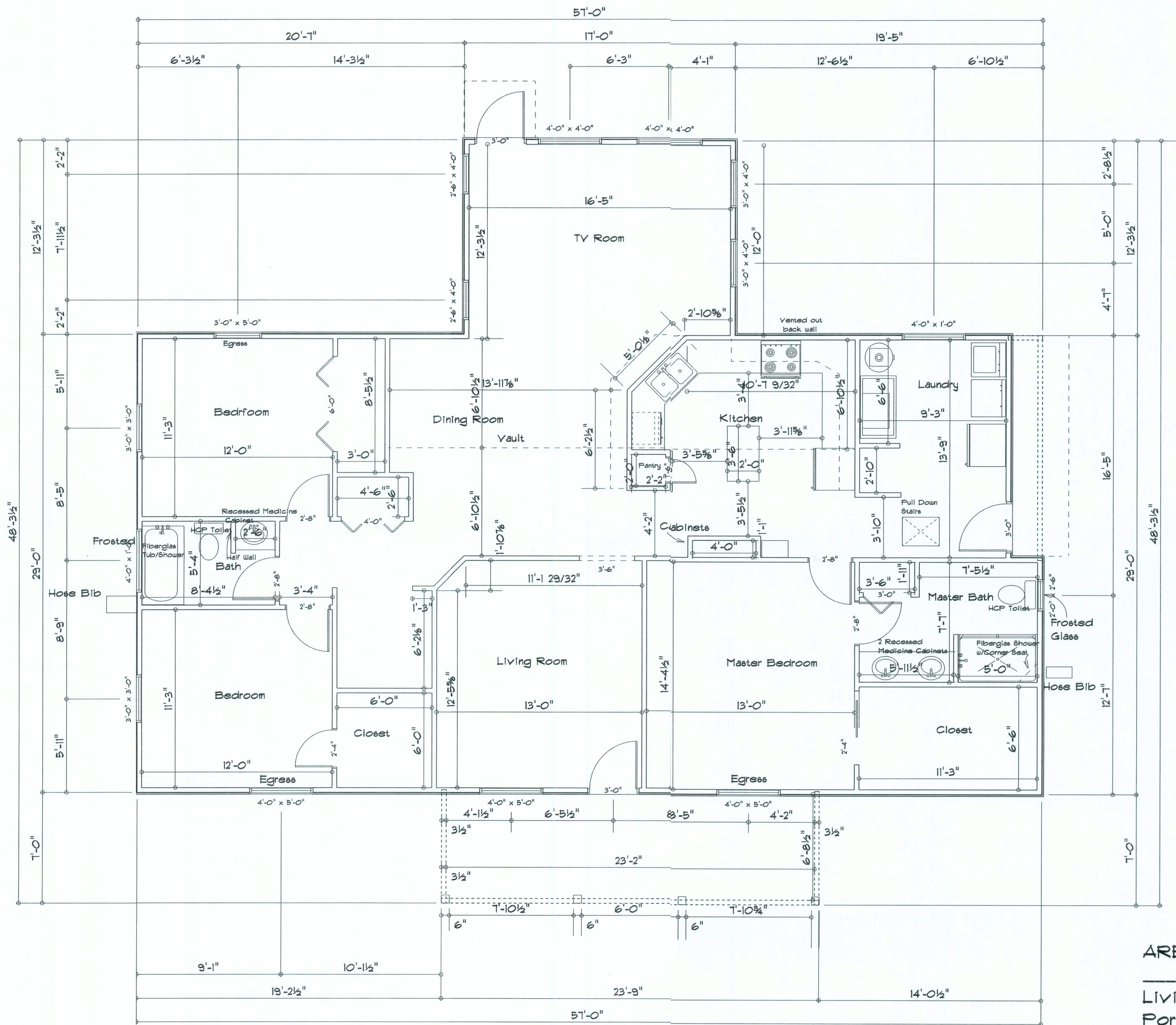


REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



Note: Plumbing in the Attic
Grilles in Front Windows Only



AREA SUMMERY

Living Area	1834	S.F.
Porch Area	167	S.F.
Total Area	2001	S.F.

Floor Plan
1/4" = 1'

RESIDENCE

Mixon
1557 Noegel Rd.
Lake City, FL 32055

ADDRESS:
Columbia County, Florida

Woodman Park Builders, Inc.

Lake City, Florida

Phone: (386) 755 - 2411

Fax: (386) 755-8684

Email:

PRINTED DATE:

DRAWN BY: CHECKED BY:

DESIGNED BY:
Mark Haddox

FINALS DATE:

JOB NUMBER:

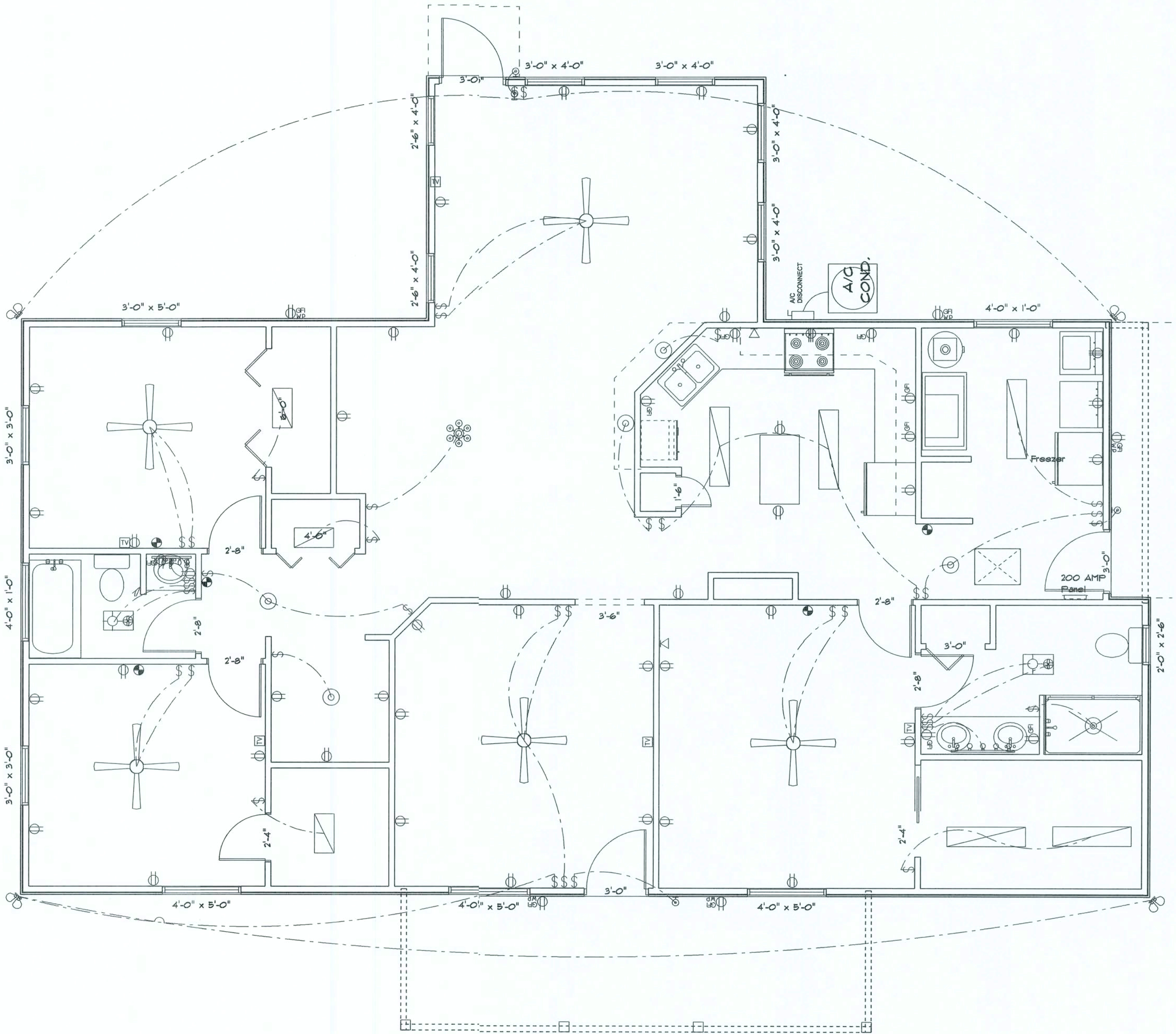
DRAWING NUMBER
A-1

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

Electrical Plan Notes:

- E-1 Wire all appliances, HVAC units and other equipment per manufactures specifications.
- E-2 Consult the owner for the number or separate telephone lines to be installed. Owner is responsible for all overages not noted on plan.
- E-3 All installations shall be per national code 2008.
- E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
- E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.
- E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.
- E-7 Entry of service (underground or overhead) to be determined by contractor agreement.
- E-8 All outlets located in residential to be tamper-resistant per NEC.
- E-9 All outlets to be located above base flood elevation.
- E-10 All exterior GFI outlets shall be weatherproof.
- E-11 Overcurrent Protection devices shall be installed on the exterior of structures on the load side of the meter to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.
- E-12 All 120-VOLT, single phase, 15 and 20 ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sun rooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed arc-fault circuit interrupter, combination-type installed to provide protection of the branch circuit.
- E-13 Carbon Monoxide alarms shall be required within 10' of all rooms for sleeping purposes in buildings having a fossil-fuel burning heater or appliance, a fireplace or attached garage.



Electrical Plan

ELECTRICAL	SYMBOL
ceiling fan globe 1	
ceiling globe light	
chandelier	
double spotlight	
fluorescent fixture	
vanity bar light	
wall sconce	
electrical panel	
AC Disconnect	
Outlet WP GFI	
cable tv outlet	
fan	
light	
outlet	
outlet 220v	
outlet gfi	
smoke detector	
switch	
telephone	

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FINALS DATE:

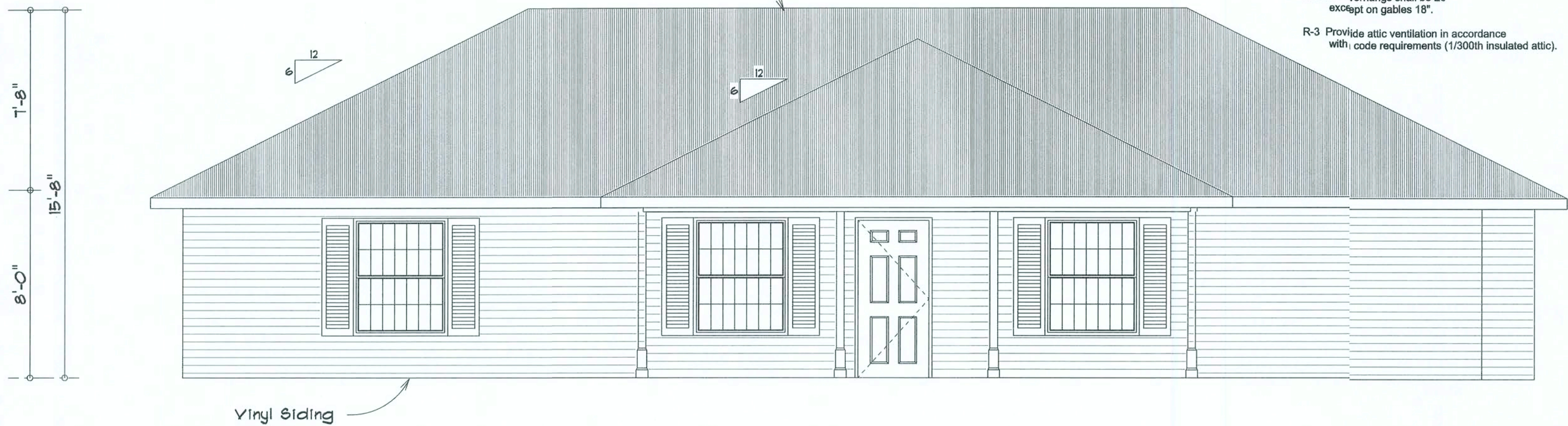
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A-2

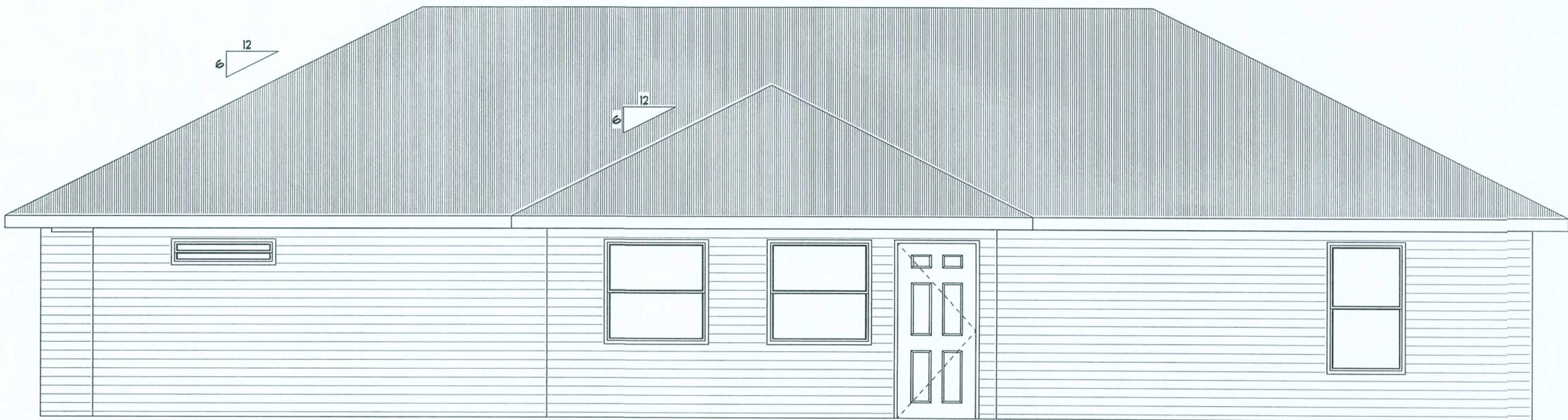
REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

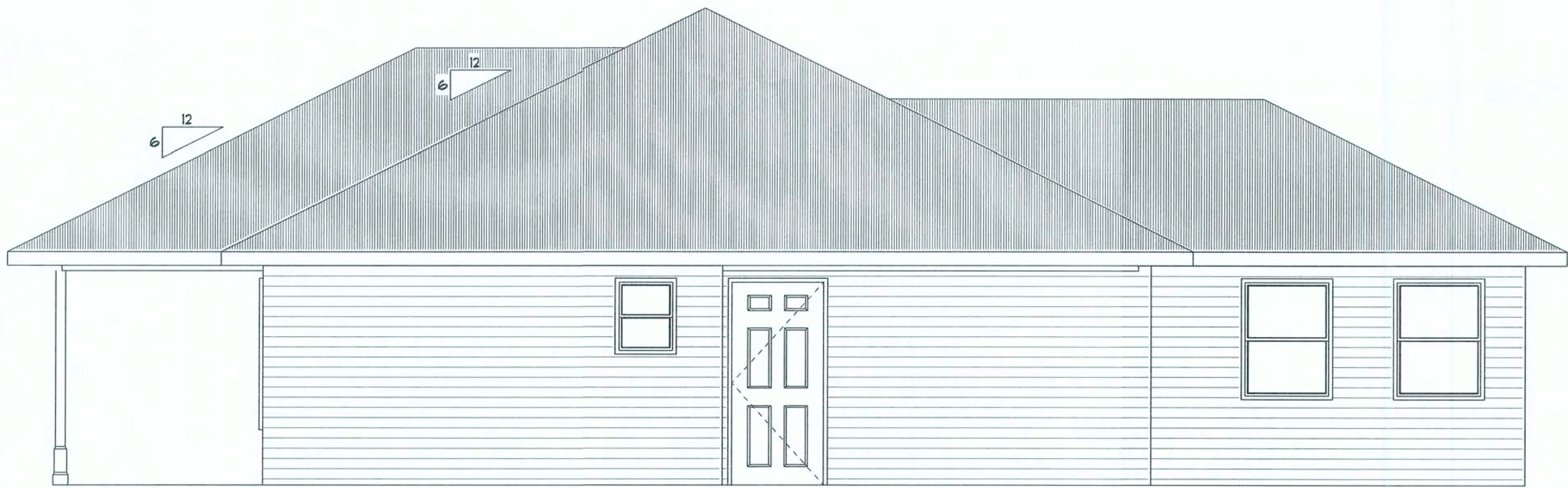
Notes:
R-1 All roof pitches shall be 6/12 unless otherwise noted.
R-2 All overhangs shall be 20" except on gables 18".
R-3 Provide attic ventilation in accordance with code requirements (1/300th insulated attic).



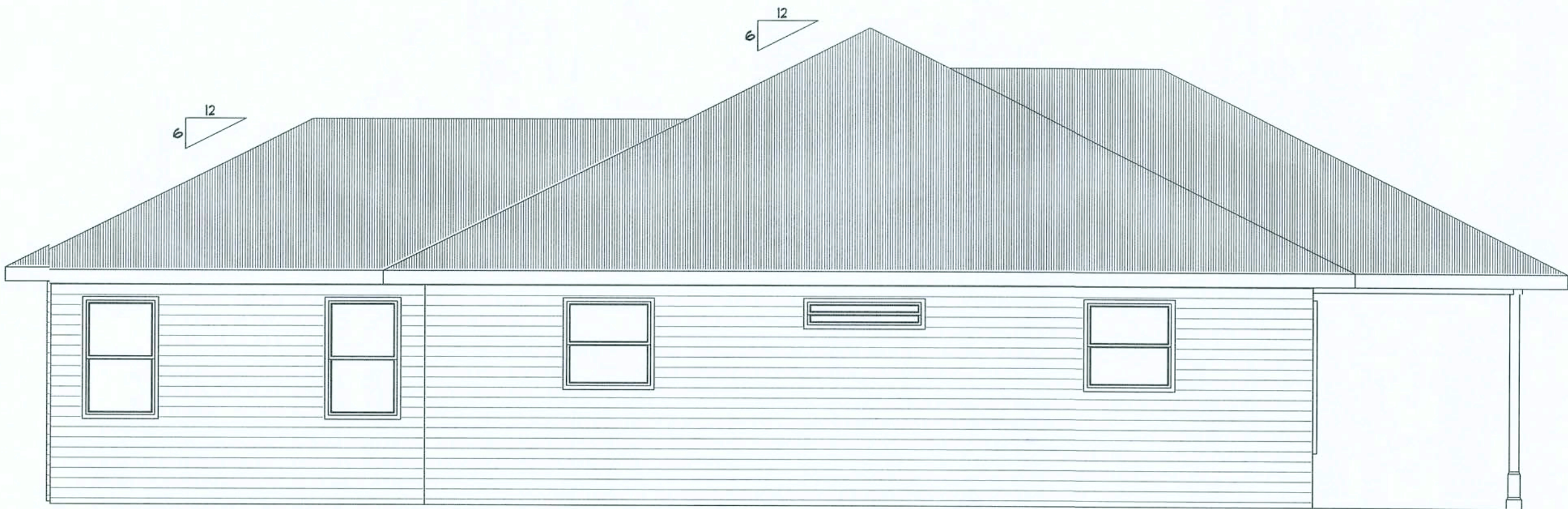
Front Elevation



Rear Elevation



Right Elevation



Left Elevation

RESIDENCE

Mixon
1557 Noegel Rd.
Lake City, FL 32055

ADDRESS:
Columbia County, Florida

Woodman Park Builders, Inc.

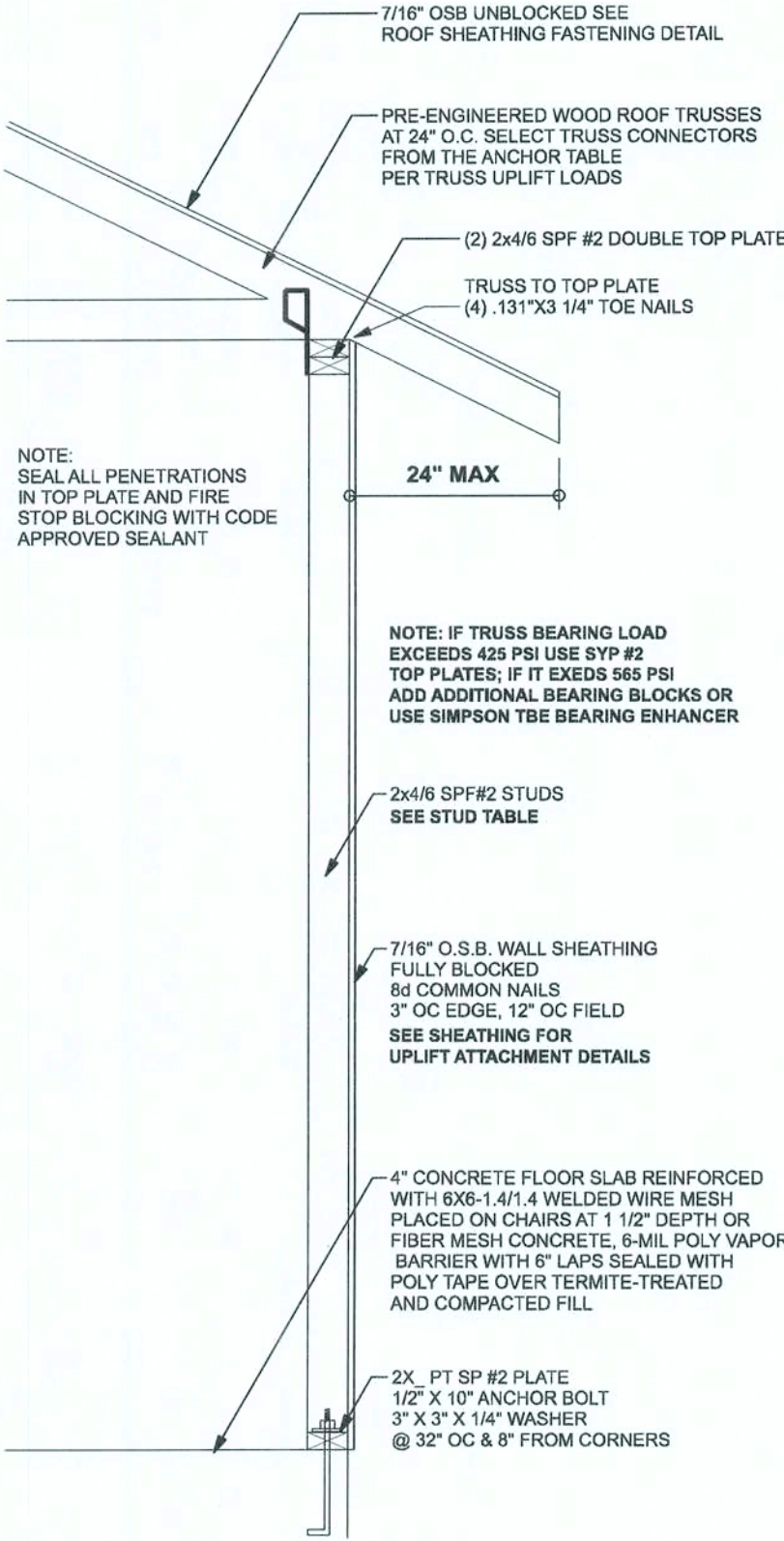
Lake City, Florida

Phone: (386) 755 - 2411

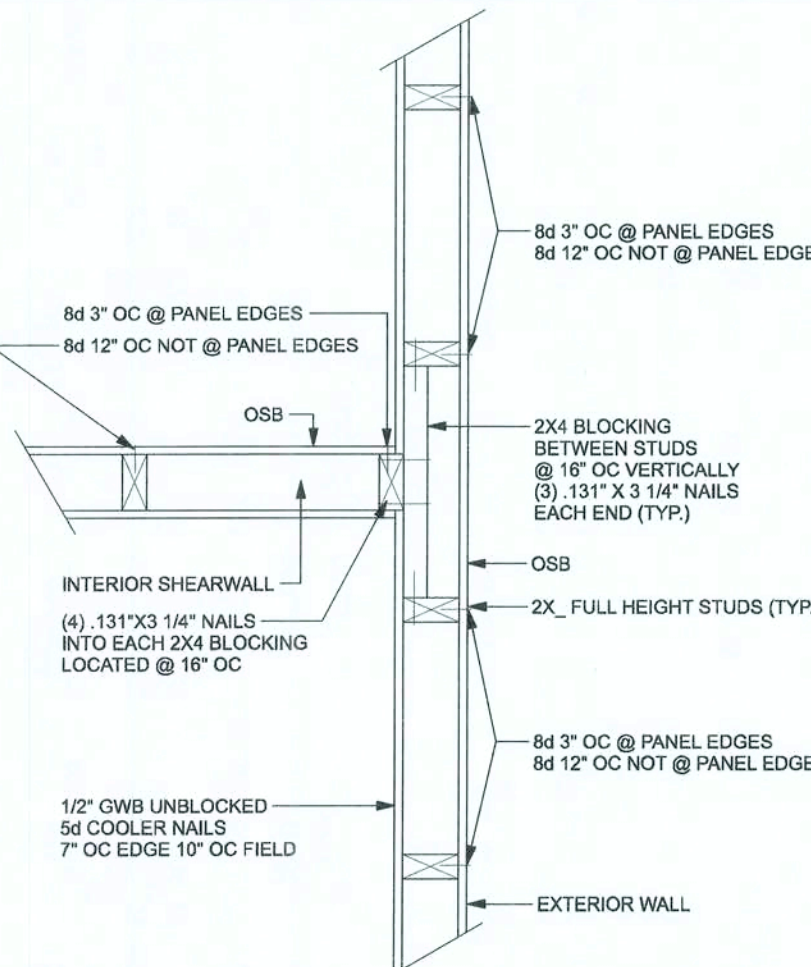
Fax: (386) 755-8684

Email:

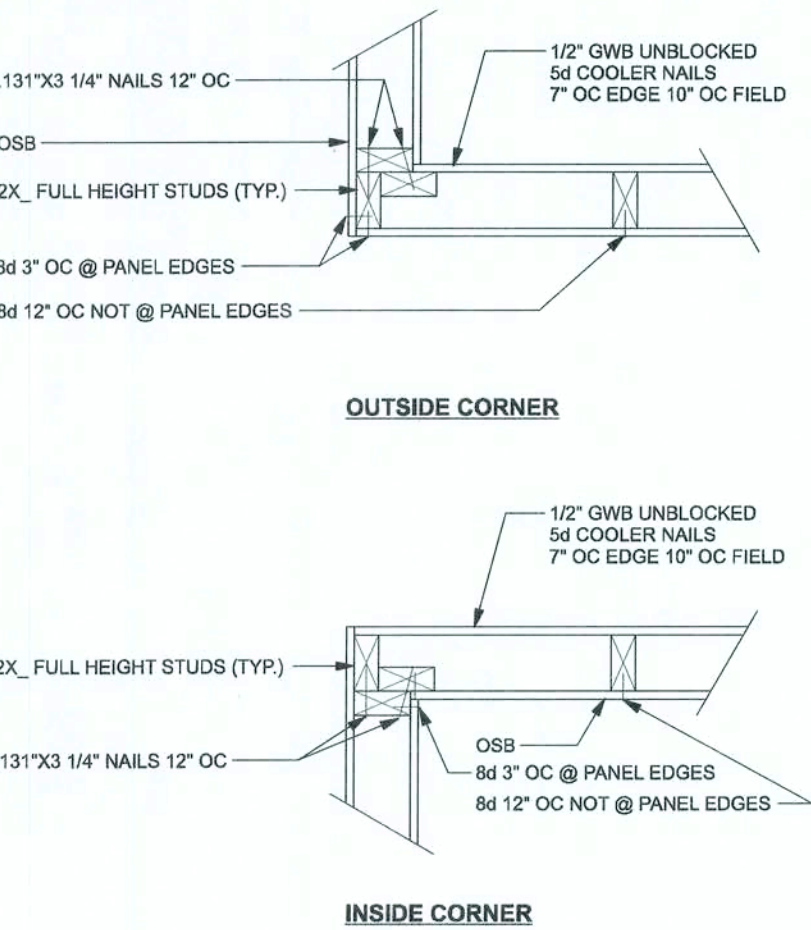
PRINTED DATE:	
DRAWN BY:	CHECKED BY:
DESIGNED BY:	Mark Haddox
FINALS DATE:	
JOB NUMBER:	
DRAWING NUMBER	A-3



ONE STORY WALL SECTION
SCALE: 3/4\"/>



(TYP.) INTERSECTING WALL FRAMING WOOD FRAME

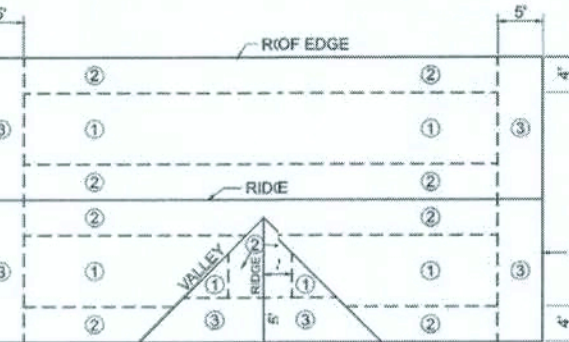


(TYP.) CORNER FRAMING WOOD FRAME

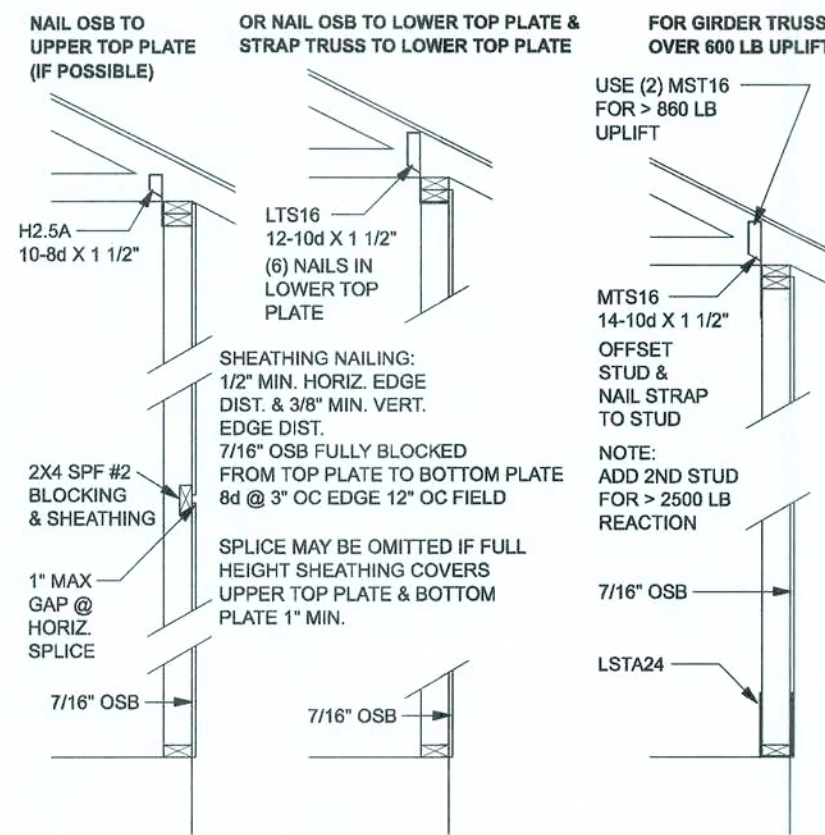
- RING-SHANK NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS:
- 0.113 INCH NOMINAL ROOFSHANK DIAMETER
 - RING DIAMETER OF 0.010 OVER SHANK DIAMETER
 - 16 TO 20 RINGS PER INCH
 - 0.280 INCH FULL ROUND HEAD DIAMETER
 - 2-3/8 INCH NAIL LENGTH

NAILING PATTERN SHALL BE

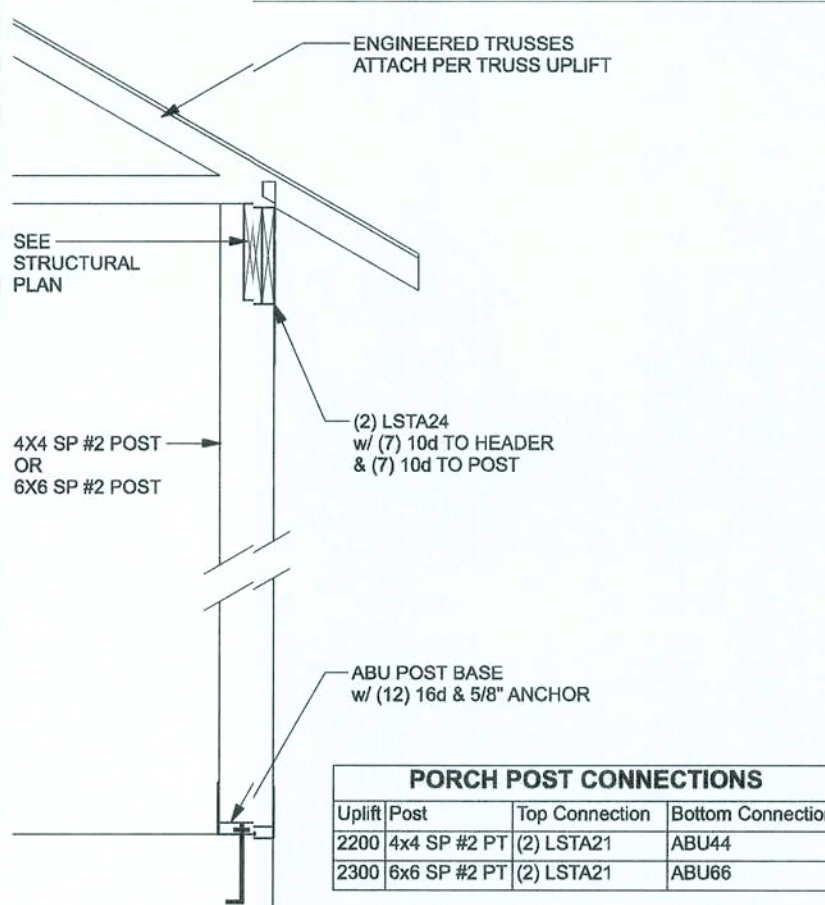
- 4\"/>



ROOF SHEATHING FASTENING



SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME

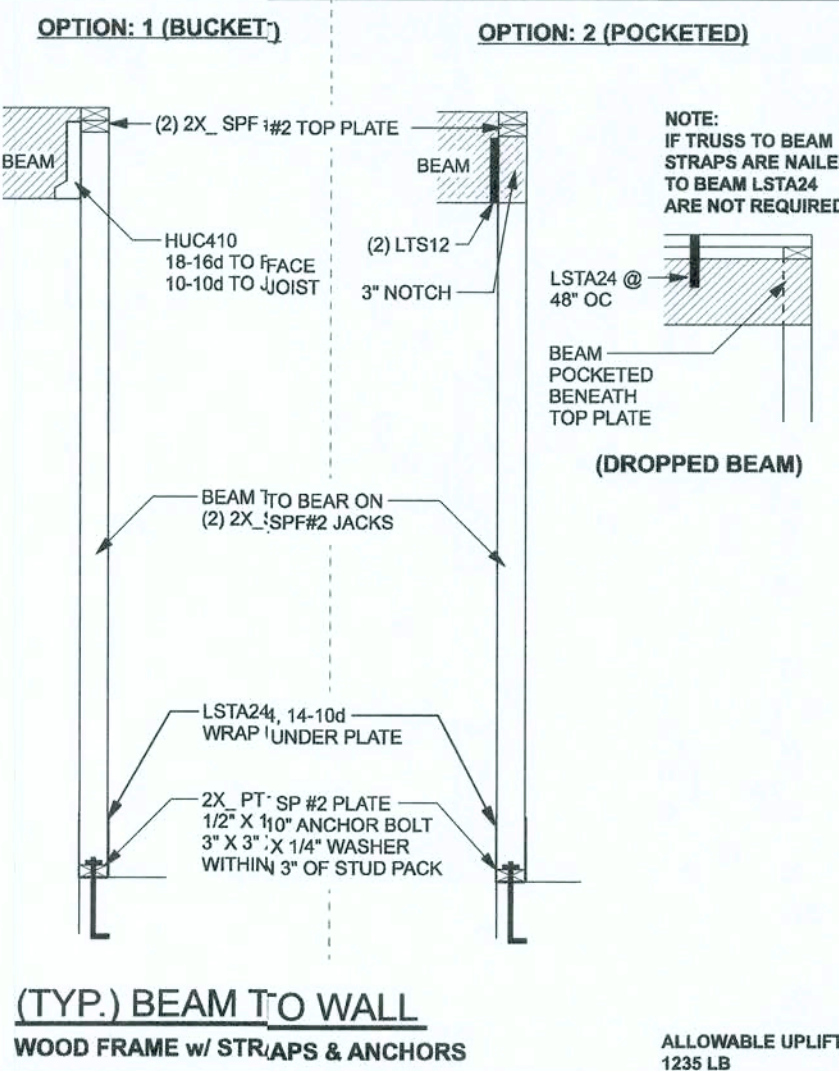


(TYP.) PORCH POST ONE STORY WOOD

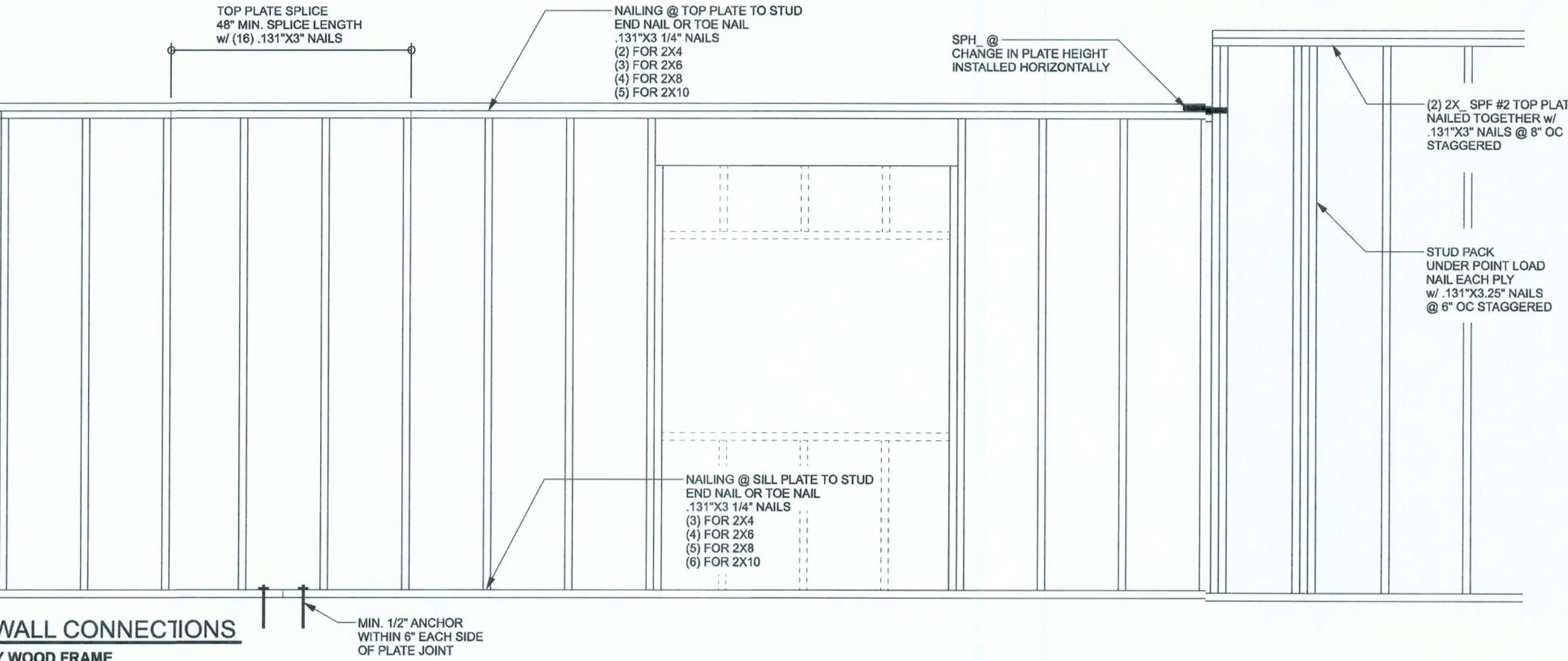
HEADER STRAP TABLE			
Uplift Post	Top Connection	Bottom Connection	
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate	1/2\"/>
< 1455	MSTA24, 18-10d header to jacks	DTT22	1/2\"/>
< 1800	(2) MST24, 18-10d header to jacks	DTT22	1/2\"/>
< 2910	(2) MST24, 18-10d header to jacks	HTT4	1/2\"/>

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.238
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

TYPICAL HEADER STRAPING DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM T-O WALL WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME

CONNECTOR TABLE			
Uplift SP	Uplift SPF	Truss Connector	To Plate
615	485	SDWC15600	-
415	290	H3	4-8d@1'1/2"
575	495	H2.5A	5-8d@1'1/2"
1340	1015	H10A	9-10d@1'1/2"
720	620	LTS12-20	6-10d@1'1/2"
1000	660	MTS12-30	7-10d@1'1/2"
1450	1245	HTS20-30	12-10d@1'1/2"
Uplift SP Uplift SPF Strap Ties			
Uplift SP	Uplift SPF	To One Member	To Other Member
1235	1235	LSTA21	8-10d
1640	1455	MSTA24	9-10d
1030	1030	CS20	7-10d
Uplift SP Uplift SPF Stud Plate Ties			
Uplift SP	Uplift SPF	To Stud	To Plate
565	535	SP1	6-10d
1085	695	SP2	6-10d
771	771	LSTA24	10-10d
1235	1235	LSTA24	14-10d
Uplift SP Uplift SPF Holdowns @ Stemwall			
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post
1625	1880	DT122	8-SDS 1/4"x1'1/2"
4235	3640	HTT4	18-16d@2'1/2"
Uplift SP Uplift SPF Post Bases @ Stemwall			
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post
2200	2200	ABU44	12-16d
2300	2300	ABU66	12-16d

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2\"/>

GRADE & SPECIES TABLE	Fb	E
2x8 SP #2	925	1.4
2x10 SP #2	800	1.4
2x12 SP #2	750	1.4
GLB 24F-V3 SP	2600	1.9
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	2950	2.0
PSL PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCL. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f'_c = 2500$ PSI. WELDED WIRE REINFORCED SLAB: 6\"/>

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1119. SUPPLIER TO PROVIDE ASTM C 1119 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE CRACKS TO CRACK ON A GROUND LINE.)

REBAR, ASTM A 615, GRADE 40, DEFORMED BARS, FT = 40 KSI, ALL LAP SPLICES 4\"/>

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 1\"/>

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCL REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCL, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL, FOR CORRECT APPLICATION OF FBCL REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN CRITERIA & LOADS:	
BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES SEE TABLE	
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 > GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)		
EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd)	-27.8(Vasd)
0 - 20	+42.6(Vult)	-46.2(Vult)
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
9x7 GARAGE DOOR	+22.6(Vasd)	-25.5(Vasd)
18x7 GARAGE DOOR	+21.7(Vasd)	-24.1(Vasd)

WOODMAN PARK BUILDERS

Mixon Res.

PROJECT ADDRESS:
1557 Noegel Rd.
Lake City, FL 32805
Colman County

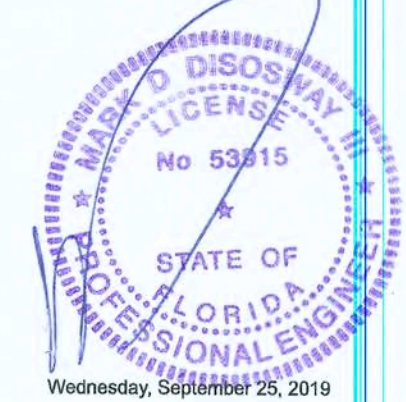
DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53945

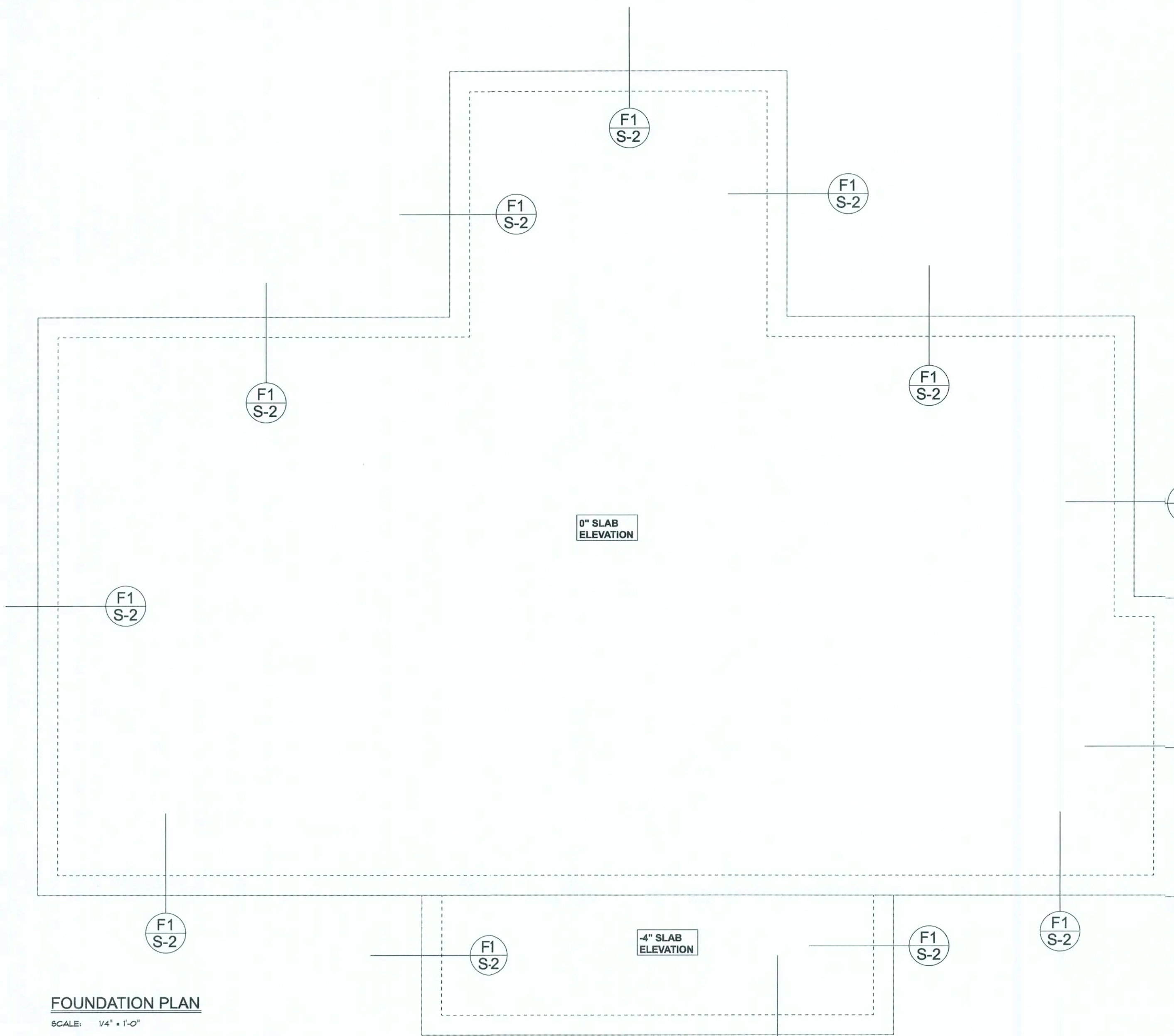


Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32805
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
191079

S-1

OF 2 SHEETS

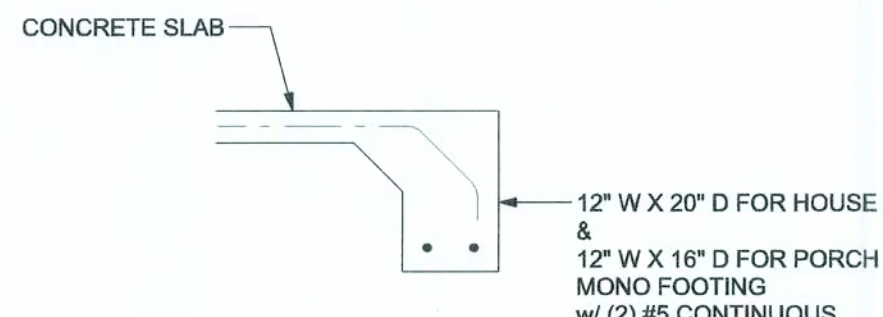


FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

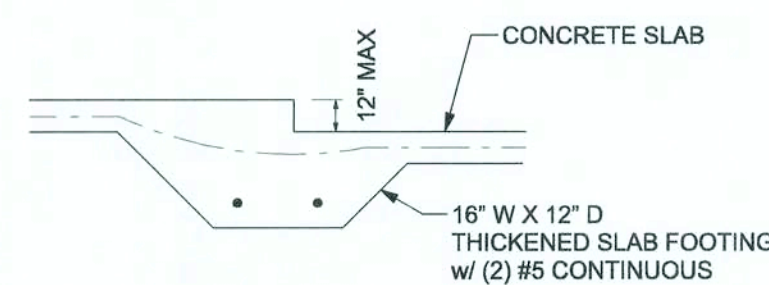
FOUNDATION NOTES

- FN-1: DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- FN-2: CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN-3: THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ 6X8-1 #14 WELDED WIRE MESH PLACED ON CHAIRS @ 12" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE-APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD).



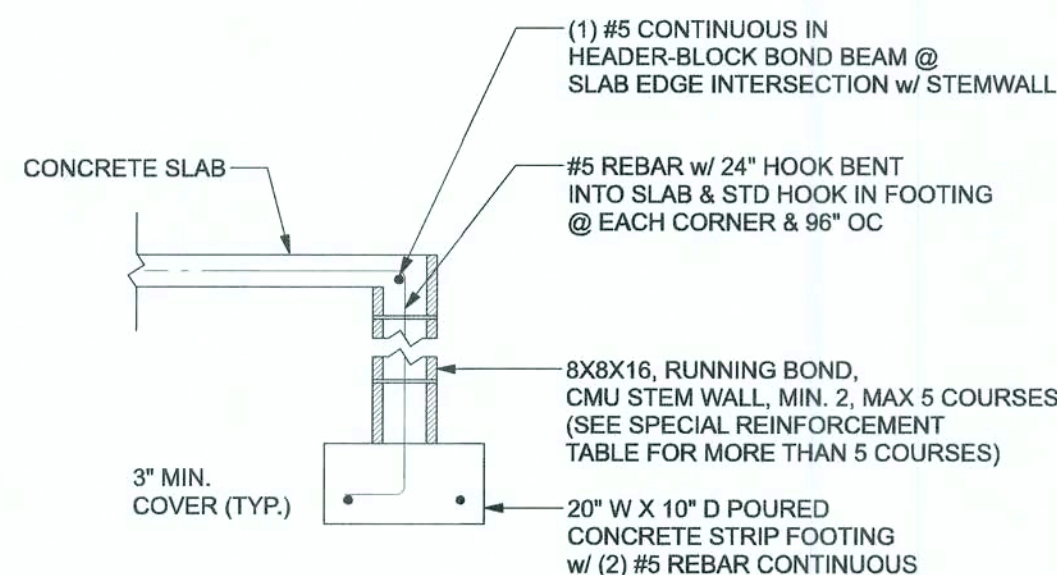
MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"



INTERIOR BEARING STEP FOOTING

SCALE: 1/2" = 1'-0"



OPTIONAL STEM WALL FOOTING

SCALE: 1/2" = 1'-0"

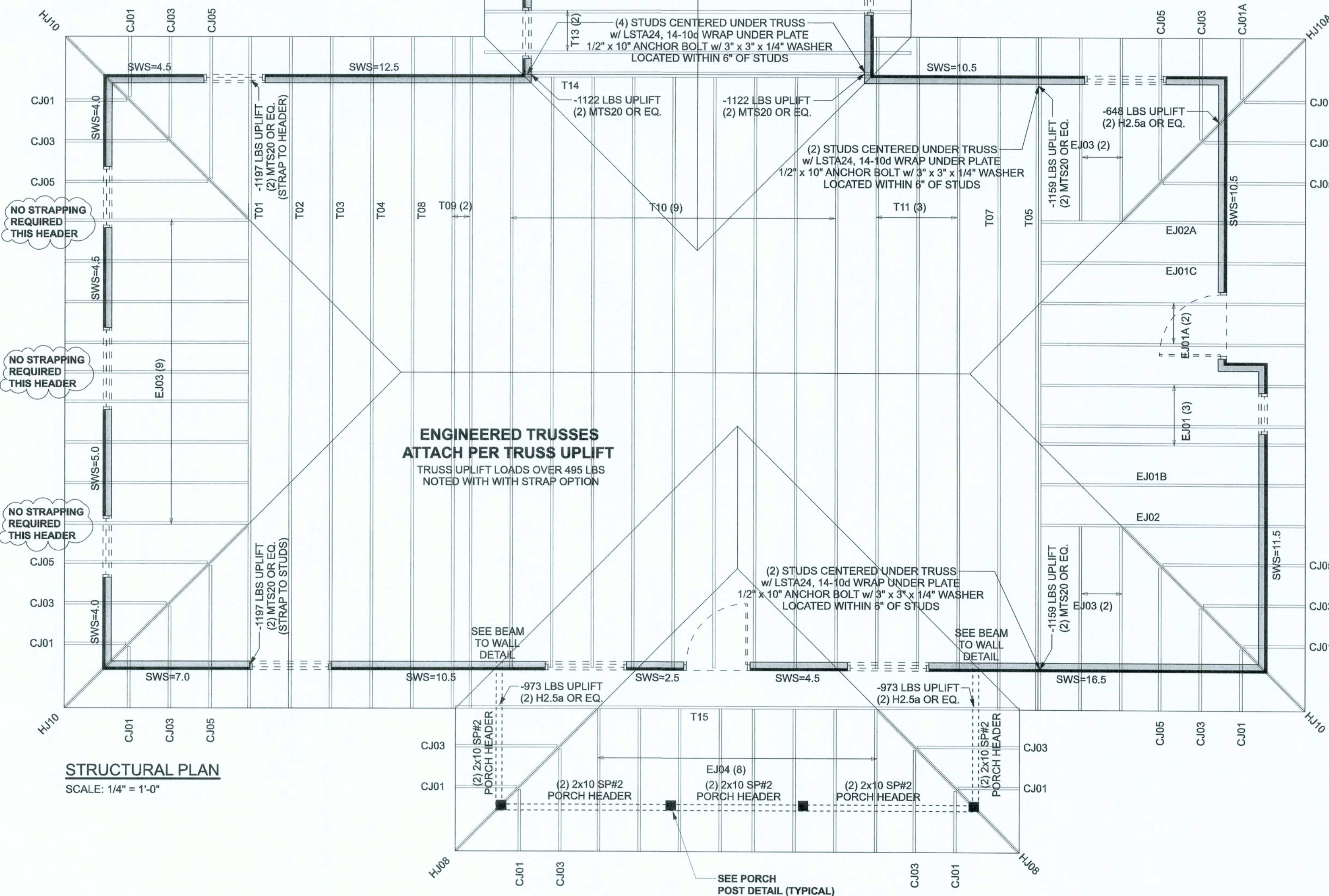
TALL STEM WALL TABLE:
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 18"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (20" for #8)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A305, Class 060, 0.60 oz/lb or 0.64SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

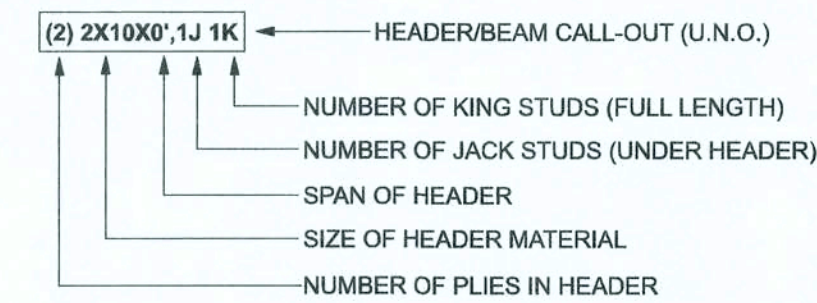
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2017-RES. SECTION R403.1.4



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	19037 LBF	27126 LBF
REQUIRED	16435 LBF	10863 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2084806

STRUCTURAL PLAN NOTES

- SN-1: ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2: ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3: ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4: USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5: DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6: PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



WOODMAN PARK BUILDERS

Mixon Res.

PROJECT ADDRESS:
1557 Noegel Rd.
Lake City, FL 32055
Columbia County

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53315



Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
191079

S-2

OF 2 SHEETS