, 5 = 0.84, 6 = 0.58, 7 = 0.83, 8 = 0.34, 9 = 0.58, 9 = 0.34, 9 = 0.35, 9 = 0.00, 10 = 0.59, 10 = 0.61, 12 = 0.58, 13 = 0.34, 14 = 0.57, 15 = 0.58, 16 = 0.34, 17 = 0.34, 18 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 24 = 0.34, 25 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.34, 44 = 0.34, 45 = 0.34, 45 = 0.34, 46 = 0.34, 47 = 0.34, 48 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.34 and 51 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1048 lb uplift at joint 2 and 1066 lb uplift at joint 10.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-5=-79(F=-25), 5-7=-79(F=-25), 7-11=-79(F=-25), 2-10=-30

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061147

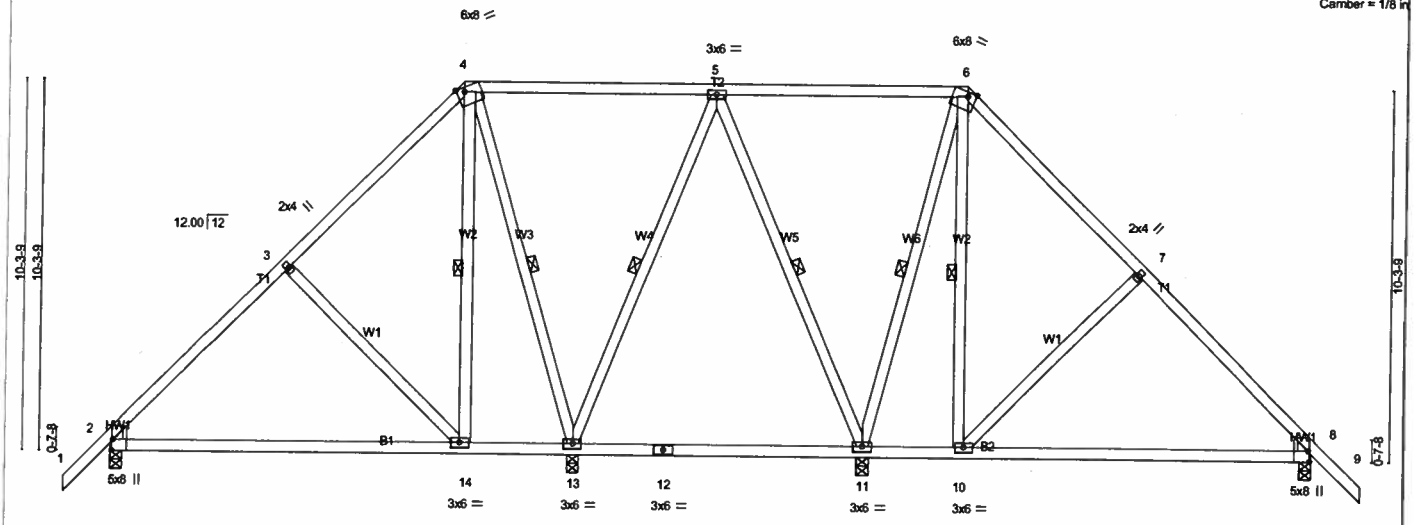
Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:30:45 2006 Page 1

1-4-0 | 4-10-8 | 9-8-1 | 16-8-0 | 23-7-15 | 28-5-7 | 33-4-0 | 34-8-0

1-4-0 | 4-10-8 | 4-9-9 | 6-11-15 | 6-11-15 | 4-9-9 | 4-10-9 | 1-4-0

Scale = 1:61.1
Camber = 1/8 in



9-8-1 | 12-10-0 | 20-10-0 | 23-7-15 | 33-4-0

9-8-1 | 3-1-15 | 8-0-0 | 2-9-15 | 9-8-1

Plate Offsets (X,Y): [2:0-3-8,Edge], [4:0-2-11,Edge], [6:0-2-11,Edge], [8:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.41	2-14	>370	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.34	2-14	>443	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 229 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 4-13, 5-13, 5-11, 6-11, 6-10

REACTIONS (lb/size) 2=589/0-4-0, 13=892/0-4-0, 11=880/0-4-0, 8=574/0-4-0
Max Horz 2=352(load case 4)
Max Uplift2=-388(load case 5), 13=-603(load case 4), 11=-502(load case 3), 8=-425(load case 6)
Max Grav 2=597(load case 9), 13=901(load case 9), 11=889(load case 10), 8=583(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-444/350, 3-4=-284/388, 4-5=0/207, 5-6=0/208, 6-7=-265/372, 7-8=-426/334, 8-9=0/47
BOT CHORD 2-14=-336/273, 13-14=-194/239, 12-13=-110/291, 11-12=-110/291, 10-11=-98/213, 8-10=-65/260
WEBS 3-14=-194/319, 4-14=-575/360, 4-13=-567/549, 5-13=-249/207, 5-11=-232/195, 6-11=-574/565, 6-10=-584/358, 7-10=-194/317

JOINT STRESS INDEX
2 = 0.59, 2 = 0.00, 3 = 0.34, 4 = 0.49, 5 = 0.49, 6 = 0.50, 7 = 0.34, 8 = 0.59, 8 = 0.00, 10 = 0.37, 11 = 0.52, 12 = 0.32, 13 = 0.51 and 14 = 0.37

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 2, 603 lb uplift at joint 13, 502 lb uplift at joint 11 and 425 lb uplift at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

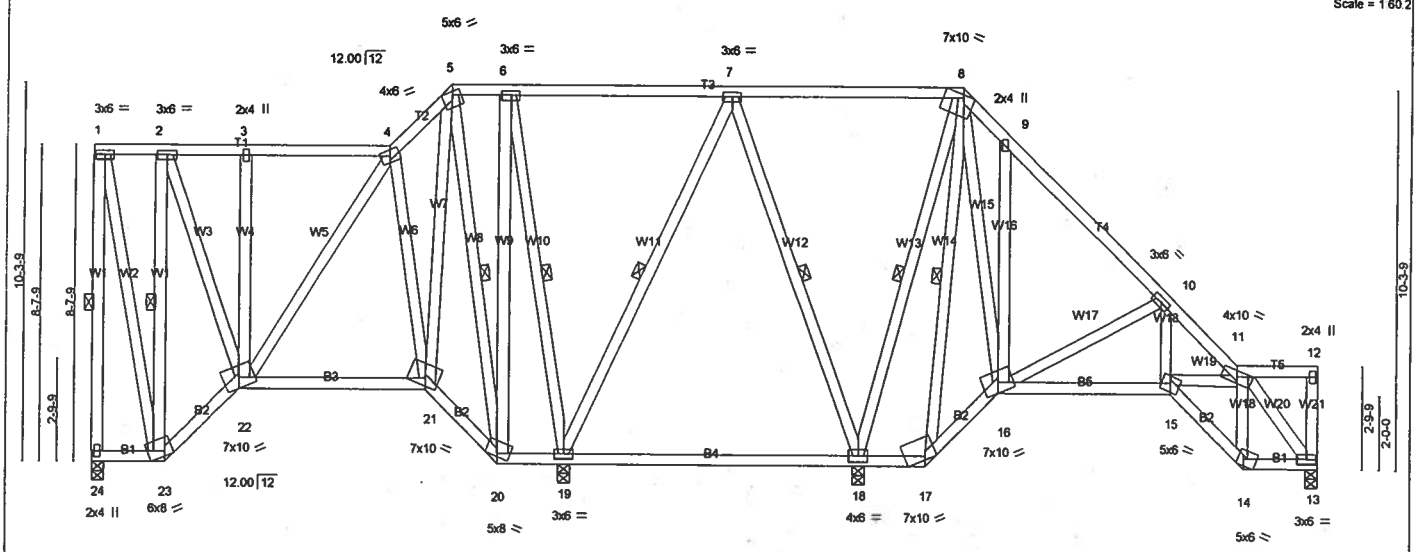
Dwg.#0829061148

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Sep 07 17:01:53 2006 Page 1

2-0-0 4-0-0 8-0-1 9-8-1 11-0-0 17-3-15 23-7-15 24-8-0 29-4-0 31-1-15 33-4-0
2-0-0 2-0-0 4-0-1 1-8-0 1-3-15 6-3-15 6-3-15 1-0-1 4-8-0 1-9-15 2-2-1

Scale = 1:60.2



2-0-0 4-0-0 8-0-0 11-0-0 12-10-0 20-10-0 22-8-0 24-8-0 29-4-0 31-4-0 33-4-0
2-0-0 2-0-0 5-0-0 2-0-0 1-10-0 8-0-0 1-10-0 2-0-0 4-8-0 2-0-0 2-0-0

Plate Offsets (X,Y): [8:0-3-11,Edge], [16:0-5-0,0-3-4], [22:0-5-0,0-3-4], [23:0-2-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.05	18-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.09	18-19	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.04	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 347 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 1-4, 5-8, 11-12.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 1-24, 2-23, 5-20, 6-19, 7-19, 7-18, 8-18, 8-17
JOINTS 1 Brace at Jt(s): 1, 12

REACTIONS (lb/size) 24=328/0-4-0, 13=307/0-4-0, 19=1073/0-4-0, 18=1067/0-4-0
Max Horz 24=-236(load case 3)
Max Uplift24=-408(load case 3), 13=-189(load case 3), 19=-688(load case 4), 18=-574(load case 3)
Max Grav 24=352(load case 9), 13=328(load case 10), 19=1125(load case 9), 18=1113(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-24=-313/448, 1-2=-61/94, 2-3=-131/124, 3-4=-131/124, 4-5=-31/164, 5-6=-14/135, 6-7=-101/249, 7-8=-61/257, 8-9=-89/256, 9-10=-67/136, 10-11=-459/436, 11-12=-14/11, 12-13=-69/56
BOT CHORD 23-24=-85/250, 22-23=-128/293, 21-22=-51/276, 20-21=-106/360, 19-20=-139/215, 18-19=-184/270, 17-18=-87/145, 16-17=-107/308, 15-16=-146/309, 14-15=-251/330, 13-14=-230/200
WEBS 1-23=-411/263, 2-23=-283/185, 2-22=-167/222, 3-22=-191/167, 4-22=-117/219, 4-21=-345/265, 5-21=-324/432, 5-20=-359/232, 6-20=-361/334, 6-19=-677/590, 7-19=-242/201, 7-18=-291/171, 8-18=-631/670, 8-17=-294/0, 8-16=-393/333, 9-16=-171/203, 10-16=-370/453, 10-15=-308/294, 11-15=-65/119, 11-14=-172/75, 11-13=-294/345

JOINT STRESS INDEX
1 = 0.54, 2 = 0.50, 3 = 0.34, 4 = 0.37, 5 = 0.58, 6 = 0.54, 7 = 0.49, 8 = 0.42, 9 = 0.34, 10 = 0.47, 11 = 0.40, 12 = 0.34, 13 = 0.42, 14 = 0.33, 15 = 0.22, 16 = 0.46, 17 = 0.52, 18 = 0.36, 19 = 0.54, 20 = 0.29, 21 = 0.46, 22 = 0.33, 23 = 0.24 and 24 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 24, 189 lb uplift at joint 13, 688 lb uplift at joint 19 and 574 lb uplift at joint 18.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061149

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:29:58 2006 Page 1

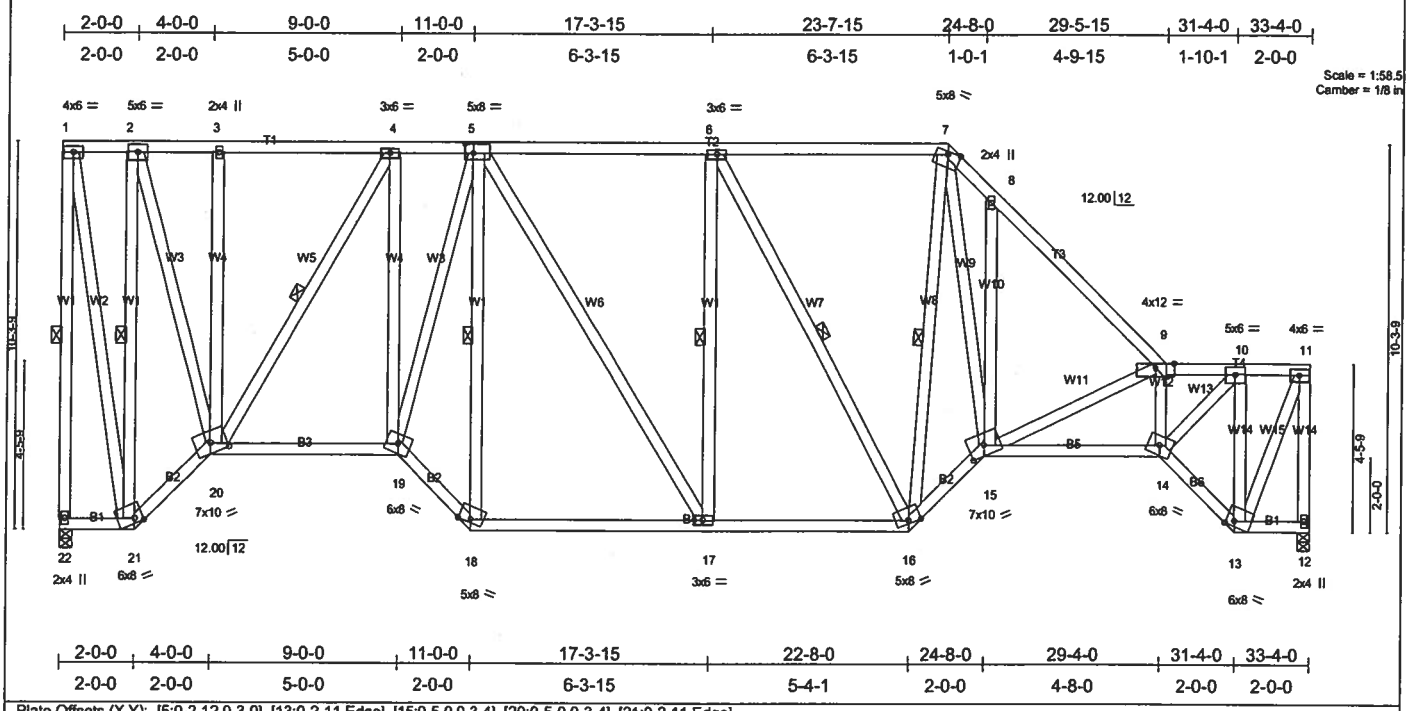


Plate Offsets (X,Y): [5:0-2-12,0-3-0], [13:0-2-11,Edge], [15:0-5-0,0-3-4], [20:0-5-0,0-3-4], [21:0-2-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.16	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.26	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.26	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 342 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-3 oc purlins, except end verticals, and 2-0-0 oc purlins (4-1-15 max.): 1-7, 9-11.
BOT CHORD Rigid ceiling directly applied or 7-3-10 oc bracing.
WEBS 1 Row at midpt 1-22, 2-21, 4-20, 5-18, 6-17, 6-16, 7-16
JOINTS 1 Brace at Jt(s): 1, 11

REACTIONS (lb/size) 22=1388/0-4-0, 12=1388/0-4-0
Max Horz 22=-187(load case 5)
Max Uplift22=-586(load case 4), 12=-392(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-22=-1317/574, 1-2=-233/99, 2-3=-625/266, 3-4=-625/266, 4-5=-1134/463, 5-6=-1171/545, 6-7=-1008/504, 7-8=-1770/833, 8-9=-1900/664, 9-10=-2217/775, 10-11=-572/188, 11-12=-1329/466
BOT CHORD 21-22=-28/250, 20-21=-154/330, 19-20=-483/1122, 18-19=-612/1450, 17-18=-436/1037, 16-17=-455/1171, 15-16=-468/1444, 14-15=-752/2162, 13-14=-235/749, 12-13=-10/27
WEBS 1-21=-517/1210, 2-21=-1397/630, 2-20=-642/1500, 3-20=-268/211, 4-20=-979/442, 4-19=-270/695, 5-19=-208/402, 5-18=-850/441, 5-17=-162/252, 6-17=-46/182, 6-16=-348/259, 7-16=-533/178, 7-15=-632/1542, 8-15=-176/229, 9-15=-968/432, 9-14=-1152/493, 10-14=-876/2460, 10-13=-1717/580, 11-13=-437/1334

JOINT STRESS INDEX
1 = 0.90, 2 = 0.79, 3 = 0.34, 4 = 0.45, 5 = 0.45, 6 = 0.46, 7 = 0.85, 8 = 0.34, 9 = 0.60, 10 = 0.83, 11 = 0.86, 12 = 0.66, 13 = 0.54, 14 = 0.91, 15 = 0.72, 16 = 0.47, 17 = 0.44, 18 = 0.55, 19 = 0.34, 20 = 0.68, 21 = 0.54 and 22 = 0.69

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 586 lb uplift at joint 22 and 392 lb uplift at joint 12.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061150

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:29:40 2006 Page 1

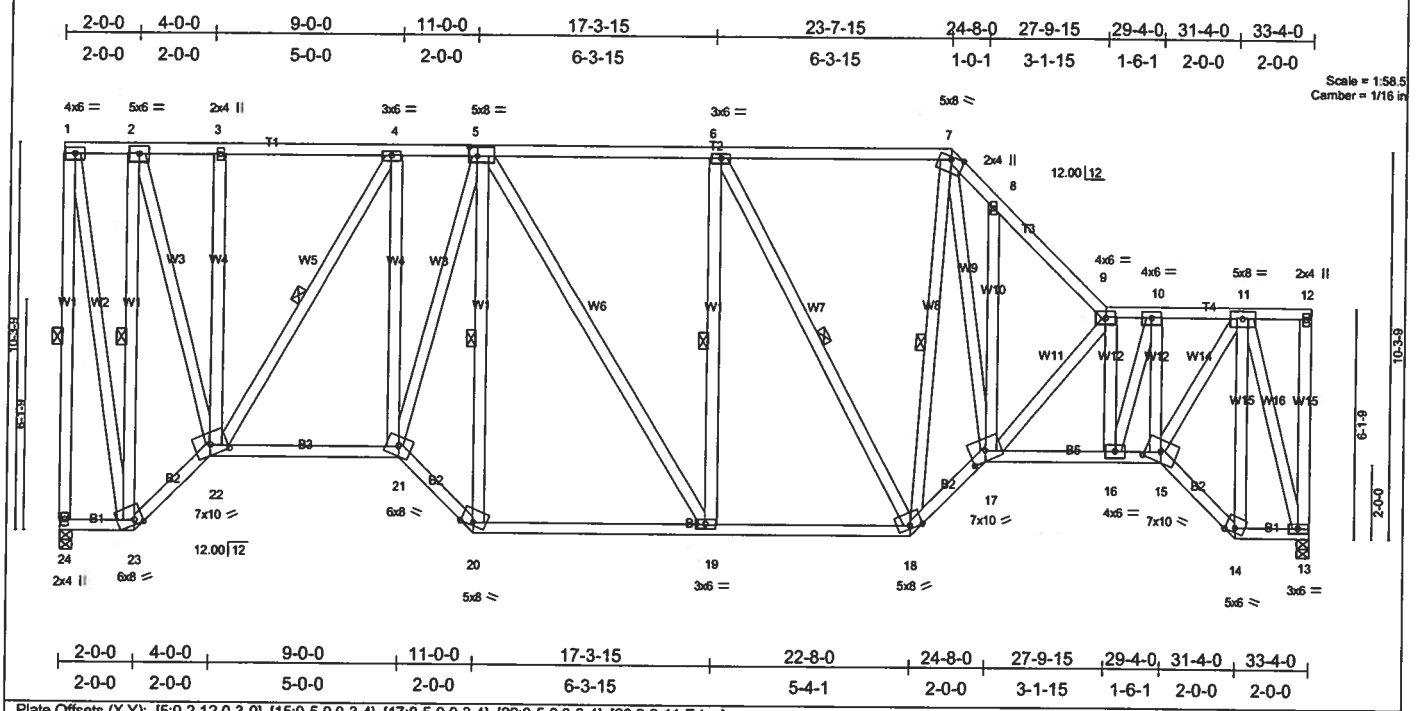


Plate Offsets (X,Y): [5:0-2-12,0-3-0], [15:0-5-0,0-3-4], [17:0-5-0,0-3-4], [22:0-5-0,0-3-4], [23:0-2-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.15	19-20	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.24	19-20	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.95	Horz(TL)	0.22	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 363 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-4 oc purlins, except end verticals, and 2-0-0 oc purlins (5-0-2 max.): 1-7, 9-12.
BOT CHORD Rigid ceiling directly applied or 8-0-0 oc bracing.
WEBS 1 Row at midpt 1-24, 2-23, 4-22, 5-20, 6-19, 6-18, 7-18
JOINTS 1 Brace at Jt(s): 1, 12

REACTIONS (lb/size) 24=1388/0-4-0, 13=1388/0-4-0
Max Horz 24=-134(load case 5)
Max Uplift24=-600(load case 4), 13=-474(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-24=-1317/588, 1-2=-233/101, 2-3=-625/271, 3-4=-625/271, 4-5=-1134/478, 5-6=-1171/527, 6-7=-1009/481, 7-8=-1708/739, 8-9=-1849/672, 9-10=-1604/557, 10-11=-1245/429, 11-12=-8/3, 12-13=-56/36
BOT CHORD 23-24=-21/177, 22-23=-148/330, 21-22=-490/1122, 20-21=-624/1449, 19-20=-445/1037, 18-19=-473/1171, 17-18=-507/1427, 16-17=-571/1639, 15-16=-444/1286, 14-15=-194/560, 13-14=-137/390
WEBS 1-23=-523/1210, 2-23=-1397/640, 2-22=-654/1500, 3-22=-268/211, 4-22=-979/430, 4-21=-262/695, 5-21=-202/401, 5-20=-850/450, 5-19=-148/252, 6-19=-48/172, 6-18=-345/242, 7-18=-510/217, 7-17=-566/1440, 8-17=-155/83, 9-17=-602/271, 9-16=-951/385, 10-16=-375/1050, 10-15=-1069/382, 11-15=-602/1759, 11-14=-326/150, 11-13=-1356/478

JOINT STRESS INDEX
1 = 0.90, 2 = 0.79, 3 = 0.34, 4 = 0.45, 5 = 0.45, 6 = 0.46, 7 = 0.79, 8 = 0.34, 9 = 0.65, 10 = 0.73, 11 = 0.89, 12 = 0.34, 13 = 0.67, 14 = 0.24, 15 = 0.70, 16 = 0.73, 17 = 0.67, 18 = 0.49, 19 = 0.44, 20 = 0.55, 21 = 0.34, 22 = 0.68, 23 = 0.54 and 24 = 0.69

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 600 lb uplift at joint 24 and 474 lb uplift at joint 13.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061151

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T21	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:29:27 2006 Page 1

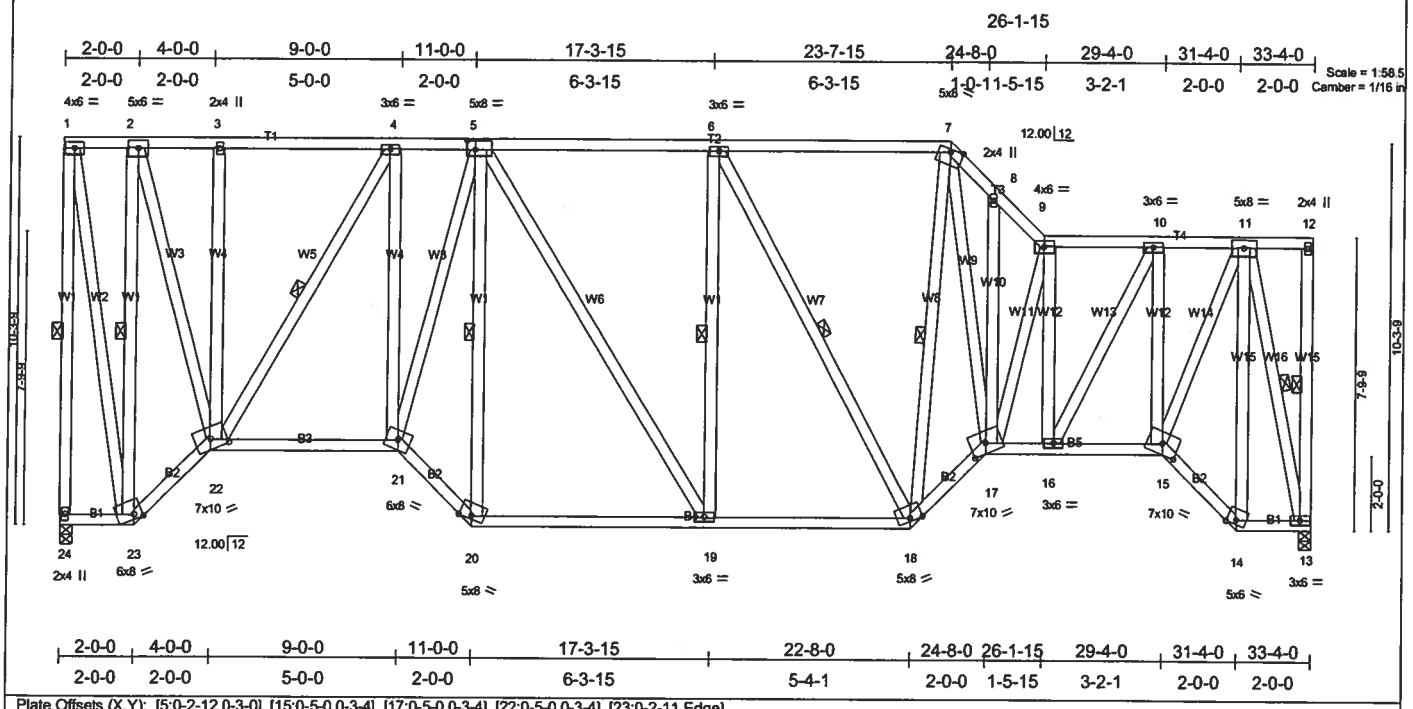


Plate Offsets (X,Y): [5:0-2-12,0-3-0], [15:0-5-0,0-3-4], [17:0-5-0,0-3-4], [22:0-5-0,0-3-4], [23:0-2-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.14	19-20	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.23	19-20	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.98	Horz(TL)	0.21	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 381 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-1 oc purlins, except end verticals, and 2-0-0 oc purlins (5-3-6 max.): 1-7, 9-12.
BOT CHORD Rigid ceiling directly applied or 7-10-7 oc bracing.
WEBS 1 Row at mldpt 1-24, 12-13, 2-23, 4-22, 5-20, 6-19, 6-18, 7-18, 11-13
JOINTS 1 Brace at Jt(s): 1, 12

REACTIONS (lb/size) 24=1388/0-4-0, 13=1388/0-4-0
Max Horz 24=-80(load case 5)
Max Uplift24=-620(load case 4), 13=-552(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-24=-1317/607, 1-2=-233/104, 2-3=-625/279, 3-4=-625/279, 4-5=-1134/497, 5-6=-1171/504, 6-7=-1010/451, 7-8=-1655/658, 8-9=-1816/685, 9-10=-1382/519, 10-11=-869/337, 11-12=-7/2, 12-13=-56/38
BOT CHORD 23-24=-15/105, 22-23=-144/330, 21-22=-503/1122, 20-21=-644/1449, 19-20=-460/1037, 18-19=-500/1171, 17-18=-561/1410, 16-17=-529/1402, 15-16=-350/898, 14-15=-168/444, 13-14=-120/302
WEBS 1-23=-541/1210, 2-23=-1397/655, 2-22=-671/1500, 3-22=-268/212, 4-22=-979/422, 4-21=-249/695, 5-21=-196/401, 5-20=-849/463, 5-19=-131/251, 6-19=-49/159, 6-18=-343/218, 7-18=-487/241, 7-17=-510/1356, 8-17=-175/160, 9-17=-631/265, 9-16=-797/328, 10-16=-367/1043, 10-15=-1074/458, 11-15=-603/1566, 11-14=-248/130, 11-13=-1331/529

JOINT STRESS INDEX
1 = 0.90, 2 = 0.79, 3 = 0.34, 4 = 0.45, 5 = 0.45, 6 = 0.46, 7 = 0.75, 8 = 0.34, 9 = 0.66, 10 = 0.88, 11 = 0.88, 12 = 0.34, 13 = 0.68, 14 = 0.24, 15 = 0.68, 16 = 0.88, 17 = 0.63, 18 = 0.52, 19 = 0.44, 20 = 0.55, 21 = 0.34, 22 = 0.68, 23 = 0.54 and 24 = 0.69

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 620 lb uplift at joint 24 and 552 lb uplift at joint 13.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061152

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T22	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:32:35 2006 Page 1

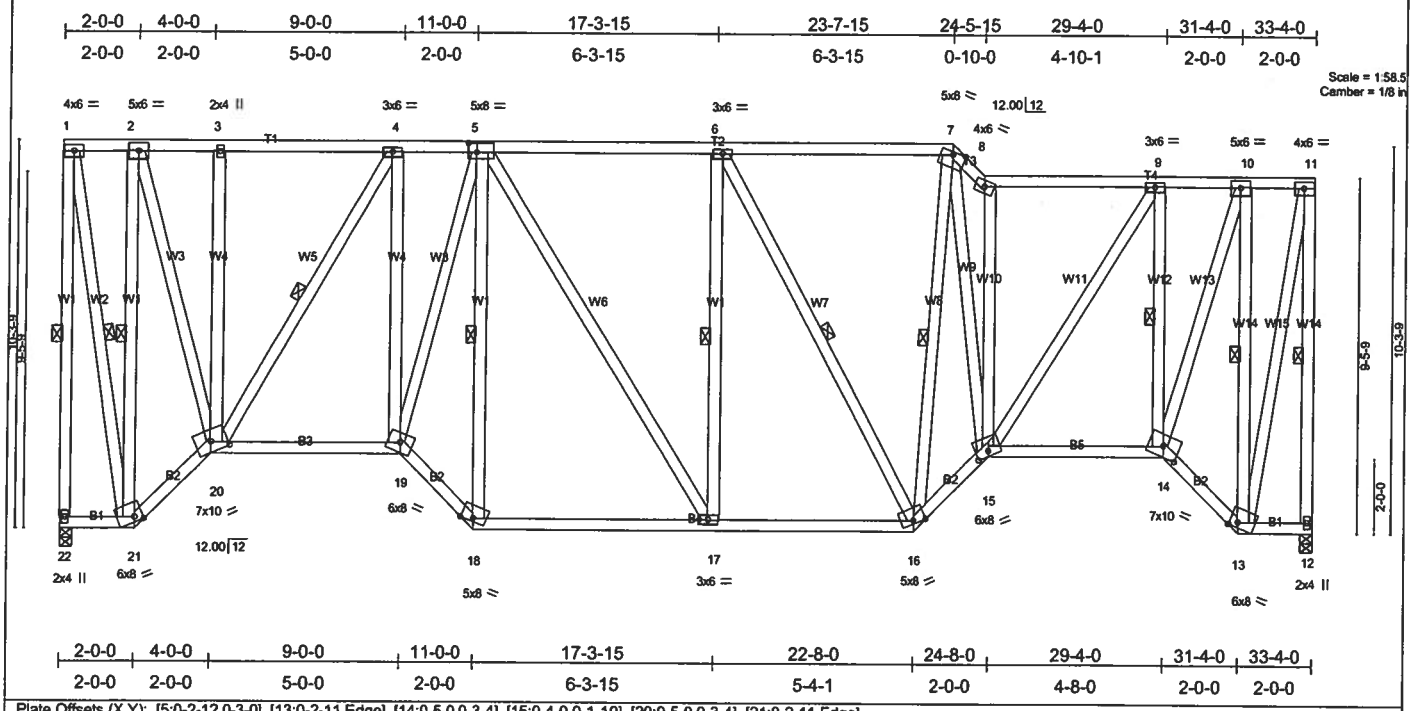


Plate Offsets (X,Y): [5:0-2-12,0-3-0], [13:0-2-11,Edge], [14:0-5-0,0-3-4], [15:0-4-0,0-1-10], [20:0-5-0,0-3-4], [21:0-2-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.16	17-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.26	17-18	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.23	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 381 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-9 oc purlins, except end verticals, and 2-0-0 oc purlins (5-3-2 max.): 1-7, 8-11.
BOT CHORD Rigid ceiling directly applied or 7-8-7 oc bracing.
WEBS 1 Row at midpt 1-22, 11-12, 1-21, 2-21, 4-20, 5-18, 6-17, 6-16, 7-16, 9-14, 10-13
JOINTS 1 Brace at Jt(s): 1, 11

REACTIONS (lb/size) 22=1388/0-4-0, 12=1388/0-4-0
Max Horz 22=-27(load case 5)
Max Uplift22=-644(load case 4), 12=-624(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-22=-1317/630, 1-2=-233/108, 2-3=-625/290, 3-4=-625/290, 4-5=-1134/523, 5-6=-1170/531, 6-7=-1013/445, 7-8=-1508/637, 8-9=-1212/534, 9-10=-682/304, 10-11=-255/115, 11-12=-1318/611
BOT CHORD 21-22=-8/32, 20-21=-141/330, 19-20=-521/1122, 18-19=-673/1450, 17-18=-480/1037, 16-17=-534/1170, 15-16=-665/1501, 14-15=-316/706, 13-14=-151/371, 12-13=-5/12
WEBS 1-21=-563/1210, 2-21=-1397/676, 2-20=-696/1501, 3-20=-268/213, 4-20=-979/450, 4-19=-263/698, 5-19=-190/399, 5-18=-850/483, 5-17=-109/248, 6-17=-44/142, 6-16=-334/182, 7-16=-616/309, 7-15=-526/1293, 8-15=-987/441, 9-15=-409/950, 9-14=-1118/576, 10-14=-697/1566, 10-13=-1441/671, 11-13=-549/1218

JOINT STRESS INDEX
1 = 0.90, 2 = 0.79, 3 = 0.34, 4 = 0.46, 5 = 0.45, 6 = 0.46, 7 = 0.63, 8 = 0.58, 9 = 0.75, 10 = 0.80, 11 = 0.90, 12 = 0.69, 13 = 0.54, 14 = 0.69, 15 = 0.56, 16 = 0.49, 17 = 0.44, 18 = 0.55, 19 = 0.34, 20 = 0.68, 21 = 0.54 and 22 = 0.69

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 644 lb uplift at joint 22 and 624 lb uplift at joint 12.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061153

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T23	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:33:42 2006 Page 1

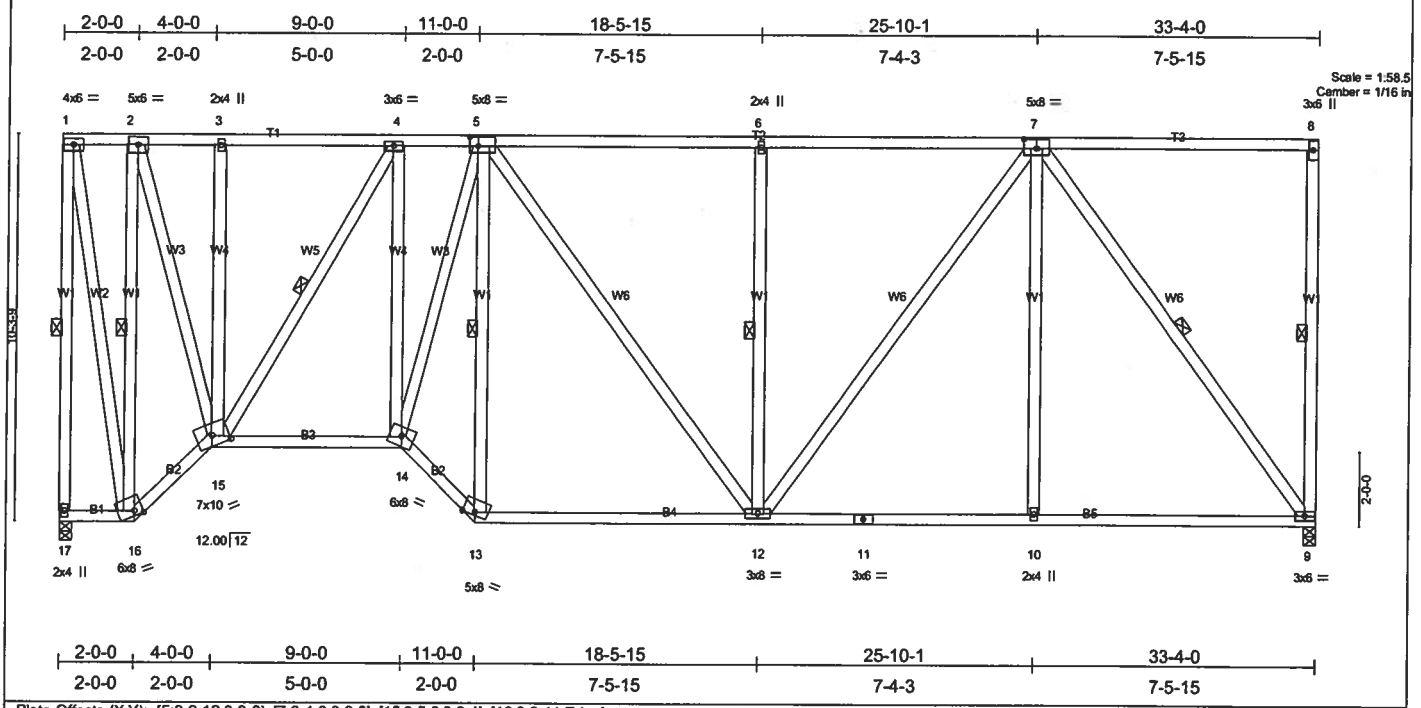


Plate Offsets (X,Y): [5:0-2-12,0-3-0], [7:0-4-0,0-3-0], [15:0-5-0,0-3-4], [16:0-2-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.14	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.23	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.13	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 317 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (5-4-3 max.): 1-8, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-3-11 oc bracing.
WEBS 1 Row at midpt 1-17, 8-9, 2-16, 4-15, 5-13, 6-12, 7-9
JOINTS 1 Brace at Jt(s): 1, 8

REACTIONS (lb/size) 17=1388/0-4-0, 9=1388/0-4-0
Max Uplift17=-524(load case 3), 9=-524(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-17=-1316/531, 1-2=-232/91, 2-3=-626/244, 3-4=-626/244, 4-5=-1133/443, 5-6=-1170/457, 6-7=-1170/457, 7-8=-21/8, 8-9=-178/125
BOT CHORD 16-17=-4/11, 15-16=-115/330, 14-15=-438/1120, 13-14=-579/1453, 12-13=-409/1046, 11-12=-326/833, 10-11=-326/833, 9-10=-326/833
WEBS 1-16=-475/1209, 2-16=-1397/568, 2-15=-586/1503, 3-15=-273/184, 4-15=-974/382, 4-14=-253/732, 5-14=-137/358, 5-13=-834/428, 5-12=-79/207, 6-12=-406/290, 7-12=-221/569, 7-10=0/228, 7-9=-1372/537

JOINT STRESS INDEX
1 = 0.90, 2 = 0.79, 3 = 0.34, 4 = 0.48, 5 = 0.54, 6 = 0.34, 7 = 0.42, 8 = 0.36, 9 = 0.55, 10 = 0.34, 11 = 0.39, 12 = 0.58, 13 = 0.66, 14 = 0.34, 15 = 0.68, 16 = 0.54 and 17 = 0.69

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 524 lb uplift at joint 17 and 524 lb uplift at joint 9.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061154

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T24	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:34:26 2006 Page 1

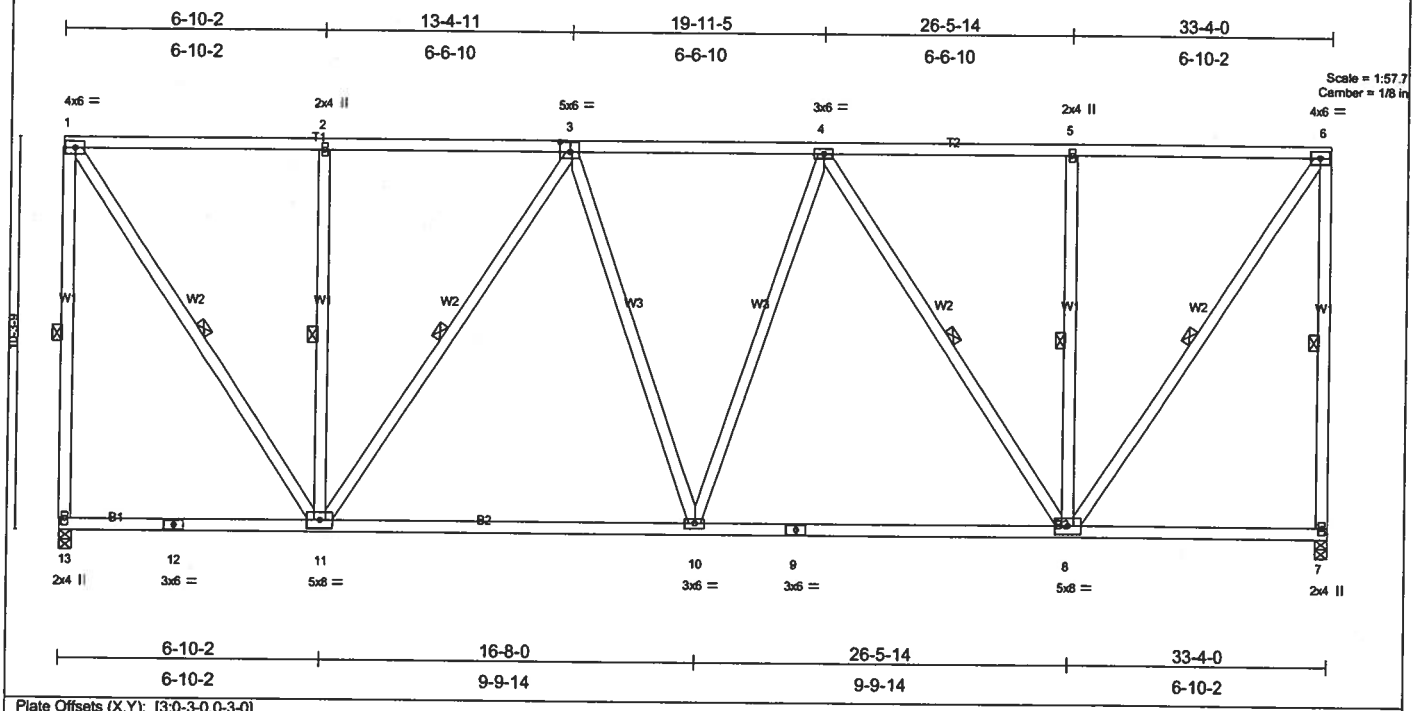


Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.19	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.33	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 257 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (5-7-10 max.): 1-6, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-5-7 oc bracing.
WEBS 1 Row at midpt 1-13, 6-7, 1-11, 2-11, 3-11, 4-8, 5-8, 6-8
JOINTS 1 Brace at Jt(s): 1, 6

REACTIONS (lb/size) 13=1388/0-4-0, 7=1388/0-4-0
Max Uplift13=-524(load case 3), 7=-524(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-13=-1305/553, 1-2=-781/301, 2-3=-781/301, 3-4=-1162/435, 4-5=-781/301, 5-6=-781/301, 6-7=-1305/553
BOT CHORD 12-13=-8/16, 11-12=-8/16, 10-11=-445/1111, 9-10=-445/1111, 8-9=-445/1111, 7-8=-8/16
WEBS 1-11=-527/1375, 2-11=-381/277, 3-11=-602/263, 3-10=0/164, 4-10=0/164, 4-8=-602/263, 5-8=-381/277, 6-8=-527/1375

JOINT STRESS INDEX
1 = 0.72, 2 = 0.34, 3 = 0.43, 4 = 0.50, 5 = 0.34, 6 = 0.72, 7 = 0.77, 8 = 0.68, 9 = 0.46, 10 = 0.50, 11 = 0.68, 12 = 0.18 and 13 = 0.77

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 524 lb uplift at joint 13 and 524 lb uplift at joint 7.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

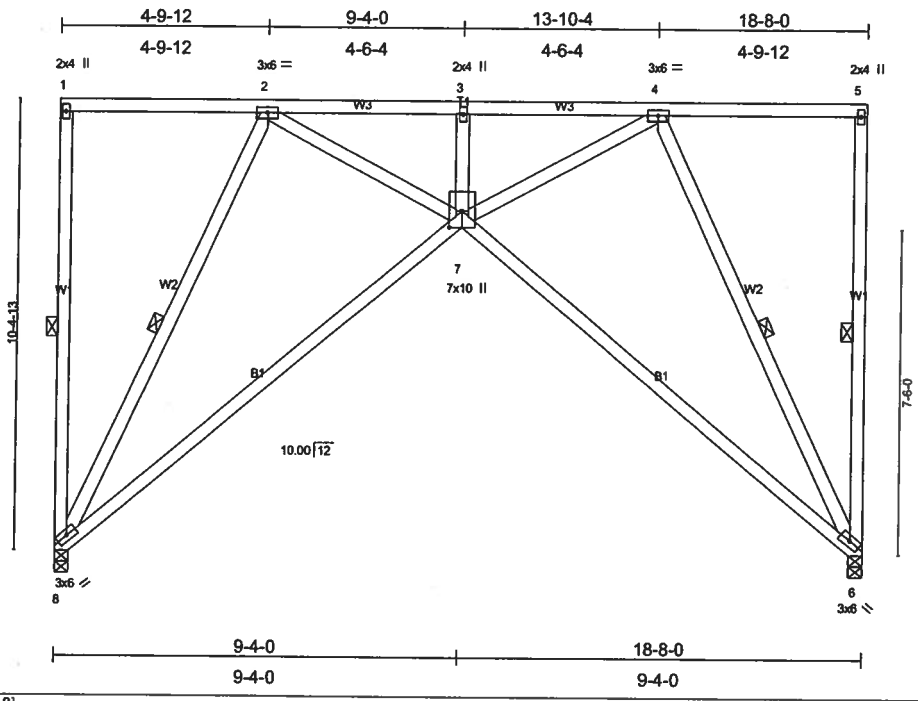
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061155

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T25	SPECIAL	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:36:05 2006 Page 1



Scale = 1:50.8
Camber = 3/16 in

Plate Offsets (X,Y): [7:0-4-9,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.19	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.33	6-7	>661	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.24	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 144 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (5-1-0 max.): 1-5, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 1-8, 5-6, 2-8, 4-6
JOINTS 1 Brace at Jt(s): 1, 5

REACTIONS (lb/size) 8=772/0-4-0, 6=772/0-4-0
Max Uplift8=-291(load case 3), 6=-291(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-8=-120/76, 1-2=-19/0, 2-3=-1499/534, 3-4=-1499/534, 4-5=-19/0, 5-6=-120/76
BOT CHORD 7-8=-252/642, 6-7=-252/642
WEBS 3-7=-227/161, 2-7=-404/1243, 2-8=-963/439, 4-7=-404/1243, 4-6=-963/439

JOINT STRESS INDEX
1 = 0.75, 2 = 0.72, 3 = 0.34, 4 = 0.72, 5 = 0.75, 6 = 0.48, 7 = 0.80 and 8 = 0.48

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 8, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 291 lb uplift at joint 8 and 291 lb uplift at joint 6.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061156

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T26	SCISSOR	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:36:33 2006 Page 1

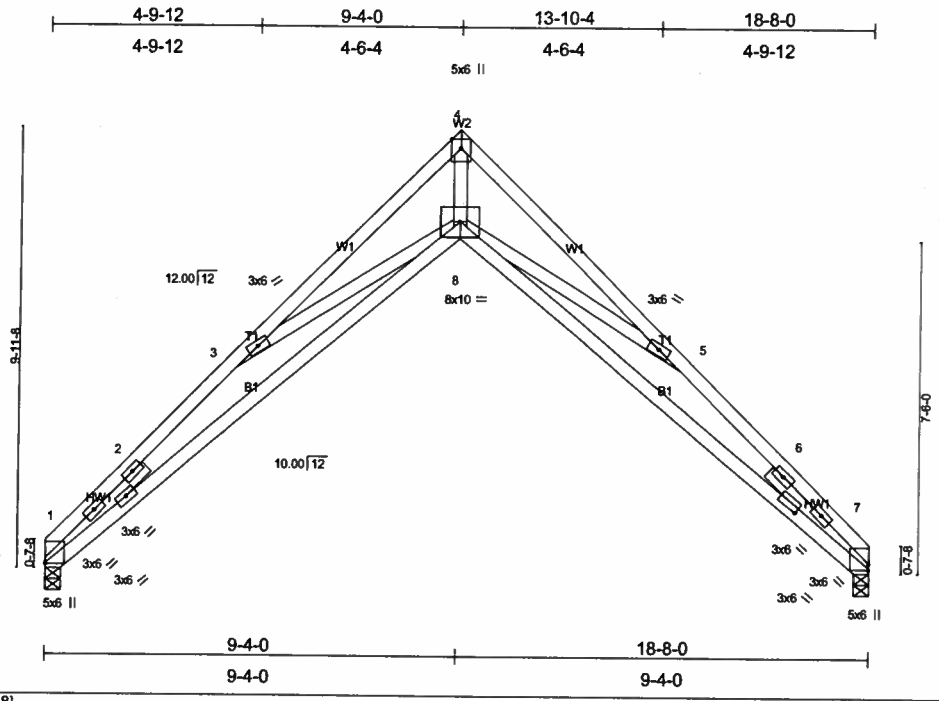


Plate Offsets (X,Y): [7:2-0-7,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.39	1-8	>564	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.63	1-8	>348	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	1.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 106 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2
SLIDER Left 2 X 4 SYP No.3 3-1-14, Right 2 X 4 SYP No.3 3-1-14

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-4 oc bracing.

REACTIONS (lb/size) 1=768/0-4-0, 7=768/0-4-0
Max Horz 1=336(load case 4)
Max Uplift1=-230(load case 6), 7=-230(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2787/1020, 2-3=-2709/1034, 3-4=-2664/503, 4-5=-2664/609, 5-6=-2709/751, 6-7=-2787/737
BOT CHORD 1-8=-1144/2433, 7-8=-538/2433
WEBS 3-8=-132/814, 4-8=-724/3326, 5-8=-405/1000

JOINT STRESS INDEX
1 = 0.85, 1 = 0.51, 1 = 0.51, 1 = 0.42, 2 = 0.00, 3 = 0.27, 4 = 0.73, 5 = 0.27, 6 = 0.00, 7 = 0.67, 7 = 0.51, 7 = 0.51, 7 = 0.42 and 8 = 0.83

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 1 and 230 lb uplift at joint 7.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061157

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T27	SCISSOR	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:40:53 2006 Page 1

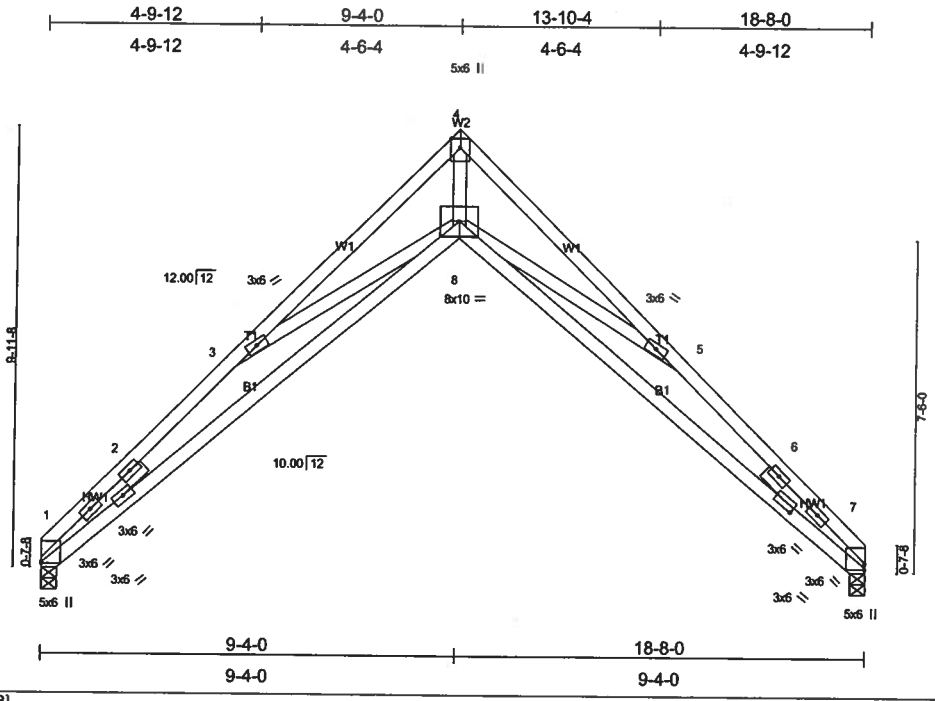


Plate Offsets (X,Y): [7:2-0-7,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.39	1-8	>564	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.63	1-8	>348	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	1.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 106 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2
SLIDER Left 2 X 4 SYP No.3 3-1-14, Right 2 X 4 SYP No.3 3-1-14

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-4 oc bracing.

REACTIONS (lb/size) 1=768/0-4-0, 7=768/0-4-0
Max Horz 1=336(load case 4)
Max Uplift1=-230(load case 6), 7=-230(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2787/1020, 2-3=-2709/1034, 3-4=-2664/503, 4-5=-2664/609, 5-6=-2709/751, 6-7=-2787/737
BOT CHORD 1-8=-1144/2433, 7-8=-538/2433
WEBS 3-8=-132/814, 4-8=-724/3326, 5-8=-405/1000

JOINT STRESS INDEX
1 = 0.85, 1 = 0.51, 1 = 0.51, 1 = 0.42, 2 = 0.00, 3 = 0.27, 4 = 0.73, 5 = 0.27, 6 = 0.00, 7 = 0.67, 7 = 0.51, 7 = 0.51, 7 = 0.42 and 8 = 0.83

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 1 and 230 lb uplift at joint 7.

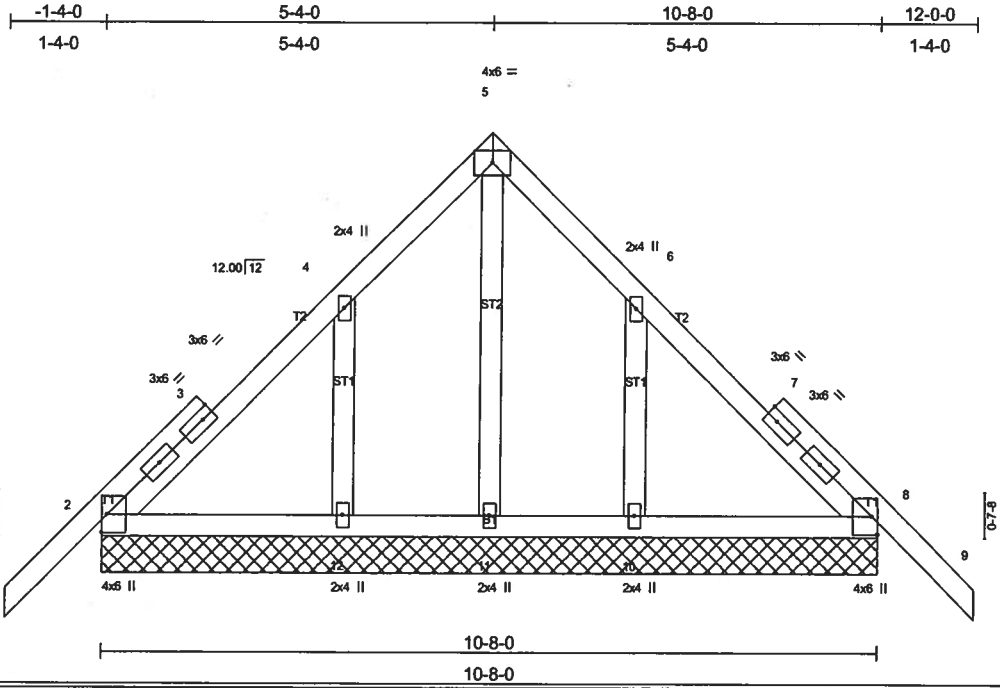
LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061158

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T28	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:42:24 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.00	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=288/10-8-0, 8=288/10-8-0, 11=134/10-8-0, 12=331/10-8-0, 10=331/10-8-0
Max Horz 2=-186(load case 3)
Max Uplift2=-93(load case 5), 8=-112(load case 6), 12=-229(load case 5), 10=-229(load case 6)
Max Grav 2=288(load case 1), 8=288(load case 1), 11=134(load case 1), 12=333(load case 9), 10=333(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1/66, 2-3=-131/95, 3-4=-126/102, 4-5=-79/112, 5-6=-79/112, 6-7=-80/55, 7-8=-99/49, 8-9=-1/66
BOT CHORD 2-12=-20/202, 11-12=-20/202, 10-11=-20/202, 8-10=-20/202
WEBS 5-11=-105/6, 4-12=-231/237, 6-10=-231/237

JOINT STRESS INDEX
2 = 0.53, 3 = 0.00, 3 = 0.20, 3 = 0.20, 4 = 0.13, 5 = 0.08, 6 = 0.13, 7 = 0.00, 7 = 0.20, 7 = 0.20, 8 = 0.53, 10 = 0.14, 11 = 0.04 and 12 = 0.14

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 2, 112 lb uplift at joint 8, 229 lb uplift at joint 12 and 229 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (pf)
Vert: 1-5=-79(F=-25), 5-9=-79(F=-25), 2-8=-30

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061159

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T29	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:43:28 2006 Page 1

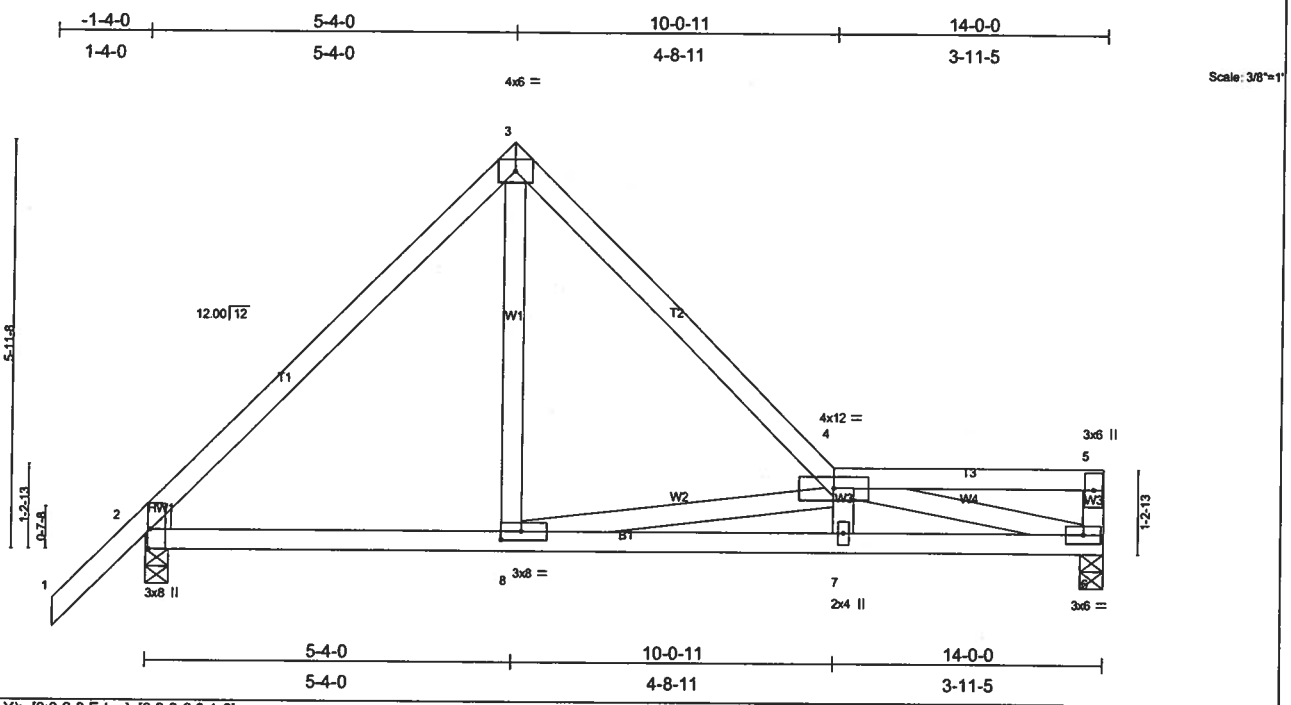


Plate Offsets (X,Y): [2:0-3-8,Edge], [8:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.10	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.16	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 74 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-8-0 oc bracing.

REACTIONS (lb/size) 6=570/0-4-0, 2=660/0-4-0
Max Horz 2=217(load case 4)
Max Uplift6=-145(load case 5), 2=-254(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-634/230, 3-4=-569/232, 4-5=-232/97, 5-6=-131/118
BOT CHORD 2-8=-87/369, 7-8=-665/1641, 6-7=-655/1649
WEBS 3-8=-113/451, 4-8=-1296/611, 4-7=0/82, 4-6=-1458/587

JOINT STRESS INDEX
2 = 0.42, 2 = 0.00, 3 = 0.51, 4 = 0.56, 5 = 0.37, 6 = 0.49, 7 = 0.06 and 8 = 0.29

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 6 and 254 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061160

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T30	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 11:43:50 2006 Page 1

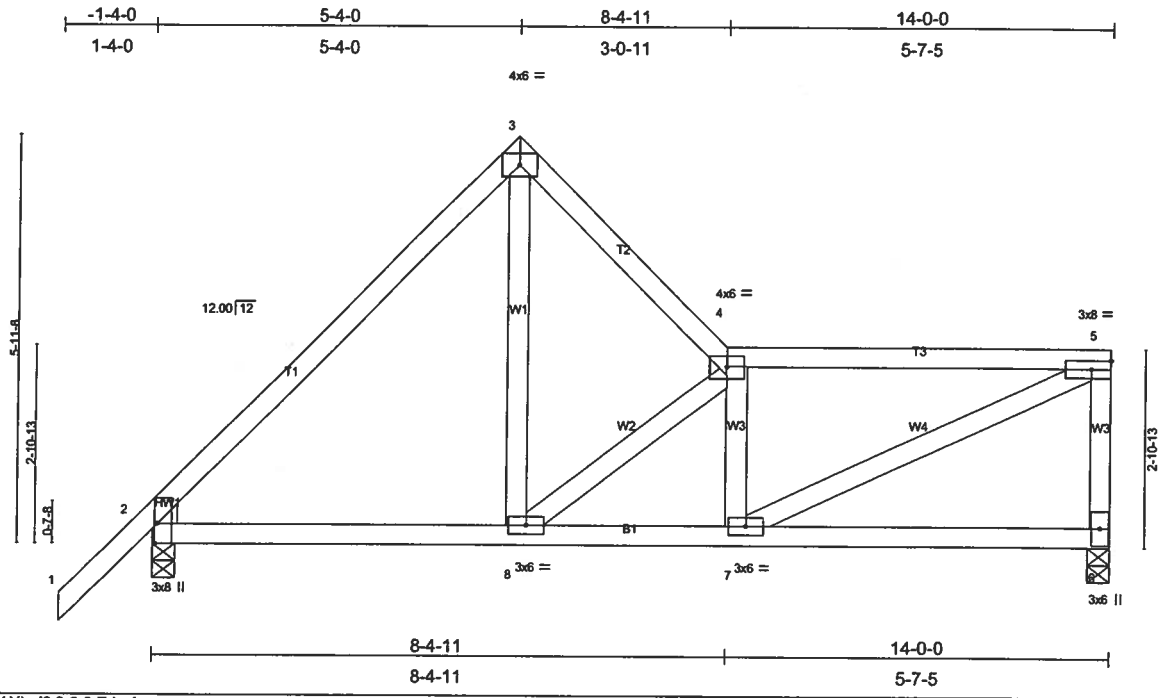


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.04	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.06	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 79 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=570/0-4-0, 2=660/0-4-0
Max Horz 2=231(load case 5)
Max Uplift6=-200(load case 4), 2=-247(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-620/219, 3-4=-547/260, 4-5=-744/303, 5-6=-479/251
BOT CHORD 2-8=-126/354, 7-8=-314/756, 6-7=-45/84
WEBS 3-8=-164/494, 4-7=-221/176, 5-7=-295/735, 4-8=-526/246

JOINT STRESS INDEX
2 = 0.43, 2 = 0.00, 3 = 0.39, 4 = 0.51, 5 = 0.56, 6 = 0.28, 7 = 0.42 and 8 = 0.32

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 200 lb uplift at joint 6 and 247 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061161

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T31	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 12:49:50 2006 Page 1

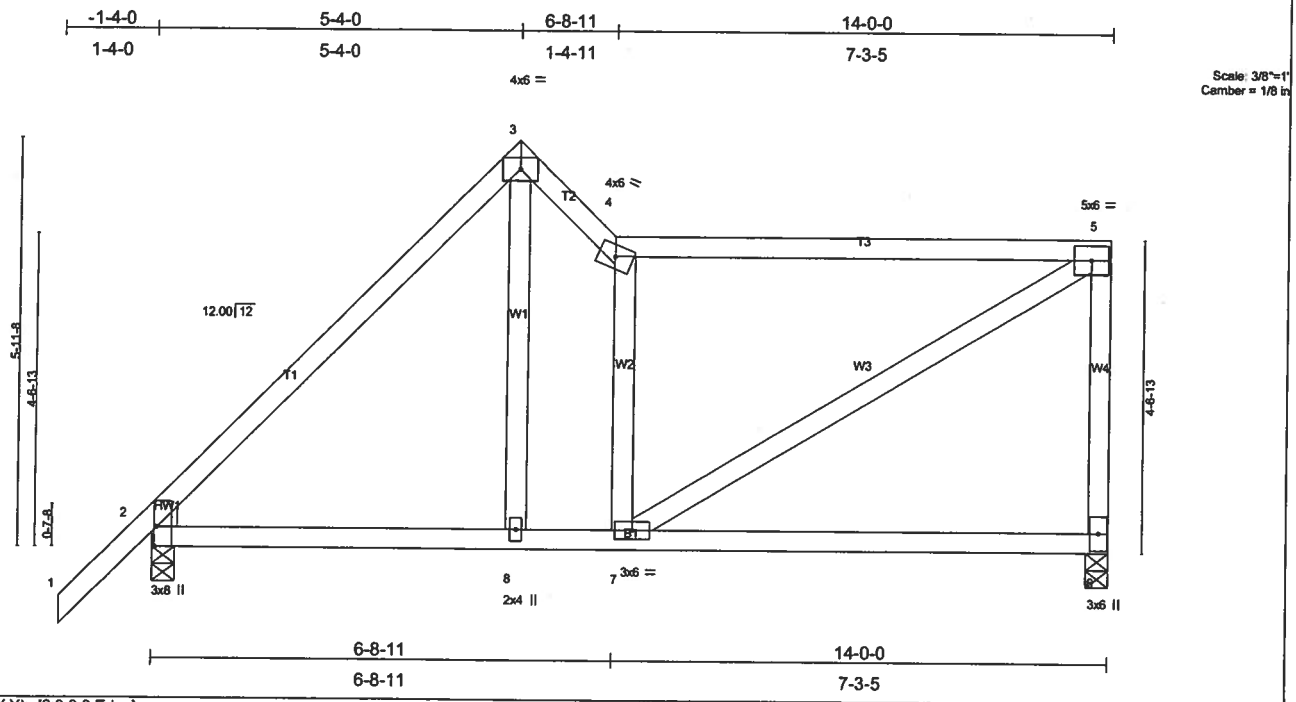


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.19	6-7	>883	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.30	6-7	>557	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 81 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.1D
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=570/0-4-0, 2=660/0-4-0
Max Horz 2=284(load case 5)
Max Uplift6=-251(load case 4), 2=-234(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-684/226, 3-4=-430/256, 4-5=-404/203, 5-6=-417/265
BOT CHORD 2-8=-213/415, 7-8=-213/415, 6-7=-50/117
WEBS 3-8=-144/391, 4-7=-336/278, 5-7=-180/338

JOINT STRESS INDEX
2 = 0.40, 2 = 0.00, 3 = 0.59, 4 = 0.58, 5 = 0.64, 6 = 0.64, 7 = 0.20 and 8 = 0.29

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 6 and 234 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061162

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T32	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 12:50:19 2006 Page 1

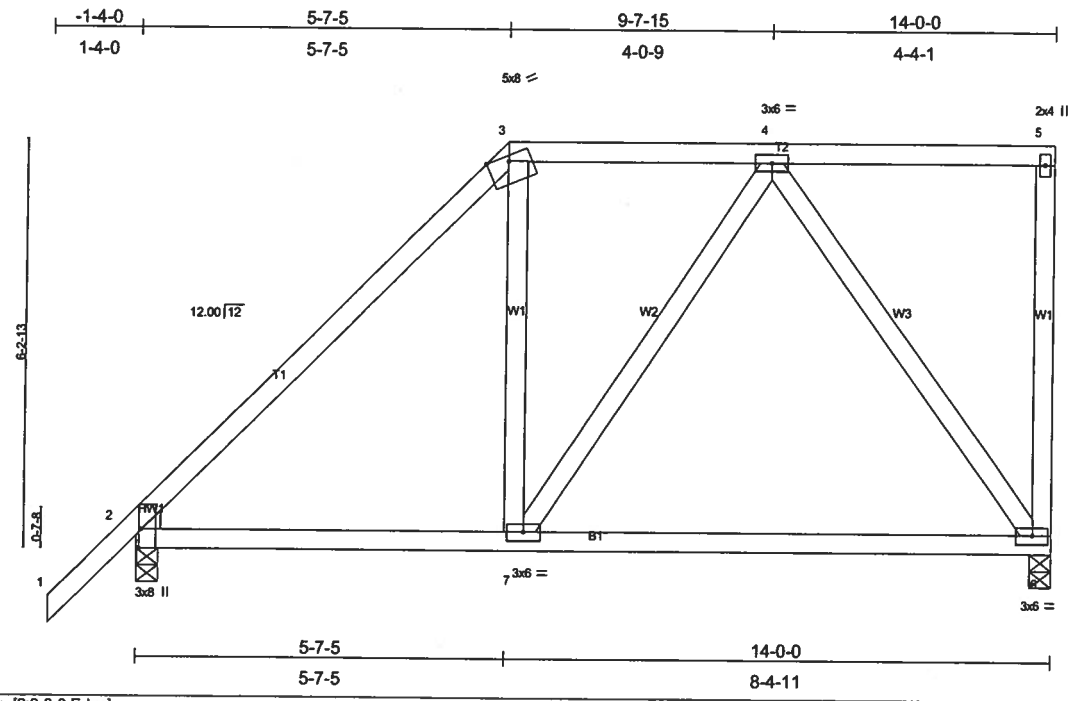


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.08	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.14	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 86 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=570/0-4-0, 2=660/0-4-0
Max Horz 2=342(load case 5)
Max Uplift6=-228(load case 4), 2=-214(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-633/159, 3-4=-363/226, 4-5=-29/2, 5-6=-114/87
BOT CHORD 2-7=-229/359, 6-7=-169/271
WEBS 3-7=-28/135, 4-7=-131/166, 4-6=-419/290

JOINT STRESS INDEX
2 = 0.47, 2 = 0.00, 3 = 0.68, 4 = 0.17, 5 = 0.68, 6 = 0.52 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 6 and 214 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061163

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T33	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 12:50:26 2006 Page 1

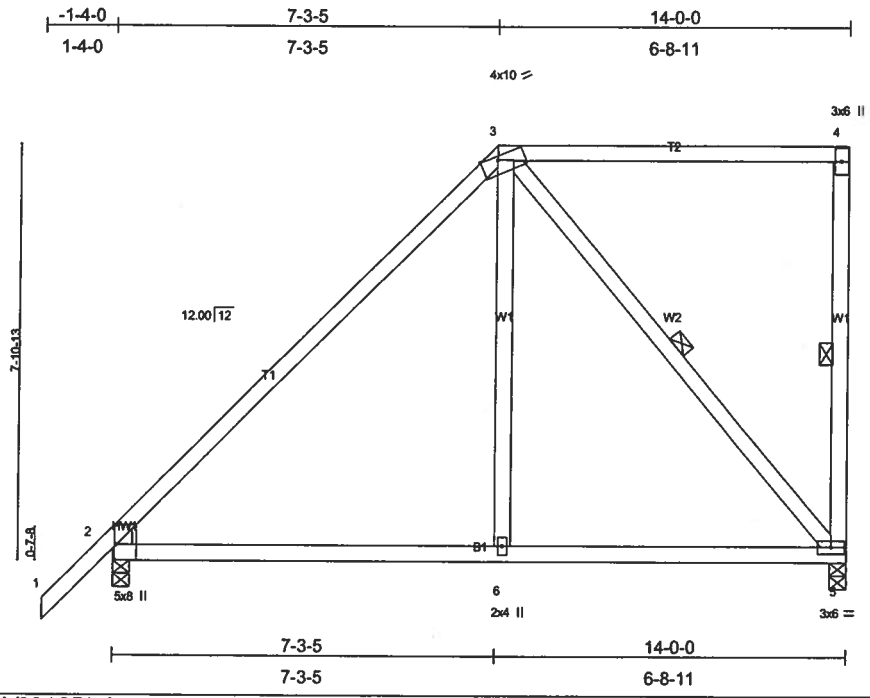


Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	0.11	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.16	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 86 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-5, 3-5

REACTIONS (lb/size) 5=570/0-4-0, 2=660/0-4-0
Max Horz 2=419(load case 5)
Max Uplift5=-237(load case 5), 2=-189(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-577/97, 3-4=-19/18, 4-5=-149/163
BOT CHORD 2-6=-226/311, 5-6=-226/316
WEBS 3-6=0/252, 3-5=-454/340

JOINT STRESS INDEX
2 = 0.69, 2 = 0.00, 3 = 0.93, 4 = 0.31, 5 = 0.29 and 6 = 0.18

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 5 and 189 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061164

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T34	MONO HIP	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 12:50:45 2006 Page 1

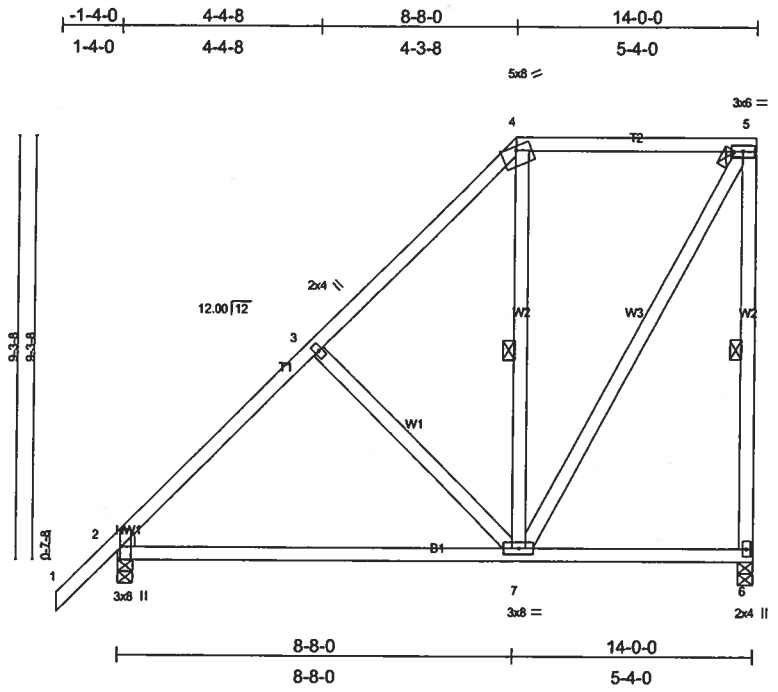


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.12	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.21	2-7	>768	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 100 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-6, 4-7
JOINTS 1 Brace at Jt(s): 5

REACTIONS (lb/size) 6=570/0-4-0, 2=660/0-4-0
Max Horz 2=483(load case 5)
Max Uplift6=-287(load case 5), 2=-159(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-554/66, 3-4=-411/99, 4-5=-231/159, 5-6=-523/369
BOT CHORD 2-7=-337/346, 6-7=-7/9
WEBS 3-7=-163/264, 4-7=-41/152, 5-7=-316/453

JOINT STRESS INDEX
2 = 0.67, 2 = 0.00, 3 = 0.15, 4 = 0.50, 5 = 0.41, 6 = 0.30 and 7 = 0.50

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 6 and 159 lb uplift at joint 2.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061165

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T35	MONO HIP	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 12:51:00 2006 Page 1

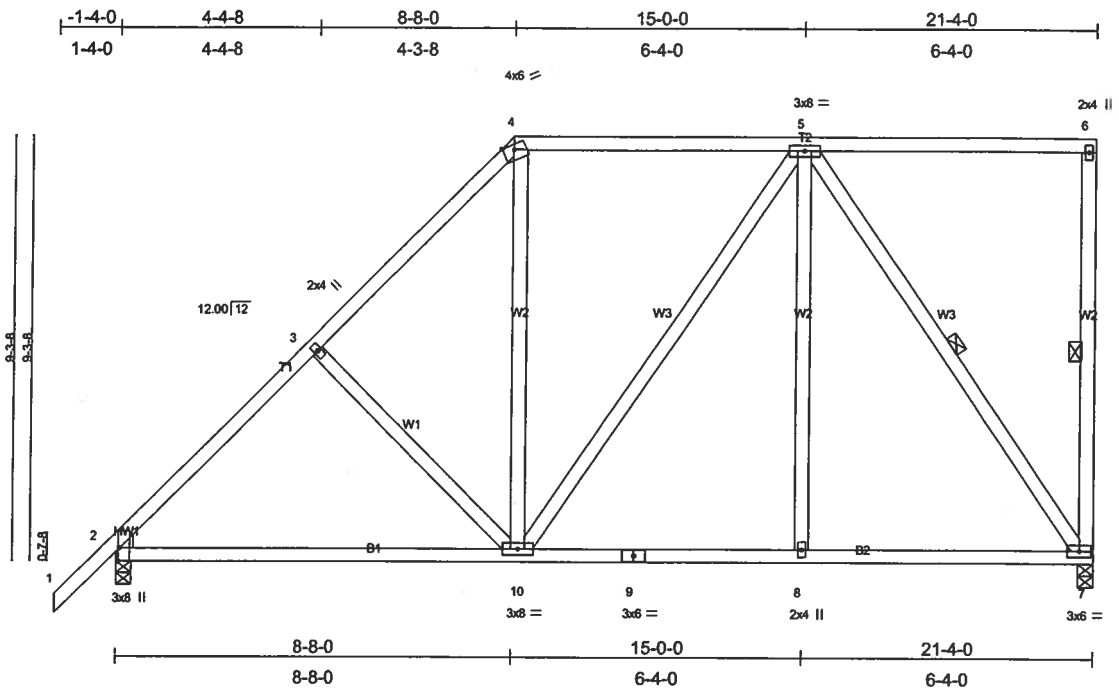


Plate Offsets (X,Y): [2:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.13	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.22	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 9-3-0 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7
JOINTS 1 Brace at Jt(s): 6

REACTIONS (lb/size) 7=880/0-4-0, 2=967/0-4-0
Max Horz 2=483(load case 5)
Max Uplift7=-349(load case 4), 2=-281(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-966/273, 3-4=-822/305, 4-5=-528/304, 5-6=-16/7, 6-7=-146/118
BOT CHORD 2-10=-465/621, 9-10=-231/458, 8-9=-231/458, 7-8=-231/458
WEBS 3-10=-139/256, 4-10=-8/242, 5-10=-160/122, 5-8=0/173, 5-7=-780/396

JOINT STRESS INDEX
2 = 0.72, 2 = 0.00, 3 = 0.34, 4 = 0.68, 5 = 0.58, 6 = 0.83, 7 = 0.42, 8 = 0.34, 9 = 0.16 and 10 = 0.58

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 349 lb uplift at joint 7 and 281 lb uplift at joint 2.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061166

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T36	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 12:51:36 2006 Page 1

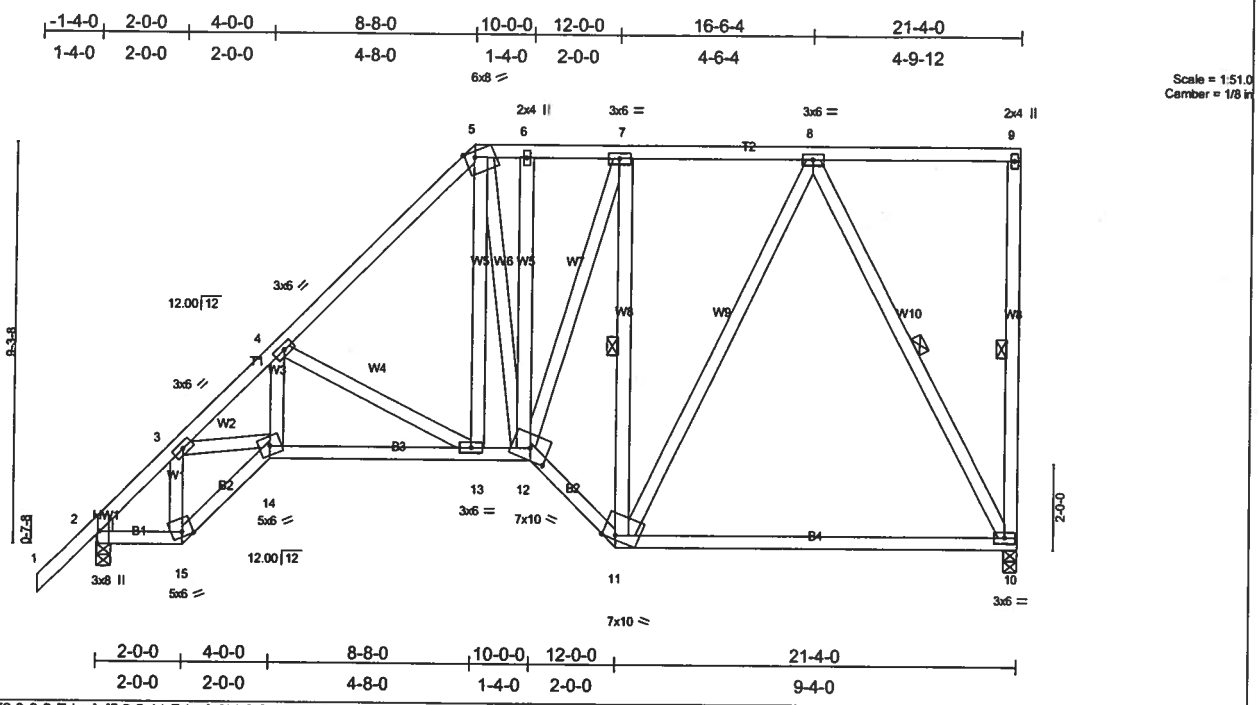


Plate Offsets (X,Y): [2:0-3-8,Edge], [5:0-2-11,Edge], [11:0-3-11,Edge], [12:0-5-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.16	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.27	10-11	>924	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 187 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-7 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-9.
BOT CHORD Rigid ceiling directly applied or 6-10-14 oc bracing.
WEBS 1 Row at midpt 9-10, 7-11, 8-10
JOINTS 1 Brace at Jt(s): 9

REACTIONS (lb/size) 10=880/0-4-0, 2=967/0-4-0
Max Horz 2=483(load case 5)
Max Uplift10=-349(load case 4), 2=-281(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1050/243, 3-4=-1763/843, 4-5=-1003/424, 5-6=-645/377, 6-7=-642/374, 7-8=-537/274, 8-9=-22/1, 9-10=-119/90
BOT CHORD 2-15=-471/617, 14-15=-617/847, 13-14=-840/1198, 12-13=-397/660, 11-12=-381/650, 10-11=-191/351
WEBS 3-15=-539/453, 3-14=-425/659, 4-14=-392/662, 4-13=-617/512, 5-13=-193/492, 5-12=-179/191, 6-12=-105/68, 7-12=-345/365, 7-11=-544/468, 8-11=-186/416, 8-10=-714/415

JOINT STRESS INDEX
2 = 0.66, 2 = 0.00, 3 = 0.47, 4 = 0.54, 5 = 0.46, 6 = 0.34, 7 = 0.51, 8 = 0.47, 9 = 0.74, 10 = 0.59, 11 = 0.67, 12 = 0.31, 13 = 0.35, 14 = 0.41 and 15 = 0.27

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 349 lb uplift at joint 10 and 281 lb uplift at joint 2.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061167

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T37	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 12:56:32 2006 Page 1

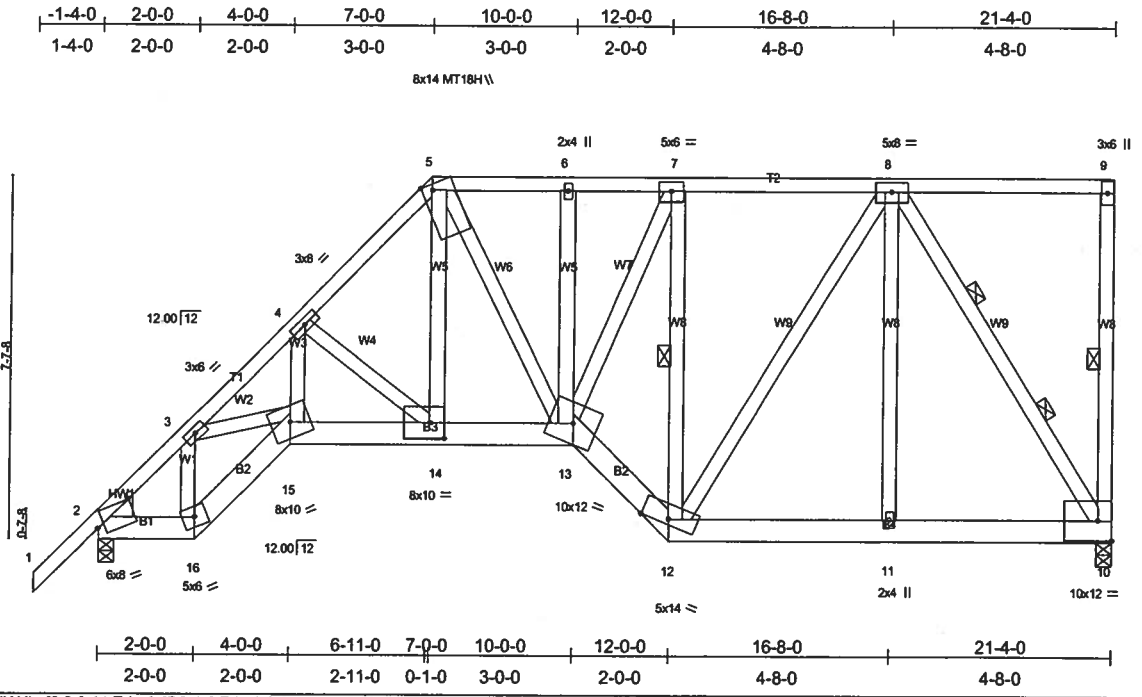


Plate Offsets (X,Y): [2:0-9-11,Edge], [5:0-1-9,Edge], [14:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.15	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.24	13-14	>999	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.20	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-1 oc bracing.
WEBS 1 Row at midpt 9-10, 7-12
2 Rows at 1/3 pts 8-10

REACTIONS (lb/size) 10=3322/0-4-0, 2=2364/0-4-0
Max Horz 2=269(load case 4)
Max Uplift10=-1501(load case 2), 2=-917(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-2961/1065, 3-4=-5092/1988, 4-5=-3738/1489, 5-6=-2901/1225, 6-7=-2899/1225, 7-8=-2274/990, 8-9=-38/19, 9-10=-370/214
BOT CHORD 2-16=-793/1854, 15-16=-992/2340, 14-15=-1414/3474, 13-14=-998/2674, 12-13=-1155/2914, 11-12=-741/1666, 10-11=-742/1662
WEBS 3-16=-1421/673, 3-15=-708/1822, 4-15=-597/1505, 4-14=-1084/555, 5-14=-777/1931, 5-13=-344/468, 6-13=-20/72, 7-13=-666/1668, 7-12=-2095/890, 8-12=-484/1125, 8-11=-252/718, 8-10=-3069/1365

JOINT STRESS INDEX
2 = 0.78, 3 = 0.83, 4 = 0.94, 5 = 0.82, 6 = 0.34, 7 = 0.77, 8 = 0.77, 9 = 0.45, 10 = 0.51, 11 = 0.54, 12 = 0.93, 13 = 0.59, 14 = 0.40, 15 = 0.77 and 16 = 0.50

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1501 lb uplift at joint 10 and 917 lb uplift at joint 2.
6) Girder carries tie-in span(s): 3-4-15 from 7-0-0 to 12-0-0; 7-5-4 from 7-0-0 to 12-0-0; 8-8-0 from 12-0-0 to 21-4-0; 8-8-0 from 12-0-0 to 21-4-0
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 778 lb down and 369 lb up at 6-11-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-7=-77(F=-23), 7-9=-187(F=-133), 2-16=-30, 15-16=-30, 14-15=-30, 13-14=-137(F=-107), 12-13=-137(F=-107), 10-12=-163(F=-133)
Concentrated Loads (lb)
Vert: 14=-778(F)

AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0829061168

Job	Truss	Truss Type	Qty	Ply	BLAKE CONSTRUCTION LOT-18 COUNTRY LAKE
L207624	T38	CAL. DUAL PITCH	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 13:43:10 2006 Page 1

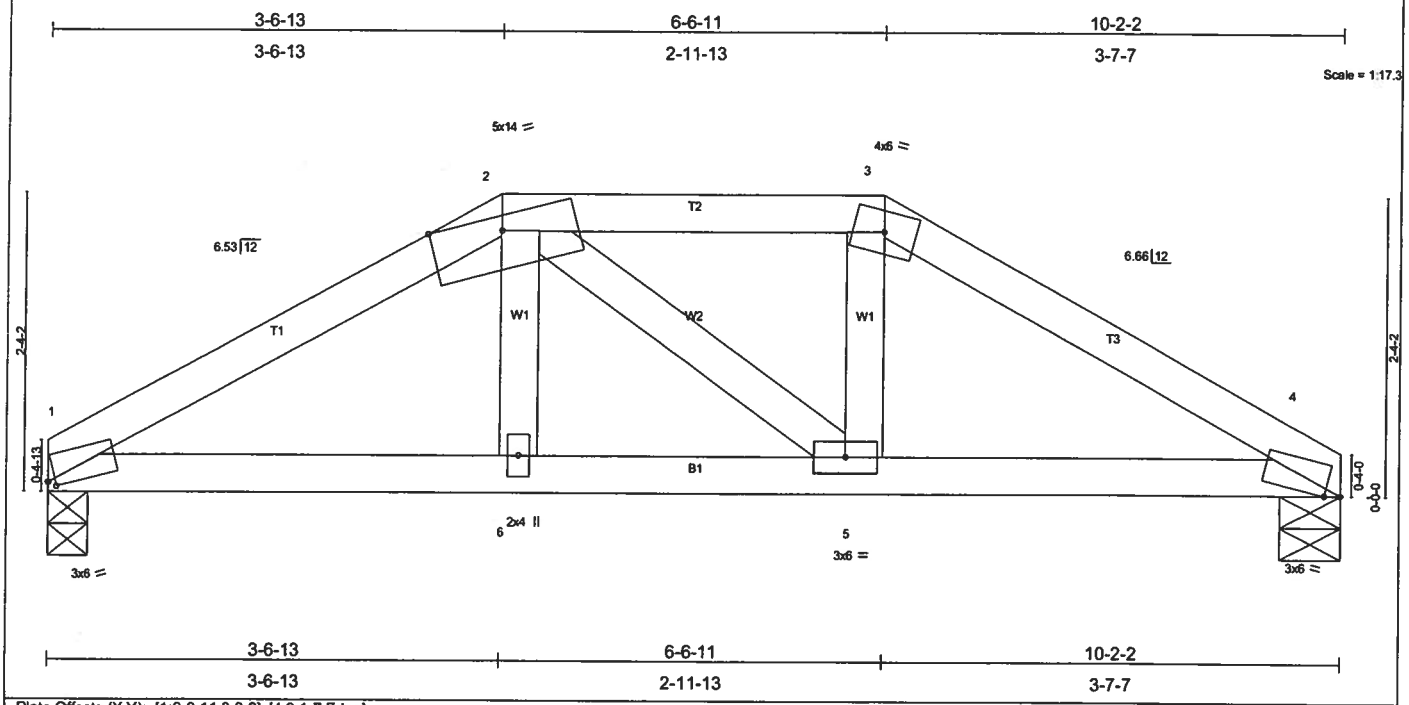


Plate Offsets (X,Y): [1:0-0-11,0-0-9], [4:0-1-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.04	1-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.07	1-6	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 42 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-1 oc bracing.

REACTIONS (lb/size) 1=1191/0-3-12, 4=1192/0-5-11
Max Horz 1=61(load case 4)
Max Uplift1=-584(load case 5), 4=-585(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1681/955, 2-3=-1465/883, 3-4=-1681/955
BOT CHORD 1-6=-774/1427, 5-6=-789/1467, 4-5=-772/1425
WEBS 2-6=-212/554, 2-5=-64/64, 3-5=-232/578

JOINT STRESS INDEX
1 = 0.79, 2 = 0.42, 3 = 0.39, 4 = 0.78, 5 = 0.38 and 6 = 0.40

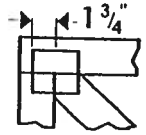
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 584 lb uplift at joint 1 and 585 lb uplift at joint 4.
6) Girder carries tie-in span(s): 8-6-8 from 0-0-0 to 10-2-2
7) Girder carries hip end with 3-7-7 right side setback, 3-6-13 left side setback, and 3-7-7 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 69 lb down and 89 lb up at 6-6-11, and 68 lb down and 88 lb up at 3-6-13 on top chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-72(B=-18), 3-4=-54, 1-4=-170(F=-130, B=-10)
Concentrated Loads (lb)
Vert: 2=-68(B) 3=-69(B)

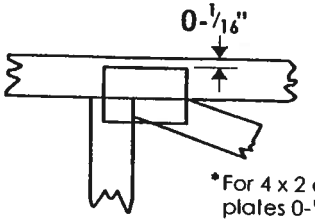
AUGUST 29, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

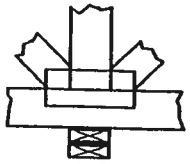
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

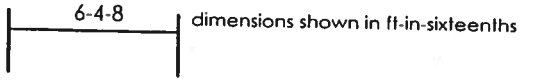


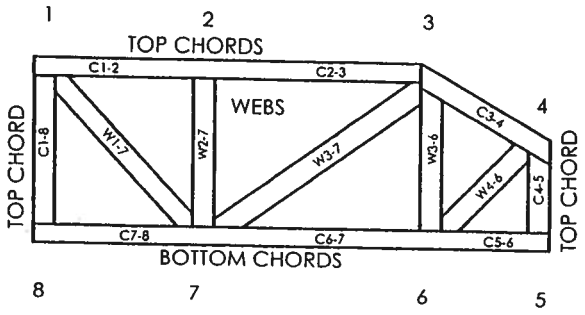
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

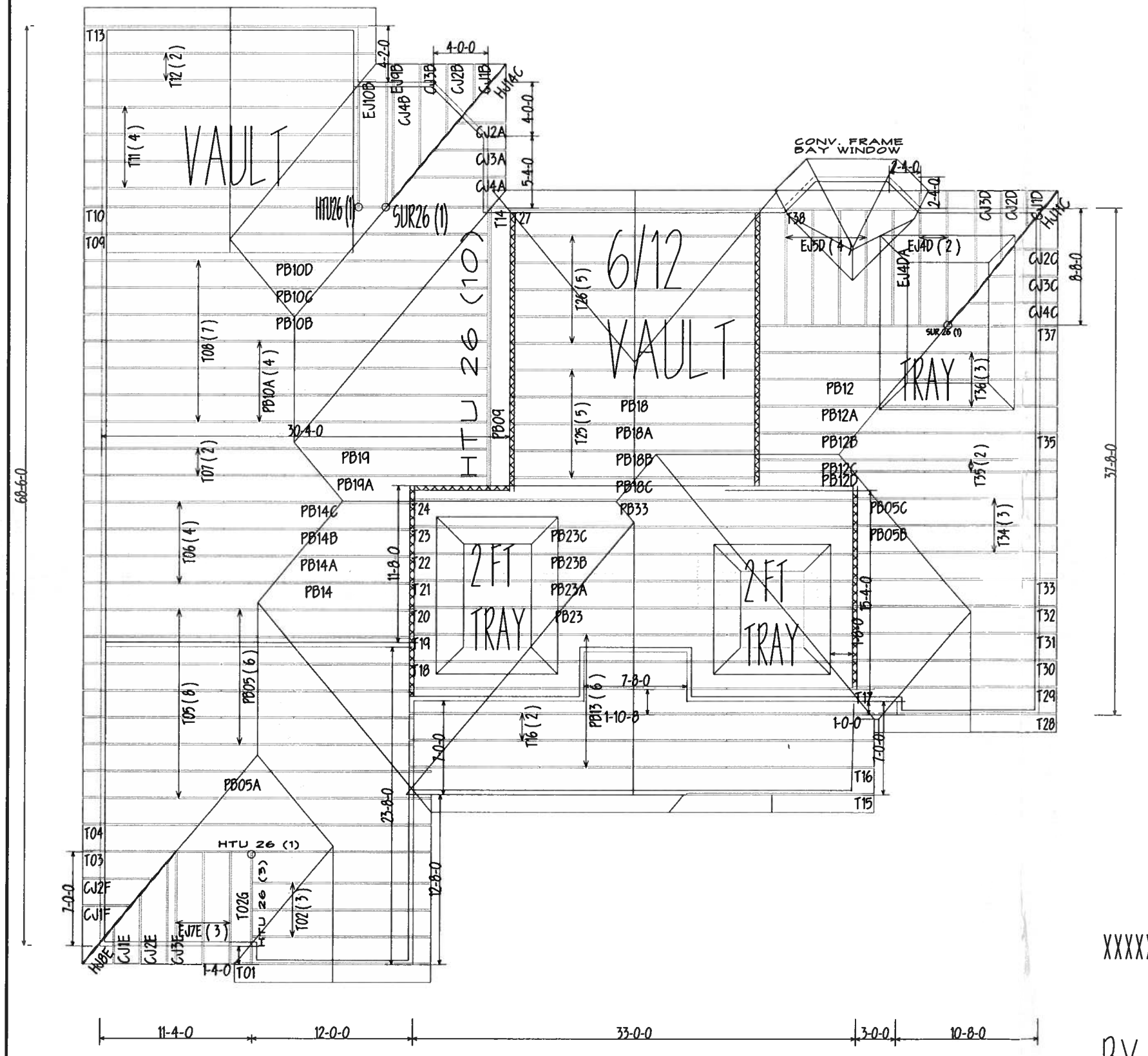
TEE-LOK®

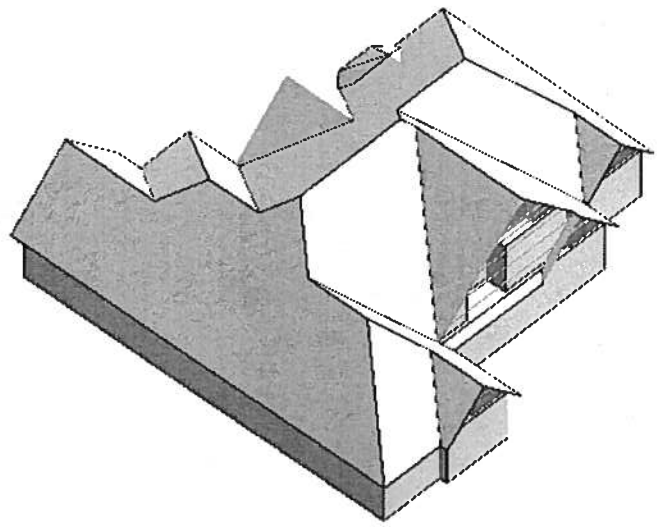
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.





9 FT INTERNAL
LOAD BEARING WALL

XXXXX TRUSSES TO BE VERIFIED AND APPROVED BY BLAKE CONSTRUCTION PRIOR TO PRODUCTION!!! XXXXX

BEARING HEIGHT SCHEDULE

9'0"

10/12, 12/12 PITCH

16" O.H.

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 386-755-6894 FAX: 386-755-7973

Sanford
PHONE: 407-322 0059 FAX: 407 322-5553

BUILDER: BLAKE CONSTRUCTION			
LEGAL ADDRESS: LOT 18 COUNTRY LAKE			
MODEL: CUSTOM		REVISION:	
		SCALE: NTS	
DATE: 8/24/06	DRAWN BY: JOE	JOB #: L207624	