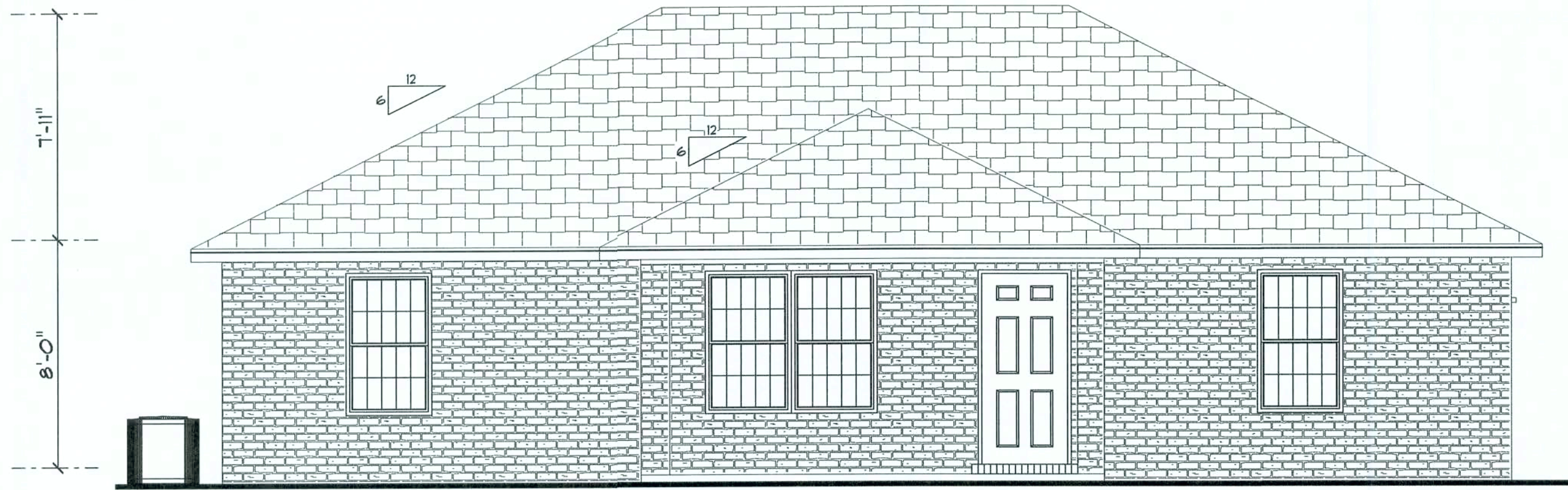


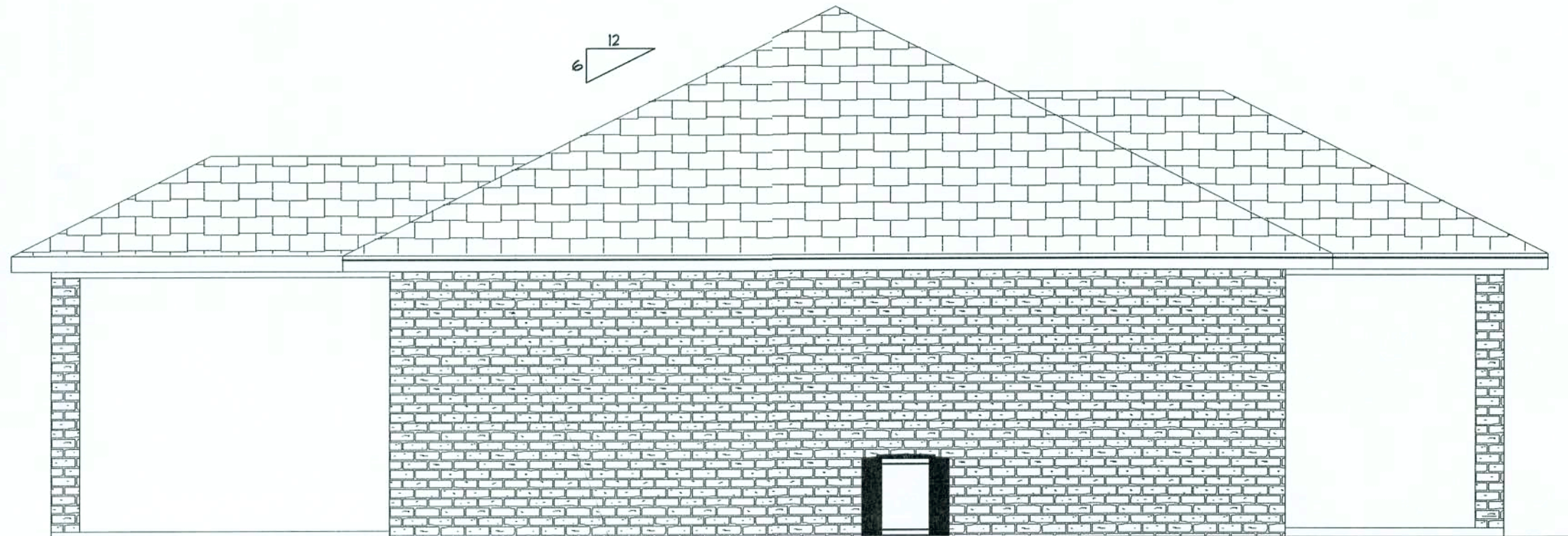
REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



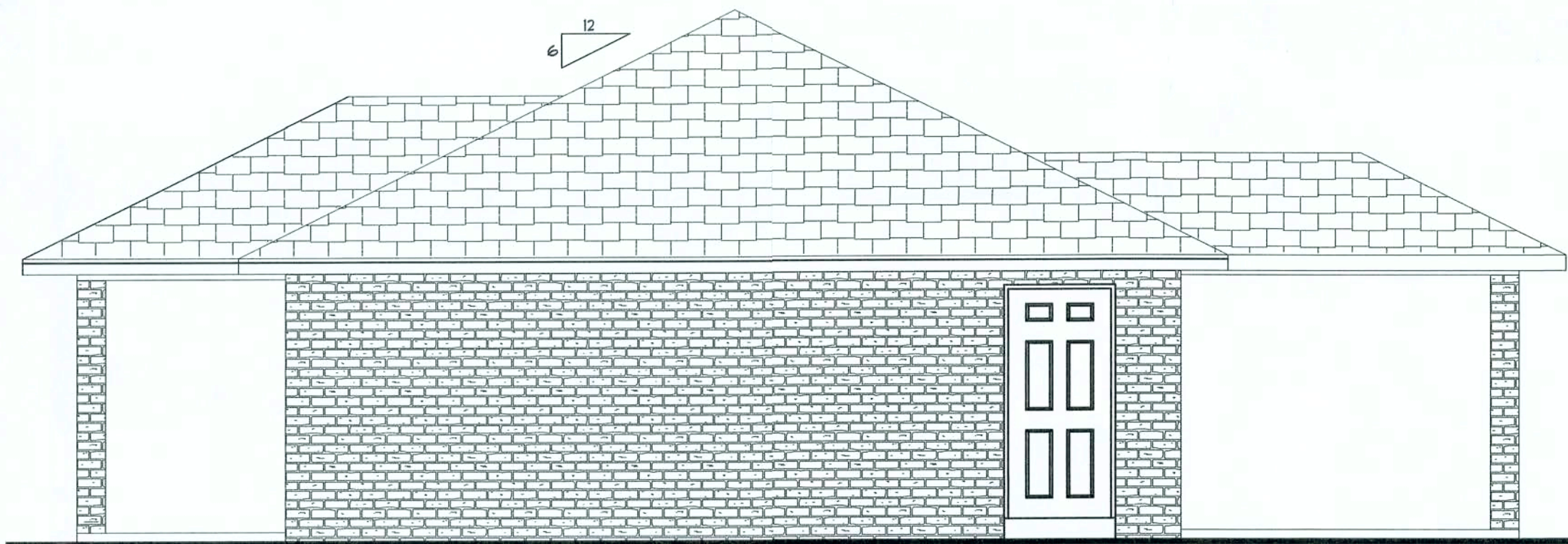
FRONT ELEVATION

SCALE: 1/4" = 1'-0"



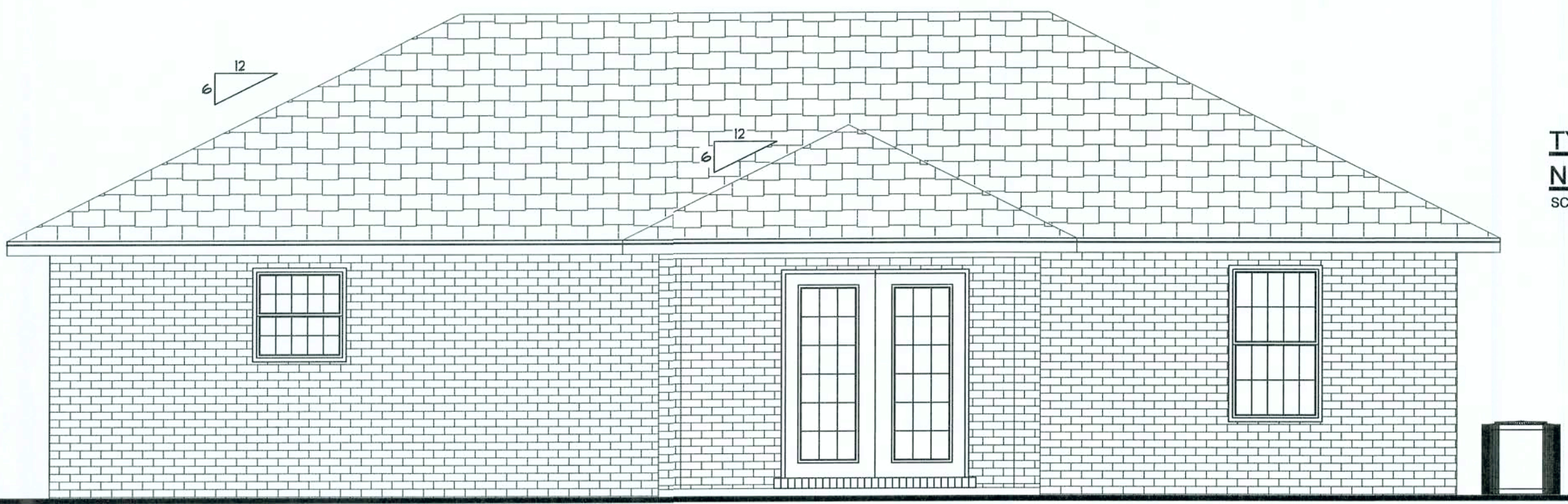
LEFT ELEVATION

SCALE: 1/4" = 1'-0"



RIGHT ELEVATION

SCALE: 1/4" = 1'-0"



REAR ELEVATION

SCALE: 1/4" = 1'-0"

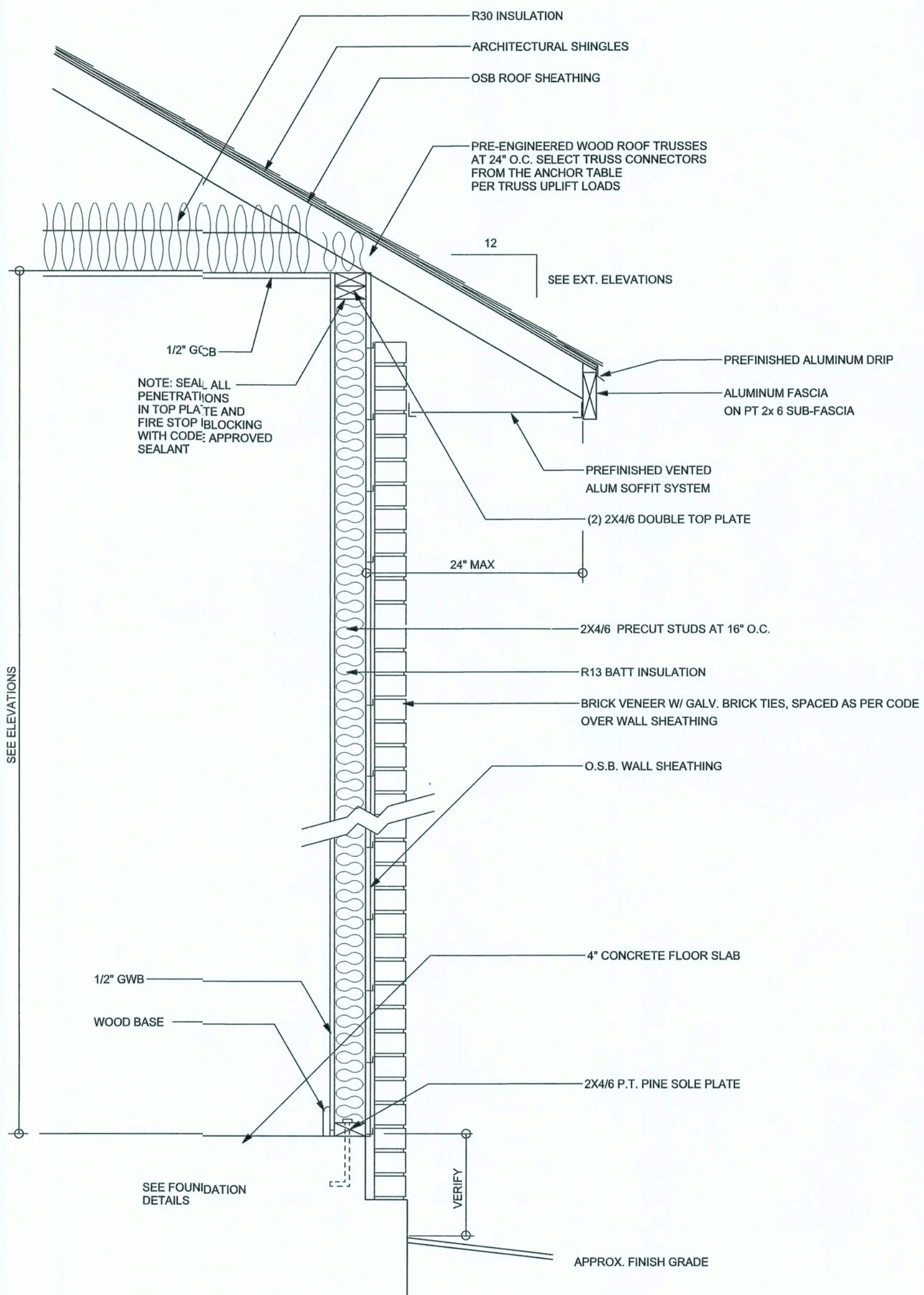
REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3" ABOVE EAVE)
1616 S.F. / 300 x 50% = 3 S.F. RIDGE VENT AREA REQUIRED
27 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT
1616 S.F. / 300 x 50% = 3 S.F. SOFFIT VENT AREA REQUIRED
100 FEET OF SOFFIT VENT REQUIRED

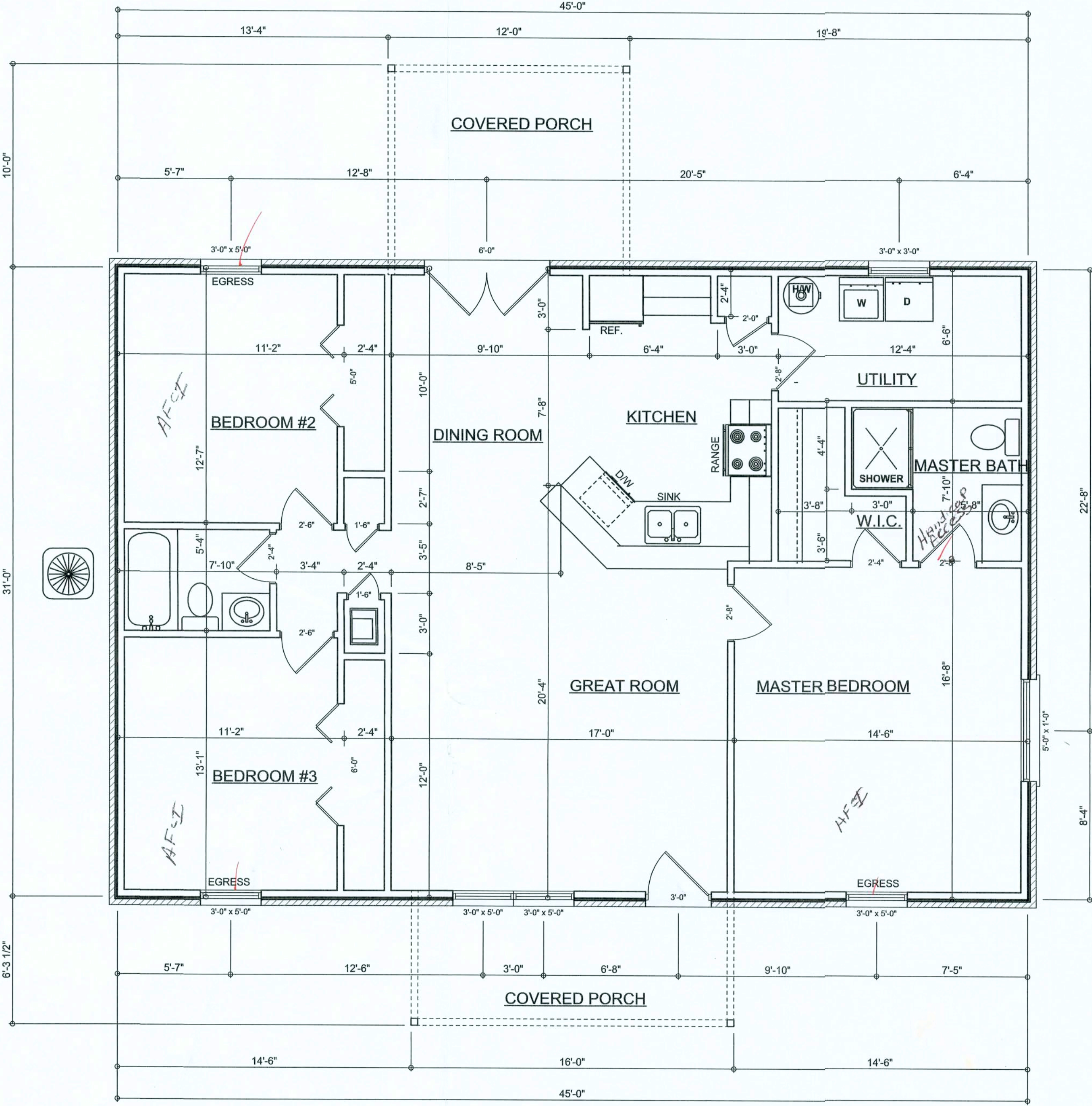
BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (11 FT2/FT)
2. OFF-RIDGE VENTS = 70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (03 FT2/FT)



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA

SCALE: 1" = 1'-0"



FLOOR PLAN

SCALE: 1/4" = 1'-0"
ALL CEILINGS TO BE 8' UNLESS NOTED OTHERWISE

AREA SUMMARY

LIVING AREA	1395	S. F.
PORCH AREA	221	S. F.
TOTAL AREA	1616	S. F.

Richard Keen

Stetz Residence

ADDRESS:
Old Itchetucknee Rd,
Columbia County, Florida

PRINTED DATE:
September 14, 2007

DRAWN BY: Ben Sparks

26321

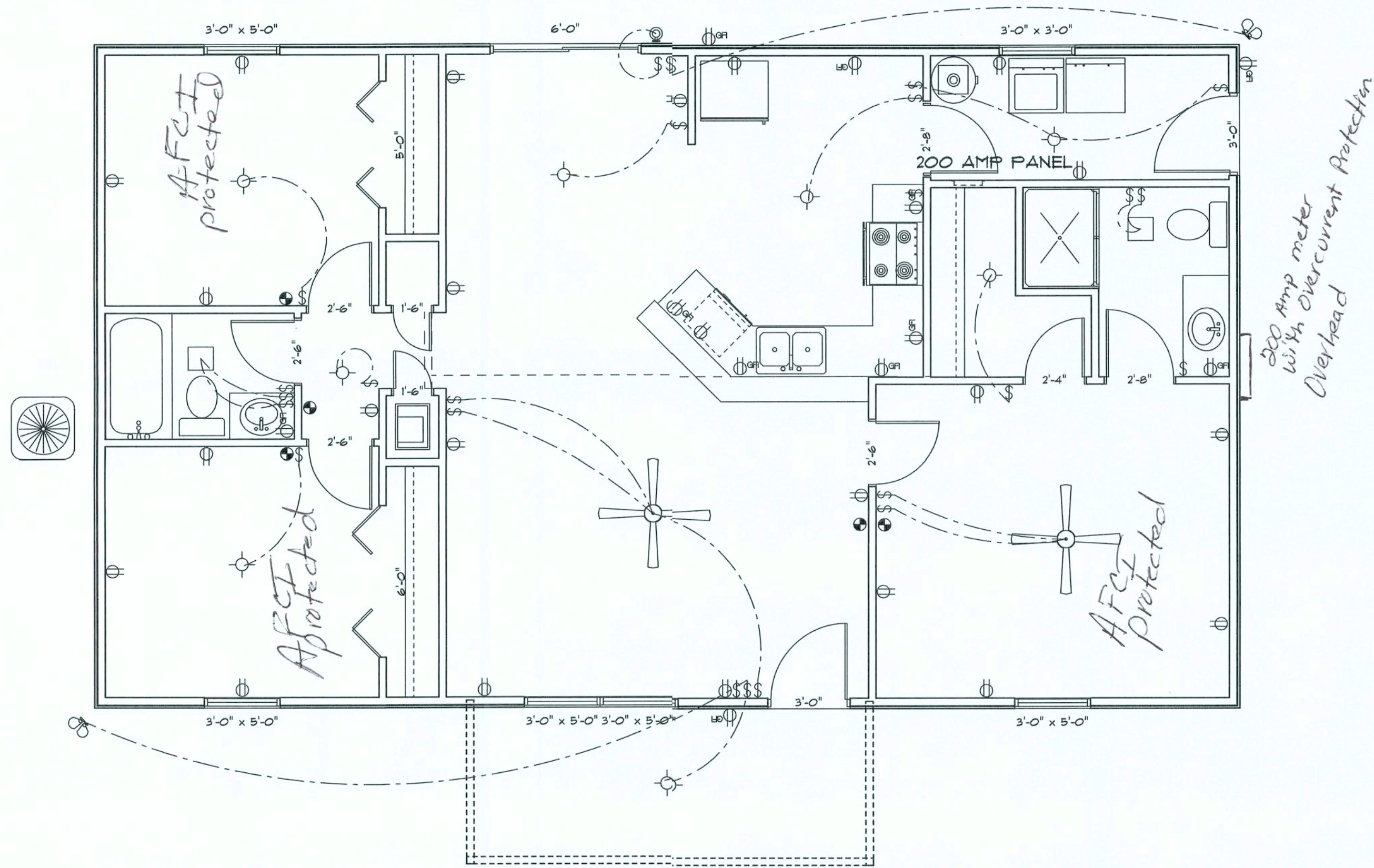
FINALS DATE:
14 / Sep / 07

JOB NUMBER:
709122

DRAWING NUMBER

A-1
OF 5 SHEETS

REVISIONS	



ELECTRICAL	SYMBOL
ceiling fan globe 2	
ceiling light vent square	
double spotlight	
wall mount 1	
electrical panel	
light	
outlet	
outlet gfi	
switch	

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ADDRESS:
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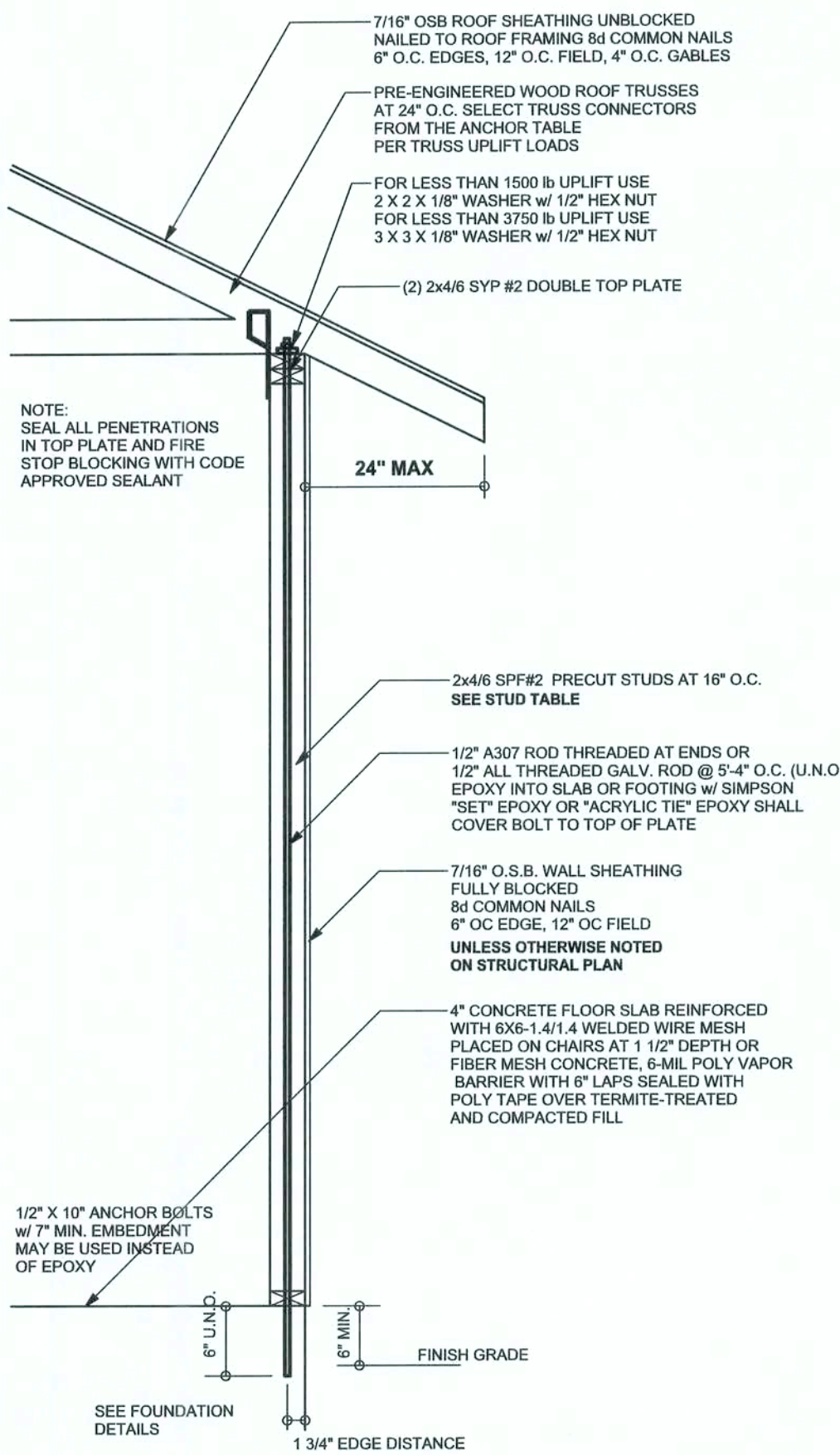
STRUCTURAL BY:

FINALS DATE:
14 / Sep / 07

JOB NUMBER:
709122

DRAWING NUMBER
A-2
OF 5 SHEETS

REVISIONS	

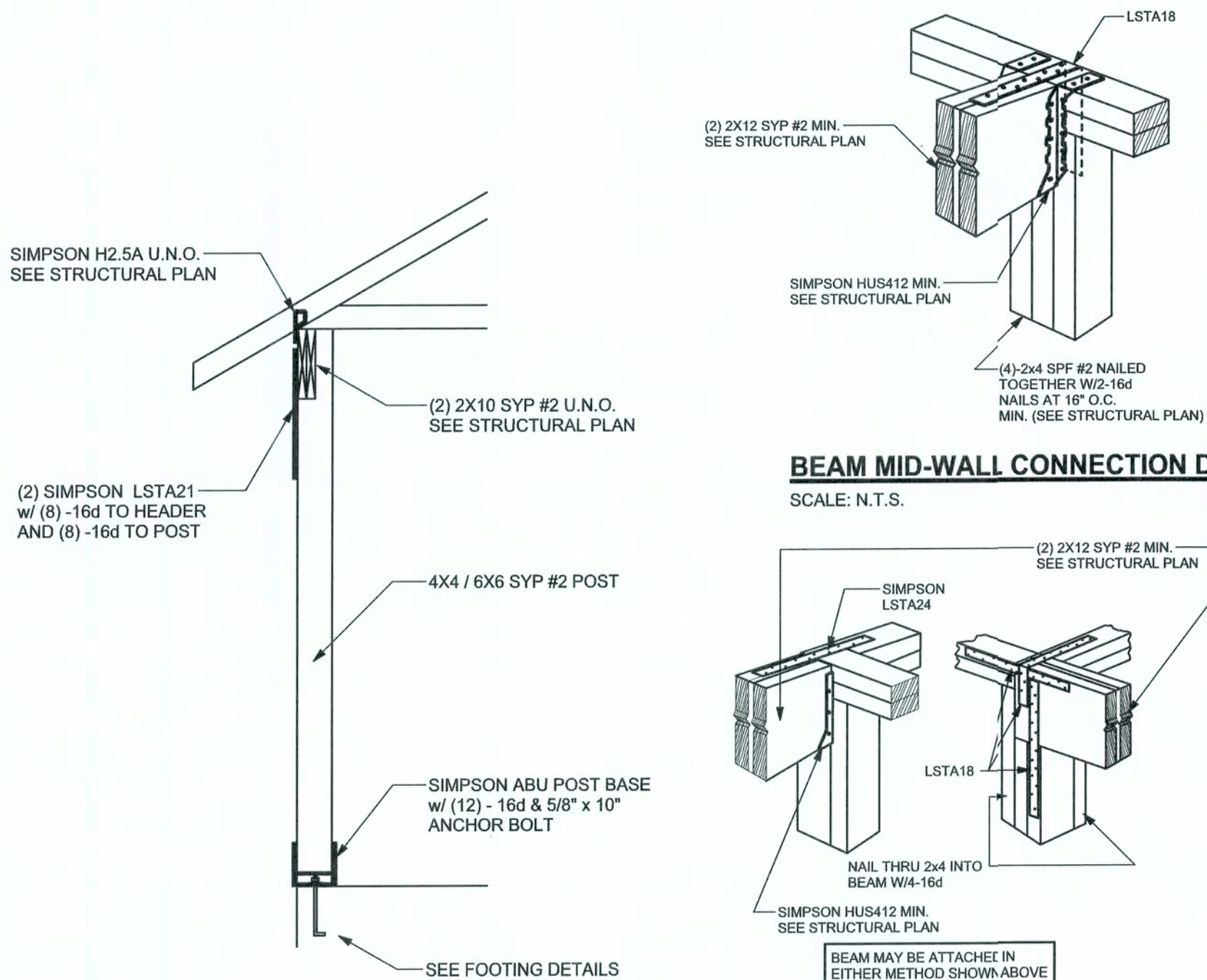


ONE STORY WALL SECTION
SCALE: 3/4\" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16\" OC	TO 11'-9\" STUD HEIGHT
(1) 2x4 @ 12\" OC	TO 13'-0\" STUD HEIGHT
(1) 2x6 @ 16\" OC	TO 18'-0\" STUD HEIGHT
(1) 2x6 @ 12\" OC	TO 20'-0\" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16\" O.C. x 0.85 = 13.6\" O.C.



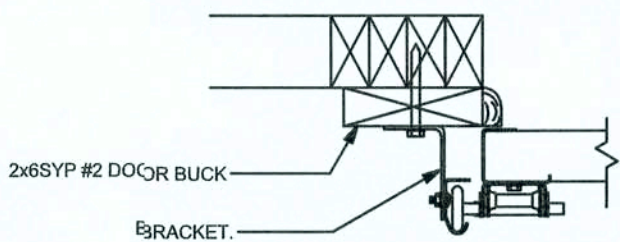
TYPICAL PORCH POST DETAIL
SCALE: 1/2\" = 1'-0"

BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.

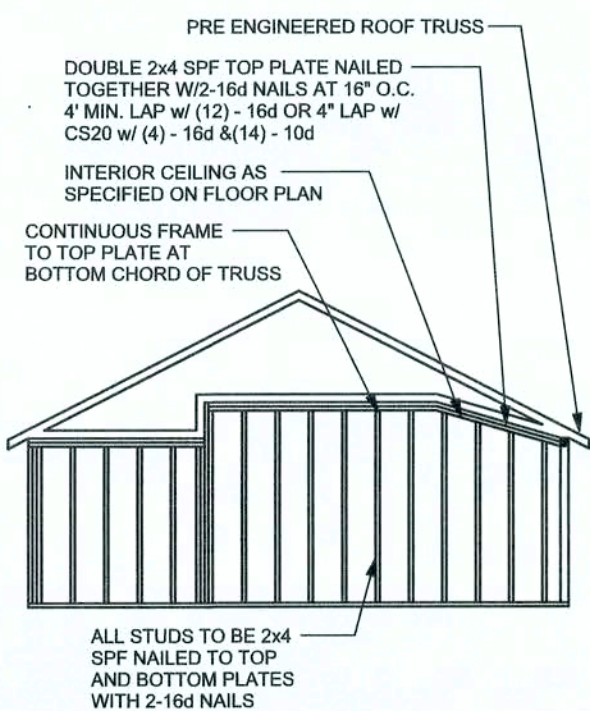
2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8\" x 4\" LAG SCREWS w/ 1\" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 1/4\" NAILS OR (2) ROWS OF .131 x 3 1/4\" ON PER TABLE BELOW:

DOOR WIDTH	3/8\" x 4\" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4\" GN
8' - 10'	24\" O.C.	5\" O.C.	5\" O.C.
11' - 15'	18\" O.C.	4\" O.C.	4\" O.C.
16' - 18'	18\" O.C.	3\" O.C.	3\" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON THE MANUFACTURER'S DESIGN ENGINEER. INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RATHERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RATHERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6\" x 6\" W1.4 x W1.4, FB = 59KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI. ALL LAP SPLICES 40\" DB (25\" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, UNO.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2,4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. **ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16\" OSB SHEATHING, UNLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6\" OC PANEL EDGES, 12\" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4\" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\" IN CONCRETE OR REINFORCED BOND BEAM OR 15\" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2\" BOLTS TO BE 2\" x 2\" x 9/64\"; WITH 5/8\" BOLTS TO BE 3\" x 3\" x 9/64\"; WITH 3/4\" BOLTS TO BE 3\" x 3\" x 9/64\"; WITH 7/8\" BOLTS TO BE 3\" x 3\" x 5/16\"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

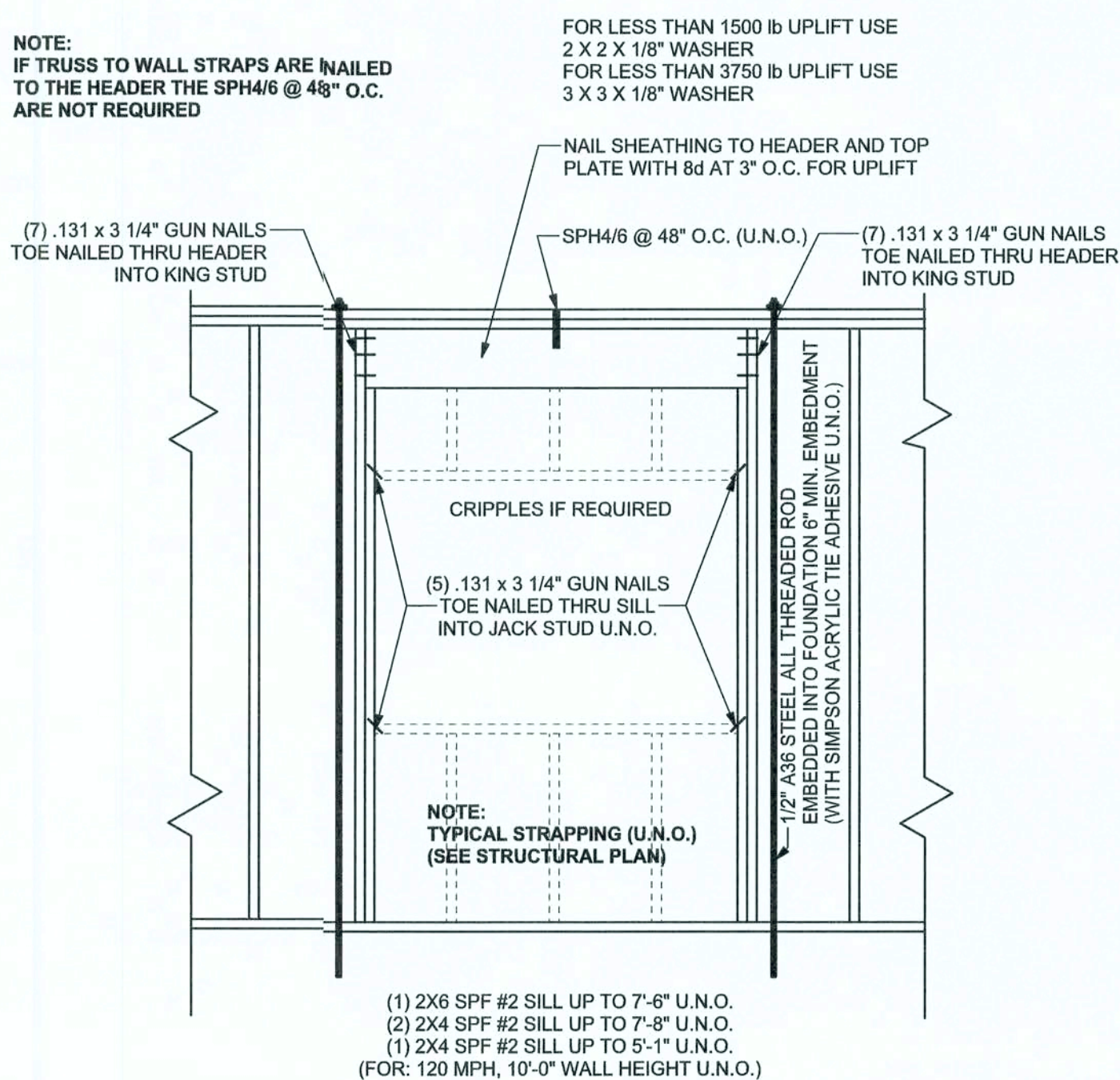
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

NOTE: IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48\" O.C. ARE NOT REQUIRED



TYPICAL 1 STORY HEADER STRAPING DETAIL
SCALE: 1/2\" = 1'-0"

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 380	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 665	H6	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG1		22 -10d	1-5/8\" THREADED ROD 12\" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8\" THREADED ROD 12\" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8\" THREADED ROD 12\" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8\" THREADED ROD 12\" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6 -10d, 1 1/2"
< 1240	< 1065	SPH4			10 -10d, 1 1/2"
< 885	< 760	SP6			6 -10d, 1 1/2"
< 1240	< 1065	SPH6			10 -10d, 1 1/2"
< 1235	< 1165	LSTA18	14 -10d		
< 1235	< 1235	LSTA21	16 -10d		
< 1030	< 1030	CS20	18 -8d		
< 1705	< 1705	CS16	28 -8d		
STUD ANCHORS*				TO STUDS	TO FOUNDATION
< 1350	< 1305	LTT19		8 -16d	12\" AB
< 2310	< 2310	LTT131	18 -10d, 1 1/2"		12\" AB
< 2775	< 2570	H23A	2-5/8\" BOLTS		5/8\" AB
< 4175	< 3695	HTT16	18 -16d		5/8\" AB
< 1400	< 1400	PAH242	16 -16d		
< 3335	< 3335	HPAH222	16 -16d		
< 2200	< 2200	ABU44	12 -16d		1/2\" AB
< 2300	< 2300	ABU66	12 -16d		1/2\" AB
< 2320	< 2320	ABU88	18 -16d		2-5/8\" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B. 30FT IN EXP. C AND >10% SLOPE AND UNOBSERVED UPWIND FOR 50X HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)		
	10	100	
1	19.9	-21.8	-18.1
2	19.9	-25.5	-18.1
2 07g		-40.6	-40.6
3	19.9	-25.5	-18.1
3 07g		-68.3	-42.4
4	21.8	-23.6	-20.4
5	21.8	-29.1	-18.5
			-22.6
Doors & Windows	21.8	-29.1	
Worst Case (Zone 5, 10 ft ²)			
8x7 Garage Door	19.5	-22.9	
16x7 Garage Door	18.5	-21.0	

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

WINDLOAD ENGINEER: Mark Disoway, P.E. No.03915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 03915
SEAL

Richard Keen

Stetz Residence

ADDRESS:
Old Ichauwee Rd.
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

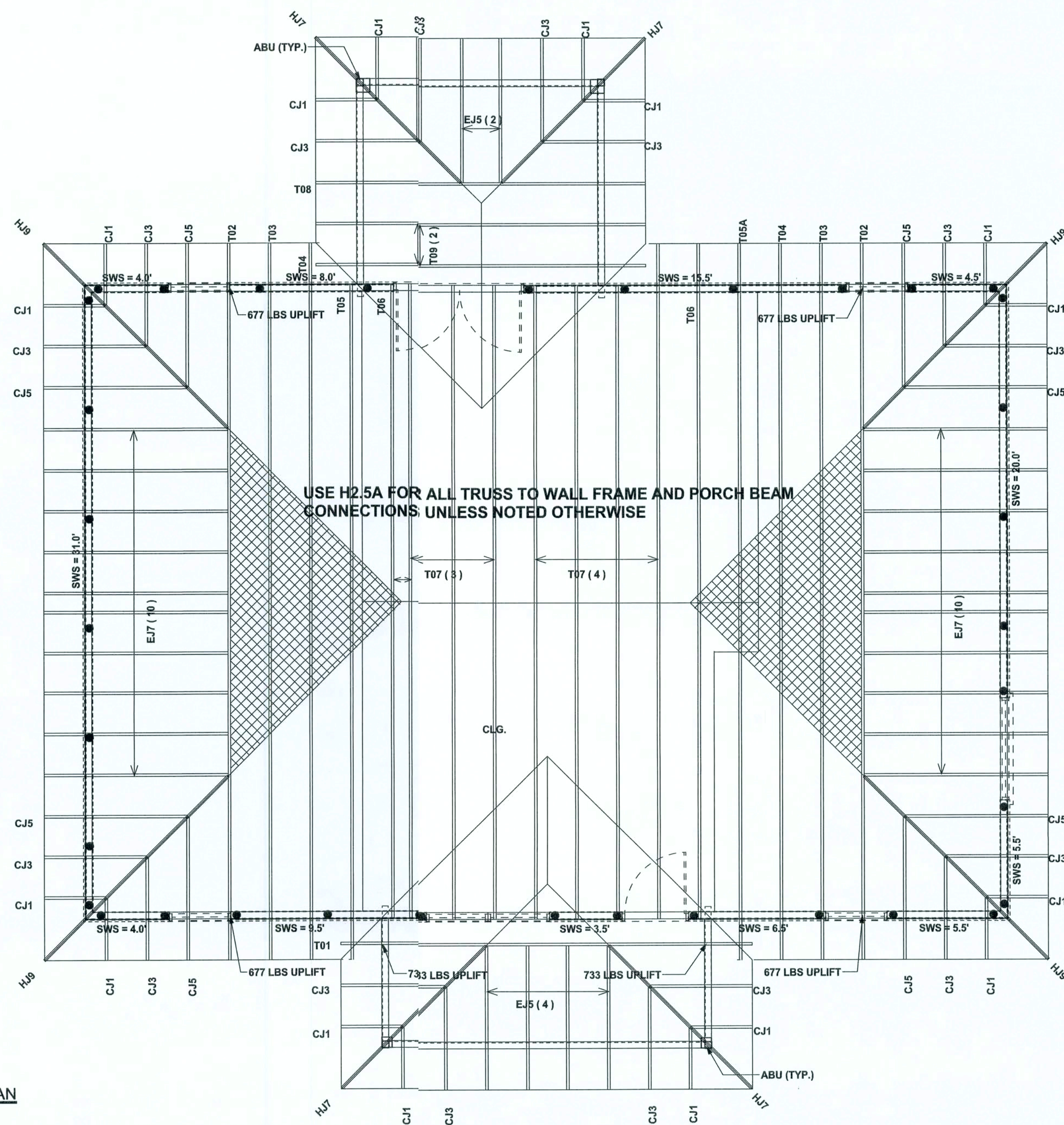
PRINTED DATE:
September 14, 2007
STRUCTURAL BY:
Ben Sparks

FINAL DATE:
14 / Sep / 07

JOB NUMBER:
709122
DRAWING NUMBER
S-1
OF 5 SHEETS

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

- INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD
- INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

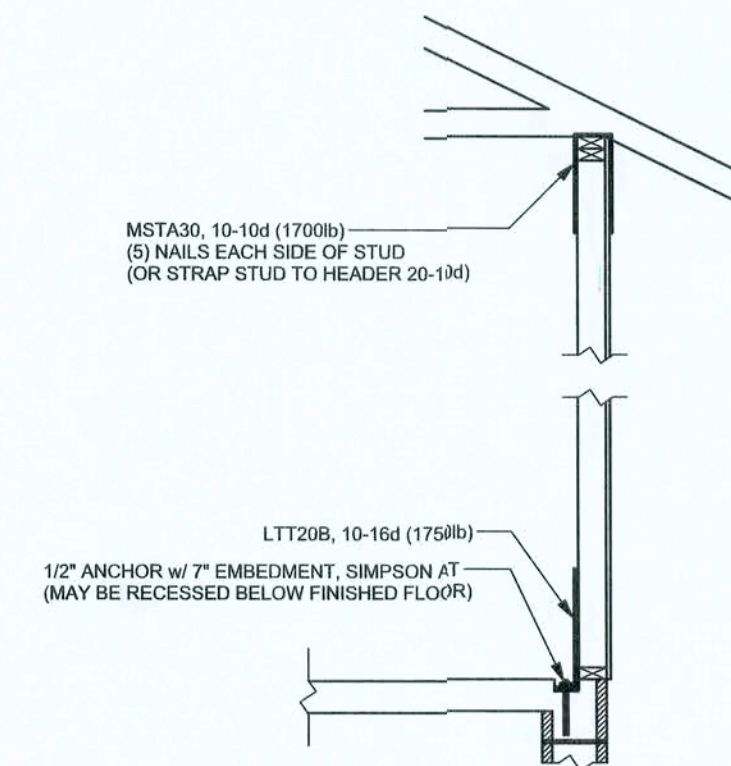
HEADER LEGEND

- (2) 2X12X0', 1J 1K ——— HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	29.0'	56.5'
LONGITUDINAL	11.5'	61.0'



ALTERNATE WALL TIE CONNECTION WHERE
THREADED ROD CANNOT BE PLACED IN WALL
SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway,
PE No.53915, POB 888, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915
Mark Disosway
11/26/07
SEAL

Richard Keen

Stetz Residence

ADDRESS:
Old Ichetucknee Rd,
Columbia County, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
September 14, 2007

STRUCTURAL BY:
Ben Sparks

FINALS DATE:
14 / Sep / 07

JOB NUMBER:
709122

DRAWING NUMBER

S-3

OF 5 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER, BUILDERS FIRST SOURCE
(JOB #L253081)