1405286

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
 Document ID 1V6E487-Z0115100241

Truss Fabricator	Anderson Truss Company
Job Identification	14-071--Affinity Construction /Carpenter Add. -- 3643 SW Piedmont Rd Lake City (3643 SW Pin
Truss Count	2
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software,Version 13.02.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: 12015EC1-GBLLETIN-GABRST10-

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	38786--A	27' Common	14135001	05/15/14
2	38787--ADG	27' Gable	14135002	05/15/14

ALPINE

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-071--Affinity Construction /Carpenter Add -- 3643 SW Pinemount Rd Lake City - A 27 Common)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #2

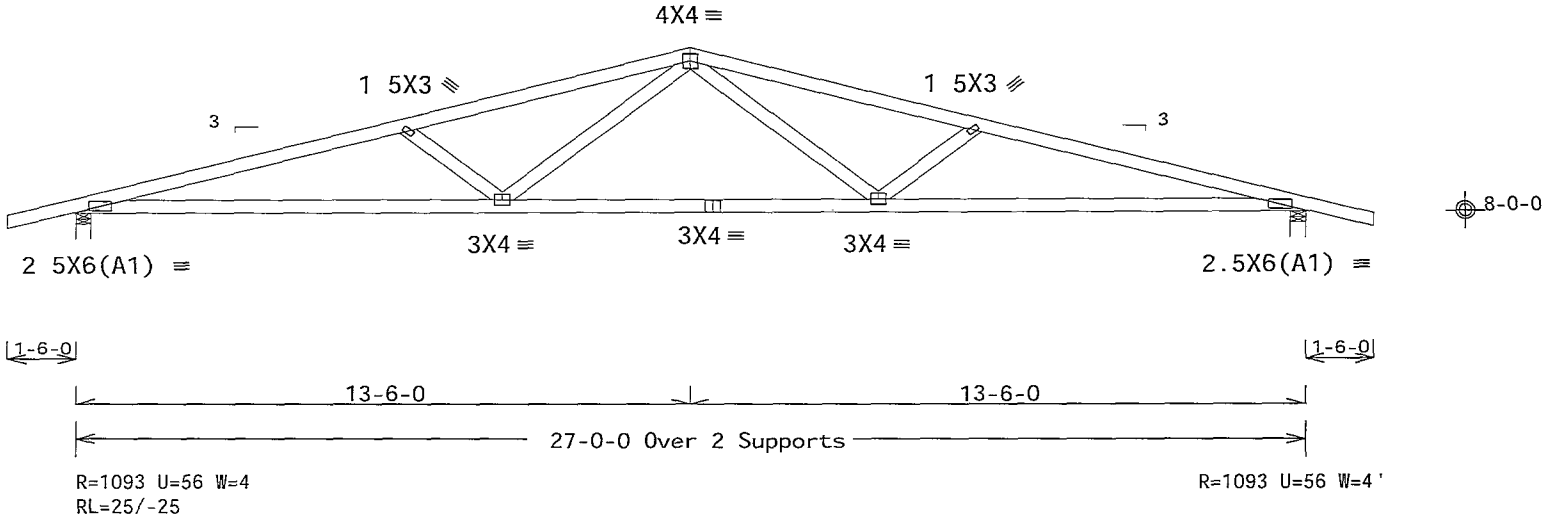
Lumber value set 13B uses design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind, 15 00 ft mean hgt ASCE 7-10, CLOSED bldg, Located anywhere in roof RISK CAT II, EXP B wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load



Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13 02 07 [illegible] QTY 8 FL/-/5/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
**IMPORTANT* FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses requ re extreme care n fabricating hand l ng sh pp ng nstall ng and brac ng Refer to and fol ow the latest edit on of BCSI (Bu ld ng Component Safety Informat on by TPI and WTCA) for safety pract ces pr or to perform ng these funct ons Installers shall prov de temporary brac ng per BCSI Unless noted otherwise top chord shall have properly attached structural sheath ng and bottom chord shall have a properly attached r g d ceil ng Locations shown for permanent lateral restra nt of webs shall have brac ng nstalled per BCSI sections B3 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviat on from th s des any fa lure to build the truss n conformance with ANSI/TPI 1 or for hand ng sh pp ng nstallat on brac ng of trusses Apply plates to each face of truss and pos t on as shown above and on the Jo nt Deta ls unless noted otherw se Refer to drawings 160A-Z for standard plate pos tions A seal on this draw ng or cover page l st ng th s draw ng ind cates acceptance of professional engineering responsibil ty solely for the design shown The su tability and use of this design for any structure s the responsibility of the Bu lding Des gner per ANSI/TPI 1 Sec 2 For more informat on see Th s job s general notes page ITW BCG www itwbcg com TPI www tp nst org WTCA www sbc ndustry com ICC www ccsafe org

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05/15/2014

TC LL	20 0 PSF	REF	R9114- 38786
TC DL	7 0 PSF	DATE	05/15/14
BC DL	10.0 PSF	DRW	HCUSR9114 14135001
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD.	37 0 PSF	SEQN-	76979
DUR FAC.	1.25		
SPACING	24 0"	JREF-	1V6E487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

(14-071--Affinity Construction /Carpenter Add -- 3643 SW Pinemount Rd Lake City - ADG 27 Gable)

Value Set 13B (Effective 6/1/2013)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #2
Stack Chord SC1 2x4 SP #1 Stack Chord SC2 2x4 SP #1

Lumber value set 13B uses design values approved 1/30/2013 by ALSC

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements

In lieu of structural panels use purlins to brace TC @ 24 OC

Bottom chord checked for 10 00 psf non-concurrent live load

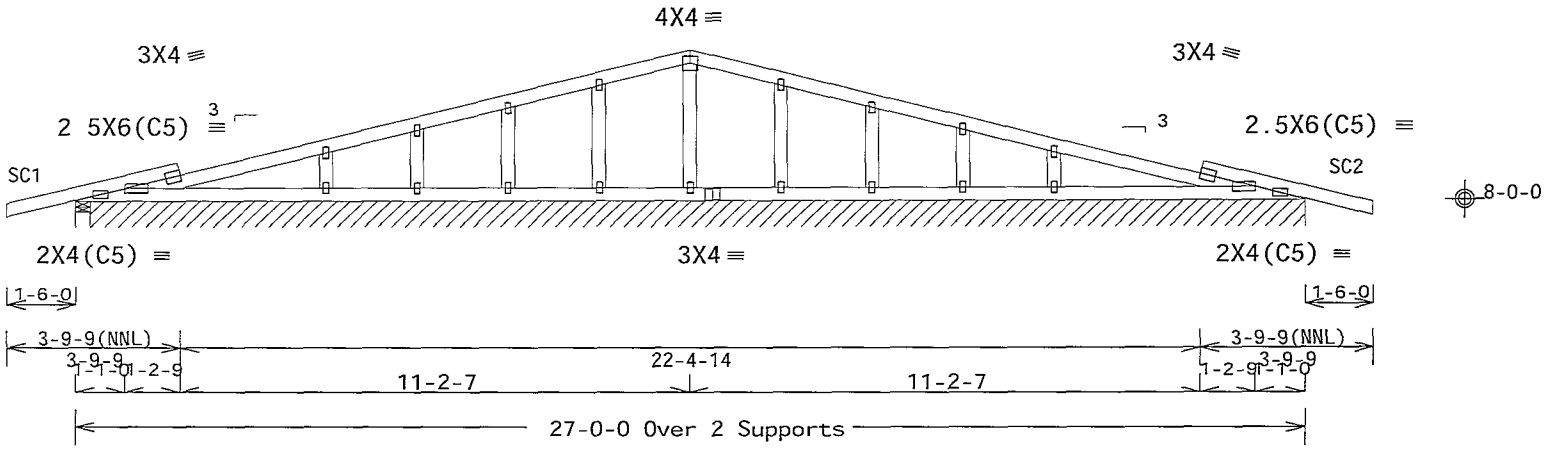
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24 0 OC designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 0 00 PSF Top chord must not be cut or notched

Stacked top chord must NOT be notched or cut in area (NNL) Dropped top chord braced at 24' o c intervals Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notchable area using 3x6



R=476 U=99 W=4
RL=50/-50

R=115 PLF U=15 PLF W=26-8-0

Note All Plates Are 1 5X3 Except As Shown

Design Crit FBC2010Res/TPI-2007(STD)

PLT TYP Wave FT/RT=10%(0%)/0(0) 13.02.07.0808.14 QTY 1 FL/-/5/-/-/R/- Scale =.25"/Ft.

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05/15/2014

TC LL	20 0 PSF	REF	R9114- 38787
TC DL	7.0 PSF	DATE	05/15/14
BC DL	10.0 PSF	DRW	HCUSR9114 14135002
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT LD	37 0 PSF	SEQN-	76985
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V6E487_Z01

Gable Stud Reinforcement Detail

ASCE 7-10 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed 15 Mean Height Partially Enclosed Exposure C Kzt = 1.00
Or 100 mph Wind Speed 15 Mean Height Enclosed, Exposure D Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacng	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" oc	SPF HF	# / #2	4 10"	8 2"	8 6"	9 8"	10 1"	11 6"	12' 0"	14 0"	14' 0"	14' 0"	14 0"
		#3	4 7"	7 9"	8 3"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14 0"	14 0"
		Stud	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14 0"	14 0"
		Standard	4 7"	8 1"	8 4"	9 7"	9 11"	11 5"	11 10"	14 0"	14 0"	14 0"	14' 0"
	SP DFL	#1	4 11"	8 3"	8 7"	9 9"	10 1"	11 7"	12 1"	14 0"	14 0"	14 0"	14' 0"
		#2	4 10"	8 2"	8 6"	9 8"	10 1"	11 6"	12 0"	14' 0"	14 0"	14 0"	14 0"
		#3	4 7"	6 11"	7 4"	9 3"	9 10"	11 5"	11 10"	14' 0"	14' 0"	14 0"	14' 0"
		Stud	4 7"	6 11"	7 4"	9 3"	9 10"	11 5"	11 10"	14 0"	14 0"	14 0"	14 0"
		Standard	4 7"	6 0"	6 4"	8 0"	8 6"	10 10"	11 7"	12 6"	13 5"	14 0"	14 0"
16" oc	SPF HF	#1 / #2	5 6"	9 5"	9 9"	11 1"	11 6"	13' 2"	13 9"	14 0"	14 0"	14 0"	14 0"
		#3	5 3"	9 3"	9 9"	10 11"	11 4"	13' 0"	13 7"	14 0"	14 0"	14 0"	14 0"
		Stud	5 3"	9 3"	9 7"	10 11"	11 4"	13 0"	13 7"	14 0"	14 0"	14 0"	14 0"
		Standard	5 3"	9 3"	9 7"	10 11"	11 4"	13' 0"	13 7"	14 0"	14 0"	14 0"	14' 0"
	SP DFL	#1	5 8"	9' 5"	9' 10"	11 2"	11 7"	13' 3"	13' 10"	14 0"	14 0"	14 0"	14 0"
		#2	5 6"	9 5"	9 9"	11 1"	11 6"	13 2"	13 9"	14 0"	14 0"	14 0"	14' 0"
		#3	5 3"	8 6"	9' 0"	10 11"	11 4"	13 0"	13 7"	14 0"	14 0"	14 0"	14 0"
		Stud	5 3"	8 6"	9 0"	10 11"	11 4"	13 0"	13 7"	14 0"	14 0"	14 0"	14 0"
		Standard	5 3"	7 4"	7 10"	9 9"	10' 5"	13 0"	13 7"	14 0"	14 0"	14' 0"	14 0"
12" oc	SPF HF	#1 / #2	6 "	10 4"	10 8"	12 2"	12 8"	13' 2"	14 0"	14' 0"	14 0"	14' 0"	14' 0"
		#3	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		Stud	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		Standard	5 9"	10 2"	10 7"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
	SP DFL	#	6 2"	10 5"	10 9"	12' 3"	12 9"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		#2	6 1"	10 4"	10 8"	12 2"	12 8"	14 0"	14 0"	14 0"	14 0"	14 0"	14' 0"
		#3	5 9"	9 9"	10 5"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		Stud	5 9"	9 9"	10 5"	12 0"	12 6"	14 0"	14 0"	14 0"	14 0"	14 0"	14 0"
		Standard	5 9"	8 6"	9 0"	11 3"	12 1"	14 0"	14 0"	14 0"	14 0"	14' 0"	14' 0"

Bracing Group Species and Grades

Group A

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January 2012 ruling

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c in 18" end zones and 6" o.c. between zones.

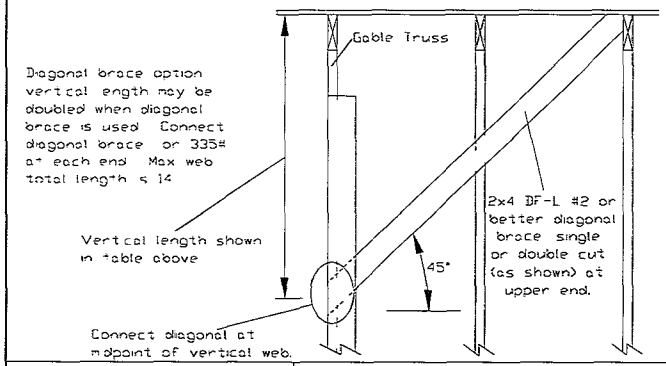
"L" bracing must be a minimum of 80% of web member length.

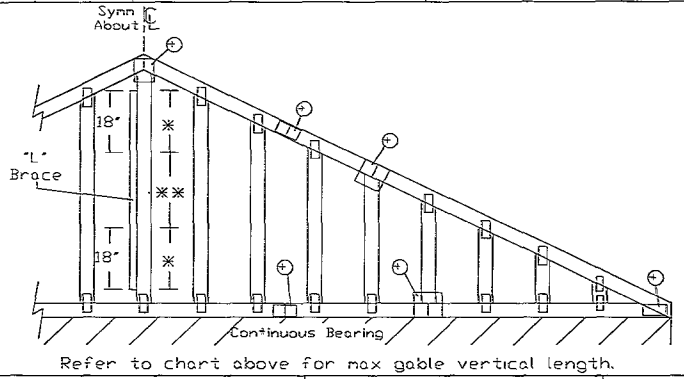
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0" but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail





Refer to chart above for max gable vertical length.

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Earth City MO 63045

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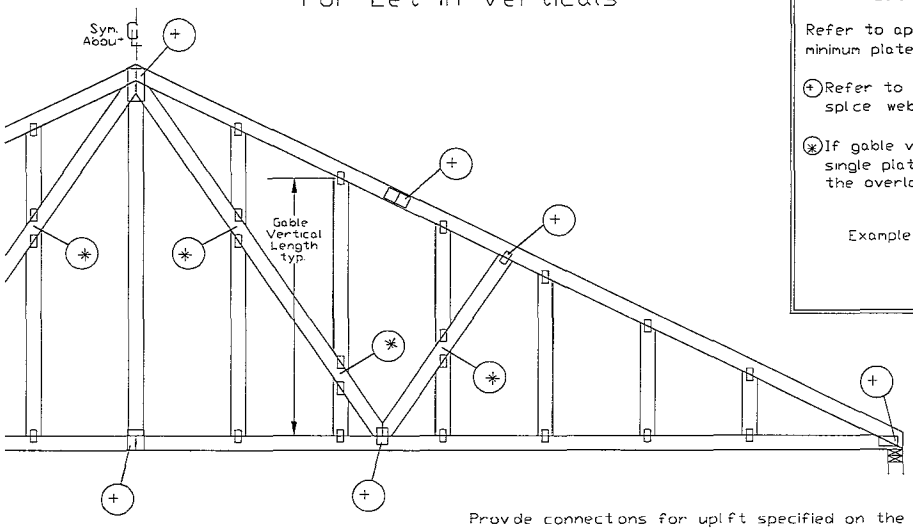
05/15/2014

MAX TOT LD 60 PSF

MAX SPACING 24 0"

REF	ASCE7-10-GAB12015
DATE	2/14/12
DRWG	A12015ENC100212

Gable Detail
For Let-in Verticals



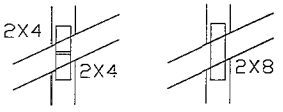
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs

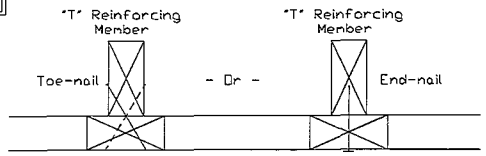
⊕ Refer to Engineered truss design for peak splice web and heel plates

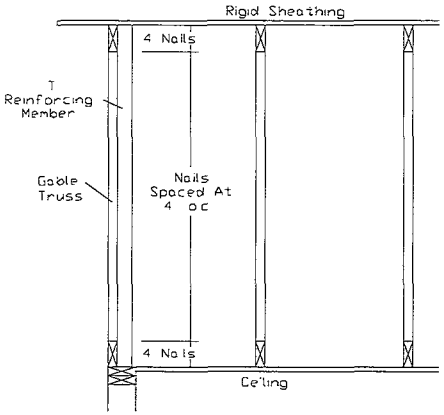
⊛ If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web

Example



"T" Reinforcement Attachment Detail





Provide connections for uplift specified on the engineered truss design

Attach each "T" reinforcing member with

End Driven Nails
- 10d Common (0.148"x 3." min) Nails at 4" o.c plus
- (4) nails in the top and bottom chords

Toenailed Nails
- 10d Common (0.148"x3" min) Toenails at 4" oc plus
- (4) toenails in the top and bottom chords

This detail to be used with the appropriate ITW gable detail for ASCE wind load

ASCE 7-98 Gable Detail Drawings
A13015980109 A12015980109 A11015980109 A10015980109
A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109 A12015020109 A11015020109, A10015020109 A14015020109
A13030020109 A12030020109 A11030020109 A10030020109 A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109 A12015050109 A11015050109 A10015050109 A14015050109
A13030050109 A12030050109 A11030050109 A10030050109 A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212 A12015ENC100212, A14015ENC100212 A16015ENC100212
A18015ENC100212 A20015ENC100212 A20015END100212 A20015PED100212
A11530ENC100212 A12030ENC100212 A14030ENC100212 A16030ENC100212
A18030ENC100212 A20030ENC100212 A20030END100212 A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length

To convert from "L" to "T" reinforcing members multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size specie and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

"T" Reinf Mbr Size	"T" Increase
2x4	30 %
2x6	20 %

Example
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft Kzt = 1.00
- Gable Vertical = 24"oc SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8' 7" = 11' 2"

Building Components Group Inc.

Earth City MO 63045

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ITWBCG: www.itwbcg.com, TPI: www.tpinst.org, WTCA: www.sbcindustry.org ICC: www.iccsafe.org

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14

MAX TOT LD 60 PSF
DUR FAC ANY
MAX SPACING 24 0"

REF LET-IN VERT
DATE 2/16/12
DRWG GBLLETIN0212

05/15/2014

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph 30ft Mean Hgt, ASCE 7-10, Part Enclosed Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates and other information not shown on this detail

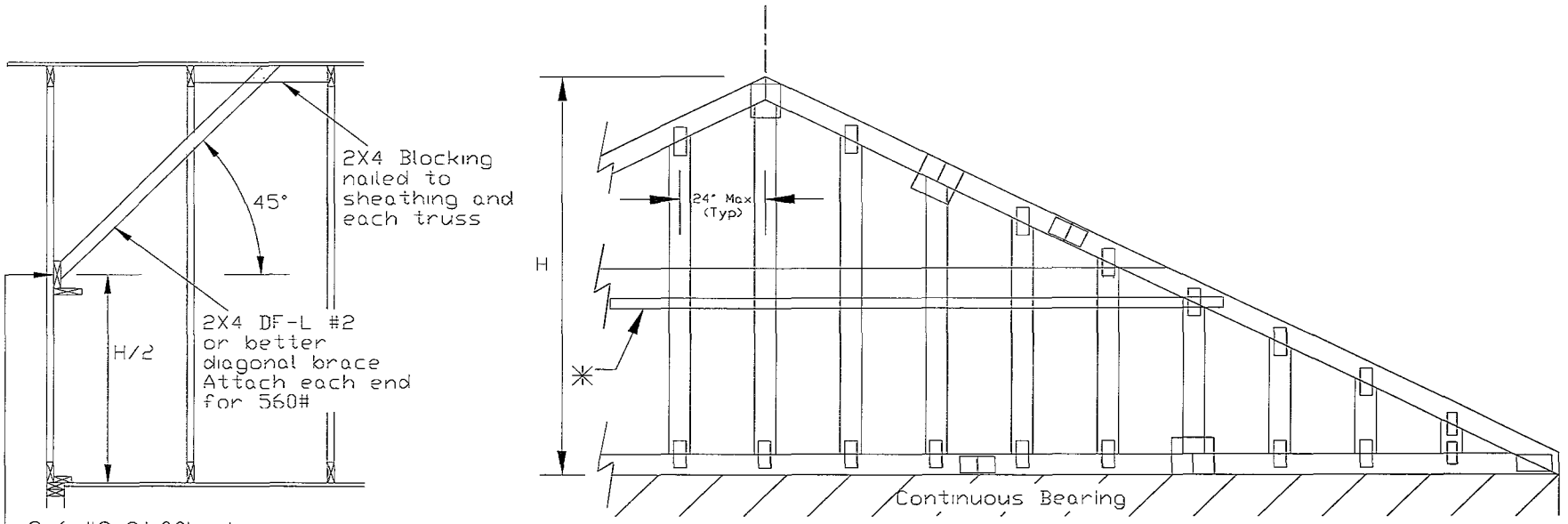
Nails 10d box or gun (0.128"x3",min) nails

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC100212)

H Greater than 7'6" to 12'0" max
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC100212)

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min) nails @ 6" oc



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ITWBCG: www.itwbcg.com, TPI: www.pinst.org, WTCA: www.sbcindustry.org ICC www.iccsafe.org

Building Components Group Inc.
Earth City MO 63045

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14

MAX TOT LD 60 PSF

MAX SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

05/15/2014