

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844

Florida Engineering Certificate of Authorization Number: 0 278

Florida Certificate of Product Approval # FL1999

Page 1 of 1 Document ID: 1U068228Z0317154802

Truss Fabricator: Anderson Truss Company

Job Identification: 10-018--Fill in later MIKE BALL -- , **

Truss Count: 39

Model Code: Florida Building Code 2007 and 2009 Supplement

Truss Criteria: FBC2007Res/TPI-2002(STD)

Engineering Software: Alpine Software, Version 9.02.

Structural Engineer of Record: The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC

Address:

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-CNNAILSP-



Seal Date: 03/17/2010

-Truss Design Engineer-

James F. Collins Jr.

Florida License Number: 52212

1950 Marley Drive

Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	92704--H7A		10076003	03/17/10
2	92705--H9A		10076009	03/17/10
3	92706--H11A		10076010	03/17/10
4	92707--AVS		10076016	03/17/10
5	92708--AV1		10076004	03/17/10
6	92709--AV2		10076005	03/17/10
7	92710--AV3		10076006	03/17/10
8	92711--AV4		10076007	03/17/10
9	92712--AV5		10076008	03/17/10
10	92713--AV6		10076009	03/17/10
11	92714--H13AV		10076010	03/17/10
12	92715--H15AV		10076011	03/17/10
13	92716--H17AV		10076012	03/17/10
14	92717--H1768AV		10076013	03/17/10
15	92718--AV7		10076014	03/17/10
16	92719--HS7B		10076015	03/17/10
17	92720--HS9B		10076016	03/17/10
18	92721--H11B		10076017	03/17/10
19	92722--H13B		10076018	03/17/10
20	92723--H15B		10076021	03/17/10
21	92724--H17B		10076019	03/17/10
22	92725--B1		10076008	03/17/10
23	92726--B2		10076015	03/17/10
24	92727--BT1		10076005	03/17/10
25	92728--BT2		10076007	03/17/10
26	92729--H7C		10076002	03/17/10
27	92730--CG		10076020	03/17/10
28	92731--H7D		10076001	03/17/10
29	92732--H9D		10076006	03/17/10
30	92733--H11D		10076014	03/17/10
31	92734--D		10076017	03/17/10
32	92735--EJ7		10076021	03/17/10
33	92736--CJ5		10076018	03/17/10
34	92737--HJ7		10076004	03/17/10
35	92738--CJ3		10076019	03/17/10
36	92739--CJ1		10076020	03/17/10

#	Ref	Description	Drawing#	Date
37	92740--HJ3		10076012	03/17/10
38	92741--EJ3		10076013	03/17/10
39	92742--H3M		10076011	03/17/10



(10-018--Fill in later MIKE BALL -- , ** - H7A)

(1) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MWFRS pressures.

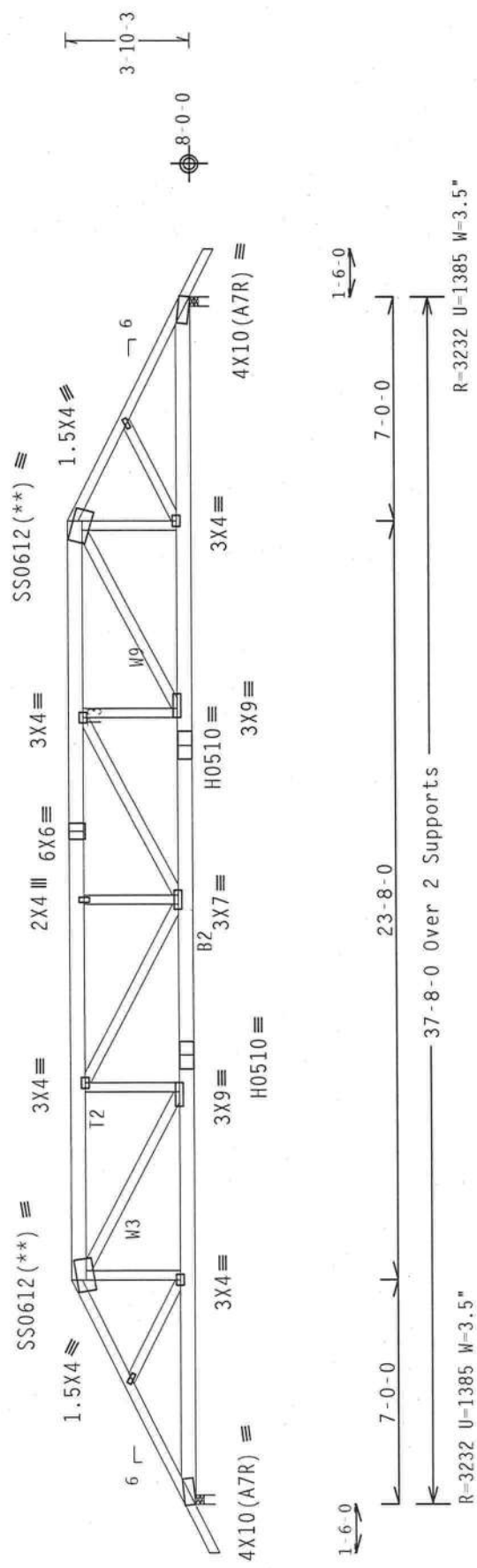
In lieu of structural panels use purlins to brace all flat IC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Top chord 2x4 SP #2 Dense : I2, I3 2x6 SP #1 Dense:
Bot chord 2x6 SP #1 Dense : B2 2x6 SP SS:
Webs 2x4 SP #3 : W3, W9 2x4 SP #2 Dense:
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
Iw=1.00 Gcpi (+/-) -0.18

Roof overhang supports 2.00 psf soffit load.

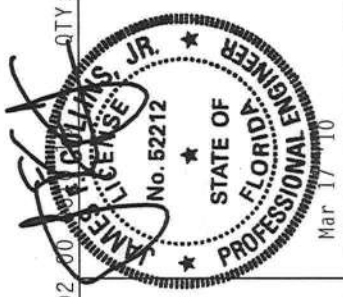
#1 hip supports 7-0-0 jacks with no webs.



PLT TYP. 20 Gauge HS, 18 Gauge HS, Design Crit: FBC2007Res/TPI-2002 (STD)
Wave FT/RT=10%(0%) 10(0)

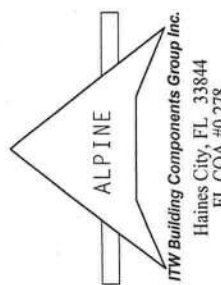
Scale = .1875" / Ft.

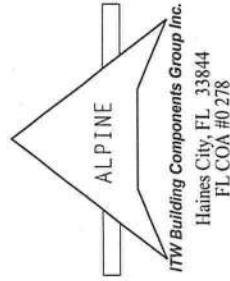
TC LL	20.0 PSF	REF	R8228- 92704
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076003
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	40.0 PSF	SEQN-	82845
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1U068228Z03



*****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH IPT1, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE DESIGN CONFORMS WITH ALL APPLICABLE PROVISIONS OF MOST NATIONAL DESIGN SPEC., STEEL, AND TRUSS CONECTOR PLATES ARE MADE OF 2018/16GA (0.0553") ASTM A583 GRADE 50C, 1/4" STEEL. PER DRWG. 160A-2, 160A-3, 160A-4, 160A-5, 160A-6, 160A-7, 160A-8, 160A-9, 160A-10, 160A-11, 160A-12, 160A-13, 160A-14, 160A-15, 160A-16, 160A-17, 160A-18, 160A-19, 160A-20, 160A-21, 160A-22, 160A-23, 160A-24, 160A-25, 160A-26, 160A-27, 160A-28, 160A-29, 160A-30, 160A-31, 160A-32, 160A-33, 160A-34, 160A-35, 160A-36, 160A-37, 160A-38, 160A-39, 160A-40, 160A-41, 160A-42, 160A-43, 160A-44, 160A-45, 160A-46, 160A-47, 160A-48, 160A-49, 160A-50, 160A-51, 160A-52, 160A-53, 160A-54, 160A-55, 160A-56, 160A-57, 160A-58, 160A-59, 160A-60, 160A-61, 160A-62, 160A-63, 160A-64, 160A-65, 160A-66, 160A-67, 160A-68, 160A-69, 160A-70, 160A-71, 160A-72, 160A-73, 160A-74, 160A-75, 160A-76, 160A-77, 160A-78, 160A-79, 160A-80, 160A-81, 160A-82, 160A-83, 160A-84, 160A-85, 160A-86, 160A-87, 160A-88, 160A-89, 160A-90, 160A-91, 160A-92, 160A-93, 160A-94, 160A-95, 160A-96, 160A-97, 160A-98, 160A-99, 160A-100, 160A-101, 160A-102, 160A-103, 160A-104, 160A-105, 160A-106, 160A-107, 160A-108, 160A-109, 160A-110, 160A-111, 160A-112, 160A-113, 160A-114, 160A-115, 160A-116, 160A-117, 160A-118, 160A-119, 160A-120, 160A-121, 160A-122, 160A-123, 160A-124, 160A-125, 160A-126, 160A-127, 160A-128, 160A-129, 160A-130, 160A-131, 160A-132, 160A-133, 160A-134, 160A-135, 160A-136, 160A-137, 160A-138, 160A-139, 160A-140, 160A-141, 160A-142, 160A-143, 160A-144, 160A-145, 160A-146, 160A-147, 160A-148, 160A-149, 160A-150, 160A-151, 160A-152, 160A-153, 160A-154, 160A-155, 160A-156, 160A-157, 160A-158, 160A-159, 160A-160, 160A-161, 160A-162, 160A-163, 160A-164, 160A-165, 160A-166, 160A-167, 160A-168, 160A-169, 160A-170, 160A-171, 160A-172, 160A-173, 160A-174, 160A-175, 160A-176, 160A-177, 160A-178, 160A-179, 160A-180, 160A-181, 160A-182, 160A-183, 160A-184, 160A-185, 160A-186, 160A-187, 160A-188, 160A-189, 160A-190, 160A-191, 160A-192, 160A-193, 160A-194, 160A-195, 160A-196, 160A-197, 160A-198, 160A-199, 160A-200, 160A-201, 160A-202, 160A-203, 160A-204, 160A-205, 160A-206, 160A-207, 160A-208, 160A-209, 160A-210, 160A-211, 160A-212, 160A-213, 160A-214, 160A-215, 160A-216, 160A-217, 160A-218, 160A-219, 160A-220, 160A-221, 160A-222, 160A-223, 160A-224, 160A-225, 160A-226, 160A-227, 160A-228, 160A-229, 160A-230, 160A-231, 160A-232, 160A-233, 160A-234, 160A-235, 160A-236, 160A-237, 160A-238, 160A-239, 160A-240, 160A-241, 160A-242, 160A-243, 160A-244, 160A-245, 160A-246, 160A-247, 160A-248, 160A-249, 160A-250, 160A-251, 160A-252, 160A-253, 160A-254, 160A-255, 160A-256, 160A-257, 160A-258, 160A-259, 160A-260, 160A-261, 160A-262, 160A-263, 160A-264, 160A-265, 160A-266, 160A-267, 160A-268, 160A-269, 160A-270, 160A-271, 160A-272, 160A-273, 160A-274, 160A-275, 160A-276, 160A-277, 160A-278, 160A-279, 160A-280, 160A-281, 160A-282, 160A-283, 160A-284, 160A-285, 160A-286, 160A-287, 160A-288, 160A-289, 160A-290, 160A-291, 160A-292, 160A-293, 160A-294, 160A-295, 160A-296, 160A-297, 160A-298, 160A-299, 160A-300, 160A-301, 160A-302, 160A-303, 160A-304, 160A-305, 160A-306, 160A-307, 160A-308, 160A-309, 160A-310, 160A-311, 160A-312, 160A-313, 160A-314, 160A-315, 160A-316, 160A-317, 160A-318, 160A-319, 160A-320, 160A-321, 160A-322, 160A-323, 160A-324, 160A-325, 160A-326, 160A-327, 160A-328, 160A-329, 160A-330, 160A-331, 160A-332, 160A-333, 160A-334, 160A-335, 160A-336, 160A-337, 160A-338, 160A-339, 160A-340, 160A-341, 160A-342, 160A-343, 160A-344, 160A-345, 160A-346, 160A-347, 160A-348, 160A-349, 160A-350, 160A-351, 160A-352, 160A-353, 160A-354, 160A-355, 160A-356, 160A-357, 160A-358, 160A-359, 160A-360, 160A-361, 160A-362, 160A-363, 160A-364, 160A-365, 160A-366, 160A-367, 160A-368, 160A-369, 160A-370, 160A-371, 160A-372, 160A-373, 160A-374, 160A-375, 160A-376, 160A-377, 160A-378, 160A-379, 160A-380, 160A-381, 160A-382, 160A-383, 160A-384, 160A-385, 160A-386, 160A-387, 160A-388, 160A-389, 160A-390, 160A-391, 160A-392, 160A-393, 160A-394, 160A-395, 160A-396, 160A-397, 160A-398, 160A-399, 160A-400, 160A-401, 160A-402, 160A-403, 160A-404, 160A-405, 160A-4





(10-018--Fill in later MIKE BALL --, ** - AVS)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Calculated horizontal deflection is 0.23" due to live load and 0.24" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

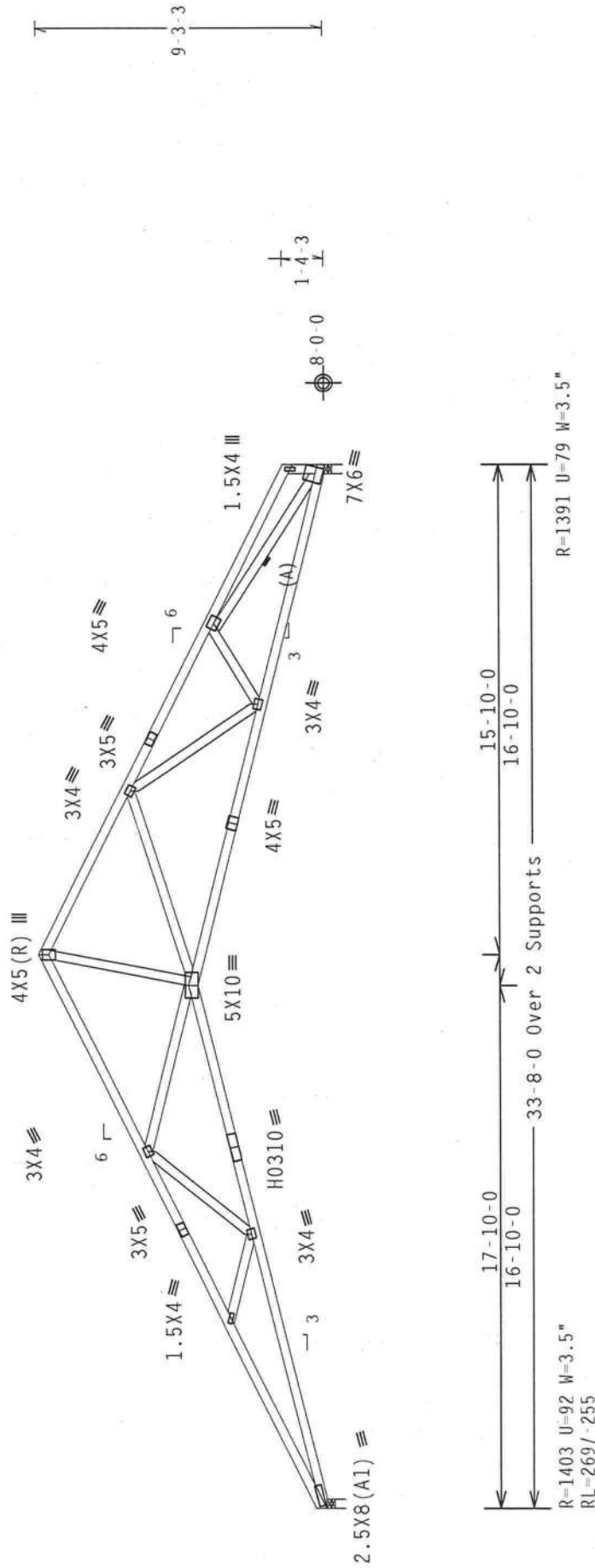
MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load.



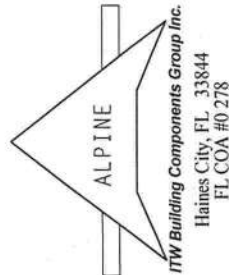
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. 20 Gauge HS.Wave

QTY:1 FL/-/4/-/R/- Scale =.1875"/Ft.

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TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8228- 92707
DATE	03/17/10
DRW	HCUSR228 10076016
HC-ENG	DLJ/DLJ *
SEQN	82958
JREF	1U068228Z03

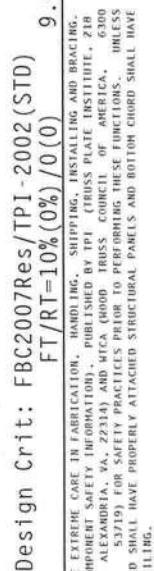
(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT-II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

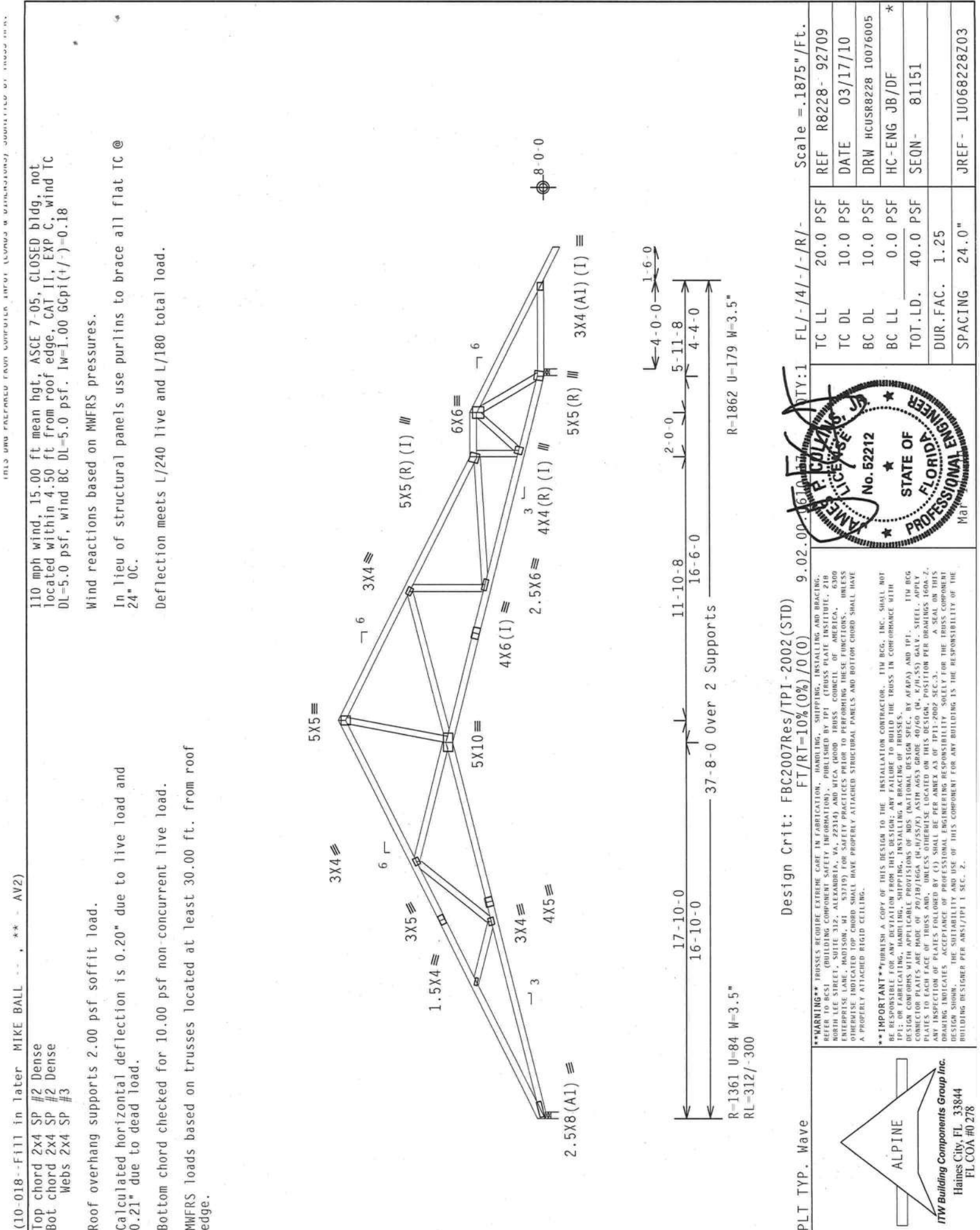
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

TC LL	20.0	PSF	REF	R8228-	92708
TC DL	10.0	PSF	DATE	03/17/10	
BC DL	10.0	PSF	DRW	HCUSR8228	1007600
BC LL	0.0	PSF	HC-ENG	JB/DF	
TOT.LD.	40.0	PSF	SEON-	81155	
DUR.FAC.	1.25				
SPACING	24.0"		JREF-	1U068228Z03	

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Mar 17 11

Mar 17/10



(10-018--Fill in later MIKE BALL ** - AV2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.20" due to live load and 0.21" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpl(+/-)=0.18

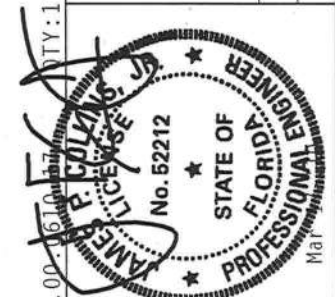
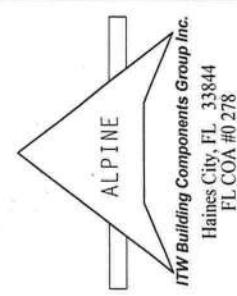
Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	Scale = .1875"/Ft.
REF R8228- 92709	TC LL 20.0 PSF
DATE 03/17/10	TC DL 10.0 PSF
DRW HCUSR8228 10076005	BC DL 10.0 PSF
HC-ENG JB/DF	BC LL 0.0 PSF
SEQN- 81151	TOT.LD. 40.0 PSF
DUR.FAC. 1.25	
SPACING 24.0"	
JREF- 1U068228Z03	



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(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

	Top chord	2x4	SP	#2	Dense
Top chord	2x4	SP	#2	Dense	
Bot chord	2x4	SP	#2	Dense	
Webbs	2x4	SP	#3		

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.20" due to live load and 0.21" due to dead load.

(A) Continuous lateral bracing equally spaced on member.

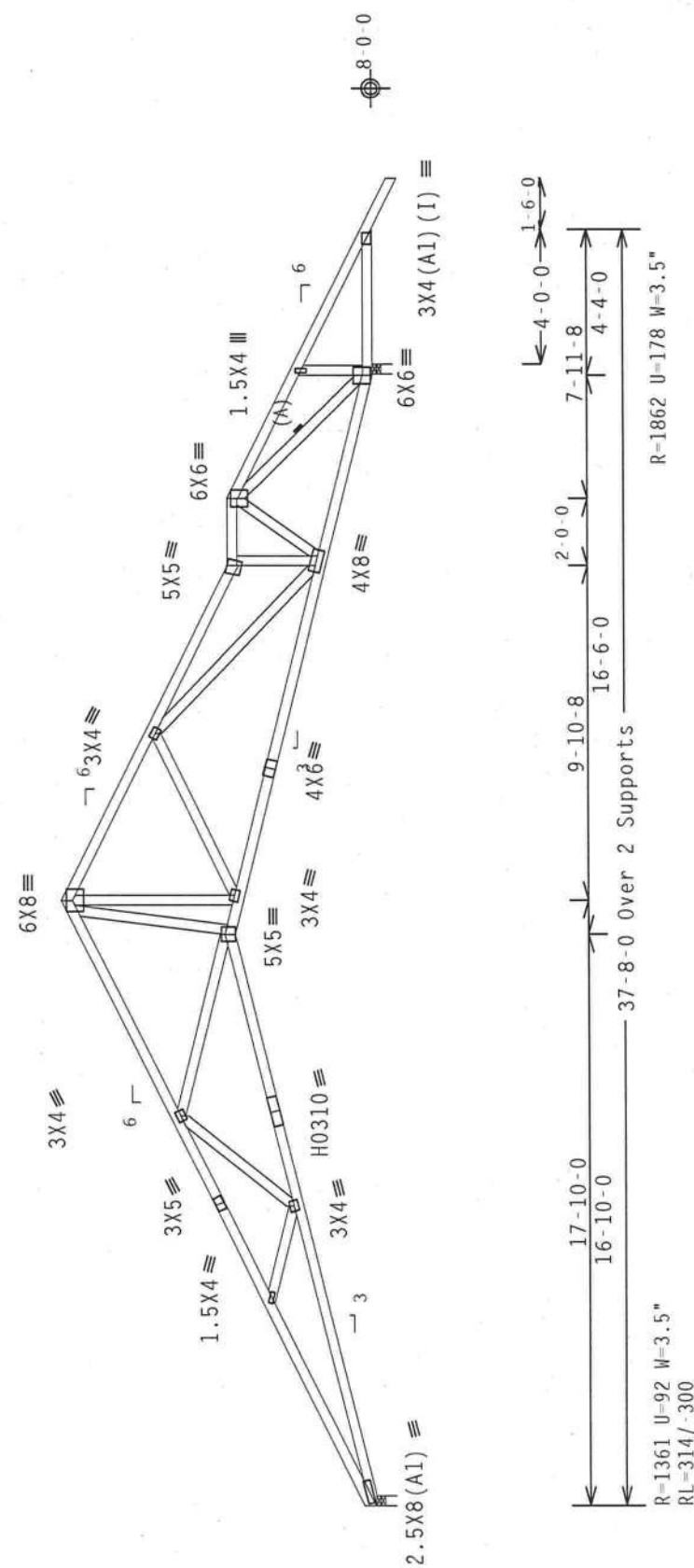
Bottom chord checked for 10.00 psf non-concurrent live load.

MMWFRS loads based on trusses located at least 15.00 ft. from roof edge.

wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



PLT TYP. 20 Gauge HS.Wave
Design Crit: FBC2007Res/TPI - 2002 (STD)
FT/RT=10%(0%)/0(0)

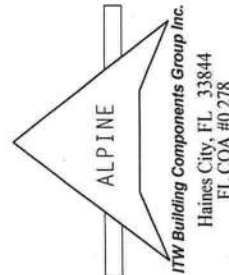
Scale = .1875"/Ft.

TC LL	20.0 PSF	REF R8228- 92710
TC DL	10.0 PSF	DATE 03/17/10
BC DL	10.0 PSF	DRW HCU8R8228 10076006
BC LL	0.0 PSF	HC-ENG JB/DF *
TOT.LD.	40.0 PSF	SEQN- 81147
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1U068228Z03



WARNING - BUILSSS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TPOUS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WPCA (WOOD PRODUCTS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN COULD BE ATTRIBUTED TO THE CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN COULD BE ATTRIBUTED TO THE CONTRACTOR.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.20" due to live load and 0.21" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

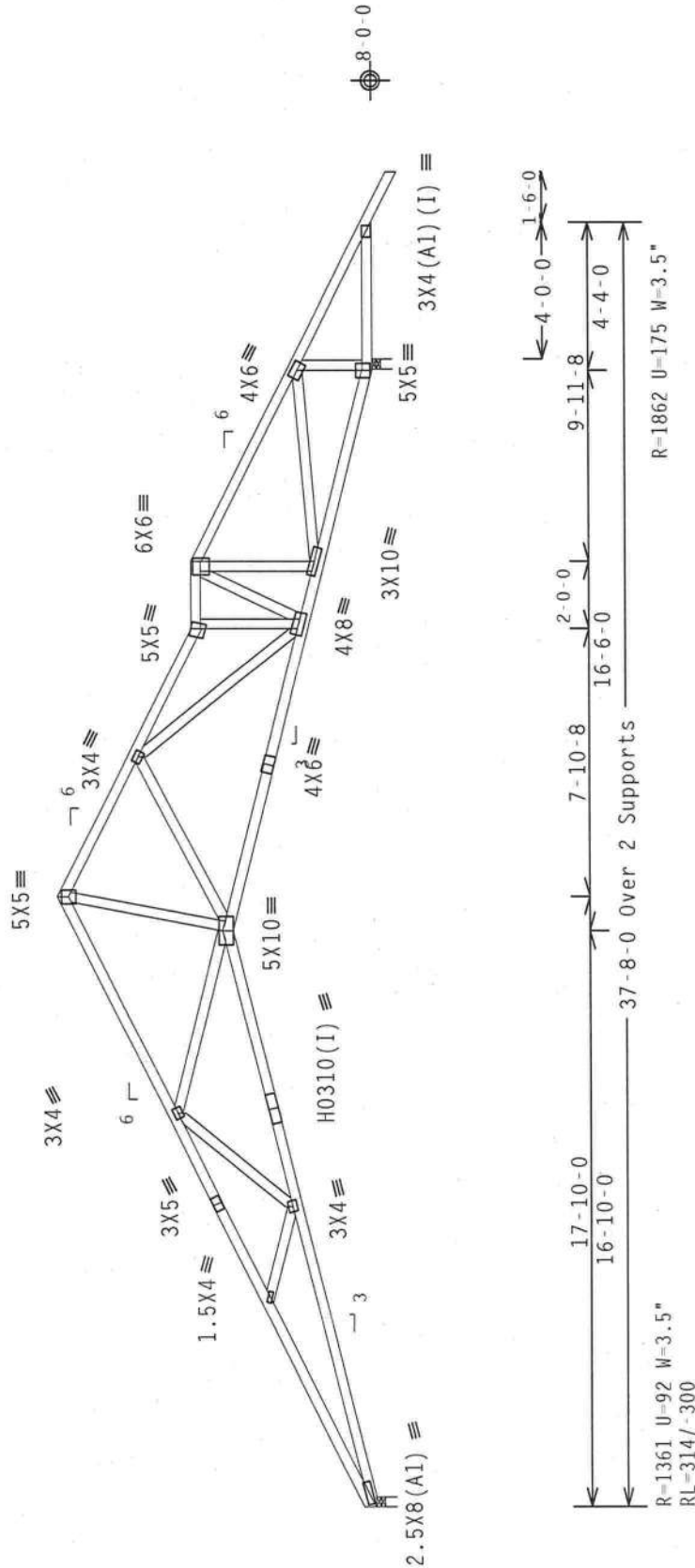
(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00 GCP(+-)=0.18$

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



PLT TYP. 20 Gauge HS.Wave
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8228- 92711
DATE	03/17/10
DRW	HCUSR8228 10076007
HC-ENG	JB/DF
SEQN	81143
JREF	1U068228Z03

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

JAMES F. BOLLANS JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719). FOR SAFETY PRACTICES PRIOR TO FABRICATING TRUSSES, UNLESS OTHERWISE INDICATED, THE TRUSS CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** WHEN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AIA/PFA) AND TPI. 1TH BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (4.0/55/PS) ASH A653 GRADE 40/60 (4.0/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
		webs	2x4	SP	#3	

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.20" due to live load and 0.21" due to dead load.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMWFRS loads based on trusses located at least 15.00 ft. from roof edge.

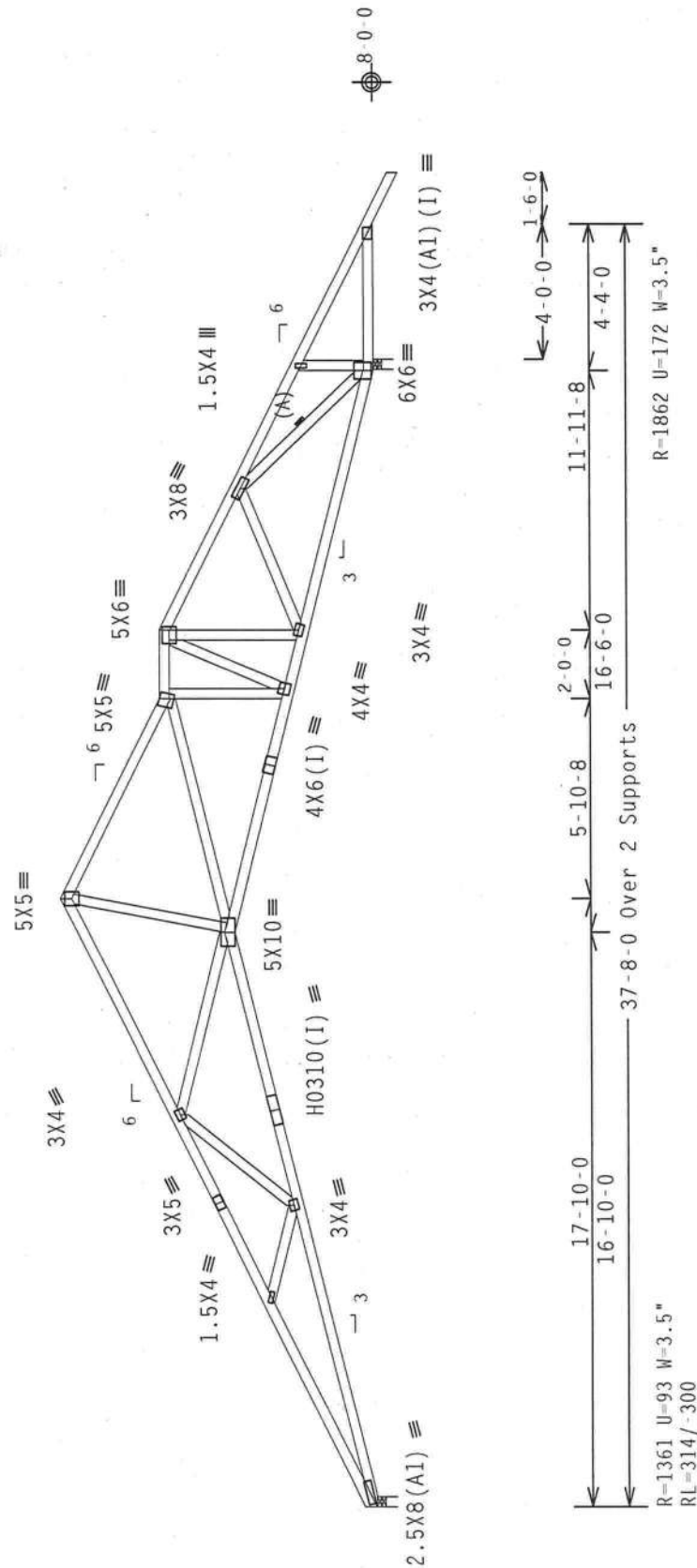
(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

1110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-) = 0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI -2002 (STD)
FT/RT=10% (0%) / 0 (0)

PLT TYP. 20 Gauge HS, Wave

Scale = .1875"/Ft.

*****WARNING**** TRUSSES REQUIRE EXTRE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE COMPONENT SAFETY INFORMATION. PUBLISHED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 5300 NORTH MICHIGAN, CHICAGO, ILL. 60640. TRUSS COUNCIL OF AMERICA, 6300 NORTH MICHIGAN, CHICAGO, ILL. 60640. 537279 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE THRUSS IN CONFORMANCE WITH THIS DESIGN OR FABRICATING THE THRUSS SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. IS NOT PROVIDING ANY TECHNICAL SUPPORT, INCLUDING THE PROVISIONS OF THIS NATIONAL DESIGN SPEC. BY AERPA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2025 AND 2018/16GA (94.5/155) ASTM A563 GRADE 40/60 (H/J,KL55) GALV. STEEL, APPLY PLATES TO EACH FACE OF THRUSS AND OVERLAP/LOCATED ON THIS DESIGN. POSITION THE PIER DRAININGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AERPA A3 OF 1911-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE THRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AERPA/TPI 1 SEC. 2.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8228-	92712
DATE	03/17/10	
PRW	HCUSR8228	10076008
IC-ENG	JB/DF	*
EQN-	81139	
REF-	1U068228Z03	

ALPINE

ITTW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.20" due to live load and 0.21" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

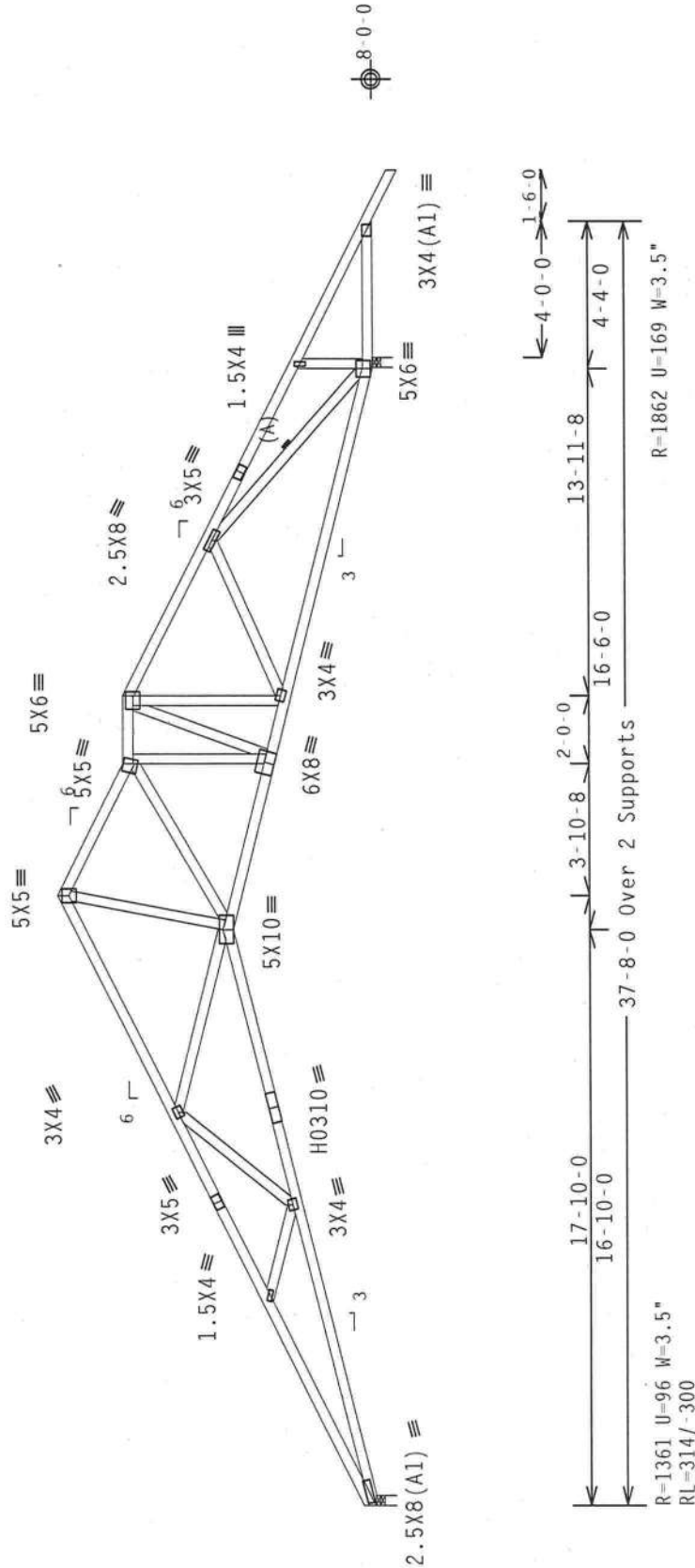
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $GCP(+/)=0.18$

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



PLT TYP. 20 Gauge HS.Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

QTY:1 FL/-/4/-/R/-

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R8228- 92713
DATE 03/17/10
DRW HGUSR8228 10076009
HC-ENG JB/DF
SEQN- 81135
JREF- 1U068228Z03

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NFCA (NATIONAL COUNCIL OF AMERICAN TRUSS ROOFING ENTERPRISE LABE, PHOENIX, AZ 85016) FOR SAFETY PRACTICES. PROPER TRUSS BRACING AND SHORING SHALL BE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PFA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/K) ASTM A653 GRADE 40/60 (M, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

JAMES P. JOHNSON, JR.
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar 11/2010

(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

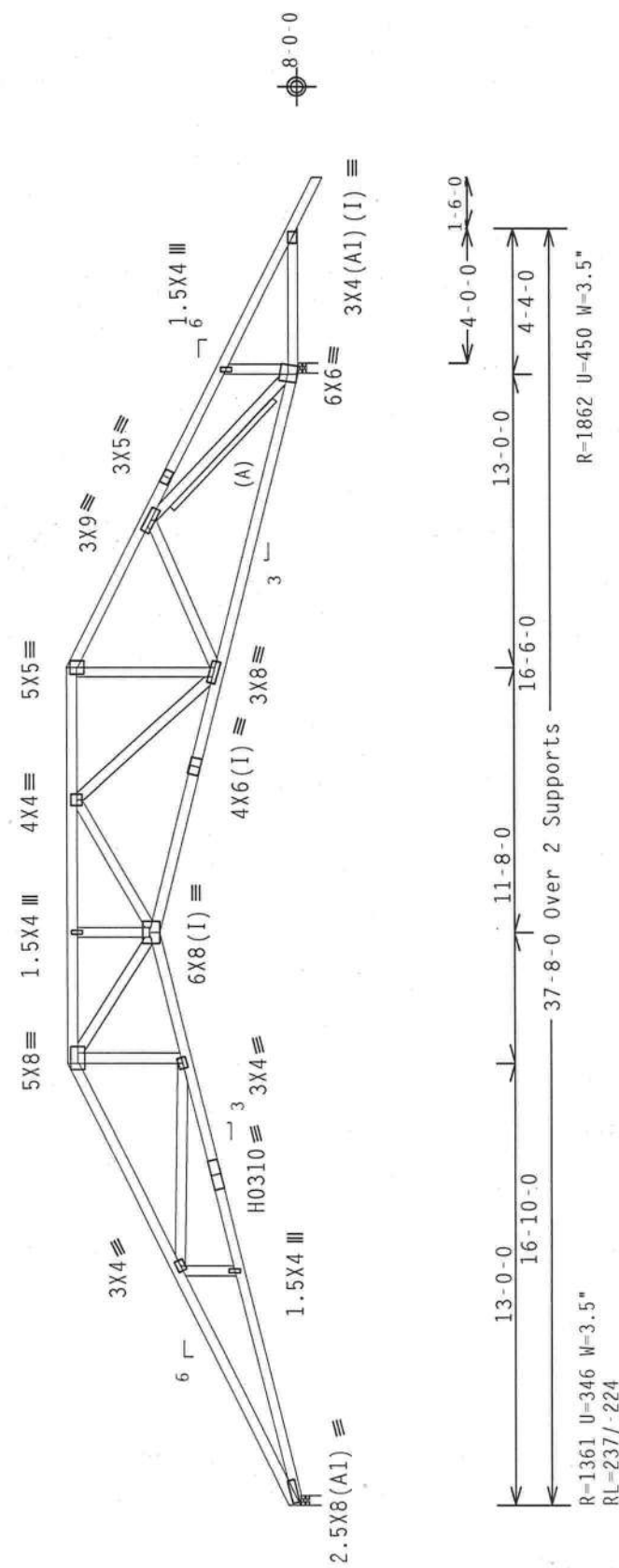
Deflection meets L/240 live and L/180 total load.

Calculated horizontal deflection is 0.24" due to live load and 0.25" due to dead load.

(A) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

Scale = .1875" / Ft.

FL/-/4/-/-/R/-

~~00~~

2002 (SID)
/0(0) 9.02

FBC200/Res/1PI
FT/RT=10%(0%)

Design Crit:

S, Wave

PLT TYP. 20 Gauge H

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WPCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, BOLDT 508, MI 49759) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****—URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF PROSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC., BY AREA AND TP1.

CONNECTION PLATES ARE MADE OF 207/196GA (47-85/75) ASTM A563 GR.50 STEEL. GALV. STEEL APPROX. 11% BGG

STATES THAT EACH FACE OF THE TRUSS CHORDS WILL BE WELDED TO THE CONNECTION PLATE PER DRAWINGS 160A-2.

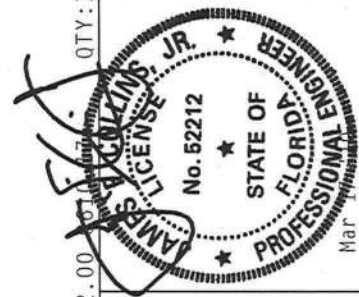
ACCEPTANCE OF THIS SHALL BE PER ANNEX A OF TP1-2002 SEC.3. A SEAL ON THIS

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOW.

THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

DESIGNING DESIGNER PER ANSI/TPI 1 SEC.-2.



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

	Top chord	2x4	SP	#2	Dense
	Bot chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind; 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI(+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.19" due to live load and 0.20" due to dead load.

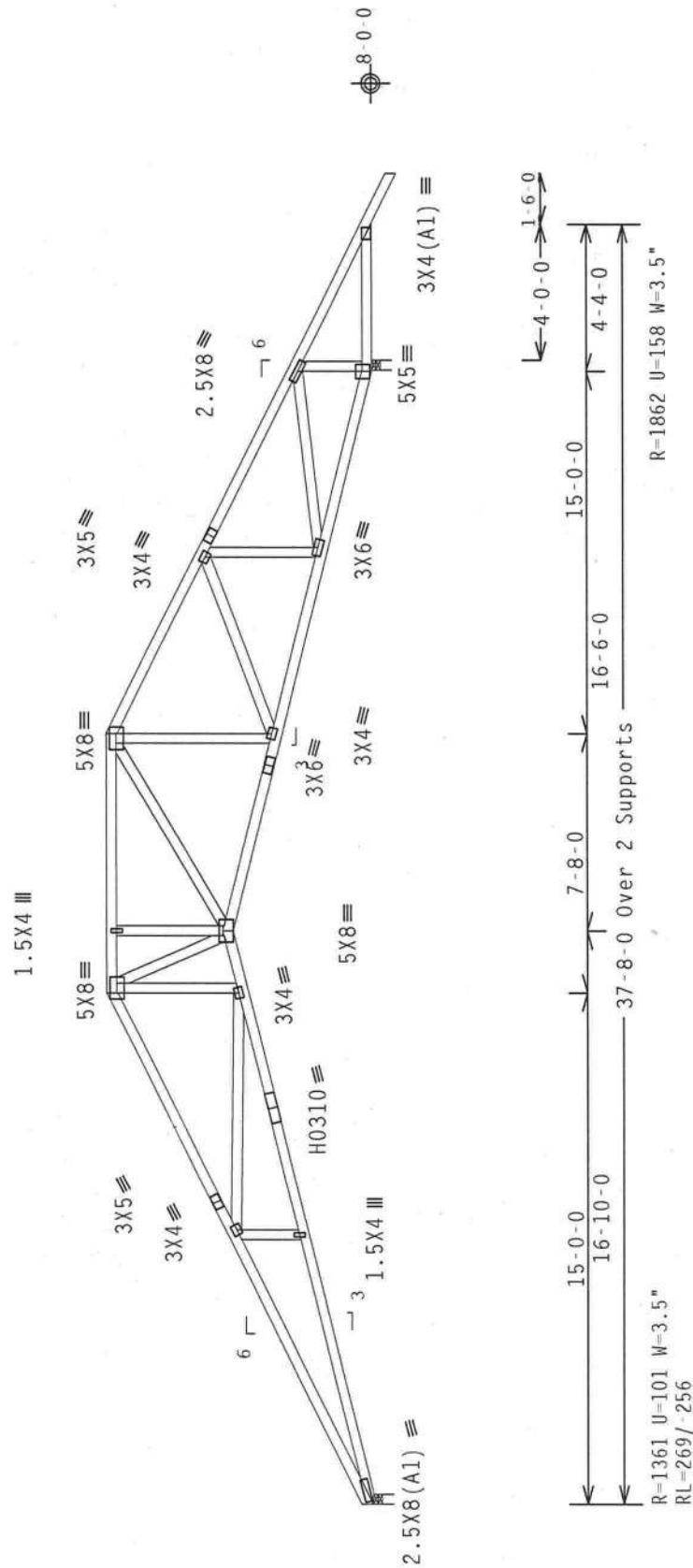
Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. 20 Gauge HS, Wave

Scale = .1875"/Ft.

[illegible]

RFF R8228- 92715

DATE 03/17/10

DATE 05/11/10

JRW HCU8R8228 100/601

HC-ENG JB/DF

SEQN- 81115

JREF- 1U068228703

[illegible]

Mar 11 1964

100

Haines City, FL 33844
FL COA #0 278

(10-018--Fill in later MIKE BALL --, ** - H17AV)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.18" due to live load and 0.19" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

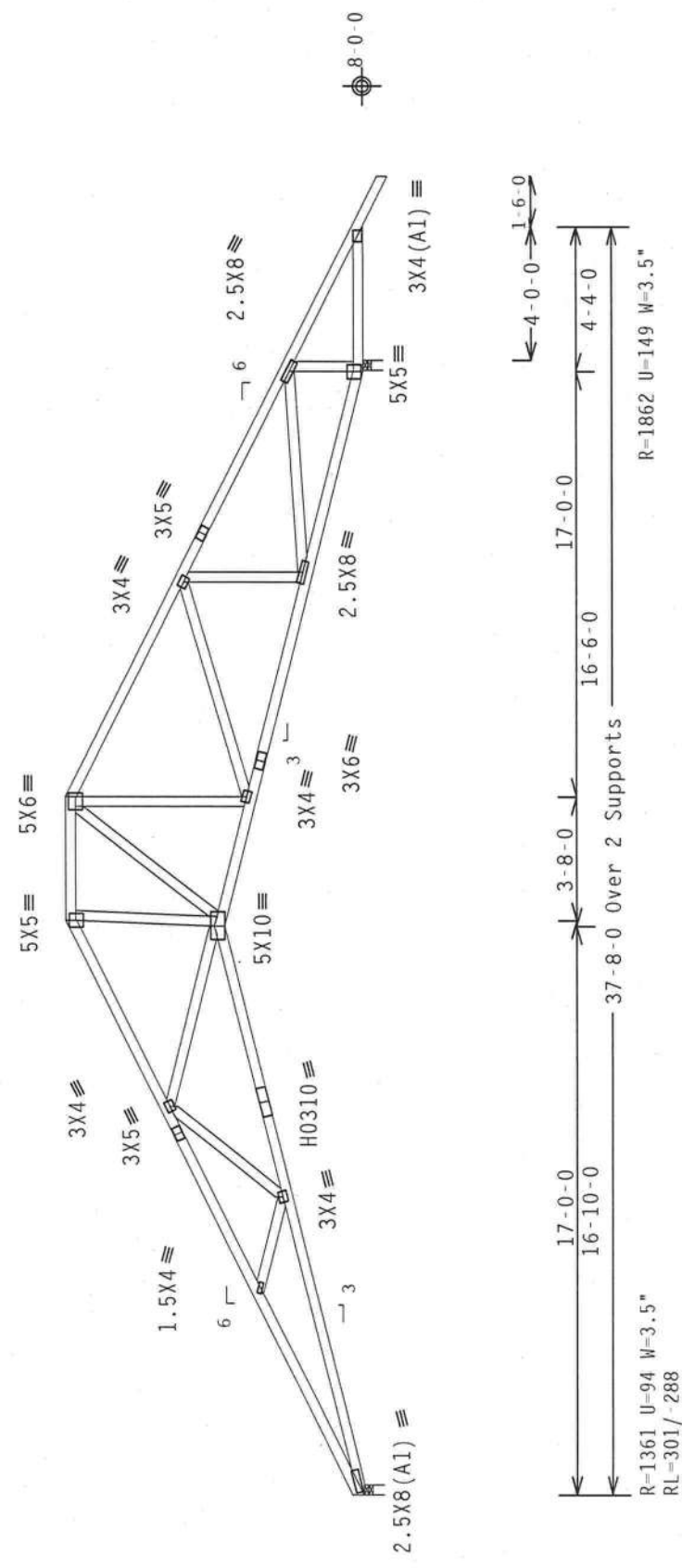
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat IC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



PLT TYP. 20 Gauge HS, Wave
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%)/0 (0)

Scale = .1875" / Ft.	QTY: 1	FL / - / 4 / - / R / -	REF R8228 - 92716
TC LL 20.0 PSF	TC DL 10.0 PSF	BC DL 10.0 PSF	DATE 03/17/10
BC LL 0.0 PSF	TOT.LD. 40.0 PSF	DUR.FAC. 1.25	DRW HCUSR8228 10076012
SPACING 24.0"	JREF- 1U068228Z03	HC-ENG JB/DF	SEQN- 81122

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WPCA (WOOD PRODUCT ASSOCIATION, 1000 WOODBURN AVENUE, SUITE 100, WOODBRIDGE, ONTARIO, CANADA) FOR THE LATEST INFORMATION ON THE PROPER USE OF TRUSSES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/K) ASTM A653 GRADE 40/60 (N. K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

AMERICAN INSTITUTE OF PROFESSIONAL ENGINEERS
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar 17 10

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.19" due to live load and 0.20" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

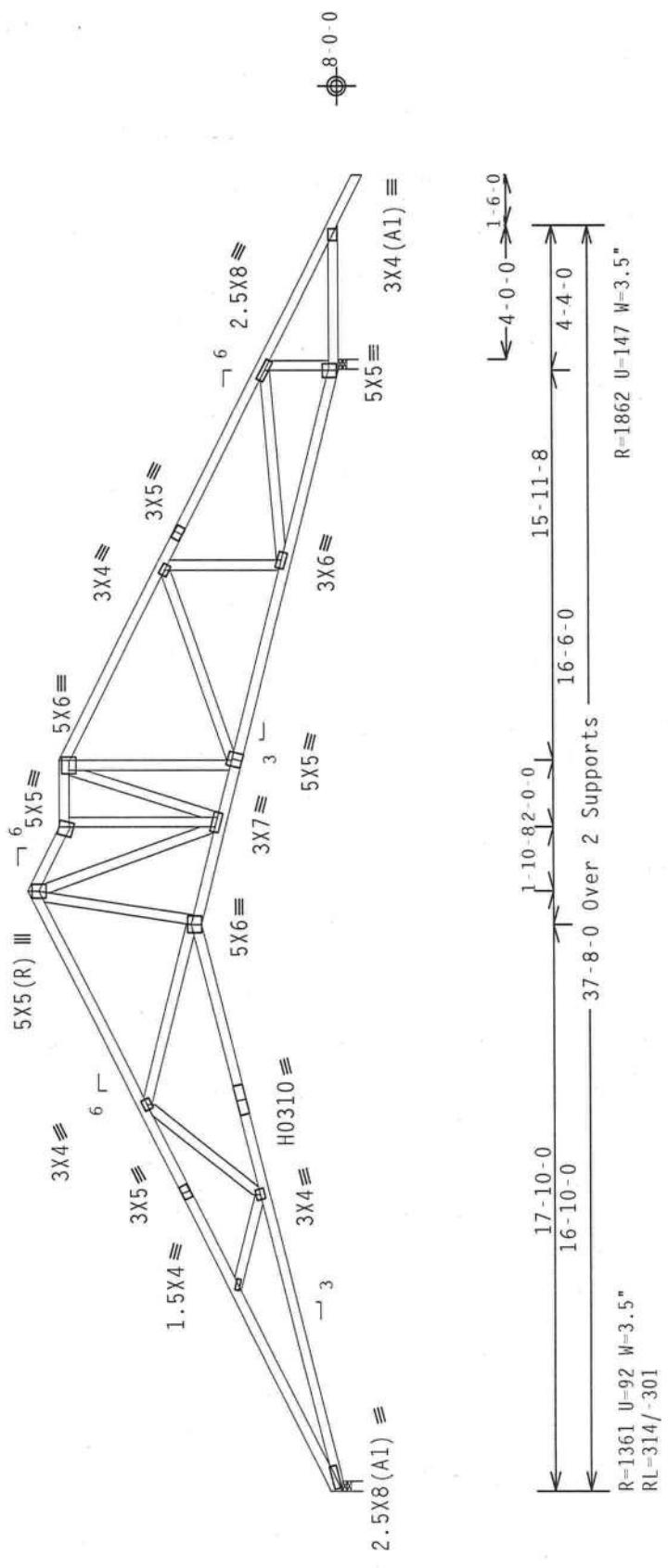
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



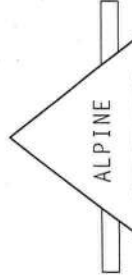
PLT TYP. 20 Gauge HS, Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/70(0)

Scale = .1875"/Ft.	REF R8228- 92718
DATE 03/17/10	TC LL 20.0 PSF
DRW HCUSR8228 10076014	TC DL 10.0 PSF
HC-ENG JB/DF	BC DL 10.0 PSF
SEQN- 81131	BC LL 0.0 PSF
DUR.FAC. 1.25	TOT.LD. 40.0 PSF
SPACING 24.0"	
JREF- 1U068228Z03	

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (CROSS PLATE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND TPI (TPI TRUSS COMPANY, 11111 11TH AVE, SUITE 100, FORT WORTH, TEXAS 76116) FOR TRUSS SAFETY INFORMATION. THESE INSTRUCTIONS ARE TO BE FOLLOWED TO PERFORM THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN OR FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/MS) AND TPI. 11TH REG. CONNECTOR PLATES ARE MADE OF 2018/16GA (4-H/55/K) ASTM A653 GRADE 40/60 (4. K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIA/MS SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



JAMES F. COLLINS, JR.
PROFESSIONAL ENGINEER
STATE OF FLORIDA
No. 52219

(10-018--Fill in later MIKE BALL --, ** - HS7B)

Top chord 2x6 SP #1 Dense :T1 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense :B1 2x6 SP #2:
Webs 2x4 SP #3 :W10 2x6 SP #2:

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Roof overhang supports 2.00 psf soffit load.

(B) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Brg blocks:0.131"x3" nails
brg x-loc #blocks length/blk #nails/blk wall plate
2 23.708' 1 12" 4 Rigid Surface
Brg block to be same size and species as bottom chord.
Refer to drawing CNAISP0109 for more information.

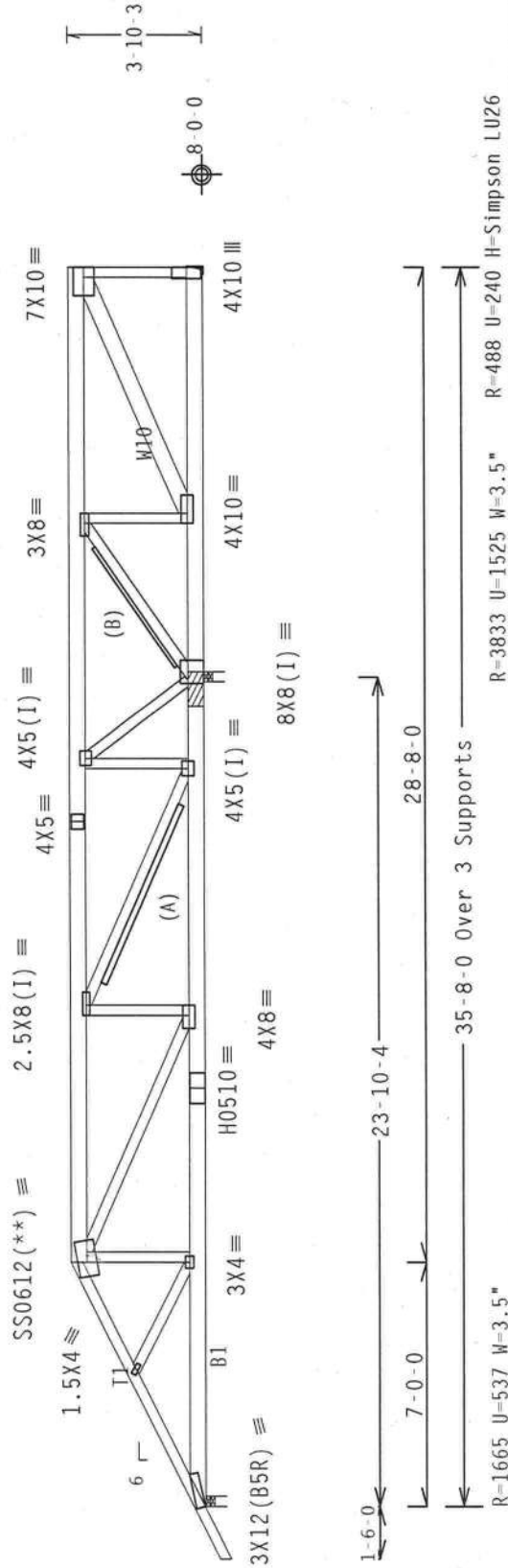
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, Exp C, wind TC DL-5.0 psf, wind BC DL-5.0 psf. Iw-1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(A) 2x6 #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

#1 hip supports 7-0-0 jacks with no webs.



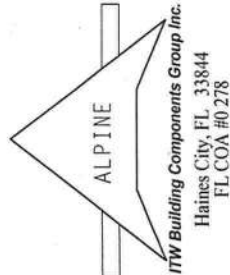
PLT TYP. 20 Gauge HS.18 Gauge HS, Design Crit: FBC2007Res/TPI-2002 (STD)
Wave FT/RT=10%(0%)/0(0)

R-3833 U=1525 W=3.5" R-488 U=240 H-Simpson LU26
w/ (4) 10d, 0.148"x1.5" nails in Truss
w/ (6) 10d Common, 0.148"x3.0" nails in Girder
Girder is (2) 1.50x 5.50 SolidSawn
Scale = .1875"/Ft.



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 6300 NORTH LIFE STREET, HOUSTON, TX 77061-1422, AND THE TPI TRUSS MANUFACTURING PRACTICES MANUAL, TPI TRUSS MANUFACTURING PRACTICES, 522101, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. 1TH BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (N/A/SS/2K) ASTM A653 GRADE 40/60 (N/A K/DL-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER AISC A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 92719
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076015
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	81173
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1U068228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(B) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpi(+/-)=0.18

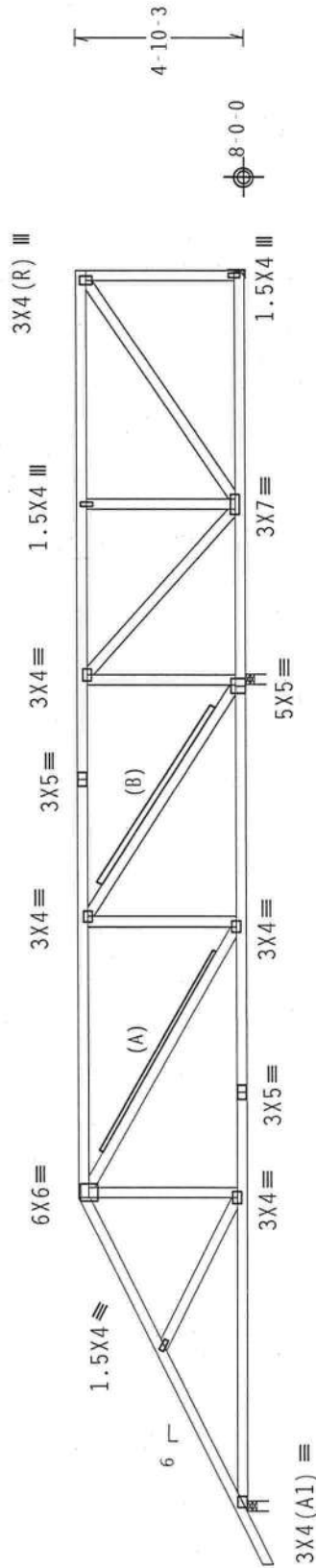
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(A) 1x4 #3SR8 SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



1-6-0
9-0-0
23-10-4
26-8-0
35-8-0 Over 3 Supports
R=1001 U=249 W=3.5"
RL=179/-69
R=1733 U=490 W=3.5"
R=303 U=102 H=Simpson LU26
w/ (4) 10d, 0.148"x1.5" nails in Truss
w/ (6) 10d Common, 0.148"x3.0" nails in Girder
Girder is (2) 1.50x 5.50 SolidSawn

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

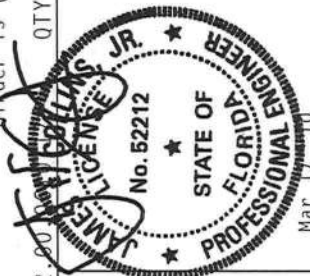
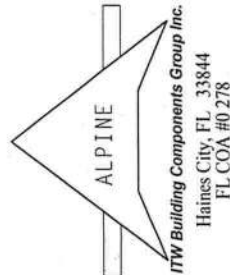
Scale = 1.875" / Ft.

QTY:1 FL/-4/-/-R/-

TC LL	20.0 PSF	REF	R8228- 92720
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076016
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	81190
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1U068228Z03

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND TPCA (TRUSS PLATE CONSTRUCTION ASSOCIATION, 10000 WILSON AVENUE, SUITE 100, WILSON, CA 94093) FOR DETAILED INSTRUCTIONS REGARDING THE PROPER USE OF TRUSSES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PFA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/8) ASTM A653 GRADE 40/60 (H, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



(10-018--Fill in later MIKE BALL --, ** - H11B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

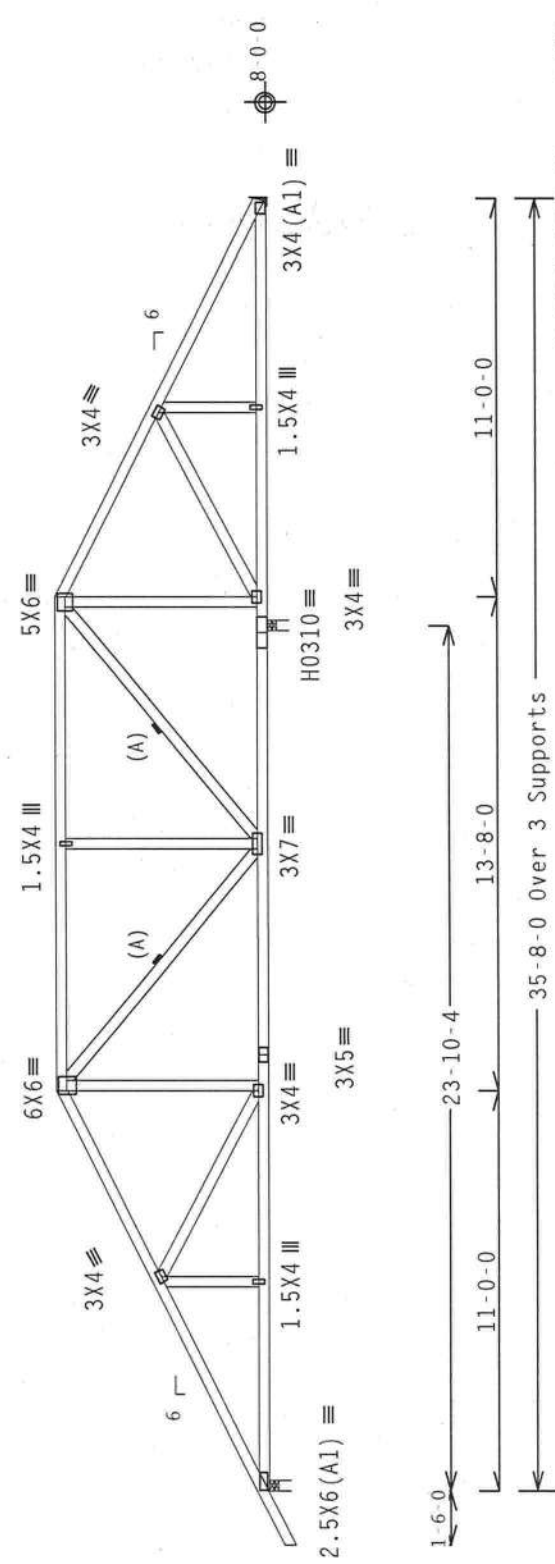
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



R=1238 U=333 W=3.5"
RL=190/-203

R=1013 U=253 W=3.5"

R=787 U=210 H-Simpson LU26
w/ (4) 10d, 0.148"x1.5" nails in Truss
w/ (6) 10d Common, 0.148"x3.0" nails in Girder
Girder is (2) 1.50x5.50 SolidSawn

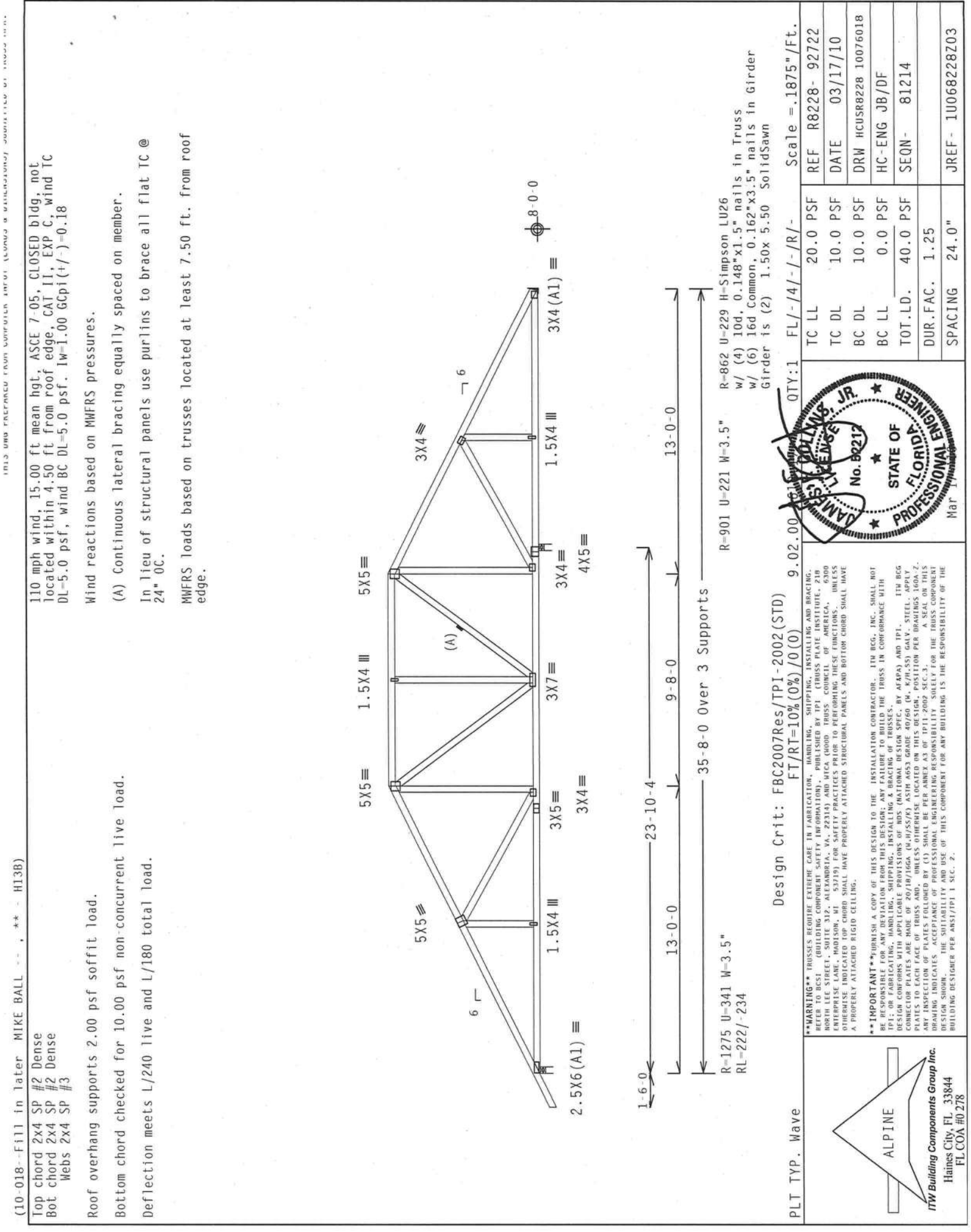
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

QTY:1 FL/-4/-/R/- Scale =.1875"/Ft.

PLT TYP. 20 Gauge HS, Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TENSION PLATE INSTITUTE), 2100 NORTH LEE STREET, SUITE 312, ALPHARETTA, GA 30009, (770) 962-2340, FOR THE LATEST EDITIONS OF THE TPI TRUSS DESIGN GUIDE, TPI TRUSS DESIGN GUIDE FOR SAFE PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.		TC LL	20.0	PSF	REF	R8228- 92721
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.		TC DL	10.0	PSF	DATE	03/17/10
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI1. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/55X) ASTM A653 GRADE 40/60 (M. K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BUILDING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.		BC DL	10.0	PSF	DRW	HCUSR8228 10076017
		BC LL	0.0	PSF	HC-ENG	JB/DF
		TOT.LD.	40.0	PSF	SEQN	81197
		DUR.FAC.	1.25			
		SPACING	24.0"		JREF	1U068228Z03





(10-018--Fill in later MIKE BALL --, ** - H13B)		Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3		Roof overhang supports 2.00 psf soffit load.		Bottom chord checked for 10.00 psf non-concurrent live load.		Deflection meets L/240 live and L/180 total load.	
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpl(+/-)=0.18		Wind reactions based on MWFRS pressures.		(A) Continuous lateral bracing equally spaced on member.		In lieu of structural panels use purlins to brace all flat TC @ 24" OC.		MWFRS loads based on trusses located at least 7.50 ft. from roof edge.	
R-1275 U-341 W=3.5"		R-901 U-221 W=3.5"		R-862 U-229 H=Simpson LU26 w/ (4) 10d, 0.148"x1.5" nails in Truss w/ (6) 16d Common, 0.162"x3.5" nails in Girder Girder is (2) 1.50x 5.50 SolidSawn		Scale = 1.875"/Ft.		QTY: 1 FL/-/4/-/R/-	
RL=222/-234		35'-8"-0 Over 3 Supports		Design Crit: FBC2007Res/TPI-2002 (STD) FT/RT=10% (0%)/0 (0)		9.02.00		9.02.00	
PLT TYP. Wave		ALPINE		ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278		R-1275 U-341 W=3.5"		R-901 U-221 W=3.5"	
R-1275 U-341 W=3.5"		R-901 U-221 W=3.5"		R-862 U-229 H=Simpson LU26 w/ (4) 10d, 0.148"x1.5" nails in Truss w/ (6) 16d Common, 0.162"x3.5" nails in Girder Girder is (2) 1.50x 5.50 SolidSawn		Scale = 1.875"/Ft.		QTY: 1 FL/-/4/-/R/-	
TC LL 20.0 PSF		TC DL 10.0 PSF		BC DL 10.0 PSF		BC LL 0.0 PSF		TOT.LD. 40.0 PSF	
DUR.FAC. 1.25		SPACING 24.0"		JREF - 1U068228203		REF R8228- 92722		DATE 03/17/10	
DRW HCUSR8228 10076018		HC-ENG JB/DF		SEQN- 81214		Scale = 1.875"/Ft.		QTY: 1 FL/-/4/-/R/-	
R-1275 U-341 W=3.5"		R-901 U-221 W=3.5"		R-862 U-229 H=Simpson LU26 w/ (4) 10d, 0.148"x1.5" nails in Truss w/ (6) 16d Common, 0.162"x3.5" nails in Girder Girder is (2) 1.50x 5.50 SolidSawn		Scale = 1.875"/Ft.		QTY: 1 FL/-/4/-/R/-	

(10-018--Fill in later MIKE BALL --, ** - H15B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

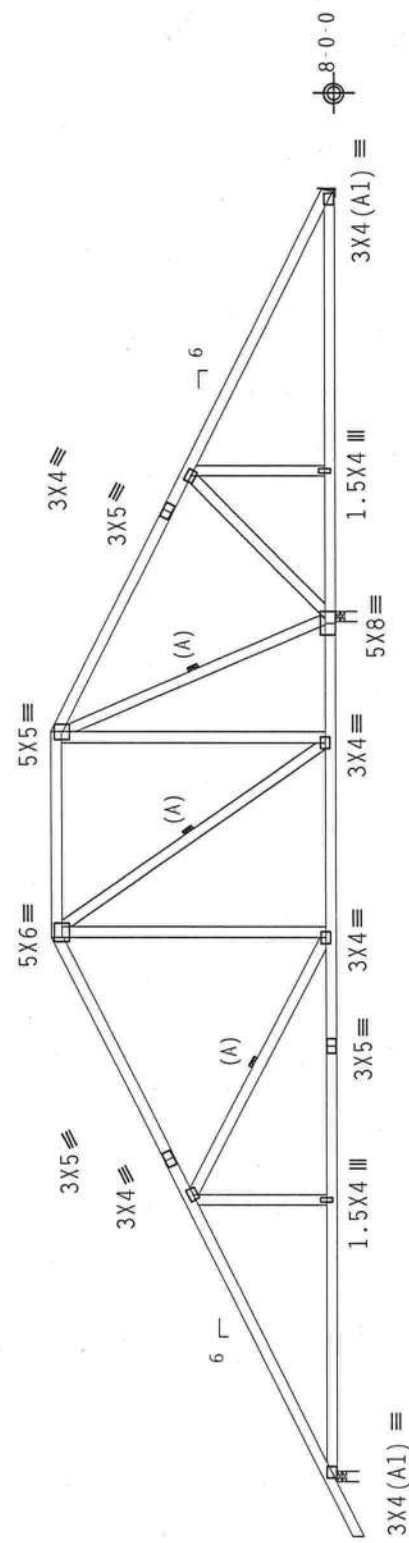
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpl (+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat IC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



1'-6-0
23'-10-4
15'-0-0
5'-8-0
15'-0-0

R=1021 U=104 W=3.5"
RL=253/-266
R=1662 U=122 W=3.5"

R=355 U=81 H-Simpson HU926
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girders (2) 1.50x5.50 SolidSawn

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

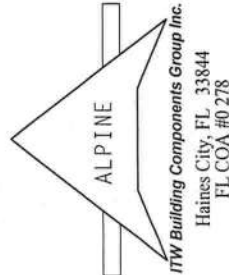
PLT TYP. Wave

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****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW REG. INC. SHALL NOT BE RESPONSIBLE FOR DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/P3) AND TPI. 1TH REG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/55/14) ASH A653 GRADE 40/60 (M. K/H-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 92723
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076021
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN	81225
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1U068228Z03



	Top	Bot	Dense
chord	2x4	2x4	2x4
chord	2x4	2x4	2x4
webs	2x4	2x4	2x4

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

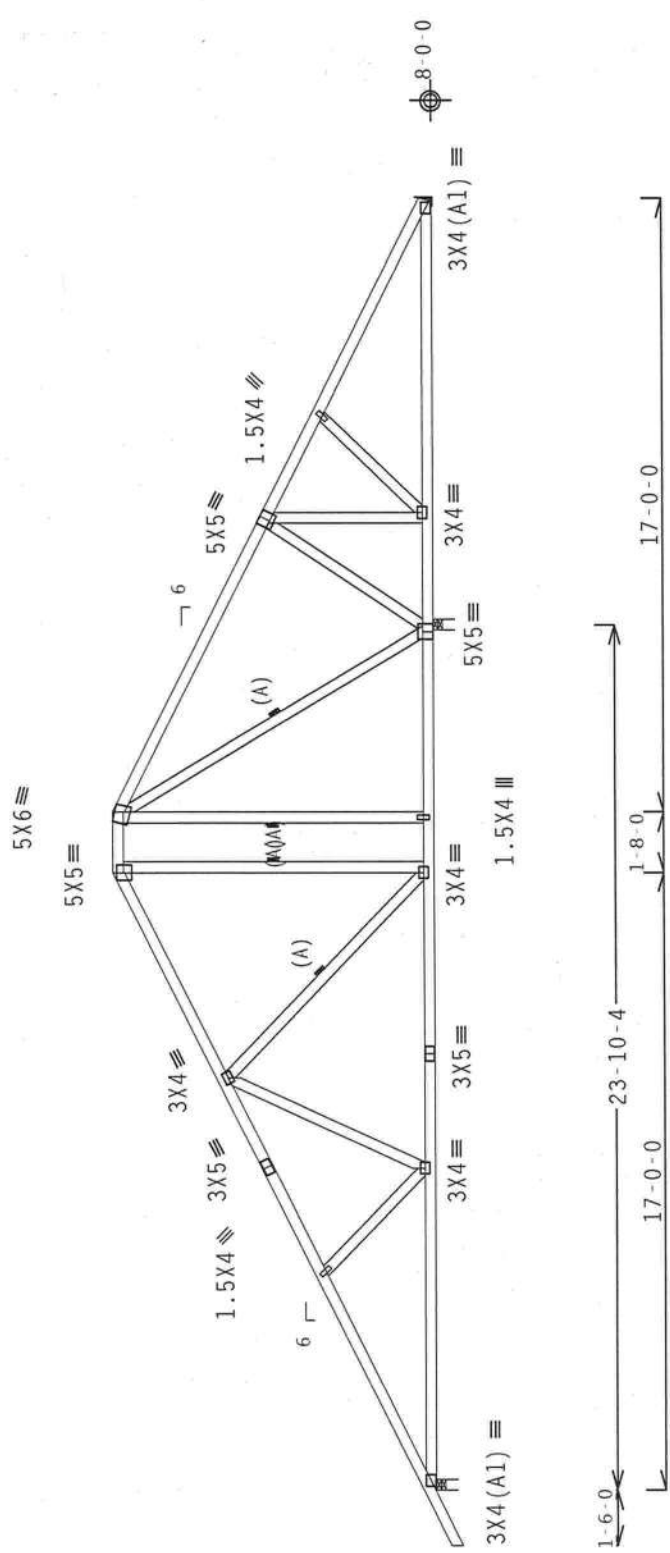
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R-543 U-23 H-Simpson LU26
w/ (4) 10d, 0.148"x1.5" nails in Truss
w/ (6) 10d Common, 0.148"x3.0" nails in Girder
Girders is (2) 1.50x 5.50 SolidSawn

QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

PLT TYP. Wave

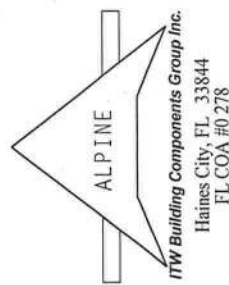
*****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY THE TRUSS PLATE INSTITUTE, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 2100 ENTERPRISE LANE, HANSDLOW, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY APPLICATION FROM THIS DESIGN. ALL TRUSSES TO BE FABRICATED IN CONFORMANCE WITH THE DESIGN COMPARE WITH AVAILABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI.

CONNECTION PLATES ARE MADE OF 2010/1608, OR 04/25/93 ASTM A663 GRADE 40/60 (N. K/H-55) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z.

IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX 43 OF TPI-2002 SEC. 3-2.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT BEING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANNEX/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 92724
TC DL	10.0 PSF	DATE 03/17/10
BC DL	10.0 PSF	DRW HCU8R8228 10076019
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 81233
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1U068228Z03

Top chord 2x4 Sp #2 Dense
Bot chord 2x4 Sp #2 Dense
Webs 2x4 Sp #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

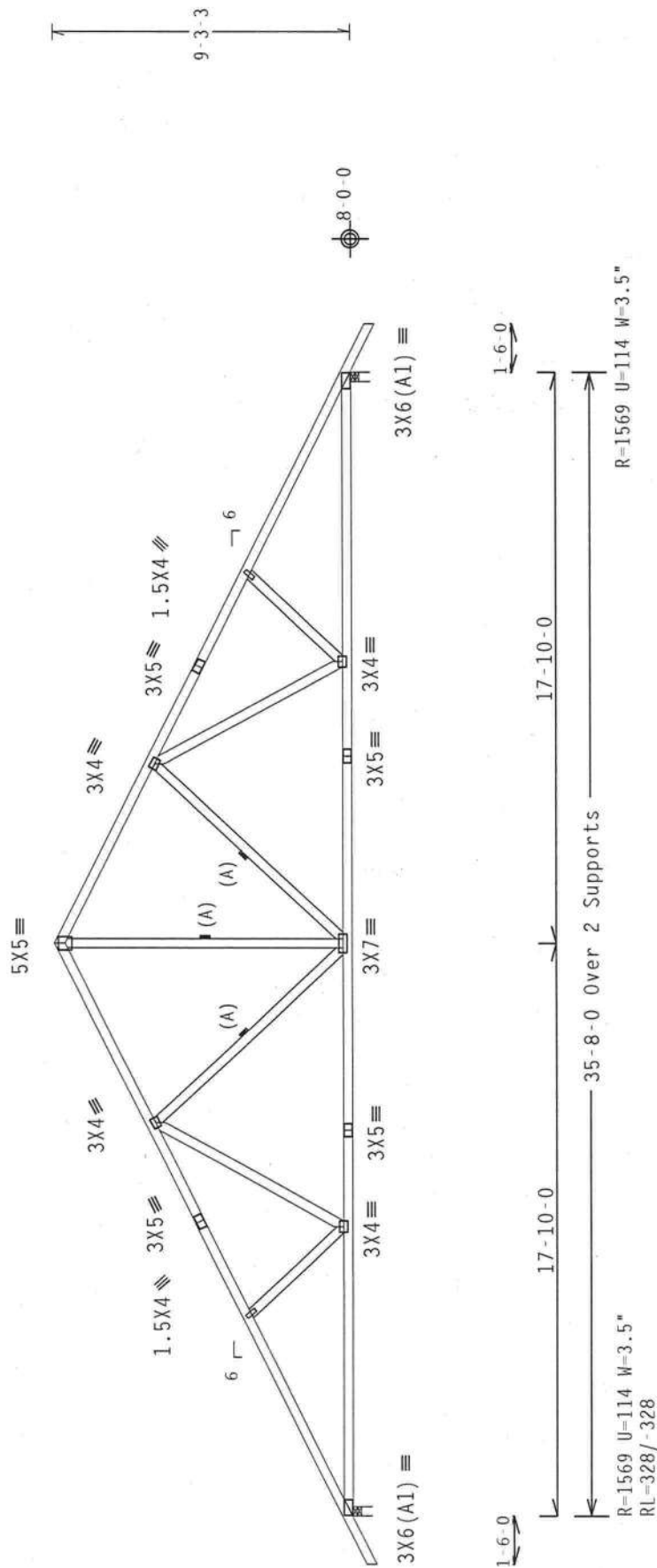
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00$ GCpi (+/-)=0.18

Wind reactions based on MMFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

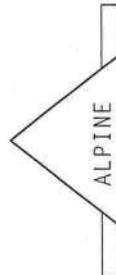
Scale = .1875" / Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8228- 92725
DATE	03/17/10
DRW	HCUSR8228 10076008
HC-ENG	DLJ/DLJ *
SEQN	83024
JREF	1U068228Z03

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2100 NORTH LEE STREET, SUITE 312, ALPHARETTA, GA 30009 (404) 233-1100 FOR MORE INFORMATION. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



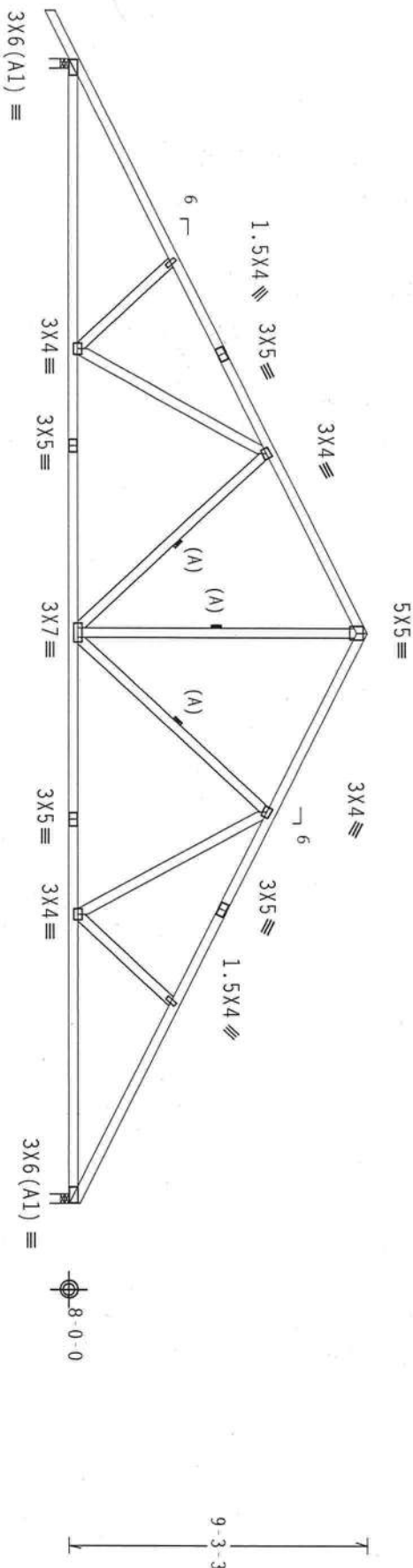
THIS WORK PREPARED FROM COMPUTER INPUT (LUAUS & DIMENSIONS) SUBMITTED BY IKUSS MTK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ gcpi (+/-)=0.18

Wind reactions based on MIFRS pressures.

Bottom chord checked for 10.00 psf non concurrent live load.

MIFRS loads based on trusses located at least 15.00 ft. from roof edge.



17-10-0

17-10-0

35-8-0 Over 2 Supports

R=1572 U=115 W=3.5"

R=1466 U=102 W=3.5"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:1

FL/-/4/-/-/R/-/-

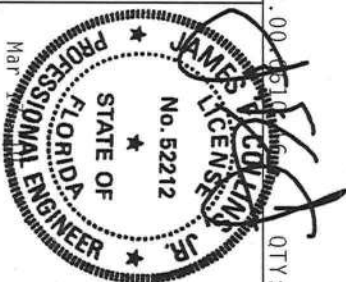
Scale = .1875"/Ft.



ITW Building Components Group Inc

Haines City, FL 33844

FL COA #0 278



TC LL	20.0 PSF	REF	R8228- 92726
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076015
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ *
TOT.LD.	40.0 PSF	SEQN-	83041
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1U068228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

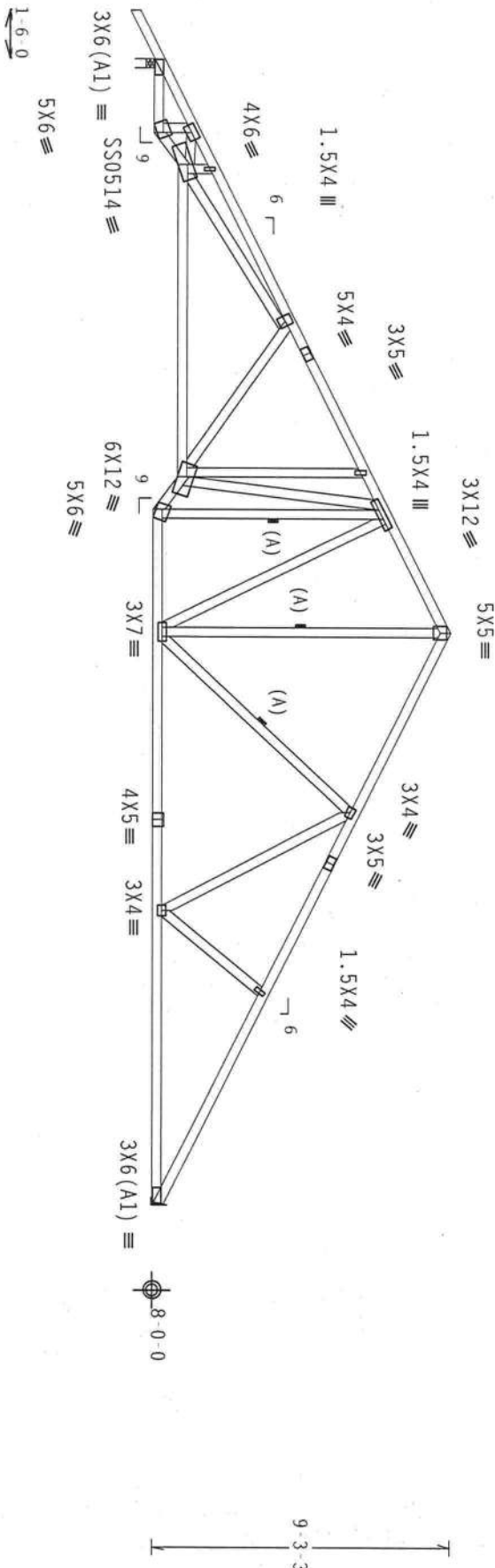
MMFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.



17'-10-0
21'-8-0
35'-8-0 Over 2 Supports
R=1623 U=110 W=3.5"
R=298/-311
R=1545 U=100

PLT TYP. 18 Gauge HS.Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

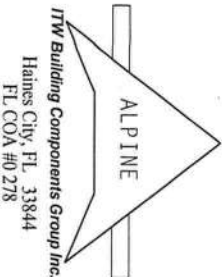
9.02.00

QTY:2 FL/-/4/-/-/R/-

Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRE EXTERIOR GABLE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (PROVIDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6100 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TIV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH DESIGN CONFORMS WITH, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, BY AREA AND TPI. TIV BCG DESIGN CONFORMS WITH, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, BY AREA AND TPI. TIV BCG CONNECTOR PLATES ARE MADE OF 2018/1664 IN ALUMINUM, ASTM A635 GRADE 40/60 OR 47N/53 GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 92727
TC DL	10.0 PSF	DATE 03/17/10
BC DL	10.0 PSF	DRW HCUR8228 10076005
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT. LD.	40.0 PSF	SEON- 83019
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1U068228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind 15.00 ft mean hgt. ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp1(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MMFRS pressures.

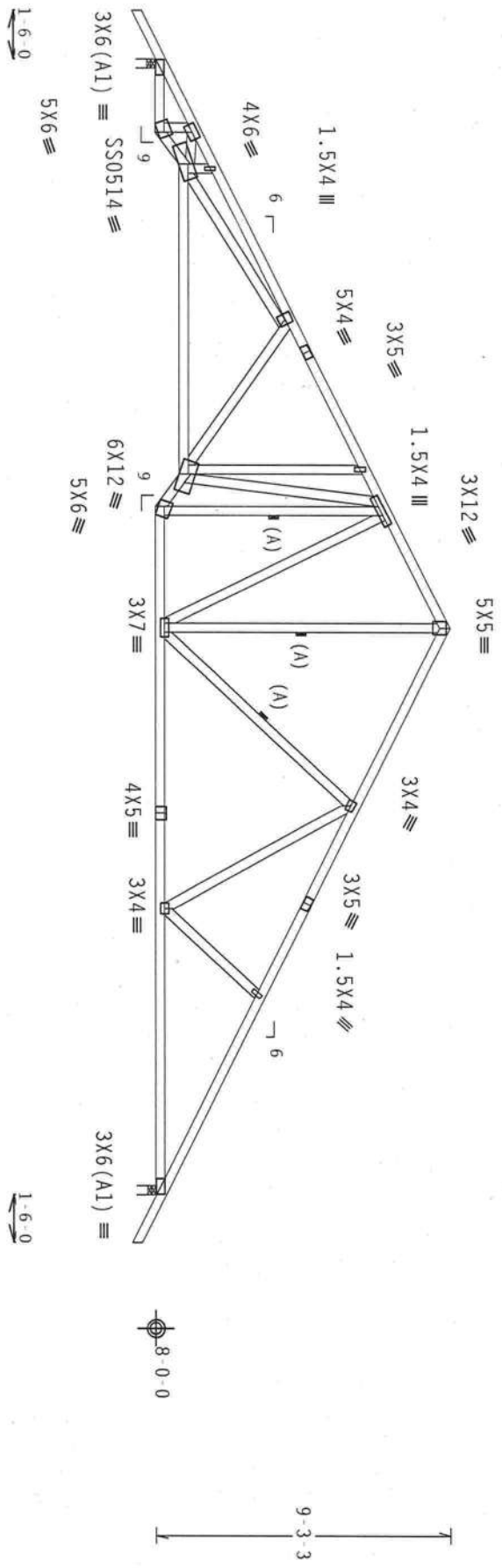
(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



17'-10-0
9'-8-8
1'-0-6
35'-8-0 Over 2 Supports
21'-8-0
17'-10-0
2'-3-81'-0-0
R=1620 U=109 W=3.5"
RL=328/-328
R=1651 U=113 W=3.5"

PLT TYP. 18 Gauge HS, Wave
Design Cr1t: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)
9.02.00
QTY:2 FL/-/4/-/-/R/-
Scale = .1875"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ALPINE

RTW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

STATE OF FLORIDA

PROFESSIONAL ENGINEER

JAMES B. KOSKOFF JR.

No. 52212

Mar 17 2010

TC LL	20.0 PSF	REF R8228- 92728
TC DL	10.0 PSF	DATE 03/17/10
BC DL	10.0 PSF	DRW HCUR8228 10076007
BC LL	0.0 PSF	HC-ENG DLJ/DLJ *
TOT. LD.	40.0 PSF	SEQN- 83013
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1U068228Z03

THIS WORK PREPARED FROM COMPUTER INPUT (LUAUS & DIMENSIONS) SUBMITTED BY IKUSS MTK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT 1, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ Gcpl (+) =0.18

Wind reactions based on MWFRS pressures.

#1 hip supports 7-0-0 jacks with no webs.


$$\overline{FT/RT}=10\%(0\%)/0(0)$$

OT:1

Scale = .3125" / Ft.



ME
LIC
No. 52219
JP

REF	R8228 - 92729
DATE	03/17/10
DDM	uclic000000 1007600

ITW Building Components Group Inc

FLCOA #0278

100

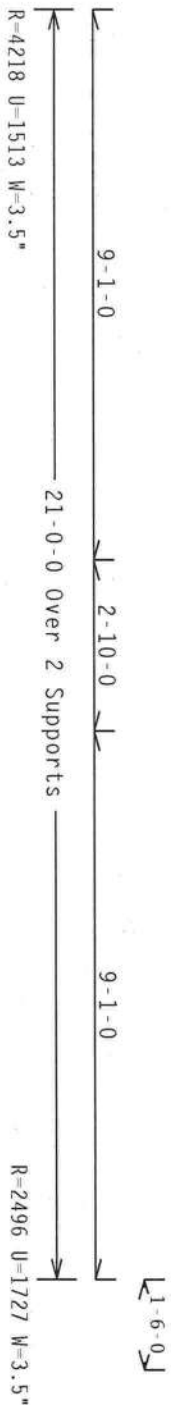
2

Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC, DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpl(+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Scale = .3125"/ft.

No. 52212

.....

10

BC DL 10.0 PSF

DRW HCUSR8228 10076020

0.0 PST

ENGINEERING

Maj

FLCOA #0278

[illegible]

BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN - 81241
DUR.FAC.	1.25	
SPACING	24.0"	JREF - 1U068228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

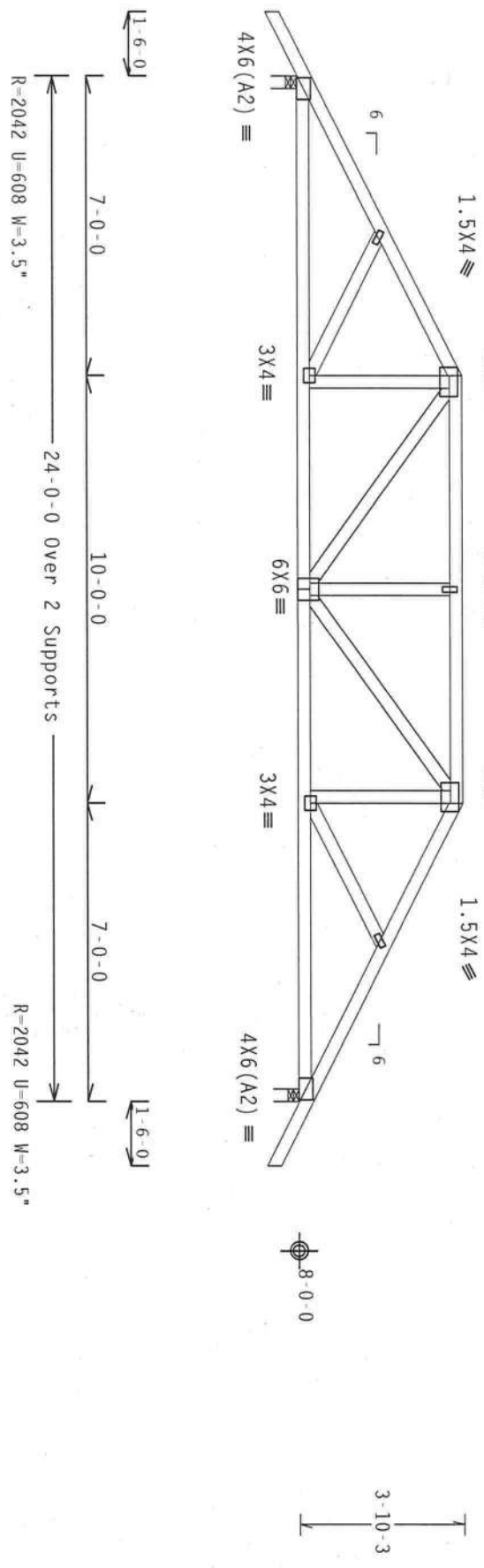
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Special loads

TC - From	Dur.Fac. = 1.25 / Plate Dur.Fac. = 1.25
62 pif at -1.50 to 7.00	62 pif at 7.00
TC - From	62 pif at 7.00 to 17.00
62 pif at 17.00 to 25.50	62 pif at 25.50
BC - From	4 pif at -1.50 to 0.00
4 pif at 0.00 to 20.00	4 pif at 20.00
BC - From	4 pif at 24.00 to 25.50
4 pif at 25.50	9.06, 11.06, 12.94
TC - 187 lb Conc. Load at 7.03	14.94, 16.97
BC - 129 lb Conc. Load at 7.03	14.94, 16.97
9.06, 11.06, 12.94	

#1 hip supports 7-0-0 jacks with no webs.



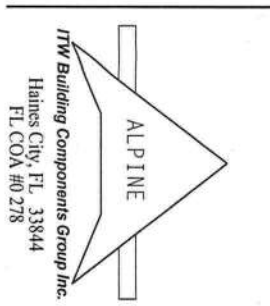
PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

9.02.001061

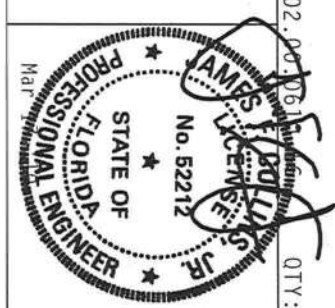
QTY:1 FL/-/4/-/-/R/-

Scale = .25"/Ft.



****WARNING**** BRUSSES BEHOLD EXTREME CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD ROSS CONSULT, OF AMERICA, 4500 ENTERPRISE LANE, MADISON, MI 48071) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, CONCORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. TNC. SHALL NOT BE AVOIDED FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF BRUSSES. THE REG. CONSTRUCTION PLATES ARE MADE OF 20/18/16GA (W/US/S) ASIN 4653 GRADE 40/60 (4, K/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 92731
TC DL	10.0 PSF	DATE 03/17/10
BC DL	10.0 PSF	DRW HCUR8228 10076001
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	40.0 PSF	SEQN- 82801
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1U068228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

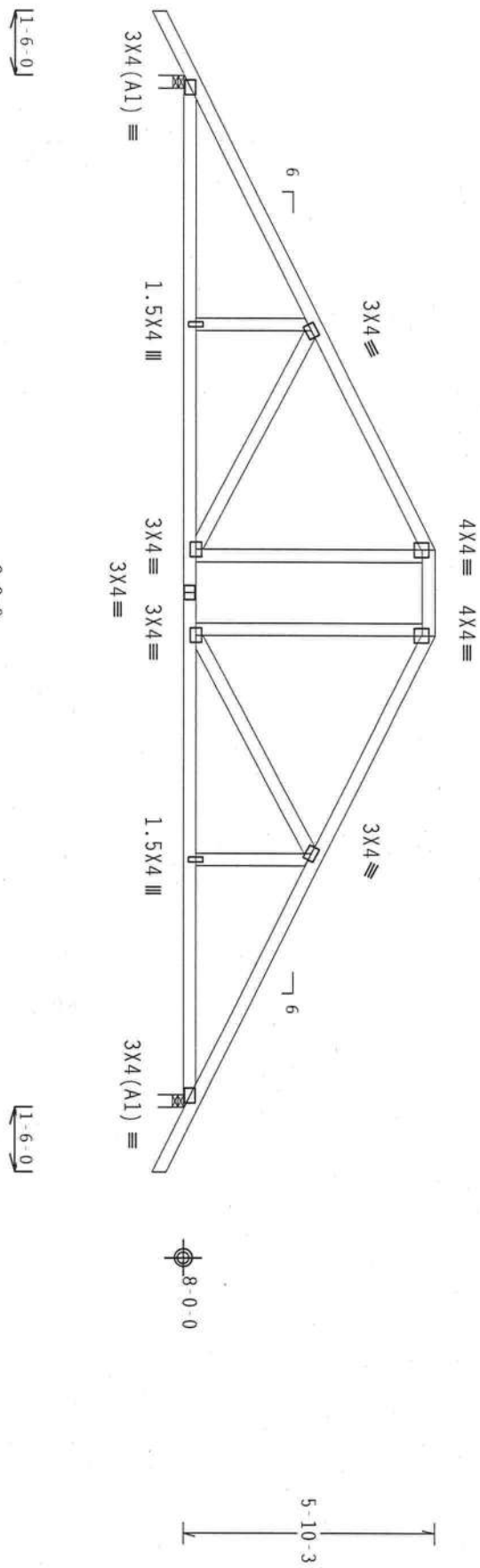
Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=10%(0%)/0(0)

9.02.20

QTY: 1

FL/-/4/-/-/R/-

Scale = .25"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PROHIBITED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WICK (WOOD TRUSS CONDUCT OF AMERICA), 6500 WICK ROAD, SUITE 100, WICK, MI 48181 FOR SAFETY PRACTICES. PERMANENTLY PERMANENTLY THESE INSTRUCTIONS. UNLESS OTHERWISE INDICATED FOR, WOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PLATES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY A/RPA AND TPI. THE RCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY A/RPA) AND TPI. THE RCG CONNECTIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY A/RPA) AND TPI. THE RCG PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

STATE OF FLORIDA

PROFESSIONAL ENGINEER

No. 52212

JR

Mar 17 2010

TC LL	20.0 PSF	REF R8228- 92733
TC DL	10.0 PSF	DATE 03/17/10
BC DL	10.0 PSF	DRW HCUR8228 10076014
BC LL	0.0 PSF	HC-ENG DLJ/DLJ *
TOT. LD.	40.0 PSF	SEQN- 82811
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1U068228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

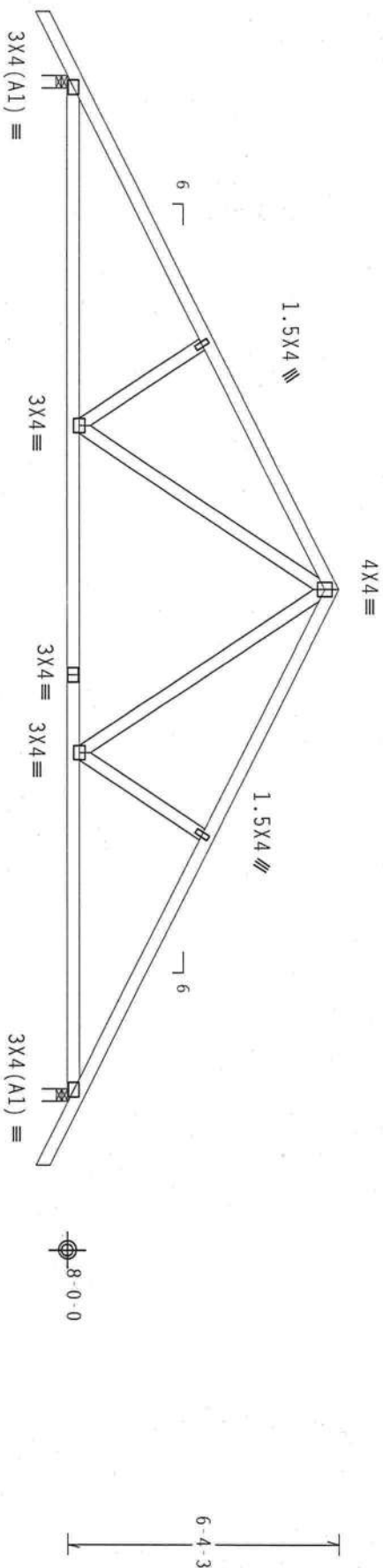
MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI(+/-)-0.18

Wind reactions based on MMFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



12'-0-0 24'-0-0 Over 2 Supports 12'-0-0
R=1140 U-284 W=3.5"
RL=228/-228
R=1140 U=284 W=3.5"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY: 2

FL/-/4/-/-/R/-

Scale = .25"/ft.

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CROSS PLATE INSTITUTE, 218 N. 1ST ST., SUITE 312, ALPHARETTA, GA 30201-3000 FOR THE LATEST RECOMMENDATIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED FIELD CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS. THE BCG CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION OF THE TRUSS.

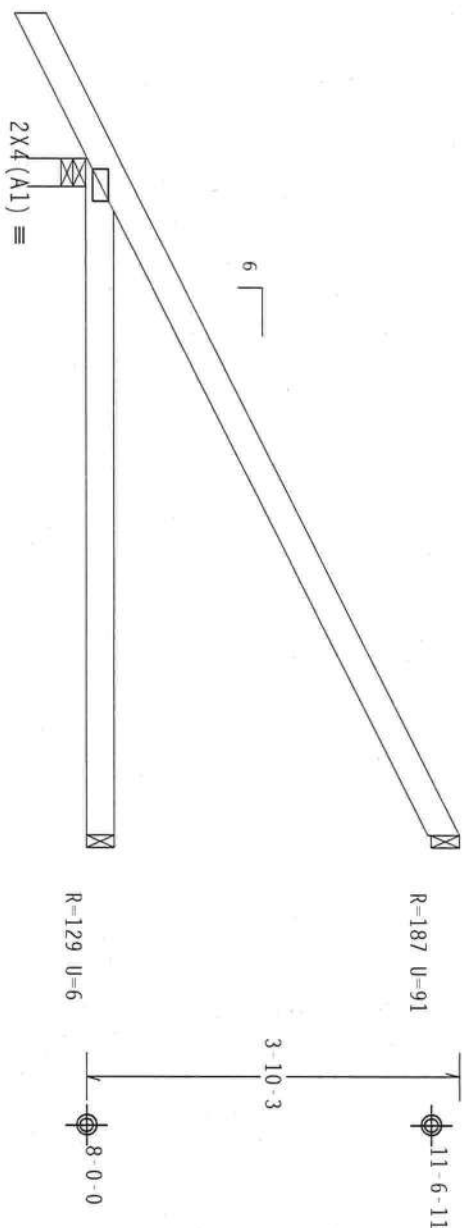
Mar 17 2009

TC LL	20.0 PSF	REF	R8228-92734
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076017
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT. LD.	40.0 PSF	SEQN-	82816
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1U068228203

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 gcpl(+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load.



Scale = .5"/Ft.

No. 52212
 JAMES W. COLLINS, JR.
 LICENSE
 218
 6,500
 5,55
 HAVE
 NG.

ITW Building Components Group Inc.

Haines City, FL 33844
FL 33844

TC LL	20.0 PSF	REF	R8228 - 92735
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076021
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	40.0 PSF	SEQN-	82782
DUR.FAC.	1.25		
SPACING	24.0"	JREF	IU068228Z03

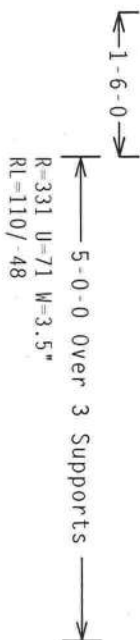
THIS WAS PREPARED FROM COMPUTER INPUT (LUNDS & VILHELMSSON)

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, closed bldg, not located within 4.50 ft from roof edge, CAT 11, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

Deflection meets L/240 live and L/180 total load.



Scale = .5" / Ft.

James L. James, Jr.
No. 52212



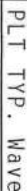
ALPINE

Haines City, FL 33844
FL COA #0278

TC LL	20.0 PSF	REF	R8228- 92736
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCSRSR8228 10076018
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	40.0 PSF	SEQN-	82779
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1U068228Z03

Hipjack supports 7-0-0 setback jacks with no webs. Deflection meets L/240 live and L/180 total load.

110 mph wind @ 5.00 ft mean hgt., ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT 1, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCFI (+/-)=0.18



Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)

$$FT/RT=10\%(0\%)/0(0$$

9.0200

QTY:

FL/-/4/-/-/R/

Scale = .5"/ft.

"WARNING" - PROJECTS INVOLVING EXTENSIVE CABLE IN FABRICATION, HANDLING, SHIPMENT, INSTALLATION AND BRACING REFER TO SC-1 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PANEL INSTITUTION), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 BUCKLE ENTERPRISE LANE, NORTHSIDE, MI 48139 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. OTHERS OTHERWISE INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELL WALL.

****IMPORTANT*** furnish a copy of this design to the installation contractor. ITW BCG, INC. SHALL NO

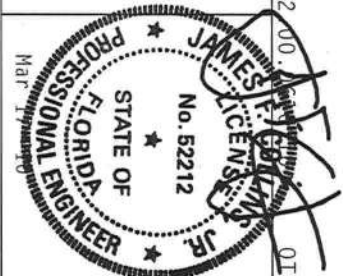
TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

PLATES TO EACH FACE OF BEAMS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-CONNECTION PLATES ARE MADE OF 20/10/1666 (H, H/55/K) ASTM A553 GRADE 40/60 (H, K/H, 55) GALV. STEEL, APPLICABLE TO EACH FACE OF BEAMS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/API 1 SEC. 2.

(continued)



TC LL	20.0 PSF	REF	R8228- 92737
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076004
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	40.0 PSF	SEQN-	82785
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	IU068228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

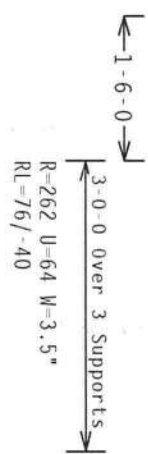
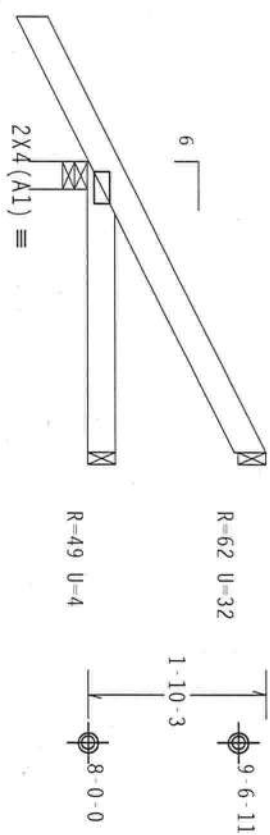
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide { 2 } 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide { 2 } 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0
psf. Iw=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

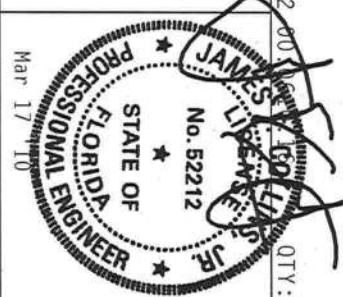
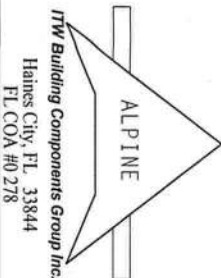
QTY:14 FL/-/4/-/-/R/-

Scale =.5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESI (BUILDING COMPONENT SAFETY INFORMATION), PROVIDED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK HOBSON TRUSS PRODUCTS CORPORATION, 21801 WICK HOBSON DRIVE, SUITE 312, ALEXANDRIA, VA, 22314 FOR TRUSS SAFETY INFORMATION. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PLATES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BRIDG CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE OF TRUSSES IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF 2001 INTERNATIONAL DESIGN SPEC. BY ACPA AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2018/1664 (40/55/4) ASTM A563 GRADE 40/60 (40, K/55) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



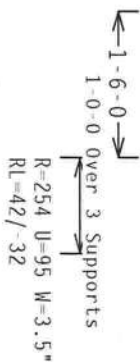
TC LL	20.0 PSF	REF R8228 - 92738
TC DL	10.0 PSF	DATE 03/17/10
BC DL	10.0 PSF	DRW HCUR8228 10076019
BC LL	0.0 PSF	HC-ENG DLJ/DLJ *
TOT. LD.	40.0 PSF	SEQN- 82776
DUR. FAC.	1.25	
SPACING	24.0"	JREF - 1U068228Z03

THIS WAS PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY IKUSZ MFK.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, closed bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

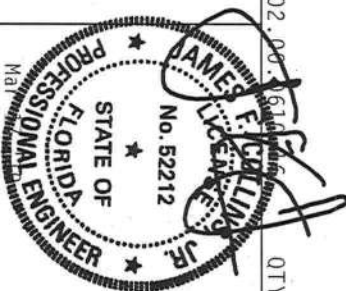


Scale = .5" / Ft.



ALPINE

Haines City, FL 33844
FL COA #0 278



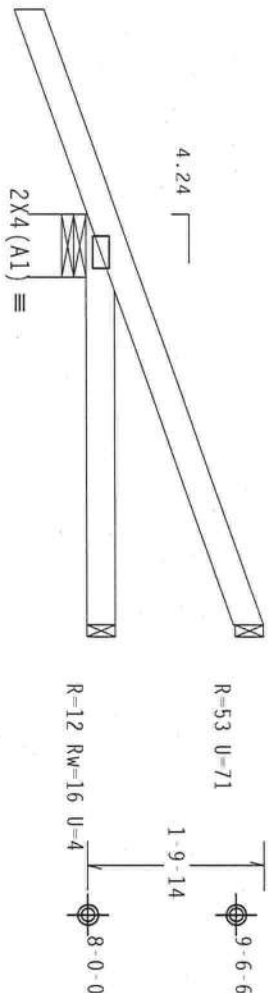
TC LL	20.0 PSF	REF	R8228- 92739
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076020
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	40.0 PSF	SEQN-	82773
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1U068228Z03

THIS WAS PREPARED FROM COMPUTER INPUT (LOANS & DIFFERENTIALS) SUBMITTED BY INDIA FIRM.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures

Provide (2) 16d common nails (0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord.



2-1-7

← 4-2-15 Over 3 Supports →
R=218 U=240 W=7.778"

PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

"MARINER"™ JOISTS REQUIRE EXPERT CARE IN FABRICATION, HANDLING, UNLOADING, INSTALLING AND BRACING. REFER TO BEST QUALITY CONTROL COMPONENT SPECIFICATIONS FOR INFORMATION. CONSULT WITH THE LATEST PAPER PUBLISHED BY THE STEEL DEPARTMENT OF AMERICA, 6700 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314 AND PRICE BOOKS AVAILABLE FROM THE STEEL DEPARTMENT OF AMERICA, 6700 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314 FOR SAFETY PRACTICES PERTAINING TO PERFORMING THESE FUNCTIONS. UNDESIRABLE CONDITIONS INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group Inc.

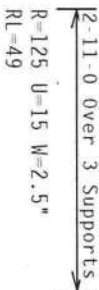
Haines City, FL 33844

TC LL	20.0 PSF	REF	R8228- 92740
TC DL	10.0 PSF	DATE	03/17/10
BC DL	10.0 PSF	DRW	HCUSR8228 10076012
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	40.0 PSF	SEQN-	82793
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	IU068228Z03

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

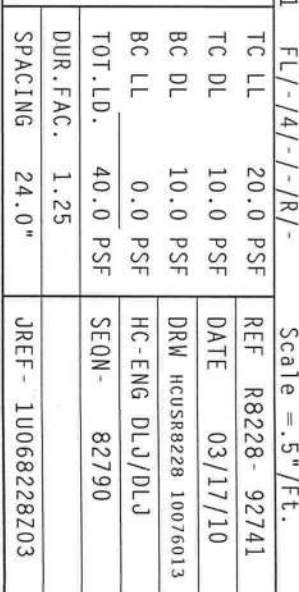
Provide (2)	16d common nails (0.162"x3.5")	toe nailed at Top chord
Provide (2)	16d common nails (0.162"x3.5")	toe nailed at Bot chord



Scale = .5"/ft.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, THE BCG, INC. SMALL MUST BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TUBS IN COMPLIANCE WITH THE TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TUBSSES, DESIGN CONFLICTING WITH APPLICABLE PROVISIONS OF NIOS (NATIONAL DESIGN SPEC., FOR ARPA) AND TPI-1, THE RECORD DRAWINGS, SHALL BE AT THE USER'S RISK. THE USER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL HEALTH DEPARTMENT. THE USER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL HEALTH DEPARTMENT. THE USER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL HEALTH DEPARTMENT. THE USER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL HEALTH DEPARTMENT.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0
psf. 1w=1.00 GCpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

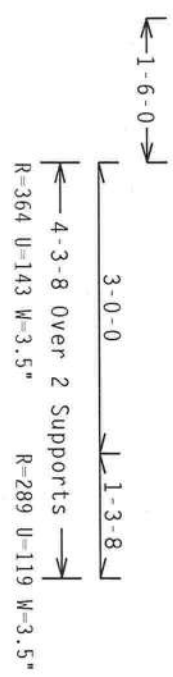
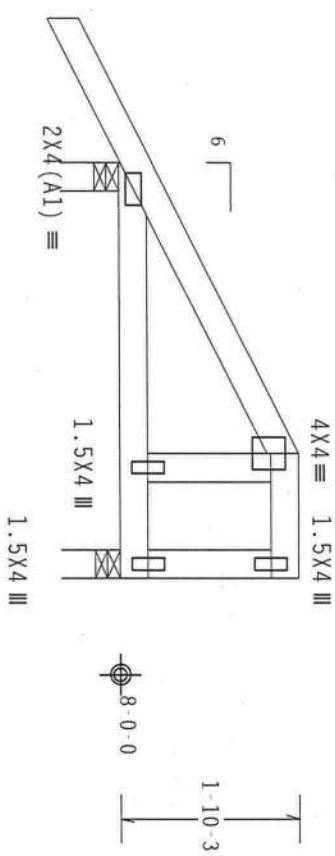
Deflection meets L/240 live and L/180 total load.

Special loads

TC - From	Dur.Fac.=1.25 / Plate Dur.Fac.=1.25	62 pif at -1.50 to 62 pif at 3.00
TC - From	62 pif at -1.50 to 62 pif at 3.00	62 pif at 3.00 to 62 pif at 4.29
BC - From	4 pif at -1.50 to 4 pif at 0.00	4 pif at 0.00 to 20 pif at 4.29
BC - From	20 pif at 0.00 to 20 pif at 4.29	
TC - 133 lb Conc. Load at 3.06		
BC - 66 lb Conc. Load at 3.06		

Roof overhang supports 2.00 psf soffit load.

#1 hip supports 3-0-0 jacks with no webs.



PLT TYP. Wave

Design Crit: FBC2007Res/TP1-2002 (STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY: 1

Scale = .5" / Ft.

ALPINE

ITW Building Components Group Inc.

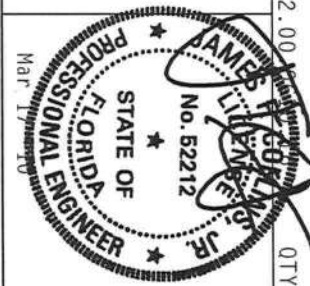
Haines City, FL 33844

FL COA #0278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2100 BORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND TPI'S (TRUSS PLATE INSTITUTE, 2100 BORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) WEBSITE (WWW.TPI-TRUSSES.COM) FOR MORE INFORMATION. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PN) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2017/1664 (U/55/K) ASTM A653 GRADE 40/60 (U, K/H, 55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



FL / - / 4 / - / - / R / -	REF	R8228- 92742
TC LL	DATE	03/17/10
TC DL	DRW	HCUSR8228 10076011
BC DL	HC-ENG	DLJ/DLJ
BC LL	SEON-	82796
TOT.LD.		
DUR.FAC.		
SPACING		

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLIP SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE.
FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE
BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE T OR L-BRACE	BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

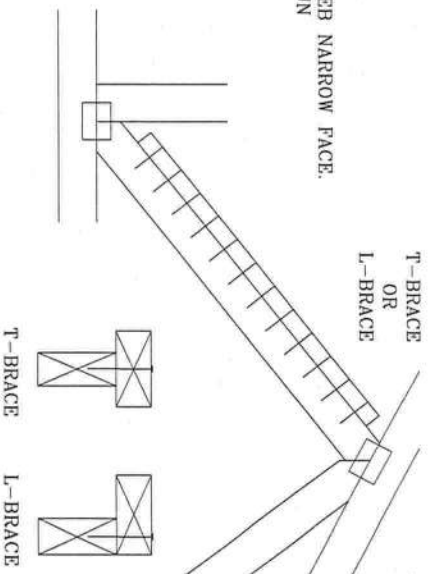


Building Components Group Inc.

Earth City, MO 63045

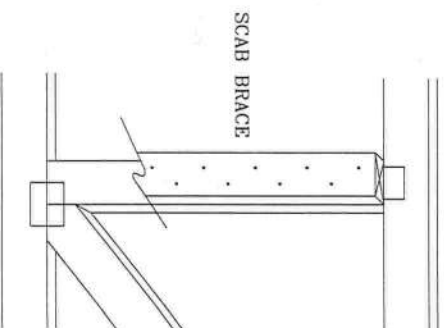
T-BRACING
OR
L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.126" x 3." MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3" MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
30% OF WEB MEMBER LENGTH



****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information) by TPI and WTA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached bracing member. Refer to the information on the back of this sheet for details on bracing and installation of the trusses. Sections B3 & B7. See this job's general notes page for more information.

****IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.**
The Building Components Group Inc. (BCEC) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with TPI or fabricating, handling, shipping, installing & bracing of trusses. Truss connector plates are made of 20/90/60CA (WTA's/60) ASTM A656 grade 51/40/60 (WTA's/60) girth plates to indicate application of the truss design. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/P1 Sec. 2.

JAMES F. COLLINS, JR.
LICENSE
No. 52212

	REF	CLB SUBST.
TC LL	PSF	
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCE THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

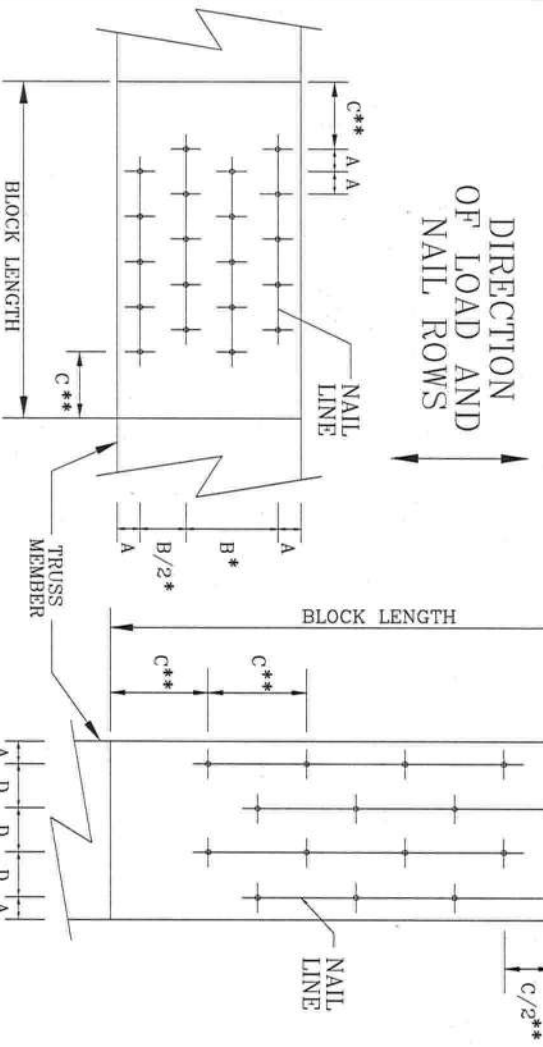
A - EDGE DISTANCE (6 NAIL DIAMETERS)

C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

- * SPACING MAY BE REDUCED BY 50%
- ** SPACING MAY BE REDUCED BY 33%



MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113" X 2.5", MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128" X 3", MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128" X 3.25", MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135" X 3.5", MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148" X 4", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131" X 2.5", MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148" X 3", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148" X 3.25", MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162" X 3.5", MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120" X 2.5", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131" X 2.5", MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120" X 3", MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131" X 3", MIN)	7/8"	1 5/8"	2"	1"

LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN



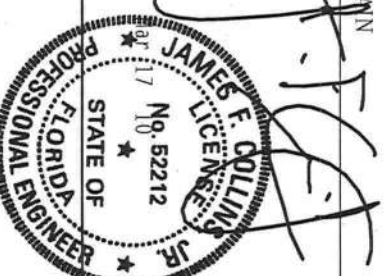
Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the BCSI (Building Component Safety Information, by TPI and WPCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord and ceiling have provisions for permanent lateral restraint of webs shall have bracing installed per BCSI sections B1 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing, or bracing (K/R/H/S) gable steel. Apply plates to each face of truss positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1, Sec. 2.

ITW - BCG: www.itwbcg.com; TPI: www.tpi.com; WPCA: www.abcdindustry.com; ICC: www.iccsafe.org



REF	NAIL SPACE
DATE	1/1/09
DRWG	CNNAILSP0109