DATE 03/03/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000024180

APPLICANT PAT HAYGOOD PHONE 752-3496

ADDRESS 1259 S US HIGHWAY 441 LAKE CITY FL 32025

OWNER JOHN B. MACLAREN PHONE

ADDRESS 1040 SW OLD BELLAMY ROAD HIGH SPRINGS FL 32643

CONTRACTOR HAYGOOD HOMES PHONE 752-3496

LOCATION OF PROPERTY 41S, TR ON OLD BELLAMY RD, CROSS OLD LAKE CITY AVE, 7/10 MILE ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 134700.00

HEATED FLOOR AREA 2694.00 TOTAL AREA 3949.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH CONC FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 23

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 04-7S-17-09888-008 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES

		CRC1326715		
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor
EXISTING	06-0125-N	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD,

Check # or Cash 2370

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation 02/07/2006 RJ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 675.00 CERTIFICATION FEE $ 19.75 SURCHARGE FEE $ 19.75

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 789.50

INSPECTORS OFFICE _____ CLERKS OFFICE CH

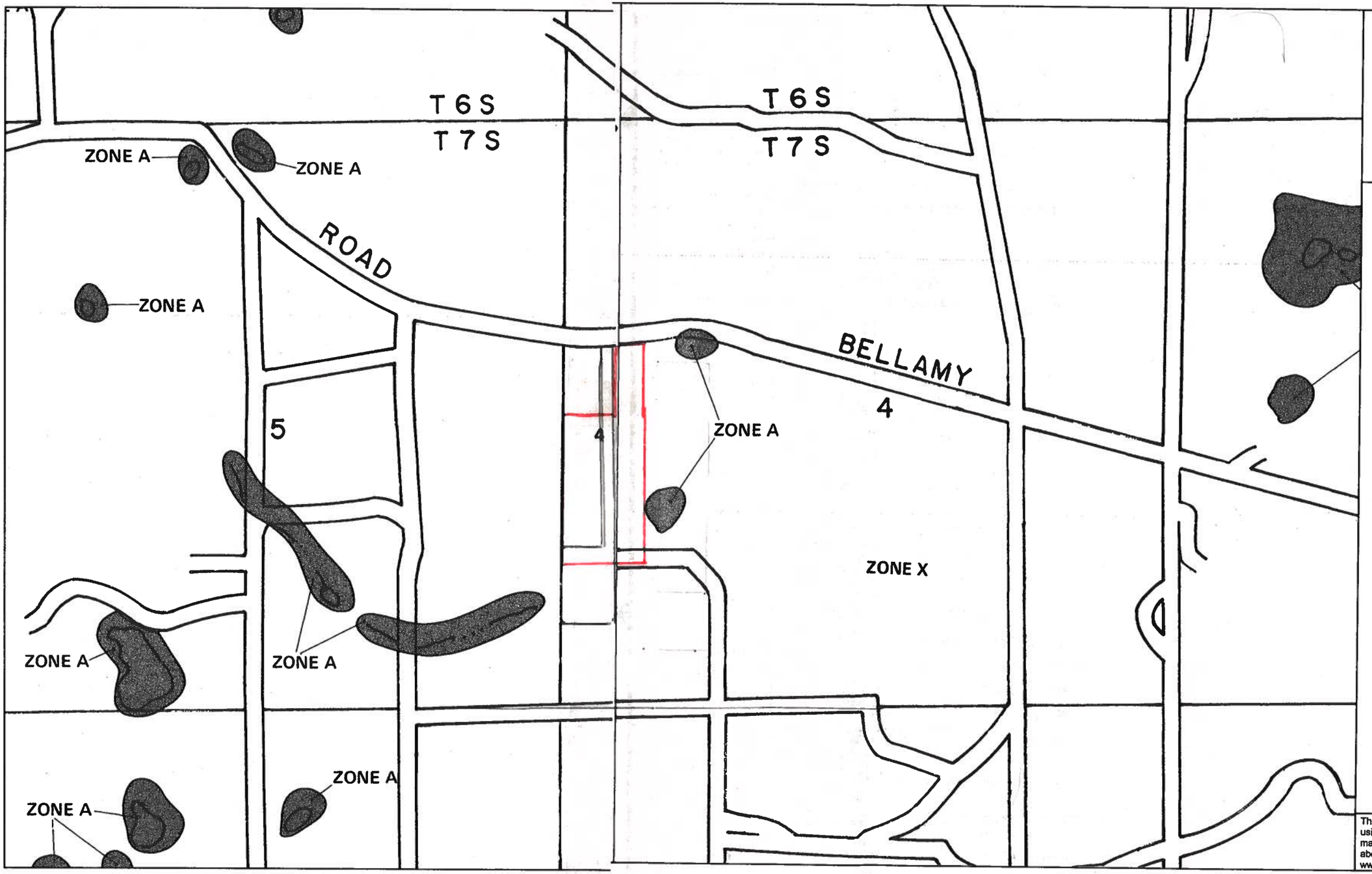
NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.



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Federal Emerge

This is an official copy of a portion of the a
using F-MIT Version 1.0. This map does n
may have been made subsequent to the d
about National Flood Insurance Program f
www.fema.gov/mit/tsd.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0602-52 Date Received 2-16-06 By G Permit # 24180

Application Approved by - Zoning Official BLK Date 02.23.06 Plans Examiner OK JTH Date 2-2-06

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments

Applicants Name Brenda Haygood Phone 386-752-3496

Address 12592 S. US Hwy 441 LC 32025

Owners Name John B. MacLaren Phone

911 Address 1040 SW Old Bellamy Rd. High Springs, Fl 32643

Contractors Name Haygood Homes, Inc Phone 386-752-3496 386-303-1981 (cell)

Address 12592 S. US Hwy 441 LC 32025

Fee Simple Owner Name & Address Campus USA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address S.P. Haygood Marty Humphries

Mortgage Lenders Name & Address Campus USA

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 04-7S-17-09888-008 Estimated Cost of Construction 204,000

Subdivision Name Lot Block Unit Phase

Driving Directions 41 South past I-75, Turn Right on old Bellamy Rd.
Cross old Lake City Ave lot on left (7/10 mile)

Type of Construction new home SFD Number of Existing Dwellings on Property 0

Total Acreage 20 Lot Size Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive)

Actual Distance of Structure from Property Lines - Front 1283 ✓ Side L 212 ✓ Side R 391 ✓ Rear 628 ✓

Total Building Height 23' 5" Number of Stories 1 Heated Floor Area 2230 ← 2694 Roof Pitch 7/12

Porch 608 BONUS Rm 464 GARAGE 647 TOTAL 3949

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
this 16th day of February 2006.

Personally known ✓ or Produced Identification

Contractor Signature
Contractors License Number
Competency Card Number
NOTARY STAMP/SEAL

Donna Higgs

Notary Signature

Donna S. Higgs MY COMMISSION EXPIRES March 31, 2007 #DD184369 Bonded thru Troy Fain-Insurance NOTARY PUBLIC, STATE OF FLORIDA

RECEIVED

FEB 0 3 2006

POC: Brenda Haygood
961-7559
Haygood Homes

Prepared by and return to: 911 Addressing
Kelley D. Jones
Attorney at Law
Kelley D. Jones, P.A.
5800 N.W. 39th Avenue Ste 102
Gainesville, FL 32606
352-377-2004
File Number: 05-012

[Space Above This Line For Recording Data]

Warranty Deed

This Warranty Deed made this 7th day of January, 2005 between Harold L. Cato and Louise Cato, husand and wife whose post office address is 1130 S.W. Old Bellamy Road, High Springs, FL 32643, grantor, and John B. MacLaren, a single person, and Jessica Slaymaker, a single person, as joint tenants with right of survivorship whose post office address is 1512 Avenue F NE, Winter Haven, FL 33841, grantee:

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida to-wit:

Begin at the NW corner of the SW 1/4 of Section 4, Township 7 South, Range 17 East, Columbia County, Florida, and run thence N 87°57'25" E along the North line thereof, 458.01 feet; thence N 02°14'18" W, 642.01 feet to the South line of Old Bellamy Road; thence N 89°27'35" E, 222.57 feet; thence S 02°13'30" E, 636.00 feet; thence S 02°11'24" E, 1314.63 feet; thence S 88°05'38" W, 678.59 feet; thence N 02°15'43" W, 1312.84 feet to the Point of Beginning.

Parcel Identification Number: R09888-002

Subject to taxes for 2005 and subsequent years; covenants, conditions, restrictions, easements, reservations and limitations of record, if any.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2004.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

#64

10240 SW Old Bellamy Rd
High Springs
32643

DoubleTimee

NOTICE OF INSPECTION AND/OR TREATMENT

24180

Date of Inspection

5/24/06

Date of Treatment

Bora-care

Pesticide Used

Subterranean Termites

Wood-Destroying Organisms Treated

Notice

It is a violation of Florida State Law (Chap. 482.226) for anyone other than the property owner to remove this notice.

Address:

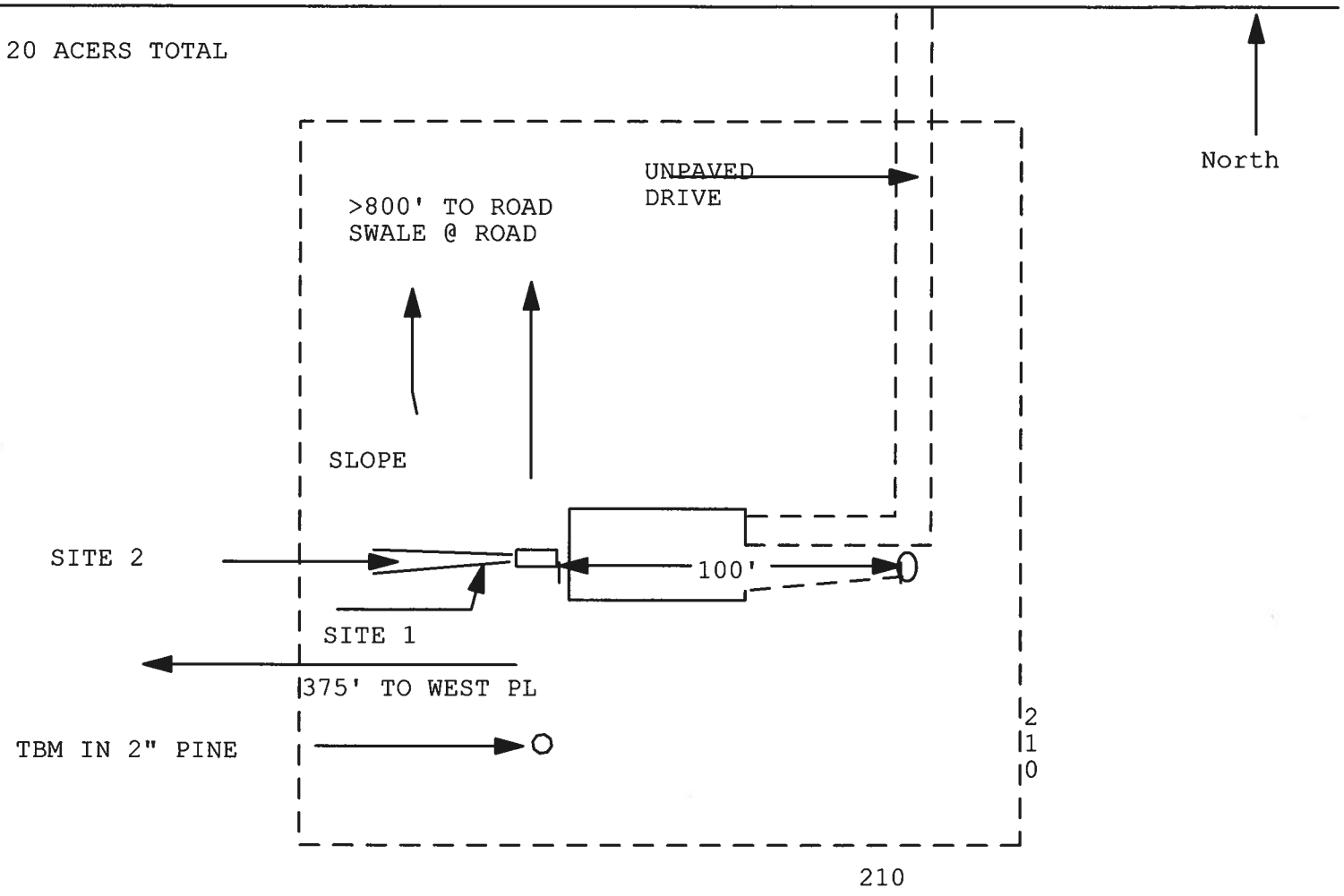
Pestmaster Services of Lake City

879 S.W. Arlington Blvd., Suite 106 • Lake City, FL 32025

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0125N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



MACLAREN/ CR# 05-3311

1 inch = 50 feet

Site Plan Submitted By Paul Lloyd **Date** 1/26/06

Plan Approved ✓ **Not Approved** ____ **Date** 2-13-06

By Marsha Land Columbia **CPHU**

Notes: _____

Columbia County Property Appraiser

DB Last Updated: 2/10/2006

2006 Proposed Values

Parcel: 04-7S-17-09888-008

Tax Record | Property Card | Interactive GIS Map | Print

Owner & Property Info

<< Prev Search Result: 4 of 4

Owner's Name	MACLAREN JOHN B &
Site Address	
Mailing Address	JESSICA SLAYMAKER (JTWRS) 1512 AVENUE F NE WINTER HAVEN, FL 33881
Brief Legal	BEG NW COR OF SW1/4 & RUN E ALONG N LINE THEREOF 458.01 FT, N 642.01 FT TO S LINE OF

Use Desc. (code)	NO AG ACRE (009900)
Neighborhood	4717.00
Tax District	3
UD Codes	MKTA02
Market Area	02
Total Land Area	23.750 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$142,500.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$142,500.00

Just Value	$142,500.00
Class Value	$0.00
Assessed Value	$142,500.00
Exempt Value	$0.00
Total Taxable Value	$142,500.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
1/7/2005	1071/2123	WD	V	U	06	$100.00
9/1/1977	386/163	WD	V	Q		$20,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
009900	AC NON-AG (MKT)	23.750 AC	1.00/1.00/1.00/1.00	$6,000.00	$142,500.00

Columbia County Property Appraiser

DB Last Updated: 2/10/2006

<< Prev 4 of 4

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 2/3/2006 **DATE ISSUED:** 2/14/2006

ENHANCED 9-1-1 ADDRESS:

1040 SW OLD BELLAMY RD

HIGH SPRINGS FL 32643

PROPERTY APPRAISER PARCEL NUMBER:

04-7S-17-09888-008

Remarks:

PARENT PARCEL 09888-002

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
904 NW Main Blvd.
LAKE CITY, FLORIDA 32055

June 12, 2002

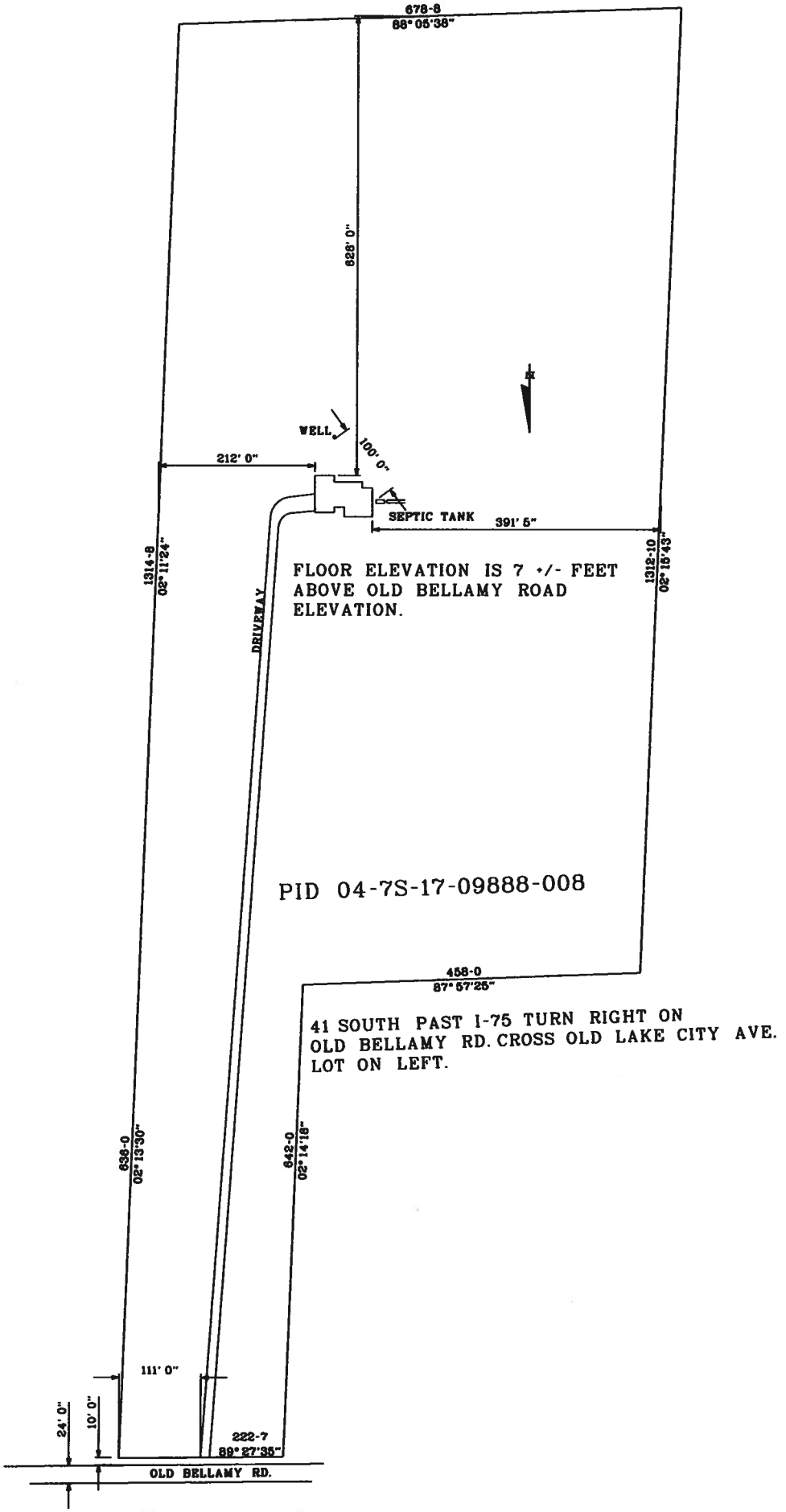
NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk



Begin at the NW corner of the SW 1/4 of Section 4, Township 7 South, Range 17 East, Columbia County, Florida and run thence N 87°57' 25" E. along the North line thereof, 458.01 feet; thence N 02°14' 18" W, 642.01 feet to the South line of Old Bellamy Road; thence N 89°27' 35" E, 222.57 feet; thence S 02°13' 30" E, 636.00 feet; thence S 02°11' 24" E, 1314.63 feet; thence S 88°05' 38" W, 678.59 feet; thence N 02°15' 43" W, 1312.84 feet to the Point of Beginning.

FORM 600B-04

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Residential Component Prescriptive Method B

NORTH 1 2 3

Compliance with Method B of Subchapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single-and multiple-family residences of three stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptives listed in this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the code.

PROJECT NAME: AND ADDRESS:	Maclaren	**BUILDER:** Haygood Homes, Inc	
		PERMITTING OFFICE: Columbia	**CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☑
OWNER:		**PERMIT NO.:** 24180	**JURISDICTION NO.:** 221000

1. New construction including additions which incorporate any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other nonvertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the code. Circle the column of the package you have choosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on "Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages," Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

	Please Print	CK
1. **Compliance package chosen (A-E)**	1.	
2. **New construction or addition**	2. new	
3. **Single-family detached or multiple-family attached**	3. single	
4. **If multiple-family–No. of units covered by this submission**	4.	
5. **Is this a worst case? (yes/no)**	5. yes	
6. **Conditioned floor area (sq. ft.)**	6. 2694	
7. **Predominant eave overhang (ft.)**	7. 2'	
8. **Glass type and area:**	Single Pane / Double Pane	
a. Clear glass	8a. ____ sq. ft. 602 sq. ft.	
b. Tint, film or solar screen	8b. ____ sq. ft. ____ sq. ft.	
9. **Percentage of glass to floor area**	9. 22 %	
10. **Floor type, area or perimeter, and insulation:**		
a. Slab-on-grade (*R*-value)	10a R = C 266 lin. ft.	
b. Wood, raised (*R*-value)	10b. R = ____ ____ sq. ft.	
c. Wood, common (*R*-value)	10c. R = ____ ____ sq. ft.	
d. Concrete, raised (*R*-value)	10d. R = ____ ____ sq. ft.	
e. Concrete, common (*R*-value)	10e. R = ____ ____ sq. Ft.	
11. **Wall type, area and insulation:**		
a. Exterior: 1. Masonry (Insulation R-value)	11a-1 R = ____ ____ sq. ft.	
a. Exterior: 2. Wood frame (Insulation *R*-value)	11a-2 R = 13 2394 sq. ft.	
b. Adjacent: 1. Masonry (Insulation *R*-value)	11b-1 R = ____ ____ sq. ft.	
b. Adjacent: 2. Wood frame (Insulation *R*-value)	11b-2 R = ____ ____ sq. ft.	
12. **Ceiling type, area and insulation:**		
a. Under attic (Insulation *R*-value)	12a. R = 30 sq. ft. 2694	
b. Single assembly (Insulation *R*-value)	12b. R = ____ ____ sq. ft.	
13. **Air distribution system: Duct insulation, location** Test report (attach if required)	13. R = 6 ____	
14. **Cooling system:** (Types: central, room unit, package terminal A.C., gas, none)	14a. Type: Central	
	14b. SEER/EER: 13	
	14c. Capacity: 4 ton	
15. **Heating system:** (Types: heat pump, elec. strip, nat. gas, LP-Gas, gas h.p., room or PTAC, none)	15a. Type: Heat Pump	
	15b. HSPF/COP/AFUE: ____	
	15c. Capacity: 50 gal	
16. **Hot water system:** (Types: elec., nat. gas, LP-gas, solar, heat rec., ded. heat pump, other, none)	16a. Type: Elect	
	16b. EF: .88	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: Brenda Haygood **DATE:** 2-10-06

I hereby certify that this building is in compliance with the Florida Energy Code:

OWNER AGENT: _____ **DATE:** 2-10-06

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

TABLE 6B-1 — MINIMUM REQUIREMENTS — Climate Zones 1 2 3

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION A	B	C	D	E	TO BE INSTALLED
GLASS	Max. % of Glass to Floor Area	15%	15%	20%	20%	25%	15 %
GLASS	Type	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Tint (DT)	DC: ☐ DT: ☐
GLASS	Overhang	1'4"	2'	2'	2'	2'	2 FEET
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE.					EXT: R = ADJ: R = COM: R =
WALLS	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD-FRAME WALLS R-11					EXT: R = 13 ADJ: R = COM: R =
CEILINGS		R-30	R-30	R-30	R-30	R-30 (NO SINGLE ASSEMBLY CEILINGS ALLOWED)	UNDER ATTIC: R= 30 COMMON: R:
FLOORS	Slab-On-Grade	R-0					R = 0
FLOORS	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)					R =
FLOORS	Raised Concrete	R-7					R =
DUCTS		R-6	R-6	R-6, TESTED	R-6	R-6, TESTED	R = 6 6
SPACE COOLING (SEER)		12.0	10.5	12.0	11.0	12.0	R = 6 COND. ☐
HEAT	Elect. (HSPF)	7.9	7.1	7.4	7.4	7.4	SEER = 13
HEAT	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)					HSPF =
HOT WATER SYSTEM	Electric Resistance**	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	AFUE =
HOT WATER SYSTEM	Gas & Oil**	MINIMUM EF OF .59				NATURAL GAS ONLY (SEE BELOW)	EF = .88
HOT WATER SYSTEM	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.					EF = DHP: ☐ EF = HRU: ☐ SOLAR: ☐ EF =

* Single package units minimum SEER=9.7, HSPF = 6.6.
** Minimum efficiencies for gas and electric hot water systems apply to 40 gallon water heaters. Refer to Table 612.1 ABC.3.2 for minimum code efficiencies for oil water heaters and other sizes.

DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions: 1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multistory house.

Wall, Ceiling and Floor Insulation Values: The *R*-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The *R*-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multiple-family building. "Adjacent" components separate conditioned space from unconditioned but enclosed space. "Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a state-approved tester.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTACs equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of Section 608.1.ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed," an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options." See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having and EF of .92 or greater, or natural gas systems with EF .59 or greater may be used in conjunction with these systems.

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Multistory Houses	606.1	Air barrier on perimeter of floor cavity between floors.	✓
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker electric or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Noncommercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	NA
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	NA
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 psig.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

Joe ,

If you have any questions regarding this Addendum or I can be of assistance to not hesitate to contact me on my Cell phone(386)362-9169. I am a little concerned with the last statement of #5 which read "These connection points shall be designed by an architect or engineer using the engineered roof truss plans.". If by that statement you mean use the layout provided by the truss manufacturer I see no problem with that request. However, if you mean I should use the truss manufacturers uplift values I would have concerns. The reason I say this is that many truss manufacturers overkill the design of their trusses by applying heavy dead loads to them during their analysis and thus they turn out a stronger truss design. However, when they do this it reduces the uplift values on the truss package. I strongly believe that the engineer doing the wind loading for the structure should calculate his/her own uplifts when specifying anchors. Otherwise in many cases you could wind up with undersized anchors. For example in the case of these plans, if I were to use the truss package uplift values I would have undersized the anchors. My calculated dead loads are much smaller than their calculated dead loads, therefore I calculate higher uplift loads. They are correct in their design values for the truss design. However, these values do not produce the same uplift I calculated for these trusses and I would not in this case use the truss package uplift values. I just wanted to caution encouraging the engineer performing the wind analysis on the structure from blindly following the uplift values of the truss package.

Sincerely,

Marty

McLearn, Columbia County FL
Windload Requirements Addendum/Modification

(In Compliance with the 2004 Florida Building Code and Ammendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

The following requirements are in addition to, and supercede (where applicable) the windload requirements prepared for the McLearn residence dated 2-6-06.

1.) The following is clarification for truss anchor requirements as related to truss designations on the truss manufacturers drawings(see attached truss layout):

Truss Designation	Anchor Required	Location of Anchor	Anchor Uplift Capacity(lbs)
A1-D9	Simpson H10	At Bearing Locations	905
D-3	2-Simpson H11Z's	At Bearing Locations	830
E1-M1	Simpson H2.5A	At Bearing Locations	600
P1-P3 (piggy-back trusses)	Connect as required by truss manufacturer		

2.) wall strap tie requirements:

At top and bottom of wall install one Simpson model SP4 at each side of each door or window under 4' or less in width. For doors or windows greater than 4' in width install 2-SP4's each side top and bottom. For garage door openings install 2-SPH4's top and bottom each side. All other wall locations install SP4's top and bottom of wall 4' on center.

In addition the walls each side of the Foyer and the wall between the Garage and the Utility/Storage areas shall be strapped with SPH4's – 32" O.C. top and bottom of the wall. Install anchor bolts for these walls as required for exterior walls. SP4 uplift capacity is 630 lbs and SPH4 uplift capacity is 1065 lbs.

3.) Dormer requirements are as follows:

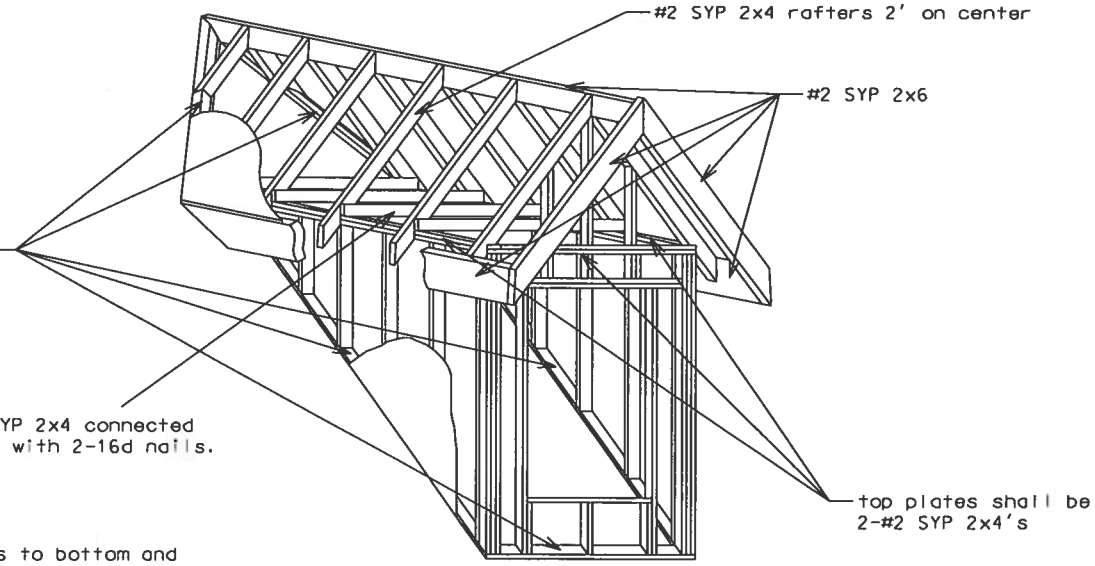
1.) Center dormer over foyer shall be framed with SPF studs 16" O.C. with 1-#2 SYP 2x4 for bottom plate and 2-#2 SYP 2x4's for top plates. Sheathing requirements and nailing patterns are same as for all exterior walls and roof sheathing. No additional strapping at top and bottom of wall; sheathing provides adequate uplift and shear capacity. Attach bottom of dormer wall to foyer walls with 1- Simpson LSTA12 48" O.C. See table above for anchors required for dormer trusses.

Marty J. Humphries
2-26-06

2.) Side fake dormers shall be framed with SPF studs 16" O.C. with 1-#2 SYP 2x4 for bottom plate and 2-#2 SYP 2x4's for top plates. Sheathing requirements and nailing patterns are same as for all exterior walls and roof sheathing. No additional strapping at top and bottom of wall; sheathing provides adequate uplift and shear capacity. (See attached detail for additional dormer construction requirements) Attach rafters to top plates of dormers with 1- Simpson H5 anchor. (H5 uplift capacity is 455 lbs)

Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

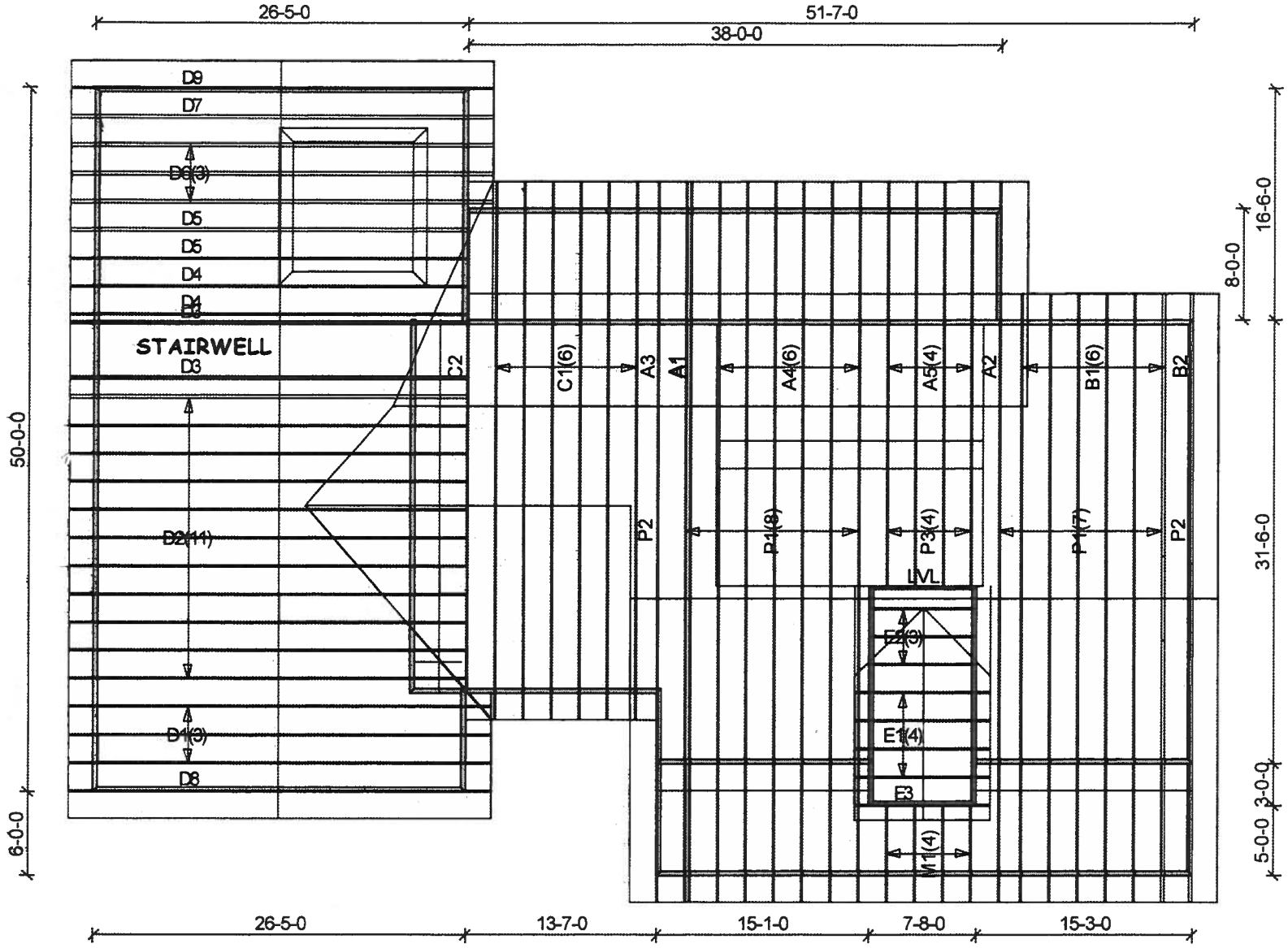
2-26-06



Note:
Connect studs to bottom and top plates with 2-16d nails and connect roof rafters to ridge plate with 2-16d nails. Also connect facia plate to rafters with 2-16d nails at each location.

DORMER FRAMING DETAIL

2-26-06



Mayo Truss Co. Inc.
362 NE CLYDE AVE.
MAYO, FL 32066
(386)294-3988
(877)-558-6262

HAYGOOD HOMES, INC.

McLEARN RESIDENCE

Roof Loading
TC Live: 20.00 psf
TC Dead: 10.00 psf
BC Live: 0.00 psf
BC Dead: 10.00 psf
TC Stress Inc: 25.00
BC Stress Inc: 25.00

Account: CONTRACTORS
Job: HAYGOOD-MCLEARN
Designer: M.MURRAY
Checker: M.MURRAY
Date: 02-07-06

McLearn, Columbia County FL
Windload Requirements Addendum/Modification

(In Compliance with the 2004 Florida Building Code and Ammendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

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Marty J. Humphries
2-26-06

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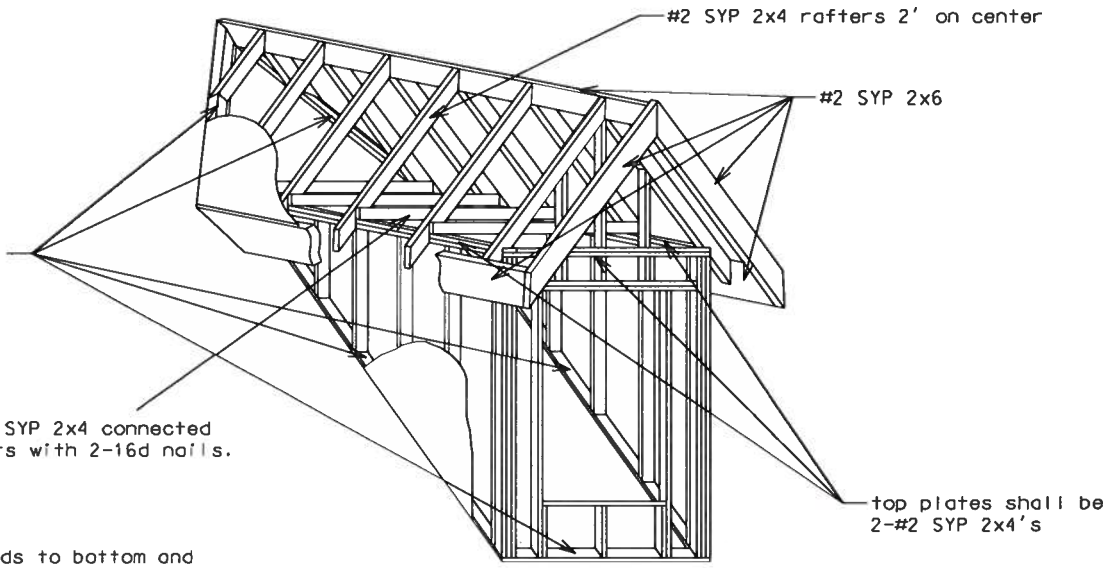
Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

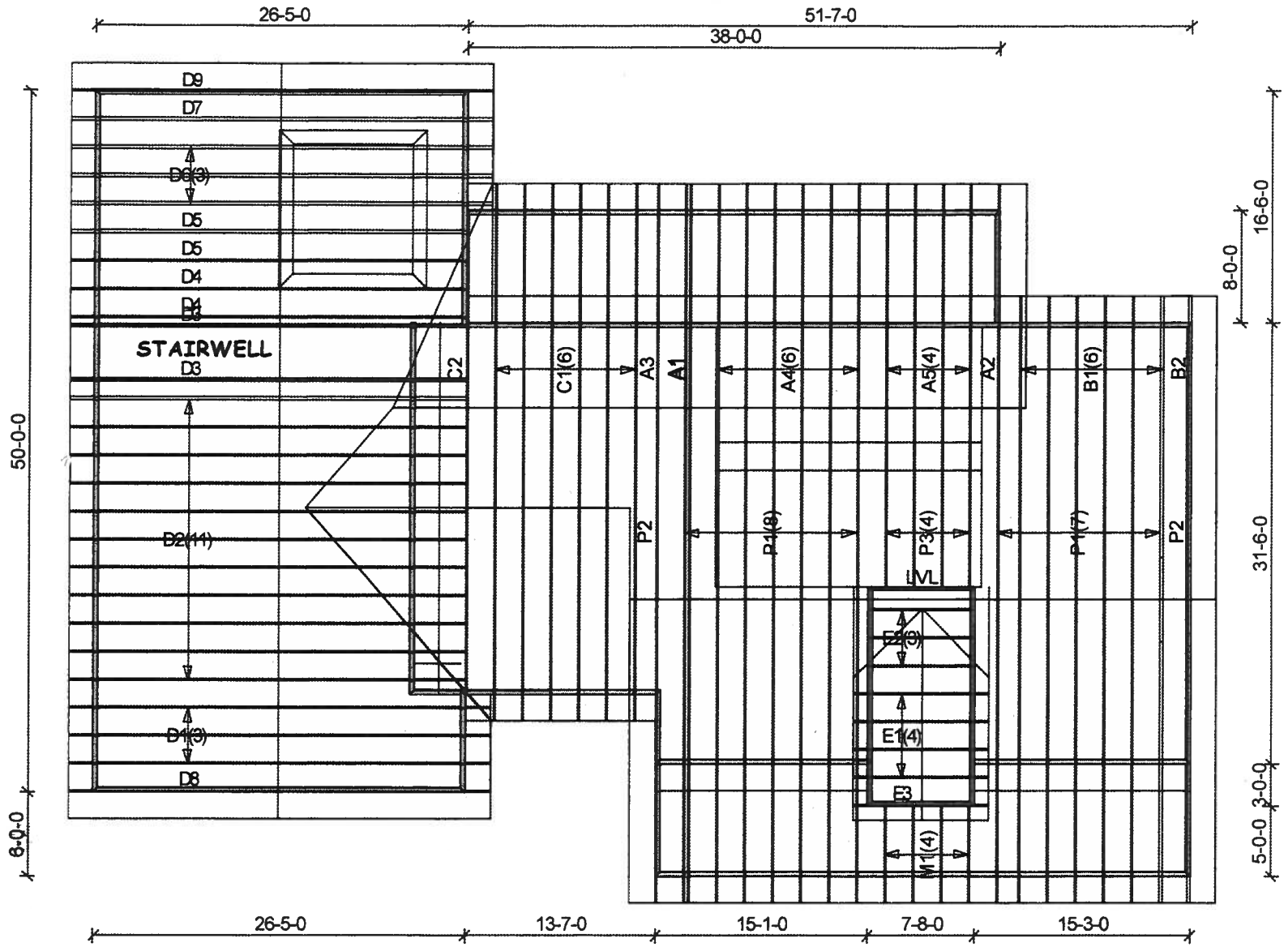
2-26-06



Note:
Connect studs to bottom and top plates with 2-16d nails and connect roof rafters to ridge plate with 2-16d nails. Also connect facia plate to rafters with 2-16d nails at each location.

DORMER FRAMING DETAIL

Monty D. Myers
3-26-06



Mayo Truss Co. Inc.

362 NE CLYDE AVE.
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(386)294-3988
(877)-558-6262

HAYGOOD HOMES, INC.

McLEARN RESIDENCE

Roof Loading
TC Live: 20.00 psf
TC Dead: 10.00 psf
BC Live: 0.00 psf
BC Dead: 10.00 psf
TC Stress Inc: 25.00
BC Stress Inc: 25.00

Account: CONTRACTORS
Job: HAYGOOD-MCLEARN
Designer: M.MURRAY
Checker: M.MURRAY
Date: 02-07-06

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0602-52

Haygood Homes Owner John Maclaren 1040 SW Old Bellamy Road

On the date of February 21, 2006 application 0602-52 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0602-52 when making reference to this application.

1. Please submit a recorded (with the Columbia County Clerk Office) a notice of commencement before any inspections can be preformed by the Columbia County Building Department.
2. Please show compliance with the FRC-2004 Section R311 Means of Egress for the stairs system that will provide egress to the bonus room. Include the total run and rise of the stairs.
3. On the conventional framing system have Mr. Humphries show the dormers design, framing layout including: Rafter size, species and spacing, attachment to wall and uplift ,ridge beam sized and valley framing and support details Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

4. In the Garage area with the bonus room above show compliance with section the FRC- 2004 the garage and residence shall be equipped with solid wood doors not less than 13/8 sectionsR309.1 Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors. Also show compliance with section R309.2 Separation required. The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.
5. Please have Mr. Humphries supply the following information, show all required connectors with uplift rating for the truss system and required number and size of fasteners for continuous tie from the roof to foundation. These connection points shall be designed by an architect or engineer using the engineered roof truss plans.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

** LAMAR BOOZER **
900 EAST PUTNAM STREET
LAKE CITY, FL 32055

PROJECT:
CLIENT: HAYGOOD HOMES (McLEARN)
DATE: 2 10 06

RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS

DESIGNER: LAMAR BOOZER

CLIENT INFORMATION:

NAME: HAYGOOD HOMES (McLEARN)
ADDRESS: .
CITY, STATE: LAKE CITY, FLORIDA 32055

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	294	9,589	0	18,808	18,808
9-I FRENCH DOOR DBL CLR GLS METL FR	84	2,851	0	6,148	6,148
12-E WALL R-11 +1/2"EXTPOLY BD(R-2.5)	2,409	8,131	0	4,808	4,808
11-C DOOR METAL POLYSTYRENE CORE	57	1,206	0	713	713
16-G CEILING R-30 INSULATION	2,694	4,662	0	4,868	4,868
22-A SLAB ON GRADE NO EDGE INSUL	289	10,535	0	0	0
SUBTOTALS FOR STRUCTURE:	5,827	36,974	0	35,345	35,345
PEOPLE	28	0	0	8,400	8,400
APPLIANCES	0	0	1,800	1,500	3,300
DUCTWORK	0	1,849	0	4,525	4,525
INFILTRATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				49,770	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		38,823	1,800	49,770	51,570

SUPPLY CFM AT 20 DEG DT: 2,262
SQUARE FT. OF ROOM AREA: 2,694

CFM PER SQUARE FOOT: 0.721
SQUARE FOOT PER TON: 730.425

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 38.823 MBH
TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 4.298 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: Series 8900
TYPE: PVC Fixed Window

Title of Test	Results
AAMA Rating	F-C50 60 x 78
Uniform Load Deflection Test Pressure	±50.0 psf
Air Infiltration	<0.01 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	±75.0 psf
Corner Weld Test	Pass
Forced Entry Resistance	Grade 40

Reference should be made to full report for test specimen description and data.

Report No: 02-46046.01
Report Date: 07/23/03
Expiration Date: 07/17/07

Architectural Testing

AAMA/WDMA 101/I.S. 2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
4661 Burbank Road, P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-46046.01
Test Date:	07/17/03
Report Date:	07/23/03
Expiration Date:	07/17/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies, to perform testing on Series 8900 PVC Fixed window. The sample tested successfully met the performance requirements for a F-C50 60 x 78 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimens were evaluated in accordance with AAMA/WDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: Series 8900

Type: PVC Fixed Window

Overall Size: 4' 11-3/4" wide by 6' 5-3/4" high

Area: 32.3 ft²

Finish: All vinyl was white.

Glazing Details: The window utilized a nominal 3/4" thick insulating glass unit fabricated from two nominal double strength sheets of annealed glass separated by a desiccant filled metal spacer system. The glass was set from the interior against a silicone sealant backbedding. PVC glazing stops were utilized on the interior.

Frame Construction: The frame corners were miter cut and welded.

Installation: The window was installed within a nominal 2" by 8" SPF wood test buck. The window was anchored to the buck with #8 by 1-5/8" wood screws spaced 6" from each corner and 8" to 10" on center. Silicone sealant was used to seal the window to the test buck.

849 Western Avenue North
Saint Paul, MN 55117-5245
phone: 651.636.3835
fax: 651.636.3843
www.archtest.com

Test Results: The results are tabulated as follows:

Paragraph	Title of Test – Test Method	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-91 (See Note #1)		
	@ 1.57 psf (25 mph)	<0.01 cfm/ft^2	0.30 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	<0.01 cfm/ft^2	--

Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/WDMA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547-00 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.1.7	Welded Corner Test	Pass	<100% break on weld
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type D		
	Grade 40		
	Lock Manipulation Test	No entry	No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00 and 331-00		
	WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3)		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 60 seconds)		
	@ 50.0 psf (positive)	0.04"	No Damage
	@ 50.0 psf (negative)	0.03"	No Damage
4.4.2	Uniform Load Structural per ASTM E 330-97		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 10 seconds)		
	@ 75.0 psf (positive)	<0.01"	0.16" max.
	@ 75.0 psf (negative)	<0.01"	0.16" max.

Note #3: The Uniform Load Deflection test is not an AAMA/WDMA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Eric J. Schoenthaler
Technician

Daniel A. Johnson
Regional Manager

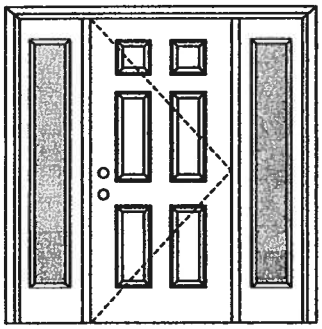
EJS/mb
02-46046.01

OXO
Opaque Inswing Unit

COP-WL-JH4104-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Design Pressure

+57.0/-57.0 with maximum sidelite panel width of 1'2"
+45.0/-45.0 with maximum sidelite panel width of 3'0"
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panels, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

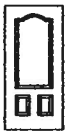
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

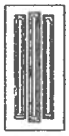
MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


Arch Top 3-panel


3-panel


6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

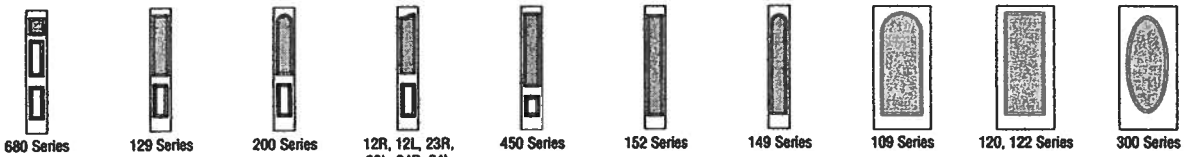
Exclusively from Masonite®
Masonite International Corporation

OXO
Opaque Inswing Unit

COP-WL-JH4104-02

WOOD-EDGE STEEL DOORS

APPROVED SIDELITE STYLES:



680 Series | 129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1905-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL-210-1880-7, 9, 10, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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June 17, 2002
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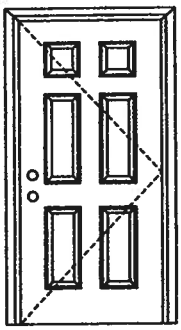
Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
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Warnock Hersey

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Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

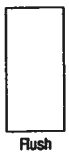
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


Arch Top 3-panel


3-panel


6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR® Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049

** LAMAR BOOZER **
300 EAST PUTNAM STREET
LAKE CITY, FL 32055

PROJECT:
CLIENT: HAYGOOD HOMES (McLEARN)
DATE: 2 10 06

RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS

DESIGNER: LAMAR BOOZER

CLIENT INFORMATION:

NAME: HAYGOOD HOMES (McLEARN)
ADDRESS:
CITY, STATE: LAKE CITY, FLORIDA 32055

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	294	9,589	0	18,808	18,808
9-I FRENCH DOOR DBL CLR GLS METL FR	84	2,851	0	6,148	6,148
12-E WALL R-11 +1/2"EXTPOLY BD(R-2.5)	2,409	8,131	0	4,808	4,808
11-C DOOR METAL POLYSTYRENE CORE	57	1,206	0	713	713
16-G CEILING R-30 INSULATION	2,694	4,662	0	4,868	4,868
22-A SLAB ON GRADE NO EDGE INSUL	289	10,535	0	0	0
SUBTOTALS FOR STRUCTURE:	5,827	36,974	0	35,345	35,345
PEOPLE	28	0	0	8,400	8,400
APPLIANCES	0	0	1,800	1,500	3,300
DUCTWORK	0	1,849	0	4,525	4,525
INFILTRATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				49,770	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		38,823	1,800	49,770	51,570

SUPPLY CFM AT 20 DEG DT: 2,262
SQUARE FT. OF ROOM AREA: 2,694

CFM PER SQUARE FOOT: 0.721
SQUARE FOOT PER TON: 730.425

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 38.823 MBH
TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 4.298 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 600B-04 — **Residential Component Prescriptive Method B** — **NORTH 1 2 3**

Compliance with Method B of Subchapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single-and multiple-family residences of three stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptives listed in this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the code.

PROJECT NAME AND ADDRESS:	maclaren	**BUILDER:** Haygood Homes, Inc	
		PERMITTING OFFICE:	**CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☑
OWNER:		**PERMIT NO.:**	**JURISDICTION NO.:**

1. New construction including additions which incorporate any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other nonvertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the code. Circle the column of the package you have choosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages," Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

	Please Print	CK
1. Compliance package chosen (A-E)	1.	
2. New construction or addition	2. new	
3. Single-family detached or multiple-family attached	3. single	
4. If multiple-family--No. of units covered by this submission	4.	
5. Is this a worst case? (yes/no)	5. yes	
6. Conditioned floor area (sq. ft.)	6. 2694	
7. Predominant eave overhang (ft.)	7. 2'	
8. Glass type and area:	Single Pane / Double Pane	
a. Clear glass	8a. ____ sq. ft. 602 sq. ft.	
b. Tint, film or solar screen	8b. ____ sq. ft. ____ sq. ft.	
9. Percentage of glass to floor area	9. 22 %	
10. Floor type, area or perimeter, and insulation:		
a. Slab-on-grade (*R*-value)	10a R= 0 266 lin. ft.	
b. Wood, raised (*R*-value)	10b. R=____ ____ sq. ft.	
c. Wood, common (*R*-value)	10c. R=____ ____ sq. ft.	
d. Concrete, raised (*R*-value)	10d. R=____ ____ sq. ft.	
e. Concrete, common (*R*-value)	10e. R=____ ____ sq. Ft.	
11. Wall type, area and insulation:		
a. Exterior: 1. Masonry (Insulation R-value)	11a-1 R=____ ____ sq. ft.	
a. Exterior: 2. Wood frame (Insulation *R*-value)	11a-2 R= 13 2394 sq. ft.	
b. Adjacent: 1. Masonry (Insulation *R*-value)	11b-1 R=____ ____ sq. ft.	
b. Adjacent: 2. Wood frame (Insulation *R*-value)	11b-2 R=____ ____ sq. ft.	
12. Ceiling type, area and insulation:		
a. Under attic (Insulation *R*-value)	12a. R= 30 sq. ft. 2694	
b. Single assembly (Insulation *R*-value)	12b. R=____ ____ sq. ft.	
13. Air distribution system: Duct insulation, location	13. R= 6 ____	
Test report (attach if required)		
14. Cooling system:	14a. Type: central	
(Types: central, room unit, package terminal A.C., gas, none)	14b. SEER/EER: 13	
	14c. Capacity: 4 ton	
15. Heating system:	15a. Type: Heat Pump	
(Types: heat pump, elec. strip, nat. gas, LP-Gas, gas h.p., room or PTAC, none)	15b. HSPF/COP/AFUE: ____	
16. Hot water system:	15c. Capacity: 50 gal	
(Types: elec., nat. gas, LP-gas, solar, heat rec., ded. heat pump, other, none)	16a. Type: Elect	
	16b. EF: .88	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: Brenda Haygood DATE: 2-10-06

I hereby certify that this building is in compliance with the Florida Energy Code:

OWNER AGENT: _____ DATE: 2-10-06

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

TABLE 6B-1 — MINIMUM REQUIREMENTS — Climate Zones 1 2 3

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION					TO BE INSTALLED
		A	B	C	D	E	
GLASS	Max. % of Glass to Floor Area	15%	15%	20%	20%	25%	15 %
GLASS	Type	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Tint (DT)	DC: ☐ DT: ☐
GLASS	Overhang	1'4"	2'	2'	2'	2'	2 FEET
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE.					EXT: R = ADJ: R = COM: R =
WALLS	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD-FRAME WALLS R-11					EXT: R = 13 ADJ: R = COM: R =
CEILINGS		R-30	R-30	R-30	R-30	R-30	UNDER ATTIC: R= 30 COMMON: R:
		(NO SINGLE ASSEMBLY CEILINGS ALLOWED)					
FLOORS	Slab-On-Grade	R-0					R = 0
FLOORS	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)					R =
FLOORS	Raised Concrete	R-7					R = 6 6
DUCTS		R-6	R-6	R-6, TESTED	R-6	R-6, TESTED	R = 6 COND. ☐
SPACE COOLING (SEER)		12.0	10.5	12.0	11.0	12.0	SEER = 13
HEAT	Elect. (HSPF)	7.9	7.1	7.4	7.4	7.4	HSPF =
HEAT	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)					AFUE =
HOT WATER SYSTEM	Electric Resistance**	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	EF = .88
HOT WATER SYSTEM	Gas & Oil**	MINIMUM EF OF .59				NATURAL GAS ONLY (SEE BELOW)	EF =
HOT WATER SYSTEM	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.					DHP: ☐ HRU: ☐ SOLAR: ☐ EF =

* Single package units minimum SEER=9.7, HSPF = 6.6.
** Minimum efficiencies for gas and electric hot water systems apply to 40 gallon water heaters. Refer to Table 612.1 ABC.3.2 for minimum code efficiencies for oil water heaters and other sizes.

DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions: 1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multistory house.

Wall, Ceiling and Floor Insulation Values: The *R*-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The *R*-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multiple-family building. "Adjacent" components separate conditioned space from unconditioned but enclosed space. "Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a state-approved tester.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTACs equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of Section 608.1.ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed," an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options." See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having and EF of .92 or greater, or natural gas systems with EF .59 or greater may be used in conjunction with these systems.

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Multistory Houses	606.1	Air barrier on perimeter of floor cavity between floors.	✓
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker electric or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Noncommercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	NA
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	NA
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 60 psig.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
~~XXXNORTH FIRSTXSTREETX~~
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8500
TYPE: PVC Single Hung Window

Title of Test	Results
AAMA/WDMA Rating	H-R40 (44 x 84)
Uniform Load Deflection Test Pressure	± 40.0 psf
Operating Force	10 lbs max.
Air Infiltration	0.21 cfm/ft^2
Water Resistance Test Pressure	6.00 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48976.02
Report Date: 02-26-04
Expiration Date: 02-25-08

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.archtest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-48976.02
Test Date:	02/25/04
Report Date:	02/26/04
Expiration Date:	02/25/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8500 Single Hung Window. The sample tested successfully met the performance requirements for a H-R40 44 x 84 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8500

Type: PVC Single Hung Window

Overall Size: 3' 8" wide by 7' 0" high

Sash Size: 3' 4-3/8" wide by 2' 5" high

Fixed D.L.O. Size: 3' 4-3/4" wide by 4' 5" high

Screen Size: 3' 4-3/4" wide by 2' 4-1/4" high

Finish: All PVC was white

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.archtest.com

Test Specimen Description: (Continued)

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two single-strength annealed sheets in the operating sash and two double-strength sheets in the fixed lite and a desiccant-filled metal spacer system. The glass for the fixed area was set from the interior into a bed of silicone sealant with PVC stops used on the interior. The sash was glazed from the exterior into a bed of silicone sealant with PVC stops used on the exterior.

Weatherstripping:

Description	Quantity	Location
0.260" high by 0.187" backed pile with center fin	1 Row	Sash top and bottom rails
0.260" high by 0.187" backed pile with center fin	2 Rows	Sash stiles

Frame Construction: Frame corners were miter-cut and welded. Aluminum reinforcement was utilized in the fixed meeting rail (Jordan part number H-2447).

Sash Construction: Sash corners were miter-cut and welded. Aluminum reinforcement was utilized in the top rail (Jordan part number H-2448).

Hardware:

Metal cam locks with keepers	2	6" from ends and meeting rail
Plastic tilt latches	2	Sash top rail corners
Metal tilt pins	2	Sash bottom rail corners
Block-and-tackle balances	2	One per jamb

Drainage:

3/16" by 5/8" slots	2	1-3/4" from ends in sill pocket to hollow below
1/8" by 1/2" slots	4	1-3/4" and 2" from each end through sill exterior face

Installation: The unit was installed into a Grade 2 SPF 2" by 8" wood test buck secured through the flange with 1-5/8" screws spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results: The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.2.1.6.1	Operating Force		
	Force to initiate motion	10 lbs	30 lbs max.
	Force to keep in motion	8 lbs	30 lbs max.
2.1.2	Air Infiltration per ASTM E 283-97 (See Note #1)		
	@ 1.57 psf (25 mph)	0.21 cfm/ft^2	0.30 cfm/ft^2

Note #1: The tested specimen meets the performance levels specified in AAMA/WDMA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction @ 70 lbs		
	Top rail	0.04"/ 8%	0.500"/100%
	Bottom rail	0.06"/12%	0.500"/100%
	In remaining direction @ 50 lbs		
	Left stile	0.04"/8%	0.500"/100%
	Right stile	0.03"/6%	0.500"/100%
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type A		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
Optional Performance:			
4.3	Water Resistance per ASTM E 547-97 WTP = 6.00 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the meeting rail) (Loads were held for 60 seconds)		
	@ 40.0 psf (positive)	0.45"	(See Note #3)
	@ 40.0 psf (negative)	0.52"	(See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 60.0 psf (positive)	0.03"	0.16" max.
	@ 60.0 psf (negative)	0.03"	0.16" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

DAJ/jb
02-48976.02

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: Series 8900
TYPE: PVC Fixed Window

Title of Test	Results
AAMA Rating	F-C50 60 x 78
Uniform Load Deflection Test Pressure	±50.0 psf
Air Infiltration	<0.01 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	±75.0 psf
Corner Weld Test	Pass
Forced Entry Resistance	Grade 40

Reference should be made to full report for test specimen description and data.

Report No: 02-46046.01
Report Date: 07/23/03
Expiration Date: 07/17/07

Architectural Testing

AAMA/WDMA 101/I.S. 2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
4661 Burbank Road, P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-46046.01
Test Date:	07/17/03
Report Date:	07/23/03
Expiration Date:	07/17/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies, to perform testing on Series 8900 PVC Fixed window. The sample tested successfully met the performance requirements for a F-C50 60 x 78 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimens were evaluated in accordance with AAMA/WDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: Series 8900

Type: PVC Fixed Window

Overall Size: 4' 11-3/4" wide by 6' 5-3/4" high

Area: 32.3 ft²

Finish: All vinyl was white.

Glazing Details: The window utilized a nominal 3/4" thick insulating glass unit fabricated from two nominal double strength sheets of annealed glass separated by a desiccant filled metal spacer system. The glass was set from the interior against a silicone sealant backbedding. PVC glazing stops were utilized on the interior.

Frame Construction: The frame corners were miter cut and welded.

Installation: The window was installed within a nominal 2" by 8" SPF wood test buck. The window was anchored to the buck with #8 by 1-5/8" wood screws spaced 6" from each corner and 8" to 10" on center. Silicone sealant was used to seal the window to the test buck.

849 Western Avenue North
Saint Paul, MN 55117-5245
phone: 651.636.3835
fax: 651.636.3843
www.archtest.com

Test Results: The results are tabulated as follows:

Paragraph	Title of Test – Test Method	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-91 (See Note #1)		
	@ 1.57 psf (25 mph)	<0.01 cfm/ft^2	0.30 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	<0.01 cfm/ft^2	--

Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/WDMA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547-00 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.1.7	Welded Corner Test	Pass	<100% break on weld
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type D		
	Grade 40		
	Lock Manipulation Test	No entry	No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00 and 331-00		
	WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3)		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 60 seconds)		
	@ 50.0 psf (positive)	0.04"	No Damage
	@ 50.0 psf (negative)	0.03"	No Damage
4.4.2	Uniform Load Structural per ASTM E 330-97		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 10 seconds)		
	@ 75.0 psf (positive)	<0.01"	0.16" max.
	@ 75.0 psf (negative)	<0.01"	0.16" max.

Note #3: The Uniform Load Deflection test is not an AAMA/WDMA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Eric J. Schoenthaler
Technician

Daniel A. Johnson
Regional Manager

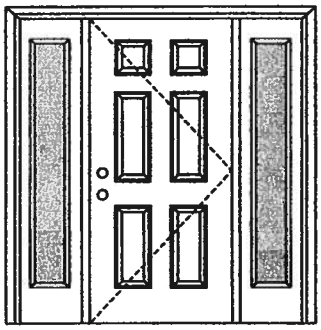
EJS/mb
02-46046.01

OXO
Opaque Inswing Unit

COP-WL-JH4104-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure

+57.0/-57.0 with maximum sidelite panel width of 1'2"
+45.0/-45.0 with maximum sidelite panel width of 3'0"
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panels, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

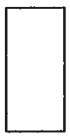
MINIMUM ASSEMBLY DETAIL:

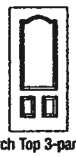
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

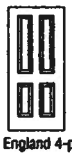
APPROVED DOOR STYLES:


Flush


Arch Top 3-panel


3-panel


6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

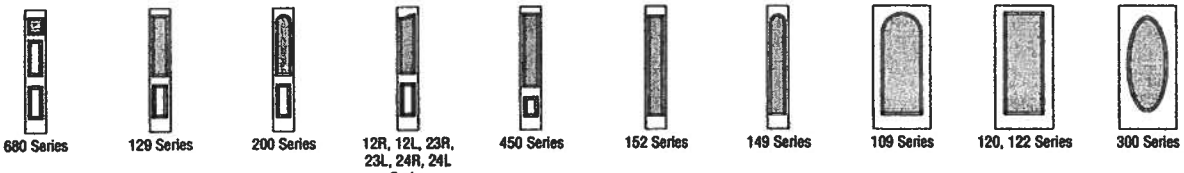
Exclusively from Masonite®
Masonite International Corporation

OXO
Opaque Inswing Unit

COP-WL-JH4104-02

WOOD-EDGE STEEL DOORS

APPROVED SIDELITE STYLES:



680 Series | 129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1905-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL-210-1880-7, 9, 10, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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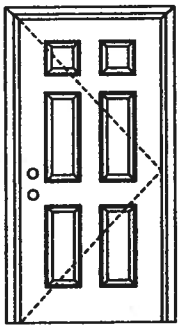
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush

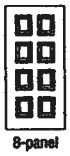

Arch Top 3-panel


3-panel


6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
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PREMDOR Collection Premium Quality Doors

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Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
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PREMDOR® Collection Premium Quality Doors

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Masonite International Corporation

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8500
TYPE: PVC Single Hung Window

Title of Test	Results
AAMA/WDMA Rating	H-R40 (44 x 84)
Uniform Load Deflection Test Pressure	± 40.0 psf
Operating Force	10 lbs max.
Air Infiltration	0.21 cfm/ft^2
Water Resistance Test Pressure	6.00 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48976.02
Report Date: 02-26-04
Expiration Date: 02-25-08

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.archtest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-48976.02
Test Date:	02/25/04
Report Date:	02/26/04
Expiration Date:	02/25/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8500 Single Hung Window. The sample tested successfully met the performance requirements for a H-R40 44 x 84 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8500

Type: PVC Single Hung Window

Overall Size: 3' 8" wide by 7' 0" high

Sash Size: 3' 4-3/8" wide by 2' 5" high

Fixed D.L.O. Size: 3' 4-3/4" wide by 4' 5" high

Screen Size: 3' 4-3/4" wide by 2' 4-1/4" high

Finish: All PVC was white

849 Western Avenue North
Saint Paul, Minnesota 55117-5245
phone: 651.636.3835
fax: 652.636.3843
www.architest.com

Test Specimen Description: (Continued)

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two single-strength annealed sheets in the operating sash and two double-strength sheets in the fixed lite and a desiccant-filled metal spacer system. The glass for the fixed area was set from the interior into a bed of silicone sealant with PVC stops used on the interior. The sash was glazed from the exterior into a bed of silicone sealant with PVC stops used on the exterior.

Weatherstripping:

Description	Quantity	Location
0.260" high by 0.187" backed pile with center fin	1 Row	Sash top and bottom rails
0.260" high by 0.187" backed pile with center fin	2 Rows	Sash stiles

Frame Construction: Frame corners were miter-cut and welded. Aluminum reinforcement was utilized in the fixed meeting rail (Jordan part number H-2447).

Sash Construction: Sash corners were miter-cut and welded. Aluminum reinforcement was utilized in the top rail (Jordan part number H-2448).

Hardware:

Metal cam locks with keepers	2	6" from ends and meeting rail
Plastic tilt latches	2	Sash top rail corners
Metal tilt pins	2	Sash bottom rail corners
Block-and-tackle balances	2	One per jamb

Drainage:

3/16" by 5/8" slots	2	1-3/4" from ends in sill pocket to hollow below
1/8" by 1/2" slots	4	1-3/4" and 2" from each end through sill exterior face

Installation: The unit was installed into a Grade 2 SPF 2" by 8" wood test buck secured through the flange with 1-5/8" screws spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results: The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.2.1.6.1	Operating Force		
	Force to initiate motion	10 lbs	30 lbs max.
	Force to keep in motion	8 lbs	30 lbs max.
2.1.2	Air Infiltration per ASTM E 283-97 (See Note #1)		
	@ 1.57 psf (25 mph)	0.21 cfm/ft^2	0.30 cfm/ft^2

Note #1: The tested specimen meets the performance levels specified in AAMA/WDMA 101/I.S.2-97 for air infiltration.

2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction @ 70 lbs		
	Top rail	0.04"/ 8%	0.500"/100%
	Bottom rail	0.06"/12%	0.500"/100%
	In remaining direction @ 50 lbs		
	Left stile	0.04"/8%	0.500"/100%
	Right stile	0.03"/6%	0.500"/100%
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type A		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
	Optional Performance:		
4.3	Water Resistance per ASTM E 547-97 WTP = 6.00 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the meeting rail) (Loads were held for 60 seconds)		
	@ 40.0 psf (positive)	0.45"	(See Note #3)
	@ 40.0 psf (negative)	0.52"	(See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 60.0 psf (positive)	0.03"	0.16" max.
	@ 60.0 psf (negative)	0.03"	0.16" max.

***Note #3**: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

DAJ/jb
02-48976.02

McLearn Residence Columbia County FL

Wind Load Analysis Requirements

(In Compliance with the 2004 Florida Building Code and Ammendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

Description of New Residence:

Footprint: 56' x 78' "L" shaped home with covered front and rear porches
(see plan 0601 by Haygood Homes)
Walls: 2x4-16" O.C. with 7/16" OSB sheathing minimum, hardiboard lap siding and ½"gypsum wall board interior.
Roof Structure: Pre-engineered roof trusses and 1/2" OSB sheathing
Roof Type: Gable construction (analyzed for 2' eave overhang and porch areas)
Foundation: footer with stemwall, with slab construction

Windload Data and Exposure:

Basic Wind Speed = 110 mph
Importance Factor = 1.0
Exposure category = B
Height and Exposure Adjustment Coefficient = 1.0
Residential Occupancy = Group R3
Analysis Method = FBC 1609.6 - Simplified Provisions for Low Rise Buildings
(see tables 1609.6A, 1609.6B, 1609.6C and 1609.6E for wind pressure values)
Mean roof height = 17' 6"
Roof Cross Slope = 8:12
Eave Overhang= (Analyzed for 2' overhang and porches)
Wall Height = 9'
Shear Wall locations = exterior walls only(all walls 3' in length or greater)
Bracing method for gable locations = framing from wall to roof diaphragm(see attached detail)

Nailing Pattern Requirements:

Wall sheathing:	Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center around edges(including around doors and windows) and 6" on center interior. Full depth blocking shall be installed At horizontal joints in sheathing.
Roof sheathing:	Shall be 1/2" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center at panel ends and eave overhang areas and 6" on center elsewhere.
Top wall plate:	Nail with 1-16d common nail 12" O.C.(average)

Marty J. Humphries
2-6-06

Strapping and Anchor Requirements:

truss to exterior wall plate and porch beam locations:	install one Simpson model H10 hurricane anchor at each location. At end-gable locations install an additional Simpson model H2.5A at the first 5 trusses. At double plated trusses install 2-H11Z anchors.
wall strap tie requirements:	At top and bottom of wall install one Simpson model SP4 at each side of each door or window under 4' or less in width. For doors or windows greater than 4' in width install 2-SP4's each side top and bottom. For garage door openings install 2-SPH4's top and bottom each side. All other wall locations install SP4's top and bottom of wall 4' on center.
Porch Columns:	Install Simpson model ABU44, ABU46 or ABU66 and Simpson model ACE4Max or ACE6Max

Lookouts: Install one Simpson model H5 where lookouts connect to end gable truss.

Gable end: Install one LSTA18 - 4' on center connecting gable end truss to wall framing.

Gable End Bracing Requirements:

At each gable end install one 2x4 SPF 8' stud spaced 6' on center horizontal along top of bottom chord of trusses, nail with 2-12d nails at each truss including end truss. In addition, install a 2x4 brace extending from this stud at the gable end truss approx. 45 degrees to truss at roof sheathing, nail with 2 -12d nails where it crosses truss members and at ends. Gable end trusses shall be built to receive sheathing with vertical members 2' on center. Vertical members of gable end truss greater than 5' in height shall be stiffened with one 2x4 SPF nailed with 12d nails 8" on center to back of vertical member. (See attached detail)

Foundation Requirements:

Stemwall: Minimum size of footer shall be 10" x 20" wide with 2-#5 rebar continuous and 1-#5 vertical rebar 48" on center. All cells shall be filled with concrete. ½" anchor bolts with 2" washers shall be installed 3' on center and 9" from corners each way and at each side of door openings. (3000 psi concrete min.)(Note: foundation designed using an allowable bearing pressure of 1000 psf)

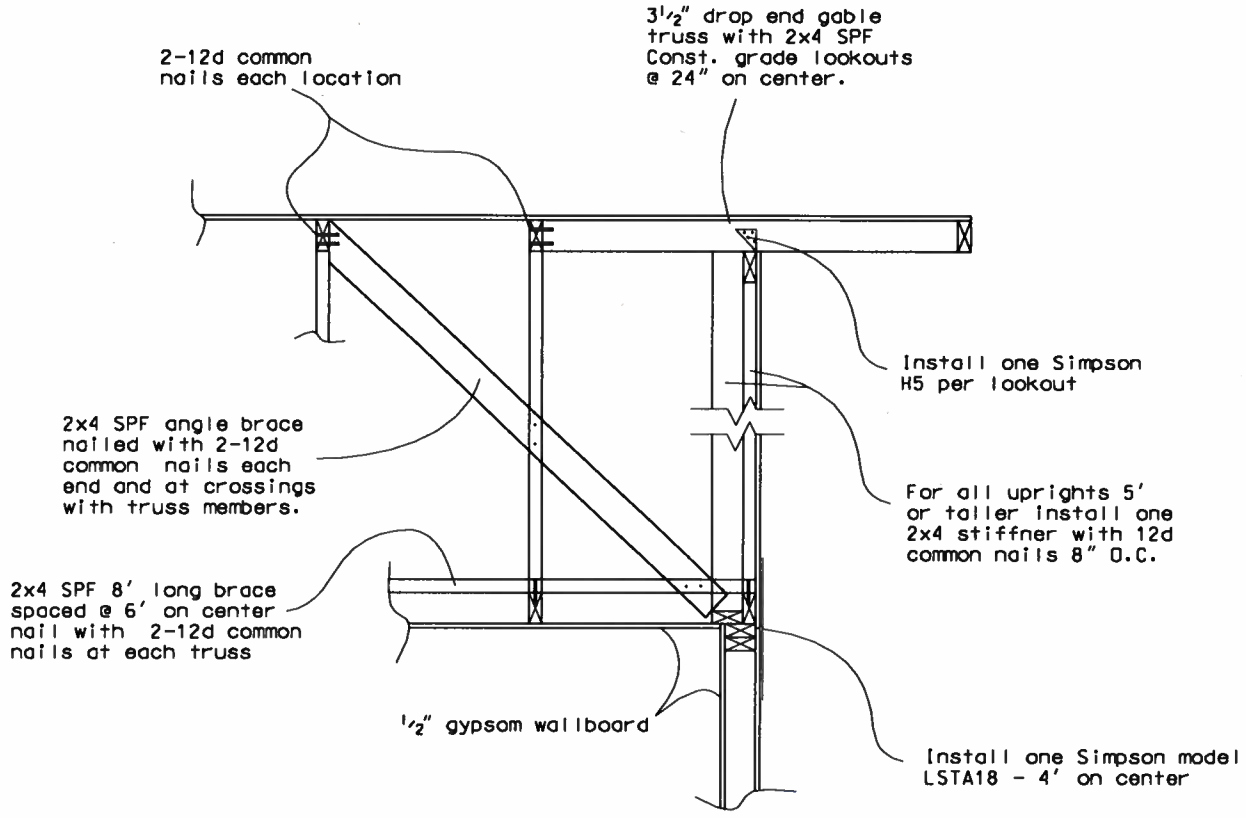
Header Requirements:

Windows, Doors & Porch Beams: Header shall be 2 - #2 SYP 2x10's with ½" plywood/OSB between. .

Garage Door Beams: Header shall be 2-#2 SYP 2x12's with ½" plywood/OSB between

Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

Marty D. Hug
2-6-06



GABLE END BRACING DETAIL (N.T.S.)

Marty J. Humphries
2-6-06

McLearn Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071

NEW! The H2.5A is symetrically designed for easy installation, with higher uplift loads to meet new code requirements. A placement mark allows easy installation on double top plates.

NEW! The H5A has an installed cost benefit, as it only requires 6 nails, to meet lower uplift requirements.

The H connector series provides wind and seismic ties for trusses and rafters.

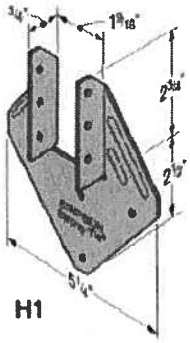
Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
Design Shear/Allowable Shear + Design Tension/Allowable Tension < 1.0

MATERIAL: See table

FINISH: Galvanized; H10-2, H11Z-Z-MAX. Other models available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- H1 can be installed with flanges facing outwards (reverse of drawing number 1). When installed inside a wall, a birdsmouth cut is required.
- H2.5, H3, H4, H5 and H6 ties are shipped in equal quantities of rights and lefts.
- Bend the H7 over the top of the truss. Install a minimum of four 8d nails into the truss, including two into the truss side.
- Hurricane Ties do not replace solid blocking.

CODES: BOCA, ICBO, SBCCI NER-422, NER-393, NER-432; NER-499; City of L.A. RR 24818; Dade Co, FL 00-0512.11 (H10); 00-0926.01 (H1, H2.5, H3, H4, H5)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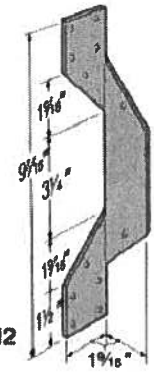
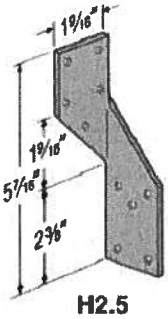
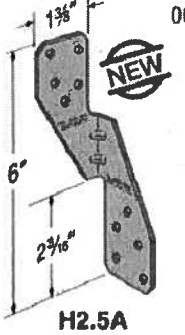
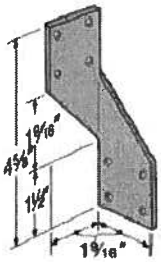

H2


H2.5



H2.5A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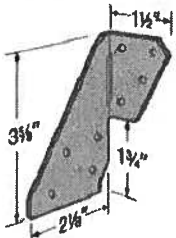
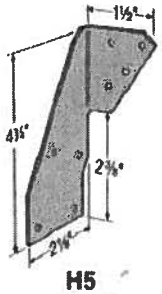

H4
U.S. Patent 4,714,372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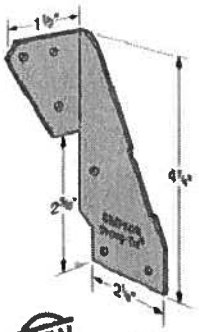
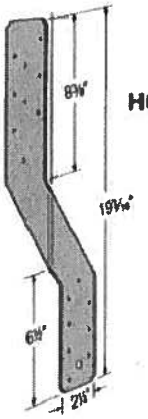
U.S. Patent 4,714,372



H5A
U.S. Patent 4,714,372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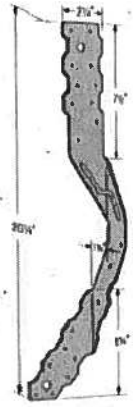
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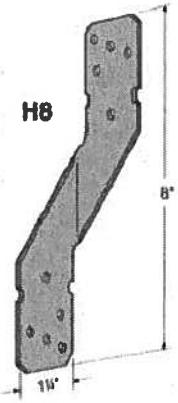
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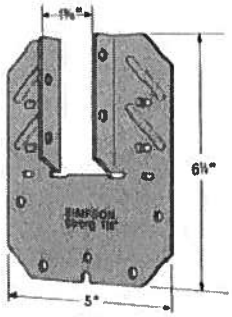
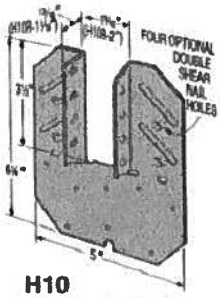
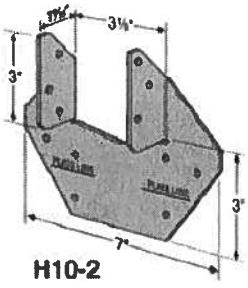
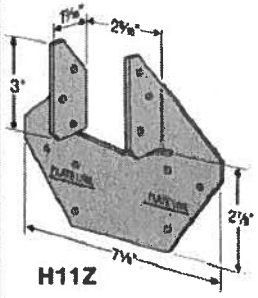

H9



H10
(H10R similar)
U.S. Patents 4,480,941; Canada Patent 1,193,418

Model No.	Ga	Fasteners: To Rafters/ Truss	Fasteners: To Plates	Fasteners: To Studs	Uplift Avg Ult	Doug-Fir Larch/So. Pine Allowable Loads[1,2]: Uplift (133)	Uplift (160)	Lateral (133/160) F_1	Lateral (133/160) F_2	Uplift Load with 8dx1½" Nails (133 & 160)	Spruce-Pine-Fir Allowable Loads[1,2]: Uplift (133)	Uplift (160)	Lateral (133/160) F_1	Lateral (133/160) F_2	Uplift Load with 8dx1½" Nails (133 & 160)
H1	18	6-8dx1½	4-8d	—	1958	490	585	485	165	455	400	400	415	140	370
H2	18	5-8d	—	5-8d	1040	335	335	—	—	335	230	230	—	—	230
H2.5	18	5-8d	5-8d	—	1300	415	415	150	150	415	365	365	130	130	365
NEW H2.5A	18	5-8d	5-8d	—	1793	600	600	110	110	480	520	535	110	110	480
H3	18	4-8d	4-8d	—	1433	455	455	125	160	415	320	320	105	140	290
H4	20	4-8d	4-8d	—	1144	360	360	165	160	360	235	235	140	135	235
H5	18	4-8d	4-8d	—	1485	455	465	115	200	455	265	265	100	170	265
NEW H5A	18	3-8d	3-8d	—	1500	350	420	115	180	280	245	245	100	120	170
H6	16	—	8-8d	8-8d	3983	915	950	650	—	—	785	820	560	—	—
H7	16	4-8d	2-8d	8-8d	2991	930	985	400	—	—	800	845	345	—	—
H8	18	5-10dx1½	5-10dx1½	—	2422	620	745	—	—	—	530	565	—	—	—
H9KT	18	4-SDS¼x1½	5-SDS¼x1½	—	2812	875	875	680	125	—	755	755	680	125	—
H10	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10R	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10-2	18	6-10d	6-10d	—	2447	760	760	455	395	—	655	655	390	340	—
H11Z	18	6-16dx2½	6-16dx2½	—	5097	830	830	525	760	—	715	715	450	655	—

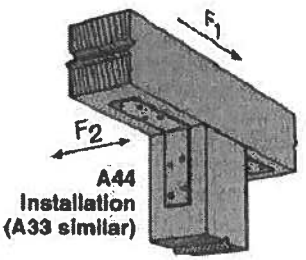

H10-2


H11Z

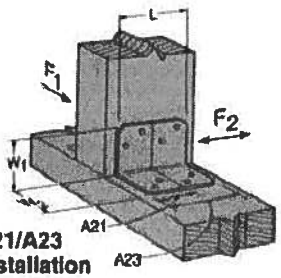
1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5); 390 lbs (H2.5A); 360 lbs (H4) and 310 lbs (H8).
4. The H9KT is sold in 20 piece packs with screws.
5. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
6. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

Z2 clips secure 2x4 flat blocking between joists or trusses to support sheathing.
MATERIAL: Z clips–see table. A21 and A23–18 ga.; all other A angles–12 ga.
FINISH: Galvanized
INSTALLATION: • Use all specified fasteners. See General Notes.
• Z clips do not provide lateral stability. Do not walk on stiffeners or apply load until diaphragm is installed and nailed to stiffeners.
CODES: BOCA, ICBO, SBCCI NER-421 (except A33, A44); City of L.A. RR 25076 (except A33, A44); Dade Co. FL 99-0623.04 (A21 and A23).

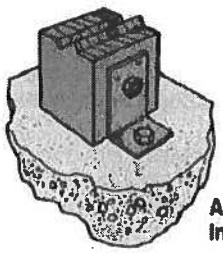
Model No.	Dimensions			Fasteners				Avg Ult F_2	Allowable Loads[1] DF/SP			
				Base		Post			(133)		(160)	
	W_1	W_2	L	Bolts	Nails	Bolts	Nails		F_1	F_2	F_1	F_2
A21	2	1½	1⅜	—	2-10dx1½	—	2-10dx1½	540	245	175	290	175
A23	2	1½	2¾	—	4-10dx1½	—	4-10dx1½	1767	485	485	585	565
A33	3	3	1½	—	4-10d	—	4-10d	2635	625	330	750	330
A44	4⅝	4⅜	1½	—	4-10d	—	4-10d	2490	625	295	750	295
A66	5⅞	5⅞	1½	2-⅞	—	2-⅞	—	N/A	N/A	N/A	N/A	N/A
A88	8	8	2	3-⅞	—	3-⅞	—	N/A	N/A	N/A	N/A	N/A
A24	3⅞	2	2½	1-½	—	1-½	2-10d	N/A	N/A	N/A	N/A	N/A
A311	11	3⅝	2	1-½	—	1-½	4-10d	N/A	N/A	N/A	N/A	N/A

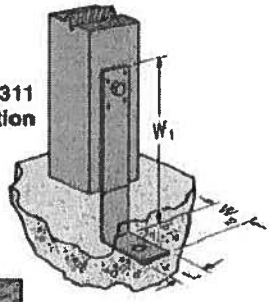


A44 Installation (A33 similar)



A21/A23 Installation

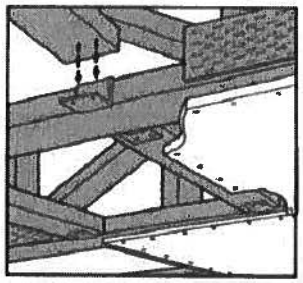

A24 Installation

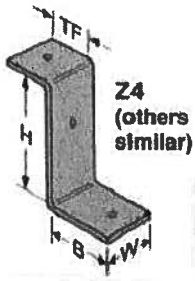


A311 Installation

Model No.	Ga	Dimensions				Fasteners[1] (Total)	Avg Ult	Allowable[2] Download (125)
		W	H	B	TF			
Z2	20	2⅝	1½	1⅜	1⅜	4-10dx1½	1507	465
Z4	12	1½	3½	2⅛	1¾	2-16d	1450	465
Z6	12	1½	5½	2	1⅜	2-16d	1517	485
Z28	28	2⅝	1½	1⅜	1⅜	10dx1½[3]	—	—
Z38	28	2⅝	2½	1⅜	1⅜	10dx1½[3]	—	—
Z44	12	2½	3½	2	1⅜	4-16d	2800	865

1. Z28 and Z38 do not have nail holes. Fastener quantities are as required.
2. Allowable loads have been increased 25% for roof loading (Z clips), 33% and 60% for earthquake or wind loading (A angles); no further increase allowed; reduce for other load durations according to the code.
3. Z4 and Z6 loads apply with a nail into the top and a nail into the seat.


Typical Z2 Installation

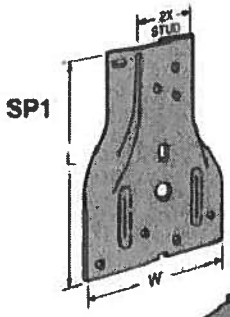


Z4 (others similar)

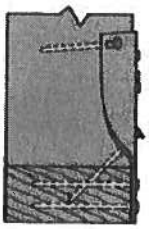
SP/SPH/RSP4 STUD PLATE TIES

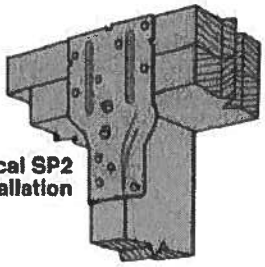
The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.
MATERIAL: SPH–18 gauge, all others–20 gauge **FINISH:** Galvanized
INSTALLATION: • Use all specified fasteners; see General Notes.
• SP–one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.
CODES: BOCA, ICBO, SBCCI NER-432, NER-443, NER-499; SBCCI 9603A; City of LA RR 25318 (RSP4); Dade Co. FL 99-0623.04 (SP1, SP2, SP4, SP6, SP8).

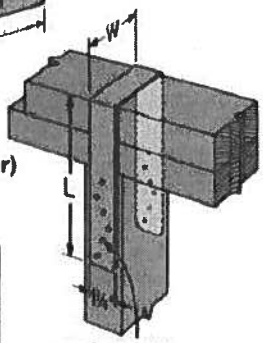
Model No.	Dimensions		Fasteners		Avg Ult	Allowable Uplift Loads DF/SP	
	W	L	Stud[1]	Plate		(133)[2]	(160)[2]
SP1	3½	5⅛	6-10d	4-10d	1950	585	585
SP2	3½	6⅝	6-10d	6-10d	3300	890	1065
SP3	4½	6⅝	6-10d	6-10d	3467	890	1065
SP4	3⅛	7¼	6-10dx1½	—	2917	735	885
SP5	4½	5⅛	6-10d	4-10d	1950	585	585
SP6	5⅛	7¼	6-10dx1½	—	2917	735	885
SP8	7⅛	8⅛	6-10dx1½	—	2917	735	885
SPH4	3⅛	8¼	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH6	5⅛	9¼	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH8	7⅛	8⅛	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
RSP4 (1)	2⅛	4½	4-8dx1½	4-8dx1½	1032	315	315
RSP4 (2)	2⅛	4½	4-8dx1½	4-8dx1½	1445	450	450



SP1

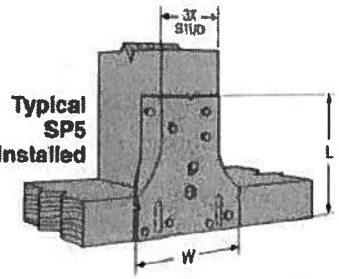

SP1 Nailing Profile


Typical SP2 Installation

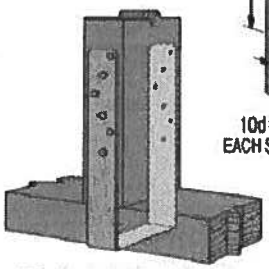


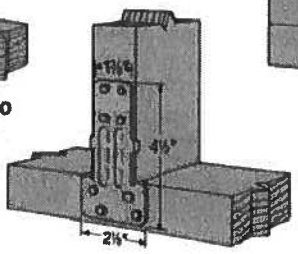
Typical SPH Installation (SP4, 6, 8 similar)

10d×1½" NAILS EACH SIDE OF STUD

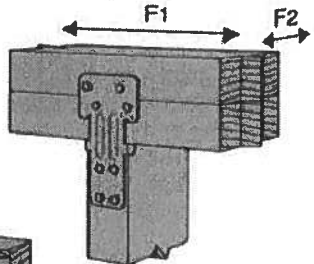


Typical SP5 Installed


Typical SPH4 Stud to Single Bottom Plate



(1) Typical RSP4 Stud to Single Bottom Plate



(2) Typical RSP4 Stud to Double Top Plate (see footnote 4)

1. SP1, 2, 3 and SP5: drive one stud nail at an angle through the stud into the plate to achieve the table load (see illustration).
2. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
3. RSP4–see installation details (1) and (2) for reference.
4. RSP4 F2 is 280 lbs (installation 1) and 305 lbs (installation 2). F1 load is 210 lbs for both installations.
5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.
6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement

U.S. Patent 5,697,725

RPS/ST/FHA/PS/HST/LSTA/LSTI/MST/MSTA/MSTC/MSTI

STRAP TIES

SIMPSON Strong-Tie CONNECTORS

The MSTC series has countersunk nail slots for a lower nailing profile. Coined edges ensure safer handling. The RPS meets UBC and City of Los Angeles code requirements for notching plates where plumbing, heating or other pipes are placed in partitions.

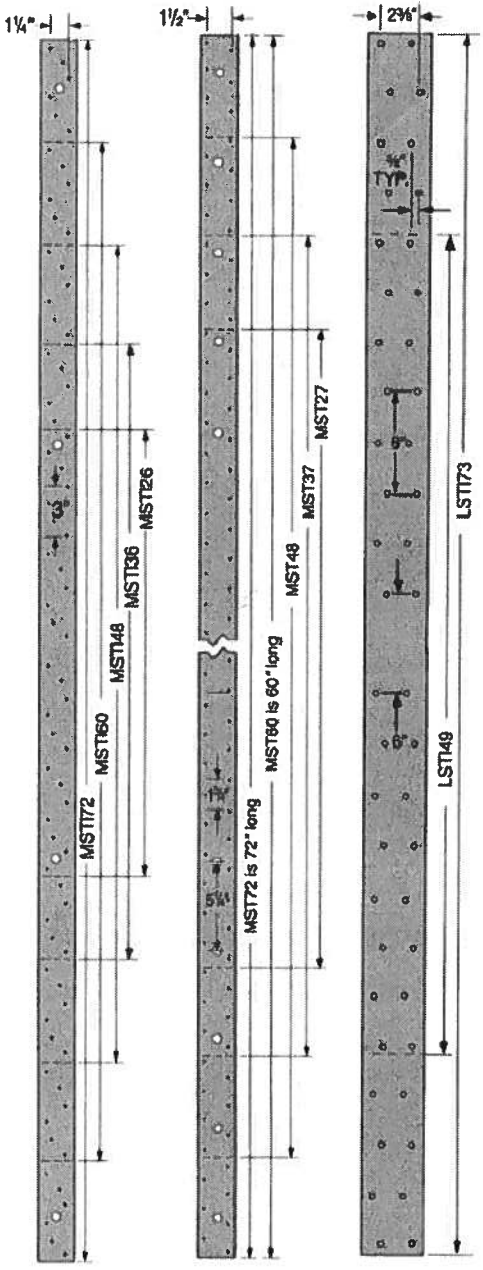
Install Strap Ties where plates or soles are cut, at wall intersections, and as ridge ties. LSTA and MSTA straps are engineered for use on 1½" members. The 3" center-to-center nail spacing reduces the possibility of splitting. For the MST, this may be a problem on lumber narrower than 3½"; either fill every nail hole with 10dx1½" nails or fill every other nail hole with 16d commons. Reduce the allowable load based on the size and quantity of fasteners used. The LSTI light strap ties are suitable where gun-nailing is necessary through diaphragm decking and wood chord open web trusses.

FINISH: HST–Simpson gray paint; PS–HDG; all others–galvanized. Some products are available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION: Use all specified fasteners. See General Notes.

OPTIONS: Special sizes can be made to order. See also HCST.

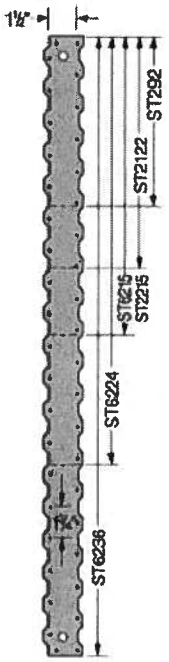
CODES: BOCA, ICBO, SBCCI NER-413, NER-443; ICBO 4935, 5357; Dade County, FL. 00-1023.05 (MSTA30, MSTA36, ST12, ST18, ST22); City of L.A. RR 25119, RR 25149, RR 25281.



MSTI

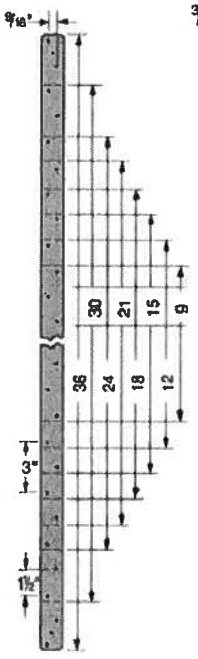
MST

LSTI

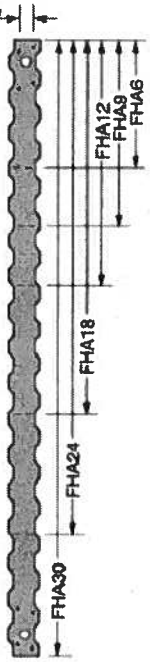


ST
U.S. Patent 4,367,973

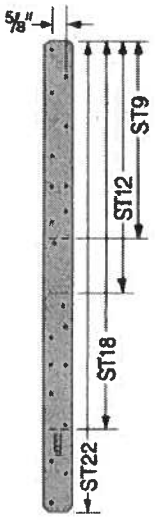
MSTAM36



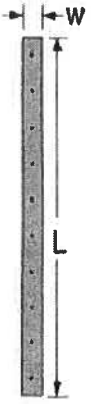
LSTA and MSTA
(pilot holes not shown)



FHA

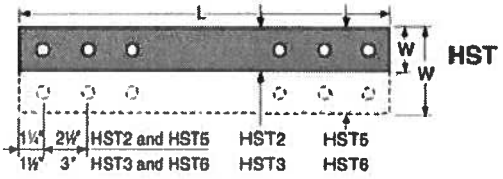


ST9, ST12, ST18, ST22

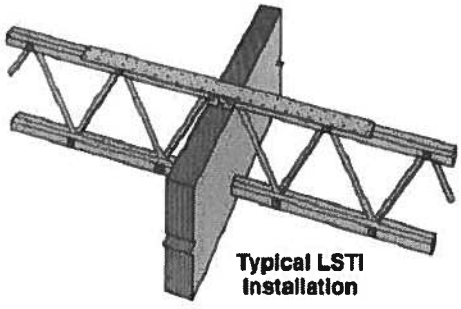


ST2115

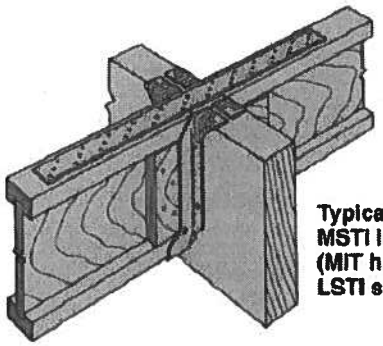
MSTC28



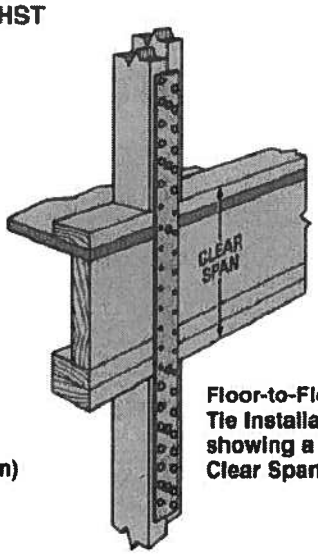
HST



Typical LSTI Installation

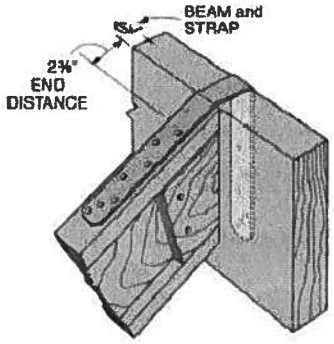


Typical MSTI Installation (MIT hanger shown) LSTI similar

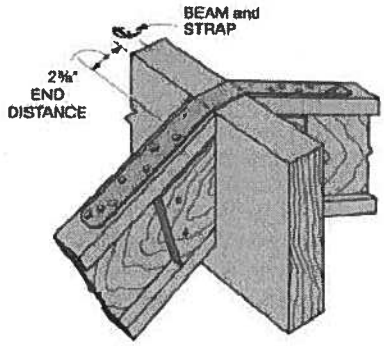


Floor-to-Floor Tie Installation showing a Clear Span

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Allowable Tension Loads Floor (100)	(133)	(160)
RPS18	16	1½	18 1/16	12-16d	810	1080	1295
RPS22	16	1½	22 5/16	16-10d	905	1205	1445
RPS28	16	1½	28 1/16	12-16d	810	1080	1295
LSTA9	20	1¼	9	8-10d	450	605	725
LSTA12	20	1¼	12	10-10d	565	755	905
LSTA15	20	1¼	15	12-10d	680	905	1085
LSTA18	20	1¼	18	14-10d	790	1055	1265
LSTA21	20	1¼	21	16-10d	905	1205	1295
LSTA24	20	1¼	24	18-10d	1015	1295	1295
ST292	20	2 1/16	9 5/16	12-16d	790	1055	1130
ST2122	20	2 1/16	12 1/16	16-16d	1070	1425	1505
ST2115	20	¾	16 1/16	10-16d	450	600	600
ST2215	20	2 1/16	16 1/16	20-16d	1270	1695	1695
LSTA30	18	1¼	30	22-10d	1255	1670	1715
LSTA36	18	1¼	36	26-10d	1480	1715	1715
LSTI49	18	3¼	49	32-10dx1½	1455	1940	2330
LSTI73	18	3¼	73	48-10dx1½	2185	2910	3495
MSTA9	18	1¼	9	8-10d	455	610	730
MSTA12	18	1¼	12	10-10d	570	760	910
MSTA15	18	1¼	15	12-10d	685	910	1095
MSTA18	18	1¼	18	14-10d	800	1065	1275
MSTA21	18	1¼	21	16-10d	910	1215	1460
MSTA24	18	1¼	24	18-10d	1025	1370	1640
MSTA30	16	1¼	30	22-10d	1265	1685	2025
MSTA36	16	1¼	36	26-10d	1495	1995	2135
ST6215	16	2 1/16	16 1/16	20-16d	1330	1775	2130
ST6224	16	2 1/16	23 1/16	28-16d	1890	2520	2630
ST9	16	1¼	9	8-16d	530	705	850
ST12	16	1¼	11½	10-16d	665	885	1065
ST18	16	1¼	17¼	14-16d	900	1200	1200
ST22	16	1¼	21½	18-16d	1025	1370	1370
MSTC28	16	3	28¼	36-16d sinkers	2070	2760	3310
MSTC40	16	3	40¼	52-16d sinkers	2990	3985	4740
MSTC52	16	3	52¼	62-16d sinkers	3555	4740	4740
MSTC66	14	3	65¾	76-16d sinkers	4390	5855	5855
MSTC78	14	3	77¼	76-16d sinkers	4390	5855	5855
ST6236	16	2 1/16	33 7/16	40-16d	2575	3430	3430
FHA6	12	1 1/16	6⅜	8-16d	550	735	885
FHA9	12	1 1/16	9	8-16d	550	735	885
FHA12	12	1 1/16	11⅞	8-16d	550	735	885
FHA18	12	1 1/16	17¼	8-16d	550	735	885
FHA24	12	1 1/16	23⅝	8-16d	550	735	885
FHA30	12	1 1/16	30	8-16d	550	735	885
MSTI26	12	2 1/16	26	26-10dx1½	1130	1510	1810
MSTI36	12	2 1/16	36	36-10dx1½	1565	2090	2505
MSTI48	12	2 1/16	48	48-10dx1½	2135	2850	3420
MSTI60	12	2 1/16	60	60-10dx1½	2760	3680	4415
MSTI72	12	2 1/16	72	72-10dx1½	3310	4415	4725



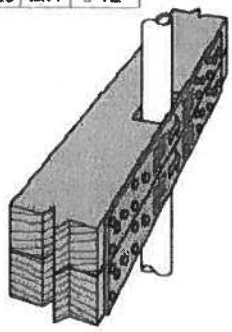
Typical LSTA Installation (hanger not shown)



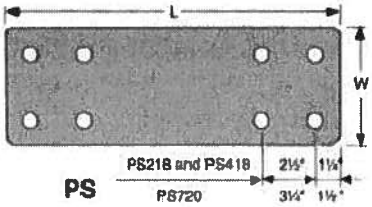
Typical LSTA Installation (hanger not shown)

Model No.	Plate	Notch Width
RPS18	2x4	≤ 5½"
RPS22	2x6	≤ 5½"
RPS28	2x4	≤ 12"

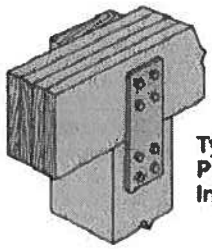
RPS



Typical RPS Installation



PS



Typical PS720 Installation

Model No.	Ga	Dimensions W	Dimensions L	Bolts Qty	Bolts Dia
PS218[8]	7	2	18	4	⅝
PS418[8]	7	4	18	4	⅝
PS720[8]	7	6¾	20	8	½

Floor-to-Floor Clear Span Table

Model No.	Clear Span	Fasteners (Total)	Allowable Tension Load (133)	(160)
MSTC28	18	12-16d sinker	920	1105
	16	16-16d sinker	1225	1470
MSTC40	18	28-16d sinker	2145	2575
	16	36-16d sinker	2455	2945
MSTC52	18	44-16d sinker	3375	4050
	16	48-16d sinker	3680	4415
MSTC66	18	64-16d sinker	5035	5855
	16	68-16d sinker	5350	5855
MSTC78	18	80-16d sinker	5855	5855
	16	80-16d sinker	5855	5855
MST37	18	20-16d	1905	2285
	16	22-16d	2100	2515
MST48	18	32-16d	3135	3765
	16	34-16d	3330	4000
MST60	18	46-16d	4785	5740
	16	48-16d	4990	5800
MST72	18	56-16d	5800	5800
	16	58-16d	5800	5800
MSTI36	18	14-10dx1½	810	975
	16	16-10dx1½	930	1115
MSTI48	18	26-10dx1½	1545	1855
	16	28-10dx1½	1660	1990
MSTI60	18	38-10dx1½	2330	2800
	16	40-10dx1½	2455	2945
MSTI72	18	50-10dx1½	3065	3680
	16	52-10dx1½	3190	3830

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Bolts Qty	Bolts Dia	Allowable Tension Loads Nails Floor (100)	Nails (133)	Nails (160)	Bolts[5] Floor (100)	Bolts[5] (133)	Bolts[5] (160)
MST27	12	2 1/16	27	30-16d	4	½	2070	2760	2790	1295	1725	2070
MST37	12	2 1/16	37½	42-16d	6	½	2860	3815	3815	1825	2435	2920
MST48	12	2 1/16	48	46-16d	8	½	3345	4460	4460	2225	2970	3560
MST60	10	2 1/16	60	56-16d	10	½	4350	5800	5800	2670	3565	4275
MST72	10	2 1/16	72	56-16d	10	½	4350	5800	5800	2670	3565	4275
HST2	7	2½	21¼	—	6	⅝	—	—	—	3130	4175	5005
HST5	7	5	21¼	—	12	⅝	—	—	—	6385	8510	10210
HST3	3	3	25½	—	6	¾	—	—	—	4645	6195	7435
HST6	3	6	25½	—	12	¾	—	—	—	9350	12465	14955

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed. Floor loads may not be increased for other load durations.
2. 10dx1½" nails may be substituted where 16d sinkers are specified at 0.80 of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers (9 gauge x 3¼") or 10d commons may be substituted where 16d commons are specified at 0.84 of the table loads.
5. Allowable bolt loads are based on parallel-to-grain loading and these minimum member thicknesses: MST–2½"; HST2 and HST5–4"; HST3 and HST6–4½".
6. PS strap design loads must be determined by the building designer for each installation. Bolts are installed both perpendicular and parallel-to-grain.
7. Use half of the nails at each member being connected to achieve the listed loads.

Locking prongs inserts into concrete. The one-piece design assures maximum strength.

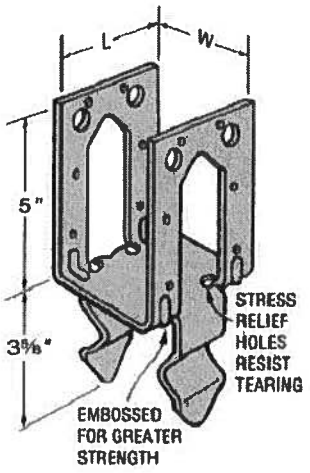
MATERIAL: 12 gauge. **FINISH:** Galvanized

INSTALLATION:
- Holes are provided for installation with either 16d commons or ½" bolts for PB66 and PB66R; all other models use 16d commons only.
- A 2" minimum sidecover is required to obtain the full load.
- Not recommended for non-top-supported installations such as fences.

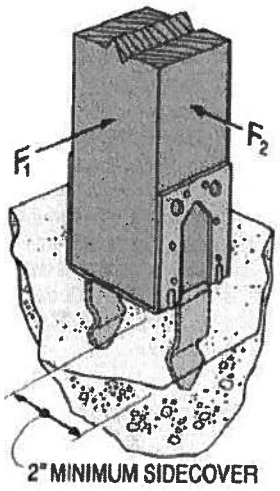
CODES: BOCA, ICBO, SBCCI NER-443; City of LA RR 25149; **Dade Co. 00-0512.11 (PB44).**

Model No.	Dimensions W	Dimensions L	Uplift Avg Ult	Allowable Loads 12-16d Nails (133 & 160) Uplift	F1	F2	2-½MB Uplift (133 & 160)
PB44	3⅝	3¼	4267	1365	765	1325	—
PB44R	4	3¼	4267	1365	765	1325	—
PB46	5⅛	3¼	4267	1365	765	1325	—
PB46R	6	3¼	4267	1365	765	1325	—
PB66	5½	5¼	5143	1640	765	1325	1640
PB66R	6	5¼	5143	1640	765	1325	1640

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading, with no further increase allowed.



PB



Typical PB Installation

AC/LPC/LCE POST CAPS

The LCE4's universal design provides high capacity while eliminating the need for rights and lefts.

The AC MAX design allows for higher load capacity to match comparable post bases.

LPC—Adjustable design allows greater connection versatility.

MATERIAL: LCE4—20 ga; AC, ACE, LPC4—18 ga; LPC6—16 ga

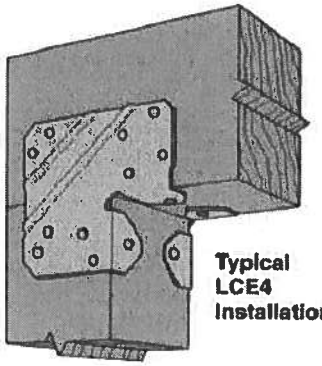
FINISH: Galvanized. Some products available with Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- Install all models in pairs. LPC—2½" beams may be used if 10dx1½" nails are substituted for 10d commons.

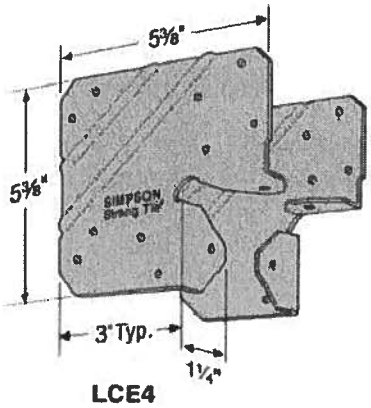
CODES: BOCA, ICBO, SBCCI NER-421, NER-443, NER-469; City of L.A. RR 25076; Dade County, FL 99-0623.04 (LPC) and Dade County, FL 99-0713.05 (AC, ACE).

Model No.	Dimensions W	Dimensions L	Total No. Fasteners Beam	Total No. Fasteners Post	Uplift Avg Ult	Allowable Loads (133 & 160)[1] Uplift	Allowable Loads (133 & 160)[1] Lateral
AC4 MIN	3⅜	6½	12-16d	8-16d	4467	1430	715
AC4 MAX	3⅜	6½	14-16d	14-16d	10000	2500	1070
AC4R MIN	4	7	12-16d	8-16d	4467	1430	715
AC4R MAX	4	7	14-16d	14-16d	10000	2500	1070
ACE4 MIN	—	4½	8-16d	6-16d	4215	1070	715
ACE4 MAX	—	4½	10-16d	10-16d	6238	1785	1070
AC6 MIN	5½	8½	12-16d	8-16d	4467	1430	715
AC6 MAX	5½	8½	14-16d	14-16d	10000	2500	1070
AC6R MIN	6	9	12-16d	8-16d	4467	1430	715
AC6R MAX	6	9	14-16d	14-16d	10000	2500	1070
ACE6 MIN	—	6½	8-16d	6-16d	4537	1070	715
ACE6 MAX	—	6½	10-16d	10-16d	6432	1785	1070
LPC4	3⅜	3½	8-10d	8-10d	2333	760	325
LPC6	5⅜	5½	8-10d	8-10d	2817	915	490
LCE4	—	5⅜	14-16d	10-16d	5518	1800	1425

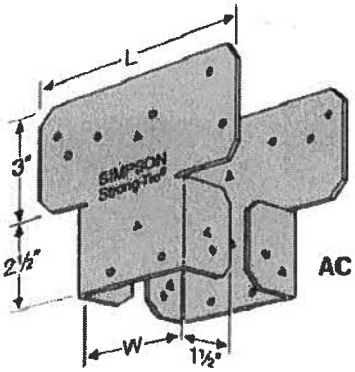
1. Allowable loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed; reduce for other load durations according to the code.
2. Loads apply only when used in pairs.
3. LPC lateral load is in the direction of the beam's axis.
4. MIN nailing quantity and load values – fill all round holes; MAX nailing quantities and load values – fill round and triangle holes.



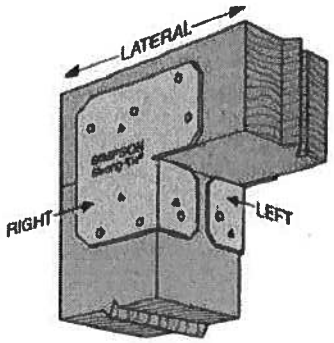
Typical LCE4 Installation



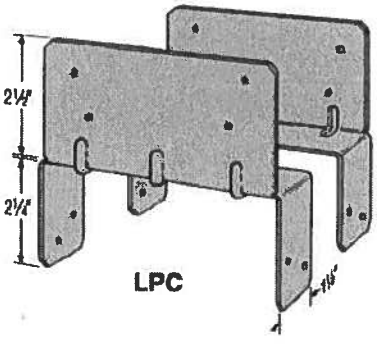
LCE4



AC



Typical ACE Installation



LPC

Caps & Bases

AB/ABA/ABE/ABU/PBS ADJUSTABLE AND STANDOFF POST BASES

SIMPSON StrongTie

The AB is a fully-adjustable post base which offers moisture protection and finished hardware appearance.

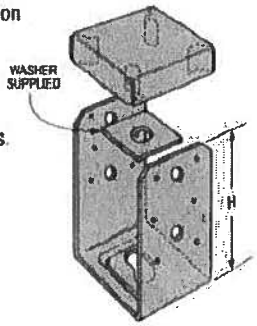
Post Bases provide tested capacity. They feature 1" standoff height above concrete floors, code-required when supporting permanent structures that are exposed to the weather or water splash, or in basements. They reduce the potential for decay at post and column ends.

MATERIAL: AB—12 ga plates; 16 ga base cover; all others—see table.

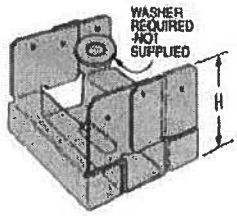
FINISH: Galvanized. Some products available in Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- Not recommended for non-top-supported installations such as fences.
- PBS embed into wet concrete up to the bottom of the 1" standoff base plate. A 2" minimum side cover is required to obtain the full load for PBS. Holes in the bottom of the PBS straps allow for free concrete flow.
- AB—Post nail holes are sized for 10d commons. Rectangular adjustment plate assumes ½" dia anchorage. Supplied as shown; position the post, secure the easy-access nut, then bend up the fourth side.
- AB, ABA, ABE and ABU—for pre-pour installed anchors. For epoxy or wedge anchors, select and install according to anchor manufacturer's recommendations; anchor diameter shown in table. Install required washer, which is not included for ABAs.
- See Simpson Anchor Systems for tested, load-rated anchors.

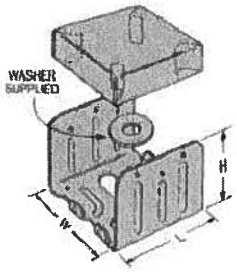
CODES: BOCA, ICBO, SBCCI NER-393, NER-422, NER-432, NER-469, NER-499; **ICBO 5670**; City of L.A. RR 24818, RR 25064, 25074, 25158; Dade Co FL. 99-0713.05 (ABA, ABE), **00-0512.11 (ABU)**.



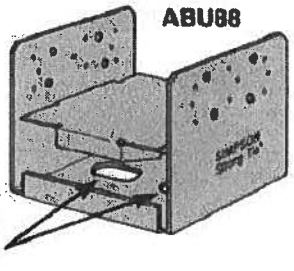
ABU44 (other sizes similar)

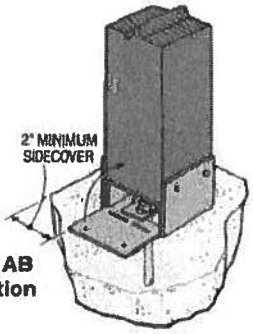


ABA44 (other sizes similar)
U.S. Patent 5,333,435

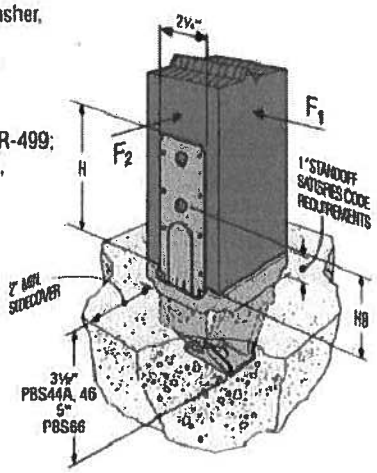


ABE44
ABE46, 46R, 66 and 66R supplied with rectangular washer.

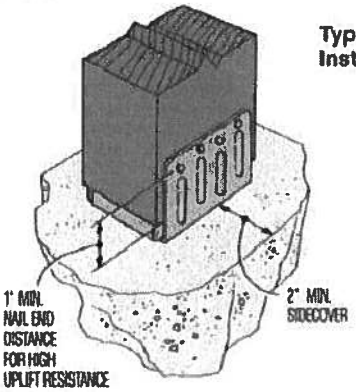




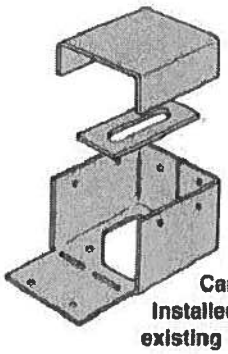
Typical AB Installation



Typical PBS44A Installation



Typical ABE46R Installation for rough lumber (ABE similar)



AB Can be installed on existing slab

Model No.	Dimensions W	Dimensions L	Allowable Downloads (100)
AB44	3⅛	3⅜	4065
AB44R	4	4⅛	4065
AB46	3⅛	5⅜	4165
AB46R	4	6	4165
AB66	5½	5⅜	5335
AB66R	6	6	5335

1. Loads may not be increased for short-term loading.

Model No.	Nominal Post Size	Material Base (Ga)	Material Strap (Ga)	Dimensions W	L	H	HB	Fasteners Anch. Dia	Post Nails	Post Bolts Qty	Post Bolts Dia	Uplift Avg Ult	Allowable Loads Uplift (133) Nails	Uplift (133) Bolts	Uplift (160) Nails	Uplift (160) Bolts	F_1 (133 & 160) Nails	F_1 (133 & 160) Bolts	F_2 (133 & 160) Nails	F_2 (133 & 160) Bolts	Down (100)
ABA44	4x4	16	16	3⅜	3½	3⅞	—	½	6-10d	—	—	2120	555	—	555	—	—	—	—	—	6000
ABE44	4x4	16	16	3⅜	3½	2¾	—	½	6-10d	—	—	1893	520	—	520	—	—	—	—	—	6665
ABU44	4x4	16	12	3⅜	3	5½	1¼	⅝	12-16d	2	½	7833	2200	1800	2200	2160	—	—	—	—	6665
PBS44A	4x4	12	14	3⅜	2¼	6¼	3⅛	—	14-16d	2	½	7733	2400	2400	2400	2400	1165	230	885	885	6665
ABA44R	RGH 4x4	16	16	4⅜	3½	2⅞	—	½	6-10d	—	—	2120	555	—	555	—	—	—	—	—	8000
ABE44R	RGH 4x4	16	16	4	3½	2⅝	—	½	6-10d	—	—	1893	400	—	400	—	—	—	—	—	6665
ABE46	4x6	12	16	3⅜	5⅛	4⅛	—	⅝	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335
PBS46	4x6	12	14	3⅜	2¼	6⅞	3⅛	—	14-16d	2	½	7733	2400	2400	2400	2400	1165	360	885	885	9335
ABA46	4x6	14	14	3⅜	5⅛	3⅛	—	⅝	8-16d	—	—	2967	700	—	700	—	—	—	—	—	9435
ABU46	4x6	12	12	3⅜	5	7	2⅛	⅝	12-16d	2	½	8633	2255	2300	2300	2300	—	—	—	—	10335
ABE46R	RGH 4x6	12	16	4⅜	5⅛	3⅝	—	⅝	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335
ABA46R	RGH 4x6	14	14	4⅜	5⅛	2⅞	—	⅝	8-16d	—	—	2967	935	—	935	—	—	—	—	—	12000
PBS66	6x6	12	12	5½	2¼	6½	3⅛	—	14-16d	2	½	13100	2630	3560	3160	4000	1865	570	1700	1700	9335
ABA66	6x6	14	14	5½	5¼	3⅛	—	⅝	8-16d	—	—	3050	720	—	720	—	—	—	—	—	10665
ABE66	6x6	12	14	5½	5⅛	3⅛	—	⅝	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000
ABU66	6x6	12	10	5½	5	6⅛	1¼	⅝	12-16d	2	½	8900	2300	2300	2300	2300	—	—	—	—	12000
ABA66R	RGH 6x6	14	14	6	5⅛	2⅞	—	⅝	8-16d	—	—	3050	985	—	985	—	—	—	—	—	12665
ABE66R	RGH 6x6	12	14	6⅜	5⅛	2⅞	—	⅝	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000
ABU88[4]	8x8	12	14	7½	7	7	—	2-⅝	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335
ABU88R	RGH 8x8	12	14	8	7	7	—	2-⅝	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335

1. Uplift and lateral loads have been increased 33% and 60% for earthquake or wind loading, no further increase allowed. Reduce by 33% and 60% for normal loading.
2. Downloads may not be increased for short-term loading.
3. Specifier to design concrete for shear capacity.
4. ABU88 and ABU88R may be installed with 8-SDS¼X3 wood screws for the same table load.

Caps & Bases

Brame Architects
608 N E First Street Gainesville, Florida 32601
Phone (352) 372-0425 FAX (352) 372-0427

FAX TRANSMITTAL LETTER

PAGE 1 OF 6

TO Jim Haltiwanger
Columbia County Building Department

DATE February 8, 2006

RE: Haven Hospice
of the Suwannee Valley
Lake City, FL
(5138.010)

ATTN:

TO FAX NO. 1 386 754-7088

COMMENTS: I have attached the letter we discussed this morning. We will mail out the original today.

Thank you very much for your assistance.

Joe,
please get with me about this!

thanks JK

cc: Sharon Breitinger
Bill Pearson

If transmission is incomplete
please call telephone (352) 372-0425
or FAX (352) 372-0427 immediately

Brame Architects

William W. "Billy" Brame AIA

Brame Architects

February 8, 2006

SUBJECT:
Haven Hospice
of the Suwannee Valley
Lake City, FL
(5138.010)

Building Permit Application Number **0512-55**

Joe Haltiwanger
Plans Examiner
Columbia County Building Department
P.O. Box 1529
Lake City, FL 32056-1529

Dear Mr. Haltiwanger:

I appreciate the time you spent with me on the phone this morning regarding your review comments on the plans for the above reference project. This letter will summarize our discussion and the conclusions.

You had referenced Table 1604.5 regarding the category classification for the importance wind factor relating to the office building portion of the project. In the category III section there is a statement that "*Buildings and other structures where more than 300 people congregate in one area*" must use an importance factor of 1.15. Our building design is based on an importance factor of 1.0.

As we discussed, we have always interpreted that portion of the code to mean the word "area" referred to the actual room or space where the people are congregating, as opposed to the entire building area. While our total building will have a population over 300 people, the actual "area" or rooms where such an assembly would occur can only accommodate 251 people.

In support of this issue, I referred you to the website for the Building Officials Association of Florida (www.boaf.net). While on the phone, you and I went through the following process.

> If you then click Interpretations of the Florida Building Code (left hand side), you will be presented with their Non-Binding Interpretations page. See Attachment A.
>
> At the bottom of that page, in the Search section, click the "Building" code button, and type in "table 1604.5" and click the submit button.
>
> That will take you to a page with a request quite similar to this situation. See Attachment B. If you click "Display" on the right side, it will take you to their Non-Binding Interpretation.
>
> Their interpretation says "*The "one area" mentioned in the first bullet in Category III refers to a large room and not the entire building.*" See Attachment C.

606 N.E. First Street
Gainesville, Florida 32601
(352) 372-0425
FAX (352) 372-0427

Lic. No. AA-0002304

Founded in 1911

This interpretation of the code also means there is no requirement for providing a fire protection sprinkler system for this portion of the complex.

You had also noted that the same table 1604.5 had special requirements for "*Health care facilities with an occupant load of 50 or more resident patients ...*". You had asked how many beds this facility will have.

This initial construction will have 16 single occupancy rooms. The State stipulates the square footage requirements per patient, and our rooms were designed for and will be limited to one patient per room. We are making provisions for a possible future expansion of eight more single patient rooms. If that ever occurs, we will then have a total of 32 patients, well below the 50 patient threshold for that portion of the code.

I trust that this information meets your needs. Based on this data, we are assuming that the plans as submitted properly meet the code requirements addressed in your review comments.

Thank you very much for your assistance in this effort. We look forward to working with you and your department as this project gets underway.

Please contact me if you have any comments or questions about these or any other matters on this project.

Sincerely,

William W. 'Billy' Brame AIA
Brame Architects

WWB:bb(F:\6138010\ltr joe haltiwanger 2.8.06 building permit review.wpd)

cc: Sharon Breitinger (AvMed)
Bill Pearson (MM Parrish Construction)

Attachments

ATTACHMENT A

Wednesday February 8, 2006

SEARCH for existing Interpretations and Declaratory Statements

REQUEST a Non-Binding Interpretation.

BCAIB Approved Cross Training Program

Calendar

Committees

Discussion Group Archives

Education

Interpretations of the Florida Building Code

Links

Marketplace

Membership Services

Non-Binding Interpretations

The Building Officials Association of Florida in accordance with an agreement with the Department of Community Affairs, now provides Non-Binding, Advisory Interpretations of the Florida Building Code.

This agreement was authorized by Section 16 Chapter 2002-293, Laws of Florida, which provides: *"It is the intent of the Legislature that the process provide for the expeditious resolution of the issues presented and publication of the resulting interpretation on the Building Code Information System. Such interpretations are to be advisory only and nonbinding on the parties or the commission."*

NOTICE: These interpretations are the result of input provided by the BOAF Code Development Committee, the Florida Department of Community Affairs (DCA), the International Code Council (ICC) and subject-matter-experts in the construction industry/professions.

Interpretations are NON-BINDING and do not affect the only legal interpretation, which is that of the Building Official of the individual jurisdictions.

Search for Interpretations and Declaratory Statements

Code: ○ **All** ◉ **Building** ○ **Plumbing** ○ **Mechanical** ○ **Fuel Gas** ○ **Electri**

Query String: table 1604.5

Submit Clear Form

(Leave Query String empty to list all)

ATTACHMENT B

Building Officials Association of Florida

Established in 1953

Wednesday February 8, 2006

BCAIB Approved Cross Training Program

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BOAF Advisory Non-binding Interpretations	
Regarding: 1605 of FBC-Building Is it the intent of Table 1604.5, under "Nature of Occupancy", Category III, 1st bullet "in one area" to mean just that...one area (like a room) or the entire building? [We are trying to determine whether our golf clubhouse falls under Wind Factor 1.0 or 1.15. Our TOTAL occupancy is 452 persons. Our greatest occupancy in ONE AREA (dining) is 182 persons.]	Display

ATTACHMENT C

Florida Building Code
Informal Interpretation

Date: Mon Jan 23 2006

Report #: 4160

Code: Building

Section: 1605

Question:
Is it the intent of Table 1604.5, under "Nature of Occupancy", Category III, 1st bullet "in one area" to mean just that...one area (like a room) or the entire building? [We are trying to determine whether our golf clubhouse falls under Wind Factor 1.0 or 1.15. Our TOTAL occupancy is 452 persons. Our greatest occupancy in ONE AREA (dining) is 182 persons.]

Answer:
The "one area " mentioned in the first bullet in Category III refers to a large room and not the entire building.

Commentary:
The subject gulf clubhouse faalls under Category II with the Importance Factor of 1.00 shown in Table 1604.5

Wednesday February 8, 2006

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Florida Building Code
Informal Interpretation

Date: Mon Jan 23 2006

Report #: 4160

Code: Building

Section: 1605

Question:
Is it the intent of Table 1604.5, under "Nature of Occupancy", Category III, 1st bullet "in one area" to mean just that...one area (like a room) or the entire building? [We are trying to determine whether our golf clubhouse falls under Wind Factor 1.0 or 1.15. Our TOTAL occupancy is 452 persons. Our greatest occupancy in ONE AREA (dining) is 182 persons.]

Answer:
The "one area " mentioned in the first bullet in Category III refers to a large room and not the entire building.

Commentary:
The subject gulf clubhouse faalls under Category II with the Importance Factor of 1.00 shown in Table 1604.5

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☐ N/A	☐	d) Location, size and height above roof of chimneys
☐ N/A	☐	e) Location and size of skylights
☐ N/A	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- ☑ ☐ a) Rooms labeled and dimensioned
- ☑ ☐ b) Shear walls
- ☑ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☐ N/A ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☑ ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☑ ☐ f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- ☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☐ N/A ☐ b) All posts and/or column footing including size and reinforcing
- ☐ N/A ☐ c) Any special support required by soil analysis such as piling
- ☐ N/A ☐ d) Location of any vertical steel

Roof System:

- ☑ ☐ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- ☑ ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- ☐ ☐ N/A a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termiticide or alternative method)
  10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☐ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ N/A c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☑ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☑ ☐ b) Floor joist size and spacing

☑ ☐ c) Girder size and spacing

☑ ☐ d) Attachment of joist to girder

☑ ☐ e) Wind load requirements where applicable

☑ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☐ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☐ N/A ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

Notice Of Commencement

Private Potable Water

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to br submitted by the owner or contractor to this office when applying for a Building Permit.

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$10.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$5.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$25.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 758-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

Record and Return to: Kelley D. Jones, P.A.
5800 N.W. 39th Ave., Ste 102
Gainesville, Florida 32606

Rec: 10.00
Doc: 2.50 Cert. Copy
Int:
Tot: 12.50

Permit No. _____ Tax Folio No: _____

NOTICE OF COMMENCEMENT

State of Florida
County of Alachua

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of real property:

   Begin at the NW corner of the SW 1/4 of Section 4, Township 7 South, Range 17 East, Columbia County, Florida, and run thence N 87°57'25" E along the North line thereof, 458.01 feet; thence N 02°14'18" W, 642.01 feet to the South line of Old Bellamy Road; thence N 89°27'35" E, 222.57 feet; thence S 02°13'30" E, 636.00 feet; thence S 02° 11'24" E, 1314.63 feet; thence S 88°05'38" W, 678.59 feet; thence N 02°15'43" W, 1312.84 feet to the Point of Beginning.

2. General description of improvement: Single Family Residence

3. Owner information: JOHN B. MACLAREN AND JESSICA SLAYMAKER
   Address: 1512 AVENUE F NE, WINTER HAVEN, FLORIDA 33881

   Interest in property: Fee Simple
   Name and address of Fee Simple Titleholder (if other than owner): N/A

4. Contractor: HAYGOOD HOMES, INC.
   Address: 12592 S. US HWY 441, LAKE CITY, FLORIDA

5. Surety: N/A
   Address: _____
   Amount of Bond: N/A

6. Lender: CAMPUS USA CREDIT UNION
   Address: Post Office Box 147029, GAINESVILLE, FL 32614

7. In addition to himself, Owner designates the following persons to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

   A. CAMPUS USA CREDIT UNION, PO BOX 147029, GAINESVILLE, FL 32614

8 Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified)_____.

JOHN B. MACLAREN

Jessica Slaymaker
JESSICA SLAYMAKER

The foregoing instrument was acknowledged before me this 28 day of February, 2006, by John B. Maclaren and Jessica Slaymaker who are personally known to me or have produced _____ as identification.

KELLEY D. JONES
MY COMMISSION # DD 343845
EXPIRES: August 20, 2008
Bonded Thru Notary Public Underwriters

Notary Public:
State of Florida at Large
My Commission Expires:

Inst:2006005482 Date:03/06/2006 Time:12:06
DC,P.DeWitt Cason,Columbia County B:1076 P:674

24180

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 04-7S-17-09888-008 **Building permit No.** 000024180

Use Classification SFD,UTILITY **Fire:** 129.56

Permit Holder HAYGOOD HOMES **Waste:** 201.00

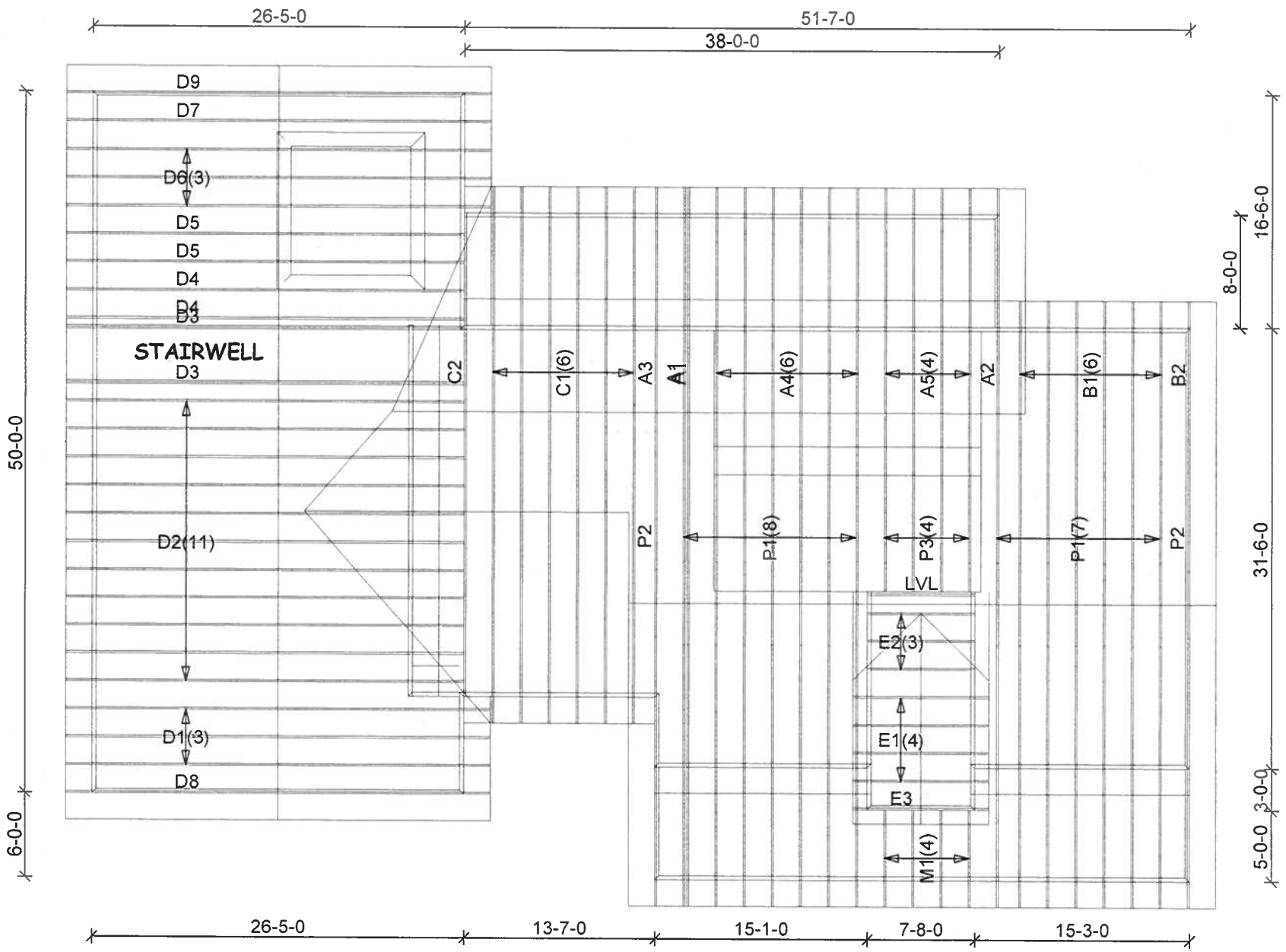
Owner of Building JOHN B. MACLAREN **Total:** 330.56

Location: 1040 SW OLD BELLAMY ROAD

Date: 10/13/2006

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



Mayo Truss Co. Inc.

362 NE CLYDE AVE.
MAYO, FL 32066

(386)294-3988
(877)-558-6262

HAYGOOD HOMES, INC.

McLEARN RESIDENCE

110 MPH ASCE WIND LOAD

Roof Loading
TC Live: 20.00 psf
TC Dead: 10.00 psf
BC Live: 0.00 psf
BC Dead: 10.00 psf
TC Stress Inc: 25.00
BC Stress Inc: 25.00
Spacing: 2- 0- 0 o.c.

Account: CONTRACTORS
Job: haygood-mclearn
Designer: M.MURRAY
Checker: M.MURRAY
Date: 02-09-06

Permit Number:_____ Lot Number:_____

Miscellaneous:_____ Address:_____

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator:Mayo Truss Company, Inc

Job Reference:haygood-mclearn - McLEARN

Standard Loading:

T.C. Live	20 psf
T.C Dead	10 psf
B.C Live	0 psf
B.C. Dead	10 psf
Total	40 psf

ANSI/ASCE 7-02
Wind Speed - 110 mph
Mean Roof Ht. - 15 ft.
Exposure Catergory - B
Occupancy Factor - 1.00
MWFRS
Enclosed

ROBBINS ENGINEERING, INC.

P.O. Box 280055
Tampa, FL 33682-0055
Phone: (813) 972-1135

Engineering Index Sheet

Index Page 1 of 1

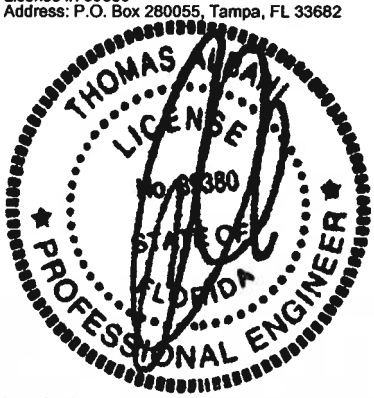
Job Number	Date	FBC - 2004 Chapter 16 and 23	Specification Quantity
T06020649	02/08/2006		25

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibilty of the Building Designer, as defined in ANSI/TPI 1-1995, Section 2.2. Permanent files of the original Truss Specificstion Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Sofware - Online Plus)

Notes: Refer to individual truss design drawings for special loading conditions.

	Date	Mark		Date	Mark		Date	Mark		Date	Mark
1	02/08/06	A1	2	02/08/06	A2	3	02/08/06	A3	4	02/08/06	A4
5	02/08/06	A5	6	02/08/06	B1	7	02/08/06	B2	8	02/08/06	C1
9	02/08/06	C2	10	02/08/06	D1	11	02/08/06	D2	12	02/08/06	D3
13	02/08/06	D4	14	02/08/06	D5	15	02/08/06	D6	16	02/08/06	D7
17	02/08/06	D8	18	02/08/06	D9	19	02/08/06	E1	20	02/08/06	E2
21	02/08/06	E3	22	02/08/06	M1	23	02/08/06	P1	24	02/08/06	P2
25	02/08/06	P3									

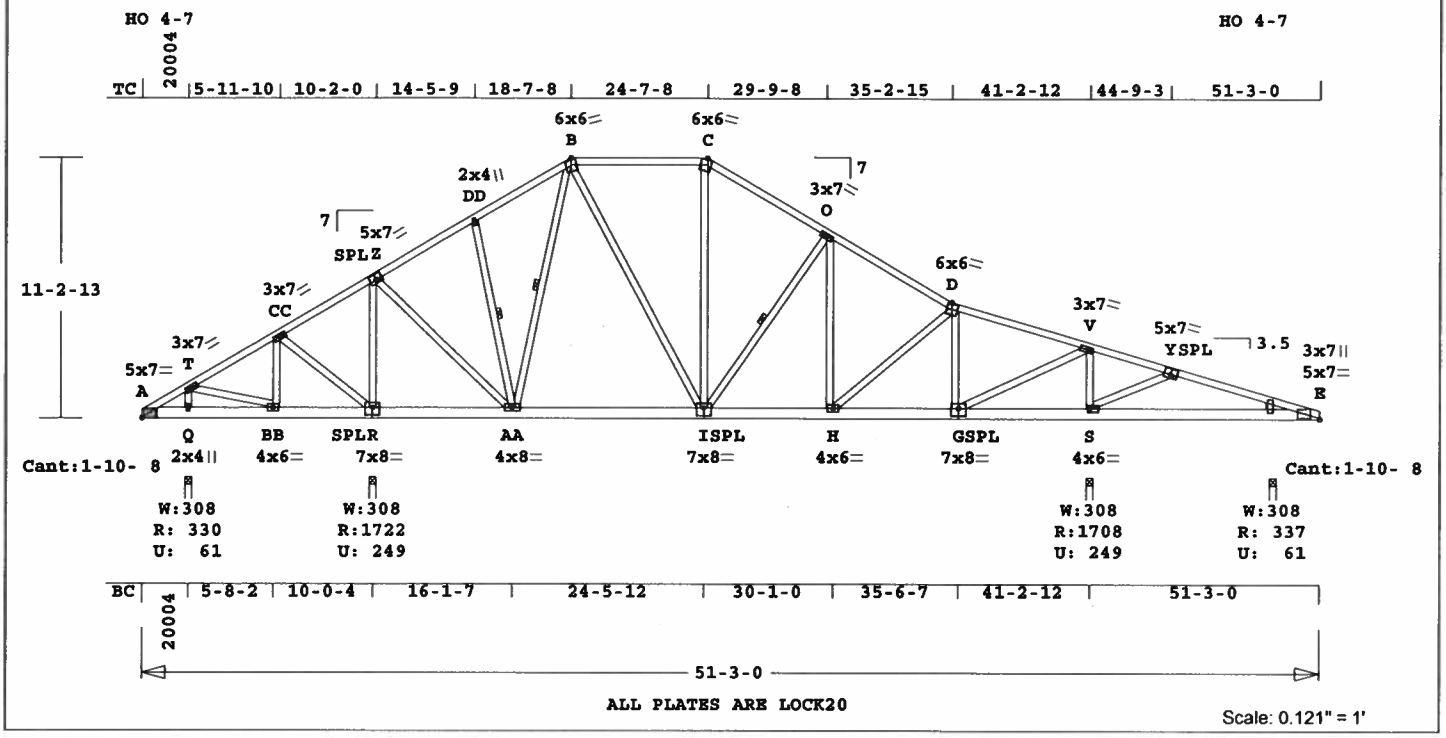
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	A1	2	SP	510300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 457.4 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.34	2x 4	SP-#2
BC	0.21	2x 6	SP-#2
WB	0.60	2x 4	SP-#2
WG	---	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18- 7- 8
TC 2-	0- 0- 0	18- 7- 8 24- 7- 8
TC Cont.	24- 7- 8	51- 3- 0
BC Cont.	0- 0- 0	51- 3- 0

WB 1 rows CLB on DD-AA
WB 1 rows CLB on AA-B
WB 1 rows CLB on I -O
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Q	331	62	3- 8	1- 8
		Hz =	-234	
R	1723	250	3- 8	1-13
S	1709	249	3- 8	1-13
E	338	62	3- 8	1- 8
		Hz =	212	

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -T	0.10	97	T	0.01	0.09
T -CC	0.16	33	T	0.01	0.15
CC-Z	0.22	237	T	0.05	0.17
Z -DD	0.17	656	C	0.00	0.17
DD-B	0.17	605	C	0.00	0.17
B -C	0.28	786	C	0.00	0.28
C -O	0.28	917	C	0.00	0.28
O -D	0.28	1169	C	0.00	0.28
D -V	0.28	1073	C	0.00	0.28
V -Y	0.34	344	T	0.06	0.28
Y -E	0.14	53	T	0.01	0.13
--------Bottom Chords---------					
A -Q	0.03	66	C	0.00	0.03
Q -BB	0.03	226	T	0.00	0.03
BB-R	0.08	182	T	0.00	0.08
R -AA	0.14	203	C	0.00	0.14
AA-I	0.18	593	T	0.04	0.14
I -H	0.21	1018	T	0.07	0.14
H -G	0.17	1040	T	0.12	0.05
G -S	0.19	324	C	0.00	0.19
S -E	0.19	158	T	0.00	0.19
------------Webs------------					
Q -T	0.02	243	C		
T -BB	0.01	67	T		
BB-CC	0.02	131	T		
CC-R	0.08	258	C		
R -Z	0.60	1436	C		
Z -AA	0.19	1057	T		
DD-AA	0.06	243	C		1 Br
AA-B	0.14	353	C		1 Br
B -I	0.07	397	T		
I -C	0.03	241	T		
I -O	0.11	401	C		1 Br
H -O	0.03	234	T		
H -D	0.03	52	C		
G -D	0.15	595	C		
G -V	0.27	1511	T		
S -V	0.18	1388	C		
S -Y	0.07	312	C		

LL Defl -0.02" in S -E L/999
TL Defl -0.08" in AA-I L/999
Shear // Grain in B -C 0.23

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

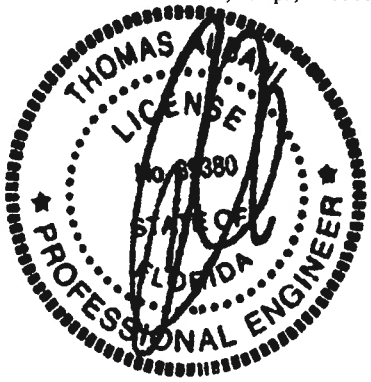
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 7.0	0.1	-0.4	0.75
T	LOCK	3.0x 7.0	Ctr	Ctr	0.47
CC	LOCK	3.0x 7.0	Ctr	Ctr	0.47
Z	LOCK	5.0x 7.0	-0.3	0.5	0.86
DD	LOCK	2.0x 4.0	Ctr	Ctr	0.55
B	LOCK	6.0x 6.0	0.1	-4.2	0.91
C	LOCK	6.0x 6.0	-1.0	-3.7	0.75
O	LOCK	3.0x 7.0	Ctr	Ctr	0.52
D	LOCK	6.0x 6.0	Ctr	Ctr	0.88
V	LOCK	3.0x 7.0	Ctr	Ctr	0.53
Y	LOCK	5.0x 7.0	0.1	0.5	0.87
E	LOCK	5.0x 7.0	Ctr	Ctr	0.87
E	LOCK	3.0x 7.0	Ctr	Ctr	0.00
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.55
BB	LOCK	4.0x 6.0	Ctr	Ctr	0.72
R	LOCK	7.0x 8.0	Ctr	-0.8	0.72
AA	LOCK	4.0x 8.0	Ctr	Ctr	0.55
I	LOCK	7.0x 8.0	Ctr	-0.8	0.72
H	LOCK	4.0x 6.0	Ctr	Ctr	0.47
G	LOCK	7.0x 8.0	Ctr	-0.8	0.72
S	LOCK	4.0x 6.0	Ctr	Ctr	0.76

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1436 Lbs
Quality Control Factor 1.25

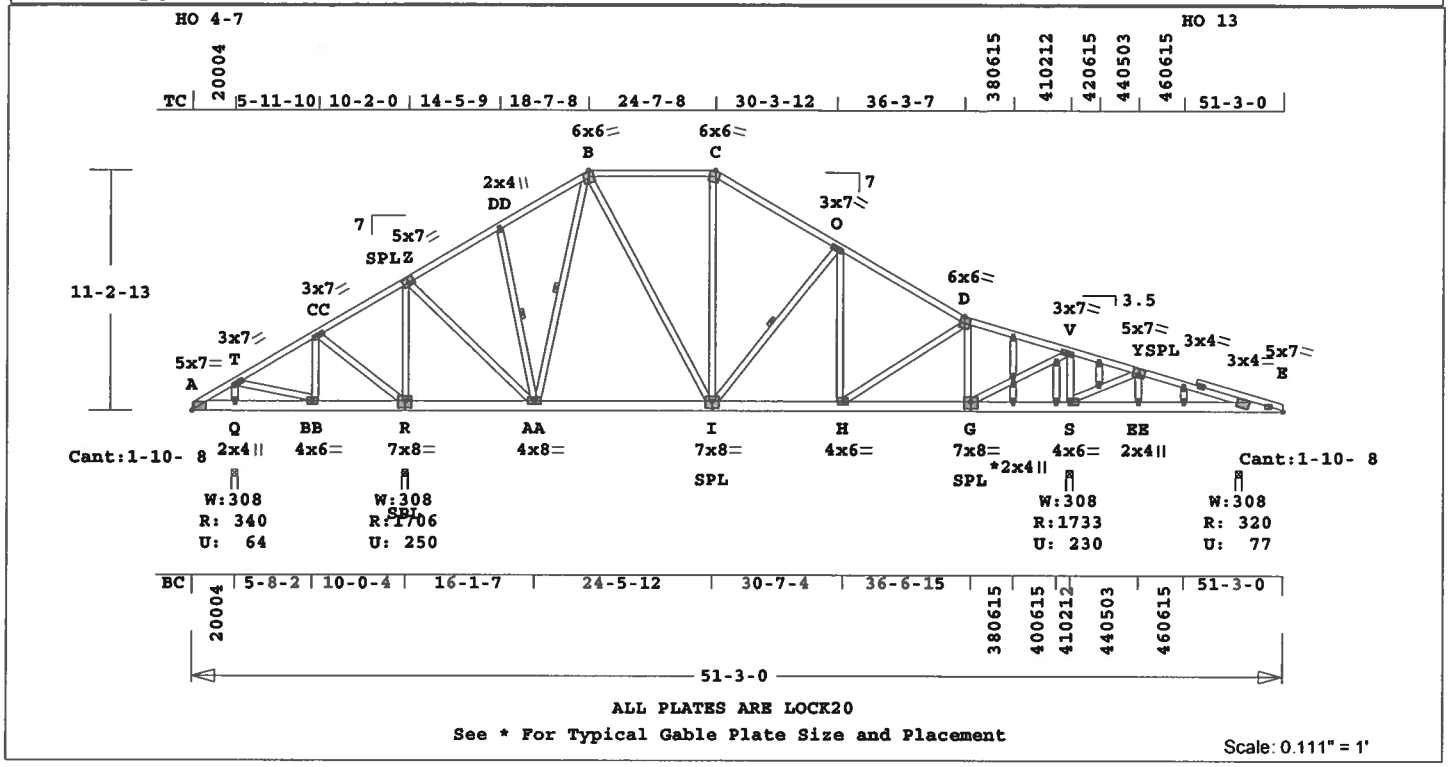
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	Pl-Ht	Left OH	Right OH	Engineering
haygood-mclearn	A2	1	SP	510300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 470.7 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	Size	Lumber
TC	0.36	2x 4	SP-#2
BC	0.21	2x 6	SP-#2
WB	0.59	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18- 7- 8
TC 2- 0- 0	18- 7- 8	24- 7- 8
TC Cont.	24- 7- 8	51- 3- 0
BC Cont.	0- 0- 0	51- 3- 0

WB 1 rows CLB on DD-AA
WB 1 rows CLB on AA-B
WB 1 rows CLB on I -O
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
Q	340	65	3- 8	1- 8
			Hz =	-235
R	1706	250	3- 8	1-13
S	1734	230	3- 8	1-14
E	320	77	3- 8	1- 8
			Hz =	216

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -T	0.10	97	T	0.01 0.09
T -CC	0.15	31	T	0.00 0.15
CC-Z	0.21	222	T	0.04 0.17
Z -DD	0.17	660	C	0.00 0.17
DD-B	0.17	609	C	0.00 0.17
B -C	0.28	785	C	0.00 0.28
C -O	0.36	917	C	0.00 0.36
O -D	0.36	1183	C	0.00 0.36
D -V	0.18	980	C	0.00 0.18
V -Y	0.26	483	T	0.08 0.18
Y -E	0.16	85	T	0.01 0.15
Bottom Chords				
A -Q	0.03	66	C	0.00 0.03
Q -BB	0.03	227	T	0.00 0.03
BB-R	0.08	184	T	0.00 0.08
R -AA	0.14	190	C	0.00 0.14
AA-I	0.18	594	T	0.04 0.14
I -H	0.21	1032	T	0.07 0.14
H -G	0.17	956	T	0.11 0.06
G -S	0.09	462	C	0.00 0.09
S -EE	0.09	138	T	0.00 0.09
EE-E	0.06	138	T	0.00 0.06
Webs				
Q -T	0.02	252	C	
T -BB	0.01	78	T	
BB-CC	0.02	130	T	
CC-R	0.08	255	C	
R -Z	0.59	1422	C	
Z -AA	0.19	1044	T	
DD-AA	0.06	243	C	1 Br
AA-B	0.13	344	C	1 Br
B -I	0.07	391	T	
I -C	0.03	229	T	
I -O	0.11	395	C	1 Br
H -O	0.02	192	T	
H -D	0.01	91	T	
G -D	0.14	682	C	
G -V	0.29	1596	T	
S -V	0.16	1420	C	
S -Y	0.07	428	C	
EE-Y	0.02	162	T	

LL Defl -0.03" in AA-I L/999
TL Defl -0.08" in AA-I L/999
Shear // Grain in B -C 0.22

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 7.0	0.1	-0.4	0.75
T	LOCK	3.0x 7.0	Ctr	Ctr	0.47
CC	LOCK	3.0x 7.0	Ctr	Ctr	0.47
Z	LOCK	5.0x 7.0	-0.3	0.5	0.86
DD	LOCK	2.0x 4.0	Ctr	Ctr	0.55
B	LOCK	6.0x 6.0	0.1	-4.2	0.91
C	LOCK	6.0x 6.0	-1.0	-3.7	0.75
O	LOCK	3.0x 7.0	Ctr	Ctr	0.50
D	LOCK	6.0x 6.0	Ctr	Ctr	0.87
V	LOCK	3.0x 7.0	Ctr	Ctr	0.58
Y	LOCK	5.0x 7.0	0.1	0.5	0.87
E	LOCK	5.0x 7.0	Ctr	-0.8	0.97
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.55
BB	LOCK	4.0x 6.0	Ctr	Ctr	0.72
R	LOCK	7.0x 8.0	Ctr	-0.8	0.72
AA	LOCK	4.0x 8.0	Ctr	Ctr	0.55
I	LOCK	7.0x 8.0	Ctr	-0.8	0.72
H	LOCK	4.0x 6.0	Ctr	Ctr	0.47
G	LOCK	7.0x 8.0	Ctr	-0.8	0.72
S	LOCK	4.0x 6.0	Ctr	Ctr	0.76
EE	LOCK	2.0x 4.0	Ctr	Ctr	0.55

5 Gable studs to be attached with 2.0x4.0 plates each end.

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

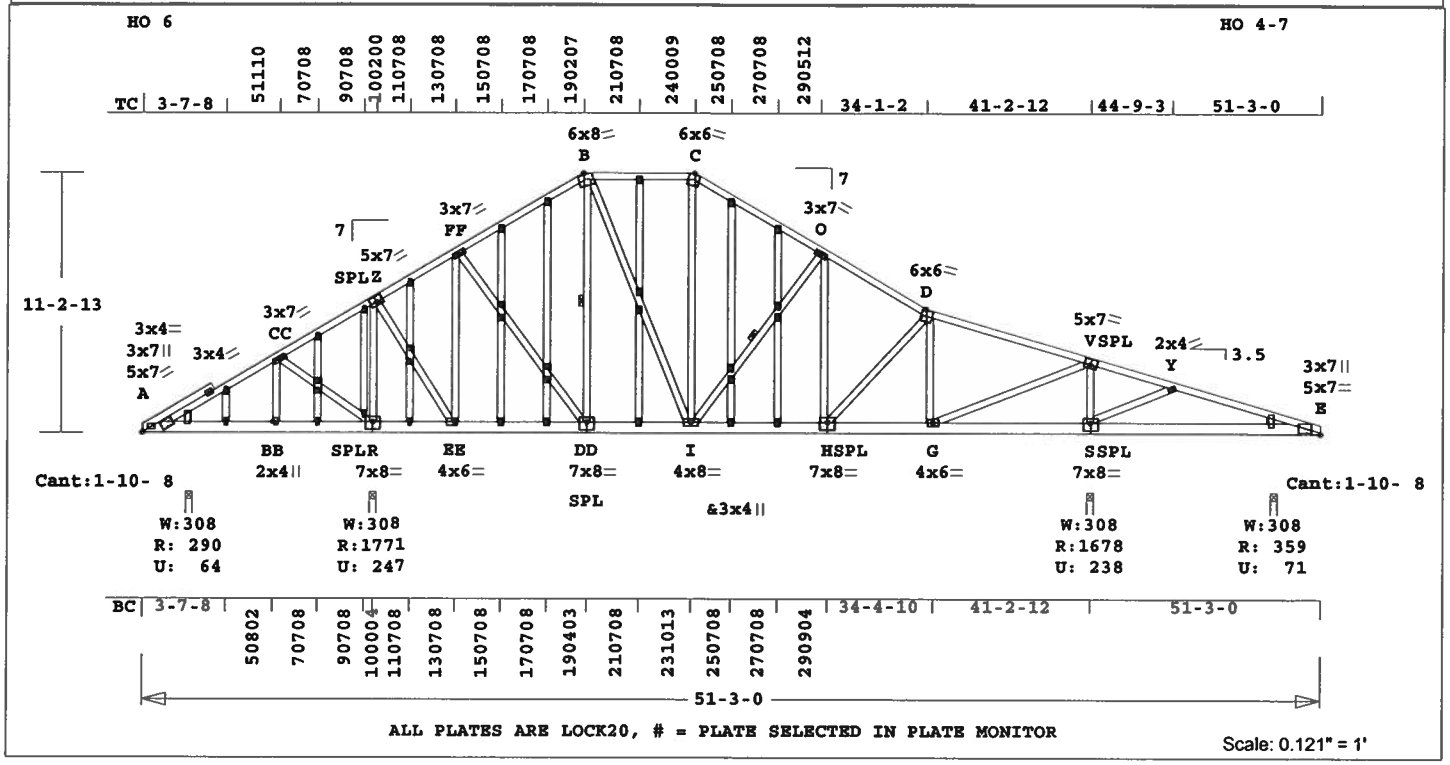
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	A2	1	SP	510300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN

Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1422 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
haygood-mclearn	A3	1	SP	510300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.121" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 583.4 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	Size	Lumber
TC	0.46	2x 4	SP-#2
BC	0.20	2x 6	SP-#2
WB	0.48	2x 4	SP-#2
GW	0.03	2x 4	SP-#2
WG	---	2x 6	SP-#2
EX E -E		2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 2- 7
TC 2- 0- 0	19- 2- 7	24- 0- 9
TC Cont.	24- 0- 9	51- 3- 0
BC Cont.	0- 0- 0	51- 3- 0

WB 1 rows CLB on DD-B
WB 1 rows CLB on I -O
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	291	65	3- 8	1- 8
		Hz =	-233	
R	1771	247	3- 8	1-14
S	1678	239	3- 8	1-13
E	360	72	3- 8	1- 8
		Hz =	210	

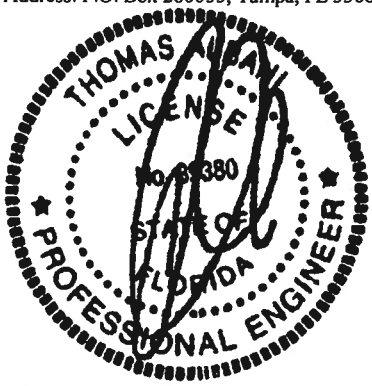
Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -KK	0.07	43 C	0.00	0.07
KK-CC	0.08	25 T	0.00	0.08
CC-MM	0.12	270 T	0.04	0.08
MM-QQ	0.12	291 T	0.05	0.07
QQ-Z	0.12	213 T	0.03	0.09
Z -SS	0.03	402 C	0.00	0.03
SS-FF	0.06	415 C	0.00	0.06
FF-WW	0.05	752 C	0.00	0.05
WW-A0	0.06	744 C	0.00	0.06
A0-B	0.06	721 C	0.00	0.06
B -GG	0.05	778 C	0.00	0.05
GG-C	0.09	777 C	0.00	0.09
C -O	0.26	907 C	0.00	0.26
O -D	0.26	1219 C	0.00	0.26
D -V	0.43	1211 C	0.01	0.42
V -Y	0.46	258 T	0.04	0.42
Y -E	0.11	36 T	0.00	0.11
Bottom Chords				
A -LL	0.09	203 T	0.00	0.09
LL-BB	0.09	203 T	0.00	0.09
BB-PP	0.07	203 T	0.00	0.07
PP-R	0.13	205 T	0.00	0.13
R -VV	0.13	242 C	0.00	0.13
VV-EE	0.11	241 C	0.00	0.11
EE-ZZ	0.15	357 T	0.04	0.11
ZZ-A3	0.15	361 T	0.04	0.11
A3-DD	0.15	366 T	0.04	0.11
DD-JJ	0.18	641 T	0.07	0.11
JJ-I	0.12	642 T	0.07	0.05
I -H	0.17	1060 T	0.12	0.05
H -G	0.17	1167 T	0.14	0.03
G -S	0.20	235 C	0.00	0.20
S -E	0.20	171 T	0.00	0.20
Webs				
BB-CC	0.03	187 T		
CC-OO	0.03	334 C	0.03	0.00
OO-RR	0.09	330 C	0.00	0.09
RR-R	0.08	478 C	0.00	0.08
R -Z	0.42	1137 C		
Z -TT	0.19	1060 T	0.19	0.00
TT-EE	0.20	1110 T	0.20	0.00
EE-FF	0.48	675 C		
FF-XX	0.12	474 T	0.08	0.04
XX-A2	0.11	476 T	0.08	0.03
A2-DD	0.09	459 T	0.08	0.01
DD-B	0.05	136 C		1 Br
B -HH	0.07	354 T	0.06	0.01
II-I	0.06	349 T	0.06	0.00
I -C	0.04	250 T		
I -O	0.13	469 C		1 Br
H -O	0.04	303 T		
H -D	0.08	155 C		
G -D	0.14	490 C		
G -V	0.27	1509 T		
S -V	0.17	1372 C		
S -Y	0.05	263 C		
Gable Webs				
LL-KK	0.00	98 C		
OO-MM	0.00	74 C		
PP-OO	0.03	93 C	0.00	0.03
RR-QQ	0.02	223 C	0.02	0.00
TT-SS	0.02	128 C	0.00	0.02
VV-TT	0.01	186 C	0.01	0.00
XX-WW	0.01	105 C	0.00	0.01
ZZ-XX	0.03	111 C	0.00	0.03
A2-A0	0.03	62 C	0.00	0.03
A3-A2	0.02	47 C	0.00	0.02
HH-GG	0.00	91 C		
JJ-II	0.01	85 C	0.00	0.01

LL Defl -0.04" in I -H L/999
TL Defl -0.09" in I -H L/999
Shear // Grain in D -V 0.28

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	5.0x 7.0	Ctr	-0.9	0.75
A LOCK	3.0x 7.0	Ctr	Ctr	0.00
KK LOCK	2.0x 4.0	Ctr	Ctr	0.55
CC LOCK	3.0x 7.0	Ctr	Ctr	0.47
MM LOCK	2.0x 4.0	Ctr	Ctr	0.55
QQ LOCK	2.0x 4.0	Ctr	Ctr	0.55
Z LOCK	5.0x 7.0	-0.3	0.5	0.86
SS LOCK	2.0x 4.0	Ctr	Ctr	0.55
FF LOCK	3.0x 7.0	Ctr	Ctr	0.51
WW#LOCK	3.0x 4.0	Ctr	Ctr	0.36
A0#LOCK	3.0x 4.0	Ctr	Ctr	0.36
B LOCK	6.0x 8.0	1.1	-3.9	0.80
GG#LOCK	3.0x 4.0	Ctr	Ctr	0.36
C LOCK	6.0x 6.0	-1.0	-3.7	0.75
O LOCK	3.0x 7.0	Ctr	Ctr	0.51
D LOCK	6.0x 6.0	Ctr	Ctr	0.88
V LOCK	5.0x 7.0	0.1	0.5	0.87
Y LOCK	2.0x 4.0	Ctr	Ctr	0.55
E LOCK	5.0x 7.0	Ctr	Ctr	0.87
E LOCK	3.0x 7.0	Ctr	Ctr	0.00
LL LOCK	2.0x 4.0	Ctr	Ctr	0.55

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

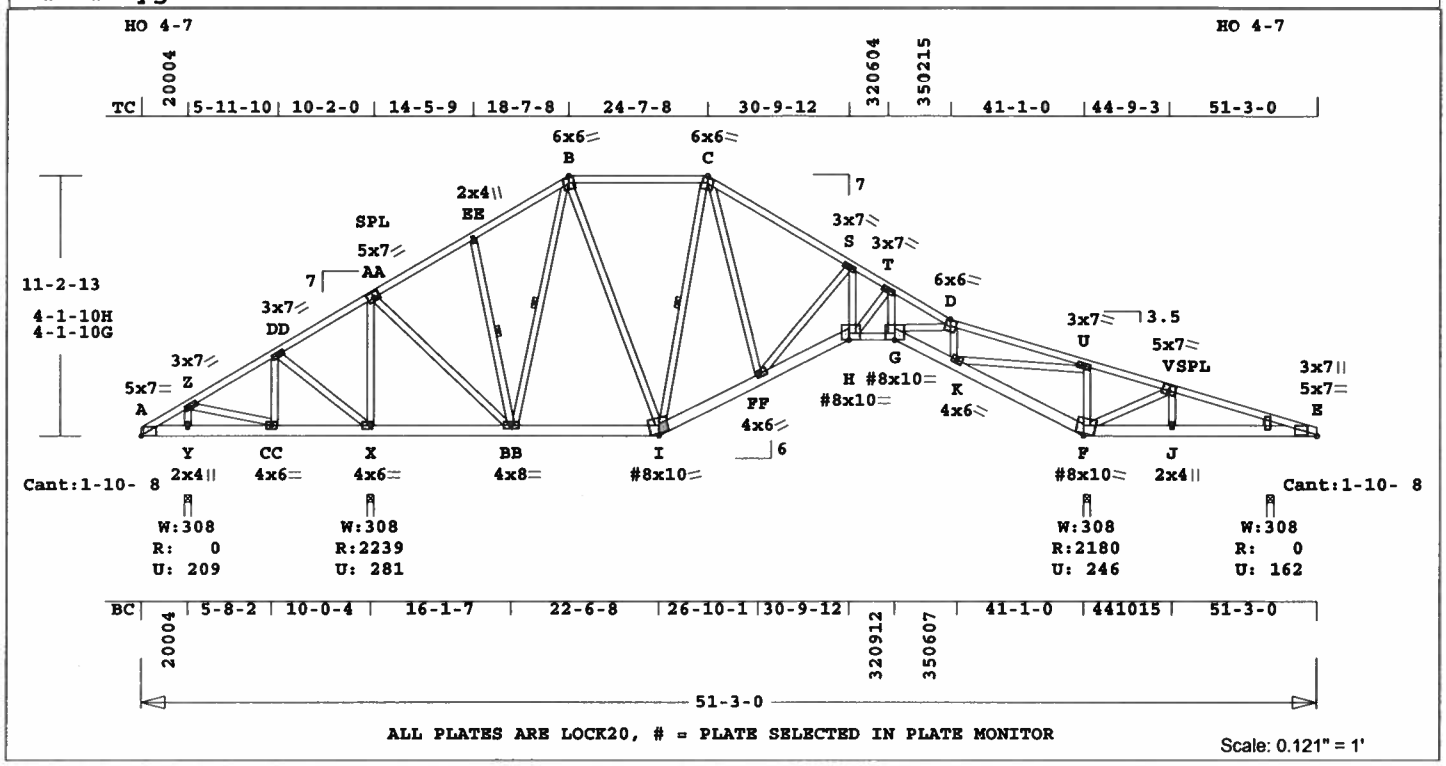
Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
haygood-mclearn	A3	1	SP	510300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN

```
BB LOCK  2.0x 4.0 Ctr Ctr 0.55
PP LOCK  2.0x 4.0 Ctr Ctr 0.55
R  LOCK  7.0x 8.0 Ctr-0.8 0.72
VV LOCK  2.0x 4.0 Ctr Ctr 0.55
EE LOCK  4.0x 6.0 Ctr Ctr 0.50
ZZ#LOCK  3.0x 4.0 Ctr Ctr 0.36
A3#LOCK  3.0x 4.0 Ctr Ctr 0.36
DD LOCK  7.0x 8.0 Ctr-0.8 0.72
JJ#LOCK  3.0x 4.0 Ctr Ctr 0.36
I  LOCK  4.0x 8.0 Ctr Ctr 0.57
H  LOCK  7.0x 8.0 Ctr-0.8 0.72
G  LOCK  4.0x 6.0 Ctr Ctr 0.50
S  LOCK  7.0x 8.0 Ctr-0.8 0.72
OO LOCK  4.0x 8.0 Ctr Ctr 0.74
RR LOCK  2.0x 4.0 Ctr Ctr 0.55
TT LOCK  4.0x10.0 Ctr Ctr 0.90
XX LOCK  4.0x10.0 Ctr Ctr 0.78
A2 LOCK  4.0x10.0 Ctr Ctr 0.78
HH#LOCK  3.0x 4.0 Ctr Ctr 0.50
II#LOCK  3.0x 4.0 Ctr Ctr 0.51

# = Plate Monitor used
  4 Gable studs to be attached
with 2.0x4.0 plates each end.

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL  33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2004
WARNING  Do Not Cut overframe
  member between outside of
  truss and first tie-plate
  to inside of heel plate.
Design checked for 10 psf non-
  concurrent LL on BC.
Refer to Gen Det 3 series for
  web bracing and plating.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
 Wind-Force Resistance System.
  Wind Speed:          110 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor   : 1.00
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       5.0 psf
  BC Dead Load :       5.0 psf
Max comp. force     1372 Lbs
Quality Control Factor 1.25
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	A4	6	SP	510300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 459.0 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	Size	Lumber
TC	0.63	2x 4	SP-#2
BC	0.31	2x 6	SP-#2
EX H -G		2x 4	SP-#2
WB	0.77	2x 4	SP-#2
WG	---	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	51- 3- 0
BC Cont.		0- 0- 0	51- 3- 0

WB 1 rows CLB on EE-BB
WB 1 rows CLB on BB-B
WB 1 rows CLB on I -C
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
Y	0	210	3- 8	1- 8
		Hz =	-234	
X	2239	282	3- 8	2- 6
F	2181	246	3- 8	2- 5
E	0	163	3- 8	1- 8
		Hz =	212	

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -Z	0.10	138 T	0.02	0.08
Z -DD	0.28	683 T	0.12	0.16
DD-AA	0.36	1064 T	0.18	0.18
AA-EE	0.18	136 T	0.00	0.18
EE-B	0.17	197 T	0.00	0.17
B -C	0.27	341 C	0.00	0.27
C -S	0.30	632 C	0.00	0.30
S -T	0.30	1151 C	0.00	0.30
T -D	0.25	1221 C	0.01	0.24
D -U	0.31	190 C	0.00	0.31
U -V	0.63	1893 T	0.32	0.31
V -E	0.38	999 T	0.17	0.21
Bottom Chords				
A -Y	0.01	102 C	0.00	0.01
Y -CC	0.03	218 T	0.00	0.03
CC-X	0.10	585 C	0.00	0.10
X -BB	0.11	918 C	0.00	0.11
BB-I	0.12	202 T	0.01	0.11
I -FF	0.11	488 T	0.06	0.05
FF-H	0.25	1144 T	0.14	0.11
H -G	0.37	1014 T	0.17	0.20
G -K	0.17	237 T	0.03	0.14
K -F	0.19	1999 C	0.01	0.18
F -J	0.28	950 C	0.00	0.28
J -E	0.31	950 C	0.00	0.31
Webs				
Y -Z	0.04	267 T		
Z -CC	0.10	503 C		
CC-DD	0.03	258 T		
DD-X	0.14	427 C		
X -AA	0.77	1839 C		
AA-BB	0.26	1421 T		
EE-BB	0.06	240 C		1 Br
BB-B	0.24	624 C		1 Br
B -I	0.06	377 T		
I -C	0.18	486 C		1 Br
C -FF	0.08	438 T		
FF-S	0.38	778 C		
H -S	0.09	507 T		
H -T	0.01	69 T		
G -T	0.00	49 T		
G -D	0.14	782 T		
K -D	0.09	918 C		
K -U	0.36	1949 T		
F -U	0.11	920 C		
F -V	0.19	945 C		
J -V	0.07	384 T		

LL Defl -0.10" in FF-H L/999
TL Defl -0.21" in FF-H L/999
Shear // Grain in D -U 0.25

Hz Disp	LL	DL	TL
Jt F	0.07"	0.07"	0.14"

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	5.0x 7.0	0.1-0.4		0.75
Z LOCK	3.0x 7.0	Ctr	Ctr	0.47
DD LOCK	3.0x 7.0	Ctr	Ctr	0.47
AA LOCK	5.0x 7.0-0.3	0.5		0.86
EE LOCK	2.0x 4.0	Ctr	Ctr	0.55
B LOCK	6.0x 6.0	0.1-4.2		0.91
C LOCK	6.0x 6.0-0.1-4.2			0.95
S LOCK	3.0x 7.0	Ctr	Ctr	0.49
T LOCK	3.0x 7.0	Ctr	Ctr	0.51
D LOCK	6.0x 6.0	Ctr	Ctr	0.88
U LOCK	3.0x 7.0	Ctr	Ctr	0.91
V LOCK	5.0x 7.0	0.1	0.5	0.87
E LOCK	5.0x 7.0	Ctr	Ctr	0.87
E LOCK	3.0x 7.0	Ctr	Ctr	0.00
Y LOCK	2.0x 4.0	Ctr	Ctr	0.55
CC LOCK	4.0x 6.0	Ctr	Ctr	0.72
X LOCK	4.0x 6.0	Ctr	Ctr	0.47
BB LOCK	4.0x 8.0	Ctr	Ctr	0.66
I# LOCK	8.0x10.0-0.4	5.4		0.64
FF LOCK	4.0x 6.0	Ctr	Ctr	0.50
H# LOCK	8.0x10.0	1.0	0.6	0.76
G# LOCK	8.0x10.0	Ctr	0.6	0.76
K LOCK	4.0x 6.0	Ctr	Ctr	0.56
F# LOCK	8.0x10.0	1.4	5.2	0.68
J LOCK	2.0x 4.0	Ctr	Ctr	0.55

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

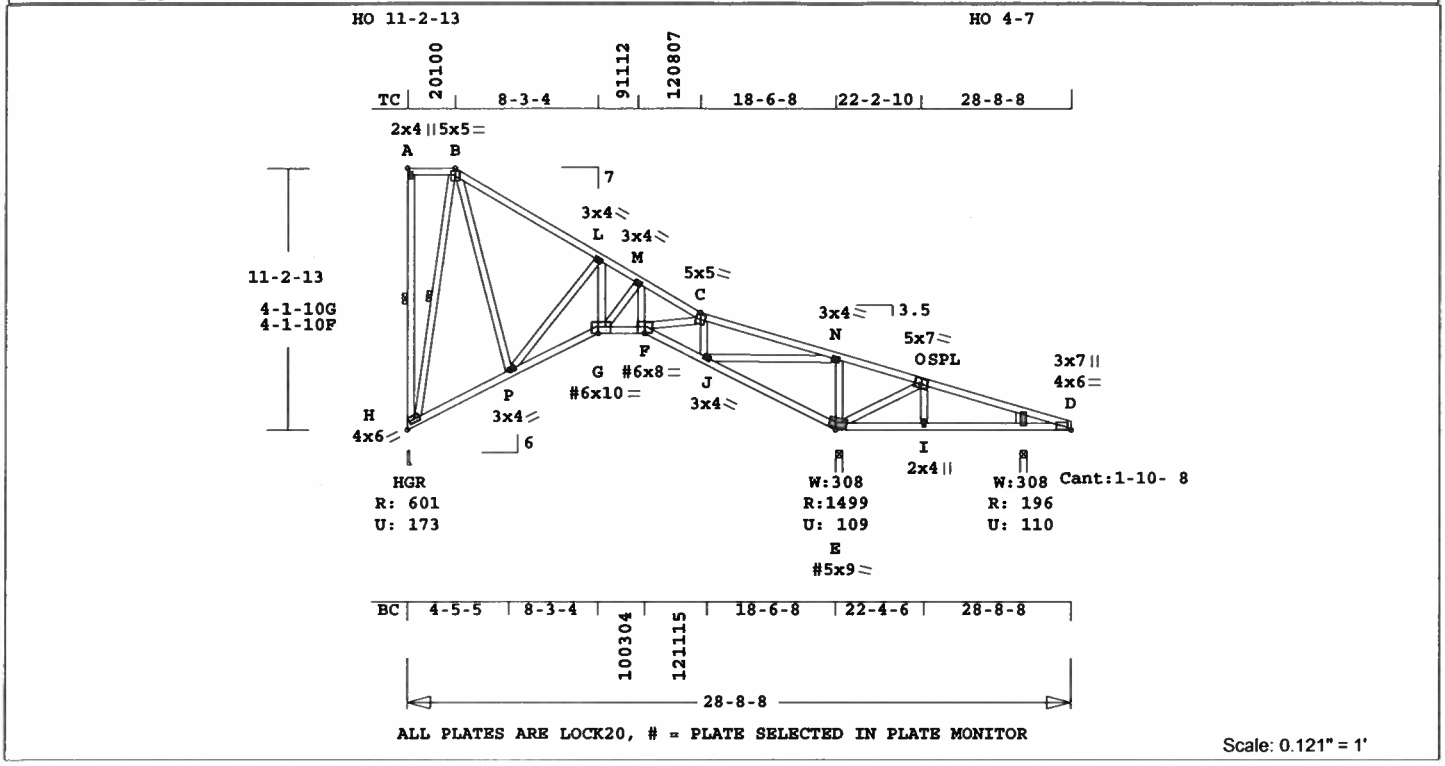
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	A4	6	SP	510300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN

```
Design checked for 10 psf non-
  concurrent LL on BC.
Prevent truss rotation at all
  bearing locations.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
 Wind-Force Resistance System.
  Wind Speed:          110 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor   : 1.00
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       5.0 psf
  BC Dead Load :       5.0 psf
Max comp. force     1999 Lbs
Quality Control Factor 1.25
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	A5	4	SP	280808	3.5	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 236.8 LBS
Tampa, FL 33682

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.41	2x 4	SP-#2
WG	---	2x 6	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	2- 0- 0	0- 0- 0	2- 1- 0
TC Cont.		2- 1- 0	28- 8- 8
BC Cont.		0- 0- 0	28- 8- 8

WB 1 rows CLB on H -A
WB 1 rows CLB on H -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
H	601	173	3- 8	1- 8
			Hz =	-378
E	1499	110	3- 8	1-10
D	197	110	3- 8	1- 8
			Hz =	145

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -B	0.10	137	C	0.00 0.10
B -L	0.33	277	C	0.00 0.33
L -M	0.33	830	C	0.00 0.33
M -C	0.15	1083	C	0.00 0.15
C -N	0.28	615	C	0.00 0.28
N -O	0.43	874	T	0.15 0.28
O -D	0.23	282	T	0.05 0.18
Bottom Chords				
H -P	0.15	328	T	0.01 0.14
P -G	0.24	835	T	0.14 0.10
G -F	0.28	896	T	0.15 0.13
F -J	0.20	695	T	0.11 0.09
J -E	0.34	915	C	0.00 0.34
E -I	0.32	264	C	0.00 0.32
I -D	0.22	264	C	0.00 0.22
Webs				
H -A	0.36	32	C	WindLd 1 Br
H -B	0.23	599	C	1 Br
B -P	0.08	470	T	
P -L	0.41	841	C	
G -L	0.10	584	T	
G -M	0.02	184	C	
F -M	0.05	287	T	
F -C	0.05	286	T	
J -C	0.07	690	C	
J -N	0.25	1391	T	
E -N	0.10	765	C	
E -O	0.13	636	C	
I -O	0.03	248	T	

LL Defl -0.06" in F -J L/999
TL Defl -0.13" in F -J L/999
Shear // Grain in C -N 0.24

Hz Disp	LL	DL	TL
Jt E	0.07"	0.06"	0.13"

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.41
B	LOCK	5.0x 5.0	Ctr	-0.5	0.99
L	LOCK	3.0x 4.0	Ctr	Ctr	0.70
M	LOCK	3.0x 4.0	Ctr	Ctr	0.70
C	LOCK	5.0x 5.0	Ctr	Ctr	0.81
N	LOCK	3.0x 4.0	Ctr	Ctr	0.70
O	LOCK	5.0x 7.0	0.1	0.5	0.67
D	LOCK	4.0x 6.0	Ctr	Ctr	0.73
D	LOCK	3.0x 7.0	Ctr	Ctr	0.00
H	LOCK	4.0x 6.0	0.1	Ctr	0.61
P	LOCK	3.0x 4.0	Ctr	Ctr	0.58
G#	LOCK	6.0x10.0	1.4	-0.5	0.58
F#	LOCK	6.0x 8.0	0.1	-0.2	0.62
J	LOCK	3.0x 4.0	Ctr	Ctr	0.68
E#	LOCK	5.0x 9.0	1.1	3.6	0.63
I	LOCK	2.0x 4.0	Ctr	Ctr	0.40

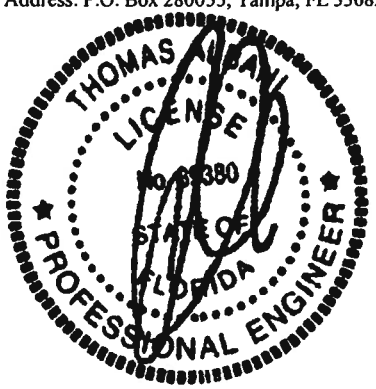
= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Provide connection to bearing for 379 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1083 Lbs
Quality Control Factor 1.25

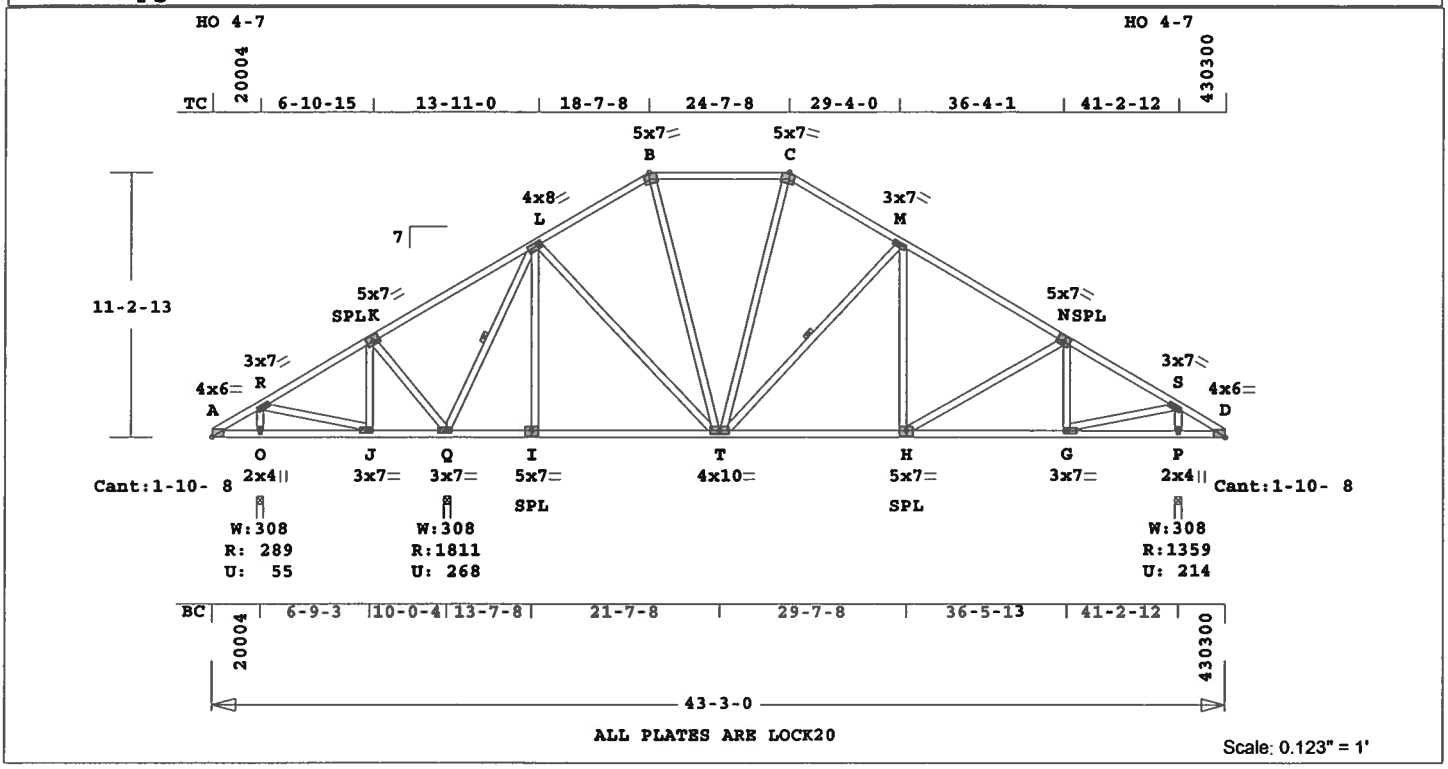
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	B1	6	HIPP	430300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20

Scale: 0.123" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 362.5 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.44	2x 4	SP-#2
BC	0.46	2x 4	SP-#2
WB	0.41	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18- 7- 8
TC 2- 0- 0	18- 7- 8	24- 7- 8
TC Cont.	24- 7- 8	43- 3- 0
BC Cont.	0- 0- 0	43- 3- 0

WB 1 rows CLB on Q -L
WB 1 rows CLB on T -M
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
O	289	56	3- 8	1- 8
			Hz =	-239
Q	1811	268	3- 8	1-15
P	1359	214	3- 8	1- 8
			Hz =	240

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -R	0.10	88	T	0.01	0.09
R -K	0.42	79	T	0.02	0.40
K -L	0.44	358	T	0.04	0.40
L -B	0.36	816	C	0.00	0.36
B -C	0.32	720	C	0.00	0.32
C -M	0.36	816	C	0.00	0.36
M -N	0.38	1242	C	0.03	0.35
N -S	0.37	1394	C	0.01	0.36
S -D	0.13	64	T	0.01	0.12
Bottom Chords					
A -O	0.09	62	C	0.00	0.09
O -J	0.09	237	T	0.00	0.09
J -Q	0.09	173	T	0.00	0.09
Q -I	0.25	349	T	0.03	0.22
I -T	0.38	349	T	0.03	0.35
T -H	0.46	1073	T	0.11	0.35
H -G	0.41	1216	T	0.12	0.29
G -P	0.18	244	T	0.00	0.18
P -D	0.09	45	T	0.00	0.09
Webs					
O -R	0.01	195	C		
R -J	0.01	66	C		
J -K	0.02	177	T		
K -Q	0.13	407	C		
Q -L	0.41	1579	C		1 Br
I -L	0.03	251	T		
L -T	0.09	494	T		
B -T	0.03	196	T		
T -C	0.03	196	T		
T -M	0.22	543	C		1 Br
H -M	0.05	361	T		
H -N	0.13	164	C		
G -N	0.04	215	C		
G -S	0.23	1296	T		
P -S	0.12	1261	C		

LL Defl -0.07" in I -T L/999
TL Defl -0.15" in I -T L/999
LL Cant 0.00" in P -D L/999
Shear // Grain in K -L 0.24

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	0.3	0.2	0.76
R	LOCK	3.0x 7.0	Ctr	Ctr	0.44
K	LOCK	5.0x 7.0	-0.3	0.5	0.81
L	LOCK	4.0x 8.0	Ctr	Ctr	0.89
B	LOCK	5.0x 7.0	0.9	-3.3	0.71
C	LOCK	5.0x 7.0	-0.9	-3.3	0.71
M	LOCK	3.0x 7.0	Ctr	Ctr	0.46
N	LOCK	5.0x 7.0	0.3	0.5	0.81
S	LOCK	3.0x 7.0	Ctr	Ctr	0.44
D	LOCK	4.0x 6.0	-0.3	0.2	0.76
O	LOCK	2.0x 4.0	Ctr	Ctr	0.50
J	LOCK	3.0x 7.0	Ctr	Ctr	0.91
Q	LOCK	3.0x 7.0	-0.9	Ctr	0.59
I	LOCK	5.0x 7.0	Ctr	-0.5	0.83
T	LOCK	4.0x10.0	Ctr	Ctr	0.67
H	LOCK	5.0x 7.0	Ctr	-0.5	0.83
G	LOCK	3.0x 7.0	Ctr	Ctr	0.91
P	LOCK	2.0x 4.0	Ctr	Ctr	0.50

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

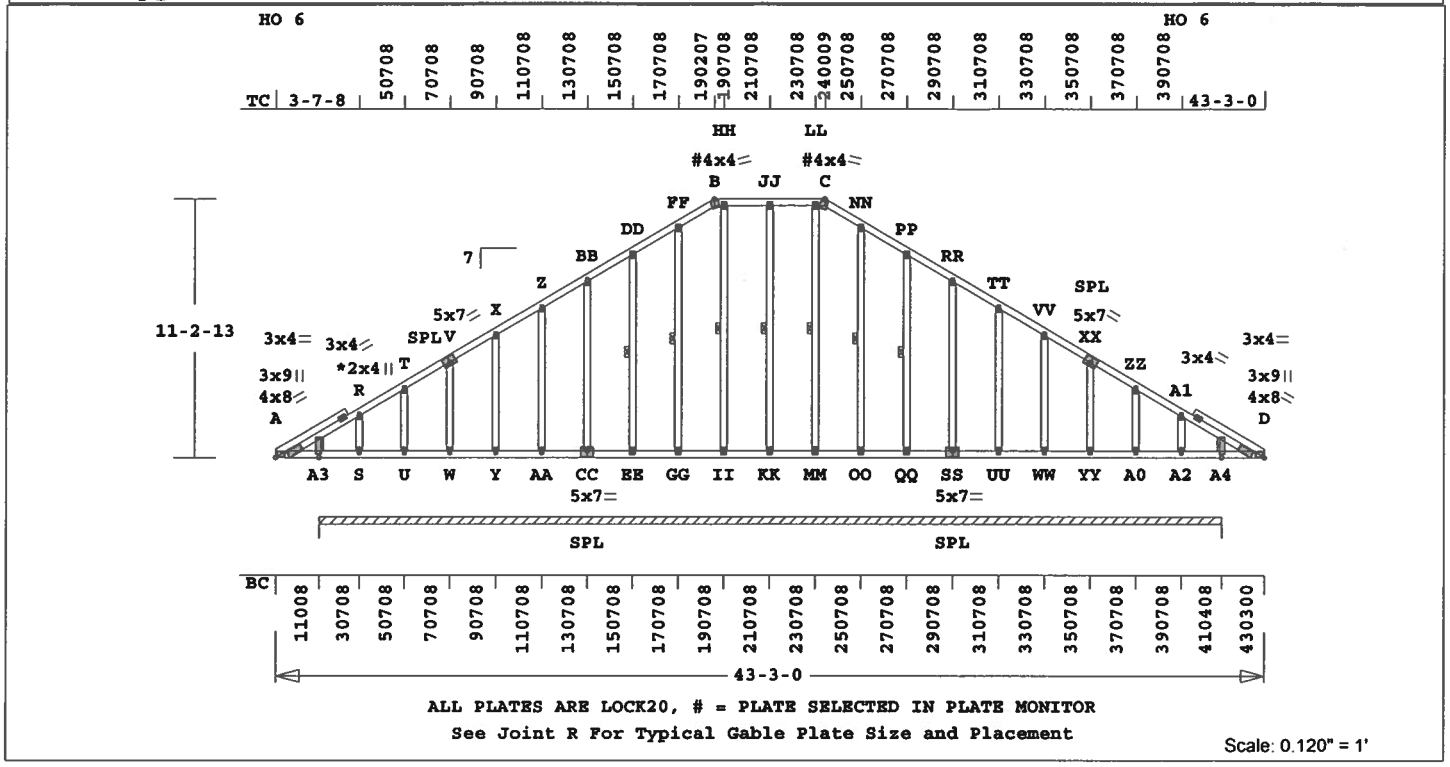
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1579 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	B2	1	HIPP	430300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

See Joint R For Typical Gable Plate Size and Placement

Scale: 0.120" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 426.7 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	Size	Lumber
TC	0.06	2x 4	SP-#2
BC	0.06	2x 4	SP-#2
GW	0.08	2x 4	SP-#2
WG	---	2x 8	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	19- 2- 7
TC 2- 0- 0	19- 2- 7	24- 0- 9
TC Cont.	24- 0- 9	43- 3- 0
BC Cont.	0- 0- 0	43- 3- 0

WB 1 rows CLB on EE-DD
WB 1 rows CLB on GG-FF
WB 1 rows CLB on II-HH
WB 1 rows CLB on KK-JJ
WB 1 rows CLB on MM-LL
WB 1 rows CLB on OO-NN
WB 1 rows CLB on QQ-PP
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 1-10- 8		to 41- 4- 8	
	3460	538	Hz =	240

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -R	0.06	127 C	0.00	0.06	
R -T	0.05	110 C	0.00	0.05	
T -V	0.03	100 C	0.00	0.03	
V -X	0.03	86 C	0.00	0.03	
X -Z	0.03	73 C	0.00	0.03	
Z -BB	0.03	110 T	0.00	0.03	
BB-DD	0.03	151 T	0.00	0.03	
DD-FF	0.03	196 T	0.00	0.03	
FF-B	0.04	224 T	0.02	0.02	
B -HH	0.03	214 T	0.00	0.03	
HH-JJ	0.04	214 T	0.02	0.02	
JJ-LL	0.04	214 T	0.02	0.02	
LL-C	0.03	214 T	0.00	0.03	
C -NN	0.04	224 T	0.02	0.02	
NN-PP	0.03	196 T	0.00	0.03	
PP-RR	0.03	151 T	0.00	0.03	
RR-TT	0.03	110 T	0.00	0.03	
TT-VV	0.03	73 C	0.00	0.03	
VV-XX	0.03	86 C	0.00	0.03	
XX-ZZ	0.03	100 C	0.00	0.03	
ZZ-A1	0.05	110 C	0.00	0.05	
A1-D	0.06	127 C	0.00	0.06	
Bottom Chords					
A -S	0.06	0 T	0.00	0.06	
S -U	0.03	0 T	0.00	0.03	
U -W	0.02	0 T	0.00	0.02	
W -Y	0.02	0 T	0.00	0.02	
Y -AA	0.02	0 T	0.00	0.02	
AA-CC	0.02	0 T	0.00	0.02	
CC-EE	0.02	0 T	0.00	0.02	
EE-GG	0.02	0 T	0.00	0.02	
GG-II	0.02	0 T	0.00	0.02	
II-KK	0.02	0 T	0.00	0.02	
KK-MM	0.02	0 T	0.00	0.02	
MM-OO	0.02	0 T	0.00	0.02	
OO-QQ	0.02	0 T	0.00	0.02	
QQ-SS	0.02	0 T	0.00	0.02	
SS-UU	0.02	0 T	0.00	0.02	
UU-WW	0.02	0 T	0.00	0.02	
WW-YY	0.02	0 T	0.00	0.02	
YY-A0	0.02	0 T	0.00	0.02	
A0-A2	0.03	0 T	0.00	0.03	
A2-D	0.06	0 T	0.00	0.06	
Gable Webs					
S -R	0.01	145 C			
U -T	0.01	113 C			
W -V	0.02	121 C			
Y -X	0.04	119 C			
AA-Z	0.06	120 C			
CC-BB	0.08	119 C			
EE-DD	0.03	120 C			1 Br
GG-FF	0.04	118 C			1 Br
II-HH	0.04	108 C			1 Br
KK-JJ	0.04	123 C			1 Br
MM-LL	0.04	108 C			1 Br
OO-NN	0.04	118 C			1 Br
QQ-PP	0.03	120 C			1 Br
SS-RR	0.08	119 C			
UU-TT	0.06	120 C			
WW-VV	0.04	119 C			
YY-XX	0.02	121 C			
A0-ZZ	0.01	113 C			
A2-A1	0.01	145 C			

LL Defl 0.00" in A2-A4 L/999
TL Defl 0.00" in A2-A4 L/999
Shear // Grain in A3-R 0.09

Plates for each ply each face.
PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	4.0x 8.0	0.9	0.2	0.71
A LOCK	3.0x 9.0	Ctr	Ctr	0.00
R LOCK	2.0x 4.0	Ctr	Ctr	0.00
T LOCK	2.0x 4.0	Ctr	Ctr	0.00
V LOCK	5.0x 7.0-0.3	0.5		0.81
X LOCK	2.0x 4.0	Ctr	Ctr	0.00
Z LOCK	2.0x 4.0	Ctr	Ctr	0.00
BB LOCK	2.0x 4.0	Ctr	Ctr	0.00
DD LOCK	2.0x 4.0	Ctr	Ctr	0.00
FF LOCK	2.0x 4.0	Ctr	Ctr	0.00
B# LOCK	4.0x 4.0	0.3-0.8		0.80
HH LOCK	2.0x 4.0	Ctr	Ctr	0.00
JJ LOCK	2.0x 4.0	Ctr	Ctr	0.00
LL LOCK	2.0x 4.0	Ctr	Ctr	0.00
C# LOCK	4.0x 4.0-0.9-1.6			0.95
NN LOCK	2.0x 4.0	Ctr	Ctr	0.00
PP LOCK	2.0x 4.0	Ctr	Ctr	0.00
RR LOCK	2.0x 4.0	Ctr	Ctr	0.00
TT LOCK	2.0x 4.0	Ctr	Ctr	0.00
VV LOCK	2.0x 4.0	Ctr	Ctr	0.00
XX LOCK	5.0x 7.0	0.3	0.5	0.81
ZZ LOCK	2.0x 4.0	Ctr	Ctr	0.00
A1 LOCK	2.0x 4.0	Ctr	Ctr	0.00
D LOCK	4.0x 8.0-0.9	0.2		0.71
D LOCK	3.0x 9.0	Ctr	Ctr	0.00
S LOCK	2.0x 4.0	Ctr	Ctr	0.00
U LOCK	2.0x 4.0	Ctr	Ctr	0.00

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

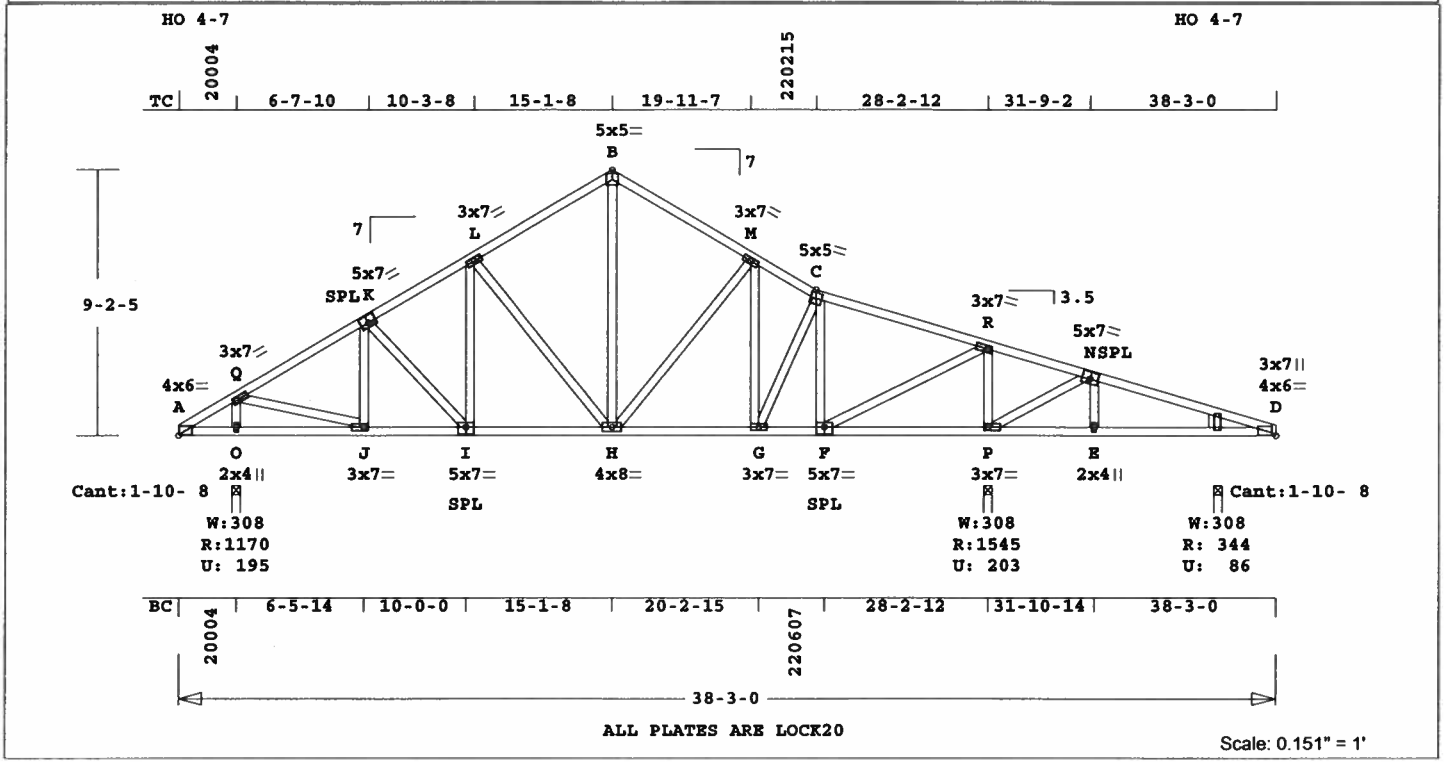
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	B2	1	HIPP	430300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN

```
W  LOCK  2.0x 4.0 Ctr Ctr 0.00
Y  LOCK  2.0x 4.0 Ctr Ctr 0.00
AA LOCK  2.0x 4.0 Ctr Ctr 0.00
CC LOCK  5.0x 7.0 Ctr-0.5 0.83
EE LOCK  2.0x 4.0 Ctr Ctr 0.00
GG LOCK  2.0x 4.0 Ctr Ctr 0.00
II LOCK  2.0x 4.0 Ctr Ctr 0.00
KK LOCK  2.0x 4.0 Ctr Ctr 0.00
MM LOCK  2.0x 4.0 Ctr Ctr 0.00
OO LOCK  2.0x 4.0 Ctr Ctr 0.00
QQ LOCK  2.0x 4.0 Ctr Ctr 0.00
SS LOCK  5.0x 7.0 Ctr-0.5 0.83
UU LOCK  2.0x 4.0 Ctr Ctr 0.00
WW LOCK  2.0x 4.0 Ctr Ctr 0.00
YY LOCK  2.0x 4.0 Ctr Ctr 0.00
A0 LOCK  2.0x 4.0 Ctr Ctr 0.00
A2 LOCK  2.0x 4.0 Ctr Ctr 0.00

# = Plate Monitor used

REVIEWED BY:
 Robbins Engineering, Inc.
 PO Box 280055
 Tampa, FL  33682

REFER TO ROBBINS ENG. GENERAL
NOTES AND SYMBOLS SHEET FOR
ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
  Mayo Truss Co. Inc.
Analysis Conforms To:
  FBC2004
WARNING  Do Not Cut overframe
  member between outside of
  truss and first tie-plate
  to inside of heel plate.
Design checked for 10 psf non-
  concurrent LL on BC.
Prevent truss rotation at all
  bearing locations.
Refer to Gen Det 3 series for
  web bracing and plating.
NOTE: USER MODIFIED PLATES
  This design may have plates
  selected through a plate
  monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
 Wind-Force Resistance System.
  Wind Speed:          110 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor  : 1.00
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       5.0 psf
  BC Dead Load :       5.0 psf
Max comp. force      145 Lbs
Quality Control Factor 1.25
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	C1	6	SP	380300	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 294.4 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 4	SP-#2
BC	0.24	2x 4	SP-#2
WB	0.24	2x 4	SP-#2
WG	---	2x 6	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	38- 3- 0
BC Cont.	0- 0- 0	38- 3- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
O	1170	195	3- 8	1- 8
			Hz =	-179
P	1545	203	3- 8	1-10
D	345	86	3- 8	1- 8
			Hz =	153

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -Q	0.18	48	T	0.01	0.17
Q -K	0.17	1096	C	0.00	0.17
K -L	0.17	1064	C	0.00	0.17
L -B	0.17	827	C	0.00	0.17
B -M	0.17	826	C	0.00	0.17
M -C	0.15	978	C	0.00	0.15
C -R	0.31	865	C	0.00	0.31
R -N	0.37	347	T	0.06	0.31
N -D	0.11	30	T	0.00	0.11
--------Bottom Chords---------					
A -O	0.10	37	T	0.00	0.10
O -J	0.10	196	T	0.00	0.10
J -I	0.20	944	T	0.15	0.05
I -H	0.24	923	T	0.15	0.09
H -G	0.23	852	T	0.14	0.09
G -F	0.20	835	T	0.14	0.06
F -P	0.17	325	C	0.00	0.17
P -E	0.17	146	T	0.00	0.17
E -D	0.06	146	T	0.00	0.06
----------Webs----------					
O -Q	0.10	1072	C		
Q -J	0.18	997	T		
J -K	0.03	195	C		
K -I	0.02	63	C		
I -L	0.02	187	T		
L -H	0.24	327	C		
H -B	0.10	529	T		
H -M	0.16	217	C		
G -M	0.01	95	T		
G -C	0.00	42	T		
F -C	0.14	506	C		
F -R	0.23	1293	T		
P -R	0.17	1260	C		
P -N	0.08	375	C		
E -N	0.02	138	T		

LL Defl -0.03" in H -G L/999
TL Defl -0.08" in H -G L/999
LL Cant 0.00" in A -O L/999
Shear // Grain in C -R 0.24

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

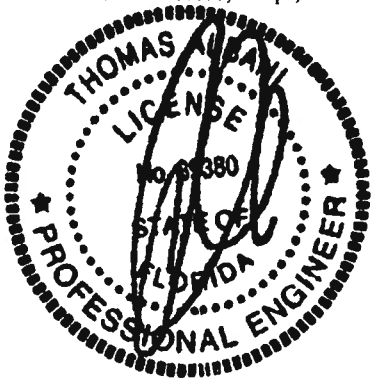
Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	4.0x	6.0	0.3	0.2	0.72
Q	LOCK	3.0x	7.0	Ctr	Ctr	0.43
K	LOCK	5.0x	7.0	-0.3	0.5	0.76
L	LOCK	3.0x	7.0	Ctr	Ctr	0.46
B	LOCK	5.0x	5.0	Ctr	Ctr	0.68
M	LOCK	3.0x	7.0	Ctr	Ctr	0.46
C	LOCK	5.0x	5.0	Ctr	Ctr	0.93
R	LOCK	3.0x	7.0	Ctr	Ctr	0.47
N	LOCK	5.0x	7.0	0.1	0.5	0.77
D	LOCK	4.0x	6.0	Ctr	Ctr	0.84
D	LOCK	3.0x	7.0	Ctr	Ctr	0.00
O	LOCK	2.0x	4.0	Ctr	Ctr	0.46
J	LOCK	3.0x	7.0	Ctr	Ctr	0.88
I	LOCK	5.0x	7.0	Ctr	-0.5	0.77
H	LOCK	4.0x	8.0	Ctr	Ctr	0.43
G	LOCK	3.0x	7.0	Ctr	Ctr	0.55
F	LOCK	5.0x	7.0	Ctr	-0.5	0.77
P	LOCK	3.0x	7.0	Ctr	Ctr	0.43
E	LOCK	2.0x	4.0	Ctr	Ctr	0.46

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1260 Lbs
Quality Control Factor 1.25

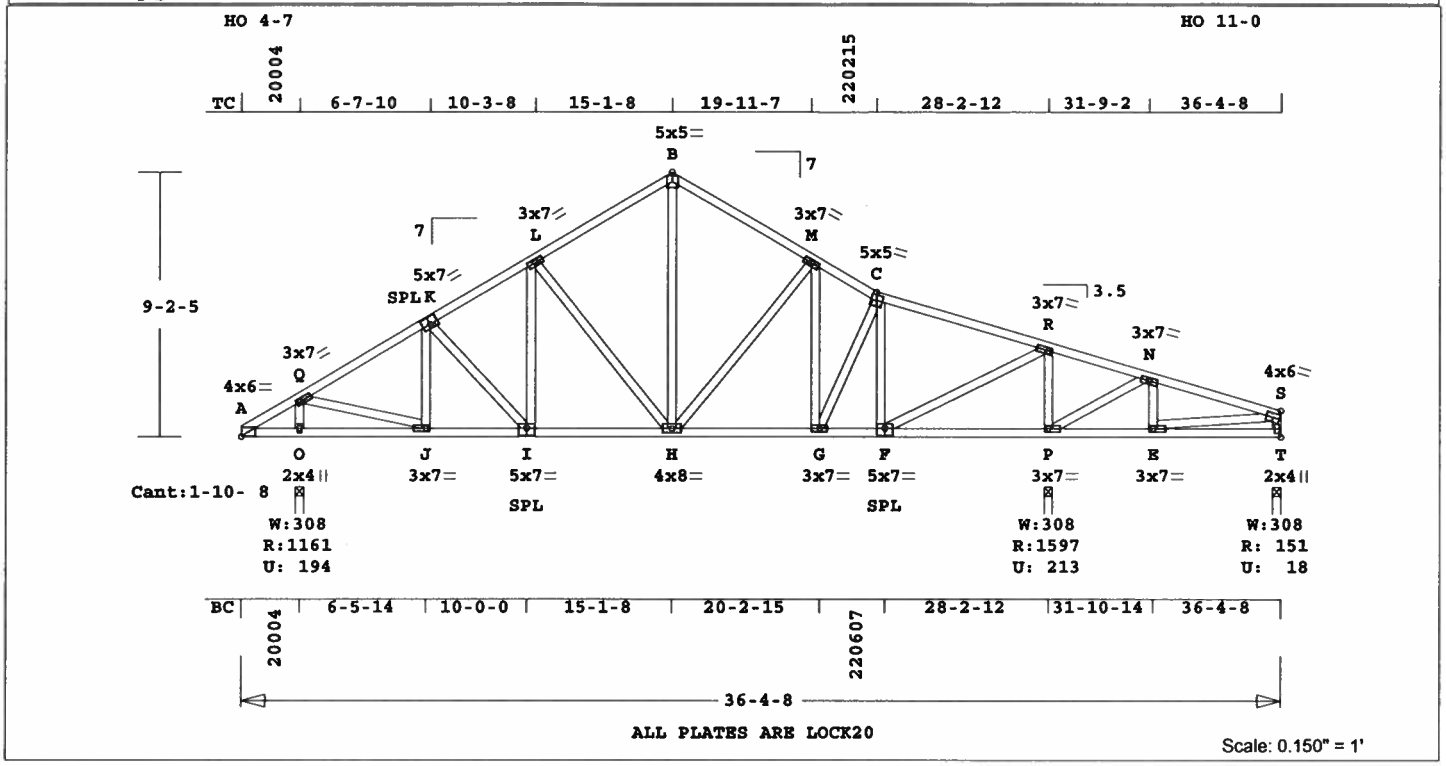
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	C2	1	SP	360408	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 291.1 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.37	2x 4	SP-#2
BC	0.24	2x 4	SP-#2
WB	0.24	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	36- 4- 8
BC Cont.		0- 0- 0	36- 4- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
O	1161	194	3- 8	1- 8
			Hz =	-182
P	1598	214	3- 8	1-11
T	151	18	3- 8	1- 8
			Hz =	157

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -Q	0.18	48	T 0.01	0.17
Q -K	0.17	1084	C 0.00	0.17
K -L	0.17	1050	C 0.00	0.17
L -B	0.17	812	C 0.00	0.17
B -M	0.17	810	C 0.00	0.17
M -C	0.15	947	C 0.00	0.15
C -R	0.30	824	C 0.00	0.30
R -N	0.37	427	T 0.07	0.30
N -S	0.18	56	T 0.00	0.18
Bottom Chords				
A -O	0.10	37	T 0.00	0.10
O -J	0.10	198	T 0.00	0.10
J -I	0.20	934	T 0.15	0.05
I -H	0.24	910	T 0.15	0.09
H -G	0.23	825	T 0.08	0.15
G -F	0.20	797	T 0.08	0.12
F -P	0.16	404	C 0.00	0.16
P -E	0.16	94	T 0.00	0.16
E -T	0.10	152	T 0.00	0.10
Webs				
O -Q	0.10	1063	C	
Q -J	0.18	986	T	
J -K	0.03	192	C	
K -I	0.02	63	C	
I -L	0.02	188	T	
L -H	0.24	329	C	
H -B	0.10	514	T	
H -M	0.14	196	C	
G -M	0.01	74	T	
G -C	0.01	64	T	
F -C	0.15	526	C	
F -R	0.24	1338	T	
P -R	0.17	1285	C	
P -N	0.09	434	C	
E -N	0.02	165	T	
E -S	0.01	59	C	
T -S	0.01	110	C WindLd	

LL Defl -0.03" in H -G L/999
TL Defl -0.08" in H -G L/999
LL Cant 0.00" in A -O L/999
Shear // Grain in C -R 0.24

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

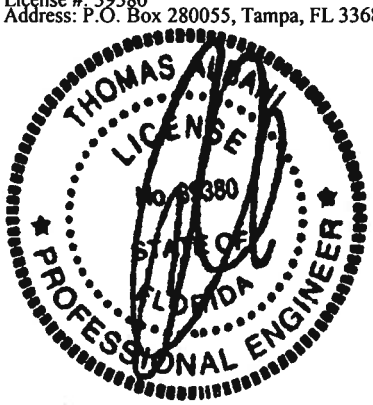
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	0.3	0.2	0.70
Q	LOCK	3.0x 7.0	Ctr	Ctr	0.42
K	LOCK	5.0x 7.0	-0.3	0.5	0.74
L	LOCK	3.0x 7.0	Ctr	Ctr	0.45
B	LOCK	5.0x 5.0	Ctr	Ctr	0.66
M	LOCK	3.0x 7.0	Ctr	Ctr	0.45
C	LOCK	5.0x 5.0	Ctr	Ctr	0.90
R	LOCK	3.0x 7.0	Ctr	Ctr	0.48
N	LOCK	3.0x 7.0	Ctr	Ctr	0.52
S	LOCK	4.0x 6.0	Ctr	Ctr	0.76
O	LOCK	2.0x 4.0	Ctr	Ctr	0.45
J	LOCK	3.0x 7.0	Ctr	Ctr	0.87
I	LOCK	5.0x 7.0	Ctr	-0.5	0.75
H	LOCK	4.0x 8.0	Ctr	Ctr	0.43
G	LOCK	3.0x 7.0	Ctr	Ctr	0.54
F	LOCK	5.0x 7.0	Ctr	-0.5	0.75
P	LOCK	3.0x 7.0	Ctr	Ctr	0.43
E	LOCK	3.0x 7.0	Ctr	Ctr	0.85
T	LOCK	2.0x 4.0	Ctr	Ctr	0.50

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1285 Lbs
Quality Control Factor 1.25

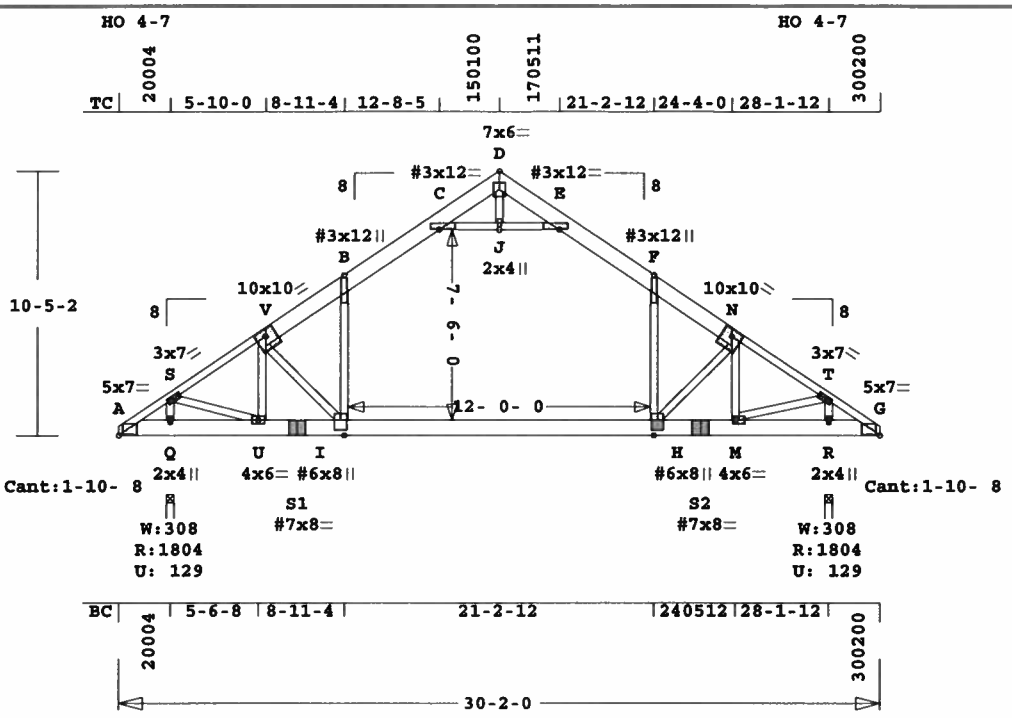
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D1	3	ATI2	300200	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.132" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 303.6 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	Size	Lumber
TC	0.23	2x 4	SP-#2
EX V -D		2x 8	SP-#1
EX D -N		2x 8	SP-#1
BC	0.37	2x 8	SP-#2
EX S1-S2		2x 8	SP-#1
WB	0.32	2x 4	SP-#2
ACT	0.21	2x 4	SP-#2
AWT	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	30- 2- 0
BC Cont.		0- 0- 0	30- 2- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.00
Plate Duration Factor 1.00
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	40	20	0.0'	30.2'
BC V	0	20	0.0'	30.2'
TC V	0	10	9.1'	12.7'
TC V	0	10	17.5'	21.1'
BC V	80	10	9.1'	21.1'
MA V	0	10	12.9'	17.3'
MA V	0	10	0.3'	5.6'
MA V	0	10	0.3'	5.6'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Q	1805	130	3- 8	1-15
			Hz =	-214
R	1804	130	3- 8	1-15
			Hz =	215

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -S	0.12	113	C	0.00	0.12
S -V	0.23	1712	C	0.02	0.21
V -B	0.76	2105	C	0.00	0.76
B -C	0.76	1478	C	0.00	0.76
C -D	0.68	480	T	0.03	0.65
D -E	0.68	480	T	0.03	0.65
E -F	0.76	1478	C	0.00	0.76
F -N	0.76	2105	C	0.00	0.76
N -T	0.23	1712	C	0.02	0.21
T -G	0.12	113	C	0.00	0.12
Bottom Chords					
A -Q	0.16	98	T	0.01	0.15
Q -U	0.16	238	T	0.01	0.15
U -S1	0.37	1446	T	0.14	0.23
S1-I	0.80	1446	T	0.11	0.69
I -H	0.82	1571	T	0.13	0.69
H -S2	0.80	1446	T	0.11	0.69
S2-M	0.37	1446	T	0.14	0.23
M -R	0.16	238	T	0.01	0.15
R -G	0.16	98	T	0.01	0.15
Webs					
Q -S	0.17	1489	C		
S -U	0.32	1424	T		
U -V	0.16	881	C		
V -I	0.08	347	T		
I -B	0.21	1123	T		
H -F	0.21	1123	T		
H -N	0.08	347	T		
M -N	0.16	881	C		
M -T	0.32	1424	T		
R -T	0.17	1489	C		
Attic Chords (Top)					
C -J	0.21	2082	C	0.21	0.00
J -E	0.21	2082	C	0.21	0.00
Attic Webs (Top)					
J -D	0.01	51	T		

LL Defl -0.39" in I -H L/798
TL Defl -0.55" in I -H L/568
LL Cant 0.03" in R -G L/799
Shear // Grain in B -C 0.56

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 7.0	Ctr-0.6		0.59
S	LOCK	3.0x 7.0	Ctr Ctr		0.65
V	LOCK	10.0x10.0	0.8-1.2		0.59
B#	LOCK	3.0x12.0	Ctr 1.3		0.33
C#	LOCK	3.0x12.0	-0.8 Ctr		0.45
D	LOCK	7.0x 6.0	Ctr Ctr		0.56
E#	LOCK	3.0x12.0	0.8 Ctr		0.45
F#	LOCK	3.0x12.0	Ctr 1.3		0.33
N	LOCK	10.0x10.0	-0.8-1.2		0.59
T	LOCK	3.0x 7.0	Ctr Ctr		0.62
G	LOCK	5.0x 7.0	Ctr-0.6		0.59
Q	LOCK	2.0x 4.0	Ctr Ctr		0.41
U	LOCK	4.0x 6.0	Ctr Ctr		0.50
S1#	LOCK	7.0x 8.0	Ctr Ctr		0.37
I#	LOCK	6.0x 8.0	Ctr-0.9		0.46
H#	LOCK	6.0x 8.0	Ctr-0.9		0.46
S2#	LOCK	7.0x 8.0	Ctr Ctr		0.37
M	LOCK	4.0x 6.0	Ctr Ctr		0.62
R	LOCK	2.0x 4.0	Ctr Ctr		0.41
J	LOCK	2.0x 4.0	Ctr Ctr		0.41

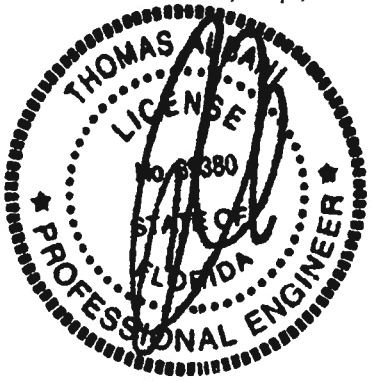
= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

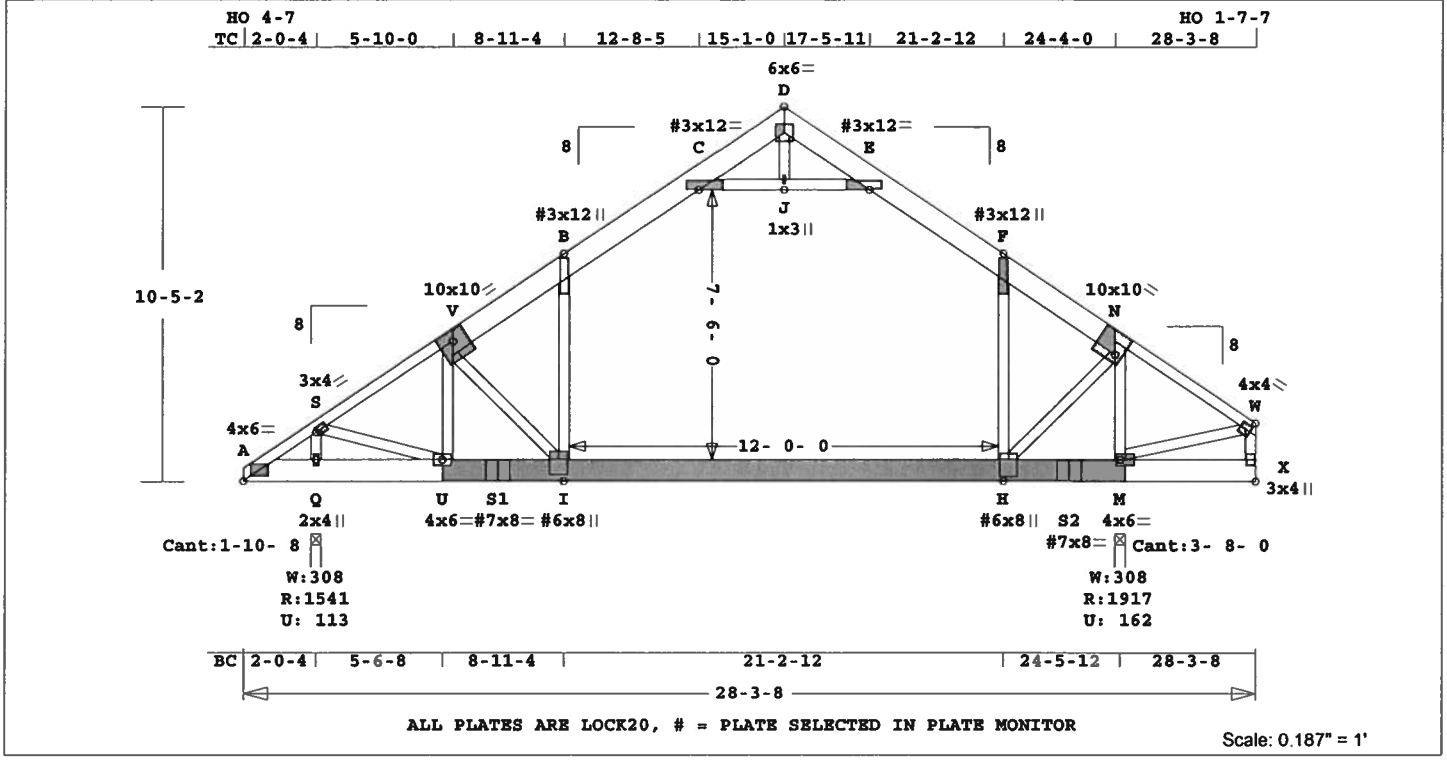
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D1	3	ATI2	300200	8	0	0	T06020649

U# J#haygood-mclearn McLEARN

Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 2105 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D2	11	ATI2	280308	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.187" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 364.3 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	Size	Lumber
TC	0.29	2x 4	SP-#2
EX V -D		2x 8	SP-#1
EX D -N		2x 8	SP-#1
BC	0.24	2x 8	SP-#2
EX S1-S2		2x 8	SP-#1
WB	0.45	2x 4	SP-#2
ACT	0.13	2x 4	SP-#2
AWT	0.01	2x 4	SP-#2
SCB	(1)	2x 8	SP-#1

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 3- 8
BC Cont.	0- 0- 0	28- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.00
Plate Duration Factor 1.00
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf - Live	Dead	From	To
TC V 40	20	0.0'	28.3'
BC V 0	20	0.0'	28.3'
TC V 0	10	9.1'	12.7'
TC V 0	10	17.5'	21.1'
BC V 80	10	9.1'	21.1'
MA V 0	10	12.9'	17.3'
MA V 0	10	0.3'	5.6'
MA V 0	10	0.3'	5.6'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Q	1542	114	3- 8	1-10
			Hz =	-221
M	1917	162	3- 8	2- 1
			Hz =	219

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -S	0.11	73 C	0.00 0.11
S -V	0.24	1427 C	0.01 0.23
V -B	0.81	1549 C	0.00 0.81
B -C	0.81	1004 C	0.00 0.81
C -D	0.25	338 T	0.01 0.24
D -E	0.58	224 T	0.00 0.58
E -F	0.58	1139 C	0.00 0.58
F -N	0.11	1303 C	0.00 0.11
N -W	0.29	181 T	0.05 0.24
Bottom Chords			
A -Q	0.13	63 T	0.00 0.13
Q -U	0.13	237 T	0.00 0.13
U -S1	0.19	1207 T	0.15 0.04
S1-I	0.22	1207 T	0.09 0.13
I -H	0.63	1083 T	0.10 0.53
H -S2	0.53	186 T	0.00 0.53
S2-M	0.24	186 T	0.00 0.24
M -X	0.05	15 C	0.00 0.05
Webs			
Q -S	0.15	1287 C	
S -U	0.27	1211 T	
U -V	0.09	493 C	
V -I	0.06	232 C	
I -B	0.18	1000 T	
H -F	0.06	404 T	
H -N	0.42	1821 T	
M -N	0.45	2401 C	
M -W	0.02	146 C	
X -W	0.01	88 T	WindLd
Attic Chords (Top)			
C -J	0.13	1324 C	0.13 0.00
J -E	0.13	1324 C	0.13 0.00
Attic Webs (Top)			
J -D	0.01	44 T	

LL Defl -0.22" in I -H L/999
TL Defl -0.33" in S1-I L/808
LL Cant 0.04" in A -Q L/573
Shear // Grain in B -C 0.43

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.62
S	LOCK	3.0x 4.0	Ctr	Ctr	0.99
V	LOCK	10.0x10.0	0.8-1.2		0.57
B#	LOCK	3.0x12.0	Ctr	1.3	0.29
C#	LOCK	3.0x12.0	-0.8	Ctr	0.28
D	LOCK	6.0x 6.0	Ctr	Ctr	0.60
E#	LOCK	3.0x12.0	0.8	Ctr	0.28
F#	LOCK	3.0x12.0	Ctr	1.3	0.27
N	LOCK	10.0x10.0	-0.8	1.2	0.63
W	LOCK	4.0x 4.0	Ctr	Ctr	0.73
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.40
U	LOCK	4.0x 6.0	Ctr	Ctr	0.48
S1#	LOCK	7.0x 8.0	Ctr	Ctr	0.36
I#	LOCK	6.0x 8.0	Ctr	-1.1	0.40
H#	LOCK	6.0x 8.0	Ctr	-1.8	0.65
S2#	LOCK	7.0x 8.0	Ctr	Ctr	0.66
M	LOCK	4.0x 6.0	Ctr	Ctr	0.61
X	LOCK	3.0x 4.0	Ctr	Ctr	0.89
J	LOCK	1.0x 3.0	Ctr	Ctr	0.81

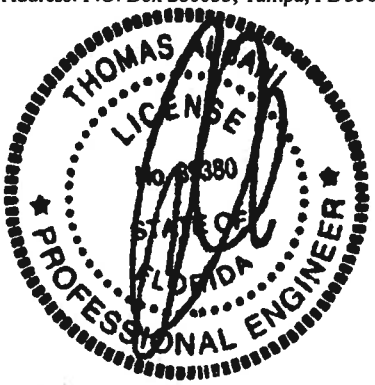
= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Fasten each scab with 2 row(s) of 10d nails at 6 In o.c. along entire length.
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

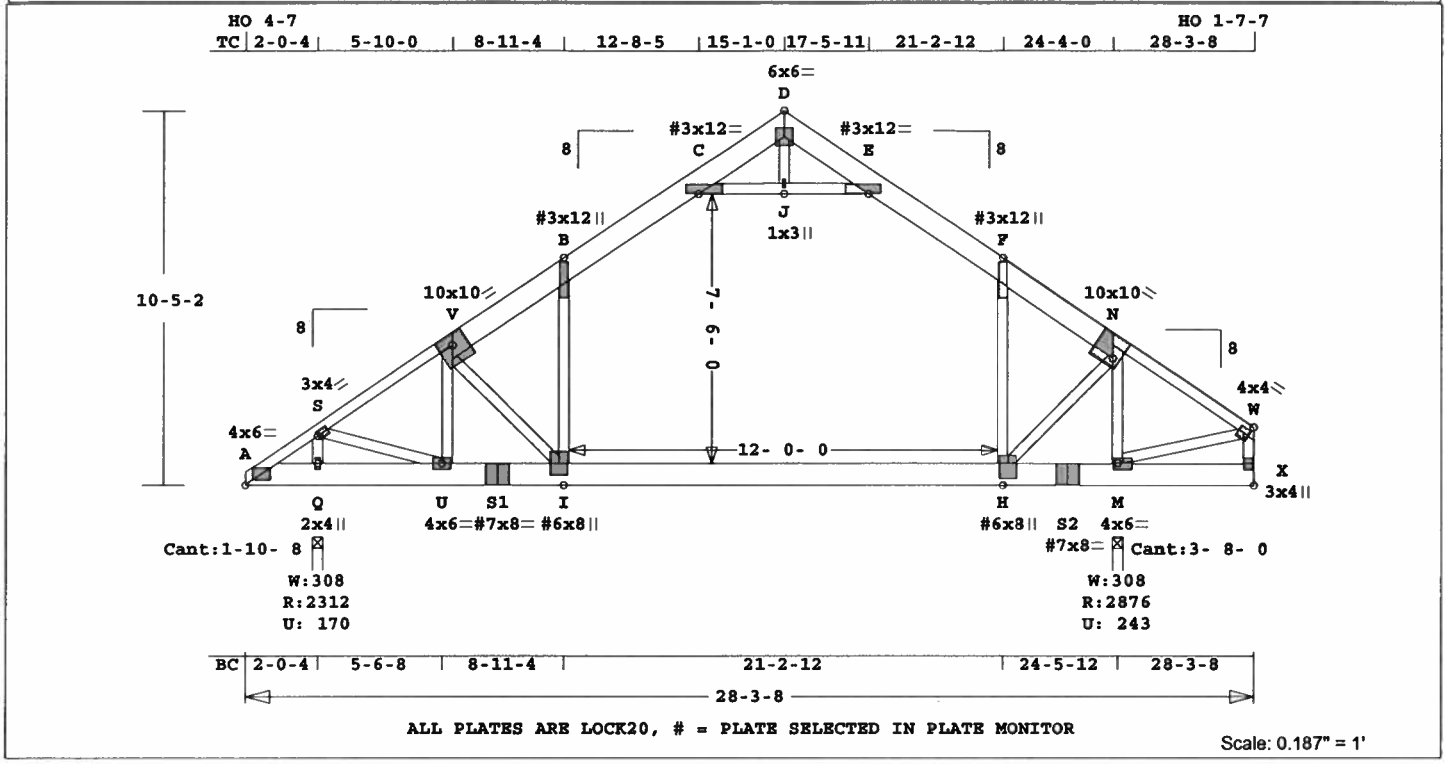
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D2	11	ATI2	280308	8	0	0	T06020649

U# J#haygood-mclearn McLEARN

```
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
 Wind-Force Resistance System.
  Wind Speed:          110 mph
  Mean Roof Height:  15-0
  Exposure  Category:    B
  Occupancy Factor  : 1.00
  Building Type: Enclosed
  Zone location: Exterior
  TC Dead Load :       5.0 psf
  BC Dead Load :       5.0 psf
Unbalanced Loads Checked
  Load Factors = 1.00 and 0.00
Max comp. force     2401 Lbs
Quality Control Factor 1.25
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D3	2*2P	ATI2	280308	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.187" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 292.2 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06
****
* 2-Ply Truss *
****

	CSI	Size	Lumber
TC	0.25	2x 4	SP-#2
EX V -D		2x 8	SP-#1
EX D -N		2x 8	SP-#1
BC	0.41	2x 8	SP-#2
EX S1-S2		2x 8	SP-#1
WB	0.31	2x 4	SP-#2
ACT	0.11	2x 4	SP-#2
AWT	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	2- 0- 0	0- 0- 0	28- 3- 8
BC	2- 0- 0	0- 0- 0	28- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			36.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	60	30	0.0'	28.3'
BC V	0	30	0.0'	28.3'
TC V	0	15	9.1'	12.7'
TC V	0	15	17.5'	21.1'
BC V	120	15	9.1'	21.1'
MA V	0	15	12.9'	17.3'
MA V	0	15	0.3'	5.6'
MA V	0	15	0.3'	5.6'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Q	2312	170	3- 8	1- 8
			Hz =	-332
M	2876	244	3- 8	1- 9
			Hz =	329

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -S	0.09	109 C	0.00 0.09
S -V	0.20	2140 C	0.01 0.19
V -B	0.70	2324 C	0.00 0.70
B -C	0.70	1506 C	0.00 0.70
C -D	0.22	508 T	0.01 0.21
D -E	0.50	337 T	0.00 0.50
E -F	0.50	1708 C	0.00 0.50
F -N	0.09	1954 C	0.00 0.09
N -W	0.25	271 T	0.04 0.21
Bottom Chords			
A -Q	0.10	95 T	0.00 0.10
Q -U	0.10	356 T	0.00 0.10
U -S1	0.18	1811 T	0.12 0.06
S1-I	0.29	1811 T	0.07 0.22
I -H	0.97	1624 T	0.08 0.89
H -S2	0.89	279 T	0.00 0.89
S2-M	0.41	279 T	0.00 0.41
M -X	0.04	23 C	0.00 0.04
Webs			
Q -S	0.11	1931 C	
S -U	0.20	1817 T	
U -V	0.04	739 C	
V -I	0.02	349 C	
I -B	0.13	1500 T	
H -F	0.04	606 T	
H -N	0.31	2731 T	
M -N	0.22	3602 C	
M -W	0.01	220 C	
X -W	0.01	133 T	WindLd
Attic Chords (Top)			
C -J	0.11	1986 C	0.11 0.00
J -E	0.11	1986 C	0.11 0.00
Attic Webs (Top)			
J -D	0.00	67 T	

LL Defl -0.26" in I -H L/999
TL Defl -0.36" in I -H L/739
LL Cant 0.03" in A -Q L/763
Shear // Grain in I -H 0.47

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.62
S	LOCK	3.0x 4.0	Ctr	Ctr	0.74
V	LOCK	10.0x10.0	0.8	-1.2	0.57
B#	LOCK	3.0x12.0	Ctr	1.3	0.27
C#	LOCK	3.0x12.0	-0.8	Ctr	0.27
D	LOCK	6.0x 6.0	Ctr	Ctr	0.60
E#	LOCK	3.0x12.0	0.8	Ctr	0.27
F#	LOCK	3.0x12.0	Ctr	1.3	0.27
N	LOCK	10.0x10.0	-0.8	-1.2	0.57
W	LOCK	4.0x 4.0	Ctr	Ctr	0.73
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.40
U	LOCK	4.0x 6.0	Ctr	Ctr	0.48
S1#	LOCK	7.0x 8.0	Ctr	Ctr	0.36
I#	LOCK	6.0x 8.0	Ctr	Ctr	0.40
H#	LOCK	6.0x 8.0	Ctr	-1.1	0.43
S2#	LOCK	7.0x 8.0	Ctr	Ctr	0.50
M	LOCK	4.0x 6.0	Ctr	Ctr	0.61
X	LOCK	3.0x 4.0	Ctr	Ctr	0.89
J	LOCK	1.0x 3.0	Ctr	Ctr	0.80

= Plate Monitor used

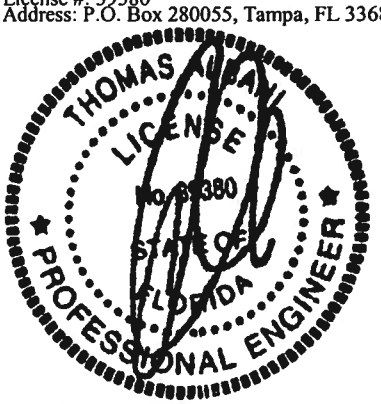
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

Rows	Nails	Screws	Bolts
----Spacing (In)----			

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

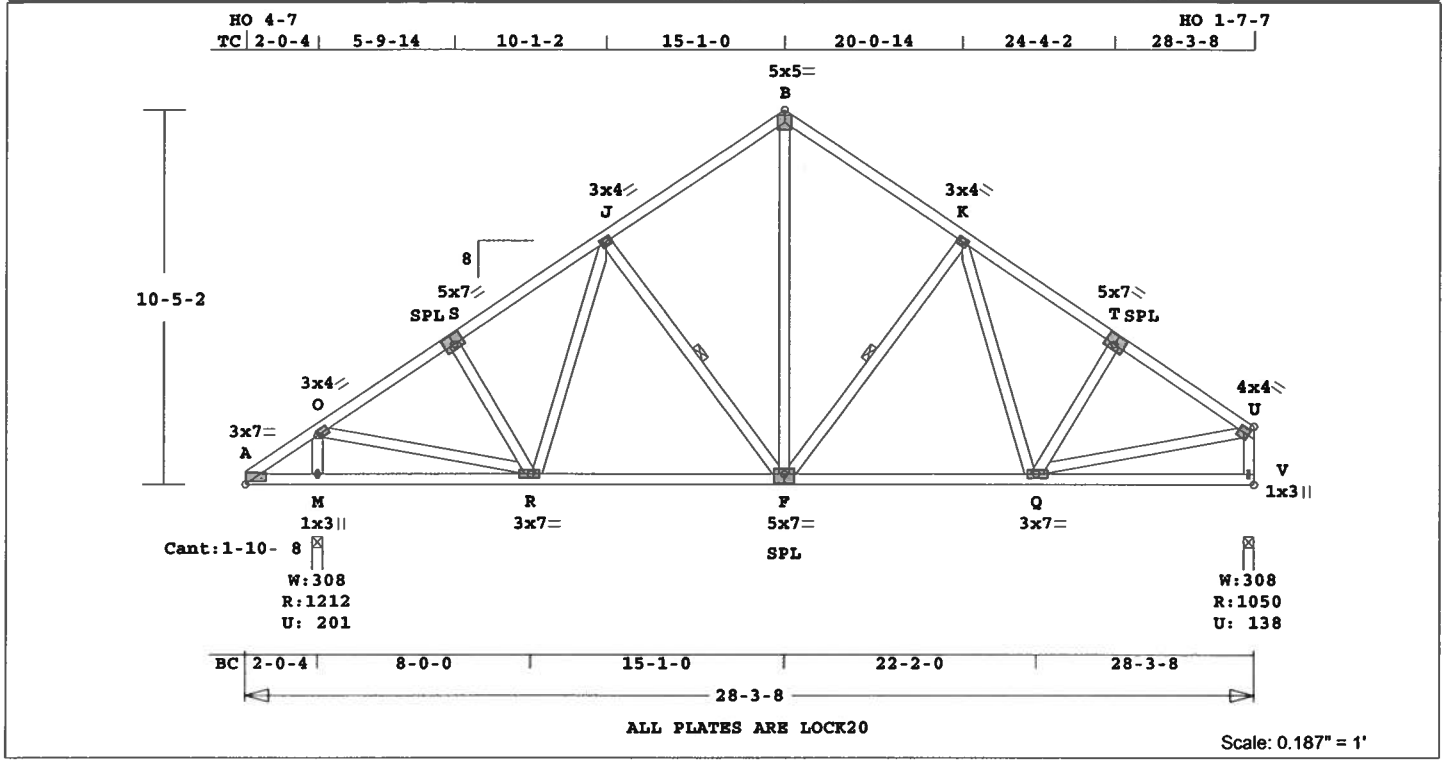
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D3	2*2P	ATI2	280308	8	0	0	T06020649

U# J#haygood-mclearn McLEARN

```
   TC  1     12      24        0
   BC  2     12      24        0
   WB  1      8       8
Provide connection to bearing
   for    333 Lbs Horiz Reaction
Design checked for 10 psf non-
   concurrent LL on BC.
Prevent truss rotation at all
   bearing locations.
NOTE: USER MODIFIED PLATES
   This design may have plates
   selected through a plate
   monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
 Wind-Force Resistance System.
   Wind Speed:             110 mph
   Mean Roof Height:  15-0
   Exposure  Category:     B
   Occupancy Factor   : 1.00
   Building Type: Enclosed
   Zone location: Exterior
   TC Dead Load :          5.0 psf
   BC Dead Load :          5.0 psf
Unbalanced Loads Checked
   Load Factors = 1.00 and 0.00
Max comp. force      3602 Lbs
Quality Control Factor 1.25
```

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D4	2	TR	280308	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 235.4 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.20	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	28- 3- 8
BC Cont.	0- 0- 0	28- 3- 8

WB 1 rows CLB on J -F
WB 1 rows CLB on F -K
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	1213	201	3- 8	1- 8
			Hz =	-231
V	1050	139	3- 8	1- 8
			Hz =	226

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -O	0.13	48	C	0.01	0.12
O -S	0.13	1173	C	0.01	0.12
S -J	0.21	1044	C	0.00	0.21
J -B	0.21	827	C	0.00	0.21
B -K	0.20	827	C	0.00	0.20
K -T	0.20	1070	C	0.00	0.20
T -U	0.14	1198	C	0.00	0.14
Bottom Chords					
A -M	0.16	52	T	0.00	0.16
M -R	0.22	241	T	0.00	0.22
R -F	0.33	857	T	0.08	0.25
F -Q	0.34	867	T	0.08	0.26
Q -V	0.26	215	T	0.00	0.26
Webs					
M -O	0.10	1090	C		
O -R	0.18	1020	T		
S -R	0.05	190	C		
R -J	0.02	163	T		
J -F	0.06	289	C		1 Br
F -B	0.20	621	T		
F -K	0.07	306	C		1 Br
K -Q	0.02	190	T		
Q -T	0.05	213	C		
Q -U	0.19	1032	T		
V -U	0.09	998	C	WindLd	

LL Defl -0.04" in Q -V L/999
TL Defl -0.10" in R -F L/999
LL Cant 0.00" in A -M L/999
Shear // Grain in J -B 0.16

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x	7.0	1.0	0.82
O	LOCK	3.0x	4.0	Ctr Ctr	0.72
S	LOCK	5.0x	7.0	-0.3 0.5	0.65
J	LOCK	3.0x	4.0	Ctr Ctr	0.67
B	LOCK	5.0x	5.0	Ctr Ctr	0.58
K	LOCK	3.0x	4.0	Ctr Ctr	0.67
T	LOCK	5.0x	7.0	0.3 0.5	0.65
U	LOCK	4.0x	4.0	Ctr Ctr	0.73
M	LOCK	1.0x	3.0	Ctr Ctr	0.81
R	LOCK	3.0x	7.0	-0.5 Ctr	0.91
F	LOCK	5.0x	7.0	Ctr-0.5	0.67
Q	LOCK	3.0x	7.0	0.5 Ctr	0.91
V	LOCK	1.0x	3.0	Ctr Ctr	0.81

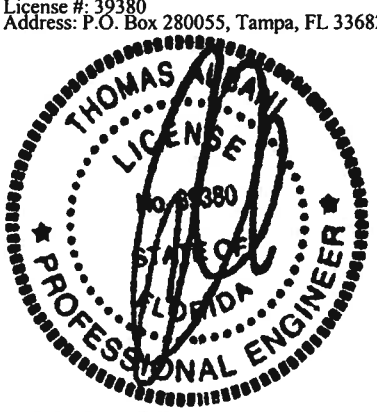
REVIEWED BY:

Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1198 Lbs
Quality Control Factor 1.25

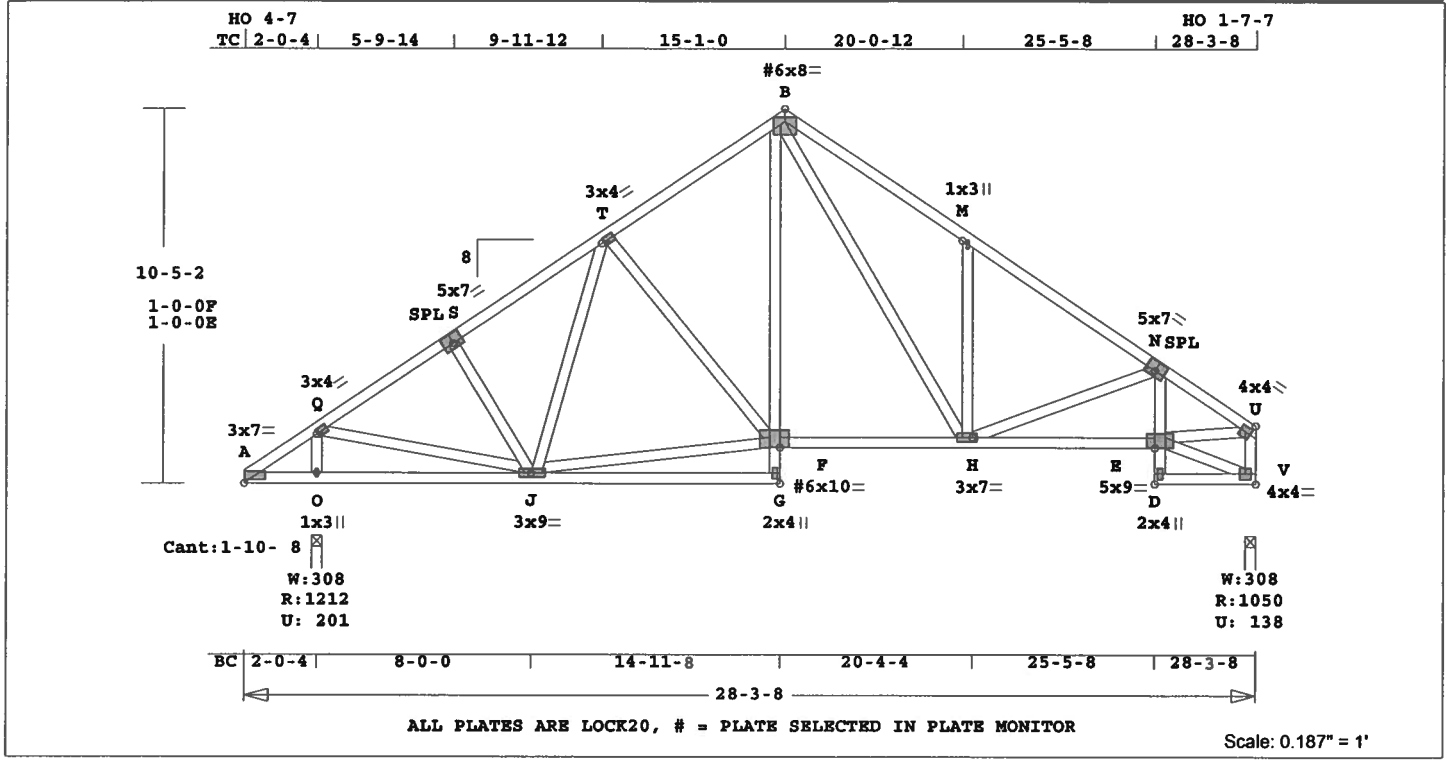
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D5	2	SP	280308	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.187" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 252.5 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	Size	Lumber
TC	0.35	2x 4	SP-#2
BC	0.28	2x 4	SP-#2
CW	0.26	2x 4	SP-#2
WB	0.24	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	28- 3- 8
BC Cont.		0- 0- 0	28- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
O	1213	201	3- 8	1- 8
			Hz = -230	
V	1050	139	3- 8	1- 8
			Hz = 226	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -Q	0.14	46	C	0.01	0.13
Q -S	0.14	1165	C	0.01	0.13
S -T	0.27	1041	C	0.00	0.27
T -B	0.28	926	C	0.01	0.27
B -M	0.35	1230	C	0.00	0.35
M -N	0.24	1241	C	0.02	0.22
N -U	0.21	1402	C	0.01	0.20
Bottom Chords					
A -O	0.15	50	T	0.00	0.15
O -J	0.22	240	T	0.00	0.22
J -G	0.23	88	T	0.01	0.22
F -H	0.22	746	T	0.07	0.15
H -E	0.28	1203	T	0.20	0.08
D -V	0.03	20	T	0.00	0.03
G -F	0.26	134	T	0.02	0.24
F -B	0.10	439	T	0.07	0.03
D -E	0.03	58	T	0.00	0.03
E -N	0.07	113	C	0.00	0.07
Webs					
O -Q	0.10	1092	C		
Q -J	0.18	1012	T		
S -J	0.04	178	C		
J -T	0.04	77	C		
J -F	0.15	835	T		
T -F	0.16	235	C		
B -H	0.24	552	T		
H -M	0.12	291	C		
H -N	0.07	187	C		
E -U	0.22	1201	T		
E -V	0.03	213	T		
V -U	0.09	1017	C	WindLd	

LL Defl -0.05" in F -H L/999
TL Defl -0.11" in J -G L/999
LL Cant 0.00" in A -O L/999
Shear // Grain in T -B 0.20

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	3.0x	7.0	1.0	0.7	0.82
Q	LOCK	3.0x	4.0	Ctr	Ctr	0.72
S	LOCK	5.0x	7.0	-0.3	0.5	0.65
T	LOCK	3.0x	4.0	Ctr	Ctr	0.93
B#	LOCK	6.0x	8.0	-0.1	-1.5	0.78
M	LOCK	1.0x	3.0	Ctr	Ctr	0.80
N	LOCK	5.0x	7.0	0.3	0.5	0.69
U	LOCK	4.0x	4.0	Ctr	Ctr	0.73
O	LOCK	1.0x	3.0	Ctr	Ctr	0.81
J	LOCK	3.0x	9.0	Ctr	Ctr	0.91
G	LOCK	2.0x	4.0	Ctr	Ctr	0.58
F#	LOCK	6.0x	10.0	-0.1	1.3	0.58
H	LOCK	3.0x	7.0	Ctr	Ctr	0.60
E	LOCK	5.0x	9.0	Ctr	0.8	0.58
D	LOCK	2.0x	4.0	Ctr	Ctr	0.58
V	LOCK	4.0x	4.0	Ctr	Ctr	0.61

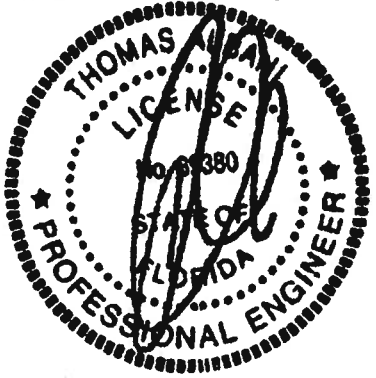
= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1402 Lbs
Quality Control Factor 1.25

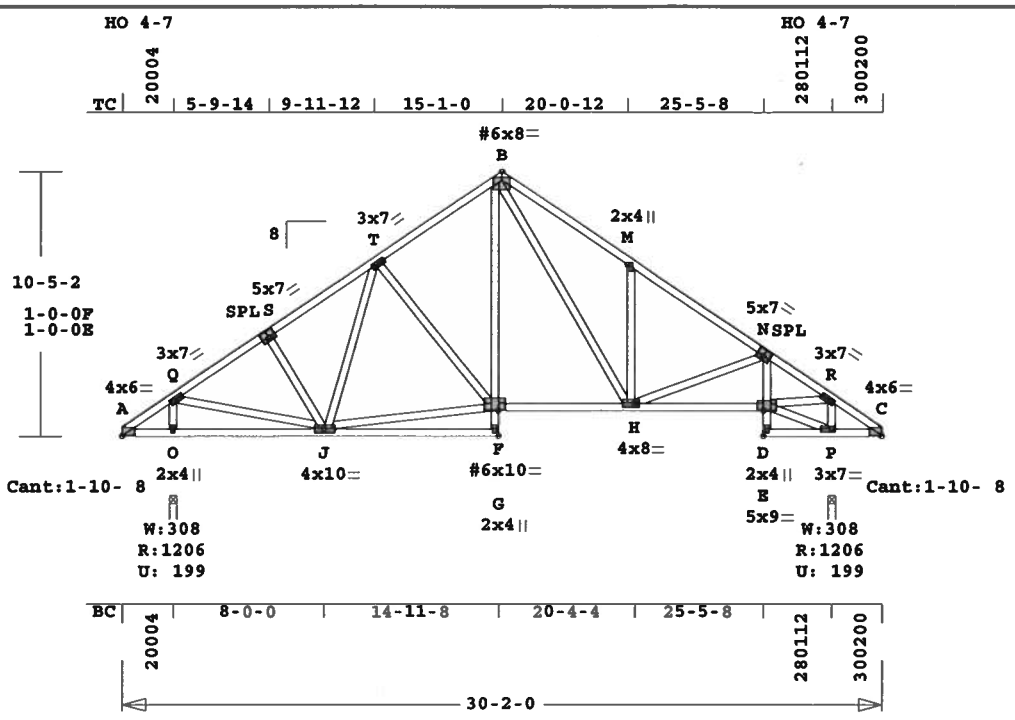
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D6	3	SP	300200	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.132" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 260.3 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
CW	0.26	2x 4	SP-#2
WB	0.23	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	30- 2- 0
BC Cont.	0- 0- 0	30- 2- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
O	1207	199	3- 8	1- 8
			Hz =	-221
P	1207	199	3- 8	1- 8
			Hz =	222

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -Q	0.14	46	C	0.01	0.13
Q -S	0.14	1157	C	0.01	0.13
S -T	0.27	1033	C	0.00	0.27
T -B	0.27	915	C	0.00	0.27
B -M	0.35	1207	C	0.00	0.35
M -N	0.24	1217	C	0.02	0.22
N -R	0.20	1322	C	0.01	0.19
R -C	0.04	103	T	0.02	0.02
Bottom Chords					
A -O	0.15	50	T	0.00	0.15
O -J	0.22	231	T	0.00	0.22
J -G	0.23	88	T	0.01	0.22
F -H	0.22	737	T	0.07	0.15
H -E	0.26	1133	T	0.19	0.07
D -P	0.06	14	C	0.00	0.06
P -C	0.06	83	C	0.00	0.06
G -F	0.26	134	T	0.02	0.24
F -B	0.10	439	T	0.07	0.03
D -E	0.04	40	T	0.00	0.04
E -N	0.07	159	C	0.00	0.07
Webs					
O -Q	0.10	1086	C		
Q -J	0.18	1006	T		
S -J	0.04	178	C		
J -T	0.04	74	C		
J -F	0.15	828	T		
T -F	0.16	236	C		
B -H	0.23	530	T		
H -M	0.12	293	C		
H -N	0.07	179	C		
E -R	0.22	1215	T		
E -P	0.02	204	T		
P -R	0.10	1082	C		

LL Defl -0.05" in F -H L/999
TL Defl -0.11" in J -G L/999
LL Cant 0.01" in P -C L/999
Shear // Grain in T -B 0.20

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt	Size	X	Y	JSI
A	LOCK	4.0x	6.0	0.5	0.4	0.65
Q	LOCK	3.0x	7.0	Ctr	Ctr	0.42
S	LOCK	5.0x	7.0-0.3	0.5	0.5	0.67
T	LOCK	3.0x	7.0	Ctr	Ctr	0.54
B#	LOCK	6.0x	8.0-0.2-1.5			0.77
M	LOCK	2.0x	4.0	Ctr	Ctr	0.41
N	LOCK	5.0x	7.0	0.3	0.5	0.71
R	LOCK	3.0x	7.0	Ctr	Ctr	0.42
C	LOCK	4.0x	6.0-0.5	0.4		0.65
O	LOCK	2.0x	4.0	Ctr	Ctr	0.41
J	LOCK	4.0x	10.0	Ctr	Ctr	0.70
G	LOCK	2.0x	4.0	Ctr	Ctr	0.58
F#	LOCK	6.0x	10.0	Ctr	1.3	0.60
H	LOCK	4.0x	8.0	Ctr	Ctr	0.48
E	LOCK	5.0x	9.0	Ctr	0.8	0.59
D	LOCK	2.0x	4.0	Ctr	Ctr	0.58
P	LOCK	3.0x	7.0	Ctr	Ctr	0.46

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.

PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

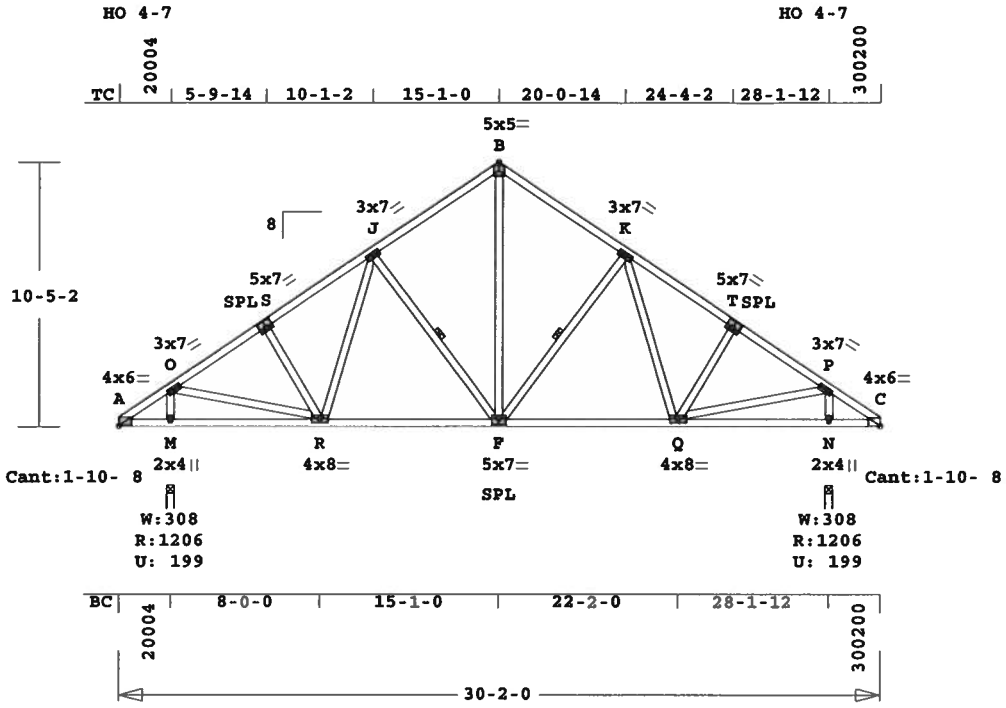
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1322 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D7	1	TR	300200	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 243.2 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.34	2x 4	SP-#2
WB	0.19	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	30- 2- 0
BC	Cont.	0- 0- 0	30- 2- 0

WB 1 rows CLB on J -F
WB 1 rows CLB on F -K
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	1207	199	3- 8	1- 8
		Hz =	-222	
N	1207	199	3- 8	1- 8
		Hz =	223	

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -O	0.13	48	C	0.01	0.12
O -S	0.13	1164	C	0.01	0.12
S -J	0.21	1036	C	0.00	0.21
J -B	0.21	818	C	0.00	0.21
B -K	0.21	818	C	0.00	0.21
K -T	0.21	1036	C	0.00	0.21
T -P	0.13	1164	C	0.01	0.12
P -C	0.13	48	C	0.01	0.12
Bottom Chords					
A -M	0.16	51	T	0.00	0.16
M -R	0.22	232	T	0.00	0.22
R -F	0.34	849	T	0.08	0.26
F -Q	0.34	849	T	0.08	0.26
Q -N	0.22	232	T	0.00	0.22
N -C	0.16	51	T	0.00	0.16
Webs					
M -O	0.10	1084	C		
O -R	0.18	1012	T		
S -R	0.05	190	C		
R -J	0.02	163	T		
J -F	0.06	290	C		1 Br
F -B	0.19	611	T		
F -K	0.06	290	C		1 Br
K -Q	0.02	163	T		
Q -T	0.05	190	C		
Q -P	0.18	1012	T		
N -P	0.10	1084	C		

LL Defl -0.04" in R -F L/999
TL Defl -0.10" in R -F L/999
LL Cant 0.00" in N -C L/999
Shear // Grain in J -B 0.16

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

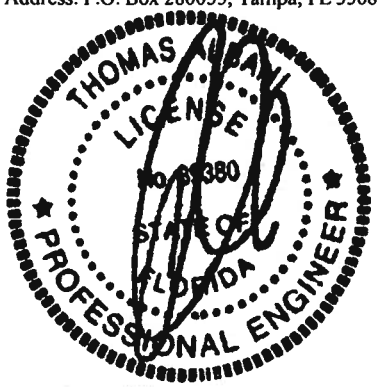
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	0.5	0.4	0.65
O	LOCK	3.0x 7.0	Ctr	Ctr	0.42
S	LOCK	5.0x 7.0	-0.3	0.5	0.67
J	LOCK	3.0x 7.0	Ctr	Ctr	0.54
B	LOCK	5.0x 5.0	Ctr	Ctr	0.60
K	LOCK	3.0x 7.0	Ctr	Ctr	0.54
T	LOCK	5.0x 7.0	0.3	0.5	0.67
P	LOCK	3.0x 7.0	Ctr	Ctr	0.42
C	LOCK	4.0x 6.0	-0.5	0.4	0.65
M	LOCK	2.0x 4.0	Ctr	Ctr	0.41
R	LOCK	4.0x 8.0	Ctr	Ctr	0.70
F	LOCK	5.0x 7.0	Ctr	-0.5	0.69
Q	LOCK	4.0x 8.0	Ctr	Ctr	0.70
N	LOCK	2.0x 4.0	Ctr	Ctr	0.41

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1164 Lbs
Quality Control Factor 1.25

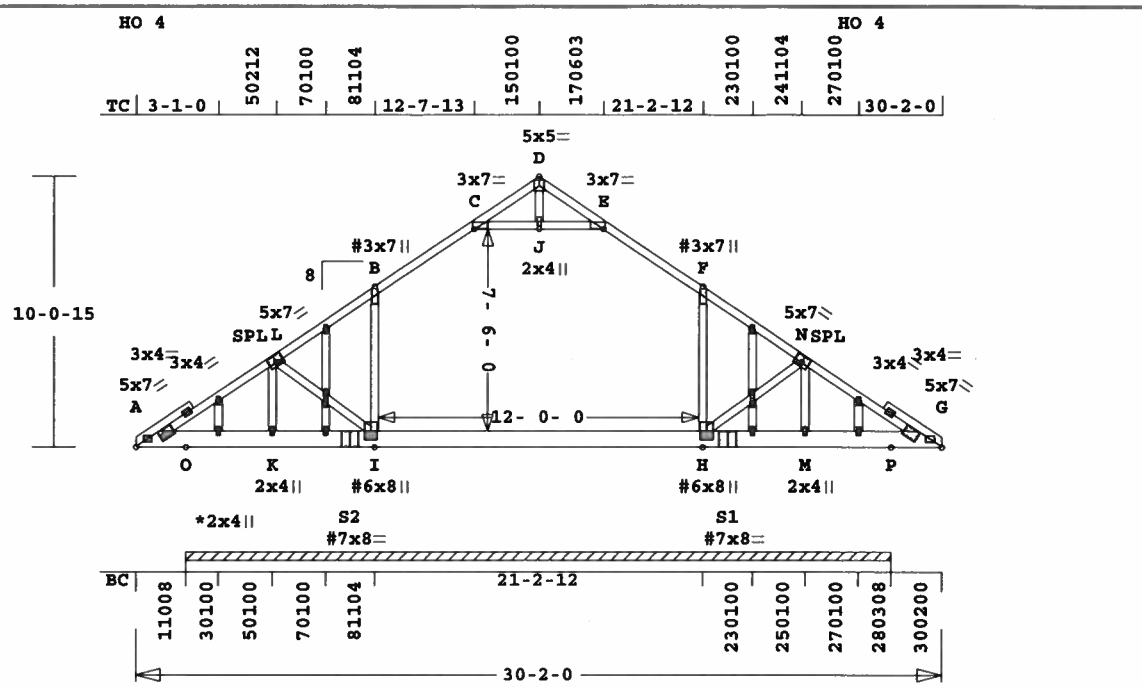
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D8	1	ATI2	300200	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
See * For Typical Gable Plate Size and Placement

Scale: 0.140" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 264.2 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	Size	Lumber
TC	0.16	2x 4	SP-#2
BC	0.20	2x 8	SP-#2
WB	0.09	2x 4	SP-#2
ACT	0.02	2x 4	SP-#2
AWT	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	30- 2- 0
BC Cont.		0- 0- 0	30- 2- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	1-10- 8 to 28- 3- 8			
	2413	399	Hz =	212

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -L	0.13	55 C	0.00	0.13
L -B	0.16	437 C	0.00	0.16
B -C	0.16	448 C	0.00	0.16
C -D	0.08	127 C	0.00	0.08
D -E	0.08	127 C	0.00	0.08
E -F	0.16	448 C	0.00	0.16
F -N	0.16	437 C	0.00	0.16
N -G	0.13	55 C	0.00	0.13
Bottom Chords				
A -O	0.09	57 T	0.00	0.09
O -K	0.09	0 T	0.00	0.09
K -S2	0.05	0 T	0.00	0.05
S2-I	0.20	0 T	0.00	0.20
I -H	0.20	0 T	0.00	0.20
H -S1	0.20	0 T	0.00	0.20
S1-M	0.05	0 T	0.00	0.05
M -P	0.09	0 T	0.00	0.09
P -G	0.09	57 T	0.00	0.09
Webs				
K -L	0.08	680 C		
L -I	0.07	401 T		
I -B	0.09	245 C		
H -F	0.09	245 C		
H -N	0.07	401 T		
M -N	0.08	680 C		
Attic Chords (Top)				
C -J	0.02	265 C	0.02	0.00
J -E	0.02	265 C	0.02	0.00
Attic Webs (Top)				
J -D	0.00	1 C		

LL Defl -0.04" in I -H L/999
TL Defl -0.08" in I -H L/999
LL Cant 0.00" in O -O L/999
Shear // Grain in S2-I 0.19

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	5.0x 7.0	Ctr-0.9		0.59
L	LOCK	5.0x 7.0	-0.3	0.5	0.67
B#	LOCK	3.0x 7.0	0.1-0.4		0.28
C	LOCK	3.0x 7.0	Ctr	Ctr	0.28
D	LOCK	5.0x 5.0	Ctr	Ctr	0.60
E	LOCK	3.0x 7.0	Ctr	Ctr	0.28
F#	LOCK	3.0x 7.0	0.1-0.4		0.28
N	LOCK	5.0x 7.0	0.3	0.5	0.67
G	LOCK	5.0x 7.0	Ctr-0.9		0.59
K	LOCK	2.0x 4.0	Ctr	Ctr	0.41
S2#	LOCK	7.0x 8.0	Ctr	Ctr	0.37
I#	LOCK	6.0x 8.0	Ctr	Ctr	0.41
H#	LOCK	6.0x 8.0	Ctr	Ctr	0.41
S1#	LOCK	7.0x 8.0	Ctr	Ctr	0.37
M	LOCK	2.0x 4.0	Ctr	Ctr	0.41
J	LOCK	2.0x 4.0	Ctr	Ctr	0.41

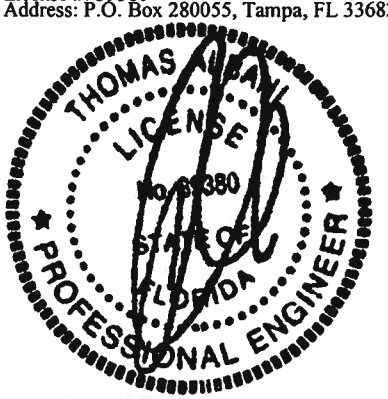
= Plate Monitor used
6 Gable studs to be attached with 2.0x4.0 plates each end.

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 680 Lbs
Quality Control Factor 1.25

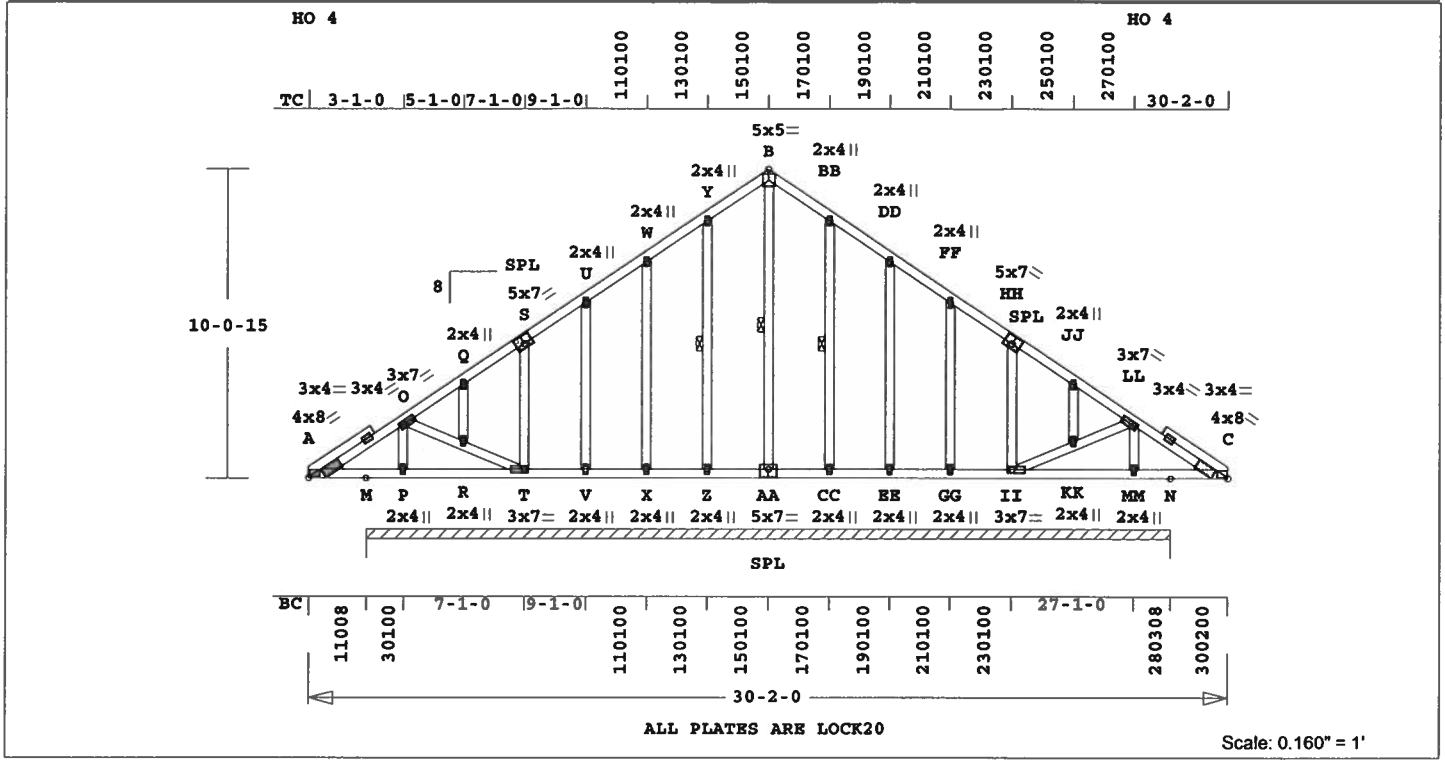
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D9	1	TR	300200	8	0	0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 273.4 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
WB	0.09	2x 4	SP-#2
GW	0.07	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	30- 2- 0
BC Cont.	0- 0- 0	30- 2- 0

WB 1 rows CLB on Z -Y
WB 1 rows CLB on AA-B
WB 1 rows CLB on CC-BB
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
Cont.	Brg 1-10- 8	to 28- 3- 8		
	2413	399	Hz = 215	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -O	0.12	134 T	0.02	0.10
O -Q	0.10	112 C	0.00	0.10
Q -S	0.09	96 T	0.01	0.08
S -U	0.08	62 T	0.00	0.08
U -W	0.04	46 T	0.01	0.03
W -Y	0.03	68 T	0.00	0.03
Y -B	0.03	114 T	0.00	0.03
B -BB	0.03	114 T	0.00	0.03
BB-DD	0.03	68 T	0.00	0.03
DD-FF	0.04	46 T	0.01	0.03
FF-HH	0.08	62 T	0.00	0.08
HH-JJ	0.09	96 T	0.01	0.08
JJ-LL	0.10	112 C	0.00	0.10
LL-C	0.12	134 T	0.02	0.10
--------Bottom Chords---------				
A -M	0.07	109 C	0.00	0.07
M -P	0.06	0 T	0.00	0.06
P -T	0.06	0 T	0.00	0.06
T -V	0.06	0 T	0.00	0.06
V -X	0.02	0 T	0.00	0.02
X -Z	0.02	0 T	0.00	0.02
Z -AA	0.02	0 T	0.00	0.02
AA-CC	0.02	0 T	0.00	0.02
CC-EE	0.02	0 T	0.00	0.02
EE-GG	0.02	0 T	0.00	0.02
GG-II	0.06	0 T	0.00	0.06
II-MM	0.06	0 T	0.00	0.06
MM-N	0.06	0 T	0.00	0.06
N -C	0.07	109 C	0.00	0.07
----------Webs----------				
O -R	0.09	125 T	0.02	0.07
R -T	0.08	135 T	0.01	0.07
II-KK	0.08	135 T	0.01	0.07
KK-LL	0.09	125 T	0.02	0.07
----------Gable Webs----------				
P -O	0.02	294 C		
R -Q	0.04	37 C	0.00	0.04
T -S	0.04	177 C		
V -U	0.04	101 C		
X -W	0.07	121 C		
Z -Y	0.03	125 C		1 Br
AA-B	0.04	137 C		1 Br
CC-BB	0.03	125 C		1 Br
EE-DD	0.07	121 C		
GG-FF	0.04	101 C		
II-HH	0.04	177 C		
KK-JJ	0.04	37 C	0.00	0.04
MM-LL	0.02	294 C		

LL Defl 0.00" in P -T L/999
TL Defl -0.01" in II-MM L/999
LL Cant 0.00" in A -M L/999
Shear // Grain in A -O 0.10

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 8.0	1.0	0.4	0.59
O	LOCK	3.0x 7.0	Ctr	Ctr	0.42
Q	LOCK	2.0x 4.0	Ctr	Ctr	0.41
S	LOCK	5.0x 7.0	-0.3	0.5	0.67
U	LOCK	2.0x 4.0	Ctr	Ctr	0.41
W	LOCK	2.0x 4.0	Ctr	Ctr	0.41
Y	LOCK	2.0x 4.0	Ctr	Ctr	0.41
B	LOCK	5.0x 5.0	Ctr	Ctr	0.60
BB	LOCK	2.0x 4.0	Ctr	Ctr	0.41
DD	LOCK	2.0x 4.0	Ctr	Ctr	0.41
FF	LOCK	2.0x 4.0	Ctr	Ctr	0.41
HH	LOCK	5.0x 7.0	0.3	0.5	0.67
JJ	LOCK	2.0x 4.0	Ctr	Ctr	0.41
LL	LOCK	3.0x 7.0	Ctr	Ctr	0.42
C	LOCK	4.0x 8.0	-1.0	0.4	0.59
P	LOCK	2.0x 4.0	Ctr	Ctr	0.41
T	LOCK	3.0x 7.0	Ctr	Ctr	0.45
V	LOCK	2.0x 4.0	Ctr	Ctr	0.41
X	LOCK	2.0x 4.0	Ctr	Ctr	0.41
Z	LOCK	2.0x 4.0	Ctr	Ctr	0.41
AA	LOCK	5.0x 7.0	Ctr	-0.5	0.69
CC	LOCK	2.0x 4.0	Ctr	Ctr	0.41
EE	LOCK	2.0x 4.0	Ctr	Ctr	0.41
GG	LOCK	2.0x 4.0	Ctr	Ctr	0.41
II	LOCK	3.0x 7.0	Ctr	Ctr	0.45
MM	LOCK	2.0x 4.0	Ctr	Ctr	0.41
R	LOCK	2.0x 4.0	Ctr	Ctr	0.41
KK	LOCK	2.0x 4.0	Ctr	Ctr	0.41

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
WARNING Do Not Cut overframe member between outside of truss and first tie-plate

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

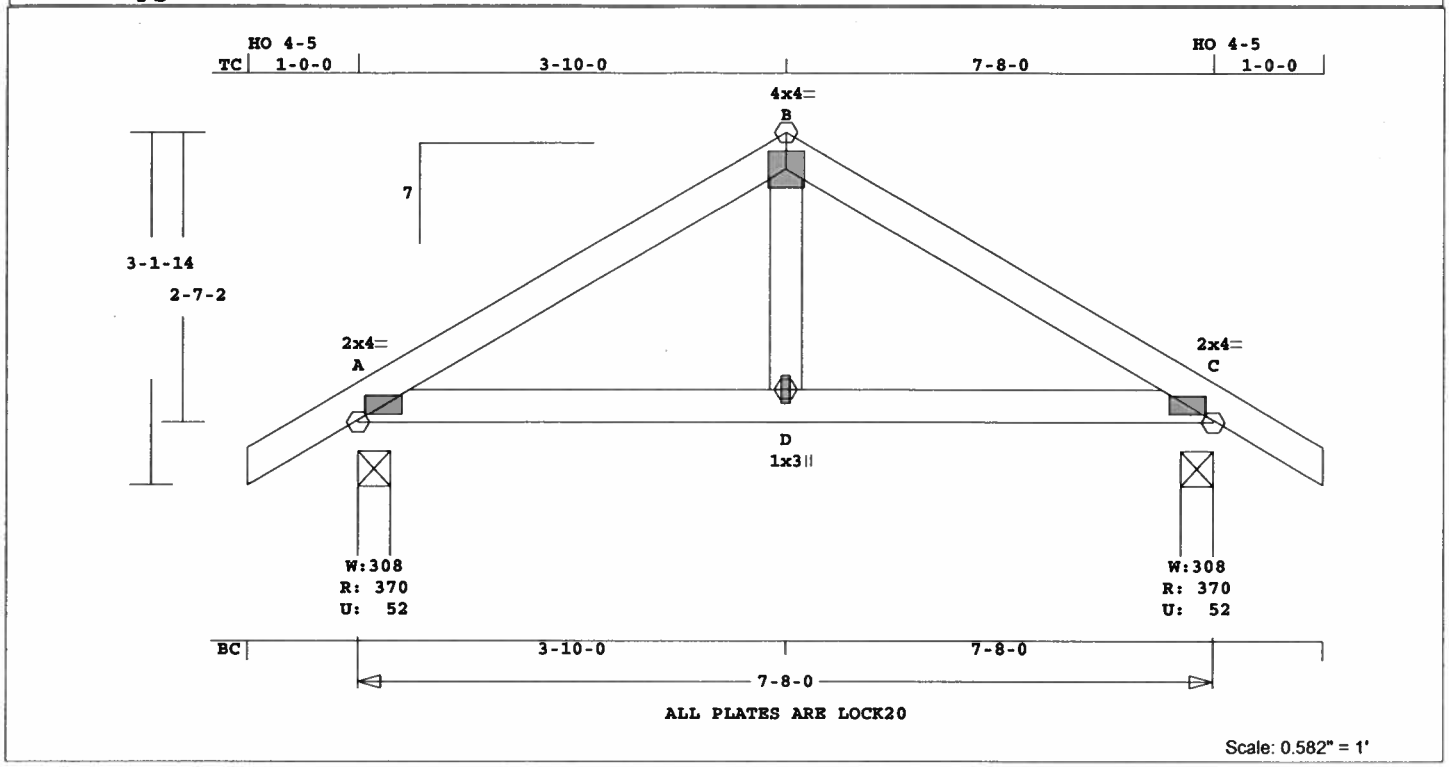
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	D9	1	TR	300200	8	0	0	T06020649

U# J#haygood-mclearn McLEARN

to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 294 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	E1	4	TR	70800	7	1- 0- 0	1- 0- 0	T06020649

U# J#haygood-mclearn McLEARN



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.0 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.09	2x 4	SP-#2
BC	0.10	2x 4	SP-#2
WB	0.02	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 8- 0
BC	Cont.	0- 0- 0	7- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	371	52	3- 8	1- 8
			Hz =	-43
C	371	52	3- 8	1- 8
			Hz =	44

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.09	317 C	0.00	0.09
B -C	0.09	317 C	0.00	0.09
--------Bottom Chords---------				
A -D	0.10	274 T	0.04	0.06
D -C	0.10	274 T	0.04	0.06
------------Webs-------------				
D -B	0.02	161 T		

LL Defl 0.00" in A -D L/999
TL Defl -0.01" in A -D L/999
Shear // Grain in A -B 0.11

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.67
B	LOCK	4.0x 4.0	Ctr	Ctr	0.46
C	LOCK	2.0x 4.0	Ctr	Ctr	0.67
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

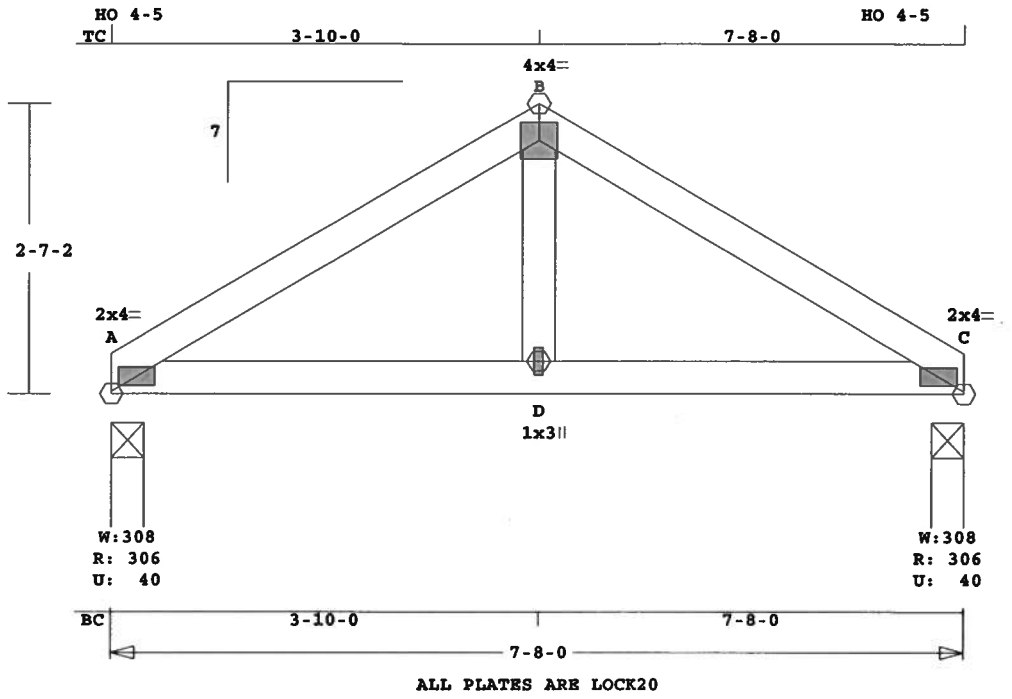
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 317 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	E2	3	TR	70800	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



Scale: 0.582" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 35.6 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.09	2x 4	SP-#2
BC	0.10	2x 4	SP-#2
WB	0.02	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 8- 0
BC Cont.	0- 0- 0	7- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	307	41	3- 8	1- 8
			Hz =	-43
C	307	41	3- 8	1- 8
			Hz =	44

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.09	317 C	0.00	0.09
B -C	0.09	317 C	0.00	0.09

--------Bottom Chords---------

A -D	0.10	274 T	0.04	0.06
D -C	0.10	274 T	0.04	0.06

----------Webs----------

D -B	0.02	161 T		

LL Defl 0.00" in A -D L/999
TL Defl -0.01" in A -D L/999
Shear // Grain in A -B 0.11

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

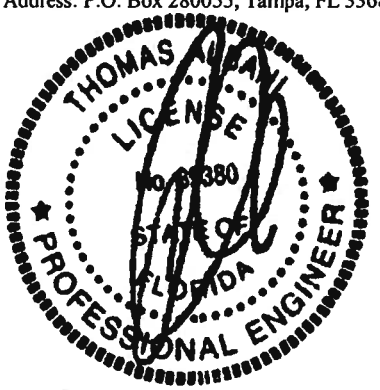
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.67
B	LOCK	4.0x 4.0	Ctr	Ctr	0.46
C	LOCK	2.0x 4.0	Ctr	Ctr	0.67
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 317 Lbs
Quality Control Factor 1.25

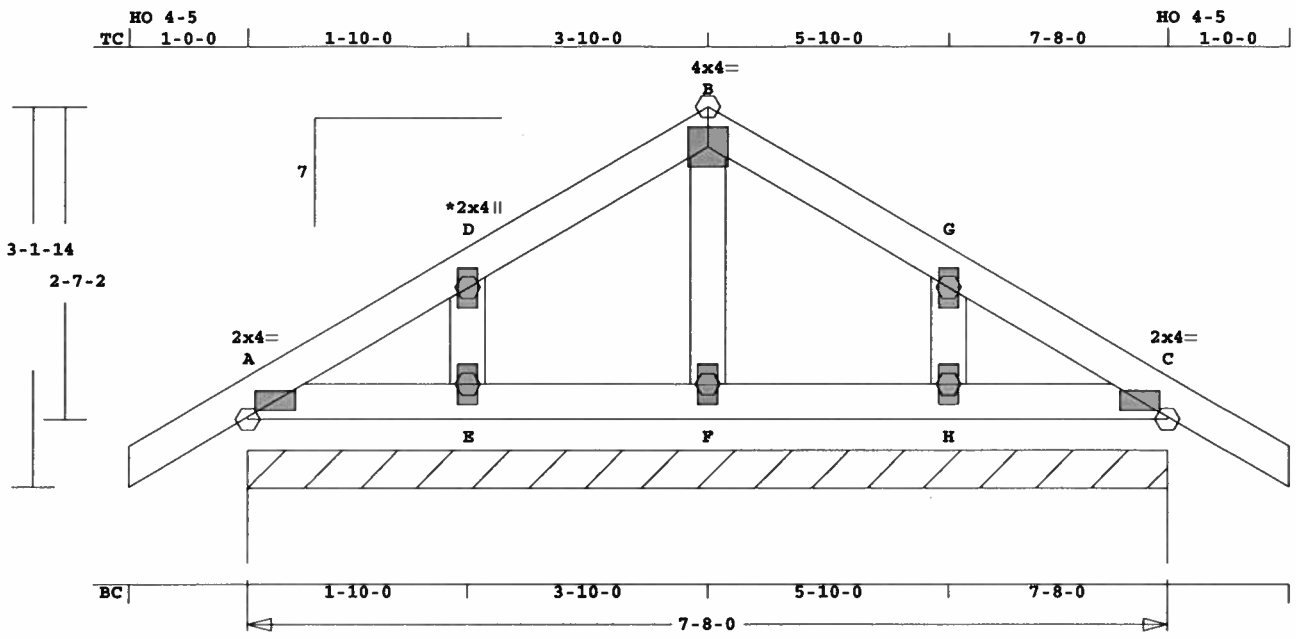
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	E3	1	TR	70800	7	1- 0- 0	1- 0- 0	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.628" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 43.3 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.03	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
GW	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 8- 0
BC	Cont.	0- 0- 0	7- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg	0- 0- 0 to	7- 8- 0	
	741	104 Hz =	44	

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -D	0.03	37	C	0.00	0.03
D -B	0.03	47	C	0.00	0.03
B -G	0.03	47	C	0.00	0.03
G -C	0.03	37	C	0.00	0.03
--------Bottom Chords---------					
A -E	0.01	1	T	0.00	0.01
E -F	0.02	0	T	0.00	0.02
F -H	0.02	0	T	0.00	0.02
H -C	0.01	1	T	0.00	0.01

----------Gable Webs----------

E -D	0.01	118	C
F -B	0.00	70	C
H -G	0.01	118	C

LL Defl 0.00" in F -H L/999
TL Defl 0.00" in F -H L/999
Shear // Grain in D -B 0.07

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

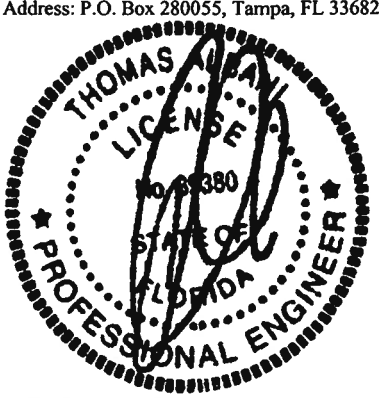
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.67
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	4.0x 4.0	Ctr	Ctr	0.46
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	2.0x 4.0	Ctr	Ctr	0.67
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 118 Lbs
Quality Control Factor 1.25

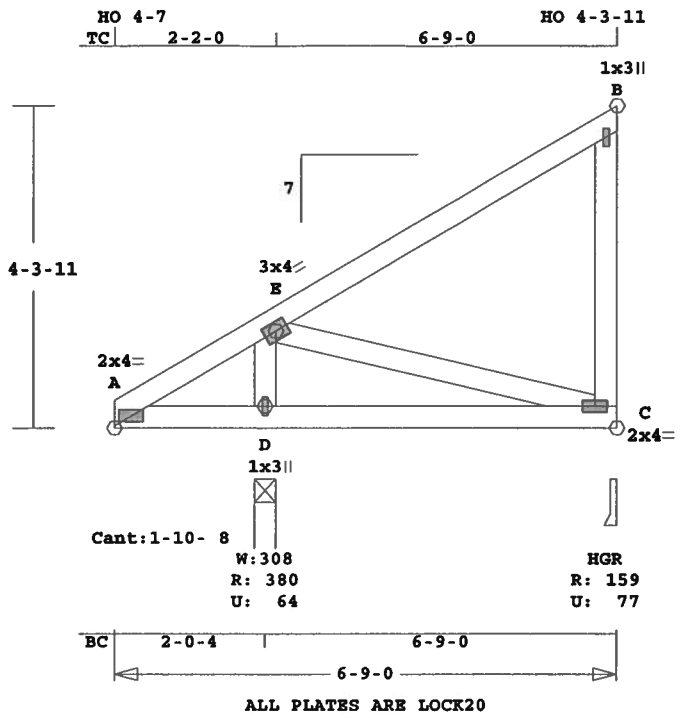
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	M1	4	JCA2	60900	7	0	0	T06020649

U# J#haygood-mclearn McLEARN



Scale: 0.389" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.0 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.22	2x 4	SP-#2
BC	0.12	2x 4	SP-#2
WB	0.04	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	6- 9- 0
BC	Cont.	0- 0- 0	6- 9- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	380	64	3- 8	1- 8
			Hz =	-108
C	160	78	3- 8	1- 8
			Hz =	179

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -E	0.22	45 T	0.01	0.21
E -B	0.21	84 C	0.00	0.21
Bottom Chords				
A -D	0.12	34 T	0.00	0.12
D -C	0.12	133 T	0.00	0.12
Webs				
D -E	0.02	292 C		
E -C	0.02	130 T		
C -B	0.04	120 C	WindLd	

LL Defl -0.01" in D -C L/999
TL Defl -0.03" in D -C L/999
Shear // Grain in E -B 0.18

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.66
E	LOCK	3.0x 4.0	Ctr	Ctr	0.43
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.

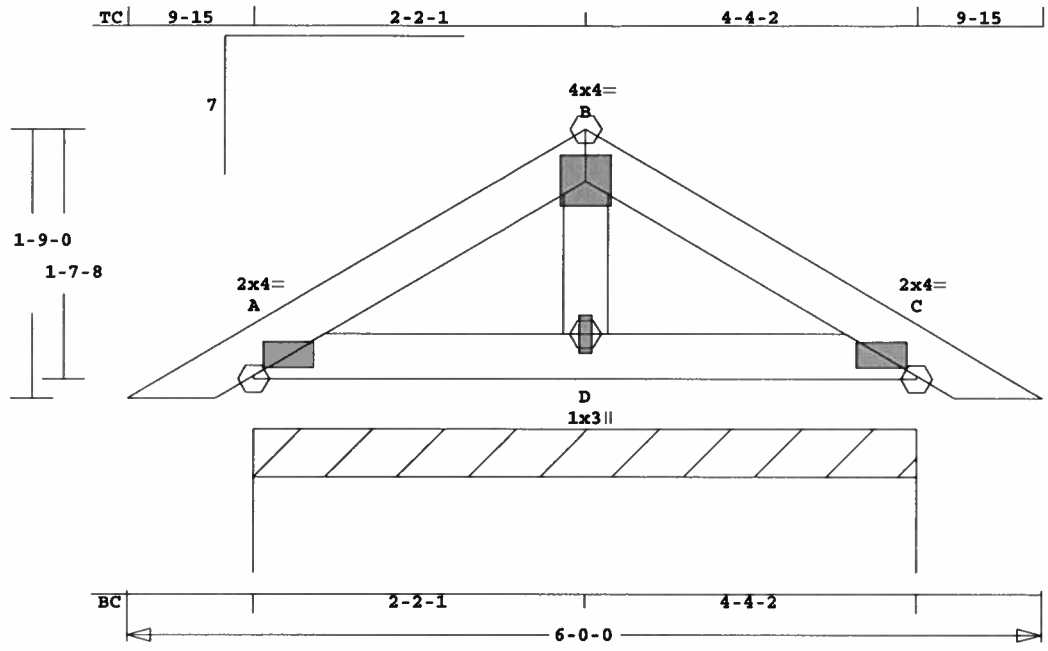
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 292 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	P1	15	TR	60000	7	9-15	9-15	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20

Scale: 0.800" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.2 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.02	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	6- 0- 0
BC Cont.	0- 0- 0	6- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		4- 4- 2	
	453	65	Hz =	24

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.02	75 C	0.00	0.02
B -C	0.02	75 C	0.00	0.02
--------Bottom Chords---------				
A -D	0.02	1 T	0.00	0.02
D -C	0.02	1 T	0.00	0.02
------------Webs------------				
D -B	0.00	41 C		

LL Defl 0.00" in D -C L/999
TL Defl 0.00" in D -C L/999
Shear // Grain in A -B 0.05

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

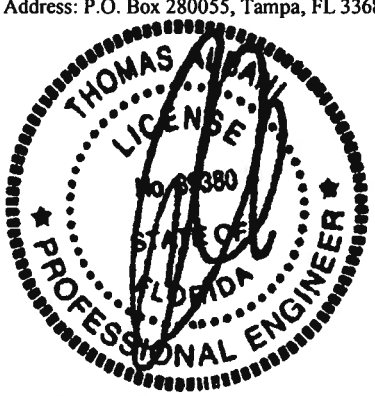
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.62
B	LOCK	4.0x 4.0	Ctr	Ctr	0.42
C	LOCK	2.0x 4.0	Ctr	Ctr	0.62
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 75 Lbs
Quality Control Factor 1.25

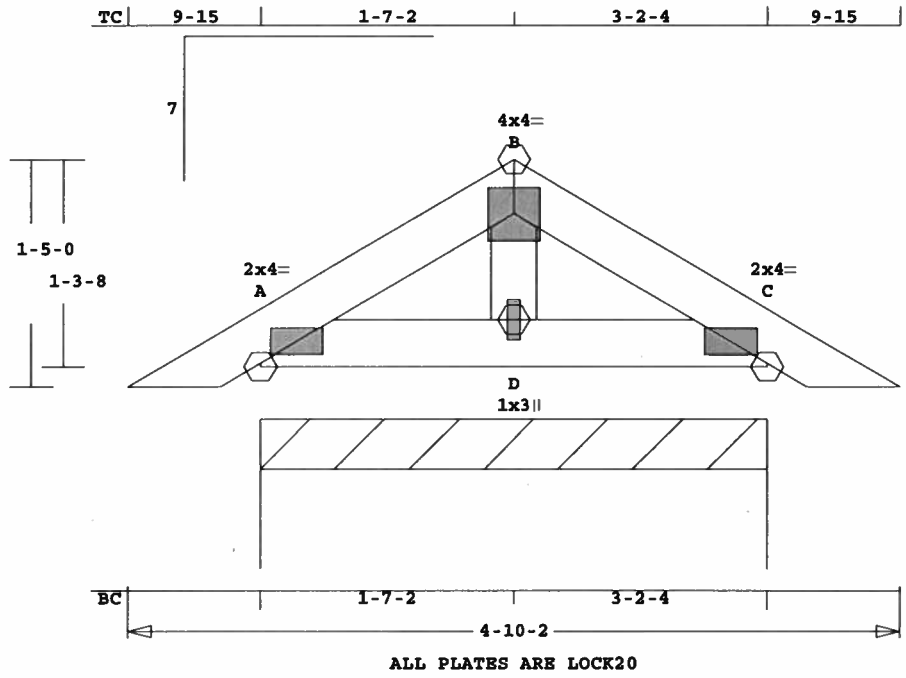
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	P2	2	TR	41002	7	9-15	9-15	T06020649

U# J#haygood-mclearn McLEARN



ALL PLATES ARE LOCK20

Scale: 0.834" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 17.9 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.01	2x 4	SP-#2
BC	0.01	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	4-10- 2
BC	Cont.	0- 0- 0	4-10- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0	to	3- 2- 4	
	361	53	Hz =	17

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.01	45 C	0.00 0.01
B -C	0.01	45 C	0.00 0.01
--------Bottom Chords---------			
A -D	0.01	1 T	0.00 0.01
D -C	0.01	1 T	0.00 0.01
----------Webs----------			
D -B	0.00	37 C	

LL Defl 0.00" in D -C L/999
TL Defl 0.00" in D -C L/999
Shear // Grain in A -B 0.03

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.62
B	LOCK	4.0x 4.0	Ctr	Ctr	0.42
C	LOCK	2.0x 4.0	Ctr	Ctr	0.62
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

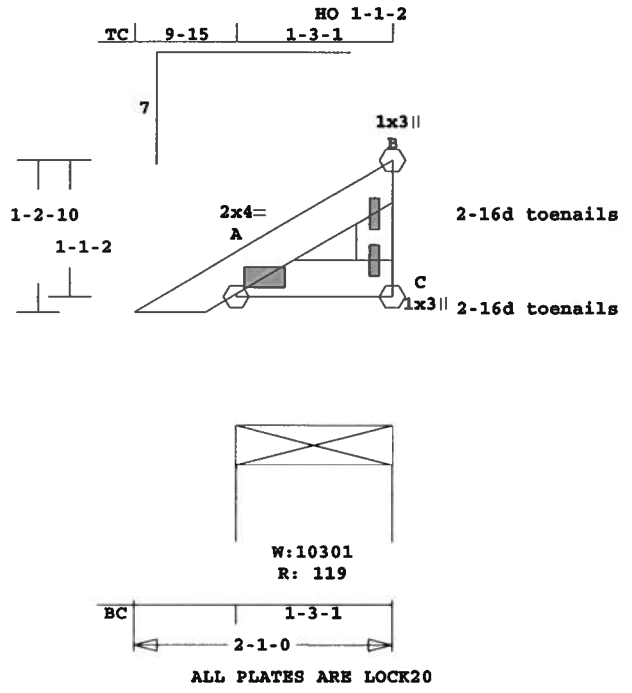
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 45 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 2/8/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
haygood-mclearn	P3	4	JCA2	20100	7	9-15	0	T06020649

U# J#haygood-mclearn McLEARN



Scale: 0.648" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 7.8 LBS

Online Plus -- Version 18.5.027
RUN DATE: 08-FEB-06

	CSI	-Size-	----Lumber----
TC	0.00	2x 4	SP-#2
BC	0.00	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	2- 1- 0
BC Cont.	0- 0- 0	2- 1- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 119	0- 0- 0 to 0 Hz =	1- 3- 1	21
B	34	18	3- 8	1- 8
C	0	0	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.00	9 C	
--------Bottom Chords---------			
A -C	0.00	13 T	
------------Webs------------			
C -B	0.00	0 T	WindLd

LL Defl 0.00" in D -C L/999
TL Defl 0.00" in D -C L/999
Shear // Grain in A -B 0.03

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.62
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	1.0x 3.0	Ctr	Ctr	0.75

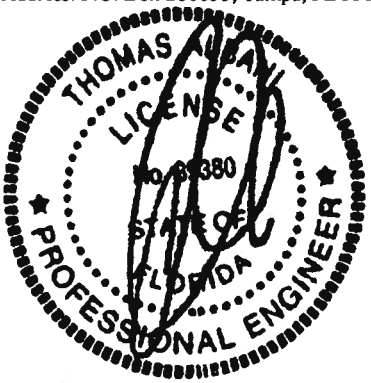
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 9 Lbs
Quality Control Factor 1.25

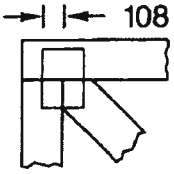
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 2/8/2006

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



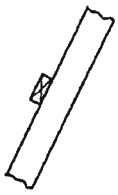
Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108).

PLATE SIZE

6 x 8

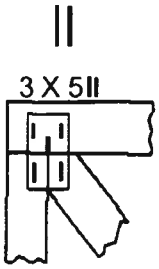
The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots.

LATERAL BRACING



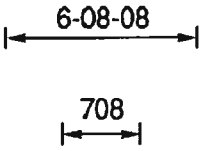
Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restained to prevent simultaneous buckling of adjacent truss members.

PLATE ORIENTATION



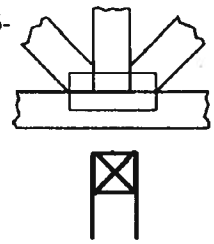
Shown next to plate size, indicates direction of slots in connector plate.

DIMENSIONS



All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).

BEARING



When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearings at each end, unless indicated otherwise. Cutting and fabrication shall be performed on equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and these designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. These designs were prepared in accordance with "National Design Specifications for Wood Construction" (AF & PA), " National Design Standard for Metal Plate Connected Wood Truss Construction" (TPI), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by the Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and "dominoing". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that the design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THESE DESIGNS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE TRUSS DESIGN DRAWINGS & VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB. TRUSS PLACEMENT DIAGRAM.

Robbins Engineering, Inc.

Corporate Headquarters

6904 Parke East Blvd
Tampa, FL 33610-4115
813-972-1135 Fax:813-971-6117