

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055



4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

<b>7</b> &	DATE	DESCRIPTION
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<u>A</u>	-/-/-	

All designs and plans indicated on this drawing are created specifically for the noted project and are the sole property of LINK Engineering, LLC. Use of these designs or plans for any purpose other than the intended application shall be prohibited without the written consent of LINK Engineering, LLC. Disclosure of any of the information enclosed within, without consent of the owner, is a violation of intellectual property and shall not be tolerated.

IMAD N. KASHIF, P.E.

FLORIDA STATE LICENSE NO.: 41374

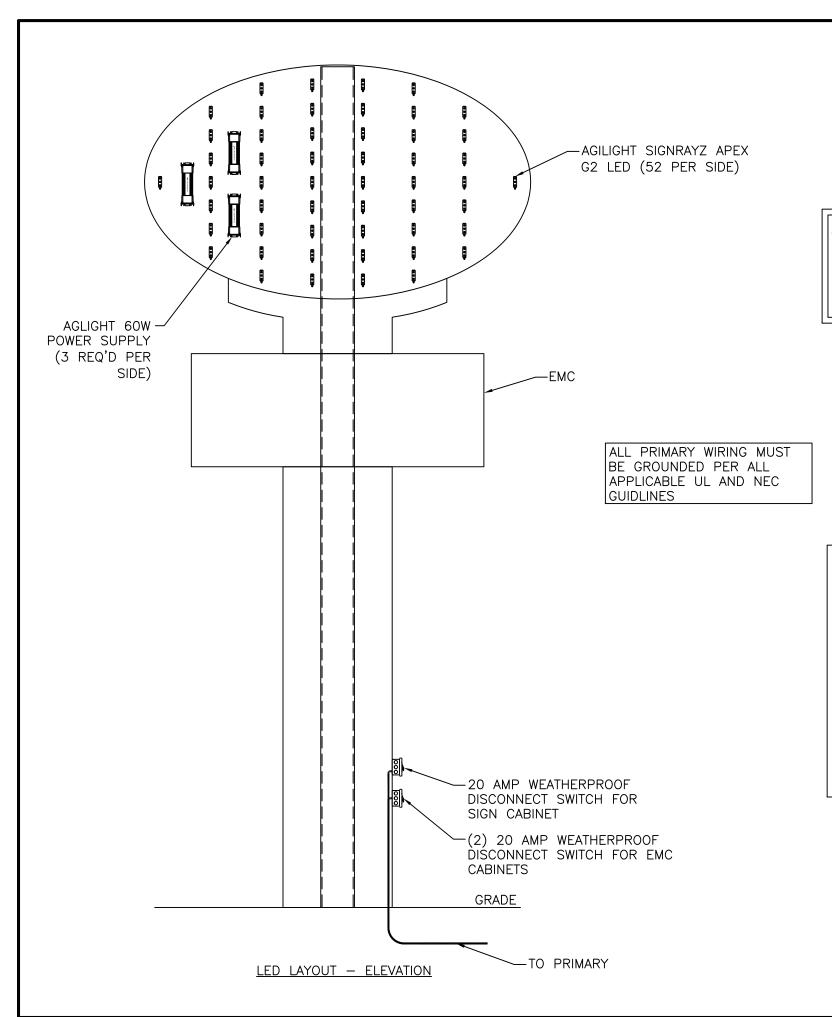
SEAL & SIGNATURE:





## LINK Engineering, L.L.C

Project Nun	nber:	Drawing Number:		
22-02	282R3	B186649R3		
SHT.	OF	DATE:	BY:	
1	3	8/12/22	GHK	



ELECTRICAL REQUIREMENTS (FOR SIGN CABINET): LEDs: (104) AGILIGHT SIGNRAYZ APEX G2 POWER SUPPLY TOTAL: (6) 12V/60W AGILIGHT #PS12-60W-100-277V @ 0.80A

LOAD: 4.8A @ 120VAC

CIRCUITS: (1) 20 AMP REQUIRED

**ELECTRICAL REQUIREMENTS (FOR EMC):** 

VOLTAGE: 120VAC

CIRCUITS: (2) 20 AMP REQUIRED

TOTAL # OF CIRCUITS REQ'D:

CIRCUITS: (3) 20 AMP REQUIRED

#### **ELECTRICAL NOTES:**

- 1. ALL ELECTRICAL COMPONENTS ARE UL LISTED AND APPROVED
- 2. SIGN GROUNDED ACCORDING TO NEC 600.7
- 3. SIGNS MANUFACTURED AND LISTED NEC 600.3 AND MARKED PER NEC 600.4
- 4. ALL BRANCH CIRCUITS PER NEC 600.5(B).1 OR (B).2
- 5. ALL SIGNS SHALL BE CONTROLLED BY PHOTOCELL OR TIME CLOCK
- 6. ONE VISIBLE 20 AMP DISCONNECT PER SIGN PER CIRCUIT PER NEC 600.6(A).1
- 7. ALL CLASS 2 RATED LED MODULES AND LED POWER SUPPLIES WILL BE IN COMPLIANCE WITH NATIONALLY RECOGNIZED TEST LABORATORY

PHOTOCELL, OR ASTRONOMICAL TIMER LIGHT CONTROL MANAGEMENT SYSTEM IS REQUIRED

INSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



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### LINK Engineering, L.L.C

135 South David Lane • Knoxville, Tennessee 3792
Phone: (865) 539-4001 • www.linkengr.com

Project N	umber:	Drawing Number:		
22-	0282R3	B186649R3		
SHT. OF		DATE:	BY:	
2	3	8/12/22	GHK	

	PROJECT#				CULVER'S				LONG BOTTOM STEEL		REBAR SIZE	4.000	
	August 12, 2022					MMONS LOOF	,				WEIGHT PER FT	0.668	
	DRAWING #	R186649R3			LAKE CITY, F						SPACING	14.000	i
	WNDLOAD	25.37	PSF		Dark Giri, i	L 02000				1	AREA PER BAR	0.200	in
	WIND SPEED	120	MPH	CL IENTI-	SPRINGFIELD	SIGN & NEO	N		BOTTOM STEEL AREA REQ	D DED ET OF WIDTH	ANDATONI	0.172	
	# COLUMNS	1	Florida Building		2531 N. PAT				DOTTOWISTELL AND NEW	DIECTION WIDTH	AREA PER FT	0.172	•
	DESIGNER	GHK		code	SPRINGFIELD						EST NO. REQ'D	3.643	
	DESIGNER	GHK	7th Ed (2020)		SPRINGFIELL	J, IVIO						4.000	
			OLIA DE	OF TROOP		TOTAL					NUMBER REQ'D		
	1 IFO I F	I AMOTE A	SHAPE	CENTROID	1051	TOTAL	100100				LENGTH	5.750	f
ITEM	HEIGHT	WIDTH	FACTOR	HEGHT	AREA	FORCE	MOMENT				WEGHT	15.364	
				=======							EDGE	4.500	-
SIGN	5.000	8.250	0.801	2.453	33.054	0.839	2.057		LONG TOP STEEL		REBAR SIZE	4.000	
CLADDING	1.167	4.667	0.610	0.663	3.321	0.923	3.092				WEIGHT PER FT	0.668	
EMC	2.417	6.250	1.000	1.208	15.104	1.306	5.785				SPACING	14.000	i
CLADDING	9.417	2.333	1.000	4.708	21.972	1.864	20.711				AREA PER BAR	0.200	in
OAH	18.000								TOP STEEL AREA REQ'D PE	R FT OF WIDTH		0.061	
											AREA PER FT	0.188	
	COLUMN CA	LCULATIONS	(CODES	P=PIPE;O=OTHER;	T=TUBE)						EST NO. REQ'D	3.643	
					DESIGN		AVAILABLE				NUMBER REQ'D	4.000	
	COLUMN	COLUMN	COLUMN	lxx	MODULUS	REQUIRED	FLEXURAL				LENGTH	5.750	f
ITEM	WIDTH	DEPTH	WALL	COLUMN	COLUMN	MOMENT	STRENGTH	UNITY			WEIGHT	15.364	
											EDGE	4.500	
SIGN		8.625	0.300	68.1	20.80	2.057	36.33	0.057	CROSS STEEL		REBAR SIZE	4.000	•
CLADDING		8.625	0.300	68.1	20.80	3.092	36.33	0.085	0.000 0.000		WEIGHT PER FT	0.668	
EMC		8.625	0.300	68.1	20.80	5.785	36.33	0.159			SPACING	13.000	1
CLADDING		8.625	0.300	68.1	20.80	20.711	36.33	0.570			LENGTH	3.750	1
CLADDING		0.025	0.300	00.1	20.00	20.711	30.33	0.570			EST NO. REQ'D	5.769	
SPREAD FOUNDATI	ON										NUMBER REQ'D	6.000	
MOMENT AT GRADI	(-1,10),1				20.744						- 12 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		
	-				20.711	1.1.					EDGE	5.000	
TOTAL FORCE					1.864	kip					WEIGHT	30.060	
WEIGHT OF SIGN					1.129	kip					TOTAL WEIGHT	60.788	
SLAB WIDTH					4.250	ft						7.22	
SLAB LENGTH					6.250	ft			FOUNDATION WIDTH			4.250	1
SLAB DEPTH					3.000	ft			FOUNDATION LENGTH			6.250	f
SLAB WEIGHT					11.953	kip							
TOTAL WEIGHT					13.082	kip			CAISSON				
OVERTURNING MON	MENT				26.302				MOMENT			20.711	FT-
FACTOR OF SAFET	Y				1.554				FORCE			1.864	К
e = OTMWT					2.011				REFERENCE IBC 1807.3.2 &	TABLE 1806.2			
L/2 - e					1.114				ASSUME SOIL CLASS #4 S	W, SP, SM, SC, GM & GC			
SOIL PRESSURE 2*	NT/(3*(L/2-e)*WIDTH)				1841				LATERAL BEARING PRESSI	JRE - PSF/FT OF DEPTH		150.0	PSF
CONCRETE					2.951	yd³			S1			750.0	
EXCAVATION					2.951	yd <sup>3</sup>			DEPTH			7.500	F
MIN. THICKNESS W	O REBAR				16.212	in			DIAMETER			2.500	F
SQRT(M*12*6*1.7*.7												11.112	F
ACTUAL THICKNES					36.000	in						2.326	F
	-								CALCULATED DEPTH			6.597	F
BOTTOM STEEL AS	EA REQ'D PER FT OF	WIDTH			0.172				MINIMUM THICKNESS WITHO	UT RENEORCEMENT		30.392	i
	EQ'D PER FT OF WIDT				0.061				THE PROPERTY OF THE PARTY OF TH	.C. ALIN ON WILLY		50.552	
TOF SILLE AREA R	EXPLEXE OF WIDI				0.001				ACTUAL DIAMETER			30.000	1
									ACTUAL DIAMETER			30.000	(1
									CONCRETE			1.364	CU

- 1. Design is based on a 120 mph, 3 second gust wind design per Florida Building Code 7th Ed. (2020). Category II, Exposure C.
- 2. Spread foundation is based on a safe vertical soil bearing pressure minimum of 2000 psf. Caisson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short—term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
- 3. A soil report was not provided. Foundation analysis assumes Soil Classification 4.
  Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact SPRINGFIELD SIGN & NEON.
- Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re—evaluated by a competent Professional Engineer. Do not place foundation in fill.
   Concrete shall be mixed to attain a
- minimum 28 day compressive strength of 3000 psi.
- Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.
- All support members shall be free from defects. Steel Pipe up to 24 inch O.D. shall meet ASTM A53 Grade B with a minimum yield strength of 35000 psi. Steel angle, channel and plate shall meet ASTM A36.
   Steel welds shall be made with E70xx low
- Steel welds shall be made with E70xx low hydrogen electrodes by persons qualified in accordance with AWS standards within the past two years.
- All structural bolts shall conform to ASTM A325, and be zinc coated unless noted otherwise. When used with structural bolts, heavy hex nuts shall conform to ASTM A563, and washers shall conform to ASTM F436. Pretension all high strength bolts using the Turn—of—Nut method unless noted otherwise.
- 10. The scope of this engineer does not include onsite observations.
- 11. LINK Engineering will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the owners, contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.
- 12. Any deviation from this design or from any part of this drawing, including the General Notes, without prior written consent from LINK Engineering voids this drawing in its entirety.
- 13. The structure designed on this drawing is intended to be installed at the address shown and should not be used at any other location.

INSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



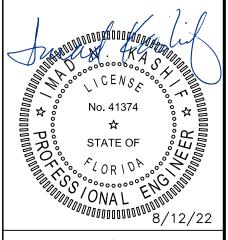
4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

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IMAD N. KASHIF, P.E.
FLORIDA STATE LICENSE NO.: 41374

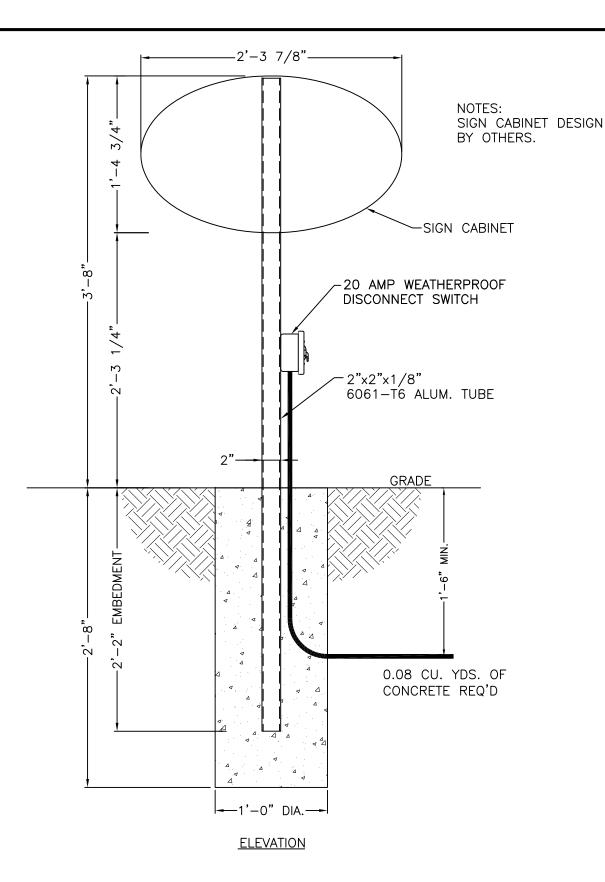
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## LINK Engineering, L.L.C

Project Nun	nber:	Drawing Number:		
22-02	282R3	B186649R3		
SHT.	OF	DATE:	BY:	
3	3	8/12/22	GHK	



EMBEDDED ALUM. SHALL BE COATED WITH CARBOLINE BITUMASTIC 50 COAL TAR OR EQUAL PRIOR TO INSTALLATION.

#### General Notes:

CONCRETE

- Design is based on a 120 mph, 3 second gust wind design per Florida Building Code, 7th Edition (2020). Category II, Exposure C.
- 2. Caisson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short—term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
- A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact SPRINGFIELD SIGN & NEON.
- Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent
  to a fill slope unless re—evaluated by a competent Professional Engineer. Do not place foundation
  in fill.
- 5. Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.
- 6. All support members shall be free from defects. Aluminum tube shall be 6061—T6 alloy, channel shall be 6063—T6 and aluminum sheet 3003 H14.
  7. Aluminum welds shall be made with 5356 filler by persons qualified in accordance with AWS
- Aluminum welds shall be made with 5356 filler by persons qualified in accordance with AWS standards within the past two years.
- 3. All fasteners shall be zinc coated to prevent corrosion.

not be used at any other location.

- 9. The scope of this engineer does not include onsite observations.
- 0. LINK Engineering will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the owners, contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.
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  12. The structure designed on this drawing is intended to be installed at the address shown and should

PROJECT # 22-0282R OWNER: CULVER'S July 14, 2022 255 N.W. COMMONS LOOP DRAWING# B186705 LAKE CITY, FL 32055 WINDLOAD **PSF** 23.96 WIND SPEED 120 MPH CLIENT: BLAIR SIGN COMPANY # COLUMNS FLORIDA BUILDING CODE 5107 KISSELL AVENUE DESIGNER SR ALTOONA, PA 7th Ed. (2020) SHAPE CENTROID TOTAL ITEM HEIGHT WIDTH **FACTOR** HEIGHT AREA FORCE MOMENT ======= ======= ======= ======= ====== ====== ====== ======= CABINET 1.396 2.323 0.698 2.547 0.061 0.043 COLUMN 2.271 0.167 1.111 1.135 0.421 0.071 0.193 OAH 3.667 COLUMN CALCULATIONS (CODES P=PIPE; O=OTHER; T=TUBE) **DESIGN** AVAILABLE COLUMN COLUMN COLUMN MODULUS REQUIRED FLEXURAL lxx DEPTH UNITY ITEM WIDTH WALL COLUMN COLUMN MOMENT STRENGTH ======= ====== ======= ======= ======= CABINET 2.000 2.000 0.125 0.552 0.552 0.043 0.897 0.047 COLUMN 2.000 2.000 0.125 0.552 0.552 0.193 0.897 0.215 CAISSON MOMENT 0.193 FT-KIP 0.071 KIP FORCE REFERENCE IBC 1807.3.2 & TABLE 1806.2 ASSUMESOIL CLASS #4 SW, SP, SM, SC, GM & GC LATERAL BEARING PRESSURE - PSF/FT OF DEPTH 150.0 PSF/FT S1 266.7 DEPTH 2.667 FT. DIAMETER 1.000 FT. 2.709 FT. FT. 0.624 FT. CALCULATED DEPTH 1.705 MINIMUM THICKNESS WITHOUT REINFORCEMENT 7.971 IN. ACTUAL DIAMETER 12.000 IN.

0.078

CU. YD.

INSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY. FL 32055

CLIENT:



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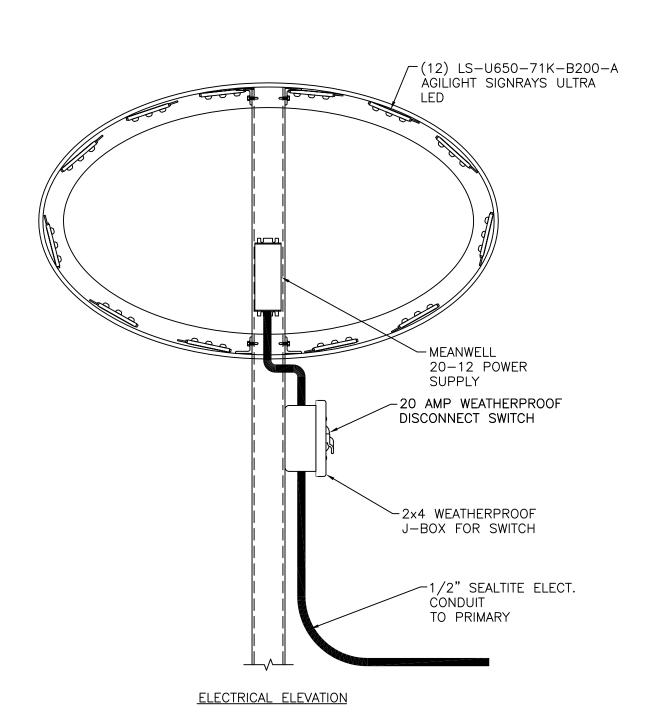
SEAL & SIGNATURE:





## LINK Engineering, L.L.O

Project Nun	nber:	Drawing Number:		
22-02	282R3	B186705R2		
SHT.	OF	DATE:	BY:	
1	2	8/12/22	GHK	



#### **ELECTRICAL NOTES:**

- 1. ALL ELECTRICAL COMPONENTS ARE UL LISTED AND APPROVED
- 2. SIGN GROUNDED ACCORDING TO NEC 600.7
- 3. SIGNS MANUFACTURED AND LISTED NEC 600.3 AND MARKED PER NEC 600.4
- 4. ALL BRANCH CIRCUITS PER NEC 600.5(B).1 OR (B).2
- 5. ALL SIGNS SHALL BE CONTROLLED BY PHOTOCELL OR TIME CLOCK
- 6. ONE VISIBLE 20 AMP DISCONNECT PER SIGN PER CIRCUIT PER NEC 600.6(A).1
- 7. ALL CLASS 2 RATED LED MODULES AND LED POWER SUPPLIES WILL BE IN COMPLIANCE WITH NATIONALLY RECOGNIZED TEST LABORATORY

**ELECTRICAL REQUIREMENTS:** 

LEDs: (12) AGILIGHT SIGNRAYS ULTRA LED

#LS-U650-71K-B200-A

POWER SUPPLY: (1) MEANWELL POWER SUPPLY

#MDR-20-12 @ 0.55A

TOTAL LOAD: .55Ä @ 120VAC CIRCUITS: (1) 20 AMP REQUIRED

PHOTOCELL, OR ASTRONOMICAL TIMER LIGHT CONTROL MANAGEMENT SYSTEM IS REQUIRED

INSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



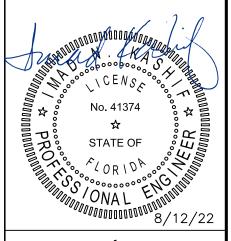
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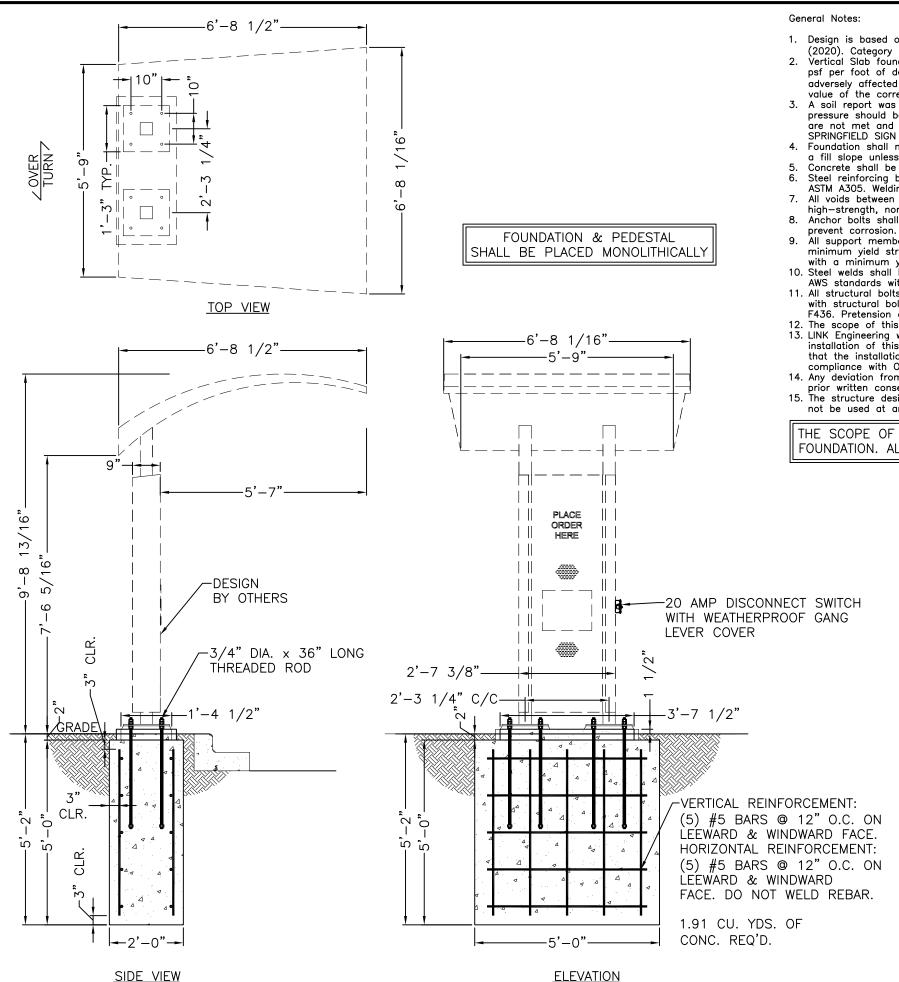
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## LINK Engineering, L.L.C.

Project N	lumber:	Drawing Number:		
22-	0282R3	B186705R2		
SHT.	OF	DATE:	BY:	
2	2	8/12/22	l GHK l	



- Design is based on a 120 mph, 3 second gust wind design per Florida Building Code, 7th Edition (2020). Category II, Exposure C.
- 2. Vertical Slab foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short-term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
- 3. A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact SPRINGFIELD SIGN & NEON.
- Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re-evaluated by a competent Professional Engineer. Do not place foundation in fill.
   Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.
   Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with
- ASTM A305. Welding of reinforcing bars is prohibited.

  All voids between column base plate and foundation surface shall be completely filled with
- high-strength, non-shrink grout.

  Anchor bolts shall meet ASTM F1554 Grade 36. Exposed surfaces shall be galvanized or coated to
- All support members shall be free from defects. Steel tube shall meet ASTM A500 Grade B with a minimum yield strength of 46000 psi. Steel Pipe up to 24 inch 0.D. shall meet ASTM A53 Grade B with a minimum yield strength of 35000 psi. Steel angle, channel and plate shall meet ASTM A36.
- 10. Steel welds shall be made with E70xx low hydrogen electrodes by persons qualified in accordance with AWS standards within the past two years.
- 11. All structural bolts shall conform to ASTM A325, and be zinc coated unless noted otherwise. When used with structural bolts, heavy hex nuts shall conform to ASTM A563, and washers shall conform to ASTM F436. Pretension all high strength bolts using the Turn-of-Nut method unless noted otherwise.
- 12. The scope of this engineer does not include onsite observations.
- 13. LINK Engineering will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the owners, contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.
- 14. Any deviation from this design or from any part of this drawing, including the General Notes, without prior written consent from LINK Engineering voids this drawing in its entirety.
- 15. The structure designed on this drawing is intended to be installed at the address shown and should not be used at any other location.

THE SCOPE OF THIS ENGINEER IS LIMITED TO THE DESIGN OF THE ANCHORS, & FOUNDATION. ALL OTHER ASPECTS OF THIS PROJECT ARE EXPLICITLY EXCLUDED.

#### **ELECTRICAL REQUIREMENTS:**

POWER SUPPLY: 40 WATT POWER SUPPLY

TOTAL LOAD: .43A @ 120VAC CIRCUITS: (1) 20 AMP REQUIRED

#### **ELECTRICAL NOTES:**

- 1. ALL ELECTRICAL COMPONENTS ARE UL LISTED AND **APPROVED**
- SIGN GROUNDED ACCORDING TO NEC 600.7
- SIGNS MANUFACTURED AND LISTED NEC 600.3 AND MARKED PER NEC 600.4
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- ALL CLASS 2 RATED LED MODULES AND LED POWER SUPPLIES WILL BE IN COMPLIANCE WITH NATIONALLY RECOGNIZED TEST LABORATORY

#### NSTALLATION ADDRESS:

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CLIENT:



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IMAD N. KASHIF, P.E. FLORIDA STATE LICENSE NO.: 41374 SEAL & SIGNATURE:

8/12/22



## LINK Engineering, L.L.

roject Number: Prawing Number: 22-0282R3 B186706R2 8/12/22 GHK

	PROJECT#	22-0282R		OWNER:	CULVER'S		
	July 14, 2022				255 N.W. CC	MMONS LOOP	
	DRAWING#	B186706, B18	6707		LAKE CITY,	FL 32055	
	WIND LOAD	24.749	PSF				
	WIND SPEED	120	MPH	CLIENT:	SPRINGFIELD	SIGN & NEON	I
	# COLUMNS	2	FLORIDA BUILDI	NG CODE	2531 N. PAT	TERSON	
	DESIGNER	SR	7TH EDITION (20	20)	SPRINGFIELD	D, MO	
			SHAPE	CENTROID		TOTAL	
ITEM	HEIGHT	WIDTH	FACTOR	HEIGHT	AREA	FORCE	MOMENT
======						======	
CANOPY	2.206	6.208	1.000	1.103	13.694	0.339	0.374
COLUMN	0.510	2.604	1.000	0.255	1.329	0.372	0.555
CLADDING	6.893	2.604	1.000	3.447	17.951	0.816	4.649
PEDESTAL	0.125	3.750	1.000	0.063	0.469	0.828	4.752
SUBGRADE	0.167			0.083	0.000	0.828	4.890
OAH	9.734						
DL MOMENT =	1.093		DL+WL	5.983			
	BOI	LT CALCULATION	ONS				
		*******		OBLIQUE			
		BOLT	BOLTS/	TENSION	BOLT	ALLOW.	ALLOWABLE
ITEM	MOMENT	SPA CING	PLATE	BOLT	DIAM.	STRESS	TENSION
BASE PL.	5.983	10.000	4.000	3.758	0.750	19.100	8.438
ATTE COMMITTEE CONTINUES	19 July 20 A. S.			CCTPACE ALLEGATE			2 No. 5 (1) No. 1 (2) 2 (1)
ANCHOR BOLT PRO	DJECTION AN	CHOR EMBEDIV	IENT	ANCHO	OR BOLT MIN. I	_ENGTH	
3.750		9.968			14.000		
		- Factor 2, - 2 See 2 H , - 5 -					
VERTICAL SLAB							
MOMENT					5.983	FT-KIP	
FORCE					0.828	KIP	
REFERENCE IBC 180	07.3.2 & TABLE 1806	.2					
ASSUME SOIL CLA	SS #4 SW, SP, SM, S	C, GM & GC					
	PRESSURE - PSF/FT				150.0	PSF/FT	
S1					500.0		
SLAB DEPTH (d)					5.000	FT.	
SLAB WIDTH (b)					5.000	FT.	
SLAB THICKNESS					2.000	FT.	
AVERAGE HEIGHT	(h)				7.229	FT.	
A = 2.34*FORCE/(S					0.719	23 (8)	
CALCULATED DEPT					2.767	FT.	
	S WITHOUT REBAR				8.033	IN.	
	75/(.178*12*WIDTH))						
					24.000	IN.	
ACTUAL THICKNES	S				24.000	IIN.	

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

\$2	DATE	DESCRIPTION
Λ	-/-/-	
2	-/-/-	
<u>A</u>	-/-/-	

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IMAD N. KASHIF, P.E.

FLORIDA STATE LICENSE NO.: 41374

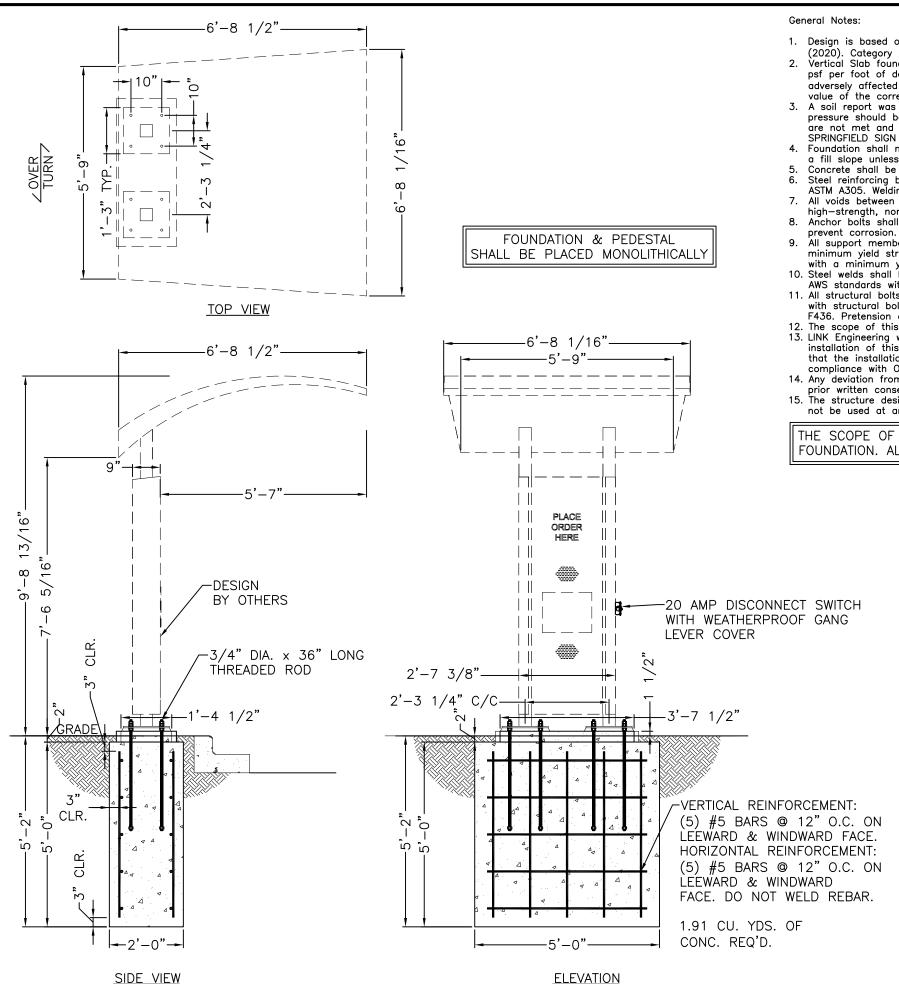
SEAL & SIGNATURE:





# LINK Engineering, L.L.C. 135 South David Lane • Knoxville, Tennessee 37922 Phone: (865) 539-4001 • www.linkengr.com

Project N	umber:	Drawing Number:		
22-	0282R3	B186706R2		
SHT.	OF	DATE:	BY:	
2	2	8/12/22	GHK	



- Design is based on a 120 mph, 3 second gust wind design per Florida Building Code, 7th Edition (2020). Category II, Exposure C.
- 2. Vertical Slab foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short-term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
- 3. A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact SPRINGFIELD SIGN & NEON.
- Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re-evaluated by a competent Professional Engineer. Do not place foundation in fill.
   Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.
   Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with
- ASTM A305. Welding of reinforcing bars is prohibited.

  All voids between column base plate and foundation surface shall be completely filled with
- high-strength, non-shrink grout.

  Anchor bolts shall meet ASTM F1554 Grade 36. Exposed surfaces shall be galvanized or coated to
- prevent corrosion.
- All support members shall be free from defects. Steel tube shall meet ASTM A500 Grade B with a minimum yield strength of 46000 psi. Steel Pipe up to 24 inch 0.D. shall meet ASTM A53 Grade B with a minimum yield strength of 35000 psi. Steel angle, channel and plate shall meet ASTM A36.
- 10. Steel welds shall be made with E70xx low hydrogen electrodes by persons qualified in accordance with AWS standards within the past two years.
- 11. All structural bolts shall conform to ASTM A325, and be zinc coated unless noted otherwise. When used with structural bolts, heavy hex nuts shall conform to ASTM A563, and washers shall conform to ASTM F436. Pretension all high strength bolts using the Turn-of-Nut method unless noted otherwise.
- 12. The scope of this engineer does not include onsite observations.
- 13. LINK Engineering will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the owners, contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.
- 14. Any deviation from this design or from any part of this drawing, including the General Notes, without prior written consent from LINK Engineering voids this drawing in its entirety.
- 15. The structure designed on this drawing is intended to be installed at the address shown and should not be used at any other location.

THE SCOPE OF THIS ENGINEER IS LIMITED TO THE DESIGN OF THE ANCHORS, & FOUNDATION. ALL OTHER ASPECTS OF THIS PROJECT ARE EXPLICITLY EXCLUDED.

#### **ELECTRICAL REQUIREMENTS:**

POWER SUPPLY: 40 WATT POWER SUPPLY

TOTAL LOAD: .43A @ 120VAC CIRCUITS: (1) 20 AMP REQUIRED

#### **ELECTRICAL NOTES:**

- 1. ALL ELECTRICAL COMPONENTS ARE UL LISTED AND **APPROVED**
- SIGN GROUNDED ACCORDING TO NEC 600.7
- SIGNS MANUFACTURED AND LISTED NEC 600.3 AND MARKED PER NEC 600.4
- ALL BRANCH CIRCUITS PER NEC 600.5(B).1 OR (B).2 ALL SIGNS SHALL BE CONTROLLED BY PHOTOCELL OR TIME CLOCK
- ONE VISIBLE 20 AMP DISCONNECT PER SIGN PER CIRCUIT PER NEC 600.6(A).1
- ALL CLASS 2 RATED LED MODULES AND LED POWER SUPPLIES WILL BE IN COMPLIANCE WITH NATIONALLY RECOGNIZED TEST LABORATORY

NSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



**4825 EAST KEARNEY STREET** SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

1	\$	DATE	DESCRIPTION
	A	-/-/-	
е	◬	-/-/-	
	ß	-/-/-	

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IMAD N. KASHIF, P.E. FLORIDA STATE LICENSE NO.: 41374

SEAL & SIGNATURE:





## LINK Engineering, L.L.

roject Number: Prawing Number: B186707R2 22-0282R3 8/12/22 GHK

	PROJECT#	22-0282R		OWNER:	CULVER'S		
	July 14, 2022				255 N.W. CO	DMMONS LOOF	<b>D</b>
	DRAWING#	B186706, B18	36707		LAKE CITY,	FL 32055	
	WINDLOAD	24.749	PSF				
	WIND SPEED	120	MPH	CLIENT:	SPRINGFIEL	D SIGN & NEO	N
	# COLUMNS	2	FLORIDA BUILDII	NG CODE	2531 N. PAT	TERSON	
	DESIGNER	SR	7TH EDITION (20)	20)	SPRINGFIEL	D, MO	
			SHAPE	CENTROID		TOTAL	
ITEM	HEIGHT	WIDTH	FACTOR	HEIGHT	AREA	FORCE	MOMENT
=======	=======	=======	=======			=======	=======
CANOPY	2.206	6.208	1.000	1.103	13.694	0.339	0.374
COLUMN	0.510	2.604	1.000	0.255	1.329	0.372	0.555
CLADDING	6.893	2.604	1.000	3.447	17.951	0.816	4.649
PEDESTAL	0.125	3.750	1.000	0.063	0.469	0.828	4.752
SUBGRADE	0.167	-		0.083	0.000	0.828	4.890
OAH	9.734					,,,,,,	
11.1							
DL MOMENT =	1.093		DL+WL	5.983			
	BOI	TCALCULAT	IONS				
	50.	******	200000000	OBLIQUE			
		BOLT	BOLTS/	TENSION	BOLT	ALLOW.	ALLOWABLE
ПЕМ	MOMENT	SPACING	PLATE	BOLT	DAM.	STRESS	TENSION
======	======	======	======	======	======	======	=======
BASE PL.	5.983	10.000	4.000	3.758	0.750	19.100	8.438
BAGETE.	0.000	10.000	4.000	0.700	0.730	13.100	0.430
ANCHOR BOLT PRO	DIFCTION AN	CHOR EMBEDI	MENT	ANCHO	R BOLT MIN.	LENGTH	
3.750	DOLOTION AN	9.968	VILLY1	ANOIR	14.000	LLINOIII	
3.730		3.300			14.000		
VERTICAL SLAB							
MOMENT					5.983	FT-KIP	
FORCE					0.828	KIP	
	07.3.2 & TABLE 1806	2			0.020	TAIL	
	SS #4 SW, SP, SM, S						
	PRESSURE - PSF/FT				150.0	PSF/FT	
S1	TALOGORE - FOR/FT	O DU-III			500.0	FORFE	
SLAB DEPTH (d)					5.000	FT.	
SLAB WIDTH (b)					5.000	FT.	
SLAB THICKNESS					2.000	FT.	
AVERAGE HEIGHT	(b)				0.000.000.000	FI.	
	1				7.229	FI.	
A = 2.34*FORCE/(S					0.719		
CALCULATED DEPT					2.767	FT.	
	S WITHOUT REBAR				8.033	IN.	
	75/(.178*12*WIDTH))				04.000		
ACTUAL THICKNES	5				24.000	IN.	
CONCRETE					1.908	CU. YD.	

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

<b>%</b>	DATE	DESCRIPTION
$\triangle$	-/-/-	
◬	-/-/-	
<u>\$</u>	-/-/-	

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IMAD N. KASHIF, P.E.

FLORIDA STATE LICENSE NO.: 41374

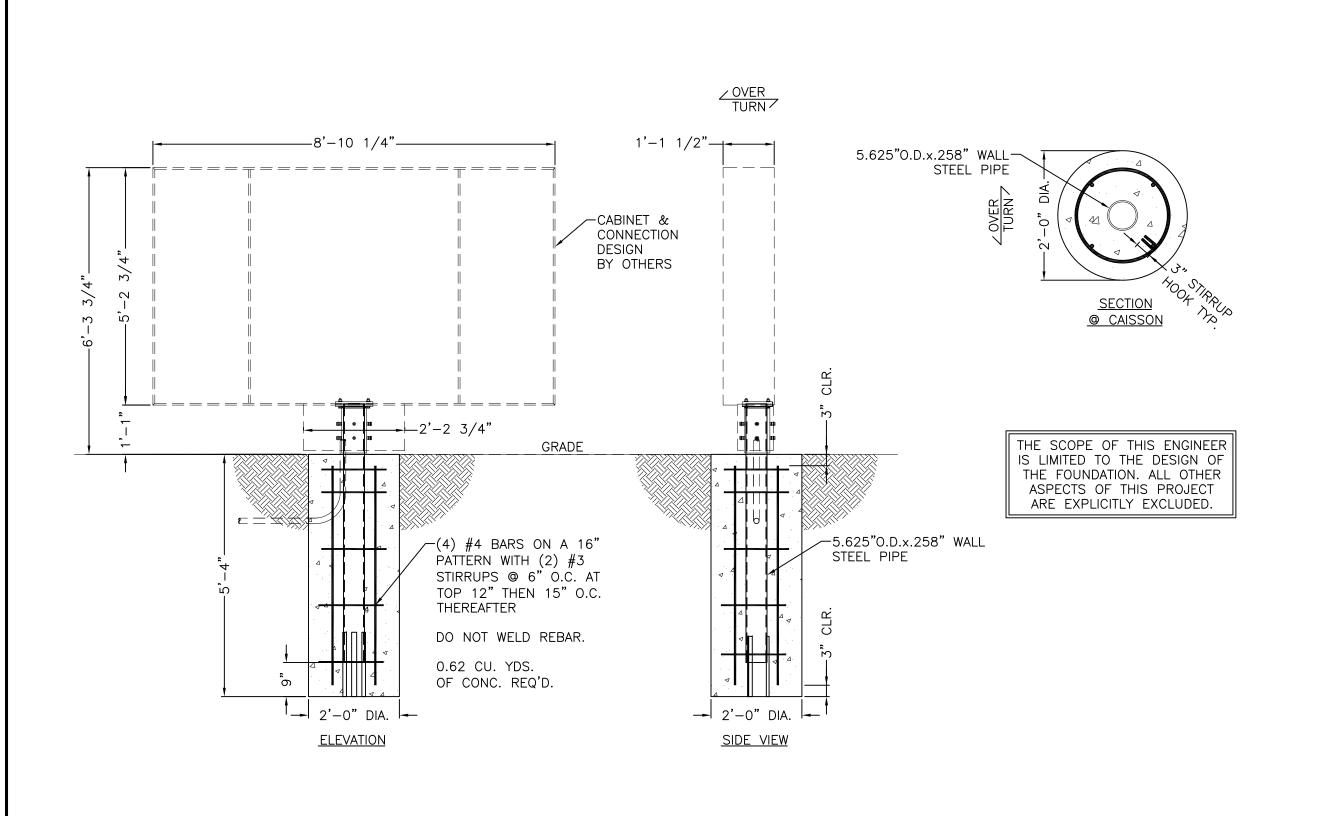
SEAL & SIGNATURE:





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Project Nun	nber:	Drawing Number:		
22-02	282R3	B186707R2		
SHT.	OF	DATE:	BY:	
2	2	8/12/22	GHK	



CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

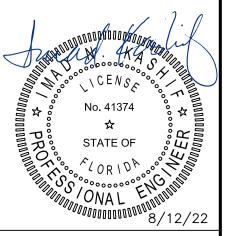
<b>%</b>	DATE	DESCRIPTION
$\triangle$	-/-/-	
◬	-/-/-	
<u>\$</u>	-/-/-	

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IMAD N. KASHIF, P.E.

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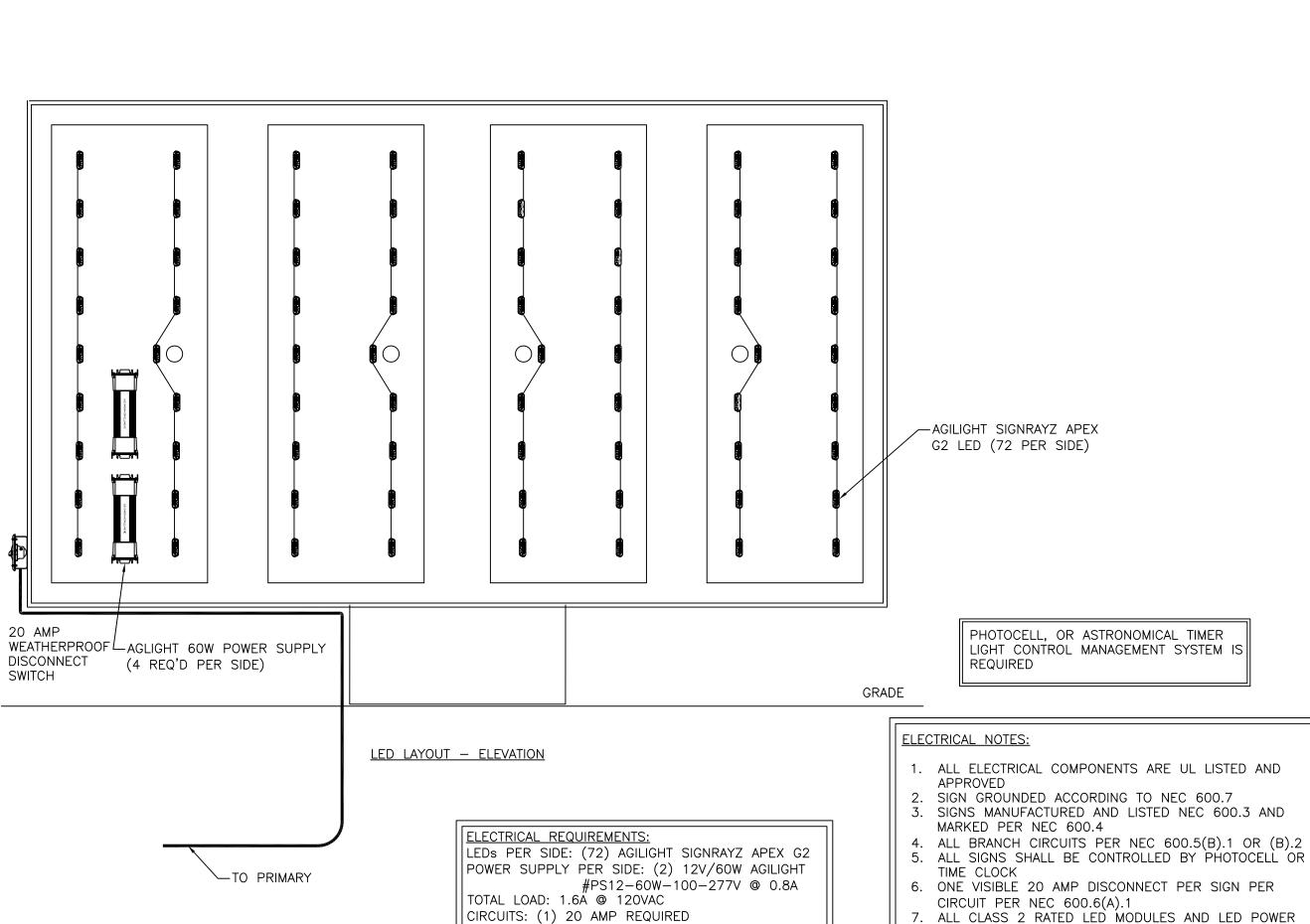
SEAL & SIGNATURE:





## LINK Engineering, L.L.C. 135 South David Lane • Knoxville, Tennessee 37922 Phone: (865) 539-4001 • www.linkengr.com

Project Nun	nber:	Drawing Number:		
22-02	282R3	B186708R2		
SHT.	OF	DATE:	BY:	
1	3	8/12/22	GHK	



CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



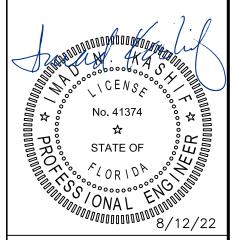
4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

<b>W</b>	DATE	DESCRIPTION
A	-/-/-	
◬	-/-/-	
ß	-/-/-	

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## LINK Engineering, L.L.C

135 South David Lane • Knoxville, Tennessee 3792: Phone: (865) 539-4001 • www.linkengr.com

SUPPLIES WILL BE IN COMPLIANCE WITH NATIONALLY

RECOGNIZED TEST LABORATORY

	-	PROJECT#	22-0282R		OWNER:	CULVER'S			
	Jul	y 14, 2022				255 N.W. CO	MIMONS LOOP		
	D	RAWING#	B186708, B18	6709		LAKE CITY, F	L 32055		
	WIND	LOAD	20.610	PSF					
	WND	SPEED	120	MPH	CLIENT:	SPRINGFIELD	SIGN & NEON	١	
	# COL	LUMNS	1	FLORIDA BUIL	DING CODE	2531 N. PAT	TERSON		
	DES	IGNER	SR	7TH EDITION (2	2020)	SPRINGFIELD	), MO		
				SHAPE	CENTROID		TOTAL		
	ITEM HE	IGHT	WIDTH	FACTOR	HEGHT	AREA	FORCE	MOMENT	
	====== ====							======	
	MENUBOARD 5.2	229	8.854	1.000	2.615	46.300	0.954	2.495	
	SKIRT 1.0	083	2.229	1.000	0.542	2.415	1.004	3.556	
	OAH 6.3	313							
	DL MOMENT 0.	069							
		COLUMN CA	<b>ALCULATIONS</b>	(CODES P	=PIPE;O=OTHE	R;T=TUBE)			
						DESIGN		AVAILABLE	
	COL	LUMN	COLUMN	COLUMN	lxx	MODULUS	REQUIRED	FLEXURAL	
	ITEM W	DTH	DEPTH	WALL	COLUMN	COLUMN	MOMENT	STRENGTH	UNITY
=	======= ====		======	======	======			======	======
Р	SKIRT		5.563	0.241	14.3	6.83	3.625	11.93	0.304
	CAISSON								
	MOMENT								
	J. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.					3.625	FT-KIP		
	FORCE					3.625 1.004	FT-KIP KIP		
		ABLE 1806.	2						
	FORCE								
	FORCE REFERENCE IBC 1807.3.2 & TA	, SP, SM, S	C, GM & GC						
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW	, SP, SM, S	C, GM & GC			1.004	KIP		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUF	, SP, SM, S	C, GM & GC			1.004	KIP		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUR S1	, SP, SM, S	C, GM & GC			1.004 150.0 533.3	KIP PSF/FT		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUR S1 DEPTH	, SP, SM, S	C, GM & GC			1.004 150.0 533.3 5.333	KIP PSF/FT FT.		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUR S1 DEPTH	, SP, SM, S	C, GM & GC			1.004 150.0 533.3 5.333 2.000	PSF/FT FT. FT.		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUR S1 DEPTH	, SP, SM, S	C, GM & GC			1.004 150.0 533.3 5.333 2.000 3.611	PSF/FT FT. FT. FT.		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUR S1 DEPTH DIAMETER	, SP, SM, SI RE - PSF/FT	C, GM & GC OF DEPTH			1.004 150.0 533.3 5.333 2.000 3.611 2.203	PSF/FT FT. FT. FT. FT.		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUR S1 DEPTH DIAMETER  CALCULATED DEPTH	, SP, SM, SI RE - PSF/FT	C, GM & GC OF DEPTH			1.004 150.0 533.3 5.333 2.000 3.611 2.203 4.245	FT. FT. FT. FT. FT. FT.		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUR S1 DEPTH DIAMETER  CALCULATED DEPTH	, SP, SM, SI RE - PSF/FT	C, GM & GC OF DEPTH			1.004 150.0 533.3 5.333 2.000 3.611 2.203 4.245	FT. FT. FT. FT. FT. FT.		
	FORCE REFERENCE IBC 1807.3.2 & TA ASSUME SOIL CLASS #4 SW LATERAL BEARING PRESSUR S1 DEPTH DIAMETER  CALCULATED DEPTH MINIMUM THICKNESS WITHOL	, SP, SM, SI RE - PSF/FT	C, GM & GC OF DEPTH			1.004 150.0 533.3 5.333 2.000 3.611 2.203 4.245 17.882	FT. FT. FT. FT. FT. IN.		

- 1. Design is based on a 120 mph, 3 second gust wind design per Florida Building Code, 7th Edition (2020). Category II, Exposure C.
- 2. Caisson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short—term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
- 3. A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact SPRINGFIELD SIGN & NEON.
- 4. Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re—evaluated by a competent Professional Engineer. Do not place foundation in fill.
- 5. Concrete shall be mixed to attain a minimum 28 day compressive strength of 3000 psi.
- 6. Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.
- 7. All support members shall be free from defects. Steel Pipe up to 24 inch O.D. shall meet ASTM A53 Grade B with a minimum yield strength of 35000 psi. Steel angle, channel and plate shall meet ASTM A36.
- 8. Steel welds shall be made with E70xx low hydrogen electrodes by persons qualified in accordance with AWS standards within the past two years.
- All structural bolts shall conform to ASTM A325, and be zinc coated unless noted otherwise. When used with structural bolts, heavy hex nuts shall conform to ASTM A563, and washers shall conform to ASTM F436. Pretension all high strength bolts using the Turn—of—Nut method unless noted otherwise.
- 10. The scope of this engineer does not include onsite observations.
- 11. LINK Engineering will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the owners, contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.
- 12. Any deviation from this design or from any part of this drawing, including the General Notes, without prior written consent from LINK Engineering voids this drawing in its entirety.13. The structure designed on this drawing is intended to be installed
- 13. The structure designed on this drawing is intended to be installed at the address shown and should not be used at any other location.

#### INSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



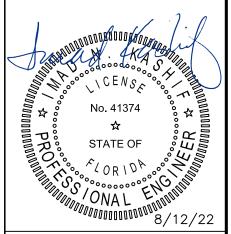
4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

<b>%</b>	DATE	DESCRIPTION
$\triangle$	-/-/-	
◬	-/-/-	
<u>A</u>	-/-/-	

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IMAD N. KASHIF, P.E. FLORIDA STATE LICENSE NO.: 41374

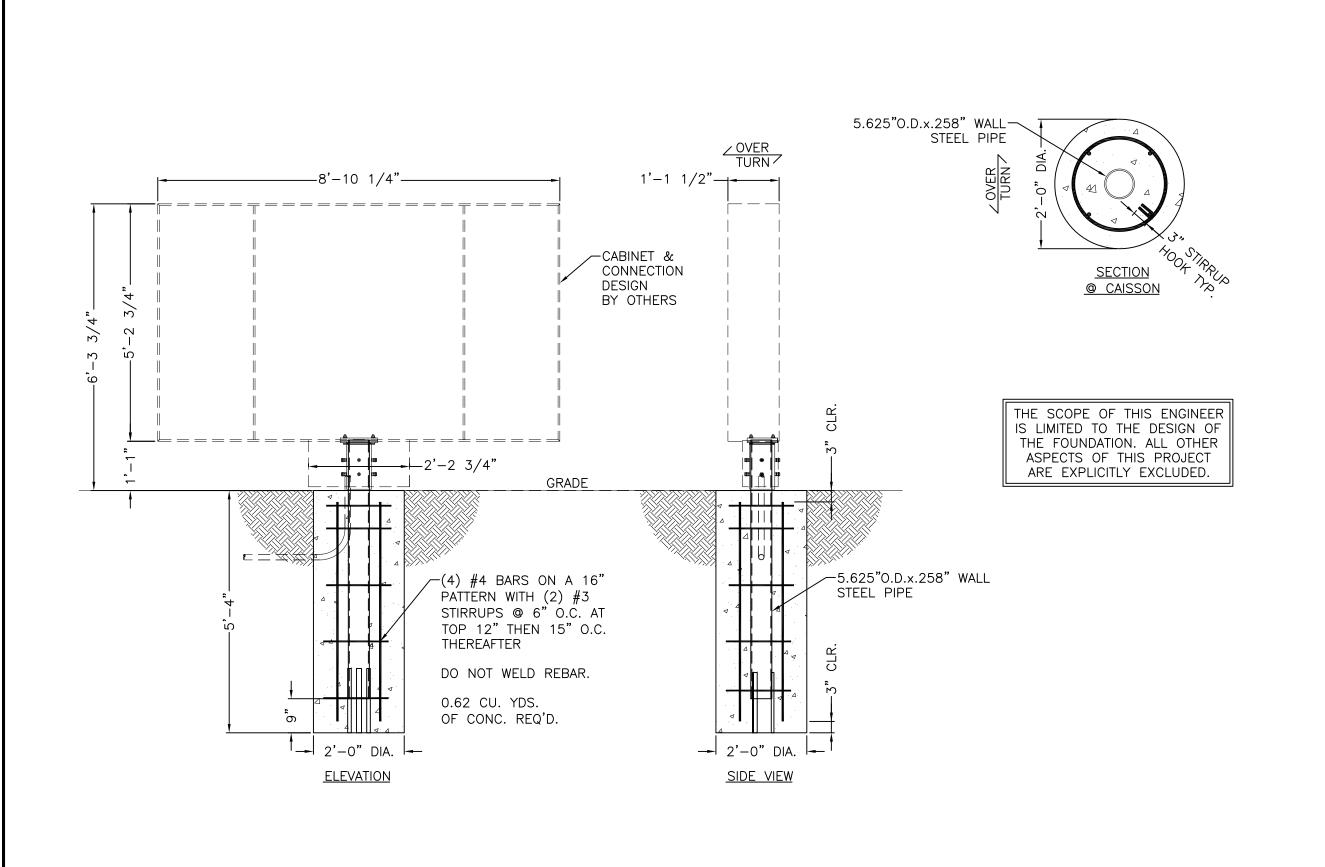
SEAL & SIGNATURE:





### LINK Engineering, L.L.C

Project N	ımber:	Drawing Numb	er:
22-0282R3		B186708	3R2
SHT. OF		DATE:	BY:
3	3	8/12/22	GHK



CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

76g	DATE	DESCRIPTION
A	-/-/-	
◬	-/-/-	
ß	-/-/-	

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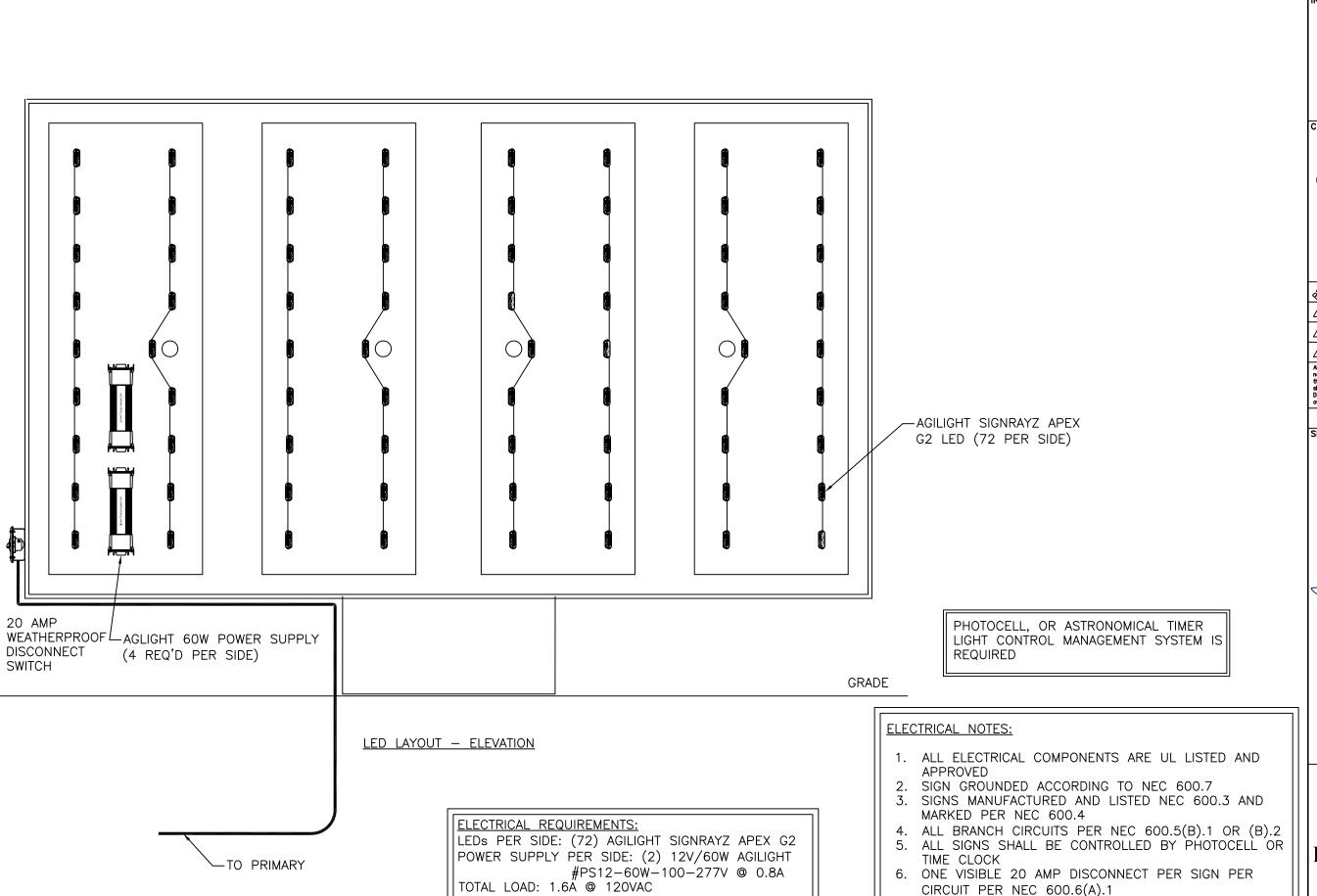
SEAL & SIGNATURE:





## LINK Engineering, L.L.C 135 South David Lane • Knoxville, Tennessee 37922 Phone: (865) 539-4001 • www.linkengr.com

Project N	lumber:	Drawing Number	er:
22-0282R3		B186709R2	
SHT. OF		DATE:	BY:
1	3	8/12/22	GHK



CIRCUITS: (1) 20 AMP REQUIRED

INSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

<b>W</b>	DATE	DESCRIPTION
A	-/-/-	
◬	-/-/-	
ß	-/-/-	

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IMAD N. KASHIF, P.E.
FLORIDA STATE LICENSE NO.: 41374

SEAL & SIGNATURE:





## LINK Engineering, L.L.C

135 South David Lane • Knoxville, Tennessee 379 Phone: (865) 539-4001 • www.linkengr.com

7. ALL CLASS 2 RATED LED MODULES AND LED POWER SUPPLIES WILL BE IN COMPLIANCE WITH NATIONALLY

RECOGNIZED TEST LABORATORY

	Project Nu	mber:	Drawing Number:		
	22-0	)282R3	B186709	)R2	
	SHT.	OF	DATE:	BY:	
_	2	3	8/12/22	GHK	

		PROJECT#	22-0282R		OWNER:	CULVER'S			
T		July 14, 2022				255 N.W. CO	MMONS LOOP		
		DRAWING #	B186708, B18	6709		LAKE CITY,	FL 32055		
		WIND LOAD	20.610	PSF					
		WIND SPEED	120	MPH	CLIENT:	SPRINGFIELD	SIGN & NEON	١	
		# COLUMNS	1	FLORIDA BUILD	OING CODE	2531 N. PAT	TERSON		
		DESIGNER	SR	7TH EDITION (2	020)	SPRINGFIELD	), MO		
					5000 pp 1000 pp 1 + 20 pp 200 pp				
L			_	SHAPE	CENTROID	_	TOTAL		
	ITEM	HEIGHT	WIDTH	FACTOR	HEIGHT	AREA	FORCE	MOMENT	
	======	======	=======			======		======	
	MENUBOARD	5.229	8.854	1.000	2.615	46.300	0.954	2.495	
	SKIRT	1.083	2.229	1.000	0.542	2.415	1.004	3.556	
	OAH	6.313							
H	DL MOMENT	0.069							
L		COLUMN CA	LCULATIONS	(CODES P=	:PIPE;O=OTHE				
		_	_			DESIGN		AVAILABLE	
		COLUMN	COLUMN	COLUMN	lхх	MODULUS	REQUIRED	FLEXURAL	
	ITEM	WIDTH	DEPTH	WALL	COLUMN	COLUMN	MOMENT	STRENGTH	UNITY
	======	=======	=======		======	=======	=======================================	======	=======
-	SKIRT		5.563	0.241	14.3	6.83	3.625	11.93	0.304
C	AISSON								
-	MOMENT					3.625	FT-KIP		
-	ORCE					1.004	KP		
-		7.3.2 & TABLE 1806.	2			1.001	1 (1)		
+		SS #4 SW, SP, SM, SC							
+		PRESSURE - PSF/FT				150.0	PSF/FT		
H	31					533.3			
-	DEPTH					5.333	FT.		
+	DIAMETER					2.000	FT.		
Ť						3.611	FT.		
						2.203	FT.		
C	ALCULATED DEPT	Н				4.245	FT.		
+-		S WITHOUT REINFORG	CEMENT			17.882	IN.		
Α	CTUAL DIAMETER					24.000	IN.		
-	ONCRETE					0.621	CU. YD.		
						0.021	OO. 1 D.		

1. Design is based on a 120 mph, 3 second gust wind design per Florida Building Code, 7th Edition (2020). Category II, Exposure C.

2. Caisson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short—term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.

3. A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact SPRINGFIELD SIGN & NEON.

4. Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re—evaluated by a competent Professional Engineer. Do not place foundation in fill.

competent Professional Engineer. Do not place foundation in fill.

5. Concrete shall be mixed to attain a minimum 28 day compressive strength

 Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.

 All support members shall be free from defects. Steel Pipe up to 24 inch O.D. shall meet ASTM A53 Grade B with a minimum yield strength of 35000 psi. Steel angle, channel and plate shall meet ASTM A36.

Steel welds shall be made with E70xx low hydrogen electrodes by persons qualified in accordance with AWS standards within the past two years.
 All structural bolts shall conform to ASTM A325, and be zinc coated unless

 All structural bolts shall conform to ASTM A325, and be zinc coated unles noted otherwise. When used with structural bolts, heavy hex nuts shall conform to ASTM A563, and washers shall conform to ASTM F436. Pretension all high strength bolts using the Turn—of—Nut method unless noted otherwise.

10. The scope of this engineer does not include onsite observations.

11. LINK Engineering will not be responsible for the safety on this job site before, during or after installation of this structure. It is the responsibility of the owners, contractors and installers to ensure that the installation and erection of this structure is performed using methods that are in full compliance with OSHA regulations.

12. Any deviation from this design or from any part of this drawing, including the General Notes, without prior written consent from LINK Engineering voids this drawing in its entirety.

13. The structure designed on this drawing is intended to be installed at the address shown and should not be used at any other location.

INSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



4825 EAST KEARNEY STREET SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

<b>W</b>	DATE	DESCRIPTION
A	-/-/-	
◬	-/-/-	
ß	-/-/-	

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IMAD N. KASHIF, P.E. FLORIDA STATE LICENSE NO.: 41374

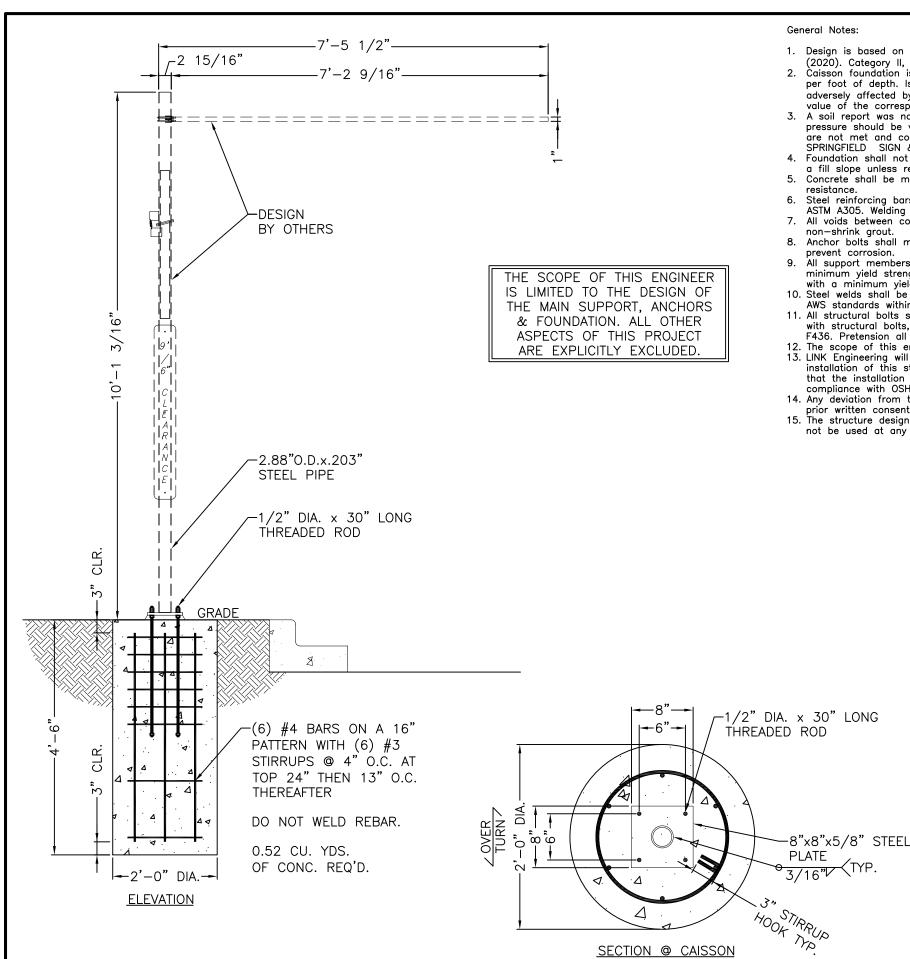
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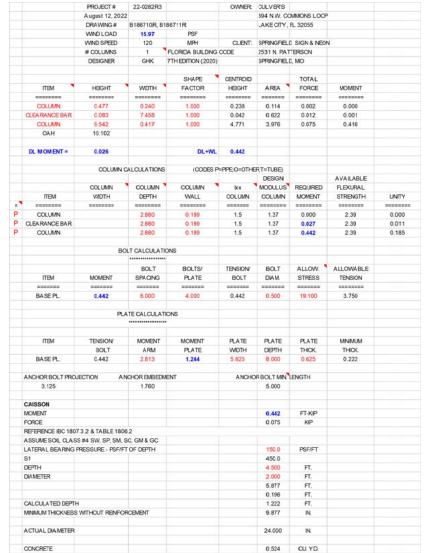


## LINK Engineering, L.L.C

Project N	umber:	Drawing Number:		
22-0282R3		B186709R2		
SHT. OF		DATE:	BY:	
3	3	8/12/22	GHK	



- 1. Design is based on a 120 mph, 3 second gust wind design per Florida Building Code, 7th Edition (2020). Category II. Exposure C.
- 2. Caisson foundation is based on a presumptive safe lateral soil bearing pressure minimum of 150 psf per foot of depth. Isolated lateral bearing footings subject to short-term lateral loads and not adversely affected by a 1/2" motion at grade are permitted to be designed using twice the tabulated value of the corresponding soil class.
- 3. A soil report was not provided. Foundation analysis assumes Soil Classification 4. Allowable bearing pressure should be verified prior to placement of concrete. In the event that the stated requirements are not met and conditions appear deleterious, cease and secure excavation and immediately contact SPRINGFIELD SIGN & NEON.
- 4. Foundation shall not be placed at the top of, or on the side of a slope exceeding 3:1, or adjacent to a fill slope unless re—evaluated by a competent Professional Engineer. Do not place foundation in fill.
- Concrete shall be mixed to attain a minimum 28 day compressive strength of 4500 psi per sulfate
- Steel reinforcing bars shall conform to ASTM A615, Grade 60 with deformations in accordance with ASTM A305. Welding of reinforcing bars is prohibited.
- All voids between column baseplate and foundation surface shall be completely filled with high-strength, non-shrink grout.
- Anchor bolts shall meet ASTM F1554 Grade 36. Exposed surfaces shall be galvanized or coated to prevent corrosion. All support members shall be free from defects. Steel tube shall meet ASTM A500 Grade B with a
- minimum yield strength of 46000 psi. Steel Pipe up to 24 inch 0.D. shall meet ASTM A53 Grade B with a minimum yield strength of 35000 psi. Steel angle, channel and plate shall meet ASTM A36.
- 10. Steel welds shall be made with E70xx low hydrogen electrodes by persons qualified in accordance with AWS standards within the past two years.
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NSTALLATION ADDRESS:

CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

CLIENT:



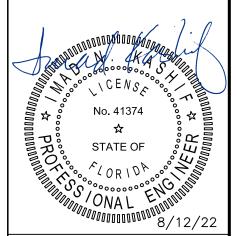
**4825 EAST KEARNEY STREET** SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

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IMAD N. KASHIF, P.E. FLORIDA STATE LICENSE NO.: 41374

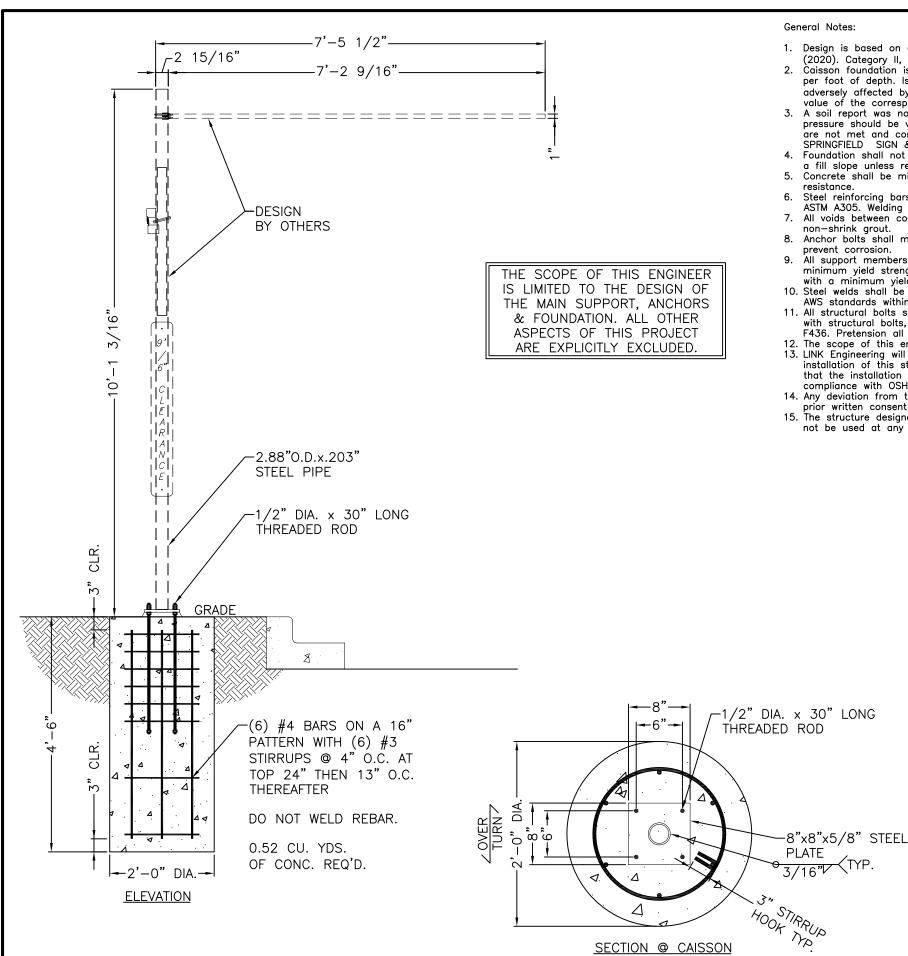
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## LINK Engineering, L.L.C

Project 1	lumber:	Drawing Number	er:	
22-	0282R3	B186710R		
SHT. OF		DATE:	BY:	
1	1	8/12/22	GHK	



- 1. Design is based on a 120 mph, 3 second gust wind design per Florida Building Code, 7th Edition (2020). Category II. Exposure C.
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CULVER'S 394 N.W. COMMONS LOOP LAKE CITY, FL 32055

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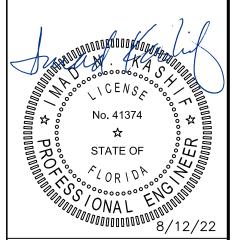
**4825 EAST KEARNEY STREET** SPRINGFIELD, MO 65803 417.862.2454 - FAX: 417.862.1887

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IMAD N. KASHIF, P.E. FLORIDA STATE LICENSE NO.: 41374

SEAL & SIGNATURE:





## LINK Engineering, L.L.C

Project Nun	nber:	Drawing Number:		
22-02	282R3	B186711R		
SHT.	OF	DATE:	BY:	
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