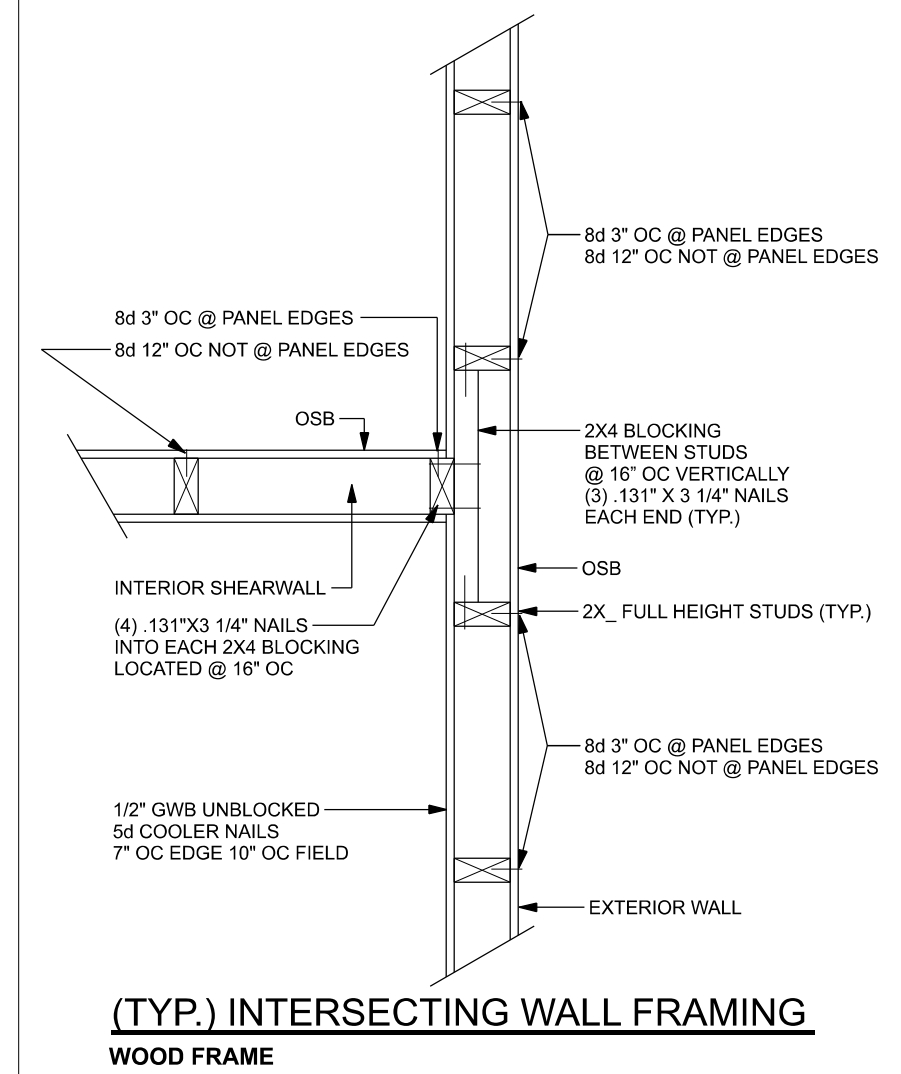
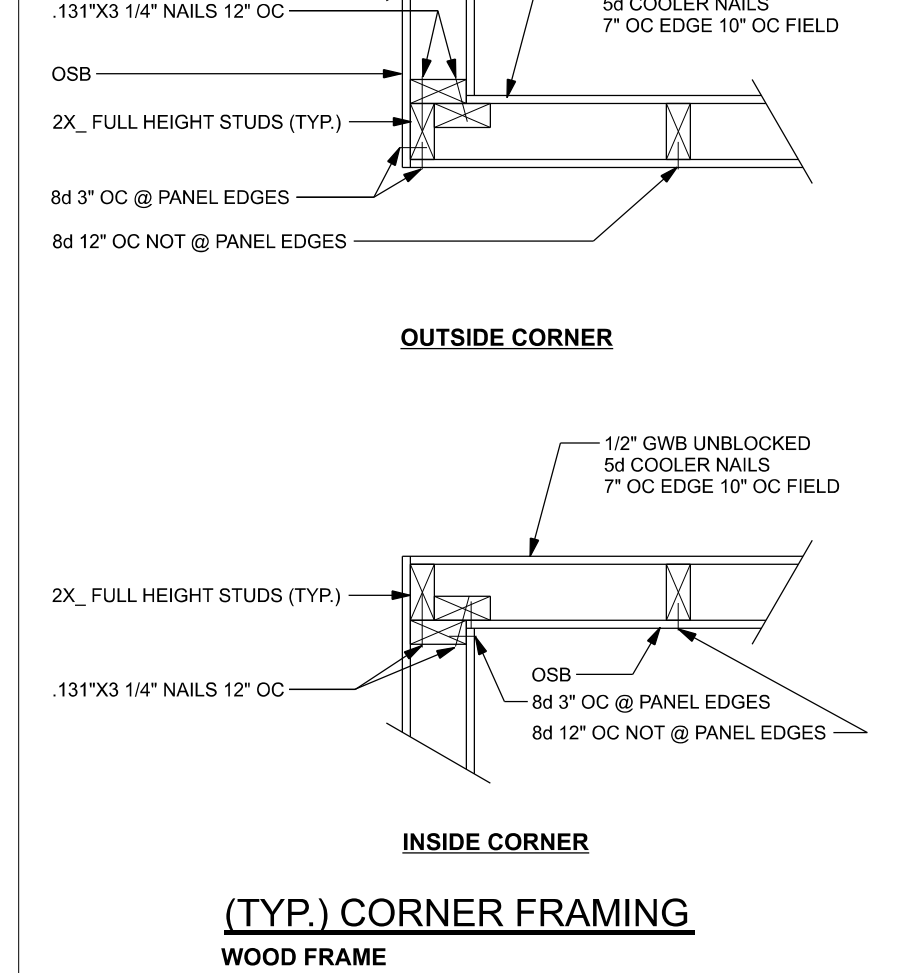


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



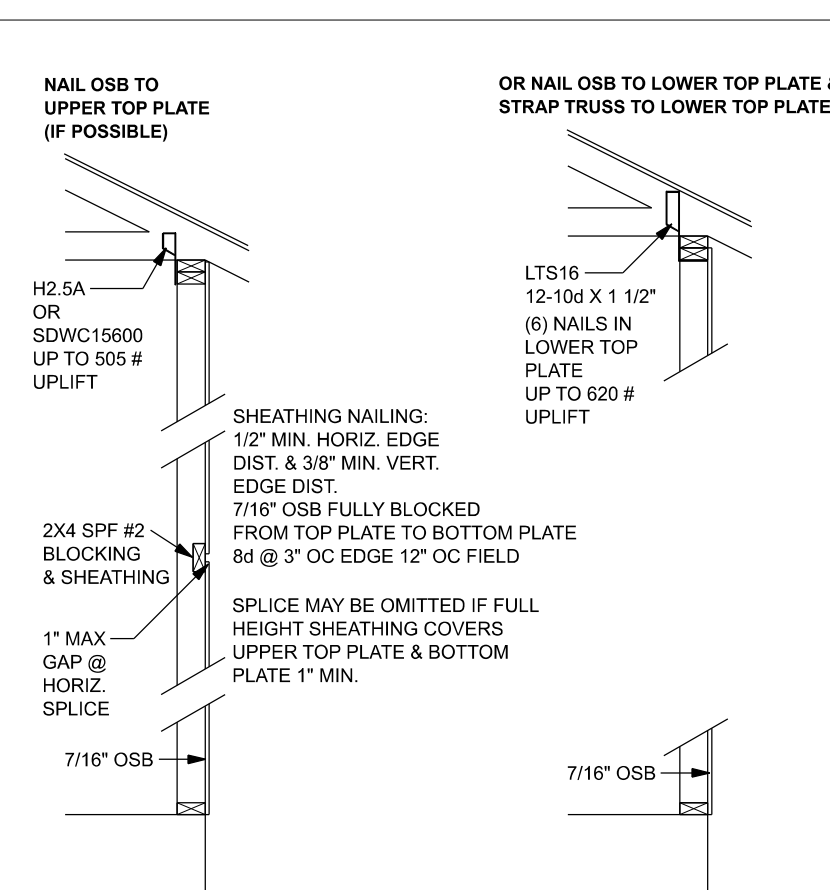
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



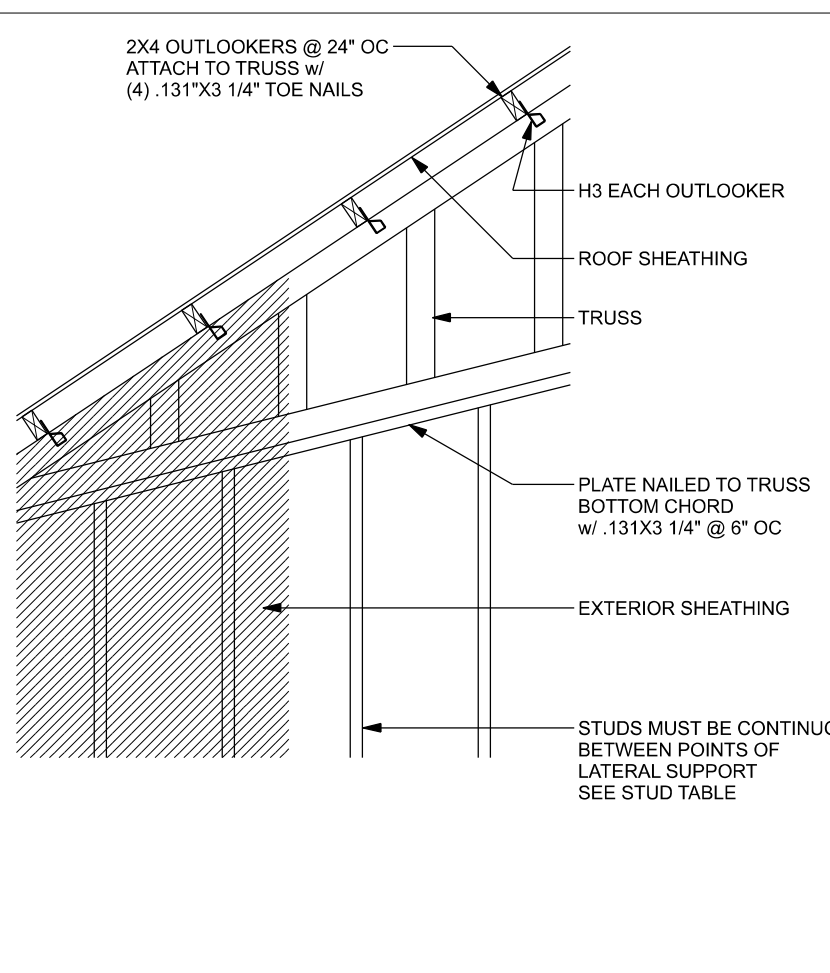
(TYP.) CORNER FRAMING
WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	12" oc
120 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2.38" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2.38" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

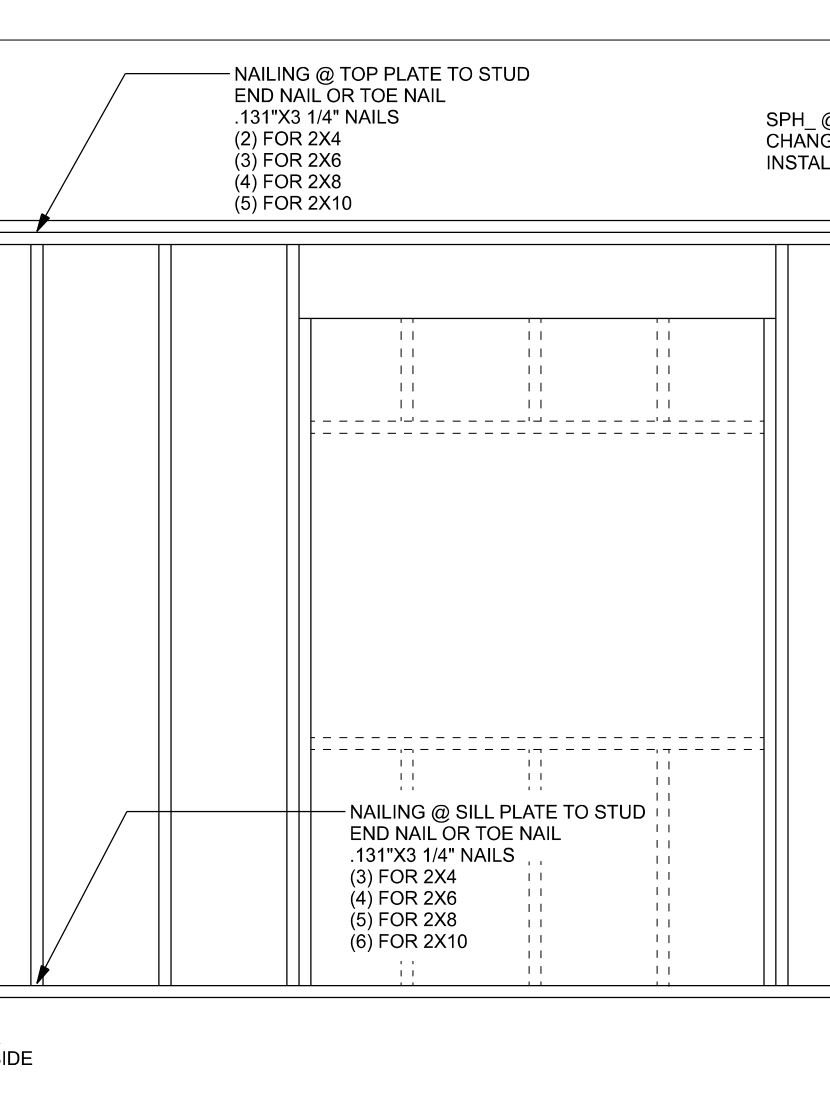
HEADER SCREWS TABLE				
Top Connection		Bottom Connection		
Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations			
Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations			

HEADER STRAP TABLE				
Top Connection		Bottom Connection		
5/ LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations			
(2) MSTA24, 18-10d header to jacks	DTT22			
(2) MSTA24, 18-10d header to jacks	DTT22			
(2) MSTA24, 18-10d header to jacks	HTT4			

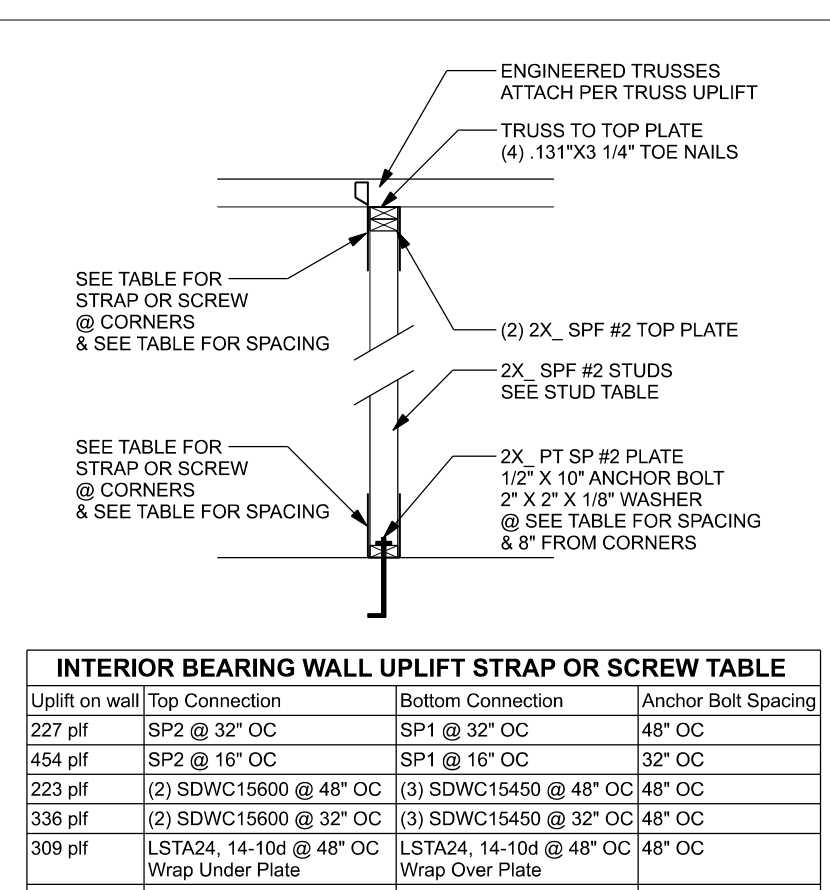
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2			BASED ON WFCM TABLE A3.3.288
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"

FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

TYPICAL HEADER STRAPING OR SCREWS DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



INTERIOR BEARING WALL UPLIFT STRAP OR SCREW TABLE

Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
----------------	----------------	-------------------	---------------------

227 pif	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
---------	--------------	--------------	--------

454 pif	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
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223 pif	(2) SDWC15600 @ 48" OC	(3) SDWC15450 @ 48" OC	48" OC
---------	------------------------	------------------------	--------

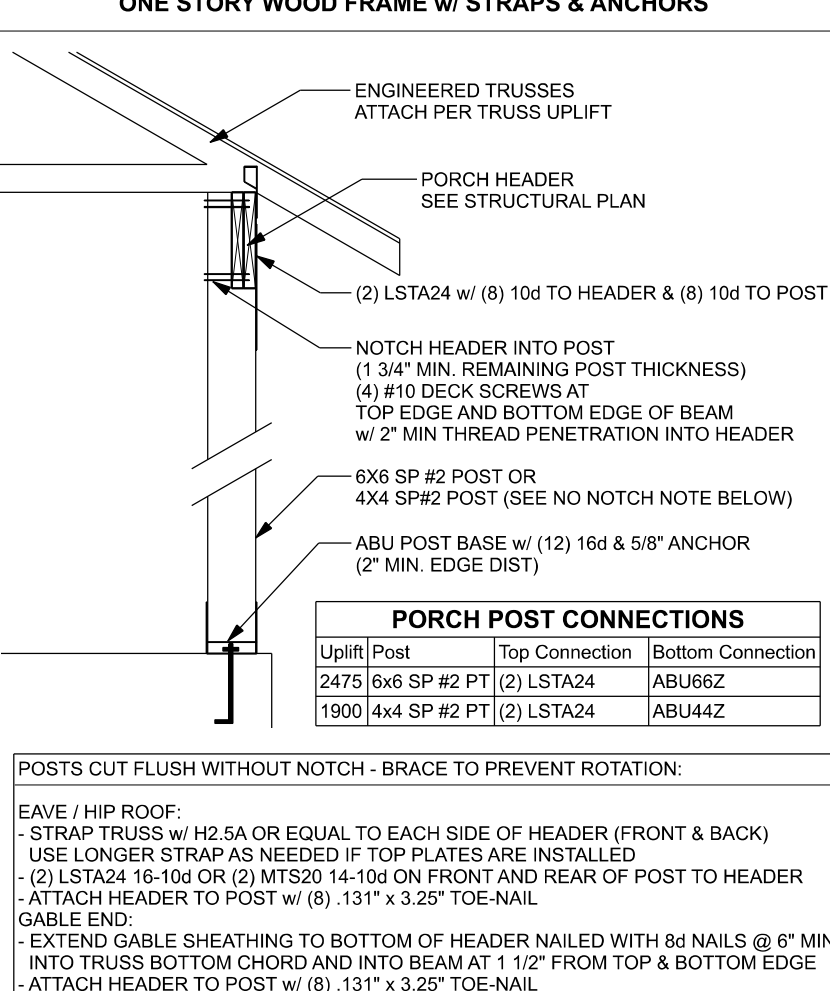
336 pif	(2) SDWC15600 @ 32" OC	(3) SDWC15450 @ 32" OC	48" OC
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309 pif	LSTA24, 14-10d @ 48" OC	LSTA24, 14-10d @ 48" OC	48" OC
---------	-------------------------	-------------------------	--------

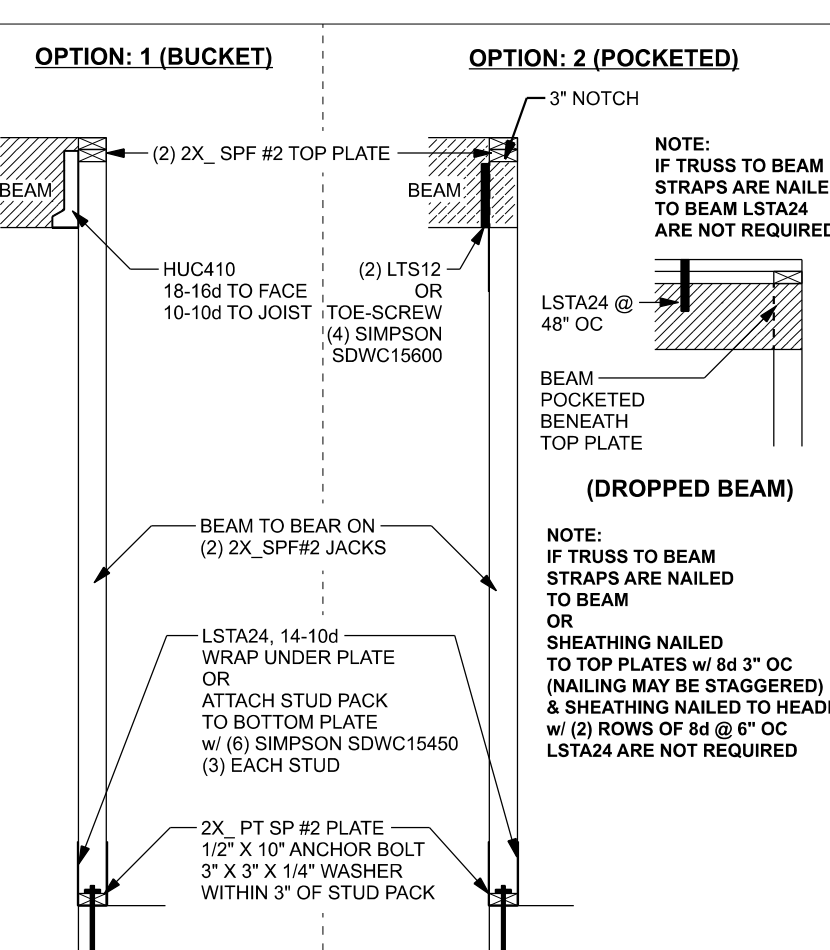
465 pif	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC
---------	-------------------------	-------------------------	--------

	Wrap Under Plate	Wrap Over Plate	
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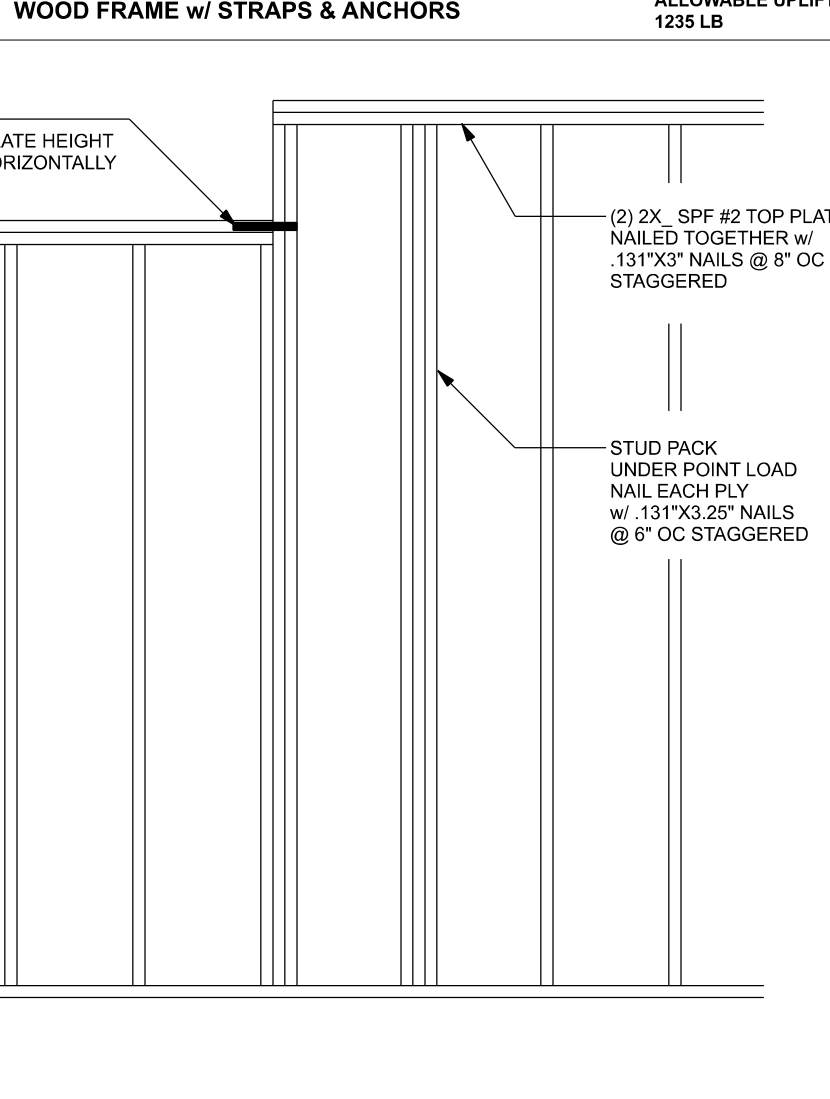
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



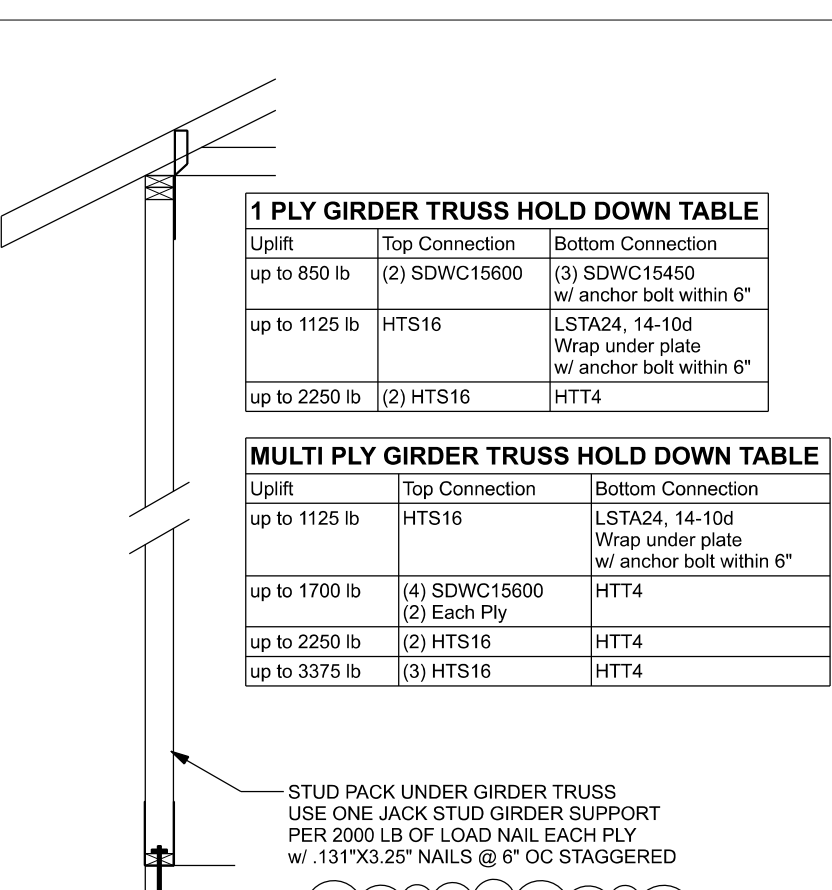
(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



1 PLY GIRDER TRUSS HOLD DOWN TABLE

Uplift	Top Connection	Bottom Connection
--------	----------------	-------------------

up to 850 lb	(2) SDWC15600	(3) SDWC15450 w/ anchor bolt within 6"
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up to 1125 lb	HTS16	LSTA24, 14-10d Wrap under plate w/ anchor bolt within 6"
---------------	-------	--

up to 2250 lb	(2) HTS16	HTT4
---------------	-----------	------

up to 1700 lb	(4) SDWC15600	HTT4
---------------	---------------	------

up to 2250 lb	(2) HTS16	HTT4
---------------	-----------	------

up to 3375 lb	(3) HTS16	HTT4
---------------	-----------	------

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

GRADE & SPECIES TABLE	Fb	E
-----------------------	----	---

2x8	SP #2	925	1.4
-----	-------	-----	-----

2x10	SP #2	800	1.4
------	-------	-----	-----

2x12	SP #2	750	1.4
------	-------	-----	-----

GLB	24F-V3 SP	2600	1.9
-----	-----------	------	-----

LVL	TIMBERSTRAND	1700	1.7
-----	--------------	------	-----

LVL	MICROLAM	2950	2.0
-----	----------	------	-----

PSL	PARALAM	2900	2.0
-----	---------	------	-----

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)



DESIGN CRITERIA & LOADS:
BUILDING CODE 7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020) CODE FOR DESIGN LOADS ASCE 7-16 WINDLOADS BASIC WIND SPEED 130 MPH WIND EXPOSURE (BUILDER MUST FIELD VERIFY) C TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY) I RISK CATEGORY II ENCLOSURE CLASSIFICATION ENCLOSED INTERNAL PRESSURE COEFFICIENT 0.18 ROOF ANGLE 7-45 DEGREES MEAN ROOF HEIGHT 30 FT C&C DESIGN PRESSURES SEE TABLE FLOOR LOADING ROOMS OTHER THAN SLEEPING ROOMS 40 PSF LIVE LOAD SLEEPING ROOMS 30 PSF LIVE LOAD ROOF LOADING FLAT OR < 4:12 20 PSF LIVE LOAD 4:12 TO < 12:12 16 PSF LIVE LOAD 12:12 & GREATER 12 PSF LIVE LOAD SOIL BEARING CAPACITY 1500 PSF FLOOD ZONE THIS BUILDING IS NOT IN THE FLOOD ZONE

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
-----------	------------	-----------------	----------	-----------------

805	505	SDWC15600	-	-
-----	-----	-----------	---	---

415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
-----	-----	----	---------------	---------------

615	540	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"
-----	-----	-------	---------------	---------------

1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
------	------	------	-------------	-------------

720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
-----	-----	----------	-------------	-------------

1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
------	-----	----------	-------------	-------------

1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
------	------	----------	--------------	--------------

Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
-----------	------------	------------	---------------	-----------------

1235	1235	LSTA21	8-10d	8-10d
------	------	--------	-------	-------

1640	1455	MSTA24	9-10d	9-10d
------	------	--------	-------	-------

1030	1030	CS20	7-10d	7-10d
------	------	------	-------	-------

Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
-----------	------------	-----------------	---------	----------

585	535	SP1	6-10d	4-10d
-----	-----	-----	-------	-------

1065	905	SP2	6-10d	6-10d
------	-----	-----	-------	-------

771	771	LSTA24	10-10d	wrap under or over plate
-----	-----	--------	--------	--------------------------

1235	1235	LSTA24	14-10d	wrap under or over plate
------	------	--------	--------	--------------------------

Uplift SP	Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor
-----------	------------	---------------------	----------------	--------

1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
------	------	-------	-------------------	-------------------

4235	3640	HTT4	18-16d x 12"	1/2"x12" Titen HD
------	------	------	--------------	-------------------

Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
-----------	------------	-----------------	----------------	--------

1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
------	------	-------	-------------------	------------------

4235	3640	HTT4	18-16d x 12"	1/2"x12" Titen HD
------	------	------	--------------	-------------------

Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
-----------	------------	-----------------------	---------	--------

1900	ABU4Z	12-16d	5/8"x12" Drill & Epoxy	
------	-------	--------	------------------------	--

2475	ABU6Z	12-16d	5/8"x12" Drill & Epoxy	
------	-------	--------	------------------------	--

Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
-----------	------------	-------------------	---------	--------

1900	ABU4Z	12-16d	5/8"x7" Drill & Epoxy	
------	-------	--------	-----------------------	--

2475	ABU6Z	12-16d	5/8"x7" Drill & Epoxy	
------	-------	--------	-----------------------	--

CONNECTOR TABLE

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
-----------	------------	-----------------	----------	-----------------

805	505	SDWC15600	-
-----	-----	-----------	---

1	AC1039, 10-2 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls $F_m = 1500$ psi
2.1	Mortar	ASTM 270-2, Type N, UNO
2.2	Grout	ASTM C 478, admixtures required approval
3	CMU standard	ASTM C 90-02, Normal weight, Hollow, minimum surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, S 5292 111-11
2.4	Reinforcing bars, #11	ASTM A615, Grade 40, F_y = 40 ksi, Lap splices min 40 bar dia, (25" for #8)
2.4F	Coating for corrosion protection	Anchor, sheet metal nails completely embedded in mortar or grout. ASTM A925, Class G60, 0.60 oz/ft ² or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture, wire, nails, anchors, sheet metal nails not completely embedded in mortar or grout. ASTM A163, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints. Not detailed on project drawings.

CONCRETE SLAB

3" MIN. COVER (TYP.)

20' W X 10' D POURED CONCRETE STRIP FOOTING w/ (2) #5 REBAR CONTINUOUS

NOTE: FOR STEM WALL FOUNDATIONS OVER 5 COURSES IN HEIGHT THE SLAB IS REQUIRED TO BE ATTACHED TO THE STEM WALL @ BOND BEAM w/ 2' X 2' #5 CORNER REBARS

(1) LEG EXTENDING INTO SLAB & (1) LEG LAPPED w/ THE HORIZONTAL BOND BEAM REBAR SPACED THE SAME AS VERTICAL REBAR

(1) #5 CONTINUOUS IN HEADER-BLOCK BOND BEAM @ SLAB EDGE INTERSECTION w/ STEMWALL

#5 VERT. REBAR w/ STD. HOOK BOTTOM IN FOOTING & STD. HOOK TOP IN BOND BEAM @ EACH CORNER & 96" OC

8X8X16, RUNNING BOND, CMU STEM WALL, MAX 5 COURSES (SEE SPECIAL REINFORCEMENT TABLE FOR MORE THAN 5 COURSES)

TALL STEM WALL TABLE:

The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

SCALE: 1/4" = 1'-0"

F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"

FN -1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, PE IS NOT RESPONSIBLE FOR ANY OF THE ERRORS IN THIS PLAN.
FN -2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN -3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1/4" LAID WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY 11020 BARRIER W/ 6MPS SEALED W/ POLY TAPE OVER TERMITE TREATMENT & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)

William & Dasashia Murphy Res.

PROJECT ADDRESS:
6148 NW Lake Jeffery
Lake City, FL 32024

FL PE 53915
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Mark Disosway PE on digital signature date.
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Disoway
2023-05-03 13:
43:36

DIMENSIONS:
 Stated dimensions supercede scaled
 dimensions. Refer all questions to
 Mark Disosway, P.E. for resolution.
 Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

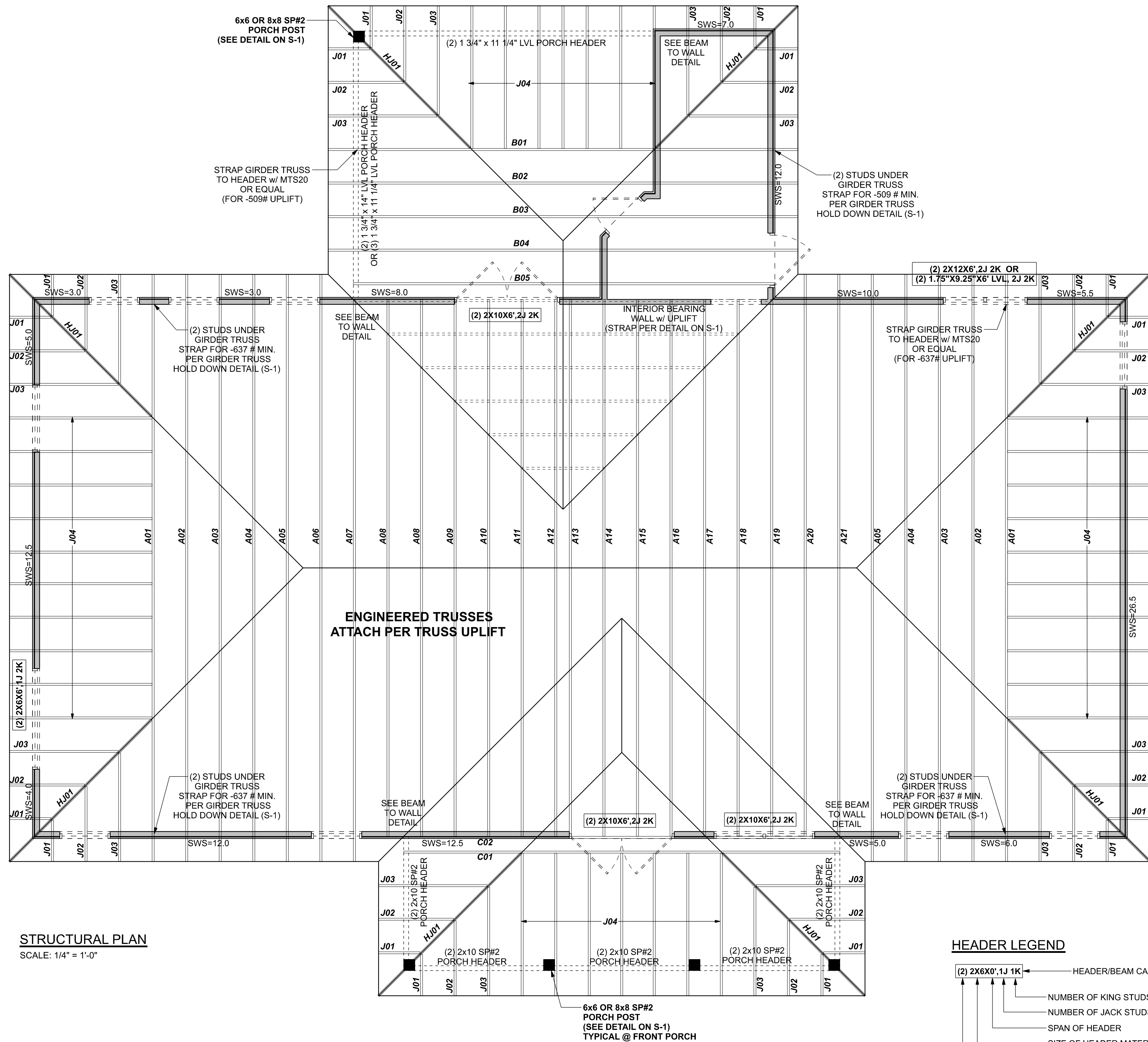
LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
163 SW Midtown Place
Suite 103
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386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
230538

S-2

OF 3 SHEETS



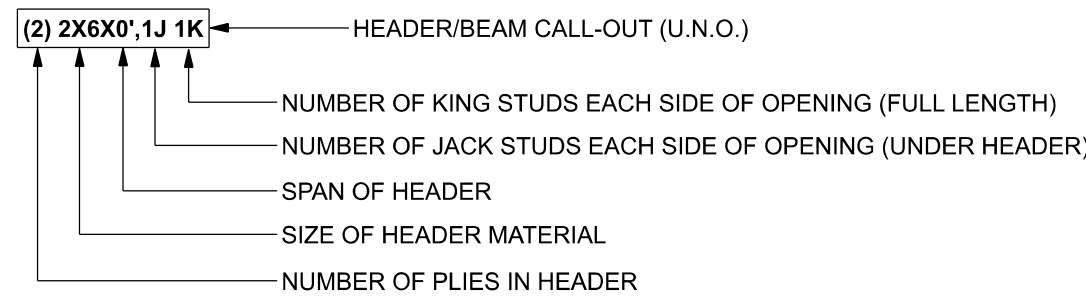
STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIERMENTS)	
SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL	TRANSVERSE	LONGITUDINAL
ACTUAL	23760 LBF	28512 LBF
REQUIRED	13250 LBF	11197 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, WB HOWLAND
JOB #22-8711

Bryan Zecher Construction

William & Dasashia Murphy Res.

PROJECT ADDRESS:
6148 NW Lake Jeffery
Lake City, FL 32024

FL PE 53915
This item has been digitally signed and sealed by Mark Disoway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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2023-05-03 13:43:56

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wood engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
230538

S-3
OF 3 SHEETS