

APPLICANTPATRICK WILSONPHONE904.296.1490

ADDRESS6800SOUTHPOINT PKWY. #300JACKSONVILLEFL32024

OWNERMARONDA HOMES INC. OF FLORIDAPHONE904.296.1490

ADDRESS319SW TIMBER RIDGE DRIVELAKE CITYFL32024

CONTRACTORTHEODORE C. BROCKPHONE904.296.1490

LOCATION OF PROPERTY90-W TO SR. 247-S,TL T0 C-252-B,TR TO TIMBER RIDGE,TL AND
IT'S THE 10TH LOT ON R.

TYPE DEVELOPMENTSFD/UTILITYESTIMATED COST OF CONSTRUCTION130850.00

HEATED FLOOR AREATOTAL AREAHIGHTSTORIES

2023.002617.001

FOUNDATIONCONCWALLSFRAMEDROOF PITCH6'12FLOORCONC

LAND USE & ZONINGRSF-2MAX. HEIGHT35

Minimum Set Back Requirments:STREET-FRONT25.00REAR15.00SIDE10.00

NO. EX.D.U.0FLOOD ZONEXPPDEVELOPMENT PERMIT NO.

PARCEL ID10-4S-16-02856-110SUBDIVISIONTIMBERLANDS

LOT10BLOCK1PHASEUNITTOTAL ACRES0.50

000001649CBC1256382

Culvert Permit No.Culvert WaiverContractor's License NumberApplicant/Owner/Contractor

18"X32'MITERED08-0459BLKHDN

Driveway ConnectionSeptic Tank NumberLU & Zoning checked byApproved for IssuanceNew Resident

COMMENTS: ELEVATION CONFIRMATION LETTER REQUIRED @ SLAB. MFE @ 101.00' PER PLAT.

NOC ON FILE.

Check # or Cash919016

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary PowerFoundationMonolithic

date/app. bydate/app. bydate/app. by

Under slab rough-in plumbingSlabSheathing/Nailing

date/app. bydate/app. bydate/app. by

FramingRough-in plumbing above slab and below wood floor

date/app. bydate/app. by

Electrical rough-inHeat & Air DuctPeri. beam (Lintel)

date/app. bydate/app. bydate/app. by

Permanent powerC.O. FinalCulvert

date/app. bydate/app. bydate/app. by

M/H tie downs, blocking, electricity and plumbingPool

date/app. bydate/app. by

ReconnectionPump poleUtility Pole

date/app. bydate/app. bydate/app. by

M/H PoleTravel TrailerRe-roof

date/app. bydate/app. bydate/app. by

BUILDING PERMIT FEE \$655.00CERTIFICATION FEE \$13.09SURCHARGE FEE \$13.09

MISC. FEES \$0.00ZONING CERT. FEE \$50.00FIRE FEE \$0.00WASTE FEE \$

FLOOD DEVELOPMENT FEE \$FLOOD ZONE FEE \$25.00CULVERT FEE \$25.00TOTAL FEE781.18

INSPECTORS OFFICECLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0807-25 Date Received 7/10/08 By GT Permit # 27235-4649
 Zoning Official BLK Date 22.07.08 Flood Zone arm pit FEMA Map # N/A Zoning R8F-2
 Land Use Res. Land Elevation N/A MFE 101.0 ft River N/A Plans Examiner ND Date 7-21-08
 Comments Elevation Configuration Letter Required at slab
☒ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel # _____
☐ Dev Permit # _____ ☐ In Floodway ☒ Letter of Authorization from Contractor
☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 08-0459 Fax (904)-332-6367
 Name Authorized Person Signing Permit Patrick Wilson Phone (904)-296-1490
 Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216
 Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490
 911 Address 319 SW TIMBER RIDGE DRIVE, Lake City, FL 32024
 Contractors Name Theodore C. Brock Phone (407)-227-3504
 Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Fee Simple Owner Name & Address N/A
 Bonding Co. Name & Address N/A
 Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771
 Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789
 Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 10-45-10-02856-110 Estimated Cost of Construction \$91,800.00
 Subdivision Name Timberlands Lot 10 Block 1 Unit _____ Phase _____
 Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge DRIVE, 10th LOT ON Right

Number of Existing Dwellings on Property 0
 Construction of Residential Single Family Dwelling Total Acreage .50 Lot Size _____
 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height N/A
 Actual Distance of Structure from Property Lines - Front 50' Side 36' (L) Side 37' (R) Rear 68'
 Number of Stories 1 Heated Floor Area 2023 Total Floor Area 2017 Roof Pitch 12/6

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Sent Email 7/22/08

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

STEVE Hogg

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

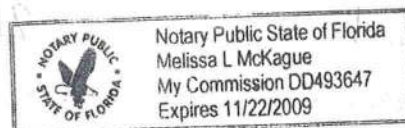
Theodore C. Brock

Contractor's License Number BC 12510382
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10 day of June 2008.
Personally known XXX or Produced Identification _____

State of Florida Notary Signature (For the Contractor)
Melissa L. McKague

SEAL:



Columbia County Building Permit Application

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

Steve Hogg

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Theodore C. Brock

Contractor's License Number

CPC 1256382

Columbia County

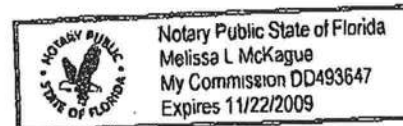
Competency Card Number

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 24 day of June 2008.
Personally known XXX or Produced Identification _____

State of Florida Notary Signature (For the Contractor)

Melissa L. McKague

SEAL:



Page 2 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

This Instrument Prepared by and Return to :

Amy Wesp
SOUTHERN TITLE HOLDING
COMPANY, LLC.
3943 BAY MEADOWS ROAD
JACKSONVILLE, Florida 32217

as a necessary incident to the fulfillment of conditions
contained in a title insurance commitment issued by it.

Property Appraisers Parcel I.D. (Folio) Number(s):

R02856-000

Grantee(s) I.D.#(s):

File No: JX0812085

Inst: 200812010775 Date: 6/6/2008 Time: 1:05 PM

Doc Stamp-Deed: 6293.00

DC, P. DeWitt Cason, Columbia County Page 1 of 1 8:1151 P:2365

WARRANTY DEED
(CORPORATION)

This Warranty Deed Made this 27th day of May, 2008, by RML HOLDINGS INC., A FLORIDA CORPORATION, and having its place of business at 703 NW BLACKBERRY CIRCLE, LAKE CITY, Florida 32055, hereinafter called the grantor,

to MARONDA HOMES, INC. OF FLORIDA, A FLORIDA CORPORATION, whose post office address is: 11200 ST. JOHNS INDUSTRIAL PARKWAY, JACKSONVILLE, FLORIDA 32246, hereinafter called the grantee,

\$899,000.00

WITNESSETH: That said grantor, for and in consideration of the sum of ~~\$14,000~~ Dollars and other valuable considerations, receipt whereof is hereby acknowledged, by these presents grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz: LOTS 1, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 24, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, AND 41, OF TIMBERLANDS, PHASE 1, ACCORDING TO PLAT THEREOF AS RECORDED IN PLAT BOOK 9, PAGE 26 AND 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007, reservations, restrictions and easements of record, if any.

(Wherever used herein the terms "grantor" and "grantee" included all the parties to this instrument, and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation.)

In Witness Whereof, the Grantor has caused these presents to be executed in its name, and its corporate seal to be hereunto affixed, by its proper officers thereunto duly authorized, the day and year first above written.

Signed, sealed and delivered in our presence:

ATTEST:

Secretary

RML HOLDINGS INC.

Witness Signature:

Printed Name:

Worth D. Morris
WORTH D. MORRIS

Witness Signature:

Printed Name:

Jody M. Coble, ACP
Jody M. Coble

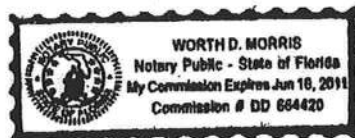
BY:

ROBERT R. LARDIZABAL, PRESIDENT

STATE OF FLORIDA
COUNTY OF DUVAL

The foregoing instrument was acknowledged before me this 28th day of May, 2008, by ROBERT R. LARDIZABAL as PRESIDENT of RML HOLDINGS INC., A FLORIDA CORPORATION, on behalf of the corporation. He/she is personally known to me or who has produced driver license(s) as identification.

My Commission Expires:



Printed Name: *WORTH D. MORRIS*
Notary Public
Serial Number

**UNANIMOUS WRITTEN CONSENT
OF THE BOARD OF DIRECTORS
OF
MARONDA HOMES, INC. OF FLORIDA**

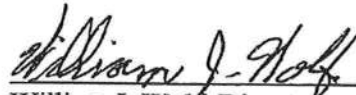
The undersigned, constituting the entire Board of Directors of Maronda Homes, Inc. of Florida (the "Corporation") and waiving all requirements of notice, consents to the corporate actions specified below and adopts the following resolutions by written consent, without a meeting, pursuant to the Florida Business Corporation Act:

RESOLVED, that the following persons be and are hereby authorized, empowered, and designated to enter into agreements to buy and sell individual lots and houses to individual purchasers, to execute deeds to effectuate the buying or selling of individual lots, and to enter into short-term leases not to exceed two (2) years for such individual houses.

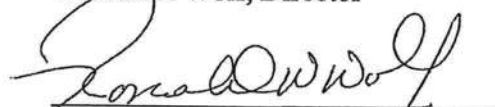
I FURTHER CERTIFY, that the following persons hold the following offices with the Corporation:

William J. Wolf	C.E.O.
Wayne Von Dreele	President
Ronald W. Wolf	Vice President
Ronald W. Wolf	Treasurer
Ronald W. Wolf	Secretary
Scott C. Howard	Vice President
Thomas Greenawalt	Vice President
William Rousch	Vice President
P. Thomas Prior	Vice President
Richard A. Brown	Vice President
Steven R. Hogg	Vice President
Dominick Donato	Vice President
Peter Chun	Vice President
George Friedman	Vice President
Mark Bowes	Vice President
Gregg S. Carlson	Vice President
Chad Johannesen	Vice President
William Berryhill	Vice President
John P. Mills	Vice President
John Williams	Vice President

IN WITNESS WHEREOF, the undersigned has executed this consent effective as of the 16th day of January 2008.



William J. Wolf, Director



Ronald W. Wolf, Director



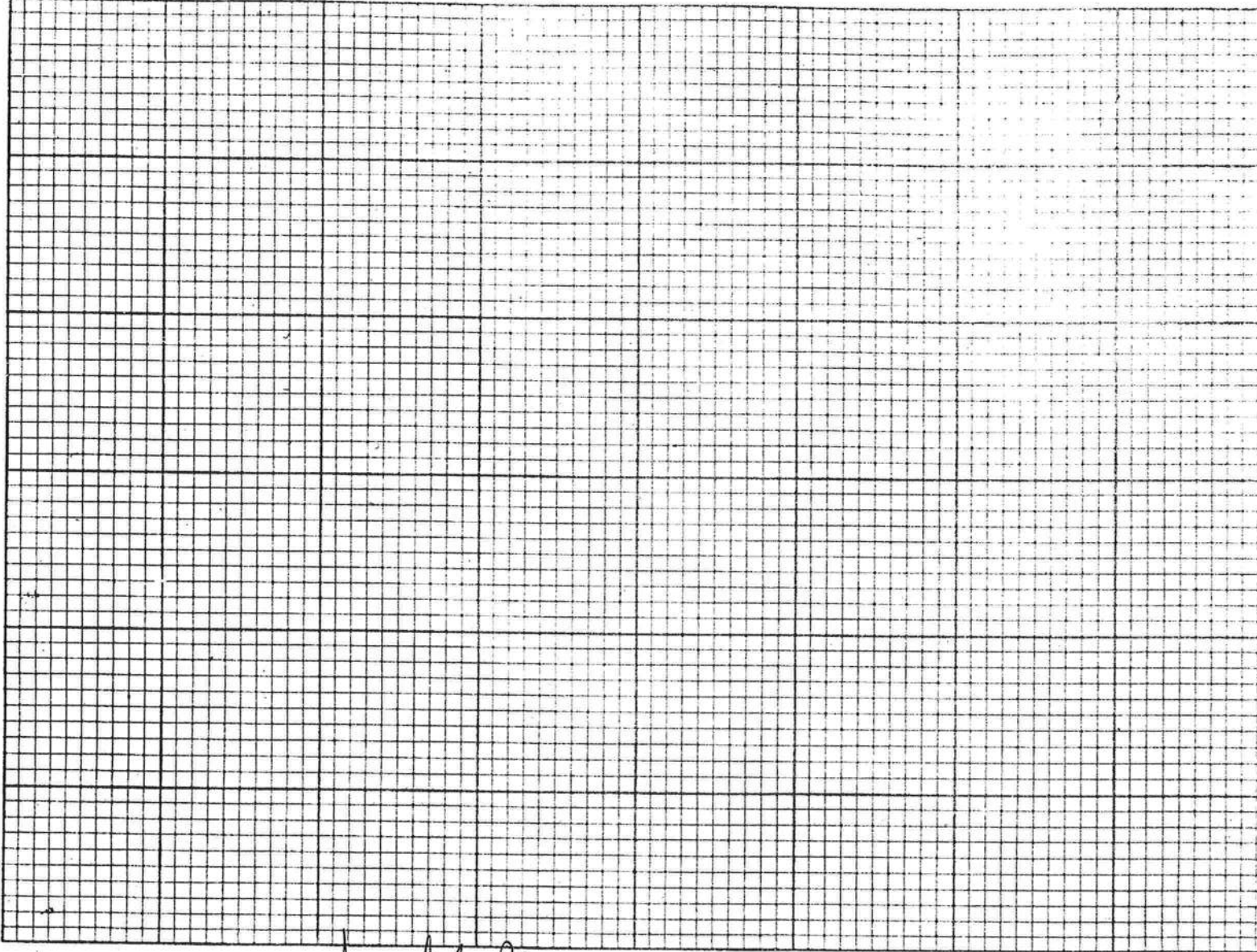
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 08-0459

PART II - SITE PLAN

Lot 10
Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: See Site Plan

Site Plan submitted by: Patricia Wilson 6/25/08
Signature

Plan Approved ☒ Not Approved ☐

By Don S. Lander Columbia County Health Department
Agent Title Date 7-3-08

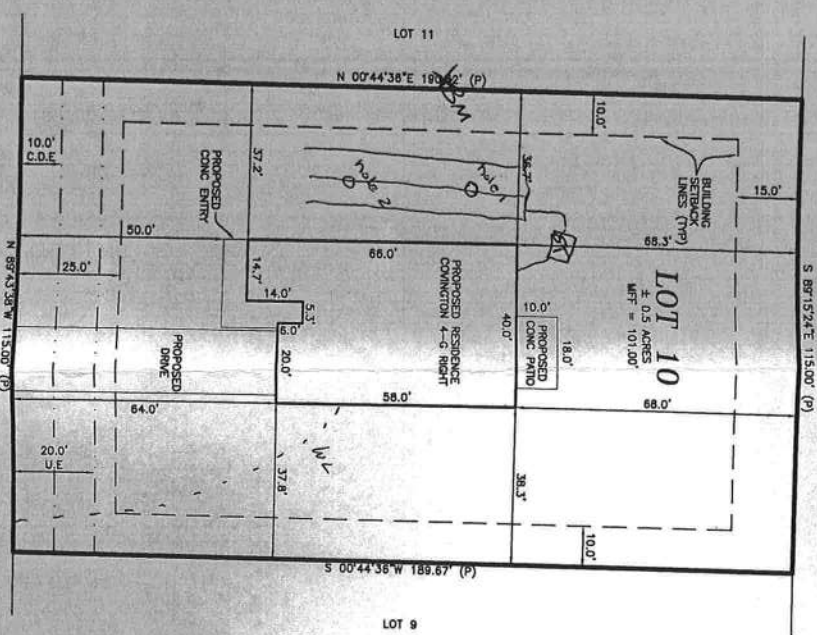
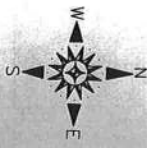
ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

PROPOSED BUILDING LAYOUT

1) MARONDA HOMES

SURVEYOR NOTES:

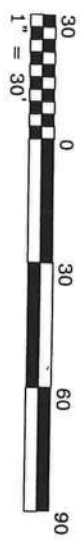
7-3-01



S.W. TIMBER RIDGE DRIVE
60 RIGHT-OF-WAY

TITLE NOTE:

THIS SURETY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURETOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

[illegible]

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A
TRUE REPRESENTATION OF A SURVEY PERFORMED UNDER
MY SUPERVISION OF THE FLORIDA DEPARTMENT OF TRANSPORTATION,
THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA
BOARD OF LAND SURVEYORS, PARAGRAPH 12, SECTION 47.027, FLORIDA
STATUTES, AND CHAPTER 119.47, FLORIDA ADMINISTRATIVE CODE.

BY: James E. Brinkman, PLS - PLS CERT# 5582

DATE: 6/12/08

BRINKMAN SURVEYING & MAPPING INC.
4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'	THE BENCHMARK IN QUALITY SERVICE™	DRAWN BY: ZL
DATE: 6/12/08		CHECKED BY: J.B.

PREPARED FOR: MARONDA

DRAWING NUMBER

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

10/1 TM

Project Name: COVINGTON GAINESVILLE	Builder: MARONDA HOMES
Address: 319 SW Timber Ridge Dr.	Permitting Office: Columbia
City, State: Leake City, FL 32165	Permit Number:
Owner: ELECTRIC	Jurisdiction Number:
Climate Zone: North	

1. New construction or existing New ☐	12. Cooling systems
2. Single family or multi-family Single family ☐	a. Central Unit Cap: 40.5 kBtu/hr ☐
3. Number of units, if multi-family 1 ☐	SEER: 13.00 ☐
4. Number of Bedrooms 4 ☐	b. N/A ☐
5. Is this a worst case? Yes ☐	c. N/A ☐
6. Conditioned floor area (ft ²) 2023 ft² ☐	13. Heating systems
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)	a. Electric Heat Pump Cap: 40.5 kBtu/hr ☐
a. U-factor: Description Area	HSPF: 8.10 ☐
(or Single or Double DEFAULT) 7a(Sngle Default) 168.0 ft² ☐	b. N/A ☐
b. SHGC:	c. N/A ☐
(or Clear or Tint DEFAULT) 7b. (Clear) 168.0 ft² ☐	14. Hot water systems
8. Floor types	a. Electric Resistance Cap: 50.0 gallons ☐
a. Slab-On-Grade Edge Insulation R=0.0, 208.0(p) ft ☐	EF: 0.90 ☐
b. N/A ☐	b. N/A ☐
c. N/A ☐	c. Conservation credits
9. Wall types	(HR-Heat recovery, Solar
a. Concrete, Int Insul, Exterior R=4.1, 1180.0 ft² ☐	DHP-Dedicated heat pump)
b. Frame, Steel, Adjacent R=13.0, 278.0 ft² ☐	15. HVAC credits PT, ☐
c. N/A ☐	(CF-Ceiling fan, CV-Cross ventilation,
d. N/A ☐	HF-Whole house fan,
e. N/A ☐	PT-Programmable Thermostat,
10. Ceiling types	MZ-C-Multizone cooling,
a. Under Attic R=19.0, 2158.0 ft² ☐	MZ-H-Multizone heating)
b. N/A ☐	
c. N/A ☐	
11. Ducts	
a. Sup: Unc. Ret: Con. AH: Interior Sup. R=6.0, 200.0 ft ☐	
b. N/A ☐	

Glass/Floor Area: 0.08

Total as-built points: 28282

Total base points: 28813

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Wayne CampbellDATE: 06/17/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Melissa McRaeDATE: 06/16/08

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
GLASS TYPES										
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points			
.18	2023.0	18.59	6769.0	1.Single, Clear	NE	1.0 6.0	48.0	33.55	0.97	1564.0
				2.Single, Clear	SE	1.0 6.0	48.0	48.65	0.96	2241.0
				3.Single, Clear	NE	1.0 8.0	40.0	33.55	0.99	1330.0
				4.Single, Clear	NW	1.0 6.0	16.0	29.42	0.97	457.0
				5.Single, Clear	SW	1.0 6.0	16.0	45.75	0.96	702.0
				As-Built Total:			168.0			6294.0
WALL TYPES										
Area X BSPM = Points				Type	R-Value		Area X SPM = Points			
Adjacent	278.0	0.70	194.6	1. Concrete, Int Insul, Exterior	4.1		1180.0	1.13		1339.3
Exterior	1180.0	1.70	2006.0	2. Frame, Steel, Adjacent	13.0		278.0	0.90		250.2
Base Total:		1458.0	2200.6	As-Built Total:			1458.0			1589.5
DOOR TYPES										
Area X BSPM = Points				Type			Area X SPM = Points			
Adjacent	18.0	2.40	43.2	1.Adjacent Wood			18.0	2.40		43.2
Exterior	20.0	6.10	122.0	2.Exterior Wood			20.0	6.10		122.0
Base Total:		38.0	165.2	As-Built Total:			38.0			165.2
CEILING TYPES										
Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points			
Under Attic	2023.0	1.73	3499.8	1. Under Attic	19.0		2158.0	2.34 X 1.00		5049.7
Base Total:		2023.0	3499.8	As-Built Total:			2158.0			5049.7
FLOOR TYPES										
Area X BSPM = Points				Type	R-Value		Area X SPM = Points			
Slab	208.0(p)	-37.0	-7696.0	1. Slab-On-Grade Edge Insulation	0.0		208.0(p)	-41.20		-8569.6
Raised	0.0	0.00	0.0							
Base Total:		-7696.0		As-Built Total:			208.0			-8569.6
INFILTRATION										
Area X BSPM = Points							Area X SPM = Points			
	2023.0	10.21	20654.8				2023.0	10.21		20654.8

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 25593.4				Summer As-Built Points: 25183.6						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
				(sys 1: Central Unit 40500btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS)						
25593.4	0.3250		8317.9	25184	1.00	(1.08 x 1.147 x 0.91)	0.260	0.950		7018.5
				25183.6	1.00	1.128	0.260	0.950		7018.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2023.0	20.17	7345.0	1.Single, Clear	NE	1.0	6.0	48.0	32.04	1.00	1539.0
				2.Single, Clear	SE	1.0	6.0	48.0	21.82	1.04	1087.0
				3.Single, Clear	NE	1.0	8.0	40.0	32.04	1.00	1280.0
				4.Single, Clear	NW	1.0	6.0	16.0	32.93	1.00	526.0
				5.Single, Clear	SW	1.0	6.0	16.0	24.09	1.02	393.0
				As-Built Total:				168.0		4825.0	
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	278.0	3.60	1000.8	1. Concrete, Int Insul, Exterior	4.1		1180.0	6.42		7575.6	
Exterior	1180.0	3.70	4366.0	2. Frame, Steel, Adjacent	13.0		278.0	4.90		1362.2	
Base Total: 1458.0 5366.8				As-Built Total:				1458.0		8937.8	
DOOR TYPES Area X BWPM = Points				Type	Area X WPM = Points						
Adjacent	18.0	11.50	207.0	1.Adjacent Wood			18.0	11.50		207.0	
Exterior	20.0	12.30	246.0	2.Exterior Wood			20.0	12.30		246.0	
Base Total: 38.0 453.0				As-Built Total:				38.0		453.0	
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	2023.0	2.05	4147.1	1. Under Attic	19.0		2158.0	2.70 X 1.00		5826.6	
Base Total: 2023.0 4147.1				As-Built Total:				2158.0		5826.6	
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	208.0(p)	8.9	1851.2	1. Slab-On-Grade Edge Insulation	0.0		208.0(p)	18.80		3910.4	
Raised	0.0	0.00	0.0								
Base Total: 1851.2				As-Built Total:				208.0		3910.4	
INFILTRATION Area X BWPM = Points				Area X WPM = Points							
2023.0 -0.59 -1193.6				2023.0 -0.59 -1193.6							

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:
----------------	-----------

BASE			AS-BUILT						
Winter Base Points: 17969.6			Winter As-Built Points: 22759.2						
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
17969.6	0.5540	9955.1	(sys 1: Electric Heat Pump 40500 btuh ,EFF(8.1) Ducts:Unc(S),Con(R),Int(AH),R6.0 22759.2 1.000 (1.060 x 1.169 x 0.93) 0.421 0.950 10489.5						
17969.6	0.5540	9955.1	22759.2	1.00	1.152	0.421	0.950	10489.5	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
WATER HEATING				Tank	EF	Number of	X	Tank	X	Credit
Number of	X	Multiplier	=	Volume		Bedrooms		Ratio	Multiplier	=
Bedrooms			Total							Total
4		2635.00	10540.0	50.0	0.90	4		1.00	2693.56	1.00
				As-Built Total:						10774.2

CODE COMPLIANCE STATUS									
BASE					AS-BUILT				
Cooling	+	Heating	+	Hot Water	Cooling	+	Heating	+	Hot Water
Points		Points		Points	Points		Points		Points
				=					=
				Total					Total
Points		Points		Points	Points		Points		Points
8318		9955		10540	7019		10489		10774
				28813					28282

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.2

The higher the score, the more efficient the home.

ELECTRIC, , , ,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 40.5 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	2023 ft ²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 40.5 kBtu/hr
(or Single or Double DEFAULT)	7a(Sngle Default) 168.0 ft ²		HSPF: 8.10
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 168.0 ft ²		
8. Floor types		c. N/A	
a. Slab-On-Grade Edge Insulation	R=0.0, 208.0(p) ft	14. Hot water systems	
b. N/A		a. Electric Resistance	Cap: 50.0 gallons
c. N/A			EF: 0.90
9. Wall types		b. N/A	
a. Concrete, Int Insul, Exterior	R=4.1, 1180.0 ft ²	c. Conservation credits	
b. Frame, Steel, Adjacent	R=13.0, 278.0 ft ²	(HR-Heat recovery, Solar	
c. N/A		DHP-Dedicated heat pump)	
d. N/A		15. HVAC credits	PT,
e. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
10. Ceiling types		HF-Whole house fan,	
a. Under Attic	R=19.0, 2158.0 ft ²	PT-Programmable Thermostat,	
b. N/A		MZ-C-Multizone cooling,	
c. N/A		MZ-H-Multizone heating)	
11. Ducts			
a. Sup: Unc. Ret: Con. AH: Interior	Sup. R=6.0, 200.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Melvin M. Maguire Date: 10/17/08

Address of New Home: 319 SW Timber Ridge Dr City/FL Zip: Lake City, FL 32015



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5.2)



Project Summary
Entire House
4005 MARONDA WAY

Job: COVINGTON
Date:
By: G.CARMACK

SANFORD, FL 32771

Project Information

For: COVINGTON

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	30914 Btuh
Ducts	5067 Btuh
Central vent (75 cfm)	3036 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	39017 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft ²)	2023	2023
Volume (ft ³)	16184	16184
Air changes/hour	0.32	0.16
Equiv. AVF (cfm)	86	43

Heating Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Model N4H342AKA

Efficiency	8.1 HSPF
Heating input	
Heating output	0 Btuh @ 47°F
Temperature rise	0 °F
Actual air flow	0 cfm
Air flow factor	0.000 cfm/Btuh
Static pressure	0.60 in H2O
Space thermostat	

Sensible Cooling Equipment Load Sizing

Structure	20727 Btuh
Ducts	10987 Btuh
Central vent (75 cfm)	1395 Btuh
Blower	0 Btuh

Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	32115 Btuh

Latent Cooling Equipment Load Sizing

Structure	1917 Btuh
Ducts	2588 Btuh
Central vent (75 cfm)	2637 Btuh
Equipment latent load	7142 Btuh

Equipment total load	39257 Btuh
Req. total capacity at 0.76 SHR	3.5 ton

Cooling Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Cond N4H342AKA
Coil FSM4X4200A

Efficiency	13 SEER
Sensible cooling	30780 Btuh
Latent cooling	9720 Btuh
Total cooling	40500 Btuh
Actual air flow	1460 cfm
Air flow factor	0.046 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.82

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



wrightsoft Right-Suite Residential 6.0.90 RSR21115

C:\Documents and Settings\gcarmack\My Documents\Man J Calcs 2007\GAINESVILLE\COVINGTON 13.rpt Cal

2008-Jun-12 11:18:14

Page 1

SANFORD, FL 32771

Project Information

For: COVINGTON

Design Conditions

Location:

Gainesville, FL, US
Elevation: 151 ft
Latitude: 30°N

Outdoor:

Dry bulb (°F)
Daily range (°F)
Wet bulb (°F)
Wind speed (mph)

Heating

33
-
-
15.0

Cooling

92
19 (M)
77
7.5

Indoor:

Indoor temperature (°F)
Design TD (°F)
Relative humidity (%)
Moisture difference (gr/lb)

Heating

70
37
30
10.6

Cooling

75
17
50
52.0

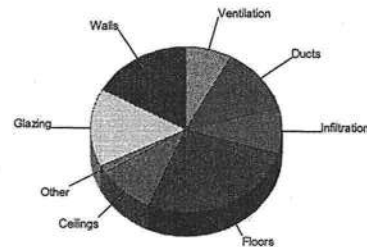
Infiltration:

Method
Construction quality
Fireplaces

Simplified
Average
0

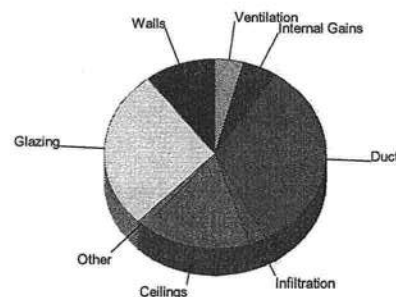
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.4	6640	17.0
Glazing	47.0	6015	15.4
Doors	14.4	548	1.4
Ceilings	1.8	3766	9.7
Floors	10.7	10451	26.8
Infiltration	2.1	3494	9.0
Ducts		5067	13.0
Piping		0	0.0
Humidification		0	0.0
Ventilation		3036	7.8
Adjustments		0	
Total		39017	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.3	3459	10.4
Glazing	70.4	9015	27.2
Doors	11.4	432	1.3
Ceilings	2.6	5358	16.2
Floors	0.0	0	0.0
Infiltration	0.5	803	2.4
Ducts		10987	33.2
Ventilation		1395	4.2
Internal gains		1660	5.0
Blower		0	0.0
Adjustments		0	
Total		33108	100.0



Overall U-value = 0.157 Btuh/ft²-°F

Data entries checked.

Duct System Summary
Entire House
4005 MARONDA WAY

Job: COVINGTON
Date: ORLANDO
By: G.CARMACK

SANFORD, FL 32771

Project Information

For: COVINGTON

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.46 / 0.14 in H2O	0.46 / 0.14 in H2O
Lowest friction rate	2.308 in/100ft	2.308 in/100ft
Actual air flow	1460 cfm	1460 cfm
Total effective length (TEL)	26 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER BDR	c 5890	200	200	3.077	7	x00	VIFx	15.0	0.0	st4
BDR #4	c 2841	120	120	3.077	6	x00	VIFx	15.0	0.0	st3
FAMILY	c 5817	260	260	9.231	8	x00	VIFx	5.0	0.0	st3
MASTER BATH	h 1417	20	20	4.615	5	x00	VIFx	10.0	0.0	st4
LIVING	c 1364	145	145	4.615	6	x00	VIFx	10.0	0.0	st3
KITCHEN/NOOK	c 3263	145	145	4.615	6	x00	VIFx	10.0	0.0	st1
BDR #2	c 2714	105	105	9.231	5	x00	VIFx	5.0	0.0	st2
HALL BATH	h 1150	25	25	3.077	4	x00	VIFx	15.0	0.0	st2
BDR #3	h 5939	175	175	2.308	7	x00	VIFx	20.0	0.0	st2
LAUNDRY	h 2058	40	40	4.615	4	x00	VIFx	10.0	0.0	st1
FOYER	h 2245	60	60	2.308	4	x00	VIFx	20.0	0.0	st2
MASTER TIOLET	h 777	20	20	9.231	4	x00	VIFx	5.0	0.0	st4
DINING	c 2930	145	145	9.231	6	x00	VIFx	5.0	0.0	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	855	855	3.077	551	14	0 x 0	VinIFlx	st1 st2
st2	Peak AVF	605	605	2.308	596	14	0 x 0	VinIFlx	
st3	Peak AVF	525	525	3.077	570	12	0 x 0	VinIFlx	
st4	Peak AVF	240	240	3.077	614	9	0 x 0	VinIFlx	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1460	1460	6.0	2.308	669	20	x0 0		VIFx	

COLUMBIA COUNTY OFFICE OF ZONING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-110

Building permit No. 000027235

Use Classification SFD/UTILITY

Fire: 70.62

Permit Holder THEODORE C. BROCK

Waste: 184.25

Owner of Building MARONDA HOMES INC. OF FLORIDA

Total: 254.87

Location: 319 SW TIMBERRIDGE DRIVE, LAKE CITY, FL

Date: 11/14/2008

Wayne A. Lane

Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)

**Columbia County Building Department
Culvert Permit**

**Culvert Permit No.
000001649**

DATE 08/04/2008 PARCEL ID # 10-4S-16-02856-110

APPLICANT PATRICK WILSON PHONE 904.296.1490

ADDRESS 6800 SOUTHPPOINT PKWY.#300 JACKSONVILLE FL 32216

OWNER MARONDA HOMES INC. OF FLORIDA PHONE 904.296.1490

ADDRESS 319 SW TIMBER RIDGE DRIVE LAKE CITY FL 32024

CONTRACTOR THEODORE C. BROCK PHONE 904.296.1490

LOCATION OF PROPERTY 90-W TO SR. 247-S,TL TO C-252-B,TR TO TIMBE RIDGE,TL AND
IT'S THE 10TH LOT ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBERLANDS 10

SIGNATURE



INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

**ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.**

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



27235

FIELD DENSITY WORKSHEET

CLIENT MAIONDA Homes DATE 5 AUG 08
Lot #10 JOB# 9700101 PROJECT NO. _____
 PROJECT NAME TIMBER LANDS LAKE CITY PERMIT NO. -
 EARTH CONTRACTOR 319 SW Timber Ridge Dr TESTED BY JWL
 COMPACTION REQUIREMENT (%) 95% ☐ Standard Proctor PATRICIA FIELD CONTACT
☒ Modified Proctor
 TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☐ Building Footing ☐ Other _____

TEST LOCATION	LAB PROCTOR		TEST DEPTH	PROBE DEPTH	% MOIST.	WET DENSITY (PCF)	DRY DENSITY (PCF)	% COMP.
	DENS.	OMC						
CTR. OF PAD	104.9	10.1	FT	12"	2.8	106.3	103.4	98.6
CTR. OF W. FTG.	L	L	L	L	4.6	104.6	100.0	95.3
CTR. OF E. FTG.	L	L	L	L	4.2	104.6	100.4	95.7

REMARKS _____

* Density failed to meet minimum project requirement
 ** Retest indicates minimum density requirement was obtained.
 () Client is aware of unsatisfactory test results.

NOTICE OF COMMENCEMENT

Tax Parcel Identification Number 10-45-16-02850-110

County Clerk's Office Stamp or Seal

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT:

1. Description of property (legal description): 10/1 Timberlands
a) Street (job) Address: 319 SW Timber Ridge Dr.
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
a) Name and address: Maronda Homes Inc of FL 10800 Southpoint Pkwy #300 Jax FL 32216
b) Name and address of fee simple titleholder (if other than owner):
c) Interest in property:
4. Contractor Information
a) Name and address: Maronda Homes Inc of FL 10800 Southpoint Pkwy #300 Jax FL 32216
b) Telephone No.: (904) 296-1490 Fax No. (Opt.): (904) 332-0315
5. Surety Information
a) Name and address:
b) Amount of Bond:
c) Telephone No.: Fax No. (Opt.):
6. Lender
a) Name and address:
b) Phone No.:
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
a) Name and address: Southern Title Holding Co LLC 3945 Bay Meadows Rd Jax FL 32217
b) Telephone No.: (904) 739-2205 Fax No. (Opt.):
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name and address:
b) Telephone No.: Fax No. (Opt.):
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY: A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

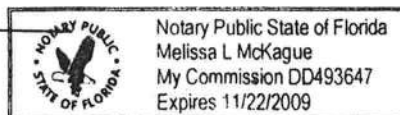
STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager
Theodore C. Brock
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 10 day of June, 2008, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney
fact) for Maronda Homes Inc of Florida (name of party on behalf of whom instrument was executed).

Personally Known ☒ OR Produced Identification _____ Type _____

Notary Signature Melissa L McKague Notary Stamp or Seal:



11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
Signature of Natural Person Signing (in line #10 above).

**HOMETEAM**

PEST DEFENSE®

27235

TREATMENT WORKORDER☐ Termite Baiting System w/Tubes-under-the slab☐ Treat Only ☒ Tubes-under-the slab and Treat ☐ Bora-Care

DATE CALLED IN:	8/6	DATE OF SCHEDULE:	8/8
TIME CALLED IN:		TIME SCHEDULE:	11:00

JOB NAME: <u>Maranda</u>		SUBDIVISION: <u>Timberland</u>	
JOB ADDRESS: <u>319 SW Timber Ridge Dr.</u>			
BILLING NAME:		BILLING PHONE:	
BILLING ADDRESS:			
CALLED IN BY:		PHONE:	PERMIT NUMBER:

LOT & MODEL NUMBER: 10

DATE & TIME COMPLETED: _____

SQUARE FOOT: 2437 LINEAR FOOT: _____ BLOCKVOIDS: _____SLAB TYPE: mono TYPE OF FILL: dirt/sand

APPROX. DEPTH OF FOOTING: Outside: _____ Inside: _____

☐ Addition ☐ Spot Treat ☐ Pool Addition ☐ Driveway☐ Final/Completion ☐ Other _____PESTICIDE USED: ImazaPro TOTAL APPLIED: 125 galPERCENT (%) USED: 1% STICKER POSTED: permit box

PRICE PER SQ. FT. =	TOTAL FOR P.T.		
	ADDITIONAL		
	TAX:		
	TOTAL AMOUNT	\$	

☒ X SSA X TECHNICIAN: Casey

I hereby acknowledge the satisfactory completion of the above described work.

GT 23 / TC

12/05

**COLUMBIA COUNTY
RESIDENTIAL MINIMUM
FOR THE FLORIDA BUILDING CODE
Supplement**

**LIST
& 2006**

ALI
**ALL BUILDING PLANS
FOR THE FLORIDA BUILDING CODE
PLANS OR DRAWING
HAVE THE SEAL AND
REGISTERED IN THE
APPROVED BY THE
AND-TWO FAMILY D**

**FOR DESIGN PURPOSE
FIGURE R301.2(4) IS
USED.**

WIND SPEED LINE SHALL

1. ALL BUILDING
2. ALL BUILDING
3. NO AREA

GENERAL REQUIREMENTS

- ☒ Two (2) complete sets
- ☒ All drawings must be
- ☒ Condition space (S)
- ☒ Designers name and official embos

Site Plan information

- ☒ Dimensions of lot
- ☒ Dimensions of all
- ☒ Location of all of
- ☒ well and septic tank
- ☒ Provide a full leg

Wind-load Engineering

- ☒ Plans or specifications
- ☒ The following information
- ☒ Basic wind speed
- ☒ Wind importance
- ☒ Wind exposure
- ☒ Wind direction shall
- ☒ The applicable wind
- ☒ terms of psf (l)
- ☒ specifically des

Elevations Drawing information

- ☒ All side views of the structure
- ☒ Roof pitch
- ☒ Overhang dimensions and detail with attic ventilation
- ☒ Location, size and height above roof of chimneys
- ☒ Location and size of skylights with Florida Product Approval
- ☒ Number of stories
- ☒ e) Building height from the established grade to the roofs highest peak

FEE:

ROAD IMPACT FEE
10100003632400

\$1,046.00

EMS IMPACT FEE
10300003632210

\$27.88

FIRE PROTECTION IMPACT FEE
10200003632220

\$78.63

CORRECTIONS IMPACT FEE
00100003632200

\$409.16

SCHOOL IMPACT FEE
00100003632900

\$1,500.00

TOTAL FEES CHARGED

\$3,063.67

UNIT 1

CODE 210

CHECK NUMBER

**CODE. ALL
S THAT
ENGINEER
OLOGIES,
R ONE-**

**E PER
ALL BE**

INTERSTATE 75.
---- 100 MPH
---- 110 MPH
REGION

be marked void

engineer, signature
s.l.

listing or proposed

Required:

and applicable wind

design wind pressure in
adding materials not

Floor Plan including:

- Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
- ✓ All exterior and interior shear walls indicated
- ✓ Shear wall opening shown (Windows, Doors and Garage doors)
- ✓ Emergency escape and rescue opening in each bedroom (net clear opening shown)
- ✓ Safety glazing of glass where needed
- ✓ Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
- ✓ Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
- ✓ Plans must show and identify accessibility of bathroom (see FRC 322)

All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

Foundation Plans Per FRC 403:

- ✓ a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- ✓ b) All posts and/or column footing including size and reinforcing
- ✓ c) Any special support required by soil analysis such as piling.
- ✓ d) Assumed load-bearing value of soil _____ (psf)
- e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

CONCRETE SLAB ON GRADE Per FRC R506

- ✓ Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports

PROTECTION AGAINST TERMITES Per FRC 320:

- Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606

- ✓ Show all materials making up walls, wall height, and Block size, mortar type
- ✓ Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

- Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers
- ✓ Girder type, size and spacing to load bearing walls, stem wall and/or piers
- ✓ Attachment of joist to girder
- ✓ Wind load requirements where applicable
- Show required under-floor crawl space
- ✓ Show required amount of ventilation opening for under-floor spaces
- ✓ Show required covering of ventilation opening.
- ✓ Show the required access opening to access to under-floor spaces
- ✓ Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- ✓ Show Draft stopping, Fire caulking and Fire blocking
- Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- ✓ Provide live and dead load rating of floor framing systems (psf).

WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- Fastener schedule for structural members per table R602.3 (1) are to be shown.
- Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- Indicate where pressure treated wood will be placed.
- Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

ROOF SYSTEMS:

- Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- Provide dead load rating of trusses

Conventional Roof Framing Layout Per FRC 802:

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- Valley framing and support details
- Provide dead load rating of rafter system.

ROOF SHEATHING FRC Table R602,3(2) FRC 803

- Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

- Include all materials which will make up the roof assemblies covering; with Florida Product Approval numbers for each component of the roof assemblies covering.

FCB Chapter 13 Florida Energy Efficiency Code for Building Construction

- Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

HVAC information shown

- Manual J sizing equipment or equivalent computation
- Exhaust fans locations in bathrooms

Plumbing Fixture layout shown

- All fixtures waste water lines shall be shown on the foundation plan

Electrical layout shown including:

- Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- Ceiling fans
- Smoke detectors
- Service panel, sub-panel, location(s) and total ampere ratings

- ✓ On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- ✓ Appliances and HVAC equipment and disconnects
- ✓ Arc Fault Circuits (AFCI) in bedrooms
- ✓ Notarized Disclosure Statement for Owner Builders
- ✓ Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice Of Commencement is required to be filed with the building department Before Any Inspections Will Be Done.

Private Potable Water

- Size of pump motor
- Size of pressure tank
- Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- ✓ Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- ✓ Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- ✓ Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- ✓ City Approval: If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- ✓ Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.** A development permit will also be required. The permit cost is \$50.00.
- ✓ Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- ✓ 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

PRODUCT APPROVAL SPECIFICATION SHEET

Location: _____ **Project Name:** _____

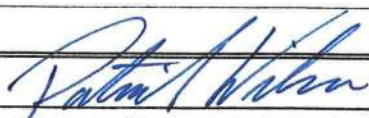
As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

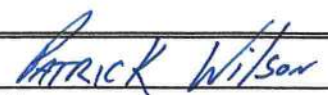
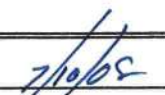
Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection


 Contractor or Contractor's Authorized Agent Signature

 
 Print Name Date

Location

Permit # (FOR STAFF USE ONLY)

Maronda Systems 4005 Maronda Way Sanford FL 32771 (407) 321-0064 Fax (407) 321-3913
Engineer/Architect of Record: Tomas Ponce, P.E. 367 Medallion PL. Chuluota, FL 32766 FL PE # 50068,
Design Criteria: TPI Design: Matrix Analysis MiTek software

COVINGTON G

Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

I, **Tomas Ponce, P.E.** the Architect/Engineer of Record for the above referenced lot

Parameters found on the set of permit drawings.

DATE: JUN 05 2008

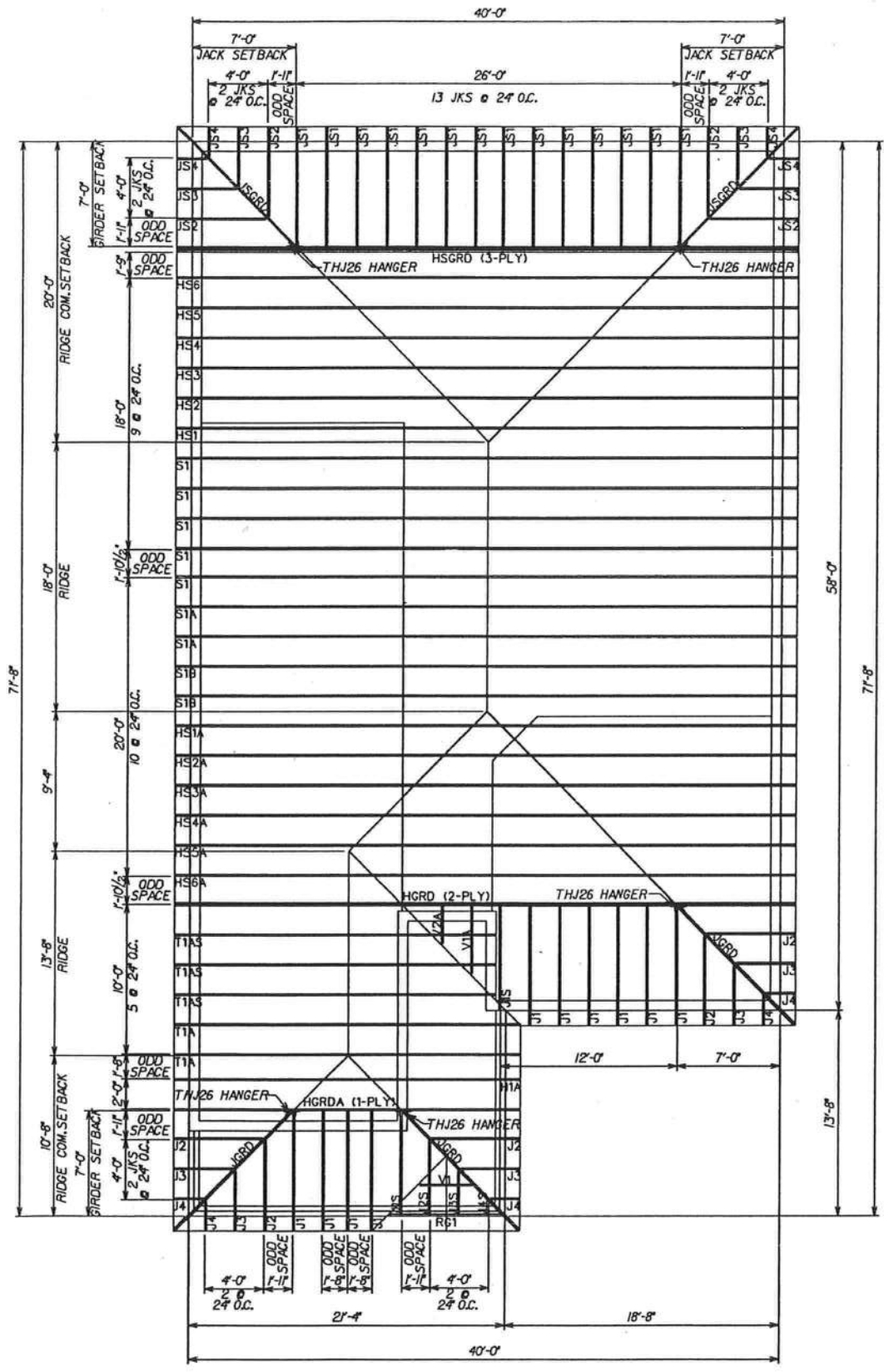
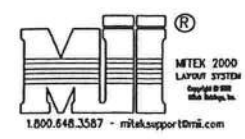
HARDWARE LEGEND

- 1 HUS26
- 2 HUS28
- 3 JUS26
- 4 MP6F
- 5 MPA1 & MPA1F
- 6 SKH26 L/R
- 7 SKHH26 L/R
- 8 SUS26
- 9 SUS28
- 10 THD26
- 11 THD28
- 12 THD28-2
- 13 THDH28-3
- 14 THD48
- 15 THJ26**
- 16 LTW12

[Signature]
JUN 05 2008

HARDWARE MANUFACTURED
BY USP
* HARDWARE MANUFACTURED
BY SIMPSON
** HARDWARE MANUFACTURED
BY CLEVELAND

SCALE: 1/8" = 1'-0"
DATE: 10/22/2007



COVINGTON ELEVATION "G" - FL

GARAGE: RIGHT



DESIGNER: NAME
CHECKER: MIKE

DRAWN BY: K WARD

LOADING-FBC2004/TPI2002

TC LIVE	16.00	SNOW LOAD	0.00
TC DEAD	7.00	LUMBER DOL	1.25
BC LIVE	10.00	PLATE DOL	1.25
BC DEAD	10.00	WIND	125
TOTAL	43.00	SPACING	2'-0"

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

Warning! The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

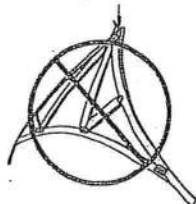
El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

Warning! Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

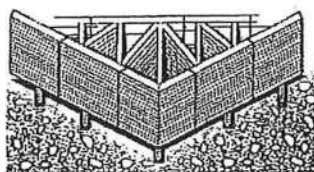
HANDLING — MANEJO

Warning! Avoid lateral bending. — Evite la flexión lateral.



Warning! The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



Checkmark! If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

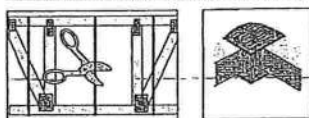
Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostres de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo.

NOTAS GENERALES

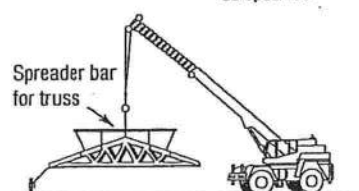
Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arrioste Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.



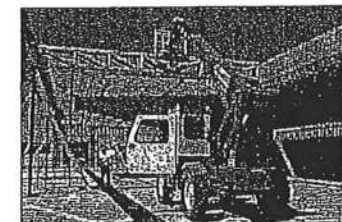
Warning! Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



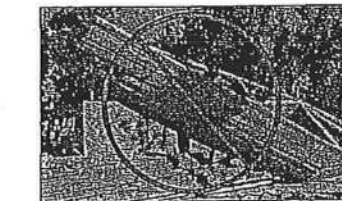
Checkmark! Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



Warning! Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



Warning! Do not store on uneven ground.

No almacene en tierra desigual.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

Warning! Don't overload the crane.
¡Advertencia! No sobrecarga la grúa!
Warning! Never use banding alone to lift a bundle. Do not lift a group of individually banded bundles. Nunca use sólo los empaques para levantar un paquete. No levante un grupo de empaques individuales.



Checkmark! A single lift point may be used for bundles with trusses up to 45'. Two lift points may be used for bundles with trusses up to 60'. Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies. Puede usar dos puntos de levantar para paquetes más de 60 pies. Use por lo menos tres puntos de levantar para paquetes más de 60 pies.

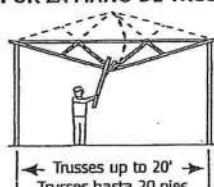
Warning! Do not over load supporting structure with truss bundle. ¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

Checkmark! Place truss bundles in stable position. Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

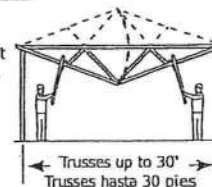
Checkmark! Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



Checkmark! Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



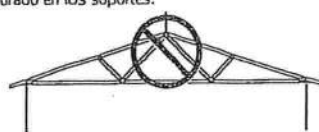
HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALES

Checkmark! Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalada y el truss está asegurado en los soportes.

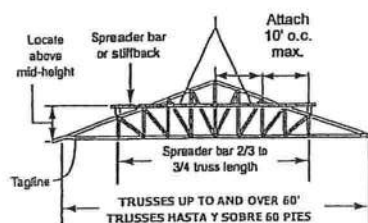
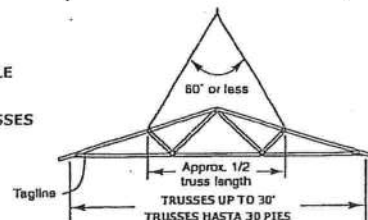
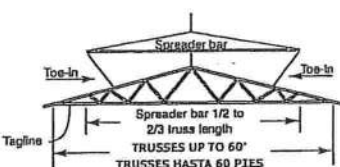
Warning! Using a single pick-point at the peak can damage the truss.

¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



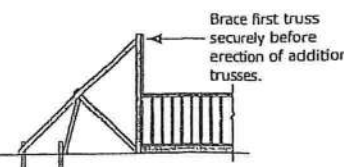
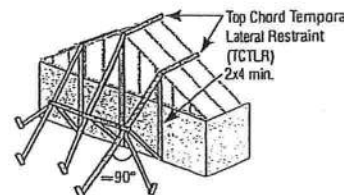
TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

Warning! Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resumen BCSI B2 - Restricción/Arrioste Temporal y Instalación de los Trusses para más información.

Checkmark! Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).



Warning! Do not walk on unbraced trusses. No camine en trusses sueltos.

EL MANEJO, INSTALACIÓN, RESTRICCIÓN Y ARRIOSTRE DE LOS TRUSSES (2006 EDICIÓN)

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

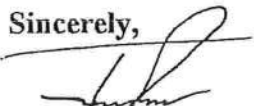
From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

Date: 11/1/06

DESIGN INFORMATION

This design is for an individual building component and does not include information provided by the client. The user of this design disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the user by the designer by the client and the completeness or accuracy of this information as it relates to a specific project and accepts no responsibility or warranty as to any liability related to its use.

This truss has been designed as an individual building component in accordance with ANSI/TPI-1-1995 and NDS-97, to be incorporated as part of the building design by the building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to make sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by framing unless otherwise specified. Vertical members shall be braced at each floor level. If temporary applied rigid ceiling, they should be braced at the maximum spacing of 10'-0". Connector plates shall be minimum thickness 20 gauge. All unbraced gable-end roof members factored to 60 ksi. Grade 40, unless otherwise shown, meeting ASTM A 36, Grade 40, slip resistant shop drawn.

FABRICATION-NOTES-

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knoeholes, bolts or fasteners. Members shall be cut at right fitting weld to wood framing. Connector plates shall be located on both faces of the joint with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide by 4" long. A 6/8 plate is 6" wide x 8" long. (Notes) All web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight line and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential, and erection bracing is always required. Normal precautionary section for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought from the manufacturer of the trusses.

The design leader shall not be expected to truss any line. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.

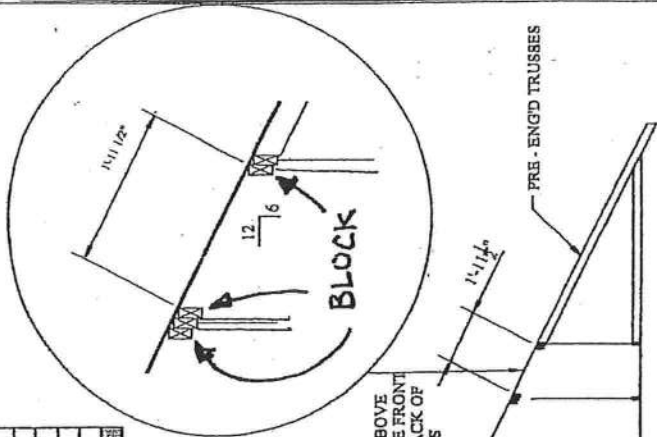
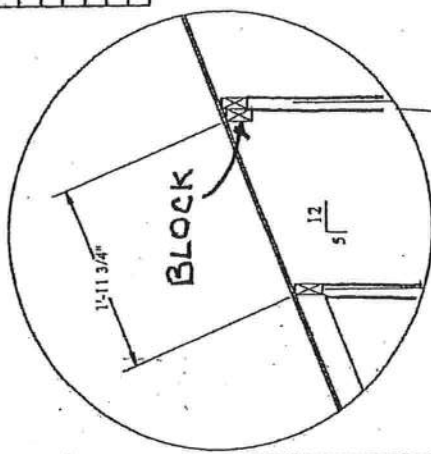
SHIP TRUSS BLOCKING REQUIREMENTS

APA-FORM NO. TT-083

SPAN RATING AND BLOCKING RECOMMENDATIONS FOR USE OVER HIP-ROOFS SUPPORT AT 24" O.C.

Roof Slope	24/18	Panel/Span Rating	40/20	48/24
≤ 1:12	OK	OK	OK	OK
≤ 2:12	OK	OK	OK	OK
≤ 3:12	OK	OK	OK	OK
≤ 4:12	OK	OK	OK	OK
≤ 5:12	OK	OK	OK	OK
≤ 6:12	OK	OK	OK	OK
≤ 7:12	OK	OK	OK	OK
≤ 8:12	OK	OK	OK	OK
≤ 9:12	OK	OK	OK	OK
≤ 10:12	OK	OK	OK	OK
≤ 11:12	OK	OK	OK	OK
≤ 12:12	OK	OK	OK	OK
≤ 13:12	OK	OK	OK	OK
≤ 14:12	OK	OK	OK	OK

NOTE: SHADED AREA REQUIRES 2X LUMBER BLOCKING BETWEEN TRUSS AT UN SUPPORTED PANEL, EDGES IN HTF ROOF AREAS



NOTE: NO BLOCKING IS REQUIRED ON 4/12 AND BELOW PITCHED ROOFS

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ARCHITECT AND THE MANUFACTURING CONTRACTOR. READING WARNING:

Bonding shown on this drawing is not section bonding, wall bonding, post bonding, or similar bonding which is a part of the building design and which must be considered by the building designer. Bonding shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 for details.)

For more information, contact Building Designer, Truss Plus Inc., 171 E. 1st St., Madison, Wisconsin 53701.

Component Engineering by Truss Engineering Co., P.A., 818 Soundside Rd., Edinburg, NC 27629

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
357 Medallion Pl. Chuluota, FL 32766

Eng. Job:

Dwa:

Dscmr: TLY Chk:

TC Live	16.0	psf	Libr DF: 1.25
---------	------	-----	---------------

TTC Dead	7.0	psi
Plt DF:	1.25	

BC Live	10-0	psf	O.C.: 2-0-0
---------	------	-----	-------------

BC	Dead	10	0	Def	TPI-02/FBC-04
----	------	----	---	-----	---------------

NO DATA	10.0	per
TOTAL:	43.0	msf
Code: FLA	7	32-0
RA	7	32-0

ВО: НТ. ПРЕТА. И.

TT: НТР

11/2/2006

Libr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC

Code: FLA
7 32-0

Trenco

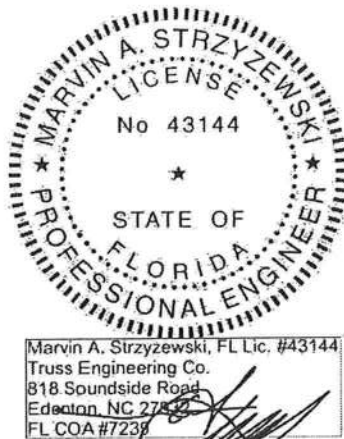
818 Soundside Rd
Edenton, NC 27932

Re: COVINGTON
COVINGTON_FLORIDA_125

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Pages or sheets covered by this seal: E4637930 thru E4637984

My license renewal date for the state of Florida is February 28, 2009.



January 28, 2008

Strzyzewski, Marvin

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.
Engineering services provided by Truss Engineering Company.

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E463796B
COVINGTON	S1	SPECIAL	9	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08 29 2008 Page 1

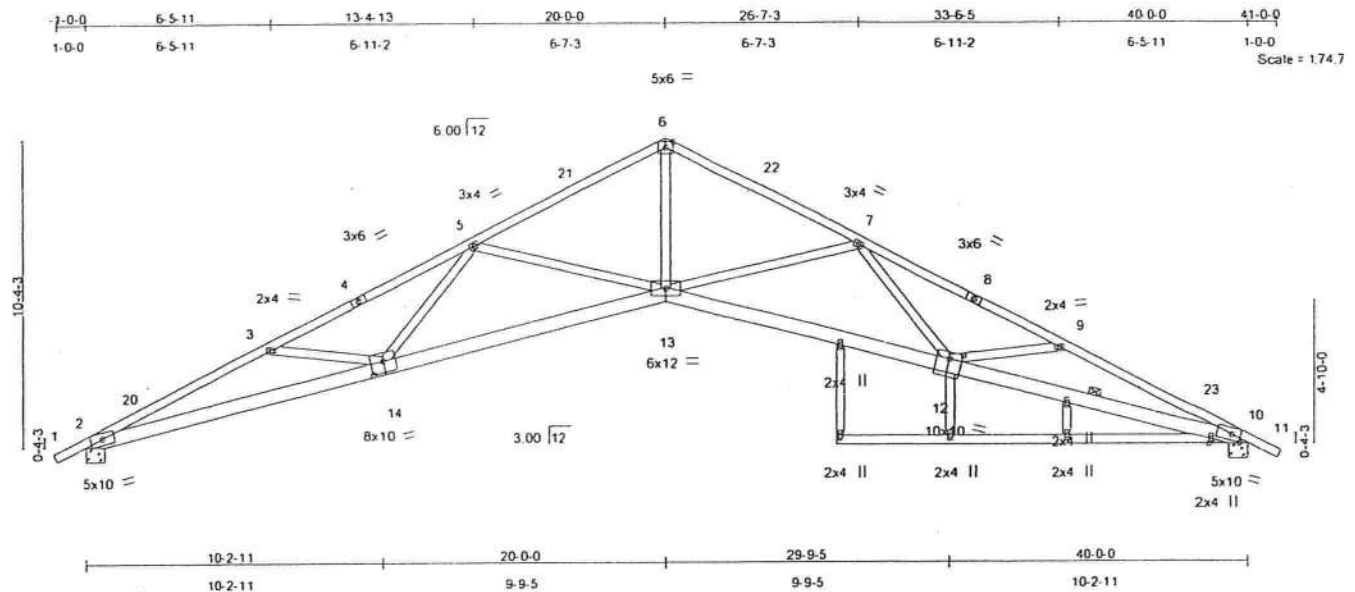


Plate Offsets (X,Y): [10-0-3-12-0-2-8], [12-0-5-0-0-2-12], [14-0-5-0-0-4-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.74	Vert(LL)	-0.58 13-14	>817	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.96	Vert(TL)	-1.16 13-14	>409	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.72	Horz(TL)	0.77 10	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 250 lb

LUMBER

TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.2 *Except*
10-15 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-1-15 oc bracing. Except:
6-2-0 oc bracing: 10-12

REACTIONS

(lb/size) 2=1756/0-8-0, 10=1756/0-8-0
Max Horz 2=182(LC 5)
Max Uplift 2=616(LC 6), 10=616(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-20=5598/1896, 3-20=5555/1917, 3-4=5309/1602, 4-5=5239/1617, 5-21=3793/1196, 6-21=3719/1218,
6-22=3719/1218, 7-22=3793/1196, 7-8=5238/1617, 8-9=5309/1602, 9-23=5555/1917, 10-23=5598/1896,
10-11=0/22
BOT CHORD 2-14=1599/5103, 13-14=1165/4424, 12-13=1165/4424, 10-12=1599/5103
WEBS 3-14=245/459, 5-14=89/769, 5-13=980/520, 6-13=736/3033, 7-13=980/520, 7-12=93/769, 9-12=245/464

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) - 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 17-0-0, Exterior(2) 17-0-0 to 20-0-0, Interior(1) 23-0-0 to 38-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 616 lb uplift at joint 2 and 616 lb uplift at joint 10.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637969
COVINGTON	StA	SPECIAL	9	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:30 2008 Page 1			

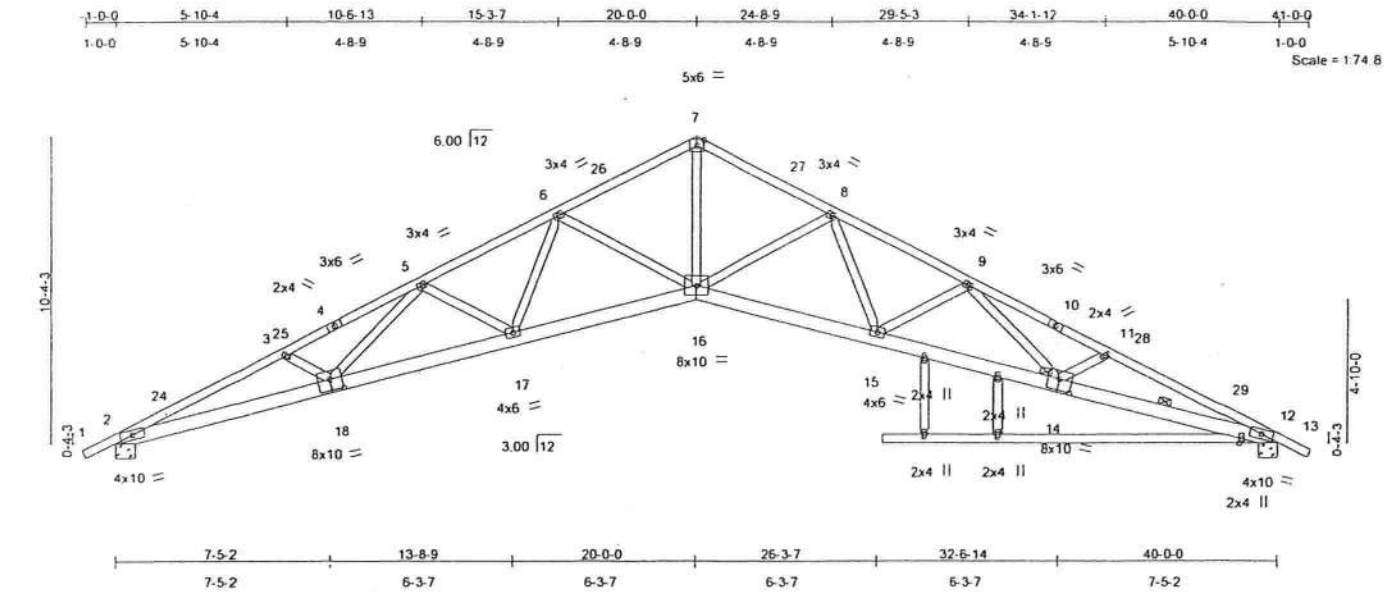


Plate Offsets (X,Y): [14'-0"-5'-0"-4'-8"], [18'-0"-5'-0"-4'-8"]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d		PLATES	GRIP
TCLL	16.0	Plates Increase 1.25		TC	0.59	Vert(LL)	-0.59 16 >798 240	MT20	244/190
TCDL	7.0	Lumber Increase 1.25		BC	0.92	Vert(TL)	-1.17 16-17 >402 180		
BCLL	10.0	Rep Stress Incr YES		WB	0.56	Horz(TL)	0.79 12 n/a n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)				Weight: 257 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2 *Except*
2-18 2 X 6 SYP No.1D, 12-14 2 X 6 SYP No.1D, 12-19 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-9-1 oc bracing. Except:
6-9-0 oc bracing: 12-14
6-10-0 oc bracing: 14-15
JOINTS 1 Brace at Jt(s): 14

REACTIONS (lb/size) 2=1756/0-8-0, 12=1756/0-8-0
Max Horz 2=182(LC 5)
Max Uplift 2=613(LC 6), 12=613(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-24=5608/1807, 3-24=5544/1827, 3-25=5476/1728, 4-25=5466/1735, 4-5=5432/1746, 5-6=4765/1561,
6-26=3705/1212, 7-26=3618/1223, 7-27=3618/1223, 8-27=3705/1212, 8-9=4765/1561, 9-10=5432/1746,
10-28=5466/1735, 11-28=5476/1728, 11-29=5544/1827, 12-29=5608/1807, 12-13=0/22
BOT CHORD 2-18=1512/5059, 17-18=1304/4763, 16-17=993/4122, 15-16=993/4122, 14-15=1304/4763, 12-14=1512/5059
WEBS 3-18=56/244, 5-18=147/465, 5-17=447/278, 6-17=178/725, 6-16=825/413, 7-16=812/3038, 8-16=825/413,
8-15=178/725, 9-15=447/278, 9-14=154/465, 11-14=56/247

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 17-0-0, Exterior(2) 17-0-0 to 20-0-0, Interior(1) 23-0-0 to 38-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcord with any other live loads.
 - * This truss has been designed for the live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 12.

LOAD CASE(S) Standard



January 28, 2008

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA, 125	E4637970
COVINGTON	S1B	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:31 2008 Page 1			

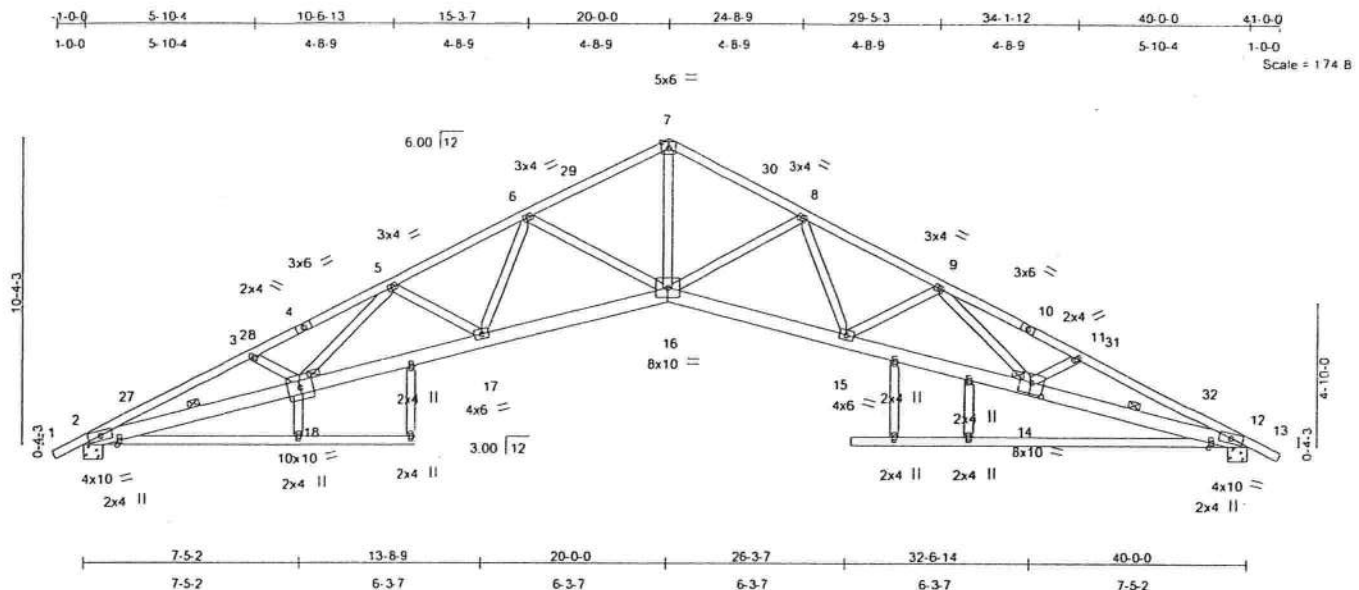


Plate Offsets (X,Y): [14-0-5-0-0-4-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.59	Vert(LL)	-0.59 16	>797	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.92	Vert(TL)	-1.18 16-17	>401	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.56	Horz(TL)	0.79 12	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 279 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 16-18 2 X 6 SYP No.2, 14-16 2 X 6 SYP No.2, 2-18 2 X 6 SYP No.1D
 12-14 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-12 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-11-5 oc bracing. Except:
 6-9-0 oc bracing: 2-18, 12-14
 6-10-0 oc bracing: 17-18, 14-15
 JOINTS 1 Brace at Jt(s): 18, 14

REACTIONS

(lb/size) 2=1756/0-8-0, 12=1756/0-8-0
 Max Horz 2=181(LC 5)
 Max Uplift 2=613(LC 6), 12=613(LC 7)

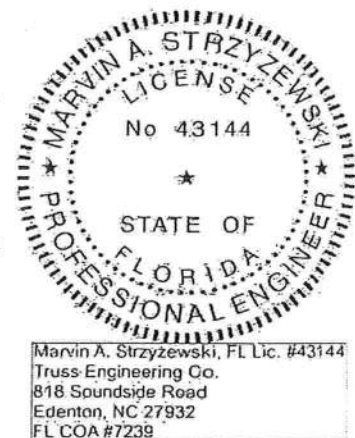
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-27=5614/1809, 3-27=5550/1828, 3-28=5482/1729, 4-28=5472/1736, 4-5=5437/1747, 5-6=4768/1561,
 6-29=3706/1212, 7-29=3619/1224, 7-30=3619/1224, 8-30=3706/1212, 8-9=4768/1561, 9-10=5437/1747,
 10-31=5472/1736, 11-31=5482/1729, 11-32=5550/1828, 12-32=5614/1809, 12-13=0/22
 BOT CHORD 2-18=1514/5065, 17-18=1305/4766, 16-17=993/4124, 15-16=993/4124, 14-15=1305/4766, 12-14=1514/5065
 WEBS 3-18=57/244, 5-18=147/467, 5-17=448/279, 6-17=179/726, 6-16=826/413, 7-16=812/3040, 8-16=826/413,
 8-15=179/726, 9-15=448/279, 9-14=154/467, 11-14=57/248

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-10-13 to 1-11-3, Interior(1) 1-11-3 to 17-0-0, Exterior(2) 17-0-0 to 20-0-0, Interior(1) 23-0-0 to 38-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 12.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637935
COVINGTON	H1A	HIP	1	1	Job Reference (optional)	
Marinda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09 08 07 2008 Page 1			

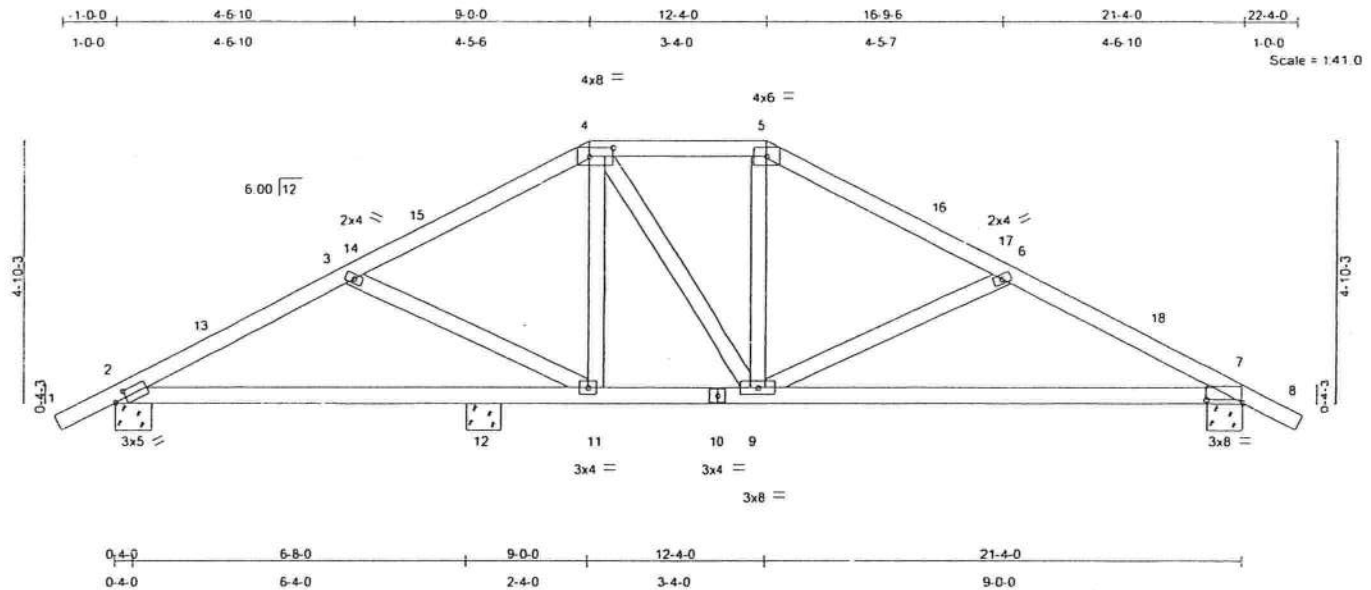


Plate Offsets (X,Y): [2-0-2-10-0-1-8], [4-0-5-4-0-2-0], [7-0-8-4-0-0-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.29	Vert(LL)	0.12	2-12	>669	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.57	Vert(TL)	-0.32	7-9	>531		
BCLL 10.0	Rep Stress Incr	YES	WB 0.10	Horz(TL)	0.03	7	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 105 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-10 oc bracing.

REACTIONS

(lb/size) 2=704/0-8-0, 7=835/0-8-0, 12=367/0-8-0
Max Horz 2=80(LC 7)
Max Uplift 2=502(LC 6), 7=403(LC 7), 12=130(LC 6)

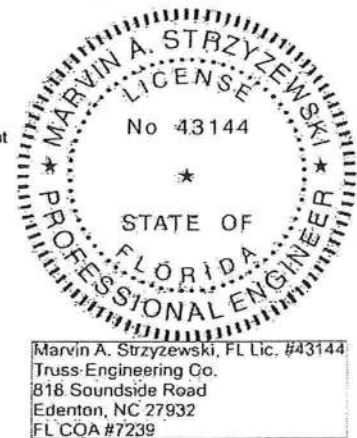
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-13=-977/663, 3-13=-951/674, 3-14=-770/471, 14-15=-762/472, 4-15=-721/488, 4-5=-811/527,
5-16=-899/512, 16-17=-945/501, 6-17=-952/495, 6-18=-1157/652, 7-18=-1184/637, 7-8=0/21
BOT CHORD 2-12=-483/834, 11-12=-483/834, 10-11=-178/641, 9-10=-178/641, 7-9=-439/1030
WEBS 3-11=-215/336, 4-11=-141/35, 4-9=-50/365, 5-9=-17/246, 6-9=-253/267

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 4-9-1, Exterior(2) 4-9-1 to 16-6-15, Interior(1) 16-6-15 to 19-4-13 zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 2, 403 lb uplift at joint 7 and 130 lb uplift at joint 12.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

TRENCO
ENGINEERING BY
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA_125	E4637938
COVINGTON	HS1	SPECIAL	2	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, Florida						7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:10 2008 Page 1

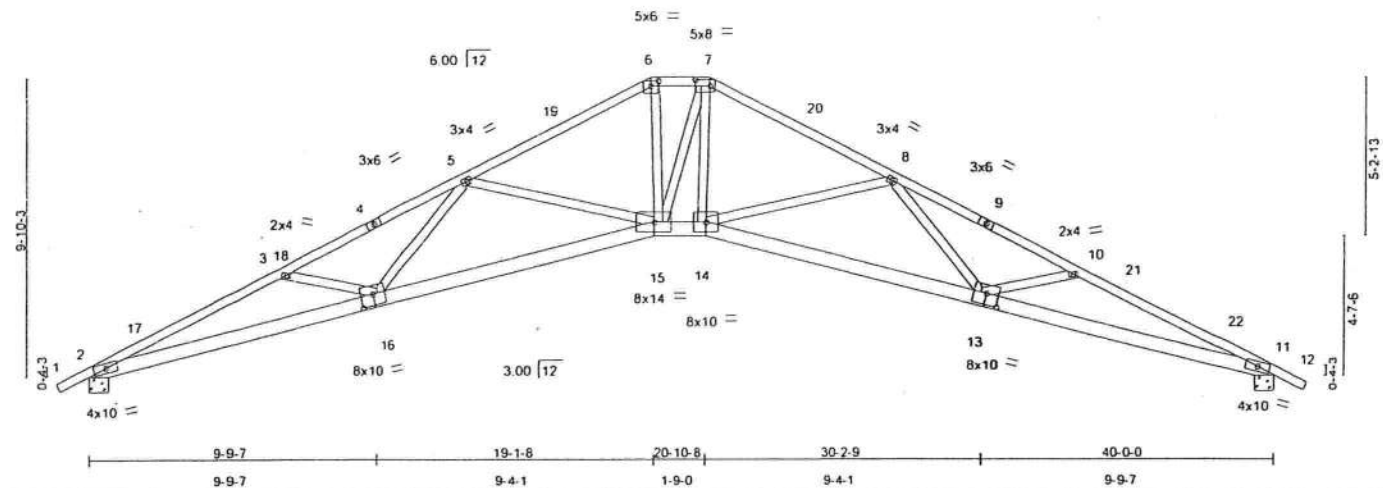
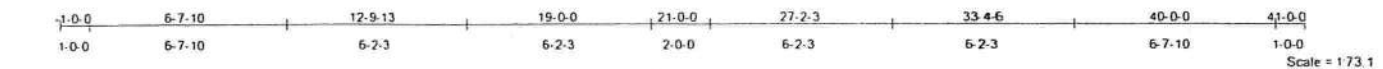


Plate Offsets (X,Y): [6:0-3-0-0-2-0], [7:0-6-0-0-2-8], [13:0-5-0-0-4-8], [16:0-5-0-0-4-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.73	Ver(LL)	-0.59 13-14	>800	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.73	Ver(TL)	-1.18 13-14	>400	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.66	Horz(TL)	0.74 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 230 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-6-7 oc bracing.

REACTIONS

(lb/size) 2=1756/0-8-0, 11=1756/0-8-0
Max Horz 2=172(LC 4)
Max Uplift 2=613(LC 6), 11=613(LC 7)

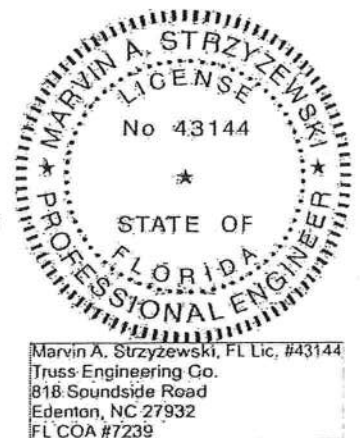
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-17=5606/1909, 17-18=5542/1929, 3-18=5475/1931, 3-4=5352/1693, 4-5=5285/1706, 5-19=3922/1310,
6-19=3855/1322, 6-7=3486/1272, 7-20=3784/1303, 8-20=3853/1292, 8-9=5296/1707, 9-10=5363/1694,
10-21=5483/1932, 21-22=5550/1930, 11-22=5614/1910, 11-12=0/22
BOT CHORD 2-16=1606/5092, 15-16=1252/4488, 14-15=723/3467, 13-14=1245/4463, 11-13=1607/5099
WEBS 3-16=190/381, 5-16=98/740, 5-15=910/497, 8-14=950/509, 8-13=108/781, 10-13=187/385, 6-15=377/1538,
7-14=359/1484, 7-15=149/231

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 16-0-0, Exterior(2) 16-0-0 to 24-0-0, Interior(1) 24-0-0 to 38-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 11.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSI-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA 125	E 463794.0
COVINGTON	HS2	SPECIAL	2	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09 08 12 2008 Page 1			

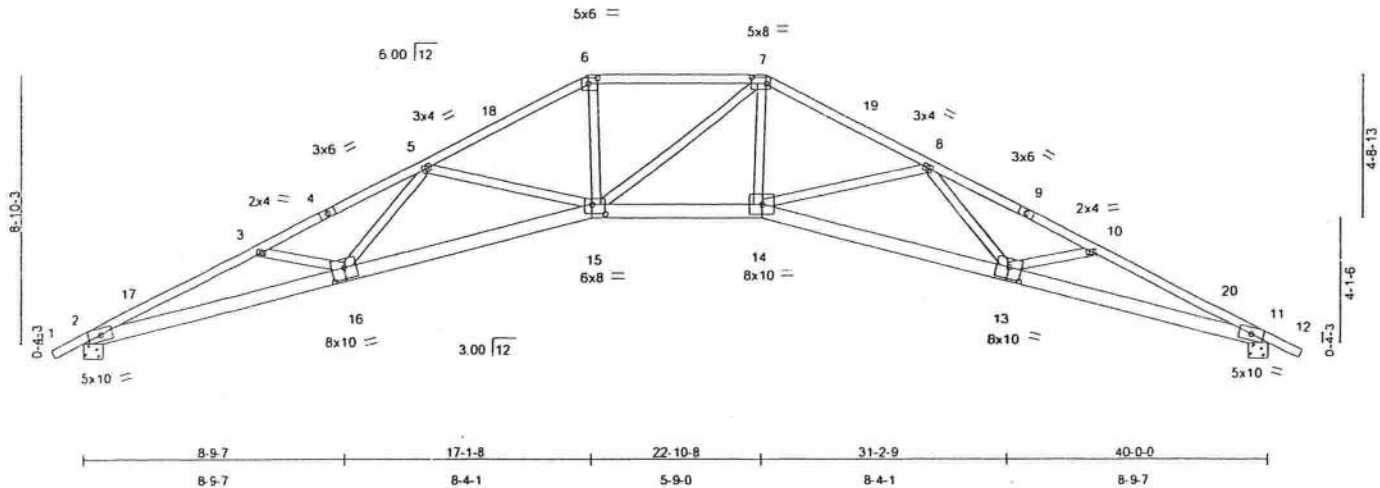
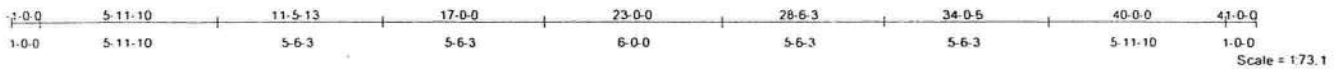


Plate Offsets (X,Y): [6.0-3.8.0-2.4], [7.0-6.0.0-2.8], [13.0-5.0.0.4-8], [15.0-5.4.0.3-12], [16.0-5.0.0.4-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.62	Ver(LL)	-0.59 13-14	>804	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.92	Ver(TL)	-1.17 13-14	>403	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.45	Horz(TL)	0.78 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 226 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-3-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS

(lb/size) 2=1756/0-8-0, 11=1756/0-8-0
Max Horz 2=-153(LC 4)
Max Uplift 2=609(LC 6), 11=609(LC 7)

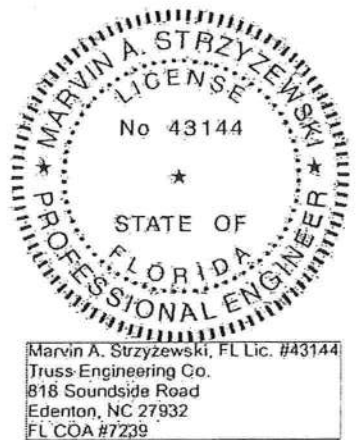
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-17=5626/2017, 3-17=5567/2036, 3-4=-5444/1842, 4-5=5390/1854, 5-18=4243/1542, 6-18=4187/1553,
6-7=-3789/1476, 7-19=-4131/1538, 8-19=-4187/1527, 8-9=-5392/1853, 9-10=5447/1841, 10-20=5569/2036,
11-20=5628/2016, 11-12=0/22
BOT CHORD 2-16=1705/5099, 15-16=1431/4653, 14-15=980/3795, 13-14=1424/4629, 11-13=1705/5101
WEBS 3-16=111/335, 5-16=76/608, 5-15=769/420, 7-15=209/195, 8-14=798/428, 8-13=84/641, 10-13=110/340,
6-15=385/1611, 7-14=378/1593

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-13 to 1-11-3, Interior(1) 1-11-3 to 14-0-0, Exterior(2) 14-0-0 to 26-0-0, Interior(1) 26-0-0 to 38-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 609 lb uplift at joint 2 and 609 lb uplift at joint 11.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss-Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information - available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E 4637942
COVINGTON	HS3	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7 030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09 08 14 2008 Page 1			

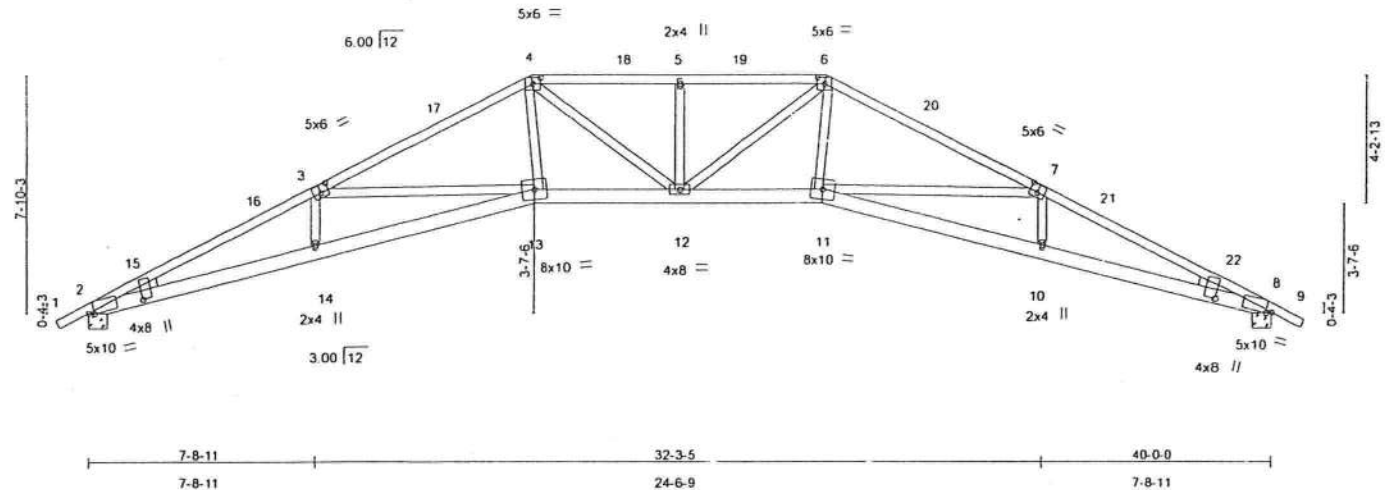


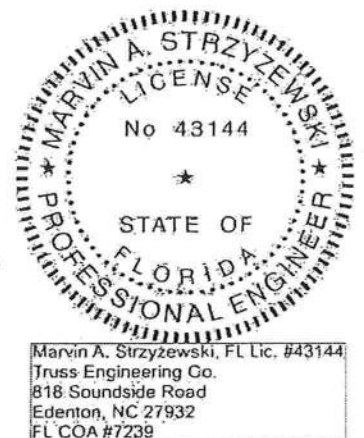
Plate Offsets (X,Y): [2-0-2-7,Edge], [2-0-0-3,1-11-5], [3-0-3-0-0-3-0], [4-0-3-0-0-2-7], [6-0-3-0-0-2-7], [7-0-3-0-0-3-0], [8-0-2-7,Edge], [8-0-0-3,1-11-5]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl
TCLL 16.0	Plates Increase	1.25	TC 0.81	Vert(LL)	-0.60	12 >781
TCDL 7.0	Lumber Increase	1.25	BC 0.94	Vert(TL)	-1.20	12 >394
BCLL 10.0	Rep Stress Incr	YES	WB 0.84	Horz(TL)	0.80	8 n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)			
						PLATES GRIP
						MT20 244/190
						Weight: 228 lb

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	2 X 6 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	2 X 4 SYP No.2		
WEDGE			
Left: 2 X 4 SYP No.2, Right: 2 X 4 SYP No.2			
REACTIONS (lb/size) 2=1756/0-8-0, 8=1756/0-8-0			
Max Horz 2=-135(LC 4)			
Max Uplift 2=-613(LC 6), 8=-613(LC 7)			

FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/22, 2-15=-5729/1992, 15-16=-5651/2003, 3-16=-5607/2018, 3-17=-4626/1684, 4-17=-4538/1697, 4-18=-4298/1678, 5-18=-4298/1678, 5-19=-4298/1678, 6-19=-4298/1678, 6-20=-4538/1697, 7-20=-4627/1684, 7-21=-5607/2018, 21-22=-5651/2003, 8-22=-5729/1992, 8-9=0/22
BOT CHORD	2-14=-1675/5184, 13-14=-1679/5194, 12-13=-1172/4110, 11-12=-1172/4110, 10-11=-1679/5194, 8-10=-1675/5184
WEBS	3-14=0/294, 3-13=-956/497, 7-11=-956/504, 7-10=0/294, 6-11=-358/1577, 6-12=-270/421, 5-12=-200/247, 4-12=-270/421, 4-13=-358/1577

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 12-0-7, Exterior(2) 12-0-7 to 15-0-7, Interior(1) 18-0-7 to 21-11-9, Exterior(2) 24-11-9 to 41-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 8.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

Marionda Homes Inc. Sanford, Florida

7:030 s Jan 3 2008 MiTeC Industries, Inc. Mon Jan 28 09:08:16 2008 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.70	Vert(LL)	-0.64 12	>742	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.91	Vert(TL)	-1.26 12-13	>374	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.70	Horz(TL)	0.81 8	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)					Weight: 221 lb	

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-11-7 oc bracing.

TOP CHORD 1-2=0/22, 2-15=5715/2027, 15-16=5645/2037, 3-16=5593/2049, 3-17=4940/1820, 4-17=4872/1831,
4-18=4417/1738, 5-18=4447/1738, 5-19=4417/1738, 6-19=4417/1738, 6-20=4872/1831, 7-20=4941/1819,
7-21=5593/2049, 21-22=5645/2037, 8-22=5715/2027, 8-9=0/22

BOT CHORD 2-14=1709/5161, 13-14=1715/5179, 12-13=1552/5013, 11-12=1552/5013, 10-11=1715/5179, 8-10=1709/5161

WEBS 3-14=0/227, 3-13=643/405, 5-13=835/452, 5-12=0/269, 5-11=835/451, 7-11=643/413, 7-10=0/227, 4-13=465/1865,
6-11=465/1865

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDF=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-13 to 1-11-3, Interior(1) 1-11-3 to 10-0-7, Exterior(2) 10-0-7 to 13-0-7, Interior(1) 16-0-7 to 23-11-9, Exterior(2) 26-11-9 to 41-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 8.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

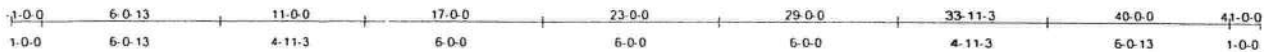
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MUTEK REFERENCE PAGE MU-7473 BEFORE USE.

Design valid for use only with Mutek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information**, available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Hiltex Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA, 125	E4637946
COVINGTON	HSS	SPECIAL	2	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, Florida			7,030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08 18 2008 Page 1			



Scale = 1/4\"/>

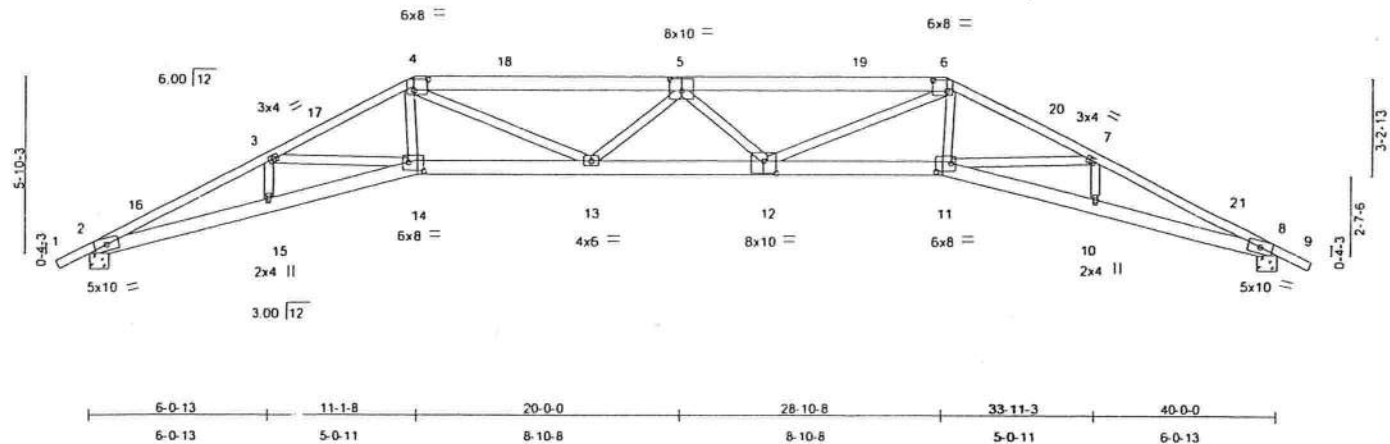


Plate Offsets (X,Y): [4-0-5-8-0-4-4], [5-0-5-0-0-4-8], [6-0-5-8-0-4-4], [11-0-6-0-0-3-4], [12-0-5-0-0-4-12], [14-0-6-0-0-3-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.69	Vert(LL)	-0.66 12-13	>710	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.90	Vert(TL)	-1.32 12-13	>357	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.37	Horz(TL)	0.78 8	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 233 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
4-5 2 X 6 SYP No.2, 5-6 2 X 6 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-5-8 oc bracing.

REACTIONS

(lb/size) 2=1756/0-8-0, 8=1756/0-8-0
Max Horz 2=96(LC 5)
Max Uplift 2=614(LC 6), 8=614(LC 7)

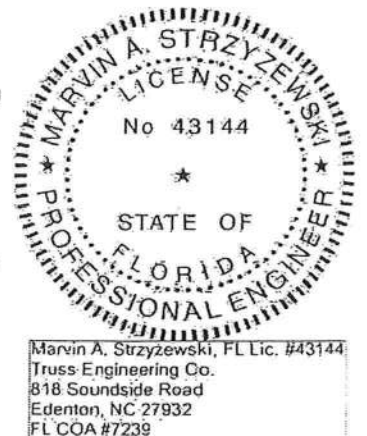
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-16=-5658/2036, 3-16=-5590/2057, 3-17=-5268/1978, 4-17=-5178/1990, 4-18=-5758/2181, 5-18=-5759/2181, 5-19=-5759/2181, 6-19=-5758/2181, 6-20=-5178/1990, 7-20=-5268/1978, 7-21=-5590/2057, 8-21=-5658/2036, 8-9=0/22
BOT CHORD 2-15=-1716/5096, 14-15=-1724/5119, 13-14=-1526/4758, 12-13=-2108/6019, 11-12=-1523/4758, 10-11=-1724/5119, 8-10=-1716/5096
WEBS 3-15=0/163, 3-14=-262/272, 4-13=-533/1224, 5-13=-431/449, 5-12=-431/450, 6-12=-533/1224, 7-11=-262/281, 7-10=0/163, 6-11=-390/1536, 4-14=-390/1536

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 8-0-0, Exterior(2) 8-0-0 to 11-0-0, Interior(1) 14-0-0 to 26-0-0, Exterior(2) 29-0-0 to 41-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 614 lb uplift at joint 2 and 614 lb uplift at joint 8.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and IBCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncofria Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637948
COVINGTON	HS6	SPECIAL	2	1	Job Reference (optional)	

Maranda Homes Inc., Sanford, Florida

7 030 5 Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:19 2008 Page 1



Scale = 1/2\"/>

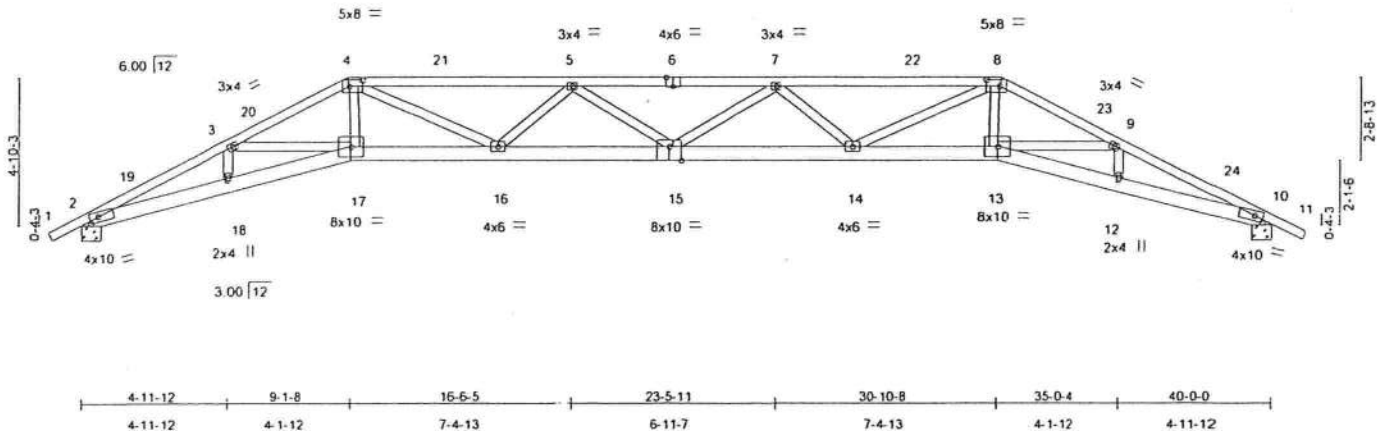


Plate Offsets (X,Y): [2-0-3-8-0-2-0] [4-0-5-4-0-2-8] [6-0-3-0-Edge] [8-0-5-4-0-2-8] [10-0-3-8-0-2-0] [15-0-5-0-Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.86	Vert(LL)	-0.83	15	>566	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.91	Vert(TL)	-1.66	15	>285	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.29	Horz(TL)	0.81	10	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 219 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
4-6 2 X 4 SYP No.1D, 6-8 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.2 *Except*
15-17 2 X 6 SYP No.1D, 13-15 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-2-7 oc bracing.

REACTIONS

(lb/size) 2=1756/0-8-0, 10=1756/0-8-0
Max Horz 2=80(LC 6)
Max Uplift 2=614(LC 6), 10=614(LC 7)

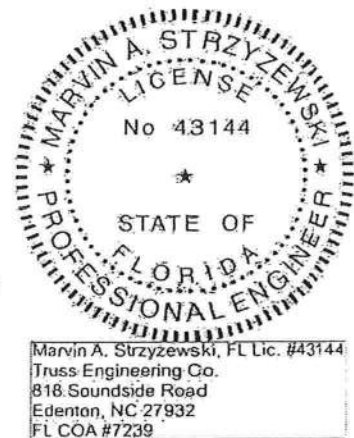
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-19=5512/2012, 3-19=5451/2024, 3-20=5373/2050, 4-20=5307/2062, 4-21=6285/2376, 5-21=6285/2376, 5-6=6984/2629, 6-7=6984/2629, 7-22=6285/2376, 8-22=6285/2376, 8-23=5307/2062, 9-23=5373/2050, 9-24=5451/2024, 10-24=5512/2012, 10-11=0/22
BOT CHORD 2-18=1695/4940, 17-18=1702/4966, 16-17=1771/4945, 15-16=2607/6785, 14-15=2540/6785, 13-14=1647/4945, 12-13=1699/4966, 10-12=1686/4940
WEBS 3-18=0/114, 3-17=256/196, 4-16=662/1564, 5-16=733/538, 5-15=0/291, 7-15=0/291, 7-14=733/538, 8-14=662/1564, 9-13=265/196, 9-12=0/114, 4-17=396/1456, 8-13=396/1456

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 6-0-7, Exterior(2) 6-0-7 to 9-0-7, Interior(1) 12-0-7 to 27-11-9, Exterior(2) 30-11-9 to 41-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 614 lb uplift at joint 2 and 614 lb uplift at joint 10.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

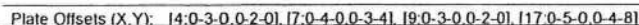
January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Maronda Homes, Inc. Sanford, Florida

7 030 s Jan 3 2008 MiTek Industries, Inc. Mon Jan 28 09:08:22 2008 Page 1



Weight: 664 lb

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-3-11 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 8-0-13 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension

NOTES

- 1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDF=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

MARVIN A. STRZYZEWSKI
LICENSE
No 43144
★
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.
Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult AWS/TP1 Quality Criteria, D58-89 and BC311 Building Component Safety Information, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliat

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA 125	E 463795.0
COVINGTON	HSGRD	SPECIAL	1	3	Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

7 030 s Jan 3 2008 MiTek Industries, Inc. Mon Jan 28 09:08:22 2008 Page 7

NOTES

- 8) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2501 lb uplift at joint 2 and 2501 lb uplift at joint 11.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 114 lb down and 248 lb up at 7-0-0, 114 lb down and 248 lb up at 9-0-12, 114 lb down and 248 lb up at 11-0-12, 114 lb down and 248 lb up at 13-0-12, 114 lb down and 248 lb up at 15-0-12, 114 lb down and 248 lb up at 17-0-12, 114 lb down and 248 lb up at 19-0-12, 114 lb down and 248 lb up at 20-11-4, 114 lb down and 248 lb up at 22-11-4, 114 lb down and 248 lb up at 24-11-4, 114 lb down and 248 lb up at 26-11-4, 114 lb down and 248 lb up at 28-11-4, and 114 lb down and 248 lb up at 30-11-4, and 114 lb down and 248 lb up at 33-0-0 on top chord, and 638 lb down and 461 lb up at 7-2-11, 92 lb down at 9-0-12, 92 lb down at 11-0-12, 92 lb down at 13-0-12, 92 lb down at 15-0-12, 92 lb down at 17-0-12, 92 lb down at 19-0-12, 92 lb down at 20-11-4, 92 lb down at 22-11-4, 92 lb down at 24-11-4, 92 lb down at 26-11-4, 92 lb down at 28-11-4, and 92 lb down at 30-11-4, and 638 lb down and 461 lb up at 32-95 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-21=-23, 4-21=-46(F=-23), 4-9=-23, 9-36=-46(F=-23), 12-36=-23, 2-19=-40(F=-20), 14-19=-20, 11-14=-40(F=-20)

Concentrated Loads (lb)

Vert: 4=-114(F) 9=-114(F) 19=638(F) 14=638(F) 23=-114(F) 24=-114(F) 25=-114(F) 26=-114(F) 27=-114(F) 28=-114(F) 29=-114(F) 30=-114(F) 31=-114(F) 32=-114(F) 33=-114(F) 34=-114(F) 37=92(F) 38=92(F) 39=92(F) 40=92(F) 41=92(F) 42=92(F) 43=92(F) 44=92(F) 45=92(F) 46=92(F) 47=92(F) 48=92(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637939
COVINGTON	HS1A	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7 030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:11 2008 Page 1			

1-0-0 5-1-1 9-8-11 14-4-6 19-0-0 21-0-0 25-7-10 30-3-5 34-10-15 40-0-0 47-0-0
1-0-0 5-1-1 4-7-10 4-7-10 4-7-10 2-0-0 4-7-10 4-7-10 4-7-10 5-1-1 1-0-0

Scale = 181.5

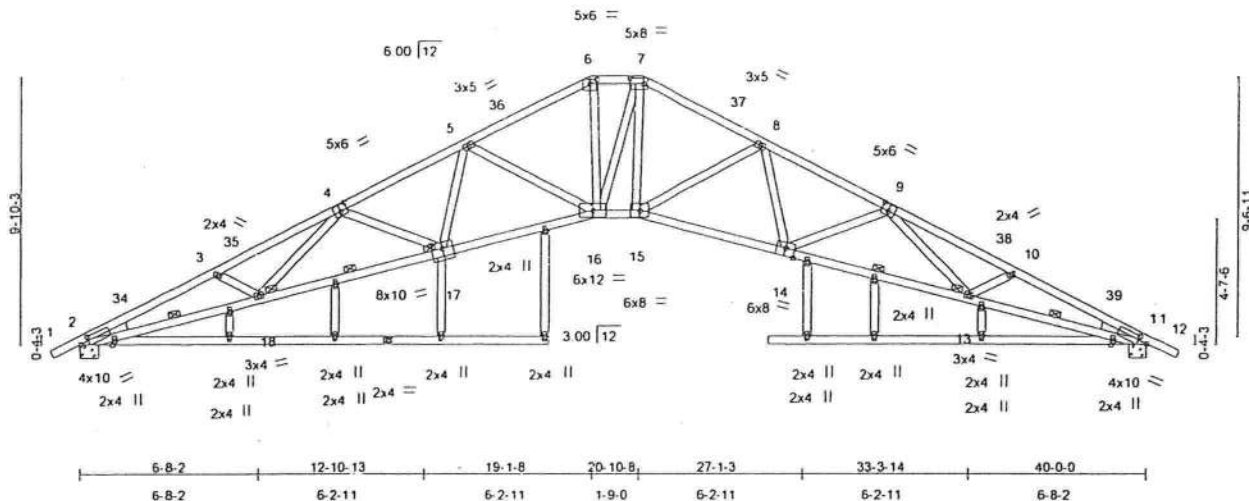


Plate Offsets (X,Y): [2:0-4-1-0-1-12], [4:0-3-0-0-3-0], [6:0-3-0-0-2-0], [7:0-6-0-0-2-8], [9:0-3-0-0-3-0], [11:0-4-1-0-1-12], [14:0-4-0-Edge], [17:0-5-0-0-2-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.41	Vert(LL)	-0.59 14-15	>803	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.83	Vert(TL)	-1.17 14-15	>403	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.39	Horz(TL)	0.86 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 282 lb

LUMBER

TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.1D "Except"
19-20 2 X 4 SYP No.2, 11-27 2 X 4 SYP No.2, 2-19 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-5-9 oc bracing. Except:
5-5-0 oc bracing: 2-18, 11-13
5-8-0 oc bracing: 17-18, 13-14
6-5-0 oc bracing: 16-17
JOINTS 1 Brace at JI(s): 17, 18, 13

REACTIONS (lb/size) 2=1756/0-8-0, 11=1756/0-8-0
Max Horz 2=175(LC 5)
Max Uplift 2=613(LC 6), 11=613(LC 7)

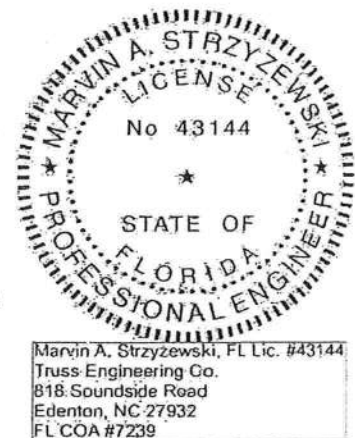
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-34=5427/1806, 3-34=5384/1823, 3-35=5379/1754, 4-35=5347/1768, 4-5=4692/1596, 5-36=3788/1313,
6-36=3742/1324, 6-7=3383/1258, 7-37=3679/1307, 8-37=3729/1296, 8-9=4696/1597, 9-38=5345/1768,
10-38=5377/1754, 10-39=5383/1823, 11-39=5426/1806, 11-12=0/18
BOT CHORD 2-18=1508/4871, 17-18=1386/4732, 16-17=1092/4182, 15-16=715/3371, 14-15=1089/4171, 13-14=1386/4733,
11-13=1508/4870
WEBS 3-18=0/205, 4-18=104/382, 4-17=459/293, 5-17=116/600, 5-16=802/391, 7-16=144/196, 8-15=850/405,
8-14=122/633, 9-14=455/295, 9-13=110/380, 10-13=0/210, 6-16=409/1508, 7-15=400/1491

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 16-0-0, Exterior(2) 16-0-0 to 24-0-0, Interior(1) 24-0-0 to 38-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 11.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

TRENCO
ENGINEERING BY
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E463794.1
COVINGTON	HS2A	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						7 030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08 13 2008 Page 1

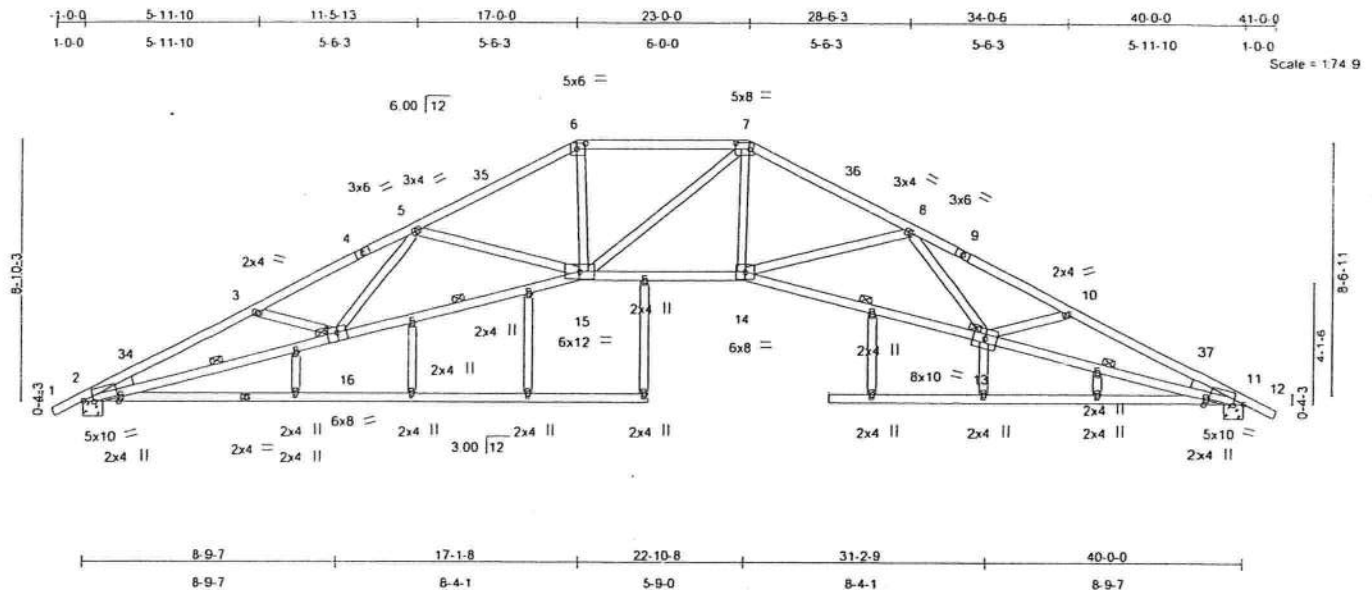


Plate Offsets (X,Y): [2-0-4-3,Edge], [6-0-3-8-0-2-4], [7-0-6-0-0-2-8], [11-0-4-3,Edge], [16-0-4-0-0-2-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.57	Vert(LL)	-0.63 13-14	>749	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.94	Vert(TL)	-1.28 13-14	>368	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.44	Horz(TL)	0.86 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 268 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D
 BOT CHORD 2 X 4 SYP No.1D "Except"
 17-18 2 X 4 SYP No.2, 24-25 2 X 4 SYP No.2, 25-26 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2
 WEDGE
 Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-7 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-9-10 oc bracing. Except:
 5-2-0 oc bracing: 2-16, 11-13
 5-7-0 oc bracing: 15-16
 5-8-0 oc bracing: 13-14
 1 Brace at Jt(s): 16, 13

JOINTS

REACTIONS (lb/size) 2=1756/0-8-0, 11=1756/0-8-0
 Max Horz2=156(LC 5)
 Max Uplift2=609(LC 6), 11=609(LC 7)

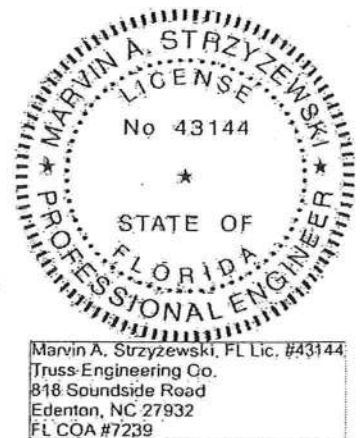
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-34=5447/1956, 3-34=5409/1976, 3-4=5317/1818, 4-5=5237/1830, 5-35=4165/1526, 6-35=4111/1538,
 6-7=3719/1463, 7-36=4057/1522, 8-36=4112/1511, 8-9=5242/1830, 9-10=5323/1819, 10-37=5412/1975,
 11-37=5451/1955, 11-12=0/18
 BOT CHORD 2-16=1646/4925, 15-16=1411/4550, 14-15=969/3723, 13-14=1405/4527, 11-13=1645/4928
 WEBS 3-16=60/307, 5-16=78/588, 5-15=744/413, 7-15=205/194, 8-14=772/421, 8-13=88/622, 10-13=58/311,
 6-15=377/1572, 7-14=369/1557

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 14-0-0, Exterior(2) 14-0-0 to 26-0-0, Interior(1) 26-0-0 to 38-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 609 lb uplift at joint 2 and 609 lb uplift at joint 11.

LOAD CASE(S) Standard



January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Oly	Ply	COVINGTON_FLORIDA_125	E4637943
COVINGTON	HS3A	SPECIAL	2	1	Job Reference (optional)	
Meronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09 08:15 2008 Page 1			

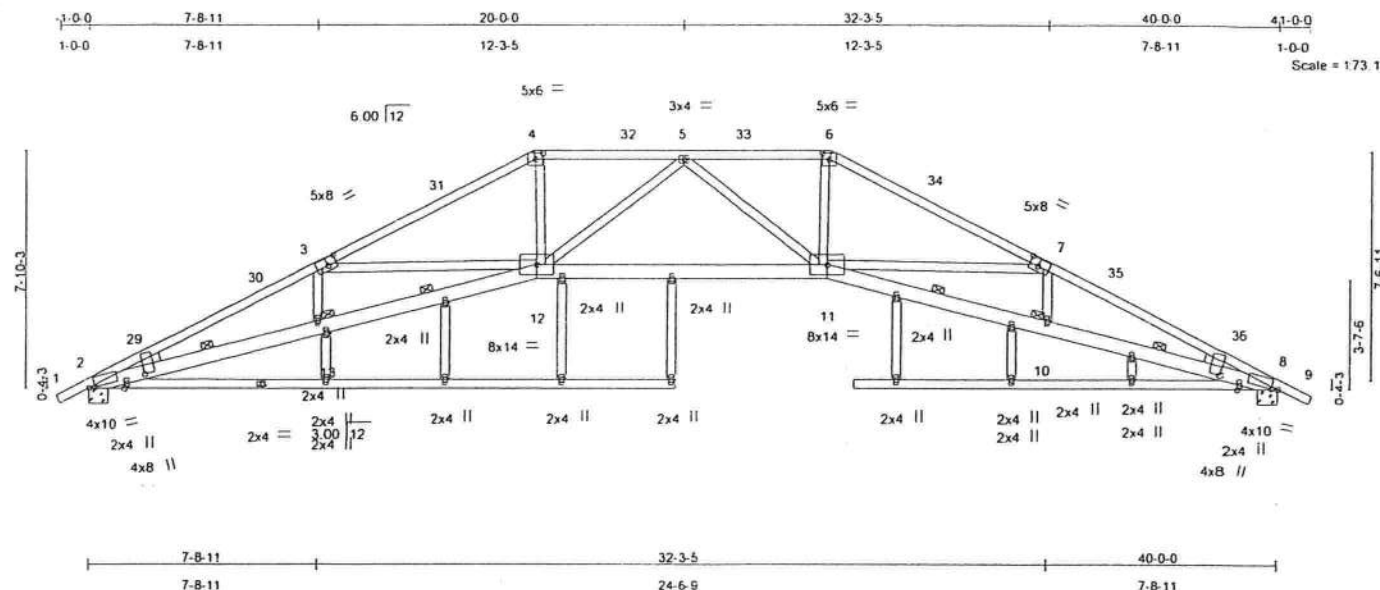


Plate Offsets (X,Y): [2-0-2-7,0-0-6], [2-0-0-3,1-11-5], [3-0-3-8,0-3-0], [4-0-3-0,0-2-7], [6-0-3-0,0-2-7], [7-0-3-8,0-3-0], [8-0-2-7,0-0-6], [8-0-0-3,1-11-5]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.82	Vert(LL)	-0.62 11-12	>766	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.78	Vert(TL)	-1.24 11-12	>380	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.80	Horz(TL)	0.76 8	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 295 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 "Except"
 2-12 2 X 6 SYP No.1D, 11-12 2 X 6 SYP No.2, 8-11 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.2
 WEDGE
 Left: 2 X 4 SYP No.2, Right: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied or 7-1-0 oc bracing. Except:
 6-5-0 oc bracing: 2-13, 12-13, 10-11, 8-10
 JOINTS 1 Brace at Jt(s): 13, 10

REACTIONS (lb/size) 2=1756/0-8-0, 8=1756/0-8-0
 Max Horz2=135(LC 5)
 Max Uplift2=613(LC 6), 8=613(LC 7)

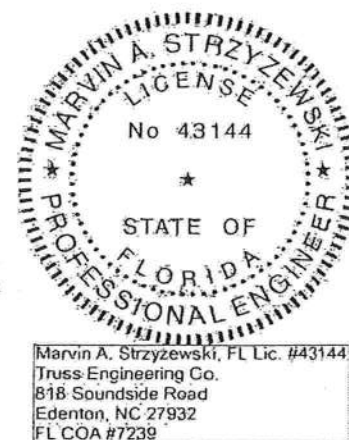
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-29=5707/1996, 29-30=5629/2007, 3-30=5585/2022, 3-31=4654/1668, 4-31=4565/1680,
 4-32=4136/1607, 5-32=4136/1607, 5-33=4136/1607, 6-33=4136/1607, 6-34=4565/1680, 7-34=4654/1668,
 7-35=5585/2022, 35-36=5629/2007, 8-36=5707/1996, 8-9=0/22
 BOT CHORD 2-13=1679/5162, 12-13=1680/5178, 11-12=1255/4246, 10-11=1680/5178, 8-10=1679/5162
 WEBS 3-13=0/268, 3-12=912/513, 5-12=319/316, 5-11=319/315, 7-11=912/520, 7-10=0/268, 4-12=397/1739,
 6-11=397/1739

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 12-0-7, Exterior(2) 12-0-7 to 15-0-7, Interior(1) 18-0-7 to 21-11-9, Exterior(2) 24-11-9 to 41-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 8.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA_125	E4637945
COVINGTON	HS4A	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7:030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:17 2008 Page 1			

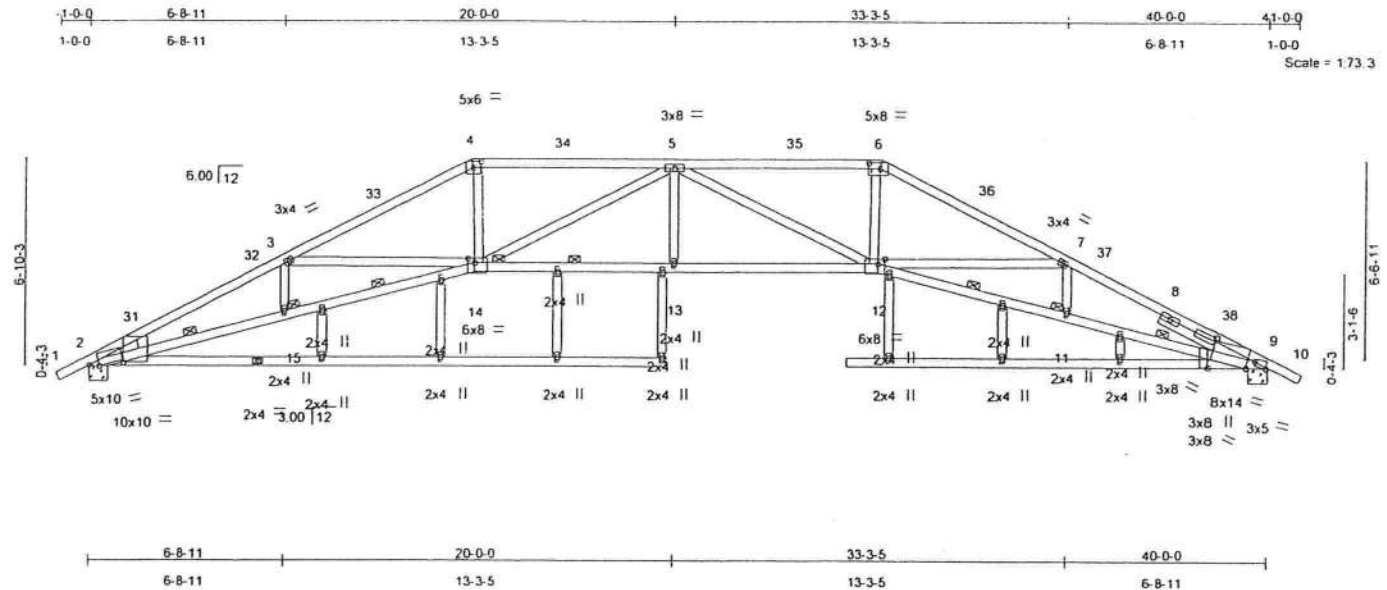


Plate Offsets (X,Y): [2:1-2-5,0-1-1], [2:0-4-3,Edge], [4:0-3-0,0-2-7], [6:0-4-12,0-2-4], [9:0-5-3,Edge], [9:1-5-11,0-1-8], [9:0-0-0,1-2-8], [9:0-2-15,0-3-12], [12:0-2-12,0-2-0], [14:0-2-12,0-1-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.70	Vert(LL)	-0.67 13-14	>709	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.95	Vert(TL)	-1.33 13-14	>355	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.70	Horz(TL)	0.89 9	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 265 lb

LUMBER

TOP CHORD 2 X 4 SYP No.1D *Except*

4-6 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.1D *Except*

9-16 2 X 4 SYP No.2, 2-22 2 X 4 SYP No.2, 22-23 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.2

SLIDER Right 2 X 4 SYP No.2 3-5-3

REACTIONS

(lb/size) 2=1756/0-8-0, 9=1756/0-8-0

Max Horz 2=119(LC 4)

Max Uplift 2=613(LC 6), 9=613(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-31=5590/1999, 31-32=5538/2005, 3-32=5479/2019, 3-33=4830/1790, 4-33=4762/1801, 4-34=4316/1711, 5-34=4316/1711, 5-35=4332/1717, 6-35=4332/1717, 6-36=4765/1802, 7-36=4820/1790, 7-37=5325/1969, 8-37=5379/1956, 8-38=5378/1956, 9-38=5451/1946, 9-10=0/18

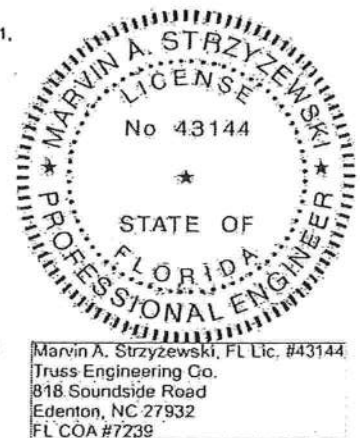
BOT CHORD 2-15=1678/5046, 14-15=1680/5046, 13-14=1522/4897, 12-13=1522/4897, 11-12=1631/4913, 9-11=1625/4892

WEBS 3-15=0/257, 3-14=620/401, 5-14=816/442, 5-13=0/272, 5-12=802/438, 7-12=477/368, 7-11=0/217, 4-14=450/1811, 6-12=436/1776

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 10-0-7, Exterior(2) 10-0-7 to 13-0-7, Interior(1) 16-0-7 to 23-11-9, Exterior(2) 26-11-9 to 41-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 9.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

1-0-0 5-11-12 11-0-0 16-10-6 23-1-10 29-0-0 34-0-4 40-0-0 41-0-0

1-0-0 5-11-12 5-0-4 5-10-6 6-3-5 5-10-6 5-0-4 5-11-12 1-0-0

Scale = 1:72.8

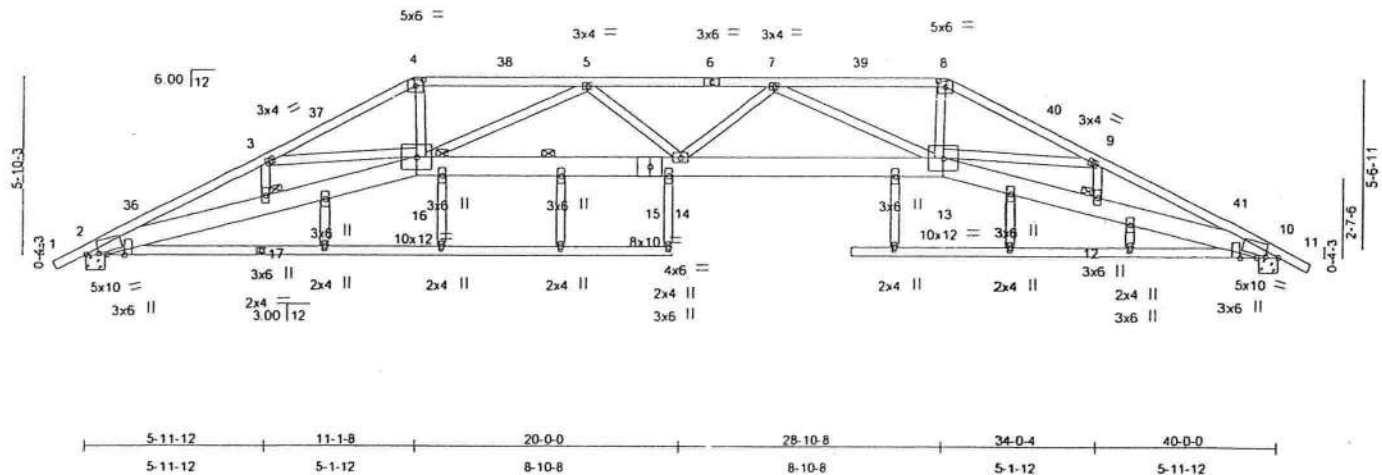


Plate Offsets (X,Y): [2:0-5-6:0-1:0], [4:0-3:0:0-2:7], [8:0-3:0-0-2:7], [10:0-5-6:0-1:0], [19:0-0-4:Edge], [26:0-0-4:Edge]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d		PLATES GRIP	
TCLL	16.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.68 14 >692 240	MT20	244/190
BCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-1.35 14 >349 180		
TCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.75 10 n/a n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)				Weight: 316 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.2 *Except*
 18-19 2 X 4 SYP No.2, 26-27 2 X 4 SYP No.2, 27-28 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 2-2-3 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-6-1 oc bracing. Except: 6-5-0 oc bracing: 14-16
JOINTS	1 Brace at Jt(s): 16, 17, 12

REACTIONS (lb/size) 2=1756/0-8-0, 10=1756/0-8-0
Max Horz 2=95(LC 5)
Max Uplift 2=613(LC 6), 10=613(LC 7)

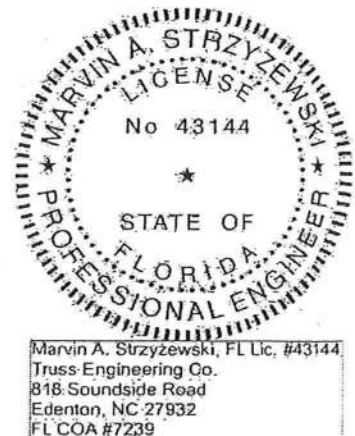
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/24, 2-36=5610/2022, 3-36=5545/2042, 3-37=5331/1956, 4-37=5285/1967, 4-38=4796/1852, 5-38=4796/1852, 5-6=5934/2200, 6-7=5934/2200, 7-39=4796/1852, 8-39=4796/1852, 8-40=5285/1967, 9-40=5331/1956, 9-41=5545/2042, 10-41=5610/2022, 10-11=0/24
BOT CHORD	2-17=1702/5067, 16-17=1721/5121, 15-16=1958/5736, 14-15=1958/5736, 13-14=1910/5736, 12-13=1721/5121, 10-12=1702/5067
WEBS	3-17=9/141, 3-16=209/285, 5-16=1168/676, 5-14=0/332, 7-14=0/332, 7-13=1168/675, 8-13=209/296, 9-12=9/141, 4-16=5852/103, 8-9=5852/103

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDF=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-13 to 1-11-3, Interior(1) 1-11-3 to 8-0-7, Exterior(2) 8-0-7 to 11-0-7, Interior(1) 14-0-7 to 25-11-9, Exterior(2) 28-11-9 to 41-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 613 lb uplift at joint 2 and 613 lb uplift at joint 10.

LOAD CASE(S) Standard



January 28, 2008

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA_125	E4637949
COVINGTON	HS6A	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:20 2008 Page 1

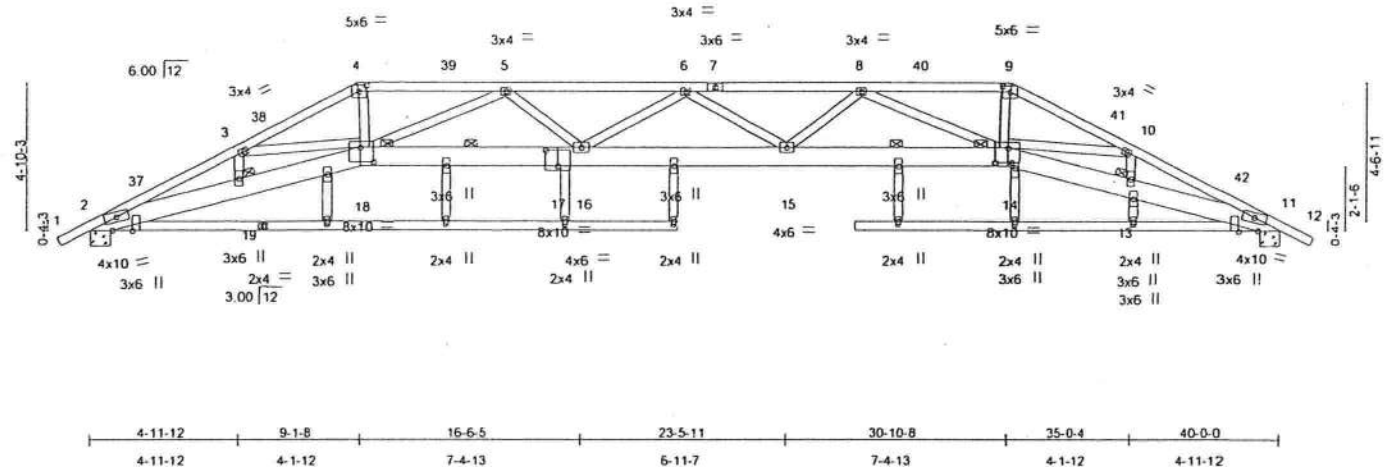
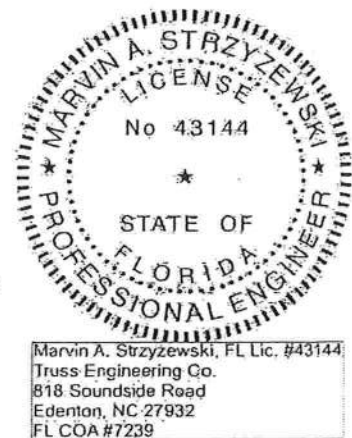


Plate Offsets (X,Y): [4-0-3-0-0-2-7] [9-0-3-0-0-2-7] [14-0-5-4-0-5-12] [17-0-5-0-0-6-2] [18-0-5-4-0-5-12] [21-0-0-4-Edge] [25-0-2-4-0-1-8] [27-0-0-4-Edge]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.81	Vert(LL)	-0.84 15-16	>565	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.95	Vert(TL)	-1.66 15-16	>285	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.69	Horz(TL)	0.78 11	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 313 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 1-11-14 oc purlins.
BOT CHORD	2 X 4 SYP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
	2-18 2 X 8 SYP No.1D, 17-18 2 X 8 SYP No.2, 11-14 2 X 8 SYP No.1D		5-9-0 oc bracing: 16-18
	14-17 2 X 8 SYP No.2		5-11-0 oc bracing: 14-15
WEBS	2 X 4 SYP No.2	JOINTS	1 Brace at Jt(s): 18, 14, 19, 13
REACTIONS			
(lb/size) 2=1756/0-8-0, 11=1756/0-8-0			
Max Horz 2=83(LC 6)			
Max Uplift 2=614(LC 6), 11=614(LC 7)			
FORCES (lb) - Maximum Compression/Maximum Tension			
TOP CHORD	1-2=0/24, 2-37=5394/1975, 3-37=5337/1987, 3-38=5558/2059, 4-38=5533/2070, 4-39=5038/1940, 5-39=5038/1940, 5-6=6922/2595, 6-7=6922/2581, 7-8=6922/2581, 8-40=5038/1940, 9-40=5038/1940, 9-41=5533/2070, 10-41=5558/2059, 10-42=5337/1987, 11-42=5394/1975, 11-12=0/24		
BOT CHORD	2-19=1674/4849, 18-19=1699/4925, 17-18=2449/6462, 16-17=2449/6462, 15-16=2697/7140, 14-15=2330/6462, 13-14=1684/4925, 11-13=1653/4849		
WEBS	3-19=166/179, 3-18=233/384, 5-18=1654/883, 5-16=126/664, 6-16=314/328, 6-15=314/328, 8-15=126/664, 8-14=1654/883, 10-14=247/384, 10-13=166/179, 4-18=712/2253, 9-14=678/2253		

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 6-0-7, Exterior(2) 6-0-7 to 9-0-7, Interior(1) 12-0-7 to 27-11-9, Exterior(2) 30-11-9 to 41-0-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 614 lb uplift at joint 2 and 614 lb uplift at joint 11.

LOAD CASE(S) Standard



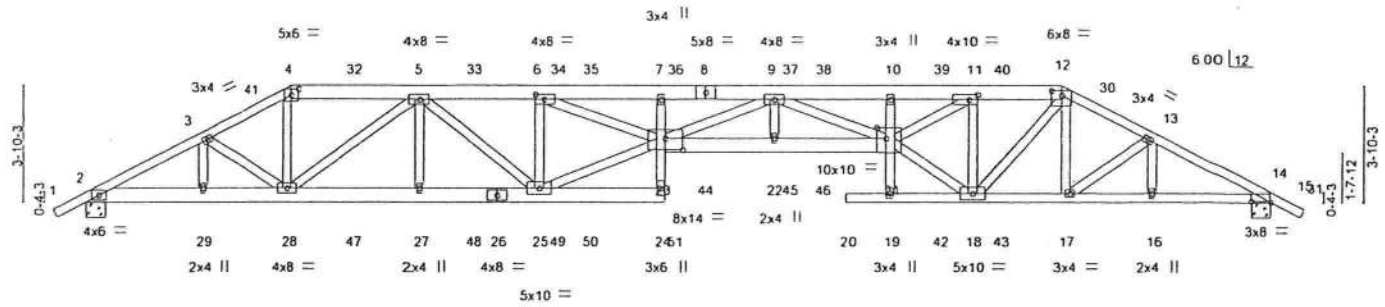
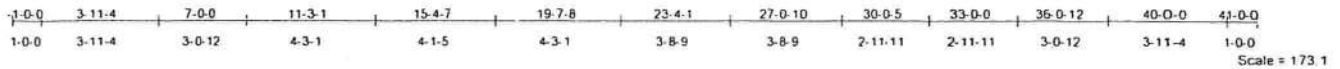
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

TRENCO
ENGINEERING BY
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637936
COVINGTON	HGRD	SPECIAL	1	2	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:08 2008 Page 1			



LOADING (psf)	SPACING	1-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.62	Vert(LL)	0.94	20	>501	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.96	Vert(TL)	-1.32	20	>357	180	244/190
BCLL 10.0	Rep Stress Incr	NO	WB 0.80	Horz(TL)	-0.41	2	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 547 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
4-8 2 X 6 SYP No.2, 8-12 2 X 6 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
21-23 2 X 6 SYP No.1D, 2-26 2 X 6 SYP No.2, 24-26 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-1 oc bracing. Except:
10-0-0 oc bracing: 19-21

REACTIONS (lb/size) 14=2232/0-8-0, 2=2673/0-8-0
Max Horz 14=41(LC 5)
Max Uplift 14=2046(LC 6), 2=1828(LC 5)
Max Grav 14=3045(LC 9), 2=2673(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 12-30=-5882/3948, 13-30=-5920/3957, 13-14=-5903/3879, 14-31=0/14, 15-31=0/8, 4-32=-4809/3328, 5-32=-4810/3329, 5-33=-7462/4820, 33-34=-7462/4820, 6-34=-7462/4820, 6-35=-13731/8525, 35-36=-13731/8525, 7-36=-13731/8525, 7-8=-13977/8653, 8-37=-13977/8653, 9-37=-13977/8653, 9-38=-14312/9125, 10-38=-14312/9125, 10-39=-14036/8959, 11-39=-14036/8959, 11-40=-6624/4350, 12-40=-6622/4349, 1-2=0/12, 2-3=-5266/3548, 3-41=-5450/3737, 4-41=-5422/3741
BOT CHORD 19-20=0/0, 19-42=-342/517, 18-42=-342/517, 18-43=-3476/5286, 17-43=-3476/5286, 16-17=-3364/5129, 14-16=-3364/5129, 19-21=-202/361, 10-21=-62/93, 23-44=-9426/15125, 44-45=-9426/15125, 22-45=-9426/15125, 22-46=-9426/15125, 21-46=-9426/15125, 23-24=-210/331, 7-23=-20/65, 2-29=-3066/4619, 28-29=-3066/4619, 28-47=-4287/6554, 27-47=-4287/6554, 27-48=-4287/6554, 26-48=-4287/6554, 26-49=-4287/6554, 25-49=-4287/6554, 25-50=-533/841, 50-51=-533/841, 24-51=-533/841
WEBS 13-16=0/96, 13-17=-247/229, 12-17=-584/773, 12-18=-1279/2067, 11-18=-5126/3246, 18-21=-4603/7156, 11-21=-5428/8711, 9-21=-2204/1602, 9-22=-294/526, 9-23=-1592/1277, 23-25=-4570/7132, 6-23=-4897/7826, 6-25=-3378/2154, 5-25=-1191/1840, 5-27=-194/333, 5-28=-2220/1399, 4-28=-1405/2122, 3-28=-324/382, 3-29=-123/151

- NOTES**
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc, 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA, 125	E4637936
COVINGTON	HGRD	SPECIAL	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:08 2008 Page 2

NOTES

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2046 lb uplift at joint 14 and 1828 lb uplift at joint 2.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 101 lb down and 124 lb up at 19-0-12, 101 lb down and 124 lb up at 17-0-12, 101 lb down and 124 lb up at 15-0-12, 101 lb down and 124 lb up at 13-0-12, 101 lb down and 124 lb up at 11-0-12, 101 lb down and 124 lb up at 9-0-12, 101 lb down and 124 lb up at 7-0-0, 101 lb down and 124 lb up at 20-11-4, 101 lb down and 124 lb up at 22-11-4, 101 lb down and 124 lb up at 24-11-4, 101 lb down and 124 lb up at 26-11-4, 101 lb down and 124 lb up at 28-11-4, and 101 lb down and 124 lb up at 30-11-4, and 101 lb down and 124 lb up at 33-0-0 on top chord, and 117 lb down and 103 lb up at 19-0-12, 117 lb down and 103 lb up at 17-0-12, 117 lb down and 103 lb up at 15-0-12, 117 lb down and 103 lb up at 13-0-12, 117 lb down and 103 lb up at 11-0-12, 117 lb down and 103 lb up at 9-0-12, 704 lb down and 610 lb up at 7-0-12, 117 lb down and 103 lb up at 20-11-4, 117 lb down and 103 lb up at 22-11-4, 117 lb down and 103 lb up at 24-11-4, 117 lb down and 103 lb up at 27-2-6, 117 lb down and 103 lb up at 28-11-4, and 117 lb down and 103 lb up at 30-11-4, and 704 lb down and 610 lb up at 32-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 12-31=-50(F=-27), 15-31=-23, 4-36=-23, 12-36=-50(F=-27), 1-4=-23, 19-20=-40(F=-20), 14-19=-40(F=-20), 21-23=-40(F=-20), 2-51=-20, 24-51=-40(F=-20)

Concentrated Loads (lb)

Vert: 4=-101(F) 27=-117(F) 5=-101(F) 28=-704(F) 32=-101(F) 33=-101(F) 34=-101(F) 35=-101(F) 36=-101(F) 47=-117(F) 48=-117(F) 49=-117(F) 50=-117(F) 51=-117(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E463797.1
COVINGTON	T1A	COMMON	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7 030 s Jan 3 2008 Mitek Industries, Inc Mon Jan 28 09 08 31 2008 Page 1			

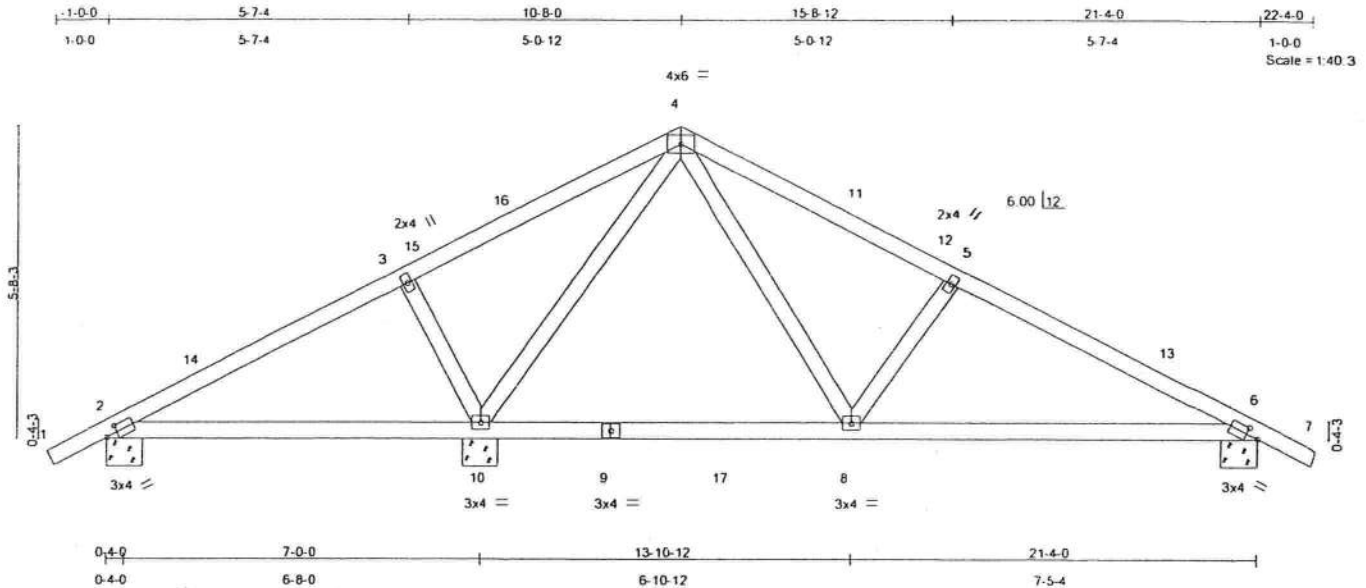


Plate Offsets (X,Y): [2-0-2-10-0-1-8], [6-0-2-10-0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.25	Vert(LL)	0.18	2-10	>439	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.41	Vert(TL)	0.15	2-10	>539	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.46	Horz(TL)	-0.01	10	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 98 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 6=625/0-8-0, 10=1143/0-8-0, 2=221/0-8-0
Max Horz 6=94(LC 5)
Max Uplift 6=309(LC 7), 10=419(LC 7), 2=309(LC 6)
Max Grav 6=625(LC 1), 10=1143(LC 1), 2=248(LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 4-11=581/271, 11-12=628/259, 5-12=638/251, 5-13=734/281, 6-13=766/263, 6-7=0/21, 1-2=0/21, 2-14=17/141,
3-14=11/167, 3-15=0/167, 15-16=0/172, 4-16=0/267
BOT CHORD 2-10=124/91, 9-10=0/203, 9-17=0/203, 8-17=0/203, 6-8=160/645
WEBS 5-8=231/266, 4-8=140/625, 4-10=707/263, 3-10=253/296

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-13 to 1-11-3, Interior(1) 1-11-3 to 7-8-0, Exterior(2) 7-8-0 to 10-8-0, Interior(1) 13-8-0 to 19-4-13 zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 6, 419 lb uplift at joint 10 and 309 lb uplift at joint 2.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E463797.2
COVINGTON	T1AS	COMMON	3	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						7.030 5 Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:32 2008 Page 1

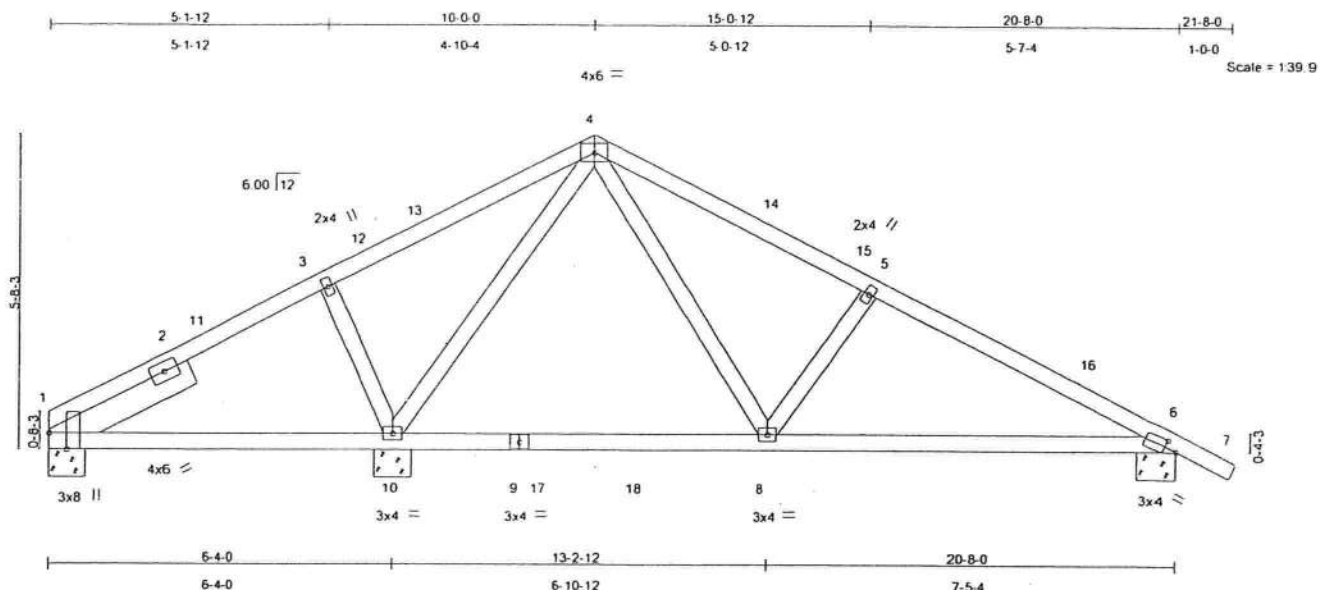


Plate Offsets (X,Y): [1:0-3-8,Edge], [6:0-2-10,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.25	Vert(LL)	-0.07	8-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.41	Vert(TL)	-0.14	6-8	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.40	Horz(TL)	0.01	6	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
SLIDER Left 2 X 6 SYP No.2 2-10-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=187/0-8-0, 10=1047/0-8-0, 6=649/0-8-0
Max Horz 1=115(LC 4)
Max Uplift 1=128(LC 6), 10=250(LC 6), 6=337(LC 7)
Max Grav 1=212(LC 10), 10=1047(LC 1), 6=649(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=86/111, 2-11=41/115, 3-11=29/125, 3-12=0/143, 12-13=0/148, 4-13=0/159, 4-14=633/354, 14-15=680/342,
5-15=689/334, 5-16=785/364, 6-16=818/345, 6-7=0/21
BOT CHORD 1-10=71/102, 9-10=0/250, 9-17=0/250, 17-18=0/250, 8-18=0/250, 6-8=178/691
WEBS 3-10=253/278, 4-10=610/200, 4-8=155/623, 5-8=231/262

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 7-0-0, Exterior(2) 7-0-0 to 10-0-0, Interior(1) 13-0-0 to 18-8-13 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 1, 250 lb uplift at joint 10 and 337 lb uplift at joint 6.

LOAD CASE(S) Standard



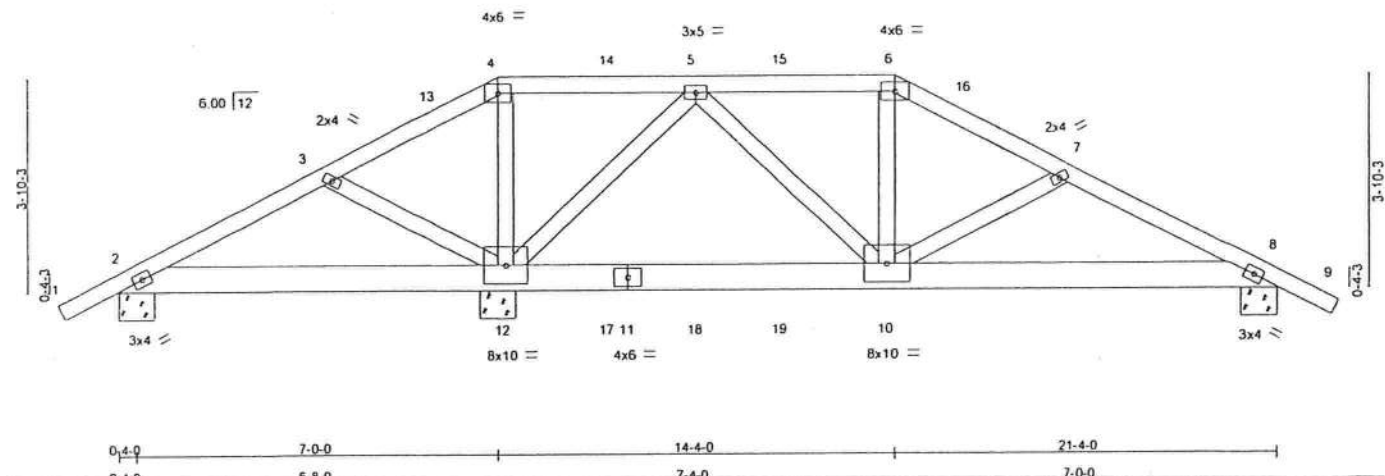
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MB-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E 4637937
COVINGTON	HGRDA	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:09 2006 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.39	Vert(LL)	0.05 8-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.29	Vert(TL)	-0.07 8-10	>999	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.57	Horz(TL)	0.01 8	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 121 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=55/0-8-0, 12=2764/0-8-0, 8=963/0-8-0
Max Horz 2=83(LC 6)
Max Uplift 2=85(LC 10), 12=2551(LC 5), 8=827(LC 6)
Max Grav 2=88(LC 8), 12=2764(LC 1), 8=972(LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/24, 2-3=415/492, 3-13=465/604, 4-13=465/604, 4-14=403/532, 5-14=404/532, 5-15=1236/1222,
6-15=1236/1222, 6-16=1339/1314, 7-16=1399/1306, 7-8=1530/1384, 8-9=0/24
BOT CHORD 2-12=418/458, 12-17=618/602, 11-17=618/602, 11-18=618/602, 18-19=618/602, 10-19=618/602, 8-10=1101/1324
WEBS 3-12=186/236, 4-12=585/676, 5-12=1552/1593, 5-10=655/906, 6-10=133/292, 7-10=115/148

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 2, 2551 lb uplift at joint 12 and 827 lb uplift at joint 8.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 126 lb down and 267 lb up at 7-0-0, 120 lb down and 251 lb up at 9-0-12, 120 lb down and 251 lb up at 10-8-0, and 120 lb down and 251 lb up at 12-3-4, and 120 lb down and 251 lb up at 14-4-0 on top chord, and 680 lb down and 507 lb up at 7-0-0, 587 lb down and 560 lb up at 14-4-0, 53 lb up at 9-0-12, and 53 lb up at 10-8-0, and 53 lb up at 12-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=46, 4-6=46, 6-9=46, 2-8=40

Continued on page 2



January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637937
COVINGTON	HGRDA	HIP	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:09 2008 Page 2

LOAD CASE(S) Standard

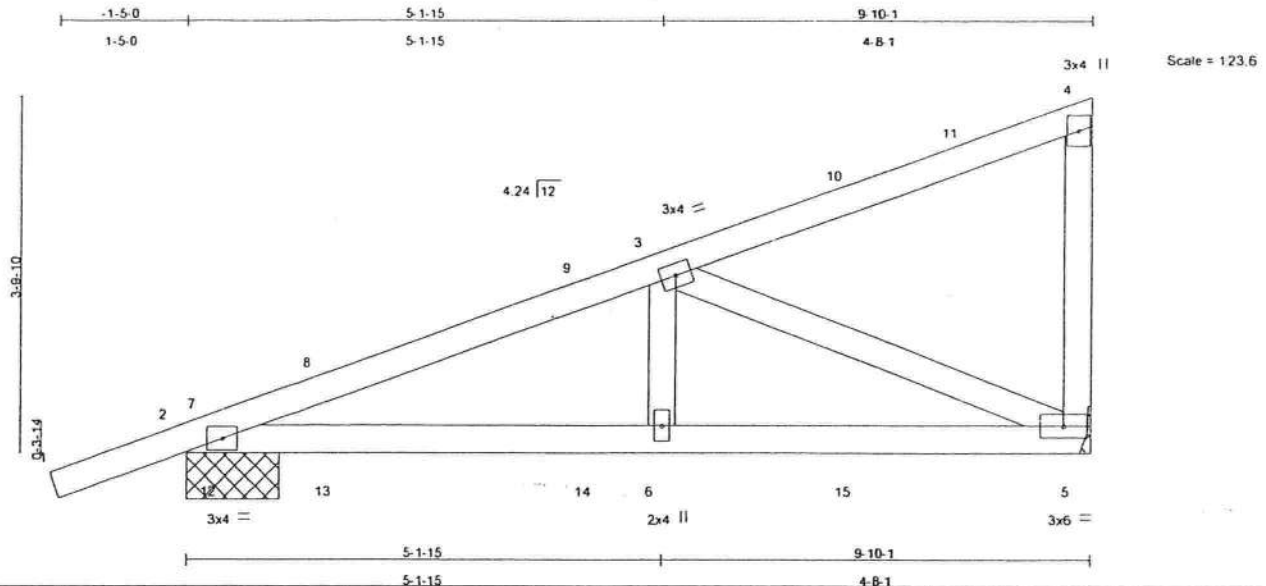
Concentrated Loads (lb)

Vert: 4=-126(F) 6=-120(F) 12=-680(F) 5=-120(F) 10=-587(F) 14=-120(F) 15=-120(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown
 is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component
 Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637961
COVINGTON	JGRD	MONO TRUSS	1	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, Florida			7 030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08 26 2008 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.38	Vert(LL)	0.05	5-6	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.33	Vert(TL)	-0.06	5-6	>999	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.30	Horz(TL)	0.01	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 46 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-13 oc bracing.

REACTIONS (lb/size) 5=590/Mechanical, 2=516/1-0-1
Max Horz 2=238(LC 3)
Max Uplift 5=547(LC 3), 2=356(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-7=932/713, 7-8=930/717, 8-9=900/738, 3-9=860/695, 3-10=123/114, 10-11=54/5, 4-11=27/11, 4-5=129/197
BOT CHORD 2-12=779/847, 12-13=779/847, 13-14=779/847, 6-14=779/847, 6-15=779/847, 5-15=779/847
WEBS 3-6=108/295, 3-5=848/759

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 547 lb uplift at joint 5 and 356 lb uplift at joint 2.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5 lb down and 67 lb up at 4-4-12, 5 lb down and 67 lb up at 4-4-12, 56 lb down and 165 lb up at 7-2-11, 56 lb down and 165 lb up at 7-2-11, and 61 lb down at 1-6-12, and 61 lb down at 1-6-12 on top chord, and 21 lb up at 1-6-12, 15 lb down and 52 lb up at 4-4-12, 15 lb down and 52 lb up at 4-4-12, and 55 lb down and 80 lb up at 7-2-11, and 55 lb down and 80 lb up at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

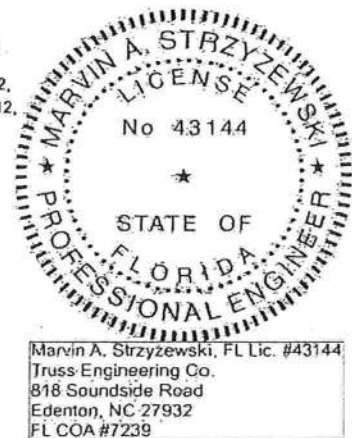
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=46, 2-7=14, 4-7=46, 2-12=20, 5-12=40

Concentrated Loads (lb)

Vert: 9=10(F=5, B=5) 10=113(F=56, B=56) 13=42(F=21, B=21) 14=31(F=15, B=15) 15=111(F=55, B=55)



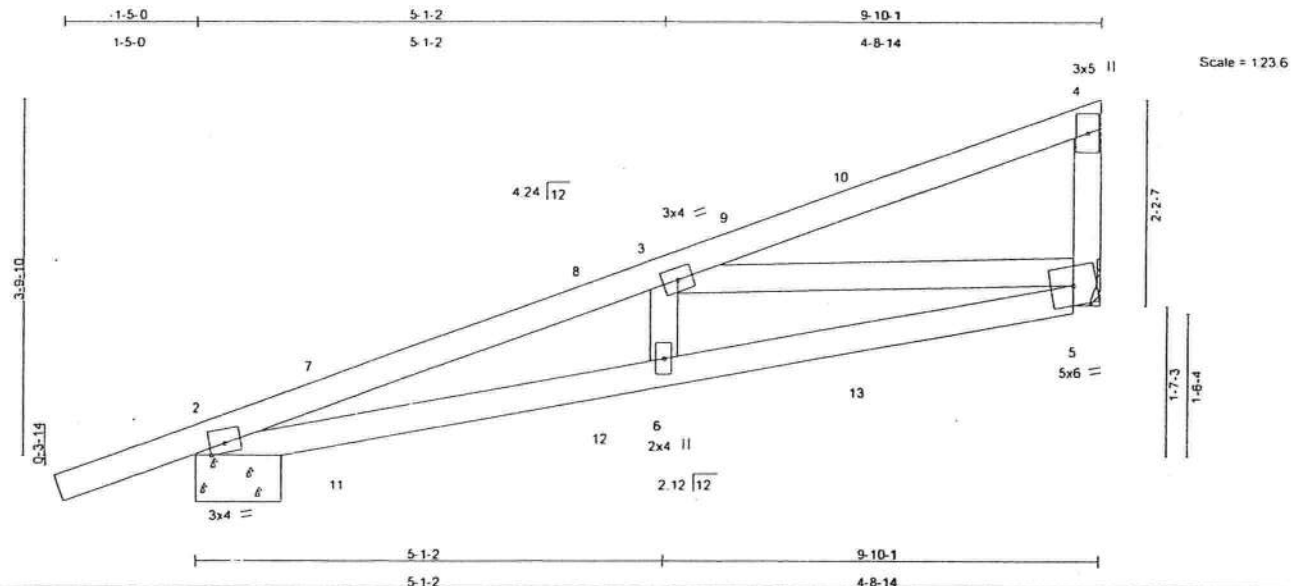
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637966
COVINGTON	JSGRD	SPECIAL	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:28 2008 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.37	Vert(LL)	0.06	6	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.44	Vert(TL)	-0.10	5-6	>999	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.33	Horz(TL)	0.02	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 43 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-5-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-10 oc bracing.

REACTIONS (lb/size) 2=514/0-11-5, 5=540/Mechanical
Max Horz 2=244(LC 3)
Max Uplift 2=272(LC 5), 5=376(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/17, 2-7=1306/669, 7-8=1266/690, 3-8=1235/657, 3-9=208/151, 9-10=175/154, 4-10=143/57, 4-5=113/209
BOT CHORD 2-11=745/1190, 11-12=741/1192, 6-12=737/1211, 6-13=754/1184, 5-13=745/1219
WEBS 3-6=0/254, 3-5=1024/613

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 2 and 376 lb uplift at joint 5.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 55 lb up at 4-4-12, 55 lb up at 4-4-12, 49 lb down and 157 lb up at 7-2-11, 49 lb down and 157 lb up at 7-2-11, and 58 lb down at 1-6-12, and 58 lb down at 1-6-12 on top chord, and 22 lb up at 1-6-12, 22 lb up at 1-6-12, 11 lb down at 4-4-12, 11 lb down at 4-4-12, and 51 lb down at 7-2-11, and 51 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-46, 2-5=-40

Concentrated Loads (lb)

Vert: 8=10(F=5, B=5) 10=-97(F=-49, B=-49) 11=44(F=22, B=22) 12=-22(F=-11, B=-11) 13=-102(F=-51, B=-51)



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Maronda Homes Inc. Sanford, Florida

7 030 s Jan 3 2008 MiTek Industries, Inc. Mon Jan 28 09:08:27 2008 Page 1



Weight: 26 lb

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

Max Horz2=238(LC 6)
Max Uplift4=-106(LC 6), 2=-191(LC 6), 5=-48(LC 6)

WEBS 3-6=-11/211

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 106 lb uplift at joint 4, 191 lb uplift at joint 2 and 48 lb uplift at joint 5.

LOAD CASE(S) Standard

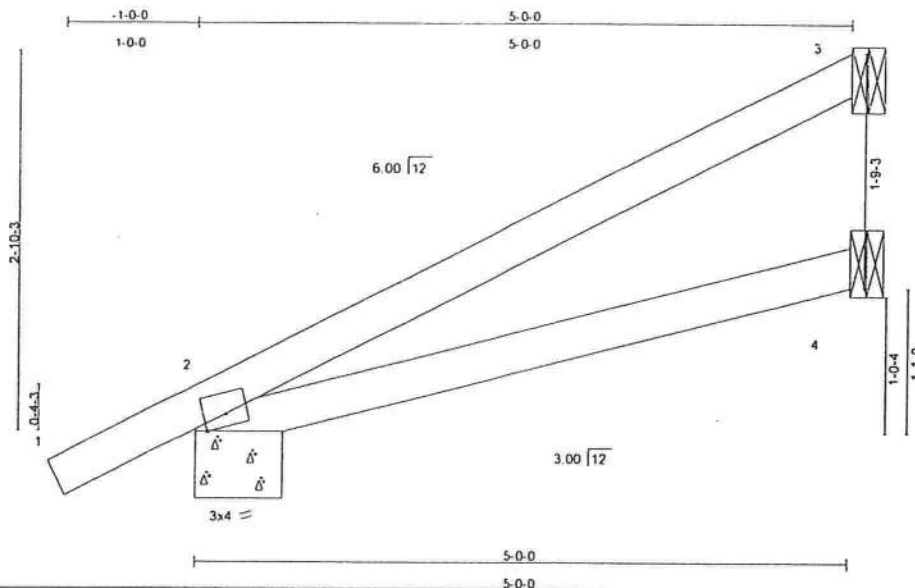


January 28, 2008

Safety Information available from Trussteel Institute, 583 D'Oroville Drive, Madison WI 53719.

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637963
COVINGTON	JS2	SPECIAL	4	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 MTek Industries, Inc. Mon Jan 28 09 08 27 2008 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.33	Vert(LL)	-0.02	2-4	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.20	Vert(TL)	-0.06	2-4	>947	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 18 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=95/Mechanical, 2=270/0-8-0, 4=91/Mechanical
Max Horz 2=187(LC 6)
Max Uplift 3=141(LC 6), 2=169(LC 6)

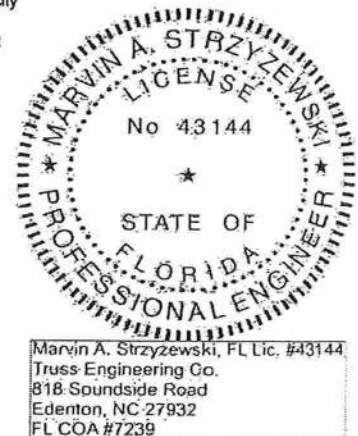
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-129/36
BOT CHORD 2-4=-18/18

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 3 and 169 lb uplift at joint 2.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

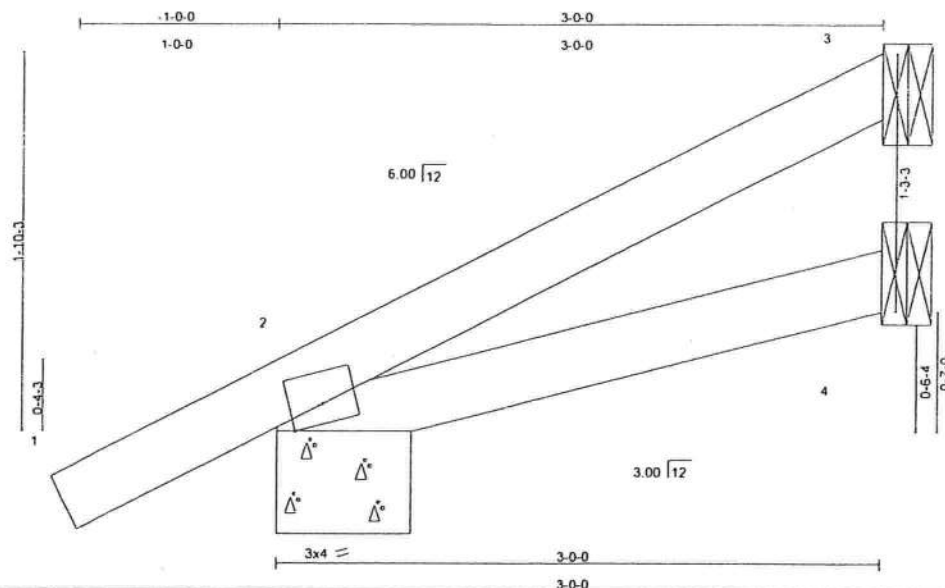
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637964
COVINGTON	JS3	SPECIAL	4	1	Job Reference (optional)	
Maronda Homes, Inc., Sanford, Florida		7 030 s Jan 3 2008 MiTek Industries, Inc. Mon Jan 28 09:08 27 2008 Page 1				



Scale = 1/10.8

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.15	Vert(LL) -0.00	2-4	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.06	Vert(TL) -0.01	2-4	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)						
							Weight: 12 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 3=41/Mechanical, 2=191/0-8-0, 4=51/Mechanical
Max Horz 2=127(LC 6)
Max Uplift 3=62(LC 6), 2=158(LC 6)

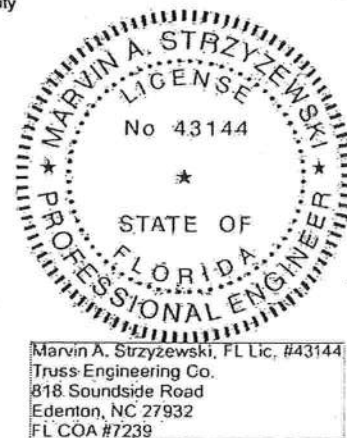
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=66/15
BOT CHORD 2-4=-10/10

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 1'-0" wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 3 and 158 lb uplift at joint 2.

LOAD CASE(S) Standard



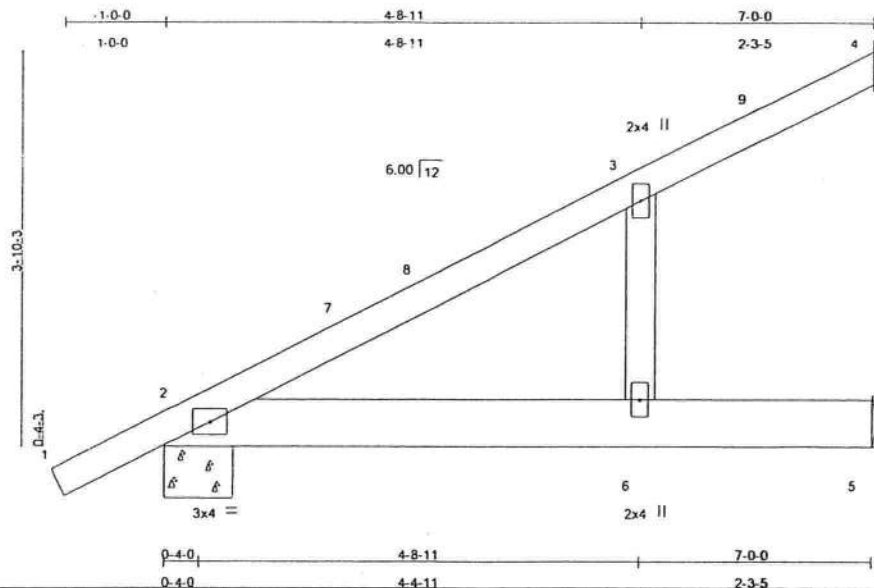
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E463795.1
COVINGTON	J1	MONO TRUSS	10	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida		7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:06 27 2008 Page 1				



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.28	Vert(LL)	0.17	2-6	>457	240	MT20
TCOL 7.0	Lumber Increase	1.25	BC 0.50	Vert(TL)	0.14	2-6	>553	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.04	Horz(TL)	-0.01	4	n/a	n/a	
BCOL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 33 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=66/Mechanical, 2=355/0-8-0, 5=211/Mechanical
Max Horz 2=244(LC 6)
Max Uplift 4=59(LC 6), 2=304(LC 6), 5=197(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/24, 2-7=200/0, 7-8=190/0, 3-8=189/29, 3-9=68/3, 4-9=64/25
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=119/246

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 1-0-13 to 1-11-3, Interior(1) 1-11-3 to 2-8-5, Exterior(2) 2-8-5 to 6-11-4 zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 4, 304 lb uplift at joint 2 and 197 lb uplift at joint 5.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

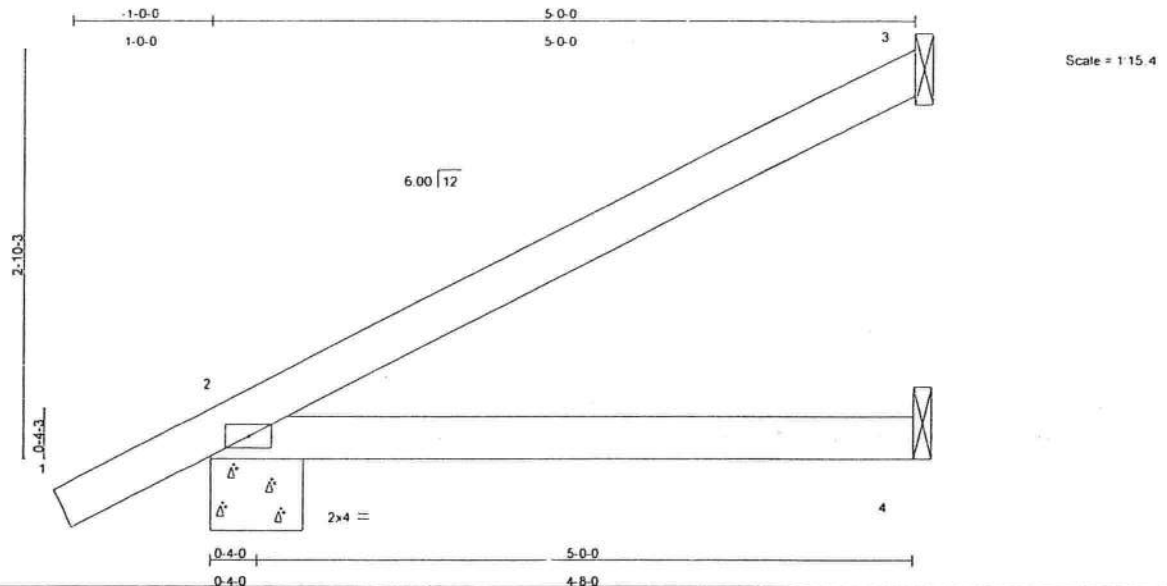
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637953
COVINGTON	J2	JACK	5	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

7:030 s Jan 3 2008 MiTek Industries, Inc. Mon Jan 28 09:08 23 2008 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.34	Vert(LL)	0.10	2-4	>579	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.27	Vert(TL)	0.07	2-4	>744	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical
Max Horz 2=190(LC 6)
Max Uplift 3=140(LC 6), 2=247(LC 6), 4=47(LC 4)

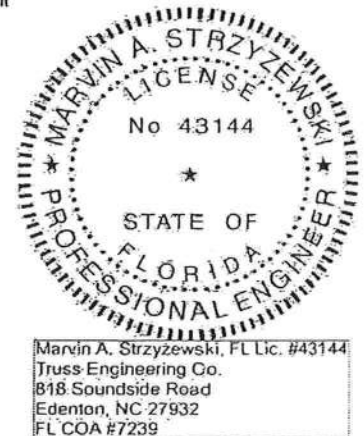
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=126/36
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 3, 247 lb uplift at joint 2 and 47 lb uplift at joint 4.

LOAD CASE(S) Standard



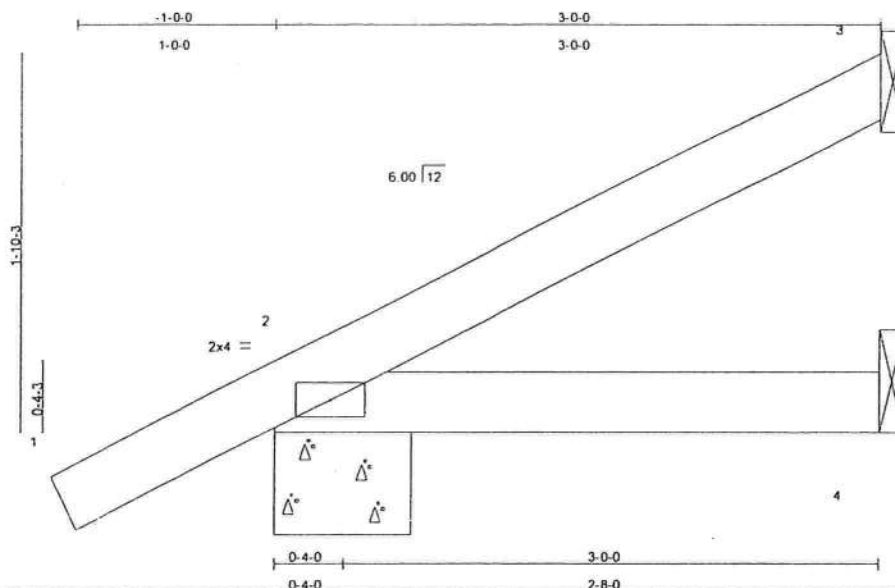
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCSII Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637955
COVINGTON	J3	JACK	5	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 MTEK Industries, Inc. Mon Jan 28 09:08 24 2008 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.15	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.09	Vert(TL)	-0.01	2-4	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 11 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical
Max Horz 2=131(LC 6)
Max Uplift 3=61(LC 6), 2=204(LC 6), 4=27(LC 4)

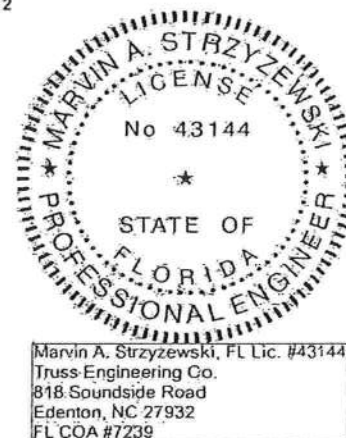
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=63/15
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 3, 204 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard



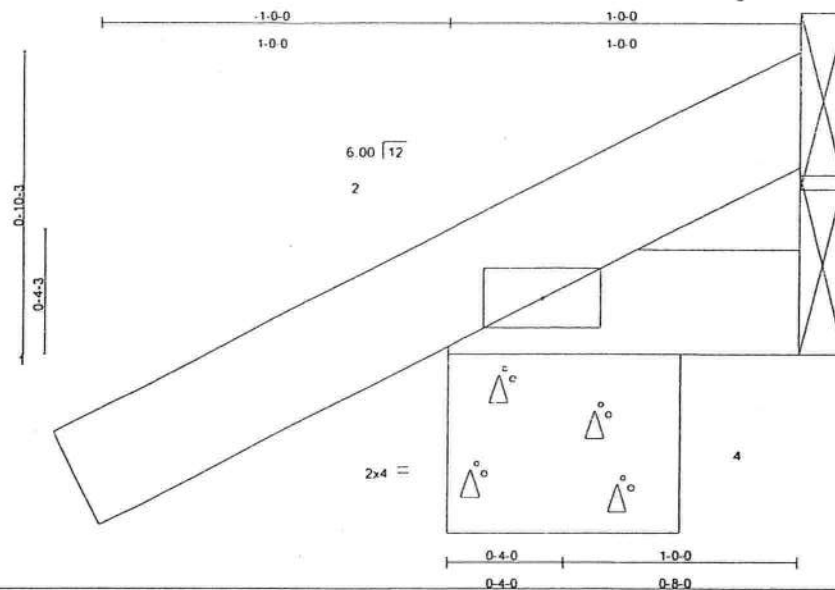
Marvin A. Strzyzewski, FL Lic. #43144
Truss-Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637957
COVINGTON	J4	JACK	5	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida		7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:25 2008 Page 1				



Scale = 1/6.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.11	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 5 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=18/Mechanical, 2=124/0-8-0, 3=11/Mechanical
Max Horz2=73(LC 6)
Max Uplift2=157(LC 6), 3=11(LC 1)
Max Grav4=18(LC 1), 2=124(LC 1), 3=30(LC 6)

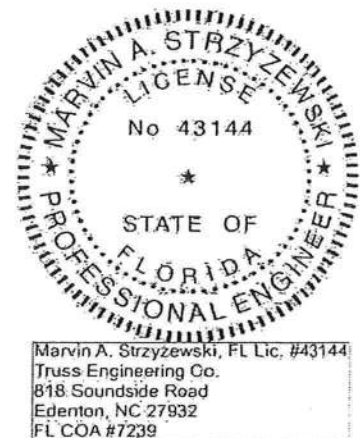
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=27/15
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 157 lb uplift at joint 2 and 11 lb uplift at joint 3.

LOAD CASE(S) Standard



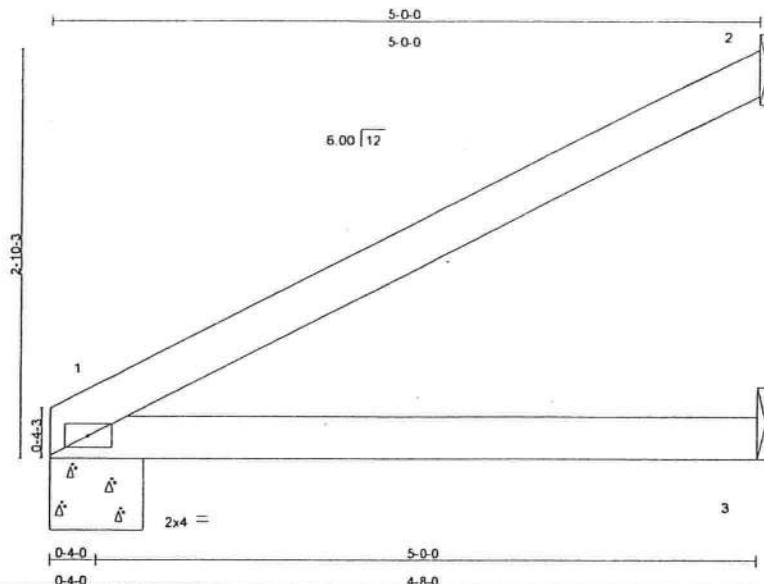
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637954
COVINGTON	J2S	JACK	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7 030 5 Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09 08 24 2008 Page 1			



Scale = 1/15.4

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	l/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.41	Vert(LL)	0.10	1-3	>579	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.27	Vert(TL)	0.07	1-3	>744	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 16 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=198/0-8-0, 2=106/Mechanical, 3=92/Mechanical
Max Horz 1=149(LC 6)
Max Uplift 1=124(LC 6), 2=158(LC 6), 3=47(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-135/41
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 1, 158 lb uplift at joint 2 and 47 lb uplift at joint 3.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

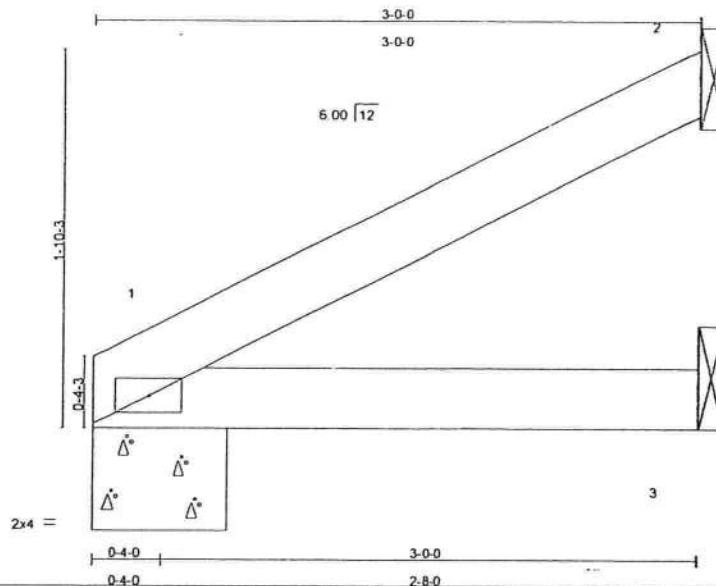
January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637956
COVINGTON	J3S	JACK	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09 08:24 2008 Page 1			



Scale = 1/10 B

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/def	L/d	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.13	Vert(LL)	0.01	1-3	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.09	Vert(TL)	-0.01	1-3	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TP12002								
								Weight: 10 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=112/0-8-0, 2=60/Mechanical, 3=52/Mechanical
Max Horz 1=90(LC 6)
Max Uplift 1=67(LC 6), 2=93(LC 6), 3=27(LC 4)

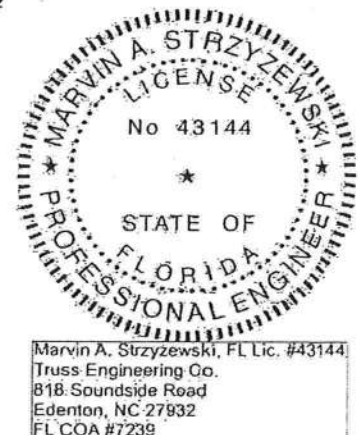
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-82/24
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 1, 93 lb uplift at joint 2 and 27 lb uplift at joint 3.

LOAD CASE(S) Standard



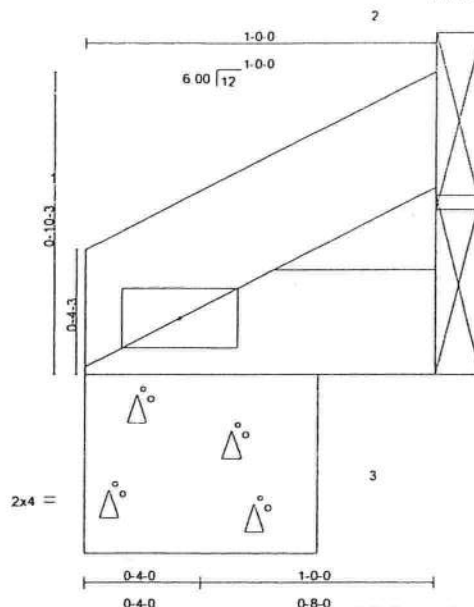
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4627958
COVINGTON	J4S	JACK	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7:030 s Jan 3 2008 MiTek Industries, Inc. Mon Jan 28 09:08:25 2008 Page 1			



Scale = 1:6.2

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.02	Vert(LL)	-0.00	1	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	1	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 3 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=40/0-8-0, 3=18/Mechanical, 2=21/Mechanical
Max Horz 1=32(LC 6)
Max Uplift 1=9(LC 6), 2=33(LC 6)

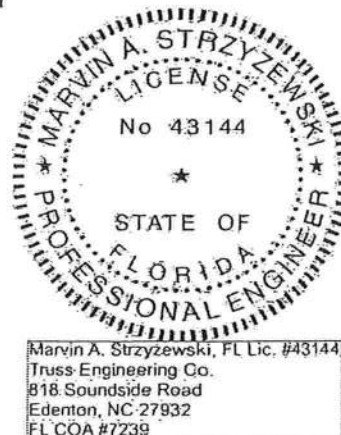
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=29/8
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1 and 33 lb uplift at joint 2.

LOAD CASE(S) Standard



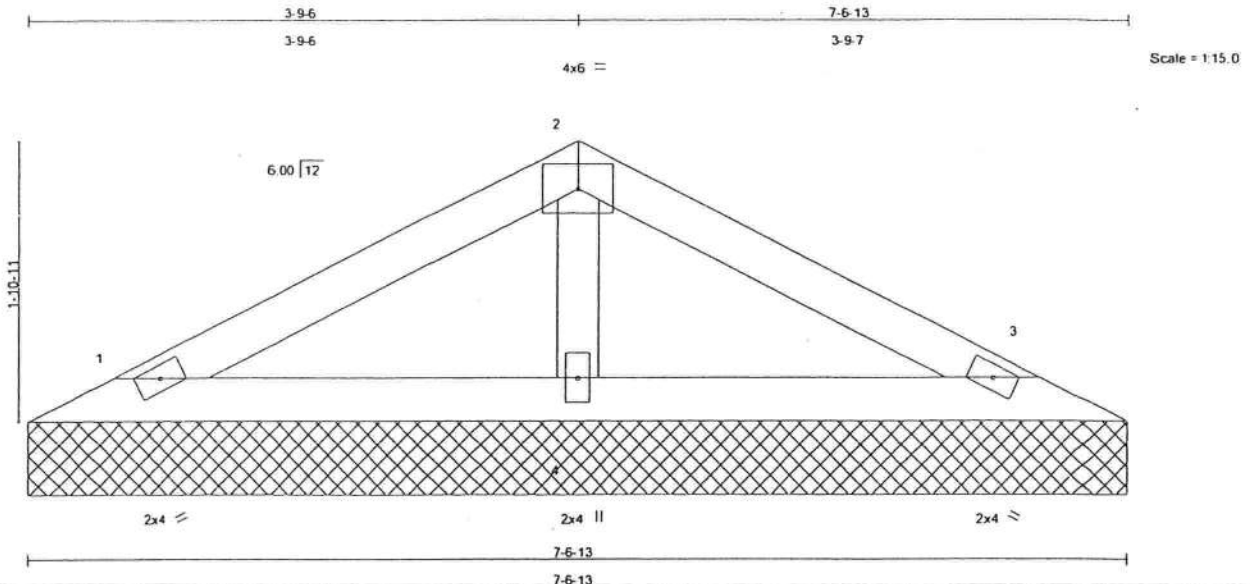
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL CQA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Orazio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON_FLORIDA_125	E4637967
COVINGTON	RG1	GABLE	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7 030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 09:08:28 2008 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.20	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.11	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	NO	WB 0.03	Horz(TL)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=127/7-6-13, 3=127/7-6-13, 4=290/7-6-13
 Max Horz 1=30(LC 4)
 Max Uplift 1=74(LC 6), 3=80(LC 7), 4=64(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-41/57, 2-3=-41/57
 BOT CHORD 1-4=-1/21, 3-4=-1/21
 WEBS 2-4=-132/189

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 1, 80 lb uplift at joint 3 and 64 lb uplift at joint 4.

LOAD CASE(S) Standard



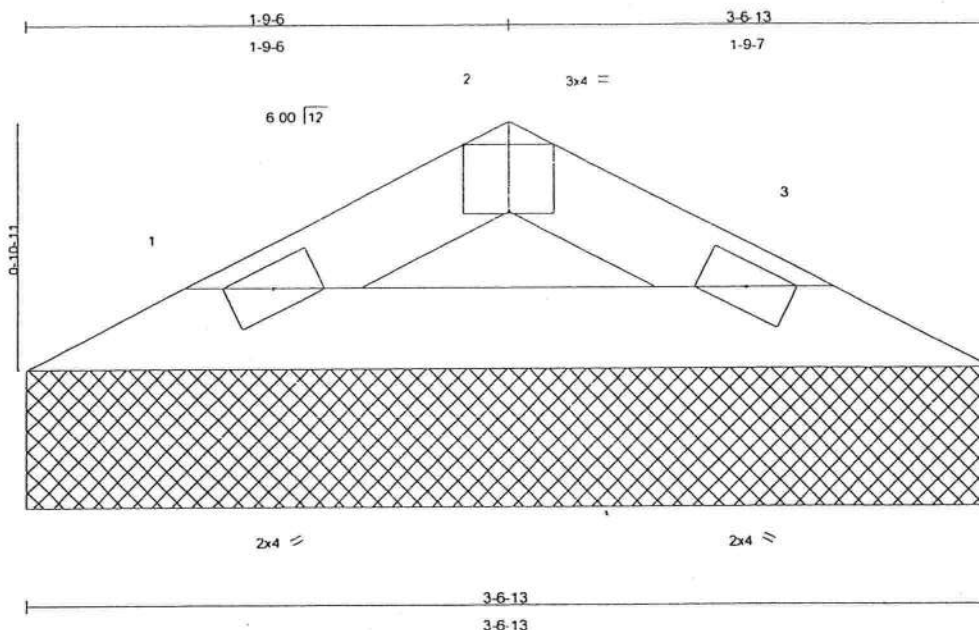
Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information - available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA_125	E4637978
COVINGTON	V1	VALLEY	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 10 23 38 2008 Page 1



Scale 1/2"=1'

Plate Offsets (X,Y): [2-0-2-0,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.04	Vert(LL)	n/a	-	n/a	999	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	n/a	-	n/a	999	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 9 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=100/3-6-13, 3=100/3-6-13
Max Horz 1=11(LC 5)
Max Uplift 1=46(LC 6), 3=46(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=70/108, 2-3=70/108
BOT CHORD 1-3=69/54

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) automatic zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- WARNING:** Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 46 lb uplift at joint 3.

LOAD CASE(S) Standard



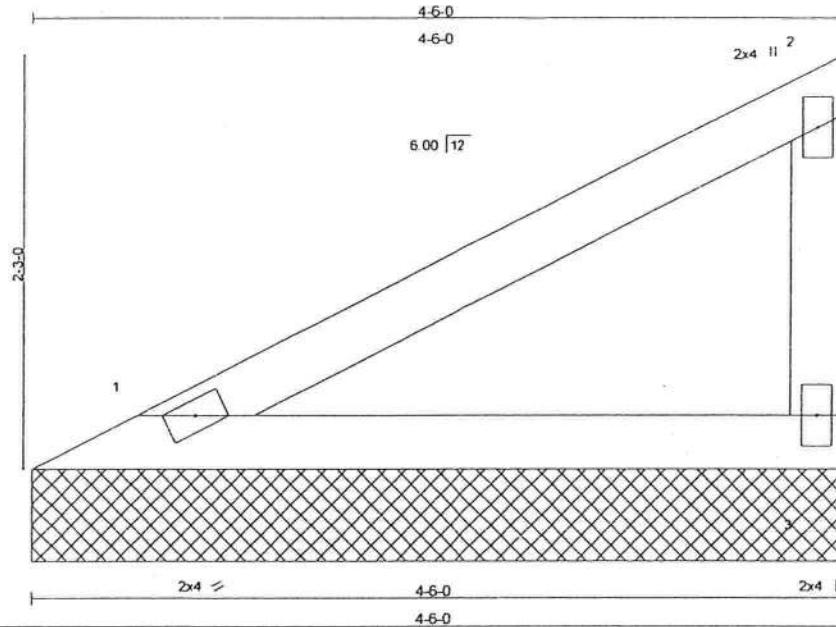
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818. Spoundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	COVINGTON, FLORIDA 125	E4637979
COVINGTON	VIA	VALLEY	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida			7.030 s Jan 3 2008 Mitek Industries, Inc. Mon Jan 28 10:23:50 2008 Page 1			



Scale: 1"=1'

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.26	Vert(LL)	n/a	-	n/a	999	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.13	Vert(TL)	n/a	-	n/a	999	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00		n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 16 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=161/4-6-0, 3=161/4-6-0
Max Horz 1=111(LC 6)
Max Uplift 1=45(LC 6), 3=101(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=99/31, 2-3=86/193
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 1 and 101 lb uplift at joint 3.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818. Soundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Maronda Homes Inc., Sanford, Florida

7 030 s Jan 3 2008 MiTek Industries, Inc. Mon Jan 28 10 24 06 2008 Page 1



LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.2

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 2-6-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=75/2-6-0, 3=75/2-6-0
Max Horz 1=52(LC 6)
Max Uplift 1=21(LC 6), 3=47(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-4=-46/0, 2-4=-43/14, 2-3=-40/90
BOT CHORD 1-5=0/0, 3-5=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDF=4.2psf; BCDF=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 1 and 47 lb uplift at joint 3.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818. Spoundside Road
Edenton, NC 27932
FL COA #7239

January 28, 2008



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Miltek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

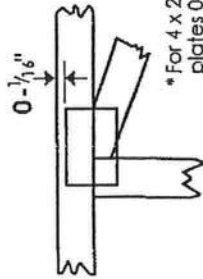
ENGINEERING BY
TRENCO
A MILCO ALFALA

818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



* For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.

* This symbol indicates the required direction of slots in connector plates.



* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 x 4

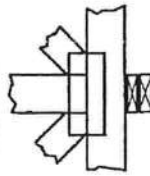
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



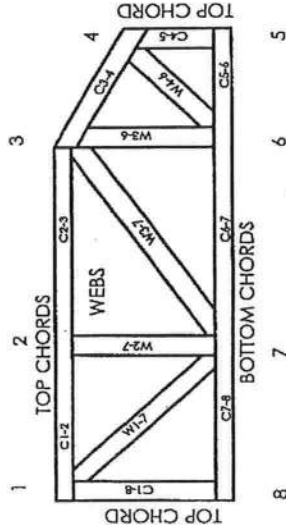
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSII: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR-5243, 9604B
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

© 2006 Mitek® All Rights Reserved



Mitek Engineering Reference Sheet: MIL-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear lightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 17% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

LEGAL DESCRIPTION:

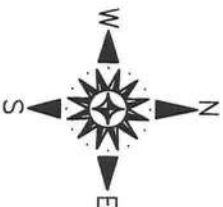
LOT TEN (10) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

BUD ESPENSHIP
14.00 AC±
NOT A PART

S 89°15'24"E 115.00' (P)
S 89°10'58"E 115.23' (M)

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA

BOUNDARY SURVEY



CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

BEARING NOTE:

BEARINGS SHOWN HEREON ARE REFERRED TO AN ASSUMED VALUE OF N. 00°44'36" E., FOR THE WEST PROPERTY LINE OF LOT 10.

BENCHMARK NOTE:

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN.

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

POLM. #
27235

LOT 11

BEARING BASIS
N 00°44'36"E 190.62' (P)
N 00°44'36"E 190.66' (M)

N 89°43'38"W 90.39' (P)
N 89°43'48"W 90.55' (C)

N 89°43'38"W 115.00' (P)
N 89°41'27"W 115.00' (M)

S 00°44'36"W 189.67' (P)
S 00°48'45"W 189.64' (M)

LOT 9

30.00' (P)
00°16'12"E 29.96' (C)
Z X

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

EDGE OF PAVEMENT
S.W. TIMBER RIDGE DRIVE
60' RIGHT-OF-WAY
±20' ASPHALT ROAD

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.



LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
 - = FOUND 1/2" REBAR & CAP
 - = SET 1/2" REBAR & CAP
 - = L.B. 6894
 - = FOUND 3/4" IRON PIPE
 - = FOUND 4" X 4" CONC. MON.
 - = NO IDENTIFICATION
 - = SET 4" X 4" CONC. MON.
 - = P.S.M. 5582
 - ✕ = SET NAIL & DISK P.S.M. 5582
 - ✕ = FOUND NAIL & DISK P.S.M. 5757
 - ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
 - ⊠ = CATV RISER
 - ⊠ = TELEPHONE PEDESTAL
 - ⊠ = WOOD POWER POLE
- ABBREVIATIONS:
- A/C = AIR CONDITIONER
 - ASPH = ASPHALT
 - C = CALCULATED FROM MEASURED
 - CATV = CABLE TELEVISION
 - C/B = CONCRETE BLOCK
 - CLF = CHAIN LINK FENCE
 - CM = CONCRETE MONUMENT
 - CONC = CONCRETE
 - ELEC = ELECTRIC
 - ELEV = ELEVATION
 - FNC = FENCE
 - FND = FOUND
 - LB = LICENSED SURVEYOR BUSINESS
 - LB = FIELD MEASURED
 - MFF = MINIMUM FINISHED FLOOR
 - OH = OVERHEAD UTILITIES
 - O.U. = PLAT
 - P = PLAT
 - P.B. = PUBLIC UTILITIES EASEMENT
 - P.U.E. = TRANSFORMER
 - TRANS = TYPICAL
 - WM = WATER METER
 - WV = WATER VALVE

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE.

BY *James E. Brinkman*
JAMES E. BRINKMAN, PSM - FLA. CERT# 5562

DATE: 8/8/08



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL

DATE: 8/8/08

CHECKED BY: J.B.

FIELD WORK COMPLETED ON 08/07/08 FIELDBOOK 97, PAGE 52

PREPARED FOR: MARONDA

DRAWING NUMBER
124-08

LOT TEN (10) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA

S 89°15'24"E 115.00' (P)

1) MARONDA HOMES

BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT
25', REAR 15', SIDE 10'

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

3) IN THE OPINION OF THIS SURETOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURETOR.

4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

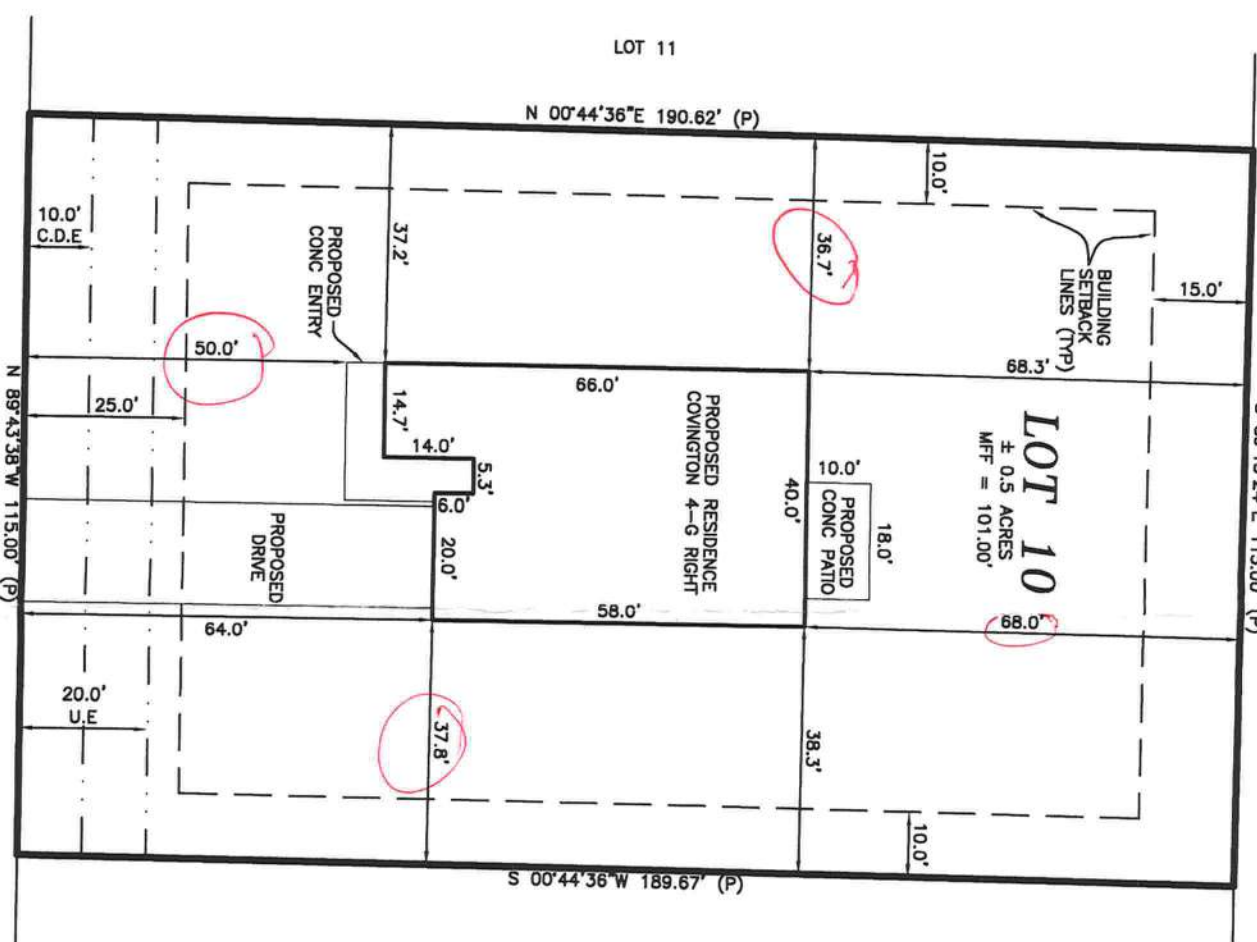
5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).

6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

S.W. **TIMBER RIDGE DRIVE**
60' RIGHT-OF-WAY

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.



● = FOUND 1/2" REBAR NO IDENTIFICATION	A/C = AIR CONDITIONER
○ = FOUND 1/2" REBAR & CAP L.B. 6894	ASPH = ASPHALT
○ = SET 1/2" REBAR & CAP L.B. 6894	C = CALCULATED FROM MEASURED
○ = SET 1/2" REBAR & CAP L.B. 6894	CATV = CABLED TELEVISION
○ = SET 1/2" REBAR & CAP L.B. 6894	C/B = CONCRETE BLOCK
○ = SET 1/2" REBAR & CAP L.B. 6894	CLF = CHAIN LINK FENCE
○ = SET 1/2" REBAR & CAP L.B. 6894	CM = CONCRETE MONUMENT
○ = SET 1/2" REBAR & CAP L.B. 6894	CONC = CONCRETE
○ = SET 1/2" REBAR & CAP L.B. 6894	ELEC = ELECTRIC
○ = SET 1/2" REBAR & CAP L.B. 6894	ELEV = ELEVATION
○ = SET 1/2" REBAR & CAP L.B. 6894	FND = FOUND
○ = SET 1/2" REBAR & CAP L.B. 6894	FNC = FENCE
○ = SET 1/2" REBAR & CAP L.B. 6894	LB = LICENSED SURVEYOR BUSINESS
○ = SET 1/2" REBAR & CAP L.B. 6894	(M) = FIELD MEASURED
○ = SET 1/2" REBAR & CAP L.B. 6894	MFF = MINIMUM FINISHED FLOOR
○ = SET 1/2" REBAR & CAP L.B. 6894	MH = MANHOLE
○ = SET 1/2" REBAR & CAP L.B. 6894	O.U. = OVERHEAD UTILITIES
○ = SET 1/2" REBAR & CAP L.B. 6894	P = PLAT
○ = SET 1/2" REBAR & CAP L.B. 6894	PB = PLAT BOOK
○ = SET 1/2" REBAR & CAP L.B. 6894	P.U.E. = PUBLIC UTILITIES EASEMENT
○ = SET 1/2" REBAR & CAP L.B. 6894	TRANS = TRANSFORMER
○ = SET 1/2" REBAR & CAP L.B. 6894	TRYP = TYPICAL
○ = SET 1/2" REBAR & CAP L.B. 6894	WM = WATER METER
○ = SET 1/2" REBAR & CAP L.B. 6894	WV = WATER VALVE

A/C	=	AIR CONDITIONER
ASPH	=	ASPHALT
C	=	CALCULATED FROM MEASURED
CATV	=	CABLE TELEVISION
C/B	=	CONCRETE BLOCK
CLF	=	CHAIN LINK FENCE
CM	=	CONCRETE MONUMENT
CONC	=	CONCRETE
ELEC	=	ELECTRIC
ELEV	=	ELEVATION
FND	=	FOUND
FNC	=	FENCE
LB	=	LICENSED SURVEYOR BUSINESS
(M)	=	FIELD MEASURED
MFF	=	MINIMUM FINISHED FLOOR
2	=	MANHOLE
O.U.	=	OVERHEAD UTILITIES
P	=	PLAY
PB	=	PLAY BOOK
P.U.E.	=	PUBLIC UTILITIES EASEMENT
TRANS	=	TRANSFORMER
TYP	=	TYPICAL
WM	=	WATER METER
WV	=	WATER VALVE

THIS IS NOT A BOUNDARY SURVEY
CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G-16, FLORIDA ADMINISTRATIVE CODE.

JAMES E. BRINKMAN, PSM - FLA. CERT# 5582

DATE: 6/12/08



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609

PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

"THE BENCHMARK IN QUALITY SERVICE"

DATE: 6/12/08

CHECKED BY: J.B.

FIELD WORK COMPLETED ON **** FIELDBOOK **, PAGE **

PREPARED FOR: MARONDA

DRAWING NUMBER