

NOTES

1) Wind: ASCE 7-02: 11 mph (3-second gust); h=20ft; TCD=1.60 plate grip DOL=1.60 psl; Category II; Exp B; enclosed, MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed. Lumber DOL=1.60 psl; BCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed, MWFRS gable end zone and C-C Exterior(2) reactions specified.

2) All bearings are assumed to be SYP No 2

3) Refer to girder(s) for russ to russ connections.

4) Provide mechanical connection (by others) of russ to bearing plate capable of withstanding 43 lb uplift at joint 3, 18.1 lb uplift at joint 2 and 33 lb uplift at joint 4.

LOAD CASE(S) Standard

Ply

JACK

L2224665



NOTES

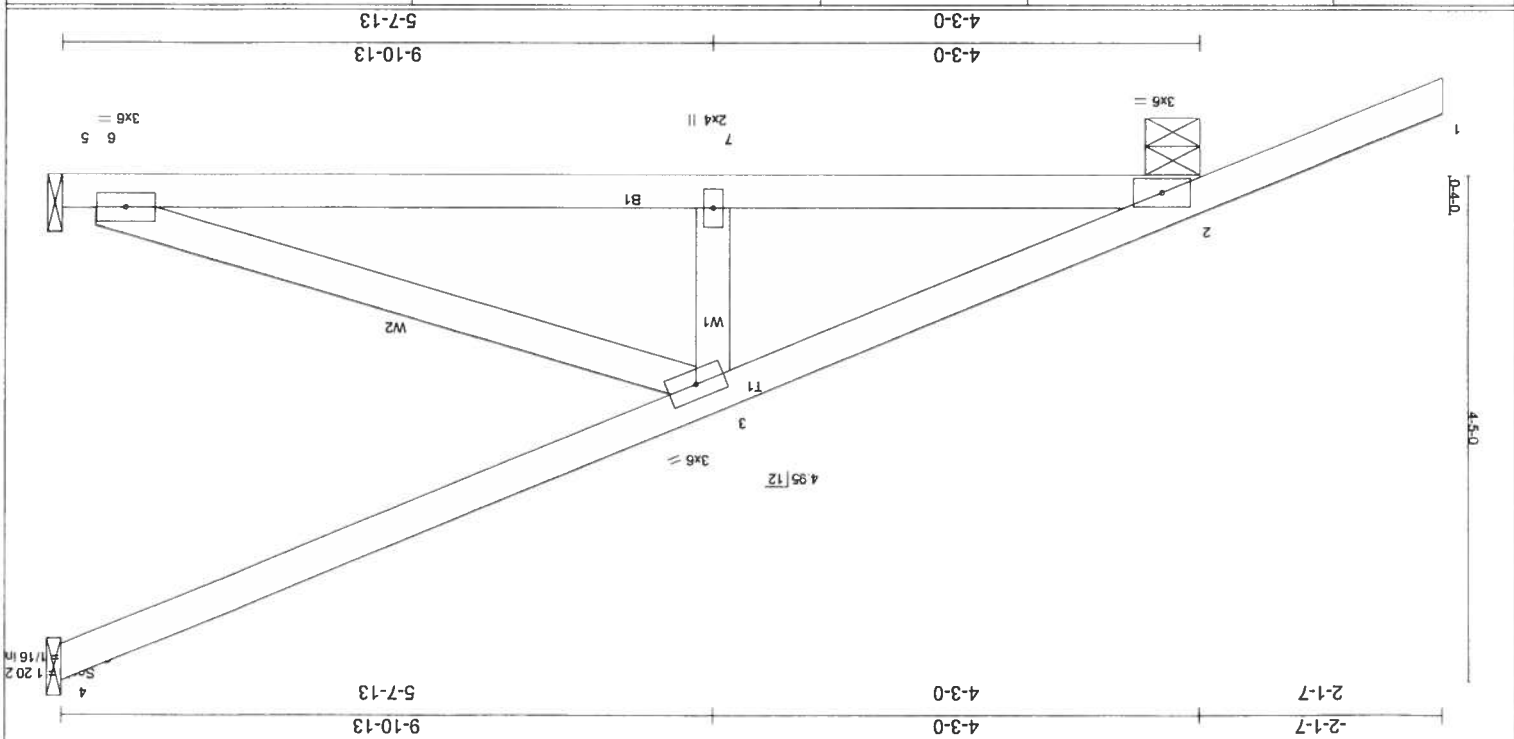
(1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDF=3.0psf; Category II; Exp B; enclosed, MWFRS gable end zone and C-C-Exterior(2) zone; porch left and right exposed. Lumber DOL=1.60 plate gfp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

(2) All bearings are assumed to be SYP No 2

(3) Refer to girder(s) for truss to truss connections

(4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 1 and 56 lb uplift at joint 4.

Standard LOAD CASE(S)



LOADING (psi)	TCL	20.0	SPACING	2.0-0	CS1	0.60	DEFL	in (loc)	5	n/a	Weight: 45 lb
	TCDL	7.0	Plates Increase	1.25	BC	0.55	Vert(TL)	-0.09	6-7	180	PLATES GRIP MT20 244/190
	BCL	10.0	Lumber Increase	1.25	TC	0.60	Vert(TL)	-0.16	> 728	180	
	BCDL	5.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.01	n/a	n/a	
			Code FBC2004/TP12002								

TOP CHORD	2 X 4 SYP No.2	BRACING	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 cc purlins.
TOP CHORD	2 X 4 SYP No.2			
BOT CHORD	2 X 4 SYP No.2			
WEBS	2 X 4 SYP No.3			

REACTIONS (lb/size) 4=266/Mechanical, 2=486/5-11, 5=388/Mechanical
Max Horiz 2=297(load case 4)
Max Uplift 4=-247(load case 4), 2=-329(load case 4), 5=-205(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-824/328, 3-4=-126/74
BOT CHORD 2-7=-540/751, 6-7=-540/751, 5-6=0/0
WEBS 3-7=-91/199, 3-6=-791/569

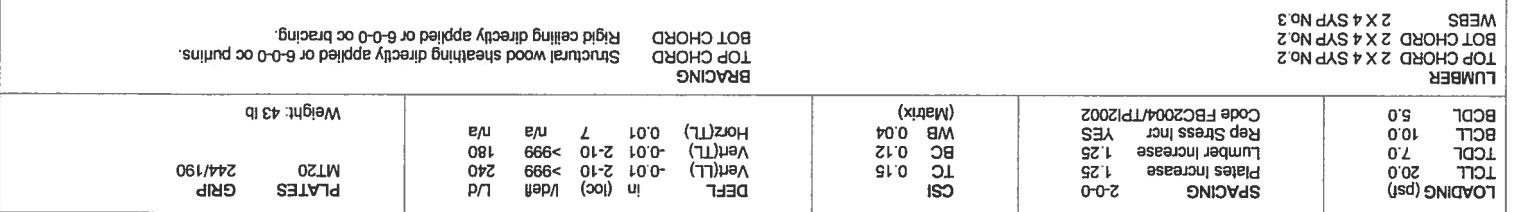
JOINT STRESS INDEX
 $2 = 0.70, 3 = 0.24, 6 = 0.22$ and $7 = 0.15$

1) Notes
WIND: ASCE 7-02: 110 mph (3-second gust), h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No 2
3) Refer to girders(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 4, 329 lb uplift at joint 2 and 205 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Trapezoidal Loads (plf)
Vent: 1-2=-54
Vent: 2-3(F)=25, B=25)-10-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-10-5=-74(F=-22, B=-22)

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=54
Trapezoidal Loads (plf)
Vert: 2=3(f)=25, B=25(-)to 4=-134(f)=40, B=40

Trapezoidal Loads (pdf)
Vert: 2=-3(F=25, B=25)-10-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-10-5=-74(F=-22, B=-22)



BRACING	Structural wood sheathing directly applied or 6-0-0 oc purlins.
TOP CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
BOT CHORD	

Max UpLift1=15(load case 6), 7=-18(load case 6), 10=-172(load case 5), 8=-158(load case 6)
Max UpLift2=113(load case 4)
Max Grav1=90(load case 9), 7=90(load case 10), 10=376(load case 1), 8=376(load case 1)

JOINT STRESS INDEX
2 = 0.27, 3 = 0.09, 4 = 0.15, 5 = 0.09, 6 = 0.27, 8 = 0.09, 9 = 0.02 and 10 = 0.09

NOTES

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-02: 11 mph (3-second gust); $h=20ft$; $TCDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber $DOL=1.60$ plate grip $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) All bearings are assumed to be STP No. 2

4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSIT/P 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 18 lb uplift at joint 7, 172 lb uplift at joint 10 and 158 lb uplift at joint 8

6) SEE MITTEL STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

LOAD CASE(S) Standard



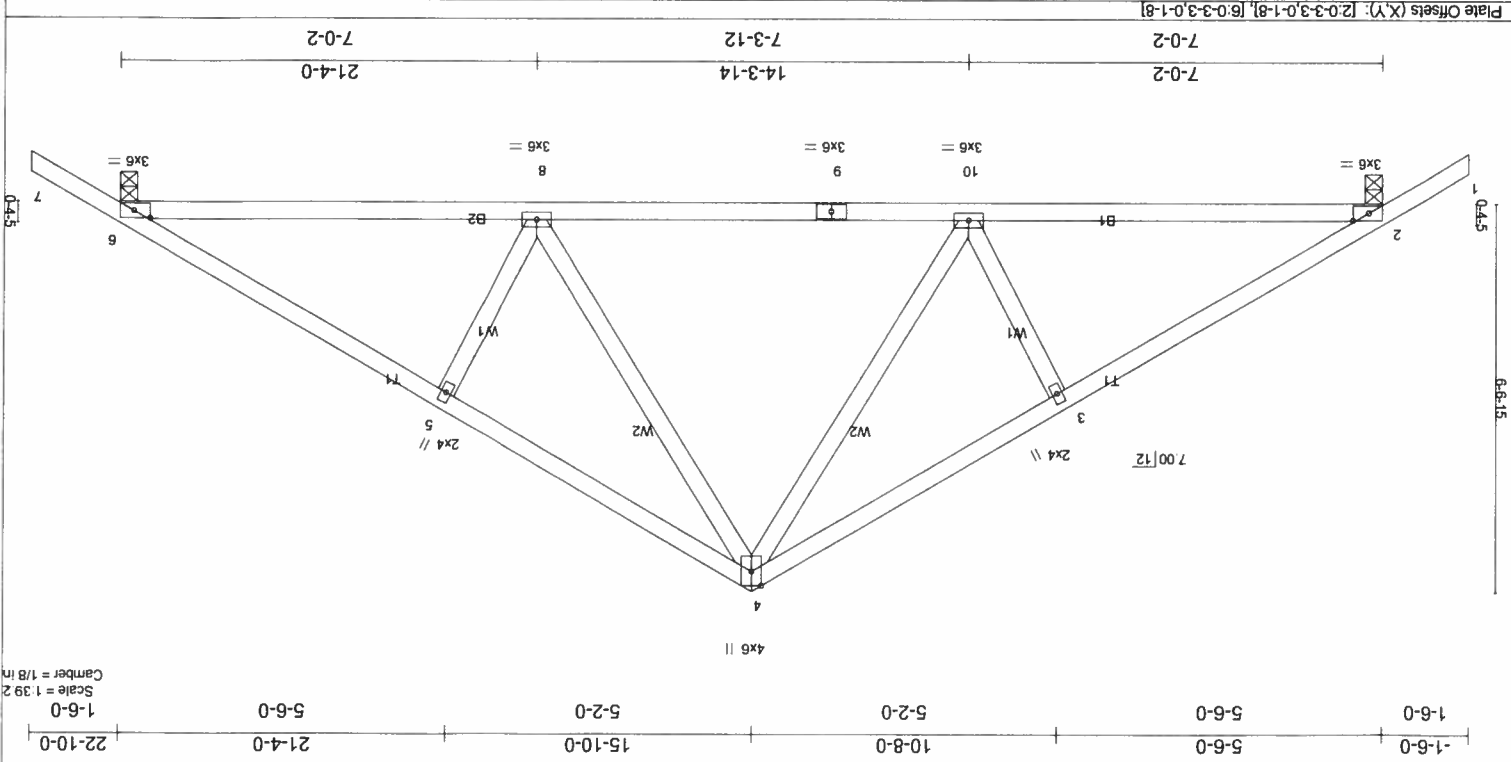
TOP CHORD	2 X 4 SYP No.2
LUMBER	
BOT CHORD	2 X 4 SYP No.2
BRACING	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
Structural wood sheathing directly applied or 6-0-0 cc purlins.	
Rigid ceiling directly applied or 6-0-0 cc bracing.	

FORCES (lb) - Maximum Compression/Maximum Tension

NOTES

- (1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft. TCDL=4.2psf, BCDL=3.0psf, Category II. Exp B; enclosed. MWFRS gable end zone and C-C Interior (1) zone. Lumber DCL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) All bearings are assumed to be SYP No. 2
- (4) Bearing all joint(s), 1, 7 considers parallel to grain value using ANSITP 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1, 19 lb uplift at joint 7, 164 lb uplift at joint 10 and 150 lb uplift at joint 8
- (6) SEE MITAK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

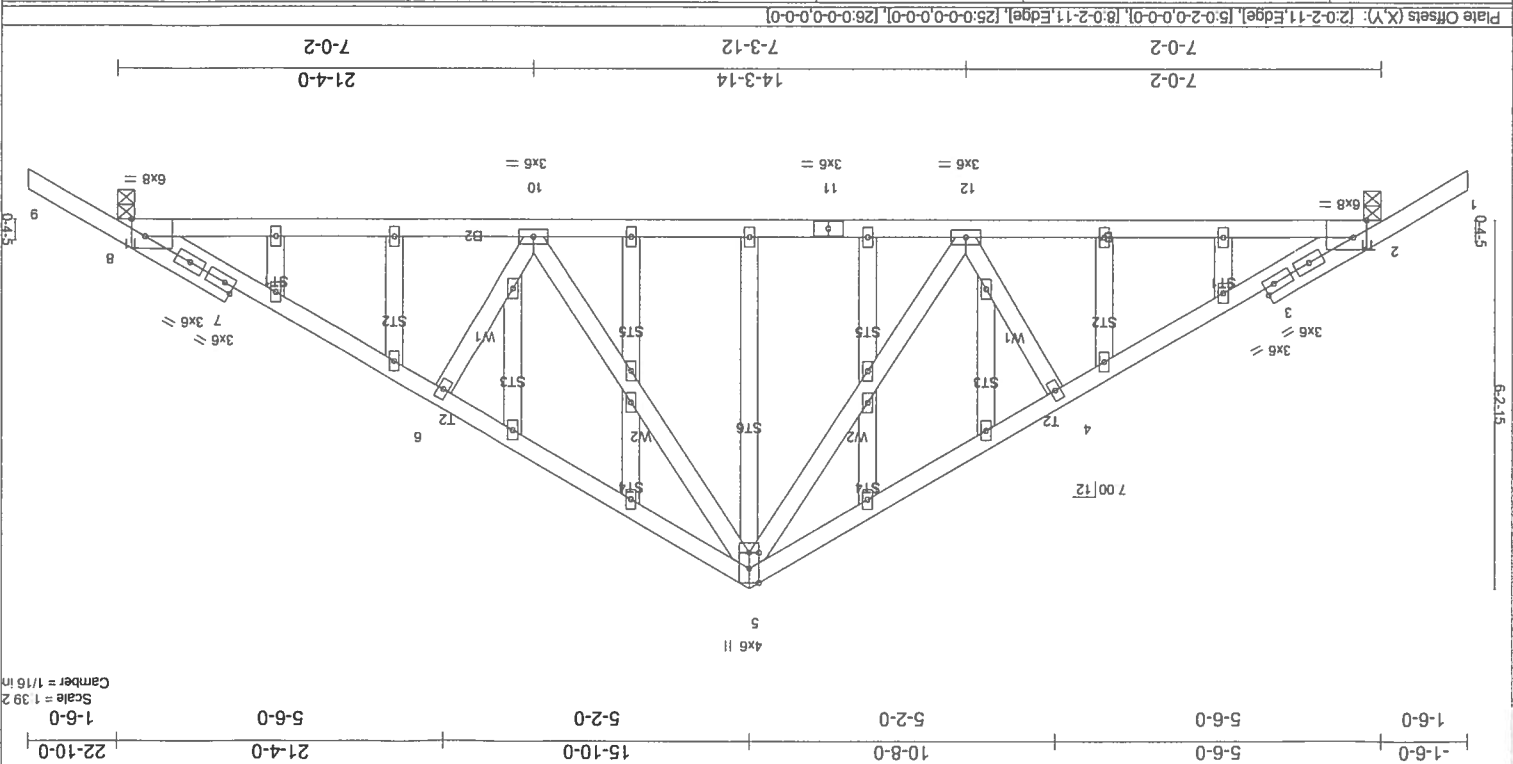
LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL	TOP CHORD	BRACING	Weight: 105 lb
TCLL 20.0	Plates Increase 1.25	TC 0.24	Ver(TL) -0.23	TOP CHORD	Structural wood sheathing directly applied or 4-7-8 oc purlins.	PLATES MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.84	Ver(TL) -0.36	BOT CHORD	Rigid ceiling directly applied or 9-2-14 oc bracing.	GRIP
BCLL 10.0	Rep Stress Incr NO	WB 0.27	Horz(TL) 0.04			
BCDL 5.0	Code FBC2004/TP12002	(Matrix)	In (loc) 6			

REACTIONS (lb/size)	FORCES (lb)	TOP CHORD	WEBS	JOINT STRESS INDEX	NOTES
Max Horz 2=-222(load case 3)	Max Uplift 2=-448(load case 5), 6=-448(load case 6)	1-2=0/40, 2-3=-1744/557, 3-4=-1613/599, 4-5=-1744/558, 6-7=0/40	3-10=-235/234, 4-10=-284/750, 4-8=-284/750, 5-8=-235/234	2 = 0.75, 3 = 0.34, 4 = 0.47, 5 = 0.34, 6 = 0.75, 8 = 0.59, 9 = 0.72 and 10 = 0.59	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02, 10 mph (3-second gust), h=20ft, TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) 3) All bearings are assumed to be SYP No. 2 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 448 lb uplift at joint 2 and 448 lb uplift at joint 6. 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 7-10=-30, 8-10=-80(F=-50), 6-8=-30



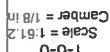
LOADING (psf)	SPACING	CSI	DEFL	Ver(TL)	Horz(TL)	BRACING
TCLL 20.0	Plates Increase 1.25	TC 0.46	in (loc)	-0.11	0.04	TOP CHORD
TCDL 7.0	Lumber Increase 1.25	BC 0.55	Ver(TL)	8-10	0.17	Structural wood sheathing directly applied or 4-4-3 oc purlins.
BCLL 10.0	Rep Stress Incr NO	WB 0.21	Horz(TL)	8-10	0.04	Bot CHORD
BCDL 5.0	Code FBC2004/TP12002	(Matix)				

LUMBER	TOP CHORD	WEBS	OTHERS
2 X 4 SYP No.2	2 X 4 SYP No.2	2 X 4 SYP No.3	2 X 4 SYP No.3
Bot CHORD	Top CHORD	WEBS	OTHERS

REACTIONS	FORCES	JOINT STRESS INDEX
(lb/size) 2=1094/0-3-8, 8=1094/0-3-8	(lb) - Maximum Compression/Maximum Tension	
Max Horz 2=-210(load case 3)	1-2=0/47, 2-3=-1647/536, 3-4=-1600/548, 4-5=-1499/567, 5-6=-1499/567, 6-7=-1600/548, 7-8=-1647/536, 8-9=0/47	
Max Uplift 2=-427(load case 5), 8=427(load case 5)		

2 = 0.53, 3 = 0.00, 3 = 0.37, 3 = 0.34, 4 = 0.56, 4 = 0.34, 5 = 0.53, 5 = 0.55, 6 = 0.34, 7 = 0.00, 7 = 0.56, 7 = 0.37, 8 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.34, 44 = 0.34, 45 = 0.34, 46 = 0.34, 47 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.34, 51 = 0.34, 52 = 0.34, 53 = 0.34, 54 = 0.34, 55 = 0.34, 56 = 0.34, 57 = 0.34, 58 = 0.34, 59 = 0.34, 60 = 0.34, 61 = 0.34, 62 = 0.34, 63 = 0.34, 64 = 0.34, 65 = 0.34, 66 = 0.34, 67 = 0.34, 68 = 0.34, 69 = 0.34, 70 = 0.34, 71 = 0.34, 72 = 0.34, 73 = 0.34, 74 = 0.34, 75 = 0.34, 76 = 0.34, 77 = 0.34, 78 = 0.34, 79 = 0.34, 80 = 0.34, 81 = 0.34, 82 = 0.34, 83 = 0.34, 84 = 0.34, 85 = 0.34, 86 = 0.34, 87 = 0.34, 88 = 0.34, 89 = 0.34, 90 = 0.34, 91 = 0.34, 92 = 0.34, 93 = 0.34, 94 = 0.34, 95 = 0.34, 96 = 0.34, 97 = 0.34, 98 = 0.34, 99 = 0.34, 100 = 0.34

NOTES
 (1) Unbalanced roof live loads have been considered for this design.
 (2) Wind: ASCE 7-02: 110mph (3-second gust): h=20ft, TCDL=4.2psf, BCDL=3.0psf, Category II, Exp B: enclosed, MWFRS gable end zone and C-C Exterior(2) zone: Lumber DOL=1.60, This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 (3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mittek "Standard Gable End Detail".
 (4) All plates are 2x4 MT20 unless otherwise indicated.
 (5) Gable studs spaced at 2-0-0 oc.
 (6) All bearings are assumed to be SYP No.2
 (7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 2 and 427 lb uplift at joint 8.
 (8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 LOAD CASE(S) Standard
 (1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vert: 1-5=-64(F=-10), 5-9=-64(F=-10), 2-8=-30



LUMBER	TOP CHORD	2 X 4 SYP No.2
	BOT CHORD	2 X 4 SYP No.2
WEBS		2 X 4 SYP No.3
BRACING	TOP CHORD	
	BOT CHORD	
WEBS		
	1 Row at midpt	4-11, 6-11
	Structural wood sheathing directly applied or 3-8-7 oc purlins.	
	Rigid ceiling directly applied or 7-6-15 oc bracing.	

TOP CHORD
 1-2=0/40, 2-3=-2412/772, 3-4=-2213/6734, 4-5=-1540/600, 5-6=-1540/600, 6-7=-2212/772, 7-8=-2412/772, 8-9=0/40
 (b) Maximum Compression/Maximum Tension
 BOT CHORD
 2-12=-699/2025, 11-12=-467/1654, 10-11=-364/1654, 8-10=-522/2025
 WEBS
 3-12=-263/254, 4-12=-134/498, 4-11=-606/377, 5-11=-395/1139, 6-11=-606/377, 6-10=-135/498, 7-10=-263/254

JOINT STRESS INDEX

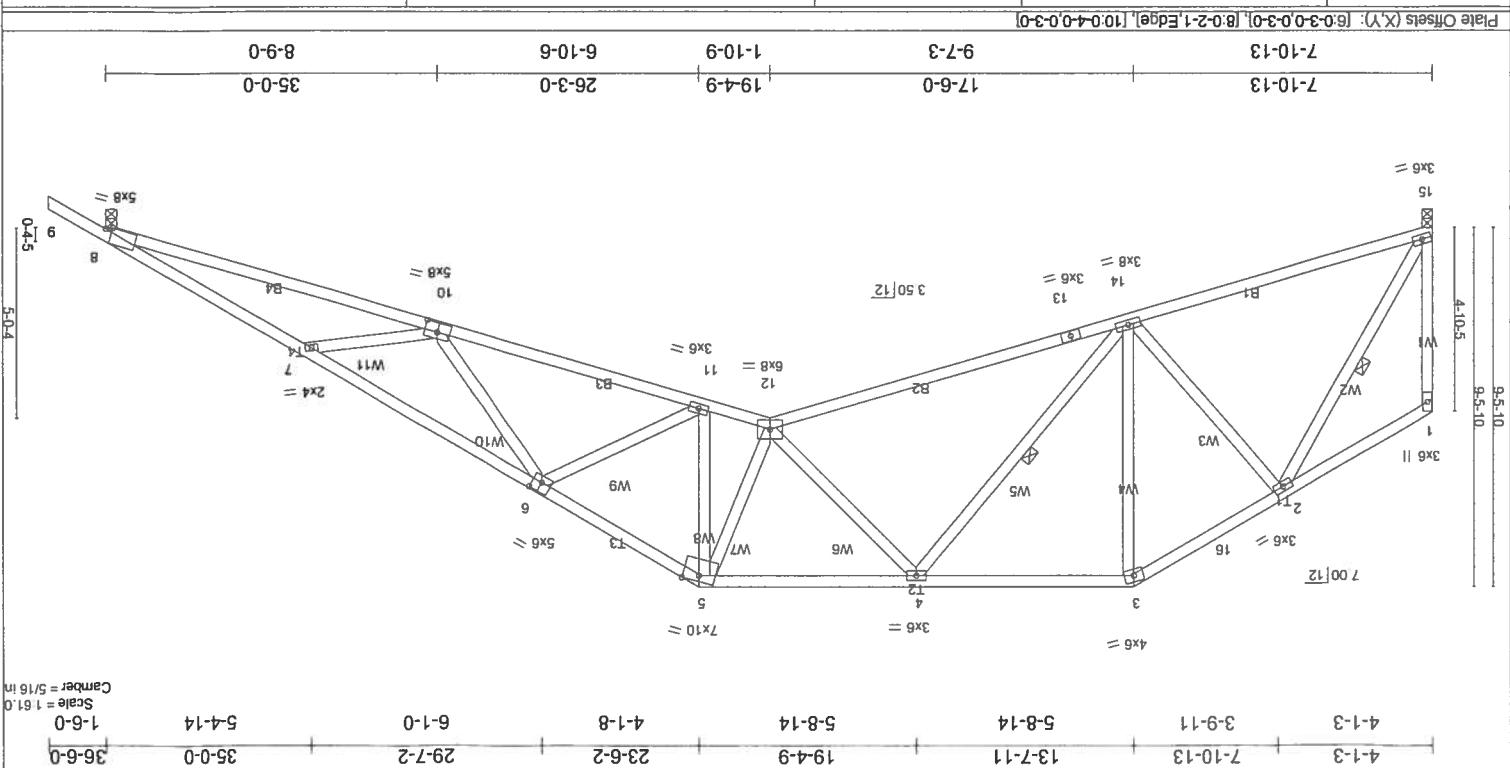
2 = 0.79, 3 = 0.40, 4 = 0.42, 5 = 0.57, 6 = 0.42, 7 = 0.40, 8 = 0.79, 10 = 0.48, 11 = 0.43 and 12 = 0.48

NOTES

LOAD CASE(S) Standard

LOAD CASE(S) Standard

[illegible]



LOADING (psi)	SPACING	2-0-0	CSI	DEFL	in (loc)	L/d	PLATES	GRIP	244/190	Weight 204 lb
TCLL	Plates increase	1.25	TC	Ver(LL)	-0.45 12-14	240	MT20			
TCDL	Lumber increase	1.25	BC	Ver(TL)	-0.75 12-14	180				
BCLL	Rep Stress Incr	NO	WB	Horz(TL)	0.53 8	n/a				
BCDL	Code FBC2004/TP12002	(Matrx)				n/a				

NUMBER	TOP CHORD	2 X 4 SYP No.2
	BOT CHORD	2 X 4 SYP No.2 "Except"
		B4 2 X 4 SYP No.1D
WEBS		2 X 4 SYP No.3
BRACING	TOP CHORD	Structural wood sheathing directly applied or 2-6-0 oc purlins, except end verticals, and 2-0-0 oc purlins (3-3-4 max.): 3-5.
	BOT CHORD	Rigid ceiling directly applied or 6-7-11 oc bracing.
		1 Row at midpt
		4-14, 2-15

REACTIONS (lb/size) 15=2162/0-3-8, 8=1608/0-3-8
Max Horiz 15=330(load case 6)
Max Uplift 15=-492(load case 5), 8=-556(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1.2-2=266/116, 2-16=-194/4339, 3-16=-173/153, 3-4=-156/4485, 4-5=-331/7716, 5-6=-338/7756, 6-7=-4357/1052, 7-8=-4690/1305, 8-9=0/39, 1-15=-419/100
BOT CHORD	14-15=472/1248, 13-14=-742/2584, 12-13=-754/2655, 11-12=-575/3029, 10-11=-604/3578, 8-10=-1026/4158
WEBS	2-14=-191/996, 3-14=-89/8616, 4-14=-1508/490, 4-12=-128/81173, 5-11=-277/488, 6-11=-607/399, 6-10=-152/536, 7-10=-274/364, 2-15=-226/1579

JOINT STRESS INDEX
1 = 0.51, 2 = 0.62, 3 = 0.58, 4 = 0.75, 5 = 0.87, 6 = 0.53, 7 = 0.34, 8 = 0.89, 10 = 0.89, 11 = 0.91, 12 = 0.38, 13 = 0.89, 14 = 0.82 and 15 = 0.82

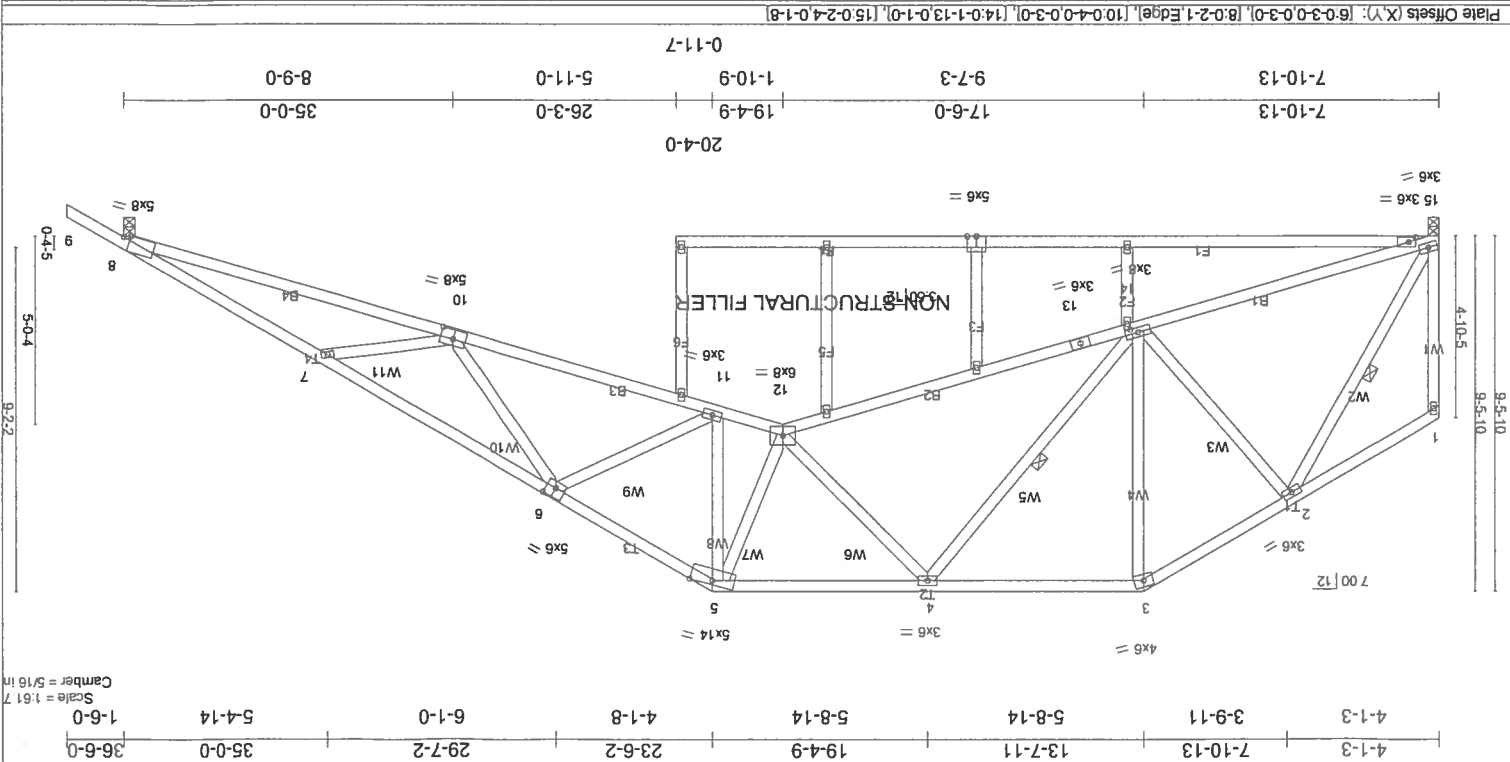
2) Unbalanced load have been considered for this design.
3) Wind: S5C 77 -10 mph (3-second gust); $\rho = 20 \text{ lb/ft}^3$; TCDF=4.2psf; BCDL=3.0psf; Category II, Exp B; enclosed; MWFRS gable end zone and C-C interior (1) zone; Lumber DOL=1.60 plate gfp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

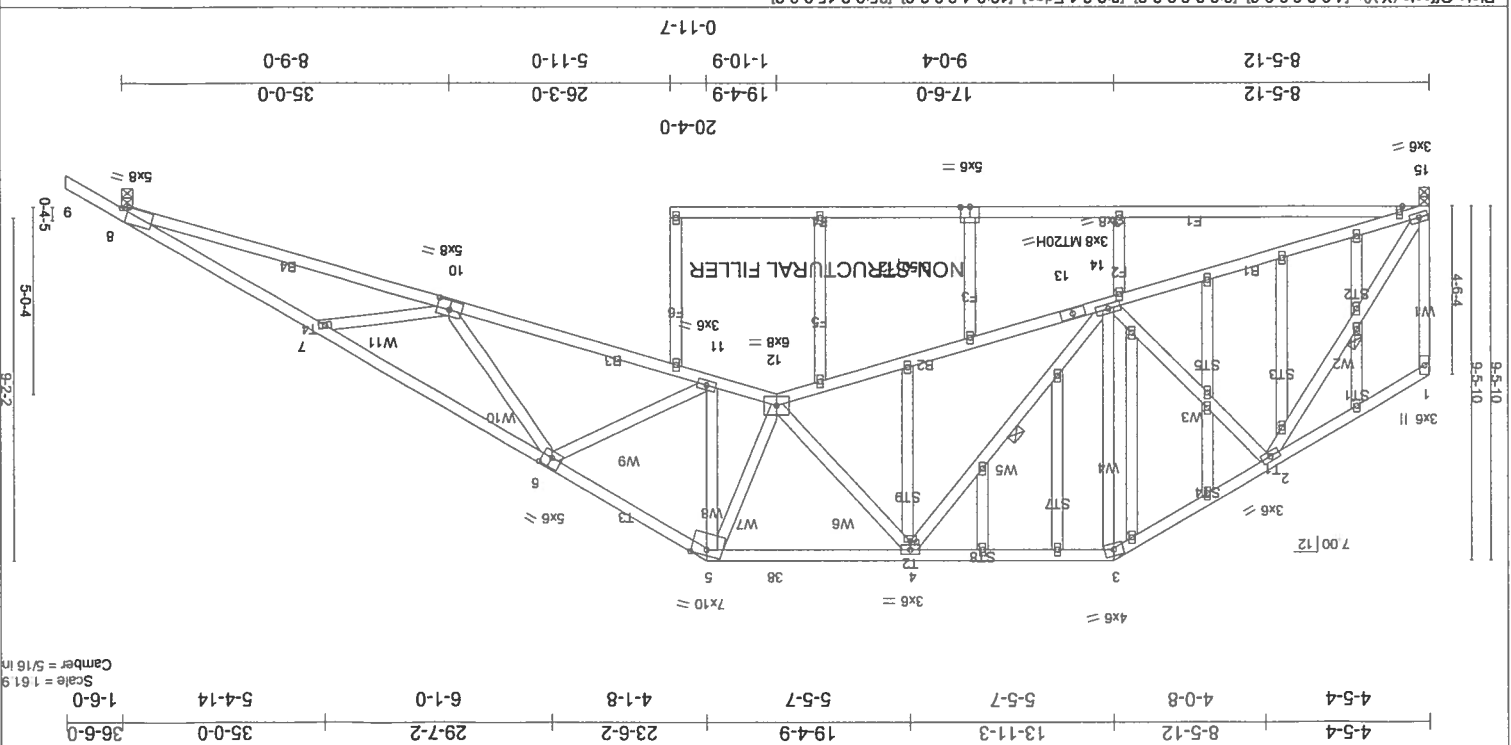
4) All bearings are assumed to be SYP No.2
5) Bearing at joints "15", 6, consider parallel to grain value using $ANSI/TFP 1$ angle to grain formula. Building designer should verify capacity of bearing surface
6) Provide mechanical connection (b) others of truss to bearing plate capable of withstanding 492 lb uplift at joint 13 and 556 lb uplift at joint 8.
7) Load case (s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC W-2, 10d nails.

LOAD CASE(S) Standard
1) Regular: Lumber increase=1.25, Plate increase=1.25
Uniform loads (k/ft)

2) C-C Vent: Lumber Increase=1.60, Plate Increase=1.64
Trapezoidal Loads (pl)
Vert: -2.12-to-16=-164
Uniform Loads (pl)
Vert: -3.16-to-28=-331, -5.5=-28, -8.9=-58, -12.15=-66, -1.3=-36, -5.8=-36, -8.9=-66
Drag -3.4=-0, -4.5=-0
Trapezoidal Loads (pl)
Vert: -1.4-to-16=61
3) MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60

3) MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60

LOAD CASE(S) Standard



LOADING (psf)	SPACING	DEFLECT	BRACING	REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS	OTHERS
BCDL 5.0	2-0-0	CSI 0.76	Weight: 302 lb	15=1937/0-3-8, 8=1709/0-3-8	Max Horiz 15=-321(load case 3)	1-2=-177/6, 2-3=-2212/793, 3-4=-1851/737, 4-38=-3718/1192, 5-38=-3718/1193, 5-6=-3777/1226, 6-7=-4729/1505, 7-8=-5051/1721, 8-9=0/39, 9-10=-1396/4477, 10-11=-1008/3921, 11-12=-773/3375, 12-13=-990/3104, 13-14=-992/3037, 14-15=-571/1361, 15-16=-269/819, 16-17=-148/616, 17-18=-178/182, 18-19=-484/1191, 19-20=-271/508, 20-21=-595/395, 21-22=-146/524, 22-23=-298/788, 23-24=-259/356, 24-25=-229/878	1 Row at midpt 14-15, 12-14	2 X 4 SYP No.3	2 X 4 SYP No.3

JOINT STRESS INDEX

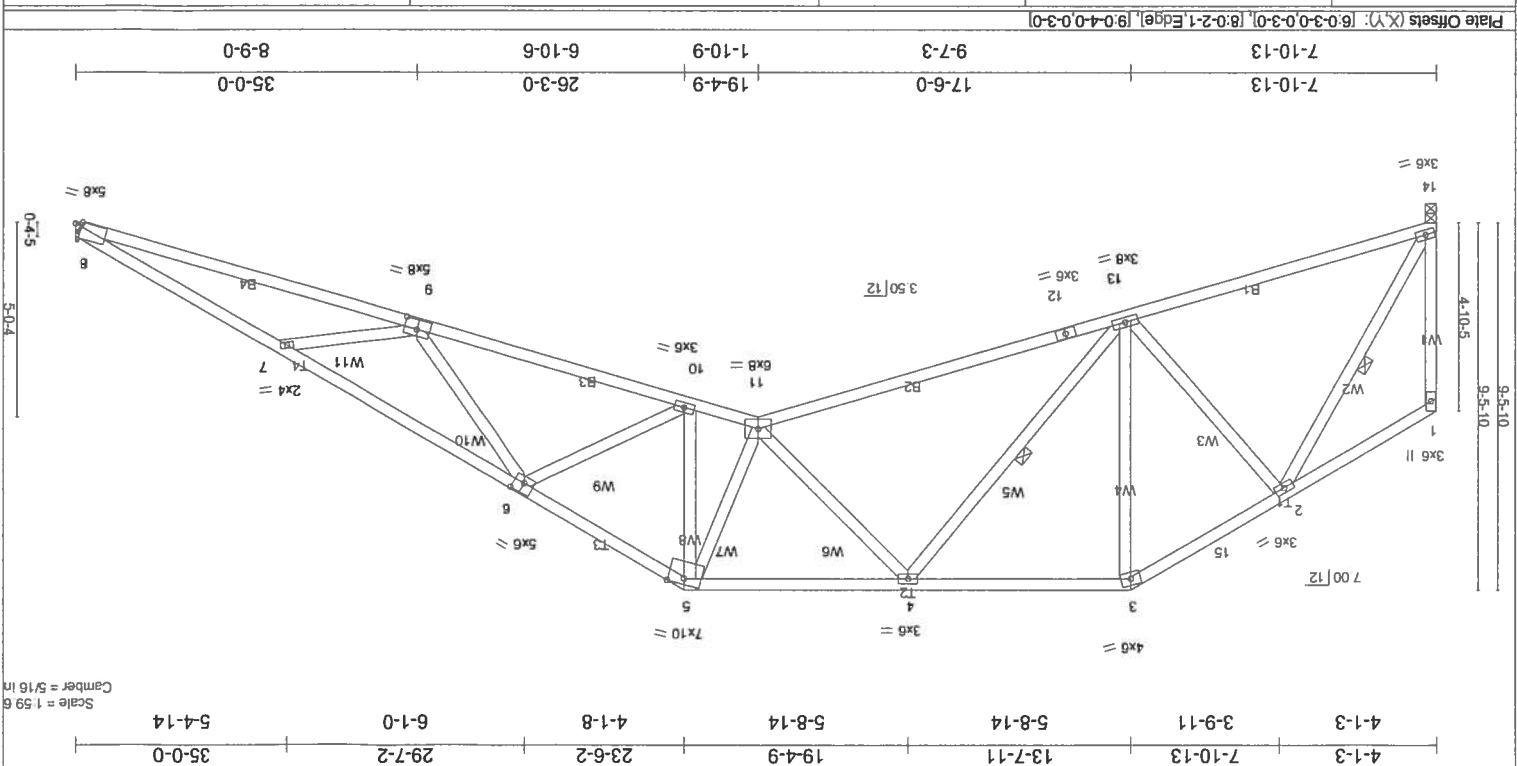
1 = 0.38, 2 = 0.65, 3 = 0.81, 4 = 0.78, 5 = 0.66, 6 = 0.97, 7 = 0.55, 8 = 0.34, 9 = 0.96, 10 = 0.98, 11 = 0.38, 12 = 0.86, 13 = 0.93, 14 = 0.97, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.00, 36 = 0.14, 37 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 110mph (3-second gust), h=20ft, TCFL=4.2psf, Category II, Exp B, enclosed, MWFRS, gable end zone and C-C Exterior.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mittek "Standard Gable End Detail".
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 2x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 2'-0" oc.
8) All bearings are assumed to be SYP No.2.
9) Bearing at joint(s) 15, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 586 lb uplift at joint 15 and 606 lb uplift at joint 8.
11) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber increase=1.25, Plate increase=1.25
Uniform Loads (psf)
Vert. 1-3=-91(F=-37), 3-38=-91(F=-37), 3-38=-54, 5-9=-54, 12-15=-30, 8-12=-30



LOADING (psi)	SPACING	CS1	DEFL	L/d	PLATES	GRIP	Weight: 202 lb
20.0	Plates increase 1.25	TC 0.67	in (loc) -0.45	i/den 240	M/T20	244/190	
TCLL	Lumber increase 1.25	BC 0.90	Ver(T/L) -0.75	>554			
TCDL	Rep Stress Incr NO	WB 0.71	Horz(T/L) 0.54	8	n/a		
BCLL							
BCDL							

TOP CHORD	2 X 4 SYP No. 2	2 Except*
BOT CHORD	2 X 4 SYP No. 1D	
WEBS	2 X 4 SYP No. 3	
TOP CHORD	Structural wood sheathing directly applied or 2-4-14 oc purlins, except end verticals, and 2-0-0 oc purlins (3-3-4 max.); 3-5.	
BOT CHORD	Rigid ceiling directly applied or 6-1-9 oc bracing.	
WEBS	1 Row at midpt	
	4-13, 2-14	

REACTIONS (lb/size) 14=2164/0-3-8, 8=1517/Mechanical
Max Horiz 14=-301(load case 3)
Max Up/lt 14=-493(load case 5), 8=-461(load case 6)

JOINT STRESS INDEX = 1 = 0.51, 2 = 0.62, 3 = 0.58, 4 = 0.75, 5 = 0.87, 6 = 0.52, 7 = 0.34, 8 = 0.89, 9 = 0.92, 10 = 0.38, 11 = 0.92, 12 = 0.89, 13 = 0.83 and 14 = 0.82

UNOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 11 mph (-3-second gust); TCDF=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed. MWFRS for reactions specified.
3) Zone: Lumber DOL=1.60 plate girth DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) All bearings are assumed to be SYP No.2.
5) Refer to girders(s) for truss to truss connections.
6) Bearing at joint(s) 14 considers parallel to grain value using ANS/ITP 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 493 lb uplift at joint 14 and 451 lb uplift at joint 8.
8) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.60
Uniform Loads (plf)
Vert: 3-15=-54, 3-5=-54, 5-8=-54, 11-14=-30, 8-11=-30
Trapezoidal Loads (plf)
Vert: 1-212-10-15=-164
Wind: 2) C-C Wind: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 3-15=28, 3-5=31, 5-8=28, 11-14=-6, 8-11=-6
Horz: 1-3=-36, 5-8=-36
Drag: 3-4=-0, 4-5=0
Trapezoidal Loads (plf)
Vert: 1-14-10-15=61
Wind: 3) MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60

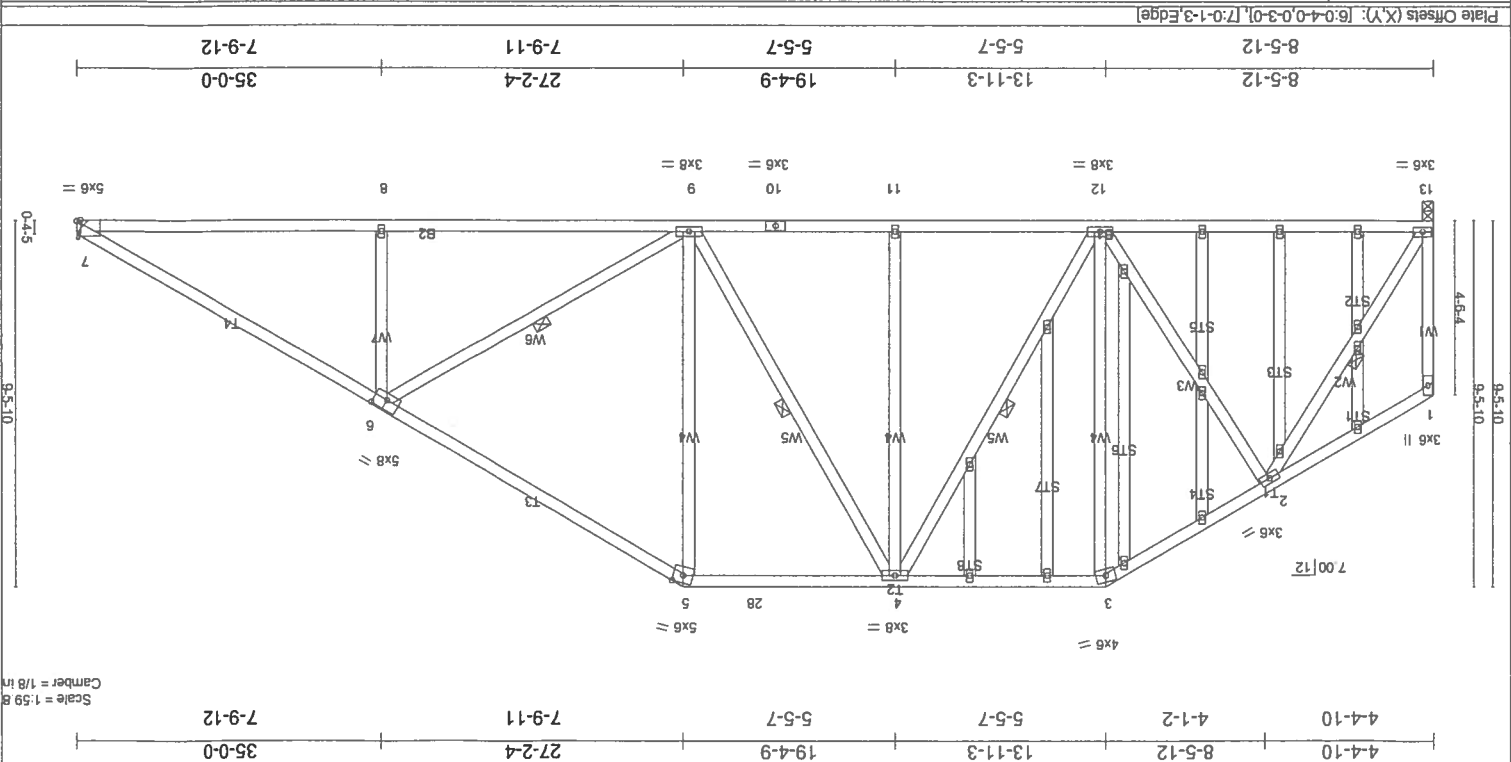
Job	Truss	T04	SPECIAL	Qty	Ply	AARON SIMQUE - GRAND ROSS
L224665				1		1

Builders FirstSource, Lake City, FI 32055

6 300 s Apr 19 2006 Mittek Industries, Inc. Thu Feb 15 07:32 36 2007 Page 2

LOAD CASE(S) Standard

- Uniform Loads (plf)
 Vert: 3-15=-12, 3-5=46, 5-8=22, 11-14=6, 8-11=6
 Drag: 3-4=-0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-7-to-15=40
 4) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-15=22, 3-5=46, 5-8=-12, 11-14=6, 8-11=-6
 Drag: 3-4=-0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-28-to-15=75
 5) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-15=38, 3-5=24, 5-8=24, 11-14=6, 8-11=6
 Drag: 3-4=-0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=32-to-15=79
 6) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-15=24, 3-5=24, 5-8=38, 11-14=6, 8-11=6
 Drag: 3-4=-0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-18-to-15=65
 7) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-15=18, 3-5=12, 5-8=12, 11-14=6, 8-11=6
 Drag: 3-4=-0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-14-to-15=33
 8) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-15=12, 3-5=12, 5-8=18, 11-14=6, 8-11=6
 Drag: 3-4=-0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-20-to-15=27
 9) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 3-15=-54, 3-5=-54, 5-8=-14, 11-14=-30, 8-11=-30
 Trapezoidal Loads (plf)
 Vert: 1=-212-to-15=-164
 10) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 3-15=-14, 3-5=-54, 5-8=-54, 11-14=-30, 8-11=-30
 Trapezoidal Loads (plf)
 Vert: 1=-172-to-15=-124



LOADING (psf)	SPACING	CS	DEFL	BRACING	REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS	OTHERS
BCDL 5.0	Code FBC2004/TP2002	WB 0.58	Horz(TL) 0.10	1 Row at midpt	Max Horiz 13=301 (load case 3)	Max Uplift 13=587 (load case 5), 7=511 (load case 6)	12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	2 X 4 SYP No. 3	2 X 4 SYP No. 3
BCIL 10.0	Rep Stress Incr NO	BC 0.83	Vert(TL) -0.31	Structural wood sheathing directly applied or 3-1-1 oc purlins, except end verticals.			12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	2 X 4 SYP No. 2	2 X 4 SYP No. 2
TCIL 7.0	Lumber Increase 1.25	TC 0.67	in (loc) 7.8				12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	2 X 4 SYP No. 2	2 X 4 SYP No. 2
TCBL 20.0	Plates Increase 1.25		L/delt 180				12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	2 X 4 SYP No. 2	2 X 4 SYP No. 2
BCDL 5.0	2-0-0		L/delt 180				12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266	2 X 4 SYP No. 3	2 X 4 SYP No. 3

REACTIONS (lb/size) 13=1939/0-3-8, 7=1618/Mechanical
 Max Horiz 13=301 (load case 3)
 Max Uplift 13=587 (load case 5), 7=511 (load case 6)
 FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266
 BOT CHORD 12-13=462/1043, 11-12=548/1686, 9-10=548/1686, 8-9=726/2262, 7-8=725/2266
 WEBS 2-12=-269/608, 3-12=-100/340, 4-12=-718/372, 4-11=0/121, 4-9=-214/310, 5-9=-148/578, 6-9=-695/413, 6-8=0/280, 2-13=-1857/32
 19 = 0.34, 20 = 0.53, 3 = 0.75, 4 = 0.60, 5 = 0.73, 6 = 0.65, 7 = 0.68, 8 = 0.34, 9 = 0.60, 10 = 0.60, 11 = 0.34, 12 = 0.64, 13 = 0.84, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34 and 27 = 0.34
 JOINT STRESS INDEX
 1 = 0.37, 2 = 0.53, 3 = 0.75, 4 = 0.60, 5 = 0.73, 6 = 0.65, 7 = 0.68, 8 = 0.34, 9 = 0.60, 10 = 0.60, 11 = 0.34, 12 = 0.64, 13 = 0.84, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34 and 27 = 0.34
 NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS for reactions specified.
 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mitek "Standard Gable End Detail".
 4) Provide adequate drainage to prevent water ponding.
 5) All plates are 2x4 MT20 unless otherwise indicated.
 6) Gable studs spaced at 2'-0" oc.
 7) All bearings are assumed to be SYP No. 2.
 8) Refer to girder(s) for truss connections.
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 587 lb uplift at joint 13 and 511 lb uplift at joint 7.
 10) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (psf)
 Vert: 1-3=-91(F=-37), 3-28=-54, 5-7=-54, 7-13=-30

[illegible]

REACTIONS (lb/size) 2=1549/0-3-8, 8=1456/Mechanical
Max Hot 3-375/ood 3350 A)

Max Upfit2=-564(load case 5), 8=-467(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

2-11=-721/2028, 10-11=-490/1658, 9-10=-388/1662, 8-9=-603/2045
3-11=-263/254, 4-11=-134/498, 4-10=-606/377, 5-10=-400/1142, 6-10=-612/385, 6-9=-153/512, 7-9=-273/

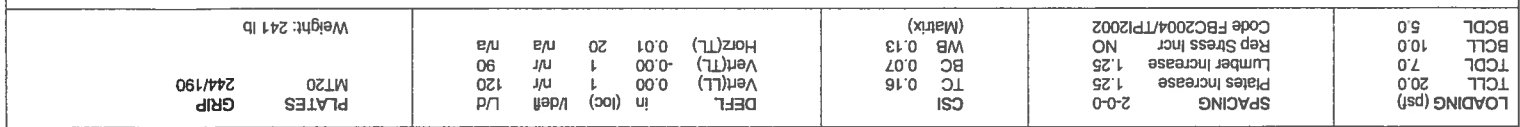
JOINT STRESS INDEX
3 = 0.79, 2 = 0.61, 1 = 0.43, 5 = 0.58, 6 = 0.43, 7 = 0.61, 8 = 0.79, 9 = 0.48, 10 = 0.43 and 11 = 0.48

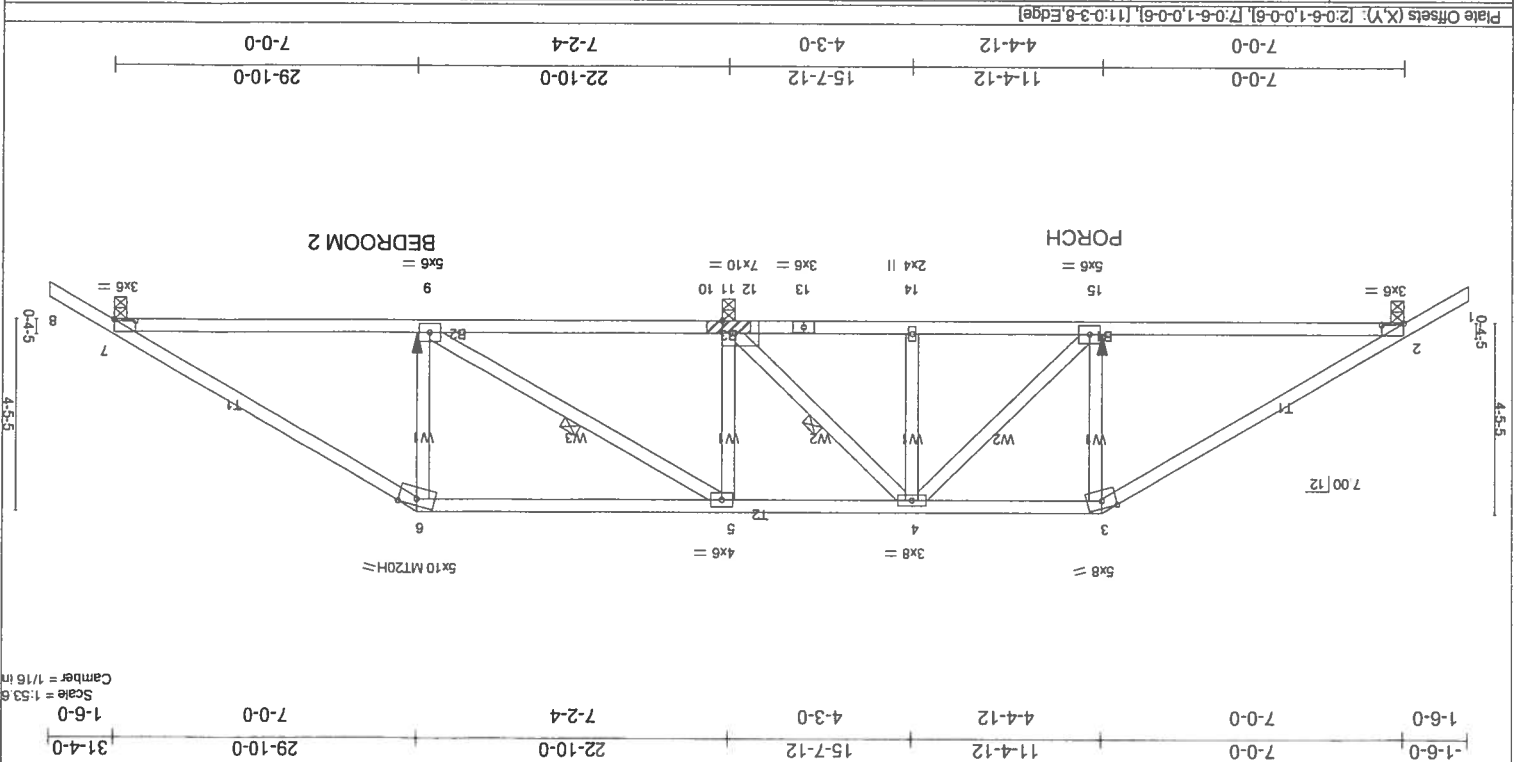
$2 = 0.79, 3 = 0.51, 4 = 0.42, 5 = 0.58, 6 = 0.42, 7 = 0.51, 8 = 0.79, 9 = 0.48, 10 = 0.43$ and $11 = 0.48$

NOTES

- LOAD CASE(S) Standard

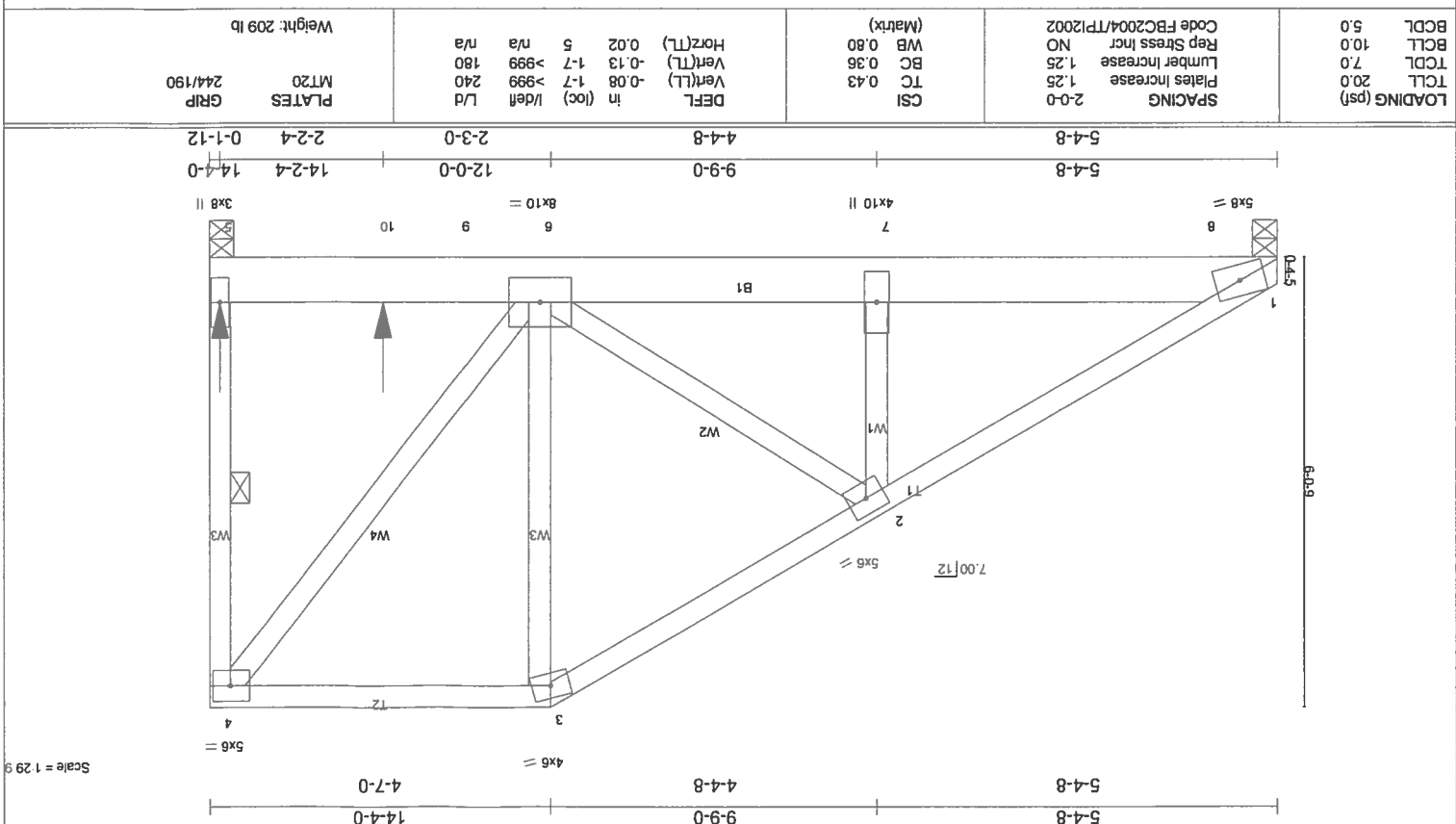
LOAD CASE(S) Standard





LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/dell	L/D	PLATES	GRP	MT20	MT20H	Weight: 150 lb
20.0	1.25	0.86	0.13	2-15	>999	240	MT20	244/190	187/143		
TCDL 7.0	Lumber Increase 1.25	BC 0.66	Vert(TL) 0.19	9-11	>882	180	MT20H				
BCLL 10.0	Rep Stress Incr NO	WB 0.61	Horz(TL) 0.02	7	n/a	n/a					
BCDL 5.0	Code FBC2004/TP12002	(Matbx)									
LUMBER 2 X 4 SYP No. 2 "Except"											
TOP CHORD T2 2 X 4 SYP No. 1D											
BOT CHORD 2 X 4 SYP No. 2											
WEBS 2 X 4 SYP No. 3											
BRACING BOT CHORD TOP CHORD											
Structural wood sheathing directly applied or 4-7-10 oc purlins.											
Rigid ceiling directly applied or 6-0-0 oc bracing.											
1 Row at midpt 4-11, 5-9											

REACTIONS (lb/size) 2=1051/0-3-8, 11=3355/0-3-15 (0-3-8 + bearing block), 7=886/0-3-8
Max Horiz Z=-148(lb/case 2)
Max UpliftZ=-750(lb/case 4), 11=-2143(lb/case 3), 7=-491(lb/case 5)
Max GravZ=1055(lb/case 8), 11=3355(lb/case 1), 7=897(lb/case 9)
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1483/981, 3-4=-1207/915, 4-5=-396/666, 5-6=-980/613, 6-7=-1219/612, 7-8=0/40
BOT CHORD 2-15=-904/1199, 14-15=-489/574, 13-14=-489/574, 12-13=-489/574, 11-12=-489/574, 10-11=-666/468, 9-10=-666/468, 7-9=-442/982
WEBS 3-15=-242/303, 4-15=-586/886, 4-14=-67/206, 4-11=-1721/1236, 5-11=-1725/1301, 5-9=-1011/1888, 6-9=-63/224
JOINT STRESS INDEX
2 = 0.71, 3 = 0.70, 4 = 0.95, 5 = 0.77, 6 = 0.91, 7 = 0.66, 9 = 0.61, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.34 and 15 = 0.30
NOTES
1) 2 X 4 SYP No. 2 bearing block 12" long at jlt. 11 attached to back face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed;
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) All bearings are assumed to be SYP No. 2
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 750 lb uplift at joint 2, 2143 lb uplift at joint 11 and 491 lb uplift at joint 7.
8) Girder carries hip and with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 374 lb up at 22-10-0, and 539 lb down and 374 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-118(B=-64), 6-8=-54, 8-15=-30, 9-15=-65(B=-35), 7-9=-30
Concentrated Loads (lb)
Vert: 15=-539(B) 9=-539(B)



TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 8 SYP 240F 2.0E
WEBS	2 X 4 SYP No.3

TOP CHORD	WEBS
BOT CHORD	1 Row at midpt
Structural wood sheathing directly applied or 4-9-6 oc purlins,	Rigid ceiling directly applied or 10-0-0 oc bracing.
	4-5

REACTIONS (lb/size) 1=4853/0-4-0, 5=6273/0-4-0
Max Uplift=2160(load case 4), 5=-2782(load case 2)
Max Horiz=258(load case 4)
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-7145/3151, 2-3=-3571/1541, 3-4=-3068/1393, 4-5=-4060/1839
BOT CHORD 1-8=-2907/6139, 7-8=-2907/6139, 6-7=-2849/49, 5-10=-28/49
WEBS 2-7=-1670/3587, 2-6=-3742/1842, 3-6=-670/1541, 4-6=-2258/4963
JOINT STRESS INDEX 1 = 0.71, 2 = 0.80, 3 = 0.43, 4 = 0.90, 5 = 0.38, 6 = 0.54 and 7 = 0.44

NOTES

1) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1621 lb down and 769 lb up at 12'-0-0", and 1458 lb down and 692 lb up at 14'-2-4" on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

2) Z-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webbs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1621 lb down and 769 lb up at 12'-0-0", and 1458 lb down and 692 lb up at 14'-2-4" on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

3) All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

4) Wind: ASCE 7-02; 1.0 mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gmp DOL=1.60

5) Provide adequate drainage to prevent water ponding.

6) All bearings are assumed to be SYP No.2

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2160 lb uplift at joint 1 and 2782 lb uplift at joint 5.

8) Girder carries tie-in span(s); 35-0-0 from 1-0-0 to 11-0-0

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1621 lb down and 769 lb up at 12'-0-0", and 1458 lb down and 692 lb up at 14'-2-4" on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

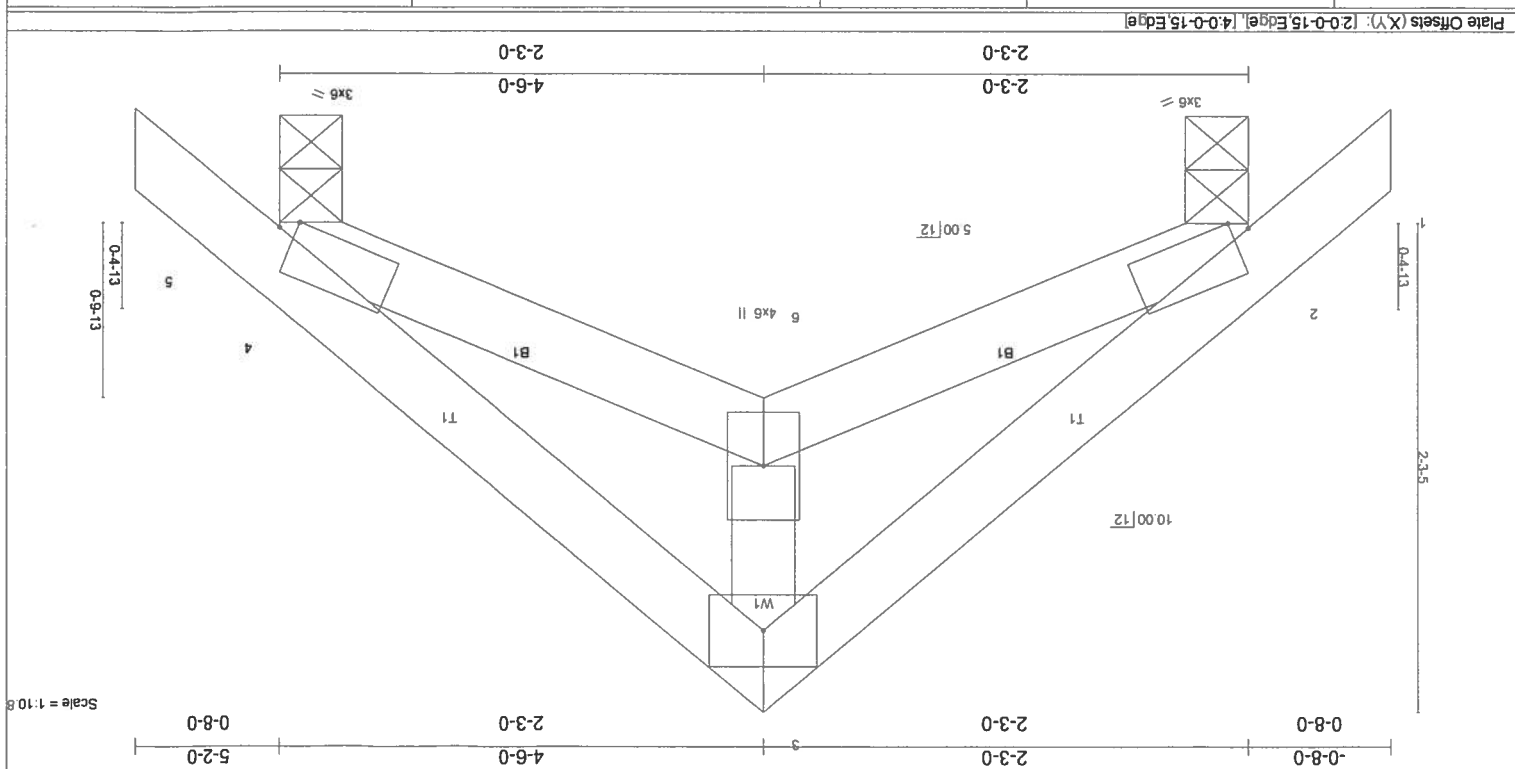
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Ver: 1-3=-54, 3-4=-54, 1-8=-30, 8-9=-77(B=-687), 5-9=-30

Concentrated Loads (lb)

Ver: -1458(B) 10=-1621(B)

[illegible]

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REACTIONS
  (lb/size)
    Max Horiz z2=71(1oad case 3)
    Max Uplift=-102(1oad case 5), -102(1oad case 6)
FORCES (lb) - Maximum Compression/Maximum Tension
  TOP CHORD 1-2=20/121, 2-3=-253/11, 3-4=-253/35, 4-5=0/21
  BOT CHORD 2-6=-20/173, 4-6=-20/173
WEBS
  3-6=-1/169
JOINT STRESS INDEX
  2 = 0.13, 3 = 0.11, 4 = 0.13 and 6 = 0.09

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NOTES

- (1) Unbraced roof live loads have been considered for this design.
- (2) Wind: ASCE 7-02; 11 mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=4.2psf; Category II; Exp. B; enclosed, MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- (3) All bearings are assumed to be SPV No.2
- (4) Bearing at joints(2, 4 considers parallel to grain value using ANSITP11 angle to grain formula. Building designer should verify capacity of bearing surface.
- (5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 2 and 102 lb uplift at joint 4.

NOTES

1) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) All bearings are assumed to be SYP No.2

3) Bearing at joints: 6 considers parallel to grain value using ANSITP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 k uplift at joint 2 and 177 k uplift at joint 6.

5) Load cases: 1, 2, 3, 4, 5, 6, 7, 8 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss or such connection device(s) is the responsibility of others.

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 142 k down at 3'-0" on top chord. The design/selection of such connection device(s) is the responsibility of others.

NOTES

$$2 = 0.31, 3 = 0.28, 4 = 0.13 \text{ and } 5 = 0.30$$

JOINT STRESS INDEX

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=10/40, 2-3=66/50, 3-4=203/118, 5-6=698/386, 4-5=301/163
BOT CHORD 2-5=95/539
WEBS 3-5=62/110

REACTIO

REACTIONS (lb/size) 2=571/0-3-8, 6=698/0-3-8

Max Horz 2=221(load case 5)
Max Uplift 2=-164(load case 5), 6=-177(load case 5)

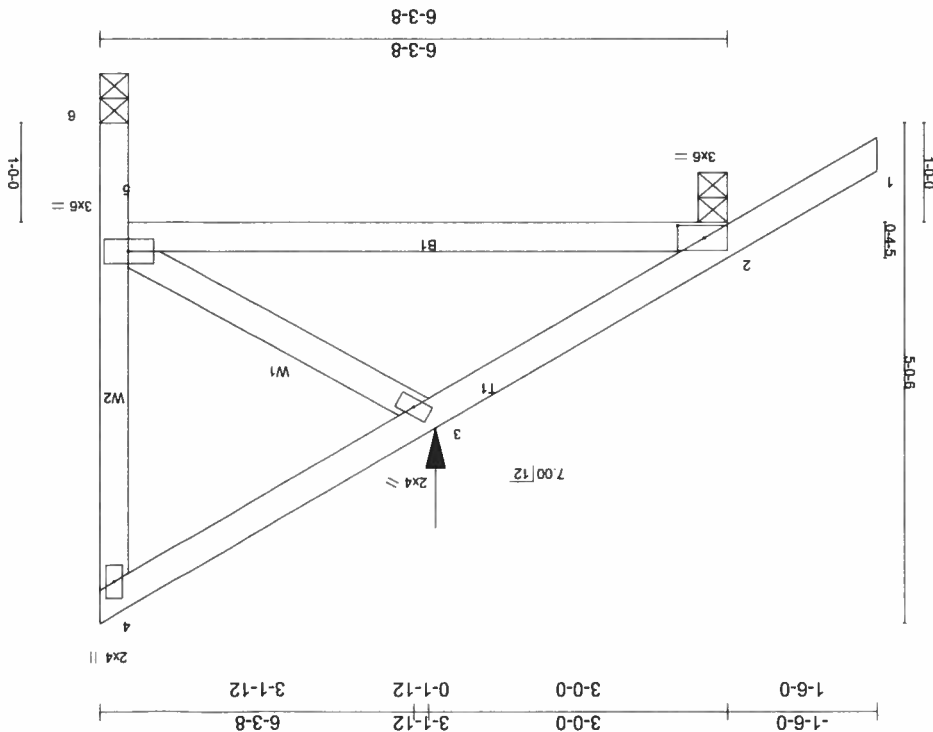
LUMBE

BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals
Rigid ceiling directly applied or 10-0-0 oc bracing.

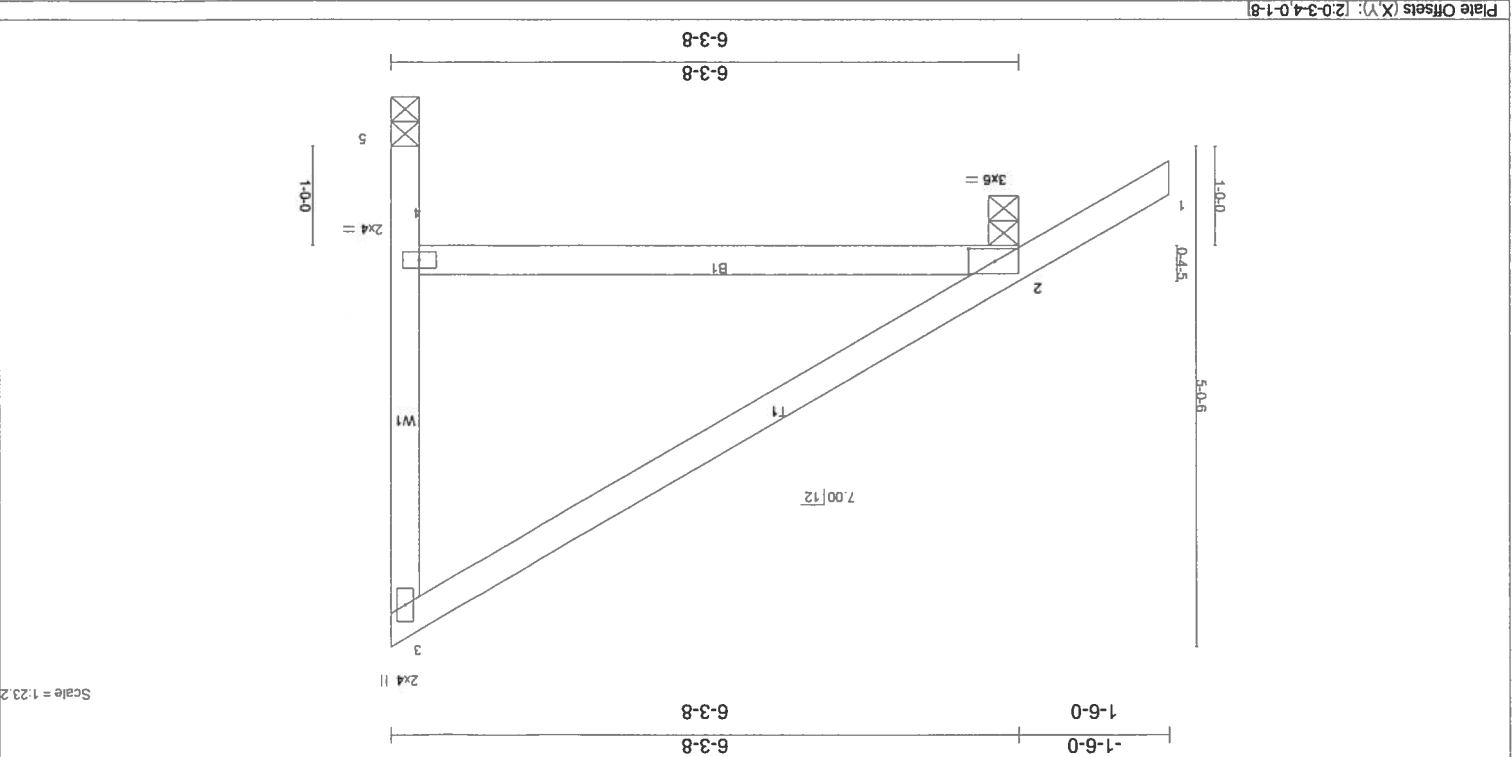
LOADING (psi)	20.0	TCLL	7.0	TCDL	10.0	BCLL	5.0	BCDL
SPACING	2-0-0	Plates Increase	1.25	Rep Stress Incr	NO	Code FBC2004/TP12002		
CSI	TC	0.44	BC	0.42	WB	0.16		
DEFL	in (loc)	0.21	Vert(TL)	0.18	Horz(TL)	0.01		
	l/dell	2-5	> 339	> 397	n/a	n/a		
	L/d	180						
	MT20	244						
	GRIP	244/190						
	PLATES							
	Weight: 35 lb							

Plate Offsets (X,Y): [2:0-3-4,0-1-8]



Job	L224665	Truss	T10	Truss Type	MONO TRUSS	City	9	Ply	1	AARON SIMQUE - GRAND ROSS
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Job	Truss	T10	MONO TRUSS	9	ply	AARON SIMQUE - GRAND ROSS	1	Job Reference (optional)
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LOADING (psf)		SPACING		DEFL		BRACING	
TCLL	20.0	Plates Increase	1.25	Ver(L/L)	0.21	TOP CHORD	Structural wood sheathing directly applied or 10'-0" oc bracing.
TCCL	7.0	Lumber Increase	1.25	Ver(T/L)	0.18	BOT CHORD	Rigid ceiling directly applied or 10'-0" oc bracing.
BCCL	10.0	Rep Stress Incr	YES	Horz(T/L)	0.00		
BCDL	5.0	Code FBC2004/TP12002					

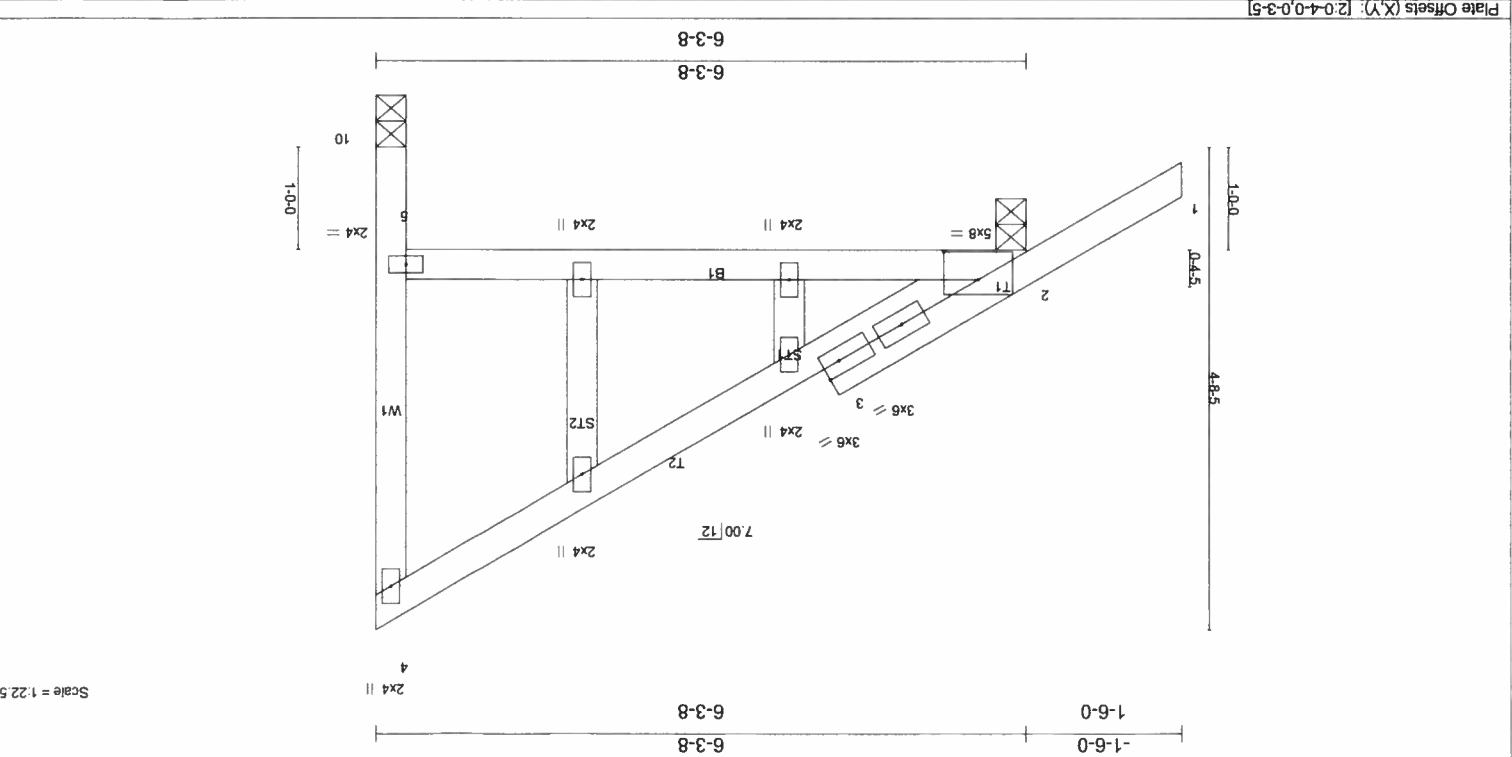
REACTIONS (lb/size) 2=353/0-3-8, 5=240/0-3-8
Max Horz 2=221(load case 5)
Max Uplift 2=229(load case 5), 5=205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-102/61, 4-5=-240/300, 3-4=-150/147
BOT CHORD 2-4=-0/0

JOINT STRESS INDEX
2 = 0.13, 3 = 0.08 and 4 = 0.14

NOTES
1) Wind: ASCE 7-02, 10mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; porch left and right exposed. Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2
3) Bearing at joint(s) 5 considers parallel to grain value using ANS/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 2 and 205 lb uplift at joint 5.

LOAD CASE(S) Standard



LOADING (psf)		SPACING		DEFL		BRACING	
TCLL	20.0	Plates Increase	1.25	TC	0.51	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
TCCL	7.0	Lumber Increase	1.25	BC	0.41	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
BCCL	10.0	Rep Stress Incr	NO	WB	0.00		
BCDL	5.0	Code FBC2004/TP12002					

REACTIONS		FORCES		JOINT STRESS INDEX	
(lb/size)	2=402/0-3-8, 10=268/0-3-8	(lb) - Maximum Compression/Maximum Tension			
Max Horiz 2=205(load case 5)		TOP CHORD	2 X 4 SYP No 2		
Max Uplift 2=271(load case 5), 10=214(load case 5)		WEBS	2 X 4 SYP No 3		
		OTHERS	2 X 4 SYP No 3		

NOTES
1) Wind: ASCE 7-02, 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DCL=1.60 plate grp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mittek "Standard Gable End Detail".
3) Gable studs spaced at 2'-0" oc.
4) All bearings are assumed to be SYP No. 2
5) Bearing at joint(s) 10 considers parallel to grain value using ANSITP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 2 and 214 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert. 1-4=-64(F=-10), 2-5=-30