

RE: 4745702 - CHURCHILL WAY MiTek, Inc.

Site Information: 16023 Swingley Ridge Rd. Chesterfield, MO 63017

Customer Info: K G CONSTRUCTION LLC Project Name: Spec Hse Model: 1549 314.434.1200

Lot/Block: N/A Subdivision: N/A

Address: 592 SW Churchill Way, TBD

City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2023/TPI2014 Design Program: MiTek 20/20 8.8

Wind Code: ASCE 7-22 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 12 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T37840246	T01	7/8/25
2	T37840247	T01G	7/8/25
3	T37840248	T02	7/8/25
4	T37840249	T03G	7/8/25
5	T37840250	T04	7/8/25
6	T37840251	T04G	7/8/25
7	T37840252	V01	7/8/25
8	T37840253	V02	7/8/25
9	T37840254	V03	7/8/25
10	T37840255	V04	7/8/25
11	T37840256	V05	7/8/25
12	T37840257	V06	7/8/25

This item has been digitally signed and sealed by Velez, Joaquin, PE on the date adjacent to the seal.

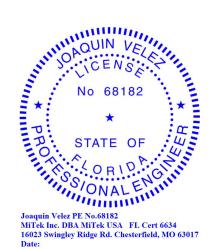
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The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Lake City, FL.

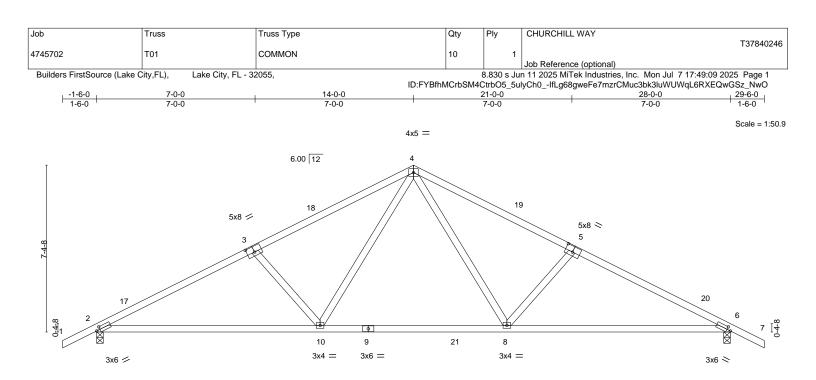
Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2027.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



July 8,2025



	'	9-10-8		'		8-3-0		'			9-10-8	ı .
Plate Off	sets (X,Y)	[2:0-1-15,0-1-8], [3:0-4-0,	0-3-0], [5:0-4-0	),0-3-0], [6:0-	-1-15,0-1-8]							
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.63	Vert(LL)	-0.21	8-16	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	ВС	1.00	Vert(CT)	-0.47	8-16	>722	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.29	Horz(CT)	0.07	6	n/a	n/a		
BCDL	10.0	Code FBC2023/TI	PI2014	Matri	x-MS	` ′					Weight: 130 lb	FT = 20%
				I		I					1	

**BRACING-**

TOP CHORD

**BOT CHORD** 

18-1-8

LUMBER-

TOP CHORD 2x4 SP No 2 **BOT CHORD** 2x4 SP No 2 WFBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=129(LC 16) Max Uplift 2=-304(LC 12), 6=-304(LC 13)

Max Grav 2=1294(LC 2), 6=1293(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

9-10-8

TOP CHORD 2-3=-2097/465, 3-4=-1879/427, 4-5=-1878/427, 5-6=-2096/465

**BOT CHORD** 2-10=-445/1844, 8-10=-170/1224, 6-8=-316/1843

WFBS 4-8=-191/757, 5-8=-446/278, 4-10=-191/758, 3-10=-446/278

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 -1-6-0 to 1-6-0, Zone1 1-6-0 to 14-0-0, Zone2 14-0-0 to 18-2-15, Zone1 18-2-15 to 29-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=304, 6=304.

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28-0-0

Structural wood sheathing directly applied or 3-4-7 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing.

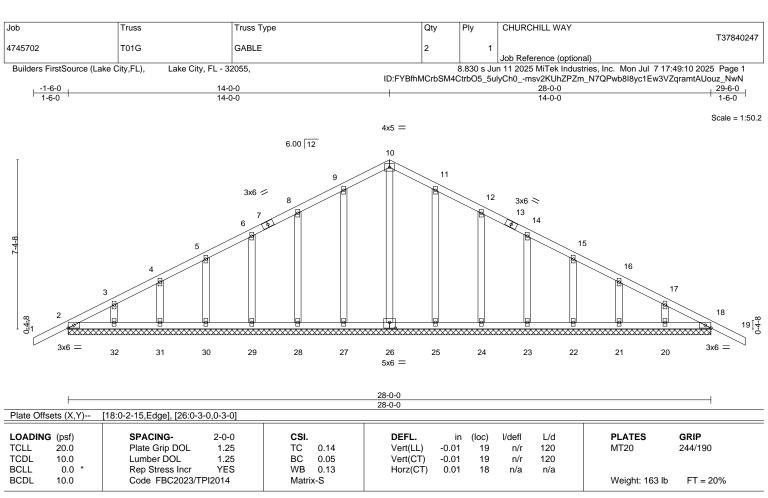
Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

July 8,2025



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.





LUMBER-TOP CHORD

**OTHERS** 

2x4 SP No 2 **BOT CHORD** 2x4 SP No 2 2x4 SP No 3

BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 28-0-0.

Max Horz 2=-129(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20, 18 Max Grav All reactions 250 lb or less at joint(s) 2, 26, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20, 18.

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2-8-0

7-0-0

Scale = 1:52.9 5x8 =

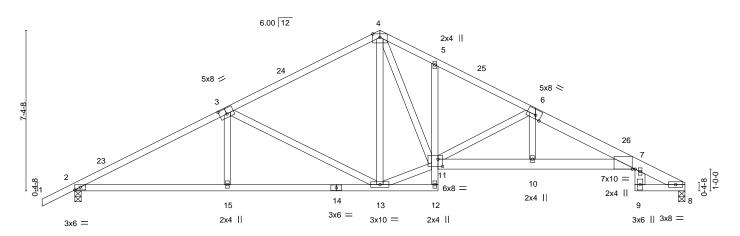
Structural wood sheathing directly applied or 3-9-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

8-8-3 oc bracing: 2-15

8-9-13 oc bracing: 13-15.

2-3-8



ı	7-0-0	14-0-0	16-8-0	21-0-0	25-8-8	28-0-0
Г	7-0-0	7-0-0	2-8-0	4-4-0	4-8-8	2-3-8
Plate Offsets (X,Y)	[3:0-4-0,0-3-0], [6:0-3-0,Edge], [7:0-	1-11,0-0-0], [11:0-2-8,0-4-0]				

LOADING	VI /	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.76	Vert(LL)	-0.20	7-10	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.70	Vert(CT)	-0.40	7-10	>850	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.82	Horz(CT)	0.24	8	n/a	n/a		
BCDL	10.0	Code FBC2023/TF	PI2014	Matri	x-MS						Weight: 164 lb	FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-TOP CHORD 2x4 SP No.2 \*Except\*

6-8: 2x6 SP 2400F 2.0E or 2x6 SP M 26

**BOT CHORD** 2x4 SP No.2 \*Except\*

5-12: 2x4 SP No.3, 7-11: 2x6 SP 2400F 2.0E or 2x6 SP M 26

7-9: 2x6 SP No.2

**WEBS** 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=141(LC 16)

Max Uplift 2=-303(LC 12), 8=-263(LC 13) Max Grav 2=1206(LC 1), 8=1112(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2009/455, 3-4=-1356/338, 4-5=-1758/466, 5-6=-1831/405, 6-7=-2771/600,

7-21=-803/207

**BOT CHORD**  $2-15 = -448/1732,\ 13-15 = -448/1736,\ 10-11 = -465/2531,\ 7-10 = -462/2503,\ 7-9 = -92/427$ **WEBS** 3-15=0/297, 3-13=-698/304, 11-13=-161/1092, 4-11=-289/1131, 6-11=-1083/330,

6-10=-54/470

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 -1-6-0 to 1-6-0, Zone1 1-6-0 to 14-0-0, Zone2 14-0-0 to 18-2-15, Zone1 18-2-15 to 27-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=303, 8=263,

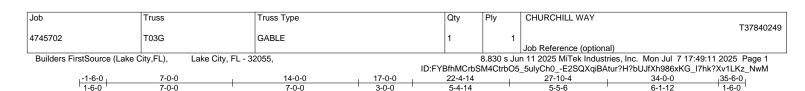
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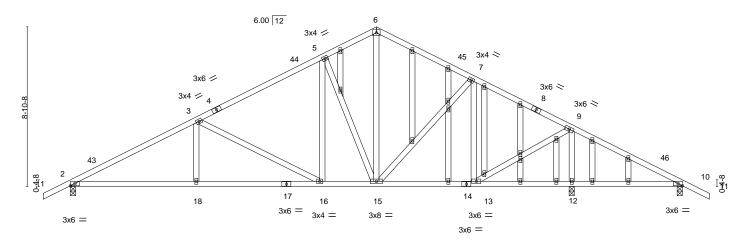


Plate Off	fsets (X,Y)	[10:0-2-15,Edge]									
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.25	TC	0.53	Vert(LL)	-0.08 16-18	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	ВС	0.61	Vert(CT)	-0.18 16-18	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.04 12	n/a	n/a		
BCDL	10.0	Code FBC2023/T	PI2014	Matrix	-MS					Weight: 238 lb	FT = 20%

BRACING-

TOP CHORD

**BOT CHORD** 

5-4-14

5-3-10

6-1-12

Structural wood sheathing directly applied or 3-10-5 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

17-0-0

3-0-0

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No 2 2x4 SP No 2

**BOT CHORD** 2x4 SP No.3 WFBS **OTHERS** 2x4 SP No.3

(size) 2=0-3-8, 10=0-3-8, 12=0-3-8

7-0-0

Max Horz 2=-154(LC 13)

Max Uplift 2=-306(LC 12), 10=-111(LC 8), 12=-326(LC 13) Max Grav 2=1158(LC 1), 10=209(LC 26), 12=1614(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1898/457, 3-5=-1249/324, 5-6=-922/311, 6-7=-945/303, 7-9=-834/241,

9-10=-108/459

**BOT CHORD** 2-18=-461/1630, 16-18=-461/1630, 15-16=-202/1028, 13-15=-63/677, 12-13=-332/134, 10-12=-332/134

3-18=0/303, 3-16=-678/291, 5-16=-94/436, 5-15=-665/286, 6-15=-218/578,

7-13=-488/152, 9-13=-227/1164, 9-12=-1454/341

## NOTES-

**WEBS** 

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 -1-6-0 to 1-6-0, Zone1 1-6-0 to 17-0-0, Zone2 17-0-0 to 21-2-15, Zone1 21-2-15 to 35-6-0 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

14-0-0

7-0-0

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=306, 10=111, 12=326.

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July 8,2025



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22-4-14

27-8-8

5-3-10

17-0-0

3-0-0

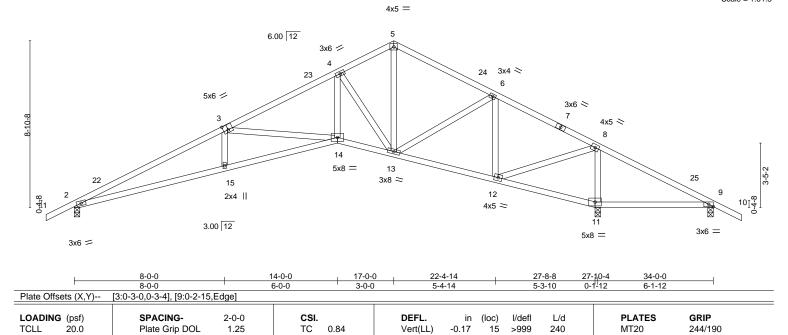
14-0-0

6-0-0

Scale = 1:61.3

34-0-0

6-3-8



LUMBER-

TCDL

**BCLL** 

BCDL

|<del>-1-6-0</del> | 1-6-0

8-0-0

8-0-0

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 2x4 SP No 3 WFBS

10.0

0.0

10.0

BRACING-

Vert(CT)

Horz(CT)

-0.35 14-15

11

0.20

>941

n/a

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 2-2-0 oc purlins.

Weight: 174 lb

FT = 20%

Rigid ceiling directly applied or 4-5-2 oc bracing.

180

n/a

REACTIONS. 2=0-3-8, 11=0-3-8, 9=0-3-8 (size)

Max Horz 2=-154(LC 13)

Lumber DOL

Rep Stress Incr

Code FBC2023/TPI2014

Max Uplift 2=-276(LC 12), 11=-454(LC 12), 9=-497(LC 25) Max Grav 2=1028(LC 1), 11=2279(LC 1), 9=142(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2803/733, 3-4=-1729/412, 4-5=-886/253, 5-6=-920/252, 6-8=-416/207, 8-9=-379/1645

BOT CHORD 2-15=-719/2517. 14-15=-717/2496. 13-14=-278/1515. 12-13=0/324. 11-12=-1529/410.

1.25

YES

BC

WB

Matrix-MS

0.79

0.68

9-11=-1387/376

WEBS 3-15=0/303, 3-14=-964/427, 4-14=-262/1148, 4-13=-1263/393, 5-13=-153/525,

6-13=-130/613, 6-12=-891/228, 8-12=-344/1793, 8-11=-1766/377

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 -1-6-0 to 1-6-0, Zone1 1-6-0 to 17-0-0, Zone2 17-0-0 to 21-2-15, Zone1 21-2-15 to 35-6-0 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=276, 11=454, 9=497,

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22-4-14

27-8-8

5-3-10

27-8-8

5-3-10

Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 4-5-2 oc bracing.

17-0-0

3-0-0

14-0-0

6-0-0

14-0-0

Scale = 1:61.3

1-6-0

34-0-0

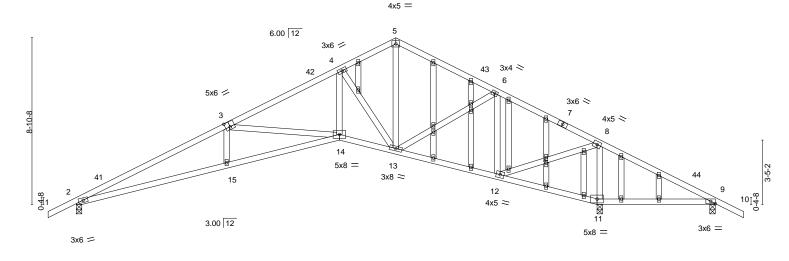


Plate Oils	Plate Offsets (X, Y) [5:0-3-0,0-3-4], [9:0-2-15,Edge], [26:0-1-11,0-1-0]											
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (l	loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.84	Vert(LL)	-0.17	15	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.79	Vert(CT)	-0.35 14	-15	>941	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.68	Horz(CT)	0.20	11	n/a	n/a		
BCDL	10.0	Code FBC2023/TF	PI2014	Matri	x-MS						Weight: 210 lb	FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

5-4-14

17-0-0

LUMBER-

|<del>-1-6-0</del> | <del>1-6-0</del>

8-0-0

TOP CHORD 2x4 SP No 2 2x4 SP No 2

**BOT CHORD** 2x4 SP No.3 WFBS **OTHERS** 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 9=0-3-8, 11=0-3-8

Max Horz 2=-154(LC 13)

8-0-0

8-0-0

Max Uplift 2=-276(LC 12), 9=-497(LC 25), 11=-454(LC 12) Max Grav 2=1028(LC 1), 9=142(LC 12), 11=2279(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2803/733, 3-4=-1729/412, 4-5=-886/253, 5-6=-920/252, 6-8=-416/207,

8-9=-379/1645

**BOT CHORD** 2-15=-719/2517, 14-15=-717/2496, 13-14=-278/1515, 12-13=0/324, 11-12=-1529/410,

9-11=-1387/376

3-15=0/303, 3-14=-964/427, 4-14=-262/1148, 4-13=-1263/393, 5-13=-153/525,

6-13=-130/613, 6-12=-891/228, 8-12=-344/1793, 8-11=-1766/377

## NOTES-

**WEBS** 

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 -1-6-0 to 1-6-0, Zone1 1-6-0 to 17-0-0, Zone2 17-0-0 to 21-2-15, Zone1 21-2-15 to 35-6-0 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=276, 9=497, 11=454.

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July 8,2025



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CHURCHILL WAY Job Truss Truss Type Qty Ply T37840252 4745702 V01 **GABLE** Job Reference (optional) Builders FirstSource (Lake City,FL), Lake City, FL - 32055, 8.830 s Jun 11 2025 MiTek Industries, Inc. Mon Jul 7 17:49:13 2025 Page 1 ID:FYBfhMCrbSM4CtrbO5\_5ulyCh0\_-BRaByVjRiU8ZEa9\_bkh?maEZc815mCa1SrO8PDz\_NwK 19-10-15 Scale = 1:42.1 4x5 = 8.00 12 10 4-10-0 24 23 12 5-0-0 0-0-3x6 / 3x6 <> 20 19 18 15 5x6 = 19-10-15

Plate Offsets (X,Y) [9:0-0-0,0-0-0], [20:0-3-0,0-3-0]									
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP					
TCLL 20.0	Plate Grip DOL 1.25	TC 0.07	Vert(LL) n/a - n/a 999	MT20 244/190					
TCDL 10.0	Lumber DOL 1.25	BC 0.25	Vert(CT) n/a - n/a 999						
BCLL 0.0 *	Rep Stress Incr YES	WB 0.07	Horz(CT) 0.00 13 n/a n/a						
BCDL 10.0	Code FBC2023/TPI2014	Matrix-S		Weight: 101 lb FT = 20%					

LUMBER-TOP CHORD

2x4 SP No 2

BOT CHORD 2x4 SP No.2 2x4 SP No.3 WFBS **OTHERS** 2x4 SP No.3 **BRACING-**TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 19-10-15.

(lb) -Max Horz 1=-156(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 18, 17, 19, 20, 21, 16, 15, 14

All reactions 250 lb or less at joint(s) 1, 13, 19, 20, 21, 16, 15, 14 except 18=405(LC 19), 17=385(LC Max Grav

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 0-5-12 to 3-5-12, Zone1 3-5-12 to 9-11-7, Zone2 9-11-7 to 13-11-7, Zone1 13-11-7 to 19-5-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 18, 17, 19, 20, 21, 16, 15, 14.

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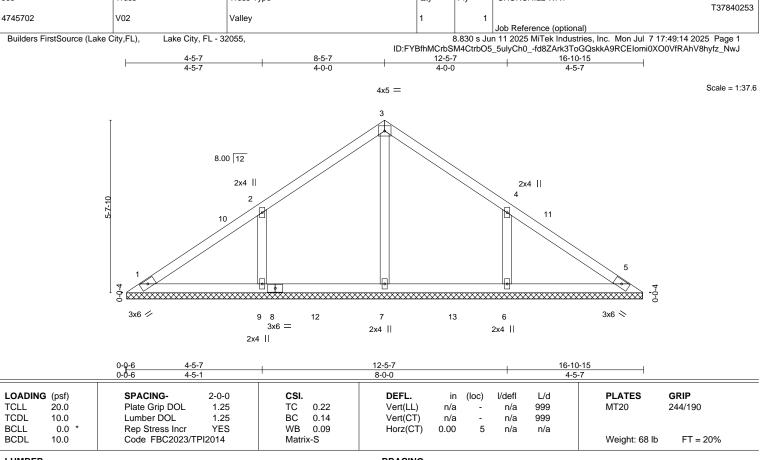
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Qty

Ply

CHURCHILL WAY

LUMBER-

Job

Truss

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 **OTHERS** 

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 16-10-3.

Max Horz 1=-131(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-200(LC 12), 6=-200(LC 13)

Truss Type

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=342(LC 19), 9=478(LC 19), 6=478(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**WEBS** 2-9=-307/220, 4-6=-307/219

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 0-5-12 to 3-5-12, Zone1 3-5-12 to 8-5-7, Zone2 8-5-7 to 12-5-7 Zone1 12-5-7 to 16-5-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=200, 6=200.

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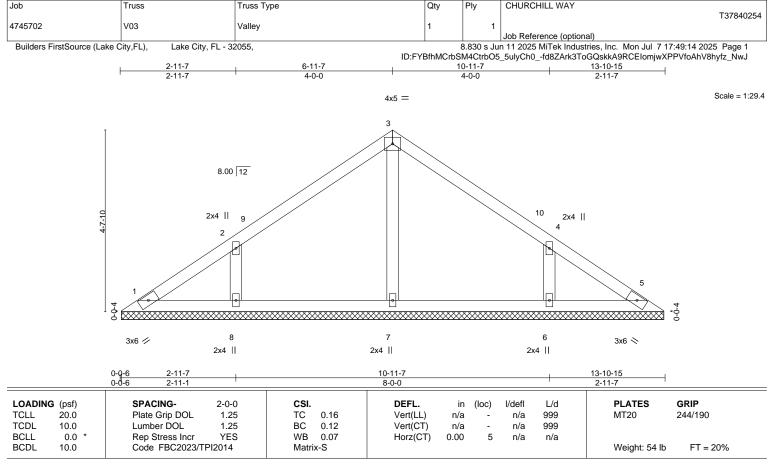
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LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2

BOT CHORD 2x4 SP No.3 **OTHERS** 

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

CHURCHILL WAY

REACTIONS. All bearings 13-10-3.

Max Horz 1=-107(LC 8)

Truss

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-166(LC 12), 6=-166(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=264(LC 1), 8=329(LC 19), 6=329(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**WEBS** 2-8=-257/187, 4-6=-256/187

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 0-5-12 to 3-5-12, Zone1 3-5-12 to 6-11-7, Zone2 6-11-7 to 10-11-7, Zone1 10-11-7 to 13-5-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=166, 6=166.

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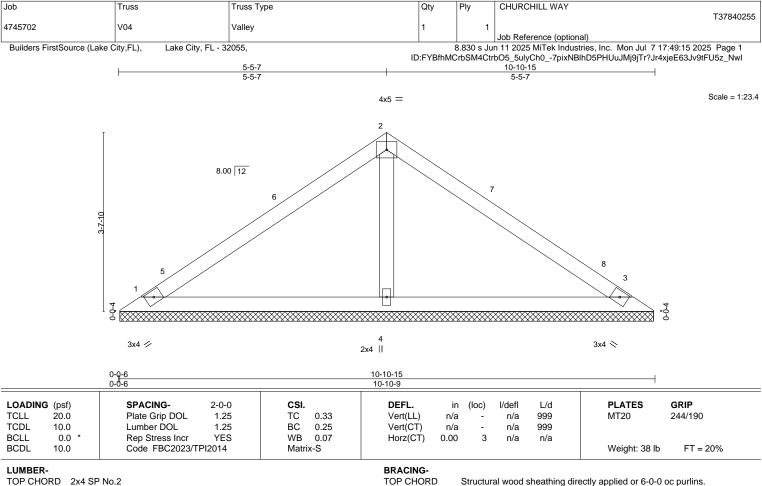
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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 **OTHERS** 

(size) 1=10-10-3, 3=10-10-3, 4=10-10-3

Max Horz 1=82(LC 9)

Max Uplift 1=-57(LC 12), 3=-68(LC 13), 4=-66(LC 12) Max Grav 1=196(LC 1), 3=196(LC 1), 4=404(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**WEBS** 2-4=-257/130

## NOTES-

REACTIONS.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 0-5-12 to 3-5-12, Zone1 3-5-12 to 5-5-7, Zone2 5-5-7 to 9-8-6, Zone1 9-8-6 to 10-5-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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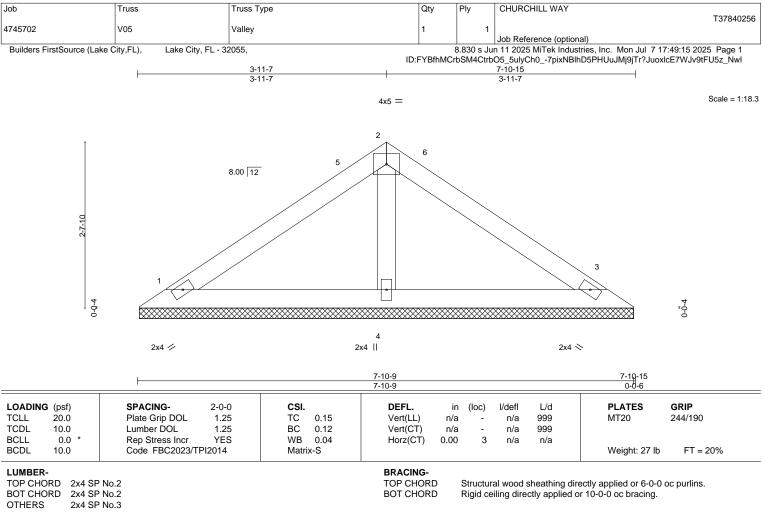
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REACTIONS. (size) 1=7-10-3, 3=7-10-3, 4=7-10-3

Max Horz 1=-57(LC 8)

Max Uplift 1=-40(LC 12), 3=-48(LC 13), 4=-46(LC 12) Max Grav 1=137(LC 1), 3=137(LC 1), 4=282(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 0-5-12 to 3-5-12, Zone1 3-5-12 to 3-11-7, Zone3 3-11-7 to 7-5-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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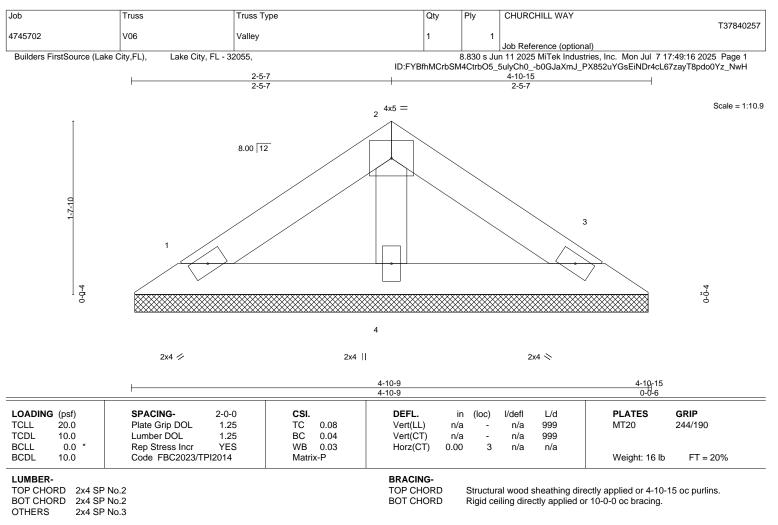
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REACTIONS. (size) 1=4-10-3, 3=4-10-3, 4=4-10-3

Max Horz 1=33(LC 9)

Max Uplift 1=-28(LC 12), 3=-32(LC 13), 4=-17(LC 12) Max Grav 1=85(LC 1), 3=85(LC 1), 4=145(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Zone3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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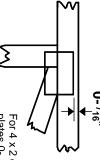


## Symbols

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-  $\frac{1}{16}$  from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in MiTek software or upon request.

## PLATE SIZE

4 × 4

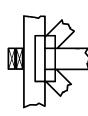
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

## BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur Min size shown is for crushing only.

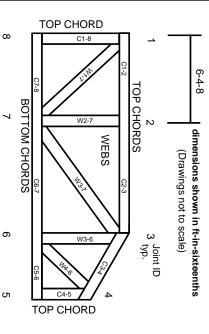
## Industry Standards:

National Design Specification for Metal Plate Connected Wood Truss Construction Design Standard for Bracing.

Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

ANSI/TPI1: DSB-22:

## **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

# Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

# Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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## 

MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

# **General Safety Notes**

## Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.

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Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.

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- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

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- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.