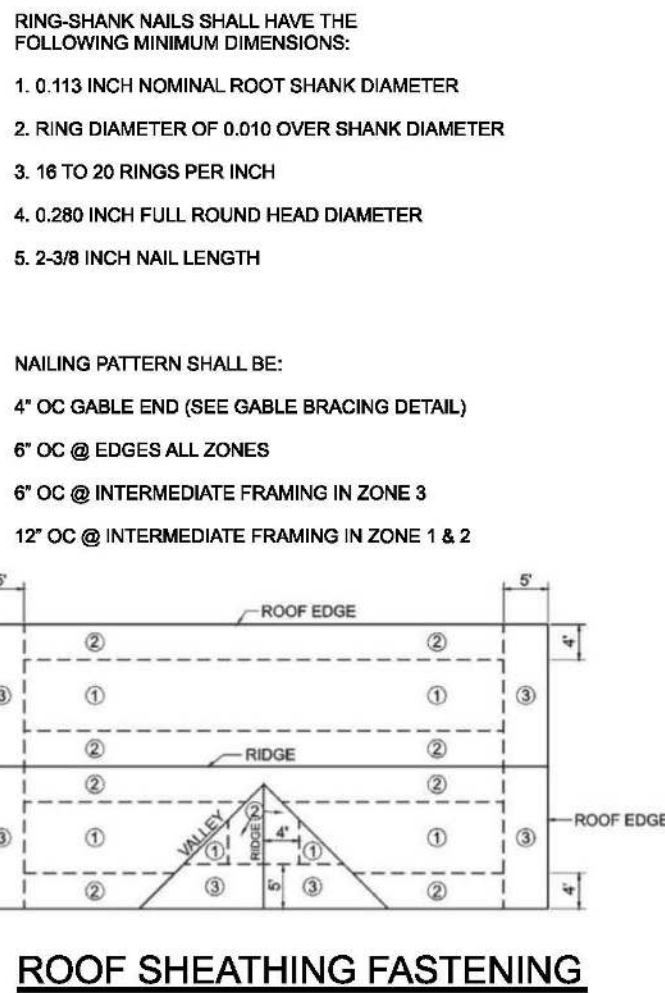
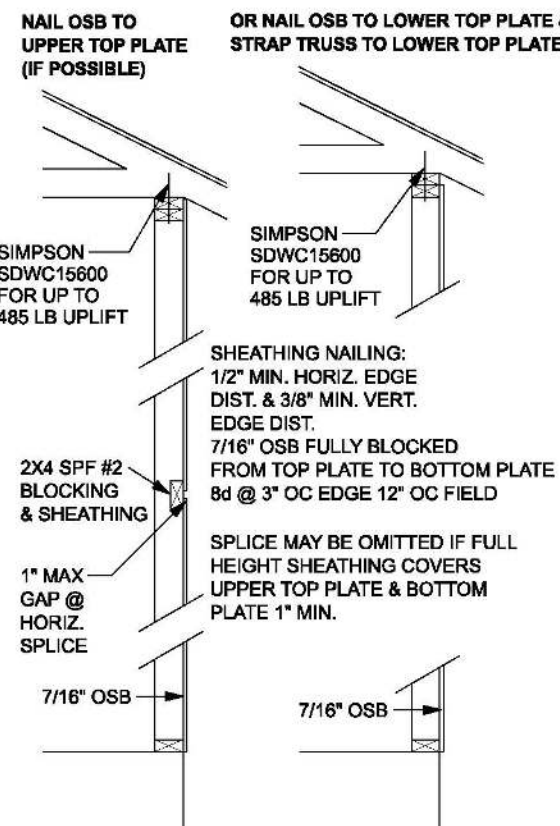


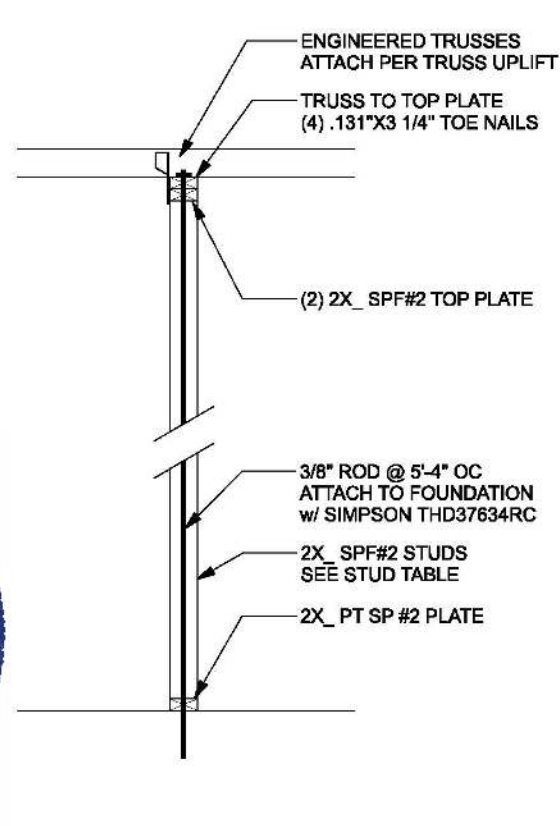
(TYP.) EXTERIOR WALL ONE STORY WOOD FRAME w/ RODS



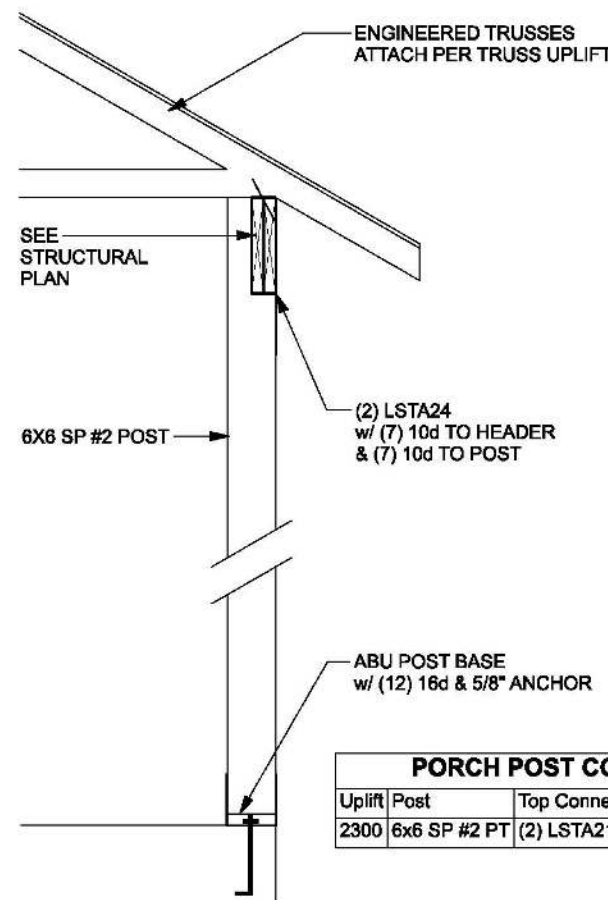
ROOF SHEATHING FASTENING



SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ RODS



(TYP.) PORCH POST ONE STORY WOOD

| CONNECTOR TABLE | | | | | |
|-----------------|------------|-----------------------|-------------------|--------------------------|--|
| Uplift SP | Uplift SPF | Truss Connector | To Plate | To Truss/Rafter | |
| 615 | 485 | SDWC15600 | | | |
| 415 | 280 | H3 | 4-8d x 1 1/2" | 4-6d x 1 1/2" | |
| 575 | 485 | H2.5A | 5-8d x 1 1/2" | 5-8d x 1 1/2" | |
| 1340 | 1015 | H10A | 9-10d1 1/2" | 9-10d1 1/2" | |
| 720 | 620 | LTS12-20 | 6-10d1 1/2" | 6-10d1 1/2" | |
| 1000 | 860 | MTS12-30 | 7-10d1 1/2" | 7-10d1 1/2" | |
| 1450 | 1245 | HTS20-30 | 12-10d1 1/2" | 12-10d1 1/2" | |
| Uplift SP | Uplift SPF | Stap Ties | To One Member | To Other Member | |
| 1235 | 1235 | LSTA1 | 8-10d | 8-10d | |
| 1640 | 1455 | MSTA24 | 9-10d | 9-10d | |
| 1030 | 1030 | CS20 | 7-10d | 7-10d | |
| Uplift SP | Uplift SPF | Stud Plate Ties | To Stud | To Plate | |
| 585 | 535 | SP1 | 6-10d | 6-10d | |
| 1085 | 865 | SP2 | 6-10d | 6-10d | |
| 771 | 771 | LSTA24 | 10-10d | 6-10d | |
| 1235 | 1235 | LSTA24 | 14-10d | wrap under or over plate | |
| Uplift SP | Uplift SPF | Holdowns @ Stenwall | To Stud / Post | Anchor | |
| 1825 | 1800 | DTT22 | 8-SDS 1/4"x1 1/2" | 1/2"x12" Titen HD | |
| 4235 | 3640 | HTT4 | 18-16d x 1/2" | 1/2"x12" Titen HD | |
| Uplift SP | Uplift SPF | Holdowns @ Mono | To Stud / Post | Anchor | |
| 1825 | 1800 | DTT22 | 8-SDS 1/4"x1 1/2" | 1/2"x8" Titen HD | |
| 4235 | 3640 | HTT4 | 18-16d x 1/2" | 1/2"x12" Titen HD | |
| Uplift SP | Uplift SPF | Post Bases @ Stenwall | To Post | Anchor | |
| 2200 | | ABU44 | 12-16d | 5/8"x12" Drill & Epoxy | |
| 2300 | | ABU66 | 12-16d | 5/8"x12" Drill & Epoxy | |
| Uplift SP | Uplift SPF | Post Bases @ Mono | To Post | Anchor | |
| 2200 | | ABU44 | 12-16d | 5/8"x7" Drill & Epoxy | |
| 2300 | | ABU66 | 12-16d | 5/8"x7" Drill & Epoxy | |

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

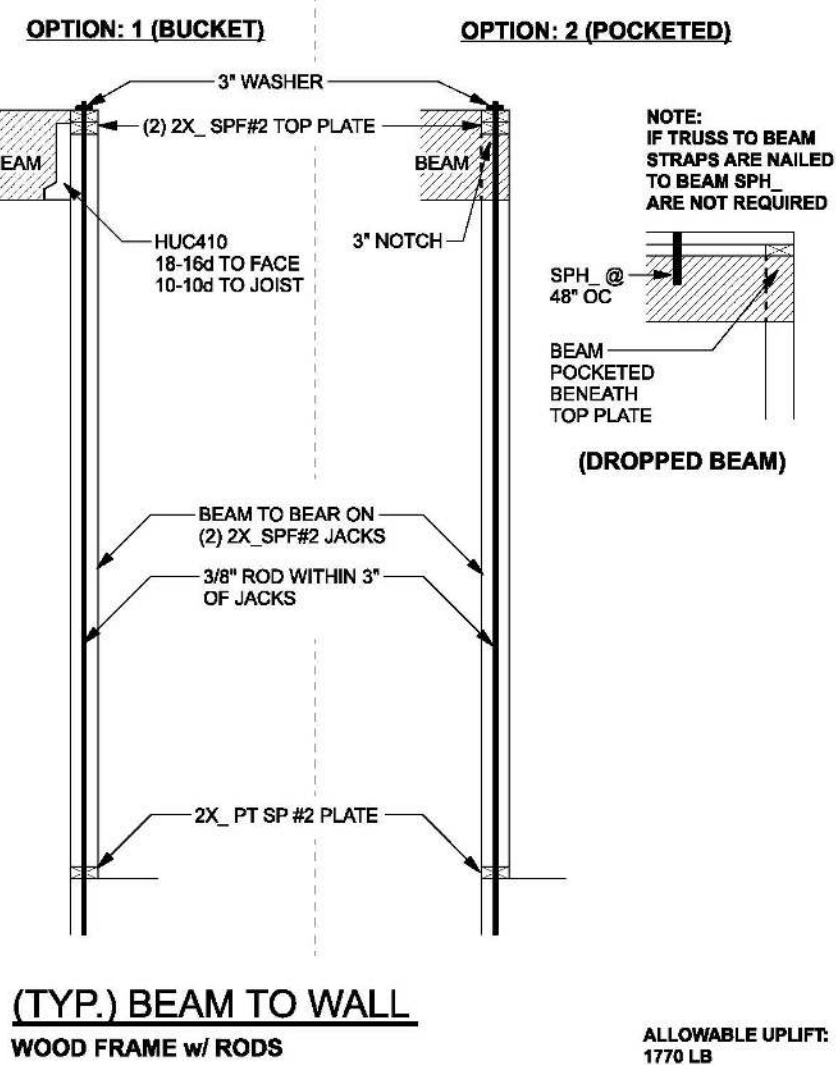
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

| | |
|------------------|-----------------------|
| (1) 2x4 @ 16" OC | TO 10'-1" STUD HEIGHT |
| (1) 2x4 @ 12" OC | TO 11'-2" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 15'-7" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 17'-3" STUD HEIGHT |

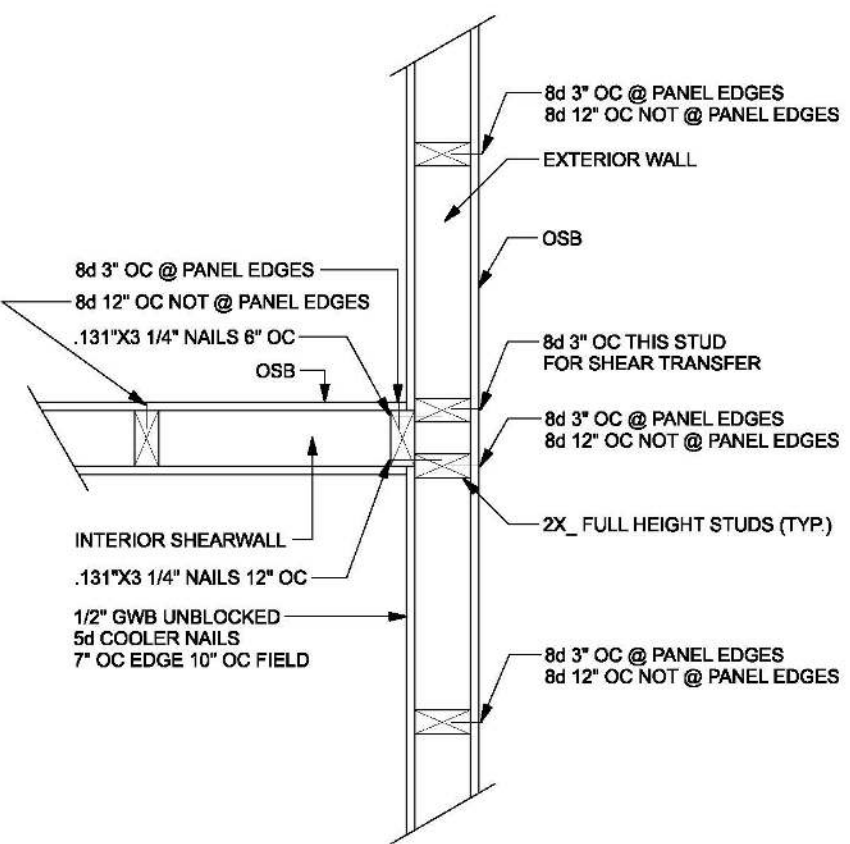
| | | Fb | E |
|------|--------------|------|-----|
| 2x8 | SP #2 | 925 | 1.4 |
| 2x10 | SP #2 | 800 | 1.4 |
| 2x12 | SP #2 | 750 | 1.4 |
| GLB | 24F-V3 SP | 2600 | 1.9 |
| LSL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MICROLAM | 2950 | 2.0 |
| PSL | PARALAM | 2900 | 2.0 |

| Uplift Post | Top Connection | Bottom Connection |
|-----------------------|----------------|-------------------|
| 2300 6x6 SP #2 PT (2) | LSTA21 | ABU66 |

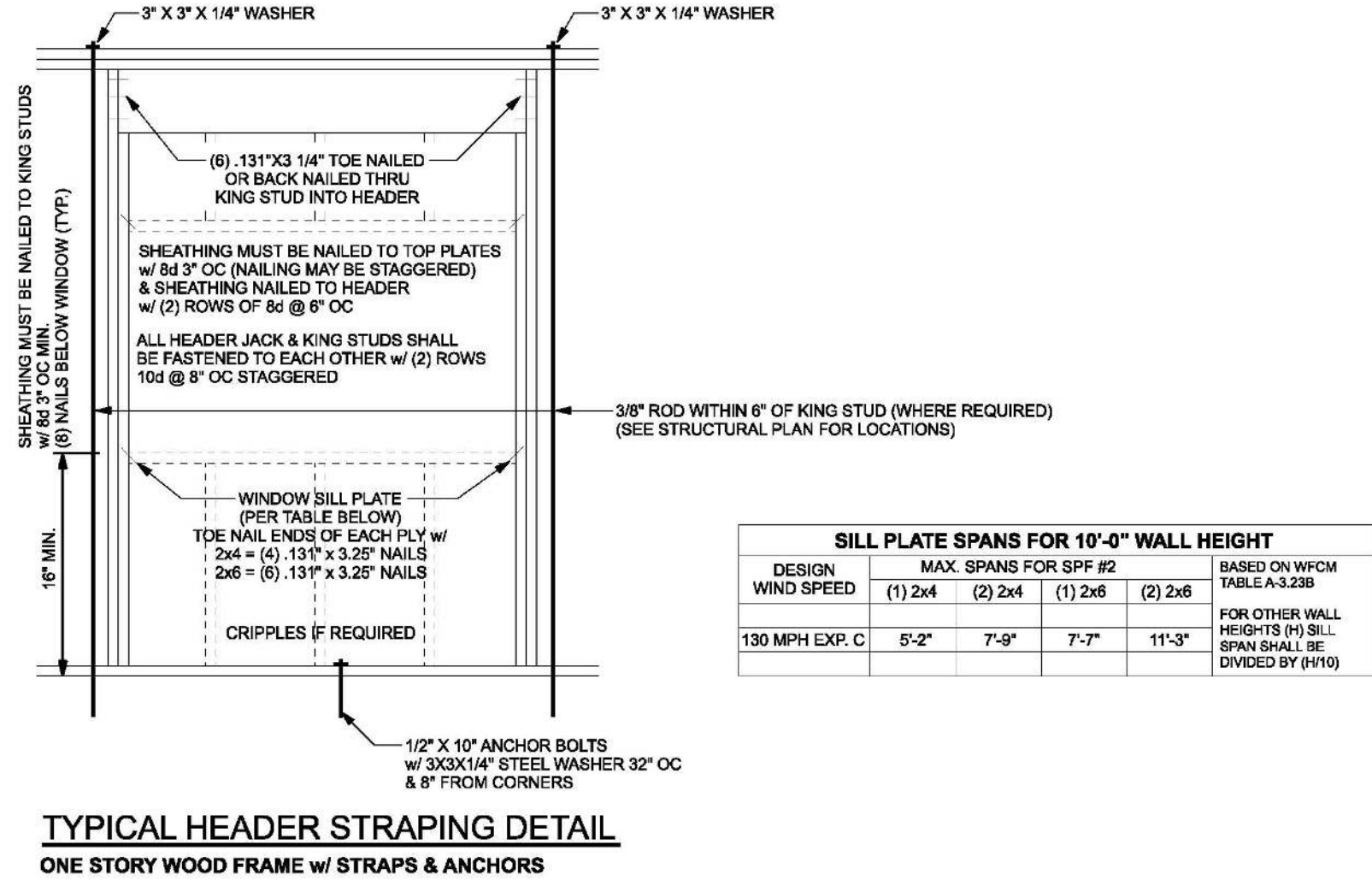
PORCH POST CONNECTIONS



(TYP.) BEAM TO WALL WOOD FRAME w/ RODS



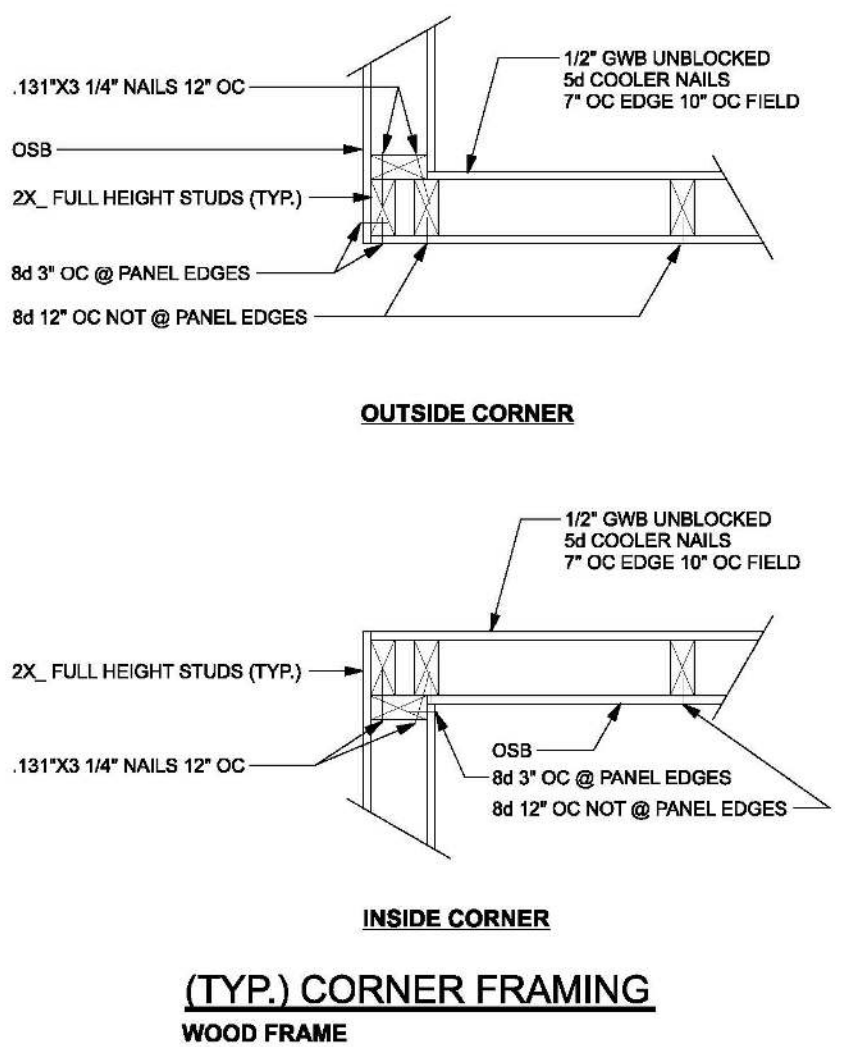
(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



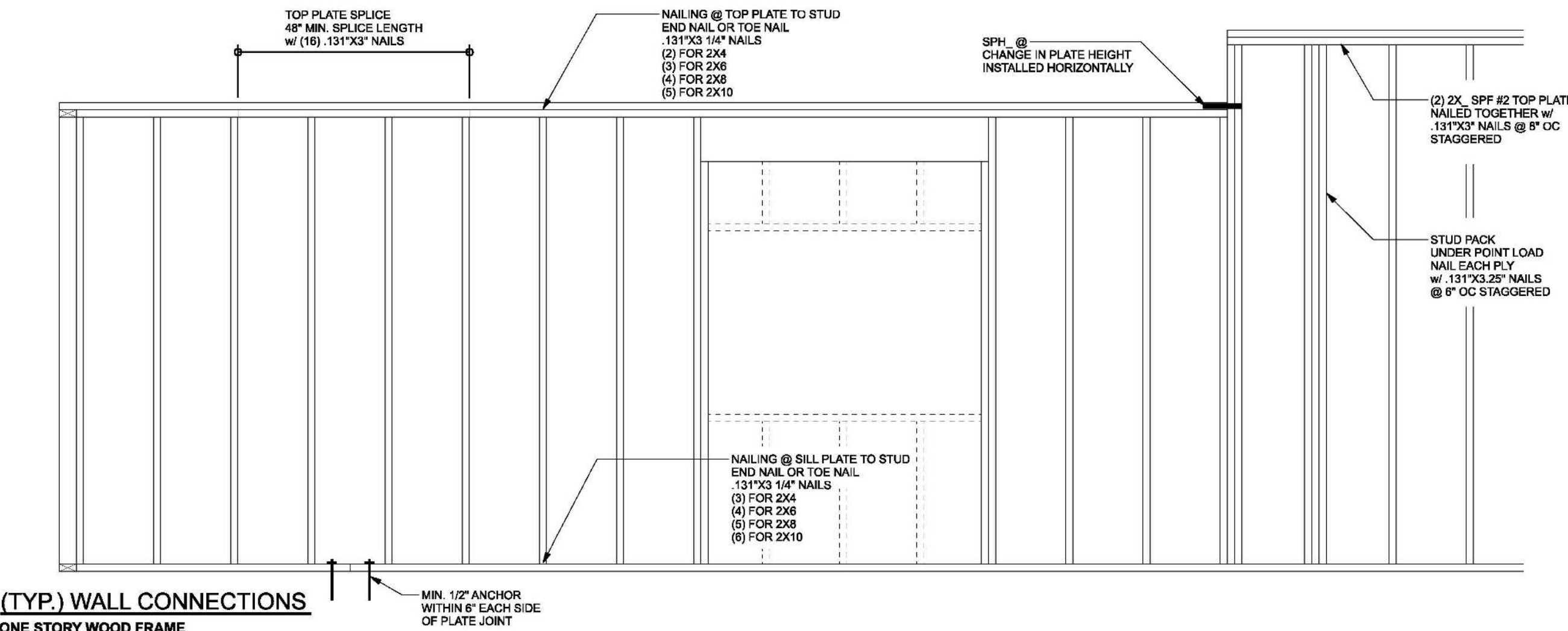
TYPICAL HEADER STRAPPING DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

| SILL PLATE SPANS FOR 10'-0" WALL HEIGHT | | | | |
|---|-----------------------|---------|---------|---------|
| DESIGN WIND SPEED | MAX. SPANS FOR SPF #2 | | | |
| | (1) 2x4 | (2) 2x4 | (1) 2x6 | (2) 2x6 |
| 130 MPH EXP. C | 5'-2" | 7'-9" | 7'-7" | 11'-3" |

BASED ON WFCM TABLE A-3.20B FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)



(TYP.) CORNER FRAMING WOOD FRAME



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F'_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 8" x 6" W1 x W1 4, FB = 88KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSEAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. LENGTH & WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPLICES 40" DB (25" FOR 45 BARS), UNO ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 315-RE, UNO.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE, NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCE REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCE, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCE REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN CRITERIA & LOADS:

| | |
|---|--|
| BUILDING CODE | 6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017) |
| CODE FOR DESIGN LOADS | ASCE 7-10 |
| WINDLOADS | |
| BASIC WIND SPEED (ASCE 7-10, 3S GUST) | 130 MPH |
| WIND EXPOSURE (BUILDER MUST FIELD VERIFY) | C |
| RISK CATEGORY | II |
| ENCLOSURE CLASSIFICATION | ENCLOSED |
| INTERNAL PRESSURE COEFFICIENT | 0.18 |
| ROOF ANGLE | 7-45 DEGREES |
| MEAN ROOF HEIGHT | 30 FT |
| C&C DESIGN PRESSURES (SEE TABLE) | |
| FLOOR LOADING | 40 PSF LIVE LOAD |
| ROOMS OTHER THAN SLEEPING ROOM | 30 PSF LIVE LOAD |
| SLEEPING ROOMS | 30 PSF LIVE LOAD |
| ROOF LOADING | 20 PSF LIVE LOAD |
| FLAT OR < 4:12 | 20 PSF LIVE LOAD |
| 4:12 TO < 12:12 | 16 PSF LIVE LOAD |
| 12:12 & GREATER | 12 PSF LIVE LOAD |
| SOIL BEARING CAPACITY | 1500 PSF |
| FLOOD ZONE | THIS BUILDING IS NOT IN THE FLOOD ZONE |

| COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) (Vuln) | | |
|--|-----------------|---------------------------------------|
| EFFECTIVE WIND AREA (FZ) | ZONE 4 INTERIOR | ZONE 5 END 4' FROM ALL OUTSIDE CORNER |
| 0 - 20 | +42.8 -46.2 | +42.8 -57 |
| GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD) | | |
| 8'x7 GARAGE DOOR | +22.8 -25.5 | |
| 16'x7 GARAGE DOOR | +21.7 -24.1 | |

Blake Construction

Josephine Abbate Res.
(Mother - In - Law suite Addition)

PROJECT ADDRESS:
Lot 9 Southern Approaches
Columbia County, FL

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

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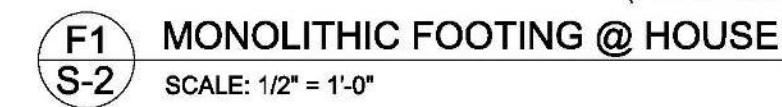


Wednesday, July 15, 2020

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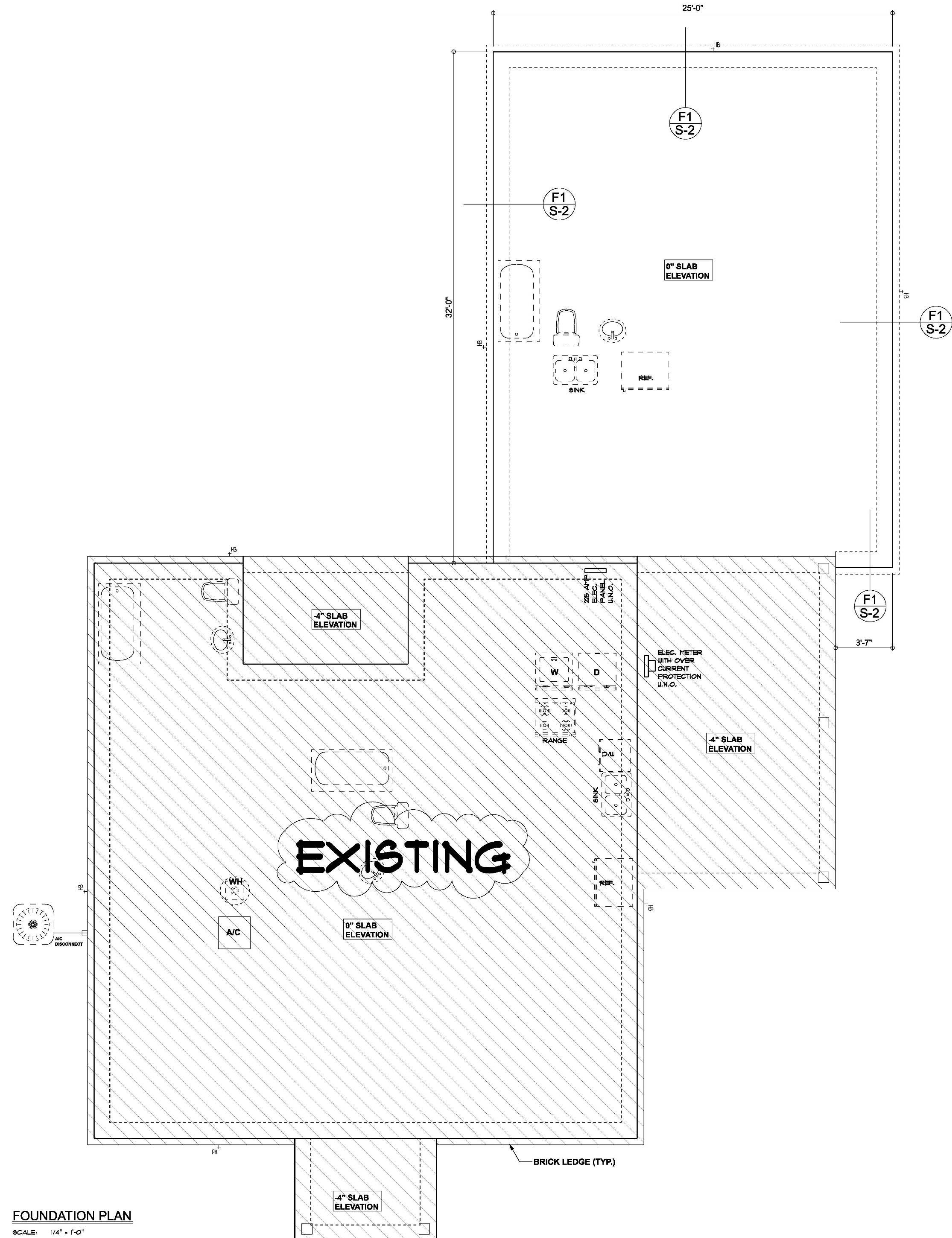
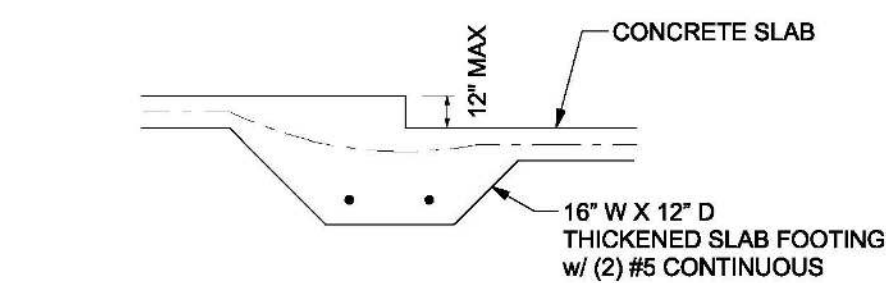
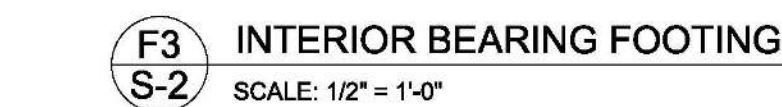
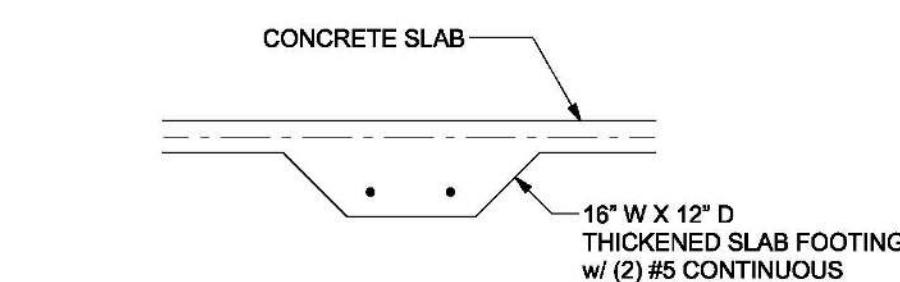
JOB NUMBER:
200118

S-1
OF 5 SHEETS



CONCRETE SLAB

12" W X 16" D FOR PORCH MONO FOOTING w/ (2) #5 CONTINUOUS



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

FN -1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, NECESSITIES IN SLAB, CHAIRS, ETC. DIMENSIONS OF DESIGN OR MATERIAL WASTE OR MISPLACEMENT, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

FN -2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING WALLS BY VISUAL INSPECTION OF EXISTING TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN -3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ Ø6X-14V A WELDED WIRE MESH PLACED ON CHAIRS (MINIMUM DEPTH OR RISE OF 4" MINIMUM) 6-#4, POLY LAP BAR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL. IF ANY OTHER METHOD OF TERMITE-TREATMENT METHOD CAN BE USED (TREATED)

Blake Construction

Josephine Abbate Res.
(Mother - In - Law suite Addition)

PROJECT ADDRESS:
109 Southern Approach
Columbia County, FL

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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Wednesday, July 15, 2020

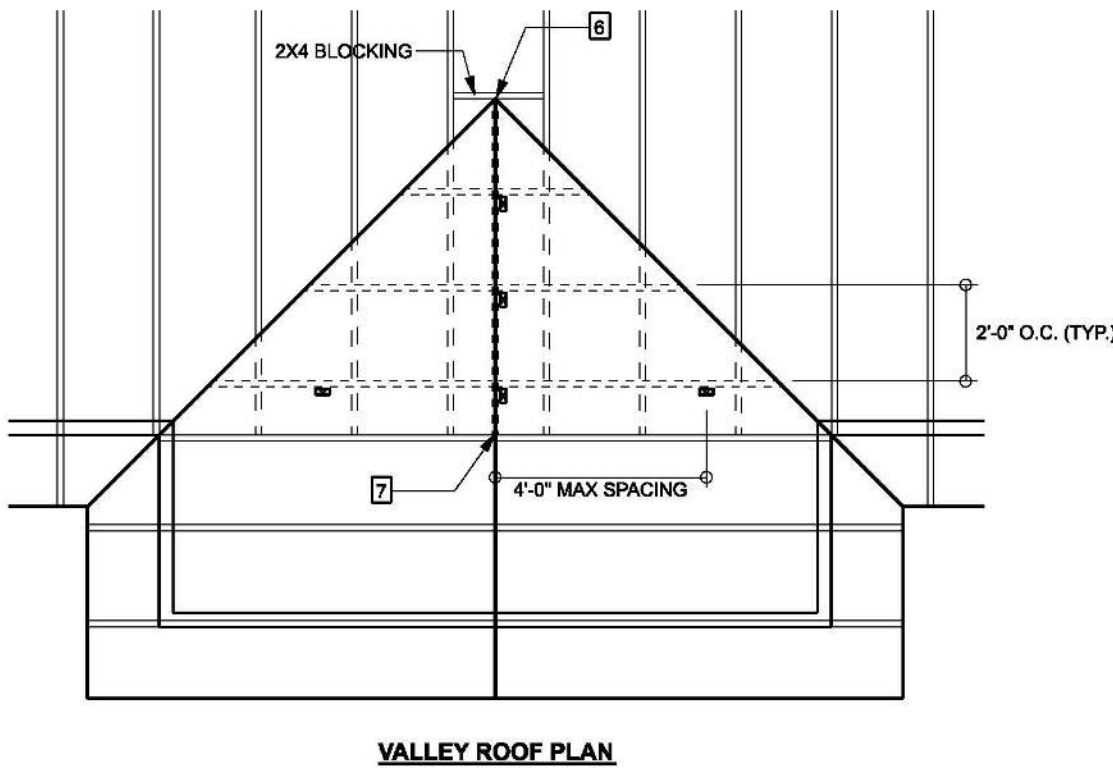
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JOB NUMBER:
200118

S-2
OF 5 SHEETS

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

| | |
|-----------------------------|---|
| CRIPPLES & BLOCKING | 2X4 SYP #2 OR BETTER |
| TRUSS BELOW | SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL |
| RIDGE BOARD | 2X6 SYP #2 |
| RAFTER SPANS 20'-0" OR LESS | 2X4 SYP #2 |
| PURLINS / LATERAL BRACING | 2X4 SYP #2 |
| SLEEPERS | 2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SYP #3 |
| CRIPPLES & BLOCKING | 2X4 SYP #2 OR BETTER |
| TRUSS BELOW | SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL |



VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

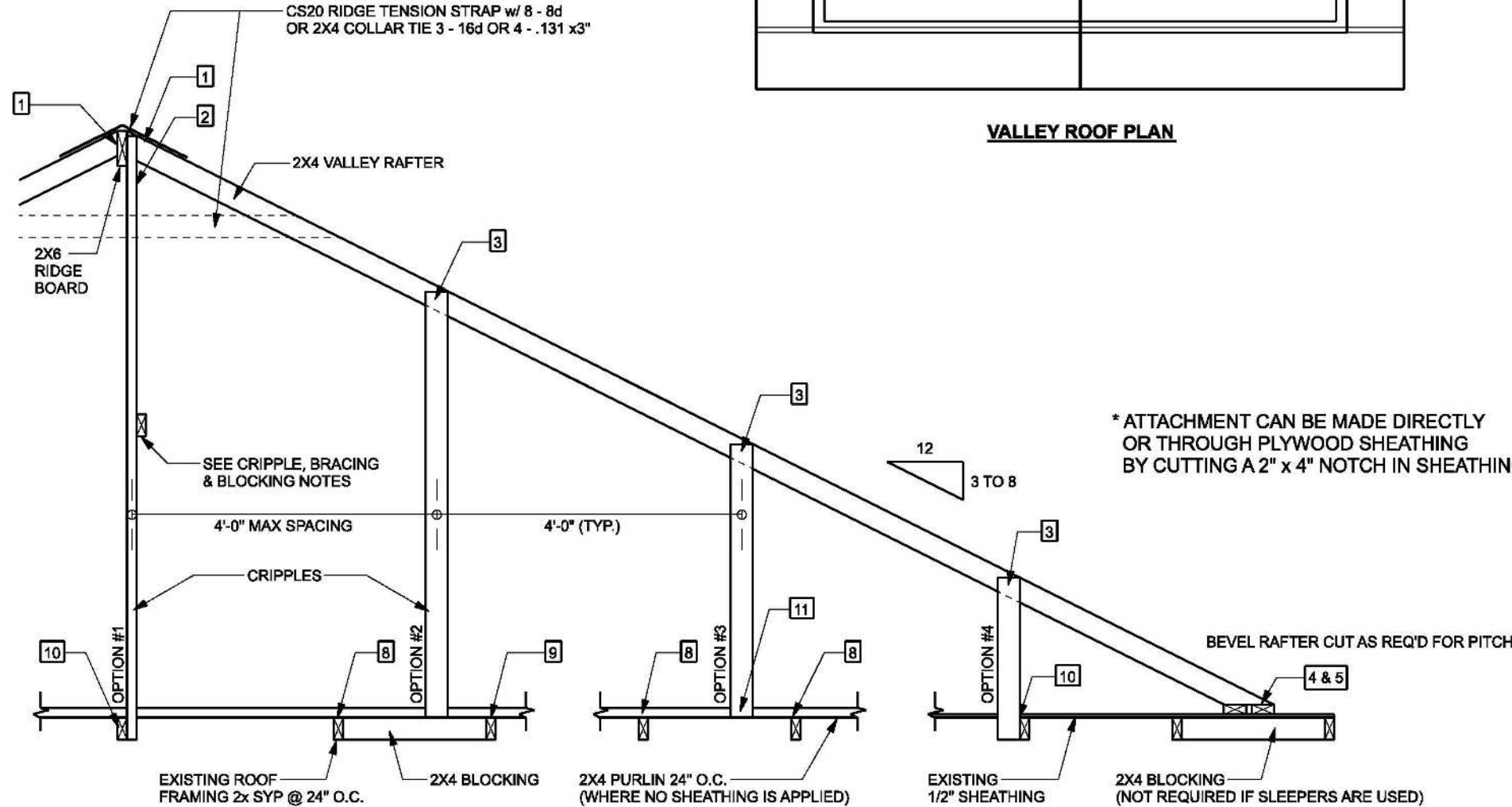
| | | |
|----|--|---|
| 1 | 2X4 RAFTERS TO RIDGE | 3-16d OR 6-131 x 3" TOE NAILS |
| 2 | CRIPPLE TO RIDGE | 3-16d OR 6-131 x 3" FACE NAILS |
| 3 | CRIPPLE TO RAFTERS | 3-16d OR 6-131 x 3" FACE NAILS |
| 4 | RAFTER TO SLEEPER OR BLOCKING | 6-16d OR 12-131 x 3" TOE NAILS |
| 5 | SLEEPER TO TRUSS | 4-16d OR 8-131 x 3" FACE NAILS EACH TRUSS |
| 6 | RIDGE BOARD TO RIDGE BLOCK | 3-16d OR 6-131 x 3" TOE NAILS |
| 7 | RIDGE BOARD TO TRUSS | 3-16d OR 6-131 x 3" TOE NAILS |
| 8 | PURLIN TO TRUSS (TYP.) | 3-16d OR 6-131 x 3" NAILS |
| 9 | PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN) | 4-16d OR 8-131 x 3" NAILS |
| 10 | CRIPPLE TO TRUSS | 3-16d OR 6-131 x 3" FACE NAILS |
| 11 | CRIPPLE TO PURLIN | 3-16d OR 6-131 x 3" FACE NAILS |

GENERAL NOTES

- MAXIMUM RAFTER SPANS: 6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
- MAXIMUM ROOF AREA PER SUPPORT: 1602 IN ZONES 2 & 3, 2402 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN 1602 OR 2'-0" X 8'-0" SPAN = 1602)
- PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
- PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
- IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8-8d COMMON WIRE NAILS.
- THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
 - SPANS (DISTANCES BETWEEN HEELS) 40'-0" OR LESS
 - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
 - MAXIMUM WIND SPEED: 130 MPH
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
 - MAXIMUM TOTAL LOADING: 40 psf
 - MEETS FBC 2014/ASCE 7-10 WIND REQUIREMENTS
 - EXPOSURE CATEGORY "C", I = 1.0, Kz = 1.0
 - ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

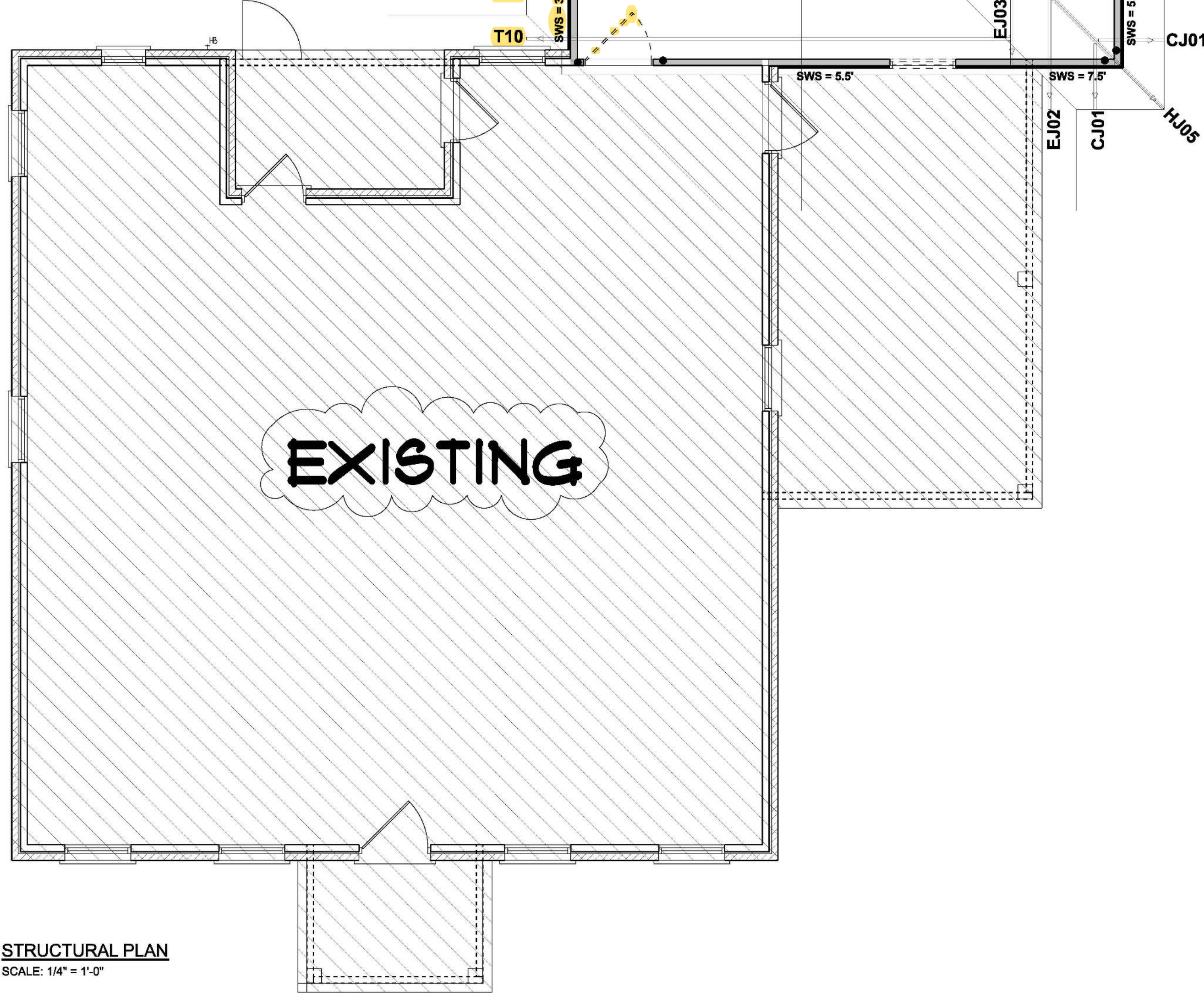
- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 6'-0" TO 10'-0" LONG NAILED W/ 2-10d NAILS OR 2X4 "T" OR SCAB BRACE NAILED TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 9" O.C. "T" OR SCAB MUST BE 80% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB'S ON BOTH FACES W/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



SECTION CUT PARALLEL TO VALLEY RAFTER

ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-8-1, BCSI-8-2, & BCSI-8-3. BCSI-8-1, BCSI-8-2, & BCSI-8-3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND

- (2) 2X10X0', 1J 1K: HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

THREADED ROD LEGEND

- INDICATES LOCATION OF: 3/8" A307 ALL THREADED ROD

ACTUAL vs REQUIRED SHEARWALL

| | TRANSVERSE | LONGITUDINAL |
|----------|------------|--------------|
| REQUIRED | 5371 LBF | 3873 LBF |
| ACTUAL | 9120 LBF | 10197 LBF |

Blake Construction

Josephine Abbate Res.
(Mother - In - Law suite Addition)

PROJECT ADDRESS:
Lot 9 Southern Approaches
Columbia County, FL

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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Wednesday, July 15, 2020

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JOB NUMBER:
200118

S-3
OF 5 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2406519