

REVISIONS

SOFTPIXAN ARCHITECTURAL DESIGN SOFTWARE

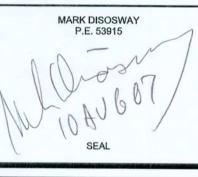
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Brad & Becky Zoeller Residence

ADDRESS: Cannon Creek Airpark S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

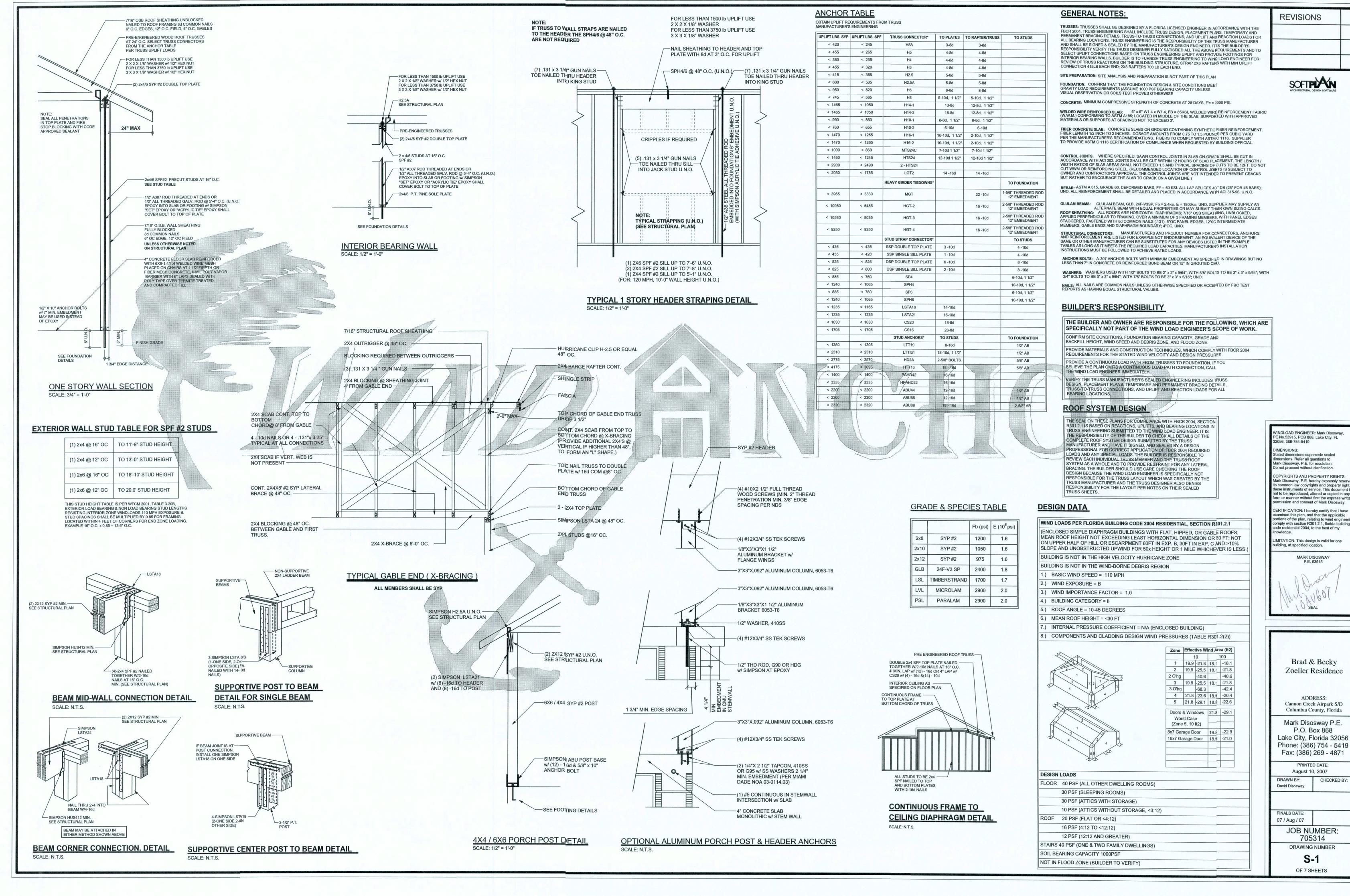
PRINTED DATE: August 10, 2007

DRAWN BY: CHECKEL David Disosway

FINALS DATE: 07 / Aug / 07

JOB NUMBER: 705314 DRAWING NUMBER

> A-4 OF 7 SHEETS

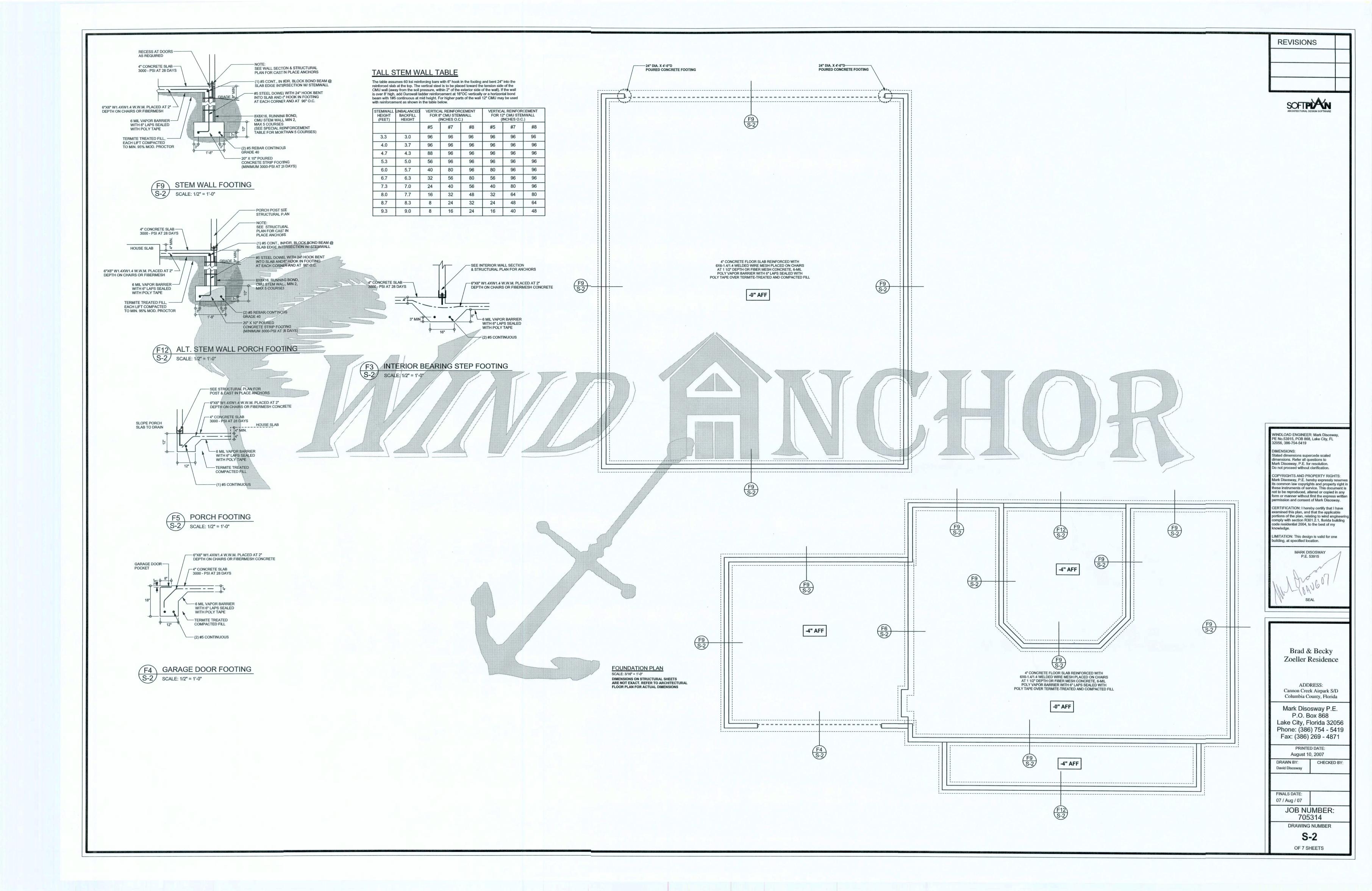


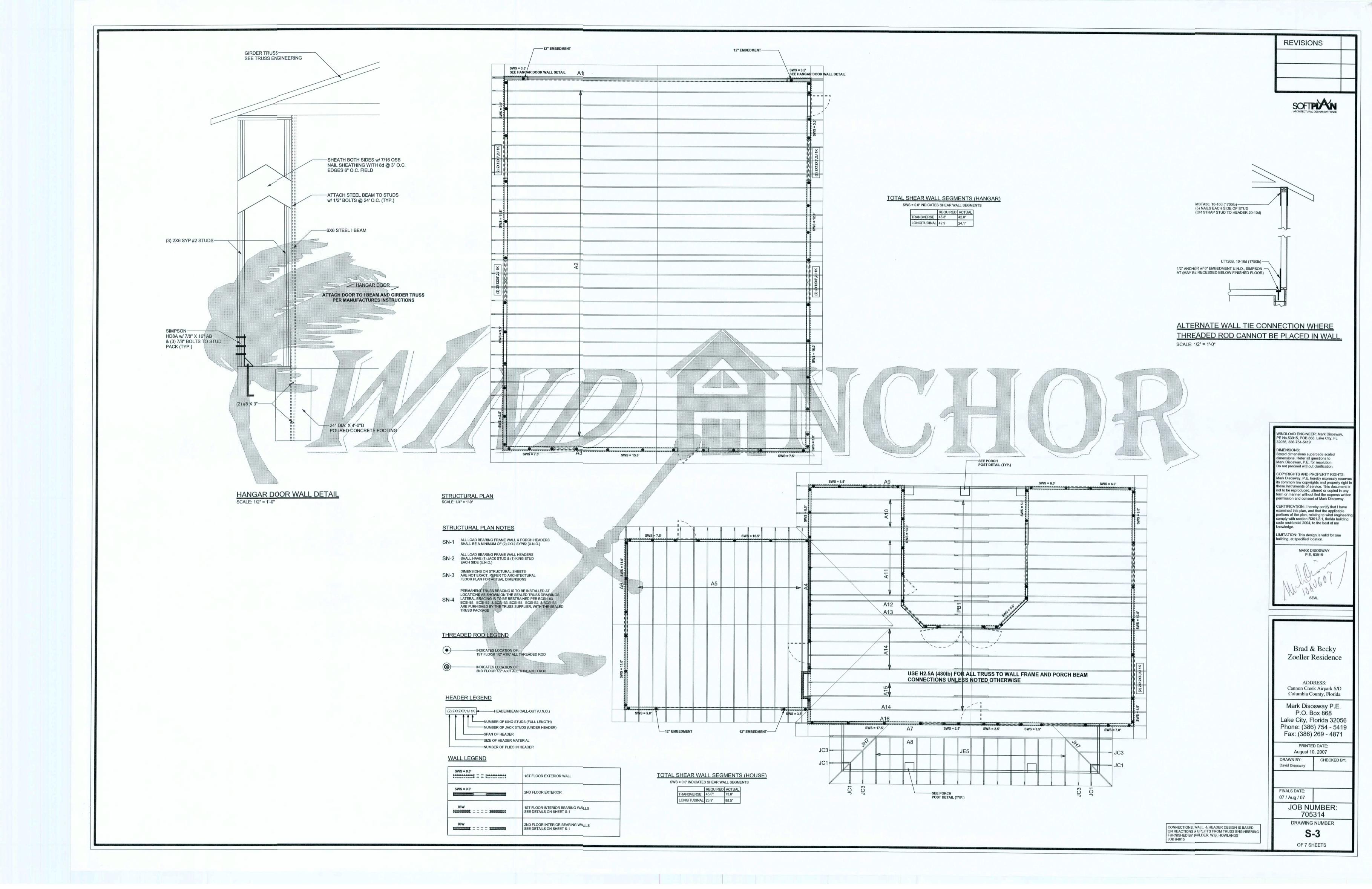
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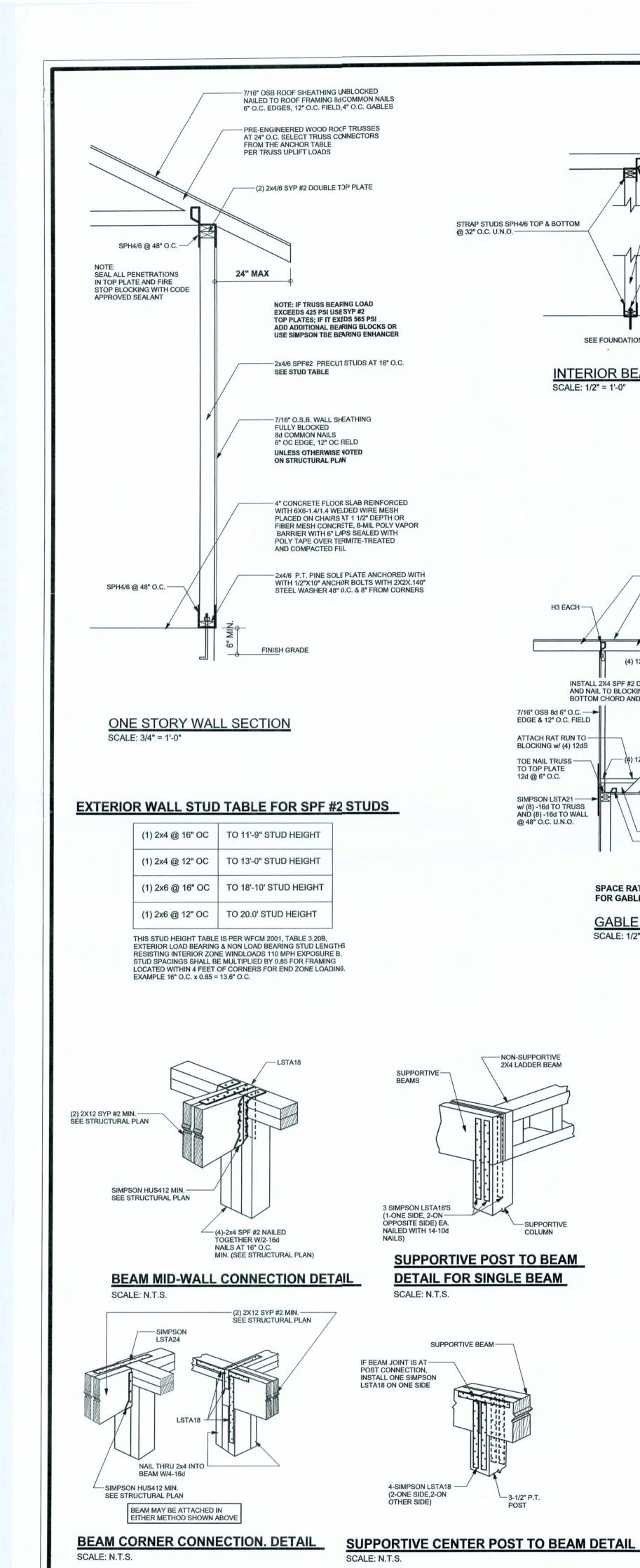
orm or manner without first the express writte ermission and consent of Mark Disosway ERTIFICATION: I hereby certify that I ha amined this plan, and that the applicable

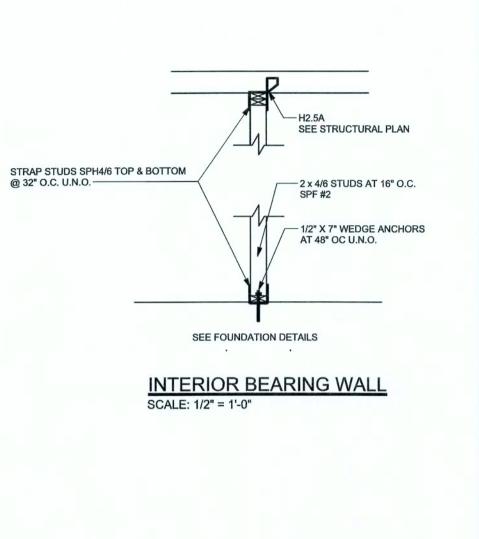
omply with section R301.2.1, florida building

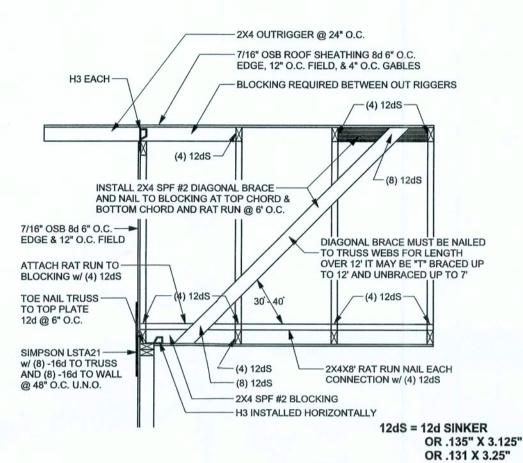
Mark Disosway P.E. Lake City, Florida 32056 Phone: (386) 754 - 5419







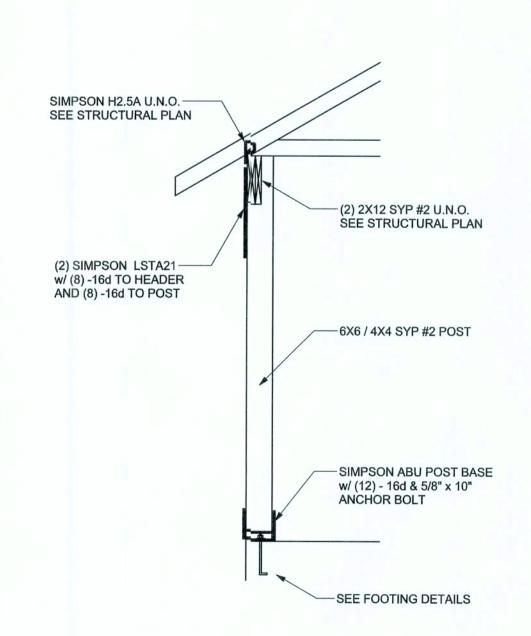




SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED SCALE: 1/2" = 1'-0"

- NON-SUPPORTIVE

2X4 LADDER BEAM



IF TRUSS TO WALL STRAPS ARE NAILED

-SPH4/6 ALL OPENINGS (U.N.O.)-

←SPH4/6 @ 48" O.C. (U.N.O.)

CRIPPLES IF REQUIRED

(4) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU SILL-INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

- SPH4/6 ALL OPENINGS (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

TYPICAL HEADER STRAPING DETAIL

(SEE STRUCTURAL PLAN)

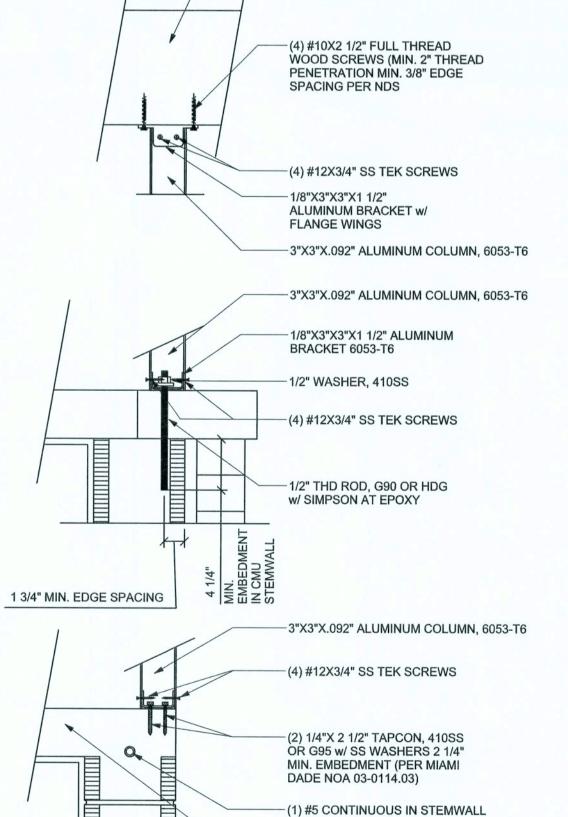
TO THE HEADER THE SPH4/6 @ 48" O.C.

ARE NOT REQUIRED

(6) .131 x 3 1/4" GUN NAILS-

INTO KIING STUD

TOE NAILED THRU HEADER



INTERSECTION w/ SLAB

MONOLITHIC w/ STEM WALL

-4" CONCRETE SLAB

OPTIONAL ALUMINUM PORCH POST & HEADER ANCHORS

SYP #2 HEADER

-(6) .131 x 3 1/4" GUN NAILS

INTO KING STUD

TÓE NAILED THRU HEADER

ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

JPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	S TO STUDS		
< 420	< 245	H5A	3-8d	3-8d			
< 455	< 455 < 265 H5		4-8d	4-8d			
< 360	< 235 H4		4-8d	4-8d			
< 455	< 320	H3	4-8d	4-8d			
< 415	< 365	H2.5	5-8d	5-8d			
< 600	< 535	H2.5A	5-8d	5-8d			
< 950	< 820	H6	8-8d	8-8d			
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"			
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"			
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"			
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"			
< 760	< 655	H10-2	6-10d	6-10d			
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"			
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"			
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"			
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"			
< 2900	< 2490	2 - HTS24					
< 2050	< 1785	LGT2	14 -16d	14 -16d			
		LICANO CIDADES THE CHARLES			22.0 - 2.0 -		
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION		
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO		
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT		
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT		
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT		
	STUD STRAP CONNECTOR				TO STUDS		
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d		
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d		
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d		
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d		
< 885	< 760	SP4			6-10d, 1 1/2"		
< 1240	< 1065	SPH4			10-10d, 1 1/2"		
< 885	< 760	SP6			6-10d, 1 1/2"		
< 1240	< 1065	SPH6			10-10d, 1 1/2"		
< 1235	< 1165	LSTA18	14-10d				
< 1235	< 1235	LSTA21	16-10d				
< 1030	< 1030	CS20	18-8d				
< 1705	< 1705	CS16	28-8d				
		STUD ANCHORS*	TO STUDS		TO FOUNDATION		
< 1350	< 1305	LTT19	8-16d		1/2" AB		
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB		
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB		
< 4175	< 3695	HTT16	18 - 16d		5/8" AB		
< 1400	< 1400	PAHD42	16-16d				
< 3335	< 3335	HPAHD22	16-16d				
< 2200	< 2200	ABU44	12-16d		1/2" AB		
< 2300	< 2300	ABU66	12-16d		1/2" AB		
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB		

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

REVISIONS

SOFTPLAN

WINDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

Stated dimensions supercede scaled

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LIMITATION: This design is valid for one

MARK DISOSWAY

P.E. 53915

Brad & Becky

Zoeller Residence

ADDRESS:

Cannon Creek Airpark S/D

Columbia County, Florida

Mark Disosway P.E. P.O. Box 868

Lake City, Florida 32056

Phone: (386) 754 - 5419

Fax: (386) 269 - 4871

PRINTED DATE:

CHECKED BY:

January 14, 2008

building, at specified location.

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dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

DIMENSIONS:

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF EEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" x 0" w1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK (IN A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Ft = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

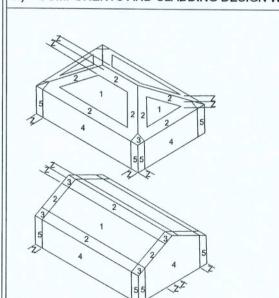
NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AF NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	ONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND IT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	ALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PI	INUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU IN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL ENGINEER IMMEDIATELY.
DESIGN, PLACE	SS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS ENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, S CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED



ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

DESIGN LOADS

Zone	Effec	Effective Wind Area (ft2)			
	10			100	
1	19.9	-21.8	18.1	-18.1	
2	19.9	-25.5	18.1	-21.8	
2 O'hg		-40.6		-40.6	
3	19.9	-25.5	18.1	-21.8	
3 O'hg		-68.3		-42.4	
4	21.8	-23.6	18.5	-20.4	
5	21.8	-29.1	18.5	-22.6	
Wo	& Winderst Case 5, 10	е	21.8	-29.1	
8x7 Ga	rage D	oor	19.5	-22.9	
16x7 G	arage l	Door	18.5	-21.0	

FLOOR 40 PSF (ALL OTHER DWELLIN 30 PSF (ATTICS WITH STORAG

rods.

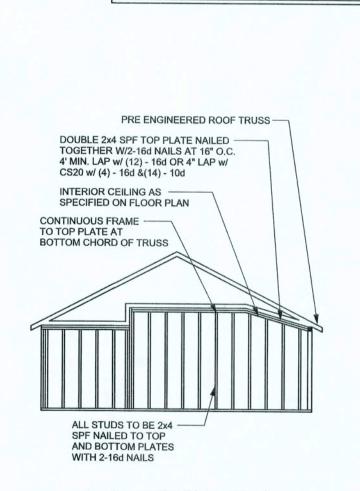
07 / Aug / 07 JOB NUMBER: 705314

DRAWN BY:

David Disosway

DRAWING NUMBER **S-1**

OF 7 SHEETS



GRADE & SPECIES TABLE

1200

1050

975

2900

2900

1.6

2.0

2.0

SYP #2

SYP #2

SYP #2

24F-V3 SP

MICROLAM

PARALAM

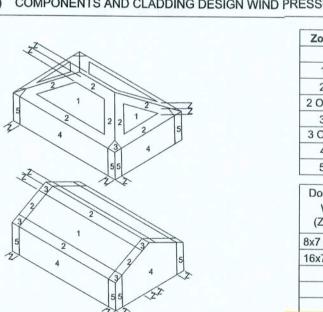
2x12

GLB

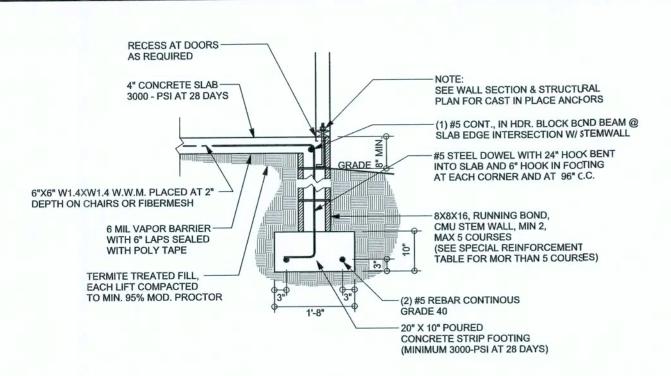
CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL SCALE: N.T.S.

DESIGN DATA

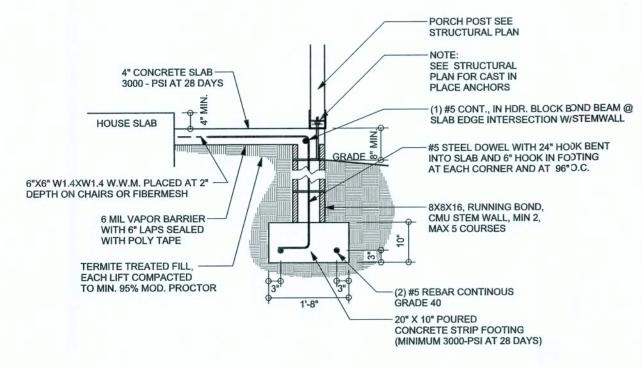
WIN	ID LOADS PER FLORIDA BUILDING CODE 2004	RESIDENTI	AL, SECTIO	N R30	1.2.1	
ME/ ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WIT AN ROOF HEIGHT NOT EXCEEDING LEAST HO UPPER HALF OF HILL OR ESCARPMENT 60FT OPE AND UNOBSTRUCTED UPWIND FOR 50x H	RIZONTAL D IN EXP. B, 3	IMENSION OFT IN EXF	OR 6	0 FT; NOT ND >10%	
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRIC	ANE ZONE				
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS RI	EGION				
1.)	BASIC WIND SPEED = 110 MPH					
2.)	WIND EXPOSURE = B					
3.)	WIND IMPORTANCE FACTOR = 1.0					
4.)	BUILDING CATEGORY = II	_				
5.)	ROOF ANGLE = 10-45 DEGREES					
6.)	MEAN ROOF HEIGHT = <30 FT					
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (E	NCLOSED E	BUILDING)			
8.)	COMPONENTS AND CLADDING DESIGN WIND	D PRESSUR	ES (TABLE	R301	.2(2))	
					3.460	
	~~	Zone	Effective Wind Area (ft2)			
			10		100	
		1	19.9 -21.8	18.1	-18.1	



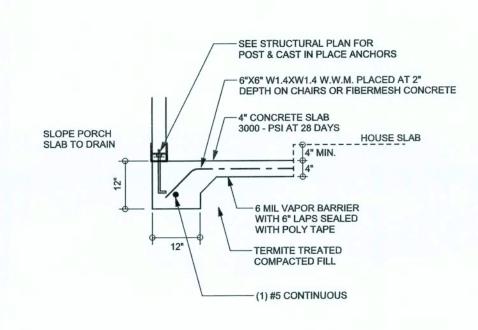
25	
N LOADS	Zoeller Jos
R 40 PSF (ALL OTHER DWELLING ROOMS)	^
30 PSF (SLEEPING ROOMS)	18mit # 21297
30 PSF (ATTICS WITH STORAGE)	1011101
10 PSF (ATTICS WITHOUT STORAGE, <3:12	
20 PSF (FLAT OR <4:12)	Quantity of the same



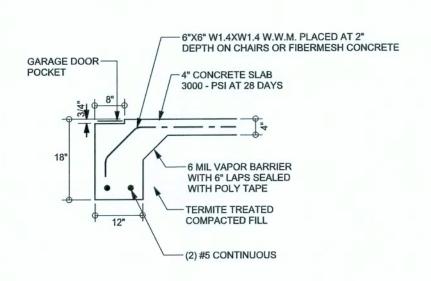
F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



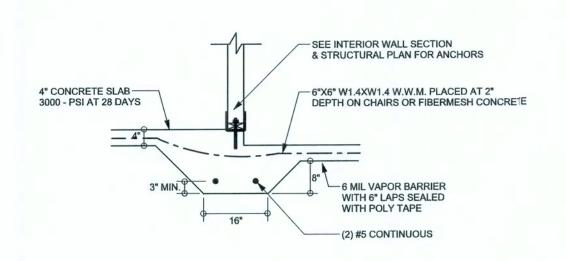
F12 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"

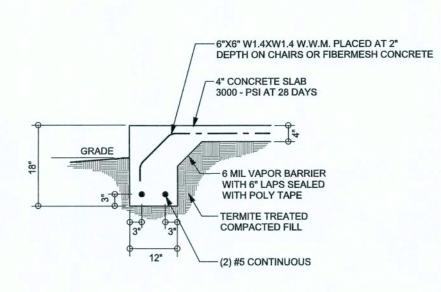


F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"

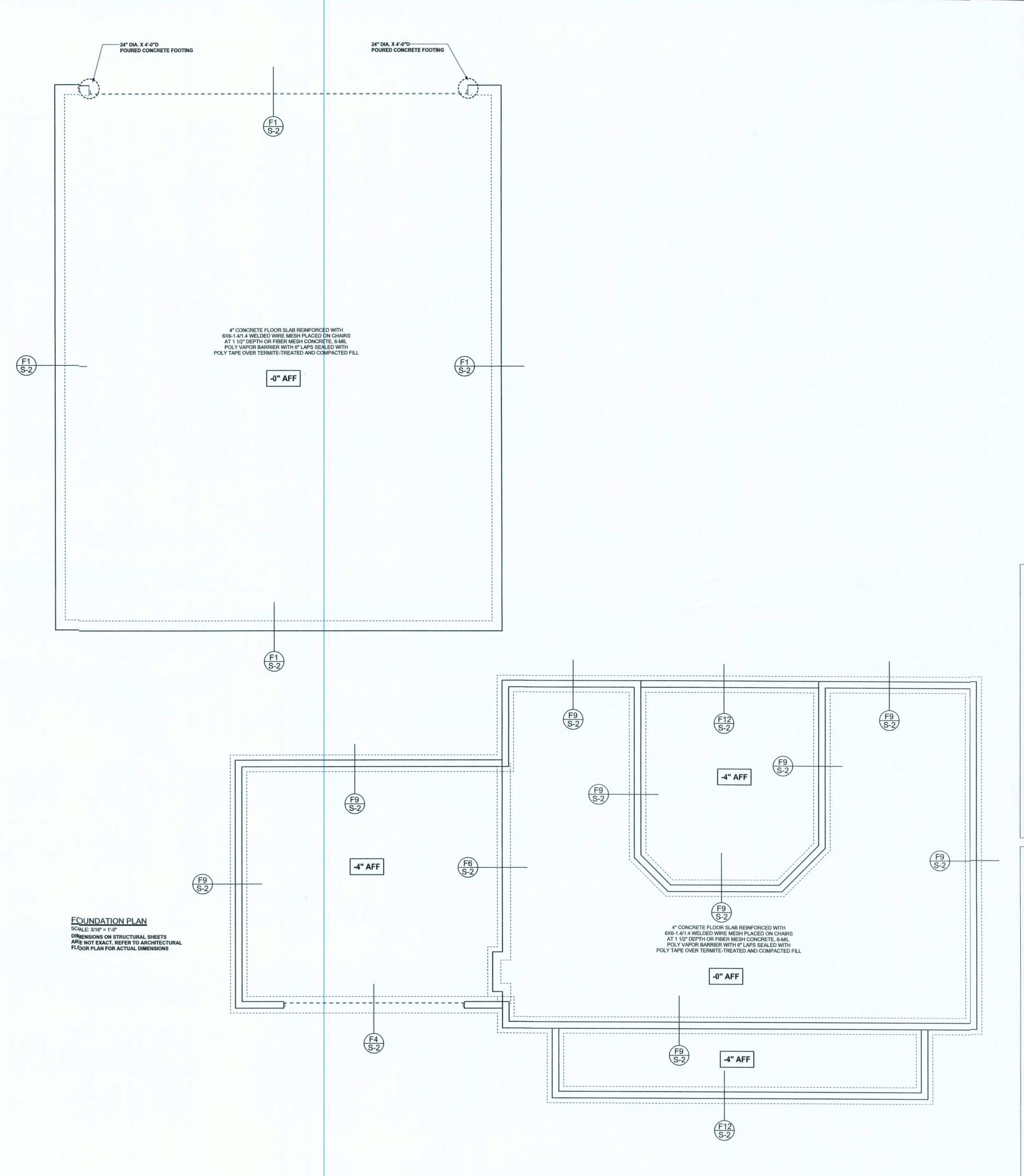
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F1 MONOLITHIC FOOTING
S-2 SCALE: 1/2" = 1'-0"



REVISIONS
1/14/08

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P.E. 53915

SEAL

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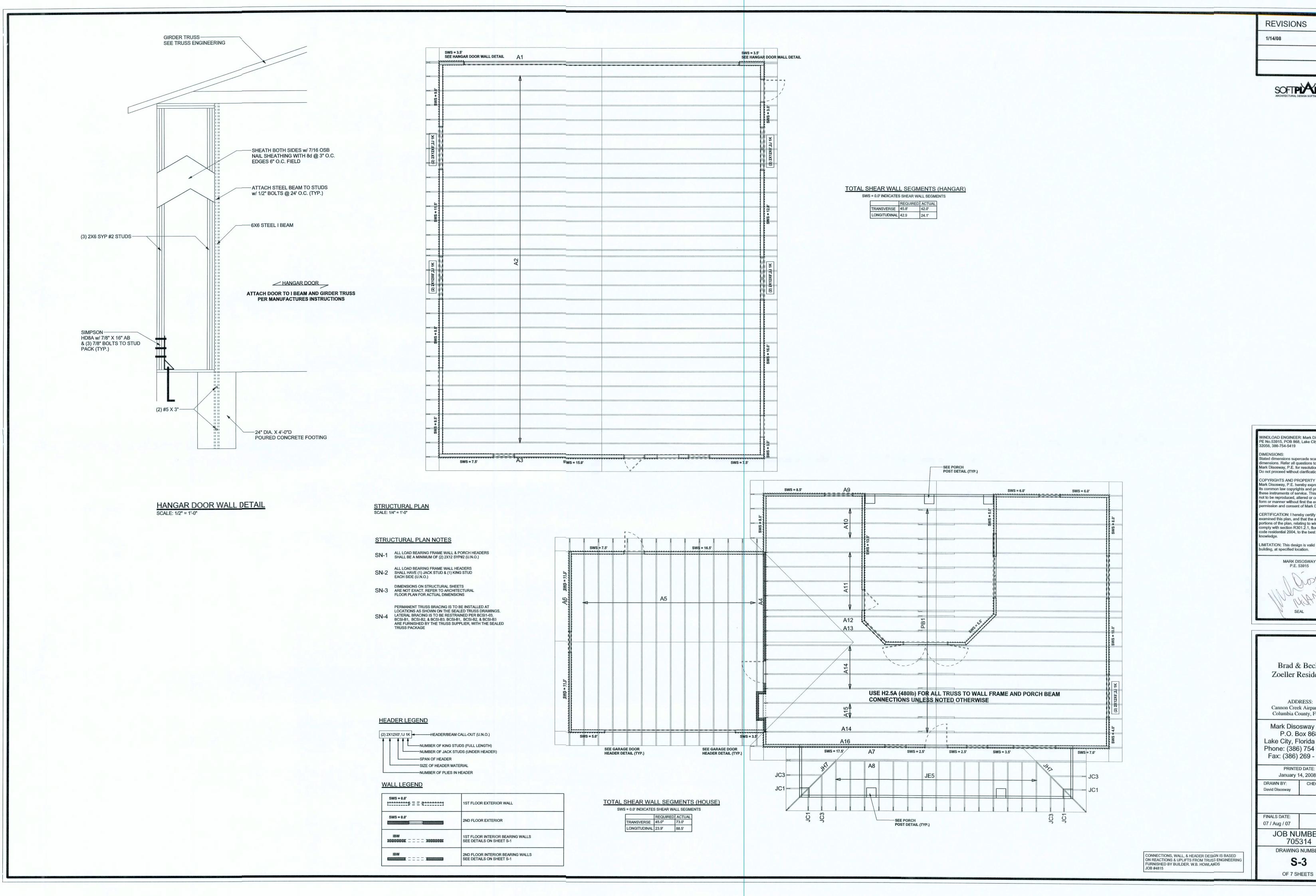
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> S-2 OF 7 SHEETS



REVISIONS

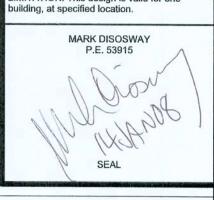
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one



Brad & Becky Zoeller Residence

ADDRESS: Cannon Creek Airpark S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: January 14, 2008 DRAWN BY: CHECKED BY: David Disosway

FINALS DATE:

JOB NUMBER: 705314

DRAWING NUMBER **S-3**