DATE 05/01/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024452

APPLICANT GARY JOHNSON PHONE 752-3444

ADDRESS P.O. BOX 1016 LAKE CITY FL 32056

OWNER PAUL & AUDREY WENDEL PHONE 719-4900

ADDRESS 472 SW SAPLING GLENN LAKE CITY FL 32024

CONTRACTOR GARY JOHNSON PHONE 752-3444

LOCATION OF PROPERTY 47S, TR ON CR 240, TL ON MAULDIN AVE, TR ON SAPLING GLEN, 4TH ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 109850.00

HEATED FLOOR AREA 2197.00 TOTAL AREA 3212.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 21

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 09-5S-16-03499-005 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 1.00

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
		RG0024685	Gary Johnson

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	06-0400-N	BK	JH	Y

COMMENTS: ONE FOOT ABOVE THE ROAD,

Check # or Cash 1583

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Computer Error date/app. by — Foundation date/app. by — Monolithic ~~12/21/2005~~ RJ date/app. by

Under slab rough-in plumbing ~~12/16/2005~~ ~~RJ~~ date/app. by — Slab date/app. by — Sheathing/Nailing ~~02/27/2006~~ ~~RJ~~ date/app. by

Framing ~~03/15/2006~~ ~~RJ~~ date/app. by — Rough-in plumbing above slab and below wood floor ~~03/15/2006~~ ~~RJ~~ date/app. by

Electrical rough-in ~~03/15/2006~~ ~~RJ~~ date/app. by — Heat & Air Duct ~~03/15/2006~~ ~~RJ~~ date/app. by — Peri. beam (Lintel) date/app. by

Permanent power ~~04/25/2006~~ ~~RJ~~ date/app. by — C.O. Final ~~04/28/2006~~ ~~RJ~~ date/app. by — Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by — Pool date/app. by

Reconnection date/app. by — Pump pole date/app. by — Utility Pole date/app. by

M/H Pole date/app. by — Travel Trailer date/app. by — Re-roof date/app. by

BUILDING PERMIT FEE $ 550.00 CERTIFICATION FEE $ 16.06 SURCHARGE FEE $ 16.06

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 657.12

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Permit Number:[type permit number]

Tax Folio Number: **03499-005**

State of: **Florida**
County of: **Columbia**

File Number: **06-347**

NOTICE OF COMMENCEMENT

The undersigned hereby gives notice that improvement will be made to certain real property, and, in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of Property:
   Section 9: Commence at the Southeast corner of Section 9, Township 5 South, Range 16 East, Columbia County, Florida and run thence South 89°22'51" West along the South line of said Section 9, 659.13 feet; thence North 0°26'36" West 440.60 feet; thence South 89°28'45" West 988.91 feet to the Point of Beginning; thence continue South 89°28'45" West 494.46 feet; thence North 0°29'07" West 884.60 feet to the North line of the South 1/2 of Southeast 1/4 of said Section 9; thence North 89°28'45" East along said North line 494.68 feet; thence South 0°28'17" East 884.61 feet to the Point of Beginning.

2. General Description of Improvements: Resdiential

3. Owner Information:
   a. Name and Address: Paul L. Wendel Jr and Gladys A. Wendel
   127 SW Kelliche, Lake City, Florida 32024

   b. Interest in property: Fee Simple

   c. Names and address of fee simple title holder (if other than owner):

4. Contractor: Gary Johnson Construction Inc

5. Surety: N/A

6. Lender: Peoples State Bank, 350 SW Main Blvd., Lake City, Florida 32025

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1) (a)7., Florida Statutes.

8. In addition to himself, Owner designates the following persons to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of Notice of Commencement (the expiration date is 1 year from date of recording unless a different date is specified):

Paul L Wendel Jr
Paul Leo Wendel Jr

Gladys Audrey Wendel

#24452

Sworn to and subscribed before me May 3, 2006 by Paul Leo Wendel Jr and Gladys Audrey Wendel who is personally known to me or who did provide Drivers Licenses as identification.

Megan M Marable
Notary Public
My Commission Expires:_____

MEGAN M MARABLE
MY COMMISSION # DD412865
EXPIRES: Mar. 30, 2009
Florida Notary Service.com

Inst:200610871 Date:05/04/2006 Time:10:36
DC,P.DeWitt Cason,Columbia County B:1082 P:

ELK Roofing Products Specifications – Tuscaloosa, AL

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE®

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 110 mph***

Prestique I *High Definition*

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 90 mph***

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 70 mph.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅝"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Vented RidgeCrest™ w/*FLX*™

Size: 13"x13¼"
Exposure: 9¼"
Pieces/Box: 26
Coverage: 5 boxes = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood.
Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

***See actual limited warranty for conditions and limitations.**
**** Effective January 1, 2004, the seven year non-prorated Umbrella Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for such products. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rake and eave edges, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, KY, MO, KS, CO, UT, NV, & OR.**
*****For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grandé, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.**

Specifications

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula *FLX*.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

Preparation of Roof Deck: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes[4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)], use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula *FLX* with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

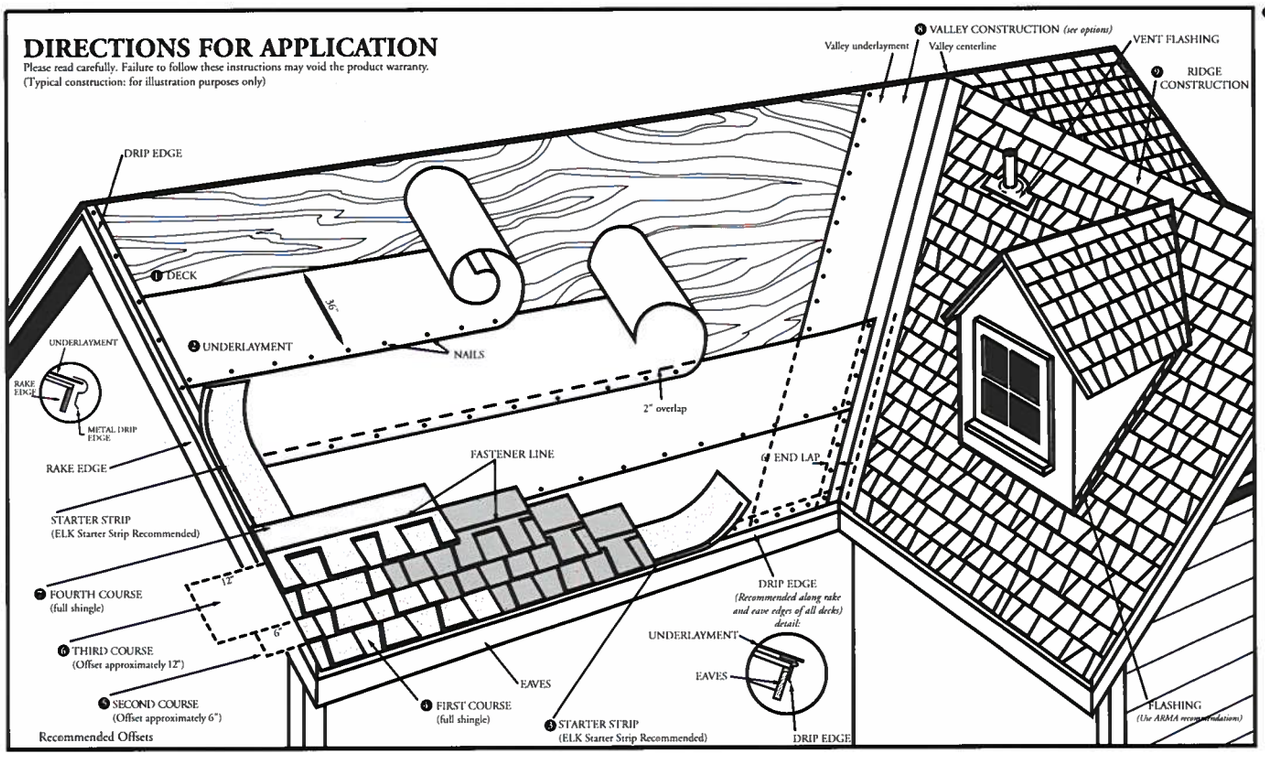
For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

Southeast & Atlantic Office: 800.945.5551

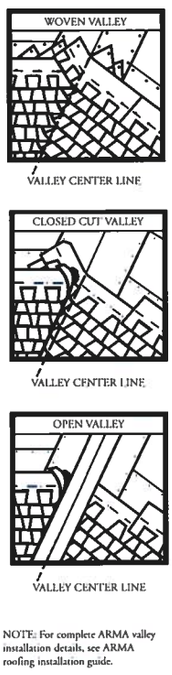
Corporate Headquarters: 800.354.7732

Plant Location: 800.945.5545

ELK The Premium Choice®
www.elkcorp.com



VALLEY CONSTRUCTION OPTION
(California Open and California Closed are also acceptable valley.)



NOTE: For complete ARMA valley installation details, see ARMA roofing installation guide.

DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

❶ DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

❷ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Elk Versashield® or self adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope(2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

❸ STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

❹ FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof

❺ SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

❻ THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

❼ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

❽ VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

❾ RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z®Ridge or Seal-A-Ridge® with formula *FLX*™ or RidgeCrest™ with *FLX* (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common bond area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

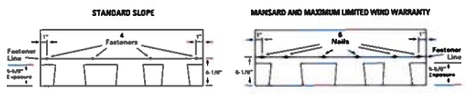
Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

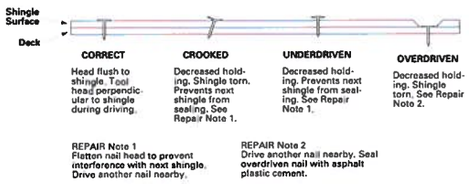
LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- * For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.





Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

ELK
The Premium Choice®
www.elkcorp.com

Columbia County Building Permit Application

left message

Revised 9-23-04

For Office Use Only Application # 0604-54 Date Received 4/20/06 By JN Permit # 24452

Application Approved by - Zoning Official BLK Date 24.04.06 Plans Examiner OKJTH Date 4-26-07

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments - NOC -

~~06-0400-N - EN. Health Site Plan-CS needed~~

Applicants Name GARY JOHNSON Phone 386-752-3444

Address PO BOX 1016 LAKE CITY, FL 32056-1016

Owners Name PAUL AND AUDREY WENDEL Phone 386-719-4900

911 Address 472 SW SAPPLING GLENN LC FL 32024

Contractors Name GARY JOHNSON CONST INC Phone 386-752-3444

Address PO BOX 1016 LC FL 32056-1016 386-961-3031

Fee Simple Owner Name & Address ______

Bonding Co. Name & Address ______

Architect/Engineer Name & Address ______

Mortgage Lenders Name & Address Peoples STATE BANK

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energy

Property ID Number 09-5S-16-03499-005 Estimated Cost of Construction 220000

Subdivision Name ______ Lot ____ Block ____ Unit ____ Phase ____

Driving Directions STATE ROAD 47, TR ON COUNTY ROAD 240, TL ON MAULDIN AVENUE, TR ON SW SAPLING GLENN, FORTH ON LEFT

Type of Construction New RESIDENCE Number of Existing Dwellings on Property 0

Total Acreage 10 Lot Size ____ Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive)

Actual Distance of Structure from Property Lines - Front 225' ✓ Side 200' ✓ Side 200' ✓ Rear 400' ✓

Total Building Height 21'6" Number of Stories 1 Heated Floor Area 2197 Roof Pitch 6/12

TOTAL 3212

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

GARY JOHNSON
Owner Builder or Agent (Including Contractor)

Gary Johnson
Contractor Signature

Contractors License Number RG0024685

Competency Card Number ______

NOTARY STAMP/SEAL

Notary Public State of Florida
Tracy L Duckett
My Commission DD462701
Expires 08/31/2009

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 20th day of April 2006.

Personally known ✓ or Produced Identification ____

Tracy L. Duckett
Notary Signature

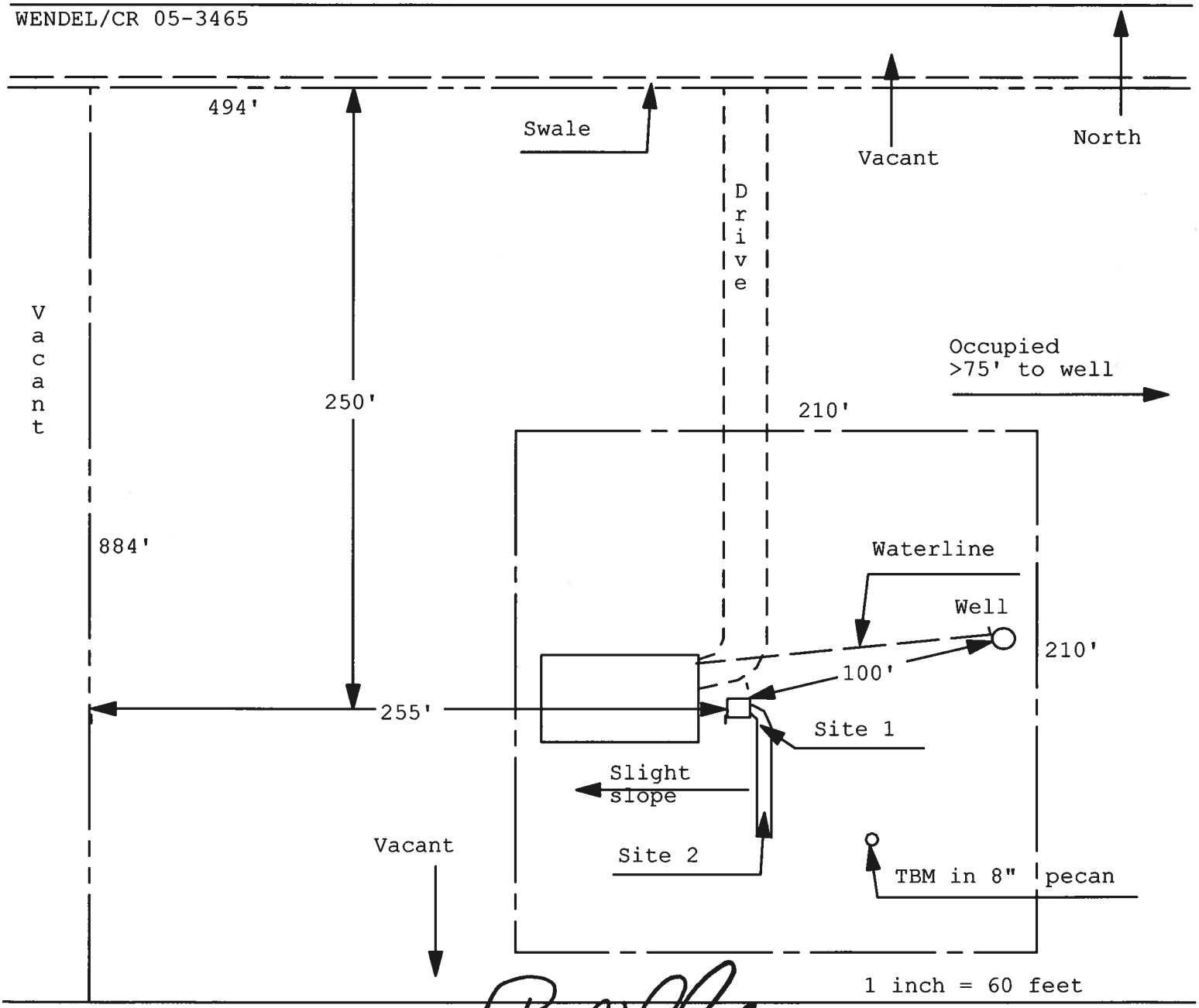
0604-5-4

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0400N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

WENDEL/CR 05-3465

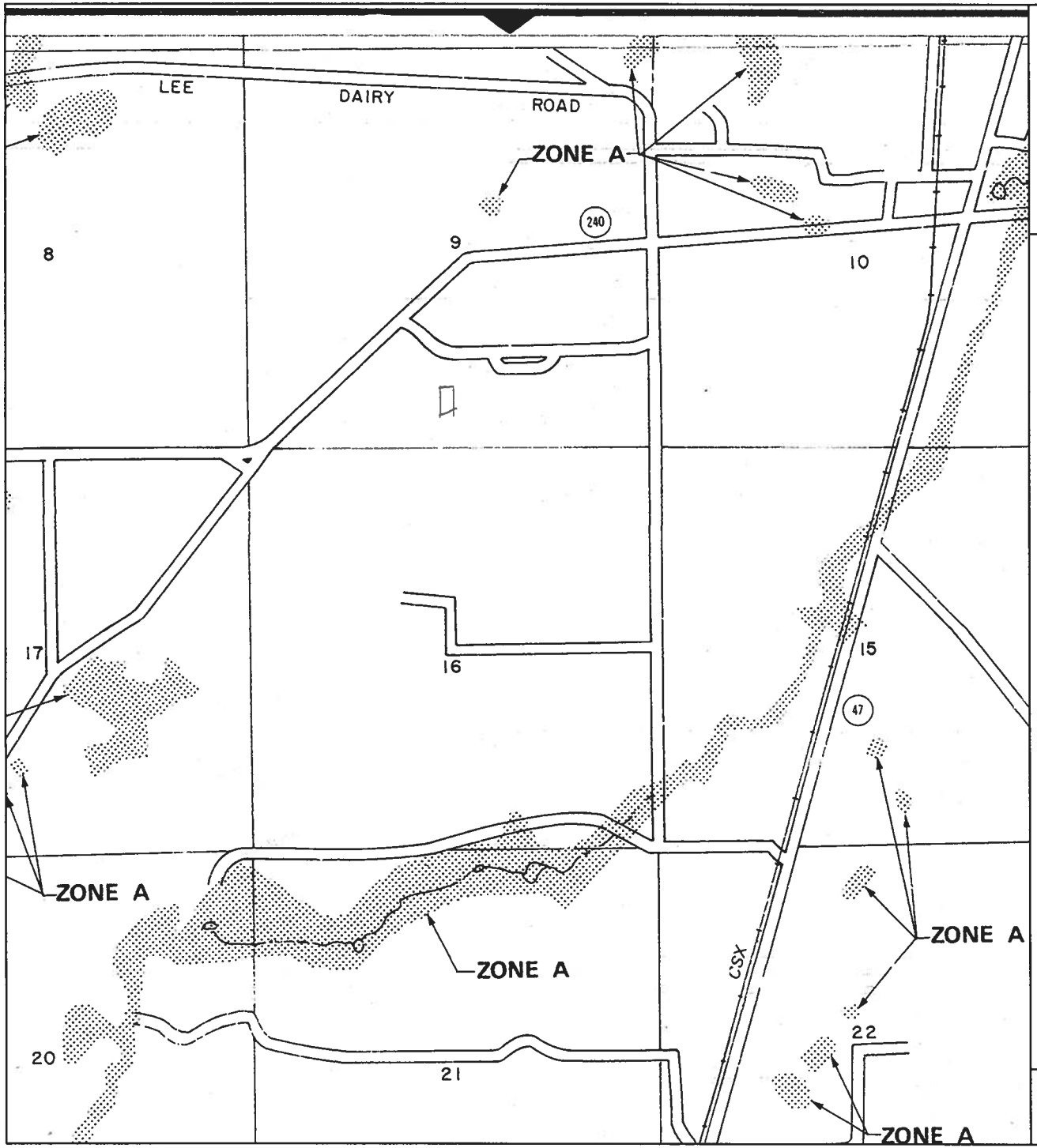


Site Plan Submitted By Paul Lloyd **Date** 4/12/06

Plan Approved ✓ **Not Approved** _____ **Date** 4/21/06

By _____ Columbia **CPHU**

Notes: _____



N

APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

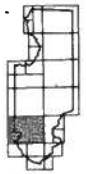
FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 225 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER
120070 0225 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 4/20/2006 (printed at scale and type A)

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 600B-97 — **Residential Component Prescriptive Method B** — **Department of Community Affairs** — **NORTH 1 2 3**

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B-97 for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C-97. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

PROJECT NAME AND ADDRESS:	WENDEL JOB	**BUILDER:** GARY JOHNSON CONST INC	
		PERMITTING OFFICE:	**CLIMATE ZONE:** 1 ☐ 2 ☐ 3 ☑
OWNER:	PAUL WENDEL	**PERMIT NO.:** 24452	**JURISDICTION NO.:** 221000

GENERAL DIRECTIONS

1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass.
2. Choose one of the component packages "A" through "E" fromTable 6B-1 by which you intend to comply with the Code. Circle the column of the package you have chosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages", Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

	Please Print	CK
1. **Compliance package chosen (A-F)**	1.	
2. **New construction or addition**	2.	
3. **Single family detached or Multifamily attached**	3.	
4. **If Multifamily—No. of units covered by this submission**	4.	
5. **Is this a worst case?** (yes / no)	5.	
6. **Conditioned floor area** (sq. ft.)	6. 2197	
7. **Predominant eave overhang** (ft.)	7. 2	
8. **Glass type and area :**	Single Pane / Double Pane	
a. Clear glass	8a. _____ sq. ft. / 275 sq. ft.	
b. Tint, film or solar screen	8b. _____ sq. ft. / _____ sq. ft.	
9. **Percentage of glass to floor area**	9. 12 %	
10. **Floor type, area or perimeter, and insulation:**		
a. Slab on grade (R-value)	10a. R= _____ _____ lin. ft.	
b. Wood, raised (R-value)	10b. R= _____ _____ sq. ft.	
c. Wood, common (R-value)	10c. R= _____ _____ sq. ft.	
d. Concrete, raised (R-value)	10d. R= _____ _____ sq. ft.	
e. Concrete, common (R-value)	10e. R= _____ _____ sq. ft.	
11. **Wall type, area and insulation:**		
a. Exterior: 1. Masonry (Insulation R-value)	11a-1 R= _____ _____ sq. ft.	
2. Wood frame (Insulation R-value)	11a-2 R= 13 _____ sq. ft.	
b. Adjacent: 1. Masonry (Insulation R-value)	11b-1 R= _____ _____ sq. ft.	
2. Wood frame (Insulation R-value)	11b-2 R= _____ _____ sq. ft.	
12. **Ceiling type, area and insulation:**		
a. Under attic (Insulation R-value)	12a. R= 30 _____ sq. ft.	
b. Single assembly (Insulation R-value)	12b. R= _____ _____ sq. ft.	
13. **Air Distribution System: Duct insulation, location**	13. R= _____	
14. **Cooling system**	14a. Type: CENTRAL	
(Types: central, room unit, package terminal A.C., gas, none)	14b. SEER/EER: 12	
	14c. Capacity: _____	
15. **Heating system:**	15a. Type: HEAT PUMP	
(Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none)	15b. HSPF/COP/AFUE:	
	15c. Capacity: 7.7	
16. **Hot water system:**	16a. Type: ELECTRIC	
(Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none)	16b. EF: .90	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: Gary Johnson **DATE:** 3/27/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

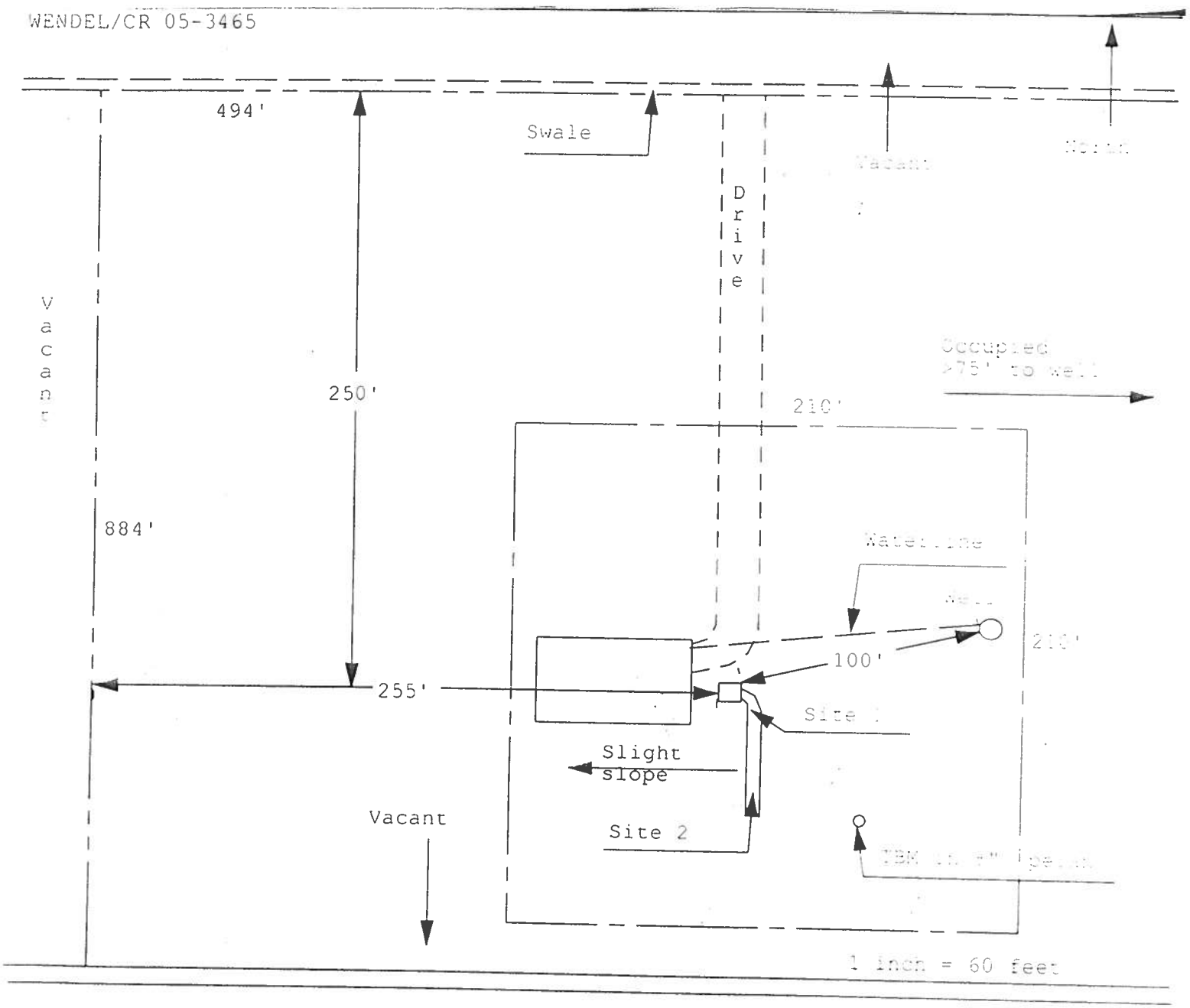
OWNER AGENT: _____ **DATE:** _____

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

WENDEL SITE PLAN



GARY JOHNSON CONST. INC.
P.O. BOX 1016
LAKE CITY, FL. 32056-1016
(386) 752-3444

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
904 NW Main Blvd.
LAKE CITY, FLORIDA 32055

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID 03-840
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Inst:200401543B Date:07/02/2004 Time:12:38
Doc Stamp-Deed : 350.00
DC,P.DeWitt Cason,Columbia County B:1019 P:2682

Property Appraiser's
Identification Number Part of R03499-000

WARRANTY DEED

THIS INDENTURE, made this 14th day of June, 2004, BETWEEN PAUL S. BRYAN and PATRICIA K. BRYAN, Husband and Wife and GLENN A. KHACHIGAN and MARTHA JO KHACHIGAN, Husband and Wife, whose post office address is Route 13, Box 190, Lake City, FL 32055, of the County of Columbia, State of Florida, grantor*, and PAUL LEO WENDEL, JR., and GLADYS AUDREY WENDEL, Husband and Wife, whose post office address is Post Office Box 2305, Lake City, FL 32056, of the County of Columbia, State of Florida, grantee*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

N.B.: Grantor herein reserves a non exclusive perpetual easement for ingress and egress over and across the East 30 feet thereof which shall at all times remain open and unobstructed.

N.B.: Grantor herein reserve a non exclusive perpetual easement for utilities over and across the North 15 feet, the West 10 feet and the West 10 feet of the East 30 feet thereof.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

*"Grantor" and "grantee" are used for singular or plural, as context requires.

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered
in our presence:

(Signature of First Witness)
Lisa C. Ogburn
(Typed Name of First Witness)

(Signature of Second Witness)
Myrtle Ann McElroy
(Typed Name of Second Witness)

(SEAL)
Grantor
PAUL S. BRYAN
Printed Name

(SEAL)
Grantor
PATRICIA K. BRYAN
Printed Name

(SEAL)
Grantor
GLENN A. KHACHIGAN
Printed Name

(SEAL)
Grantor
MARTHA JO KHACHIGAN
Printed Name

Inst:2004015438 Date:07/02/2004 Time:12:38
Doc Stamp-Deed : 350.00
DC,P.DeWitt Cason,Columbia County B:1019 P:2683

STATE OF Florida
COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 25th day of June, 2004, by PAUL S. BRYAN and PATRICIA K. BRYAN, Husband and Wife, who are personally known to me or who have produced _____ as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

LISA C. OGBURN
MY COMMISSION # DD 154314
EXPIRES: October 13, 2006
Bonded Thru Notary Public Underwriters

STATE OF Florida
COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 25th day of June, 2004, by GLENN A. KHACHIGAN and MARTHA JO KHACHIGAN, Husband and Wife, who are personally known to me or who have produced _____ as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

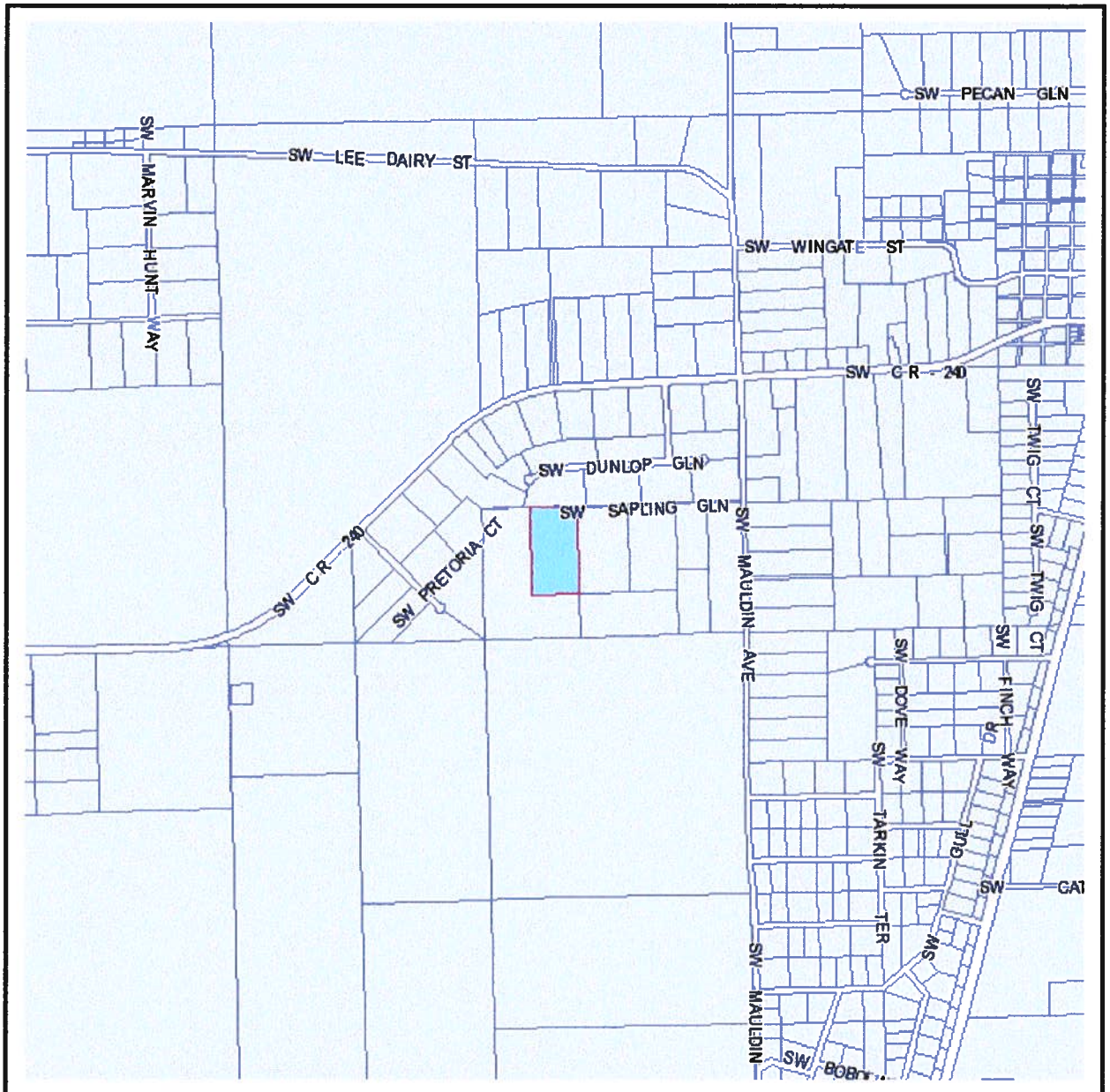
LISA C. OGBURN
MY COMMISSION # DD 154314
EXPIRES: October 13, 2006
Bonded Thru Notary Public Underwriters

EXHIBIT "A"

TOWNSHIP 5 SOUTH - RANGE 16 EAST

SECTION 9: Commence at the Southeast corner of Section 9, Township 5 South, Range 16 East, Columbia County, Florida and run thence S 89°22'51" W along the South line of said Section 9, 659.13 feet; thence N 0°26'36" W 440.60 feet; thence S 89°28'45" W 988.91 feet to the POINT OF BEGINNING; thence continue S 89°28'45" W 494.46 feet; thence N 0°29'07" W 884.60 feet to the North line of the S 1/2 of SE 1/4 of said Section 9; thence N 89°28'45" E along said North line 494.68 feet; thence S 0°28'17" E 884.61 feet to the POINT OF BEGINNING.

Inst:2004015438 Date:07/02/2004 Time:12:38
Doc Stamp-Deed : 350.00
DC,P.DeWitt Cason,Columbia County B:1019 P:2684



Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 09-5S-16-03499-005 - NO AG ACRE (009900)

COMM AT SE COR OF SEC RUN W 659.13 FT, N 440.60 FT, W 988.91 FT FOR POB, CONT

Name:	WENDEL PAUL LEO JR &	LandVal	$70,350.00
Site:		BldgVal	$0.00
	GLADYS AUDREY WENDEL	ApprVal	$70,350.00
Mail:	P O BOX 2305	JustVal	$70,350.00
	LAKE CITY, FL 32056	Assd	$70,350.00
Sales Info	6/14/2004 $50,000.00 V / Q	Exmpt	$0.00
		Taxable	$70,350.00

0 0.1 0.2 0.3 mi

This information, GIS Map Updated: 4/6/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0604-54

Gary Johnson Owner Paul Wendel 472 SW Sappling Glen.

On the date of April 21 2006 application 0604-54 and plans for placement of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0604-54 when making reference to this application.

1. On the dwelling elevation drawing show the height of the chimney flue above the roof peak and the distance from the nearest roof line intersection.
2. Show the total height of the dwelling from the established grade.
3. In the garage area show the method of protecting the appliances as required by the Florida Mechanical Code, Sections: 303.4 Protection from damage: Appliances shall not be installed in a location where

subject to mechanical damage unless protected by approved barriers.

4. Show the method of compliance for sections R309.1.1 of the FRC-2004: Duct penetration: Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage.
5. Please show compliance with the FRC-2004 sections R309 Garage: R309.1 A: Opening protection: Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.
6. In the master bathroom show compliance with the FRC-2004 sections R308.4 Hazardous locations: Glazing in doors and enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers. Glazing in any part of a building wall enclosing these compartments where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface. Each pane of glazing installed in hazardous locations as defined in Section R308.4 shall be provided with a manufacturer's or installer's label, designating the type and thickness

of glass and the safety glazing standard with which it complies, which is visible in the final installation. The label shall be acid etched, sandblasted, ceramic-fired, embossed mark, or shall be of a type which once applied cannot be removed without being destroyed.

7. Show on the electrical plan the location of the electrical service overcurrent protection device and the amperage rating of this device. This device shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.
8. Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.
9. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be preformed by the Columbia County Building Department.
10. Submit a site plan which defines the following information.
    a. Dimensions of lot
    b. Dimensions of building set backs
    c. Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
    d. Provide a full legal description of property.

I Understand and will comply with the above.

Gary Johnson

Gary Johnson 4-21-06

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

Prepared By: Harry Hts + ASR

TOTAL HEATING AND COOLING REQUIREMENTS

or:

ame: Gary Johnson const

ddress: Wendel Resd.

ity:

Constr. Type	ITEM			AREA SQUARE FEET	DESIGN TEMPERATURE DIFFERENCE 30° / 35° / 40° / 45° / 50° — HEATING MULTIPLIER (CIRCLE ONE)					HEATING (BTUH LOSS)	DESIGN TEMP 90° / 95° — COOLING MULT. (CIRCLE)		COOLING (BTUH GAIN)
	Gross Wall Area			1352									
	Glass Area (From page 1)			225						8175			15865
	Partitions, Frame												
	Finished 1 side, No Insulation				17	19	22	25	28		6.5	10.0	
	Finished 2 sides, No Insulation				9	11	12	14	16		4.5	6.0	
	Finished 2 sides, R-5				4	5	5.5	6	7		2.5	3.5	
	Finished 2 sides, R-11			288	2	3	(3)	4	4	864	2.0	2.5	720
	Other												
	Doors (Excluding glass)												
	No weatherstripping				135	160	180	200	225		10.0	13.0	
	Weatherstripped				70	85	95	110	120		10.0	13.0	
	R-5 Insulation, No weatherstripping				123	144	164	185	205		4.3	5.5	
	R-5 Insulation, weatherstripping			20	68	79	(90)	101	113	1800	4.0	(5.0)	100
	Other												
	Net Exterior Walls												
	CBS Furred, No Insulation				9	10	12	13	14		4.5	6.0	
	CBS Furred, R-3 Insulation				5	6	7	8	8		3.0	4.2	
	CBS Furred, R-4 Insulation				4	5	6	6	7		2.7	3.8	
	CBS Furred, R-5 Insulation				4	5	5	6	6		2.5	3.5	
	Frame, No Insulation				8	9	10	11	13		5.5	7.0	
	Frame, R-11 Insulation			1327	2	2	(3)	3	4	3981	2.5	3.0	3981
	Frame, R-14 Insulation				1.5	1.7	2	2.5	3		2	2 8	
	Other												
	Ceiling under attic	Roof DK	LT								9 7	10 8.5	
	No Insulation	DK	LT		18	21	24	27	30		2.5 2	3 2.5	
	R-11 Insulation	DK	LT		2.4	2.8	3.2	3.5	3.9		1.5 1.5	2 1.5	
	R-19 Insulation	DK	LT		1.5	1.7	1.9	2.2	2.4		1.5 1.0	1.5 1.5	
	R-22 Insulation	DK	LT		1.2	1.5	1.7	1.9	2.1		1.3 1	1.5 1.2	
	R-26 Insulation	DK	LT		1.1	1.3	1.4	1.6	1.8		1.1 .9	(1.3) 1.0	2856
	R-30 Insulation	(DK)	LT	2197	1	1.1	(1.3)	1.4	1.6	2856			
	Other												
	Floor, Concrete Slab			Perimeter Ft.									
	No Edge Insulation			194	35	40	(40)	45	45	7760	0	0	
	Other												
	Subtotal									26436			23522
	People @ 300 & Appl. @ 1200												7500
	Sensible BTUH Gain												
	Duct BTUH Loss & Gain									25436			31022
	2 In. Flex. or 1 In. Rigid						.10			2544		.10	3102
	1½ In. Rigid						.075					.075	
	Total BTUH Loss									27980			
	Subtotal BTUH Gain												34174
	x 1.3 = Total BTUH Gain												44361

ulated Heating Requirements 27980 BTUH
of Unit Chosen . . . . . 48,000 BTUH
ersized . . . . . .
ndersized . . . . . .

Calculated Cooling Requirements 44361 BTUH
Size of Unit Chosen . . . . . 48000 BTUH
% Oversized . . . . . .
% Undersized . . . . . .

RESIDENTIAL HEATING AND COOLING REQUIREMENTS*

HVAC WORKSHEET FOR WATT-WISE LIVING

HEATING AND COOLING REQUIREMENTS DUE TO GLASS AREA

WINDOWS & GLASS DOORS	AREA SQUARE FEET	DESIGN TEMPERATURE DIFFERENCE 30°	35°	40°	45°	50°	HEATING (BTUH LOSS)
Glass Doors, Infiltration less than 1.0 CFM/FT							
Single Glass							
Double Glass		50	60	70	75	85	
Other Sliding Glass Doors	20	40	45	(50)	55	60	1000
Single Glass							
Double Glass		75	85	100	115	125	
Windows, Infiltration less than 0.50 CFM/FT		60	70	80	90	100	
Single Glass							
Double Glass		40	50	55	60	70	
Windows, Infiltration less than 0.75 CFM/FT	205	25	30	(35)	40	45	7175
Single Glass							
Double Glass		45	50	60	66	75	
Other Windows		30	35	40	45	50	
Single Glass							
Double Glass		75	90	105	115	130	
Fixed or Picture Windows		60	70	80	90	105	
Single Glass							
Double Glass		40	50	55	60	70	
Other		25	30	35	40	45	
Total BTUH Loss (Enter on Line 2, Page 2)							8175

WINDOWS & GLASS DOORS	AREA SQUARE FEET	COOLING MULTIPLIER (CIRCLE) SINGLE GLASS 90° C	T	R	95° C	T	R	DOUBLE GLASS 90° C	T	R	95° C	T	R	COOLING (BTUH GAIN)
No Shading														
N	9	30	22	20	30	26	25	20	14	13	(25)	17	16	225
NE & NW		60	41	36	65	45	41	50	29	24	50	32	27	
E & W	200	85	60	53	90	64	57	70	44	36	(75)	47	39	15000
SE & SW		75	51	45	80	55	50	60	37	30	65	40	33	
S	16	45	31	28	50	35	33	35	21	18	(40)	24	21	640
Draperies or Blinds														
N		20	17	16	25	21	20	15	11	11	20	14	14	
NE & NW		35	33	30	40	37	34	30	22	21	36	25	24	
E & W		55	48	43	55	52	47	45	32	30	50	35	33	
SE & SW		45	39	35	50	43	39	40	26	25	40	29	28	
S		30	26	24	30	30	28	25	17	16	25	20	19	
Roller Shades														
N		25	19	17	25	23	22	20	12	11	20	15	14	
NE & NW		45	36	32	50	40	37	40	26	22	45	29	25	
E & W		65	53	47	70	57	51	55	37	32	60	40	35	
SE & SW		55	44	39	60	48	44	50	32	27	50	35	30	
S		35	28	25	40	32	30	30	20	16	35	23	19	
Awnings, Porches, Etc.														
All Directions		25	22	20	30	26	25	15	14	13	20	17	16	
Other														
Total BTUH Gain (Line 2, Page 2)														15865

*REFERENCE A.C.C.A. MANUAL "N"

ARCHITECTURAL TESTING INC.

130 Derry Court • York, PA 17402-9405
web www.testati.com • Facsimile 717-764-4129 • Telephone 717-764-7700

Twin

STRUCTURAL TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Twin Aluminum Single Hung Window

Title of Test	Results
Overall Design Pressure	35.0 psf
Operating Force	18 lb max.
Air Infiltration	0.29 cfm/ft^2
Water Resistance	5.25 psf
Structural Test Pressure	70.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-36060.01 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Scott D. Kramer, Technician

SDK:nlb

ARCHITECTURAL TESTING INC.

130 Derry Court • York, PA 17402-8405
web www.testati.com • Facsimile 717-764-4129 • Telephone 717-764-7700

STRUCTURAL TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-36060.01
Test Date: 11/04/99
Report Date: 11/29/99
Expiration Date: 11/04/03

Project Summary: Architectural Testing, Inc. (ATI) was contracted to perform tests on a Series/Model 650, twin aluminum single hung window at MI Home Products' test facility in Elizabethville, Pennsylvania. Test specimen description and results are reported herein.

Test Specification: The test specimen was evaluated in accordance with the following:

ASTM E 283-91, *Standard Test Method for Determining the Rate of Air Leakage Through Exterior Windows, Curtain Walls, and Doors Under Specified Pressure Differences Across the Specimen*

ASTM E 330-97, *Standard Test Method for Structural Performance of Exterior Windows, Curtain Walls, and Doors by Uniform Static Air Pressure Difference*

ASTM E 547-96, *Standard Test Method for Water Penetration of Exterior Windows, Curtain Walls, and Doors by Cyclic Static Air Pressure Differential*

Test Specimen Description:

Series/Model: 650

Type: Twin Aluminum Single Hung Window

Overall Size: 5' 10-1/4" wide by 5' 0" high

Active Size (2): 2' 8-3/4" wide by 2' 6-1/4" high

Fixed Daylight Opening Size (2): 2' 6- 1/4" wide by 2' 3" high

Screen Size (2): 2' 7-3/4" wide by 2' 4-1/4" high

Test Specimen Description: (Continued)

Finish: All aluminum was painted white.

Glazing Details: Both the active sash and fixed lites utilized 5/8" thick insulating glass fabricated from two sheets of 3/32" thick clear annealed glass and a desiccant filled metal spacer system. The active sash were channel glazed with a flexible wedge gasket. The fixed lites were interior glazed, back bedded with single sided adhesive foam tape and held-in place with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.270" backed by 0.190" high polypile with center fin	1 Row	Fixed meeting stile
3/8" high vinyl wrapped foam bulb	1 Row	Bottom rail
0.187" backed by 0.250" high polypile with center fin	2 Rows	Stiles
1/4" high polypile dust plug	2 Rows	Ends of bottom rail, top of each stile

Frame Construction: Frame was constructed of extruded aluminum members and all corners were coped, butted, sealed, and fastened with two screws per corner. The fixed meeting rail was attached to the jambs with a plastic clip and two screws per end.

Mullion Construction: The mullion was constructed of an extruded aluminum member. It was fastened to the head and sill with four screws per end. All screw heads were sealed as well as the butt joint at the sill.

Sash Construction: The sash were constructed of extruded aluminum members and all corners were coped, butted, and fastened with one screw per corner.

Screen Construction: The screen was constructed of rolled aluminum members and the corners were keyed. The screen mesh was held-in-place with a flexible spline.

Test Results:

The results are tabulated as follows:

Title of Test - Test Method	Results
Air Infiltration per ASTM E 283	
@ 0.56 psf (15 mph)	0.15 cfm/ft^2
@ 1.57 psf (25 mph)	0.29 cfm/ft^2
Water Resistance per ASTM E 547 (with and without screen)	
WTP = 5.25 psf	No leakage
Uniform Load Structural (Measurements reported were taken on the meeting rail) (load held for 33 seconds)	
@ 47.0 psf (exterior)	0.010"
@ 47.0 psf (interior)	0.015"
Uniform Load Structural (Measurements reported were taken on the meeting rail) (load held for 10 seconds)	
@ 70.5 psf (exterior)	0.060"
@ 70.5 psf (interior)	0.040"

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Scott D. Kramer
Technician

Bruce W. Croak
Project Manager

SDK:nlb
01-36060.01

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Plastic tilt latches	4	Ends of interior meeting rail
Metal pivot bars	4	Ends of the bottom rails
Metal sweep lock	2	Midspan of interior meeting rail
Metal keeper	2	Midspan of fixed meeting rail
Sash stops	4	One per jamb
Block and tackle balance system	4	One per jamb
Spring loaded latch pins	2	6" from ends of screen top rail

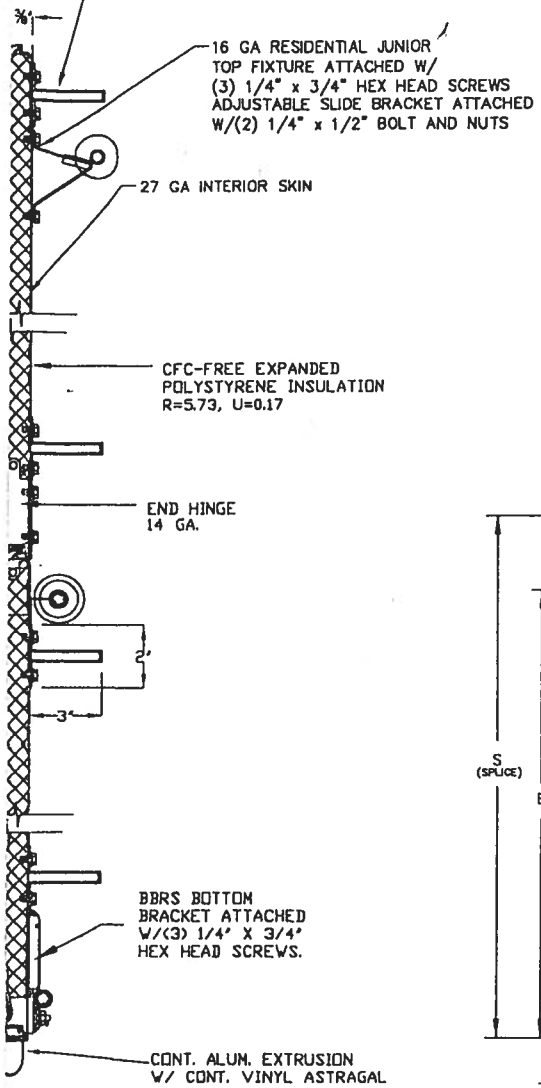
Drainage:

Description	Quantity	Location
Sloped sill	1	Sill
1/4" wide by 3/16" high weepslot	4	Ends of exterior vertical sill leg

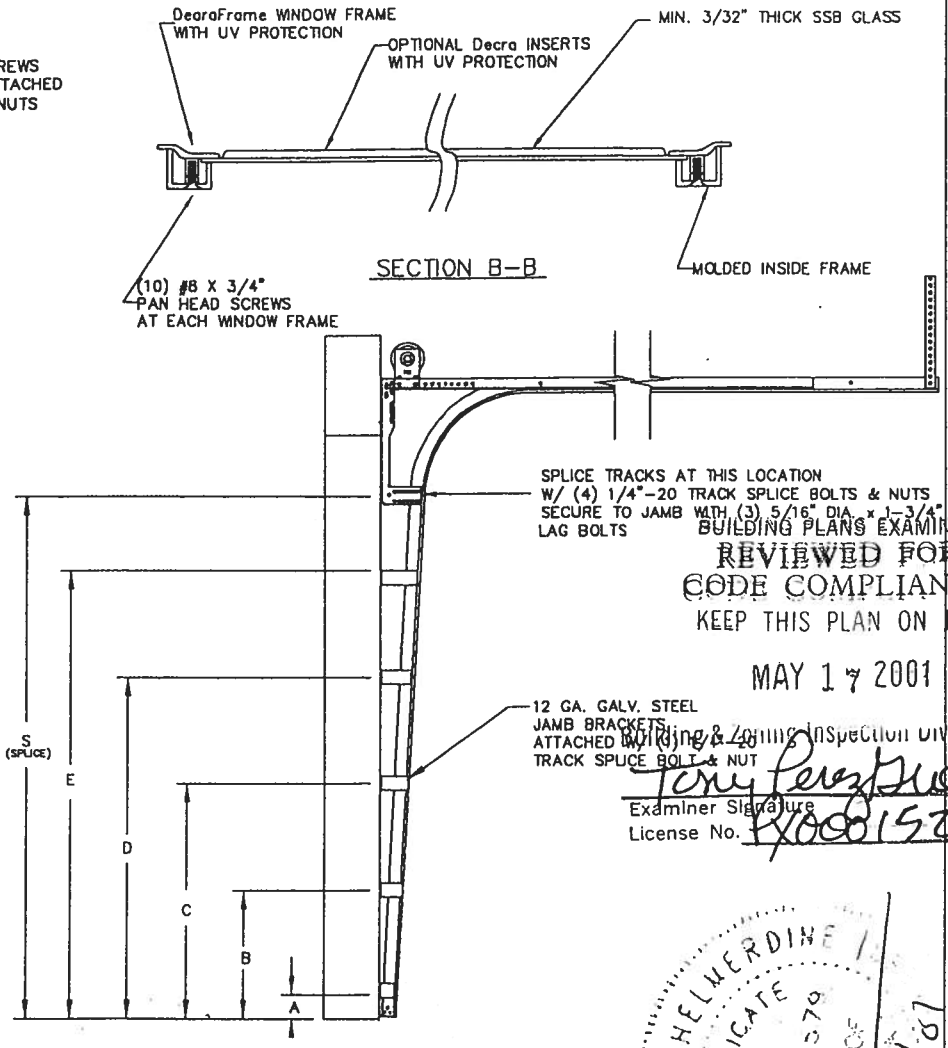
Installation: The test unit was installed into the 2" x 8" nominal Spruce-Pine-Fir #2 wood test buck utilizing the integral nailing fin and 1" roofing nails. Five per top, bottom, and sides of the nail fin were evenly spaced. The nail fin was bedded in a silicone sealant.

GLAZING OPTION CROSS SECTION

TEST No. SBC-580-020 ON MAY 24, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +49.5 PSF AND -51.9 PSF. BY COMPARISION, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' X 7' AND 16' X 8' MODEL 1500-D DOORS.



A-A (SIDE VIEW)



TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

BUILDING PLANS EXAMINER
REVIEWED FOR
CODE COMPLIANCE
KEEP THIS PLAN ON JOB

MAY 17 2001

Building & Zoning Inspection Div. - Jax., FL.

Examiner Signature

License No. RX0001520

THOMAS L. SHELWERDINE
CERTIFICATE NO. 48579
STATE OF FLORIDA

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18-1/2"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"

CHMENT TO STRUCTURE

STEST-MILE BASIC WIND SPEEDS

)OD FRAME STRUCTURE
6" FROM ENDS THEN 24" O.C.

)00 PSI CONCRETE
IG 6" FROM ENDS THEN 24" O.C.
/4" STARTING 6" FROM ENDS THEN 24" O.C.
STARTING 6" FROM ENDS THEN 24" O.C.

:90 BLOCK
/4" STARTING 6" FROM ENDS THEN 24" O.C.
)" STARTING 6" FROM ENDS, USE PAIRS OF

JNK TO PROVIDE A FLUSH MOUNTING SURFACE.
THERS

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.016") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

MAX SIZE
16' x 8'

DESIGN LOADS
+29.5 PSF
-29.5 PSF

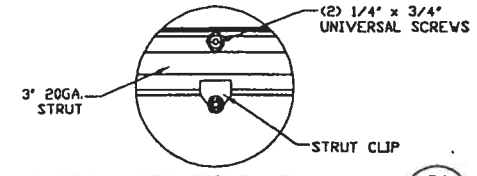
TEST LOADS
+49.3 PSF
-49.3 PSF

REV	DESCRIPTION OF REVISIONS	DATE	BY

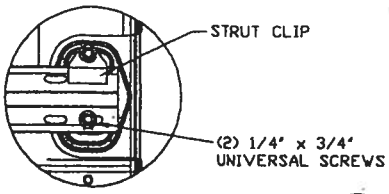
Amarr GARAGE DOORS
5921 GRASSY CREEK BLVD. WINSTON-SALEM, N.C. 27105

MODEL #1500 WeatherGuard

SIZE	DRAWN BY CJJ	DATE 04/16/01	DRAWING NUMBER
B	CHECKED BY	DATE	SBC-580-019-J
SCALE: NOT TO SCALE			SHEET 1 OF 1

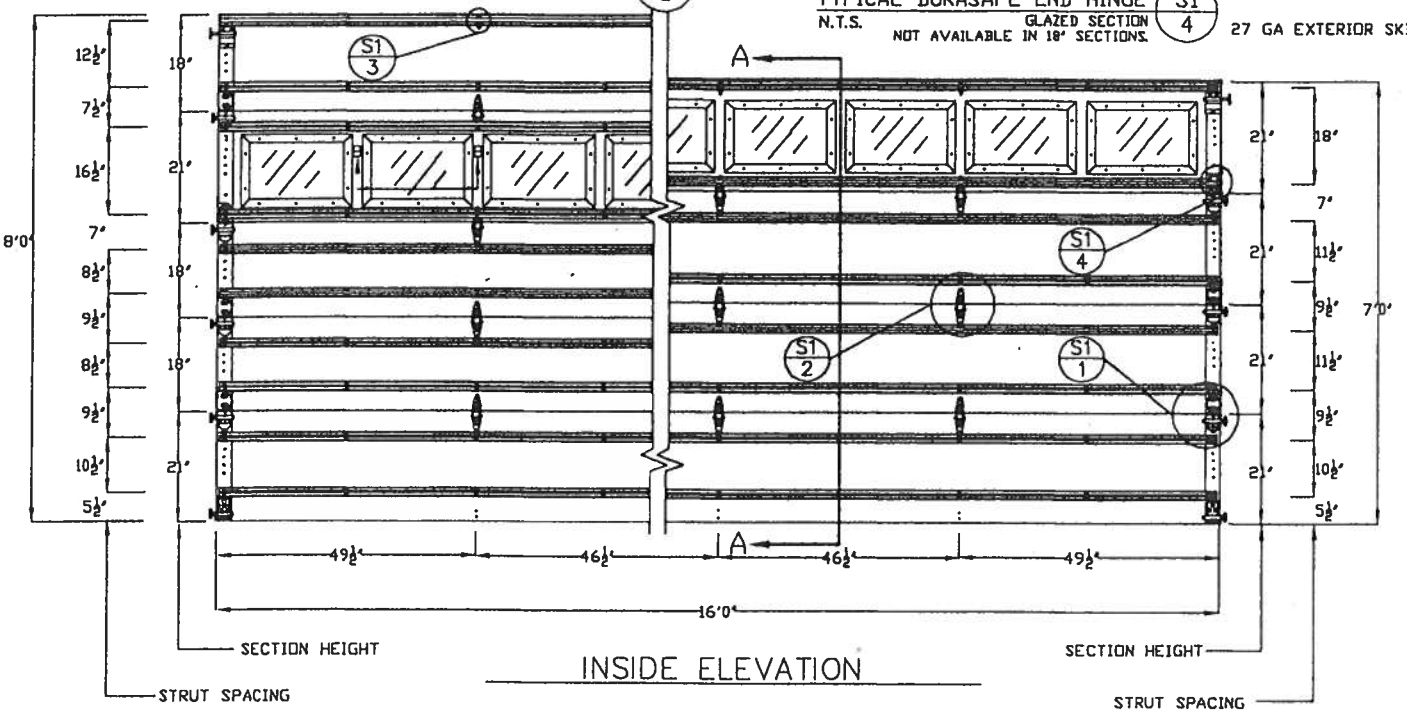


TYPICAL TOP STRUT ATTACHMENT S1/3
N.T.S.

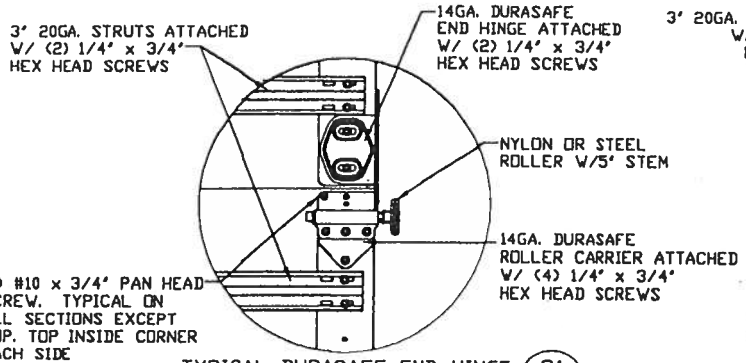


TYPICAL DURASAFE END HINGE S1/4
N.T.S.
GLAZED SECTION
NOT AVAILABLE IN 18" SECTIONS.

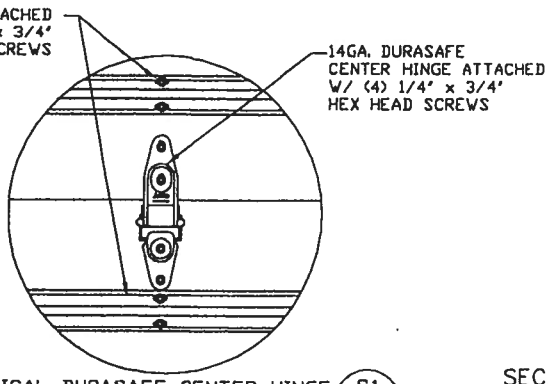
27 GA EXTERIOR SKIN



INSIDE ELEVATION

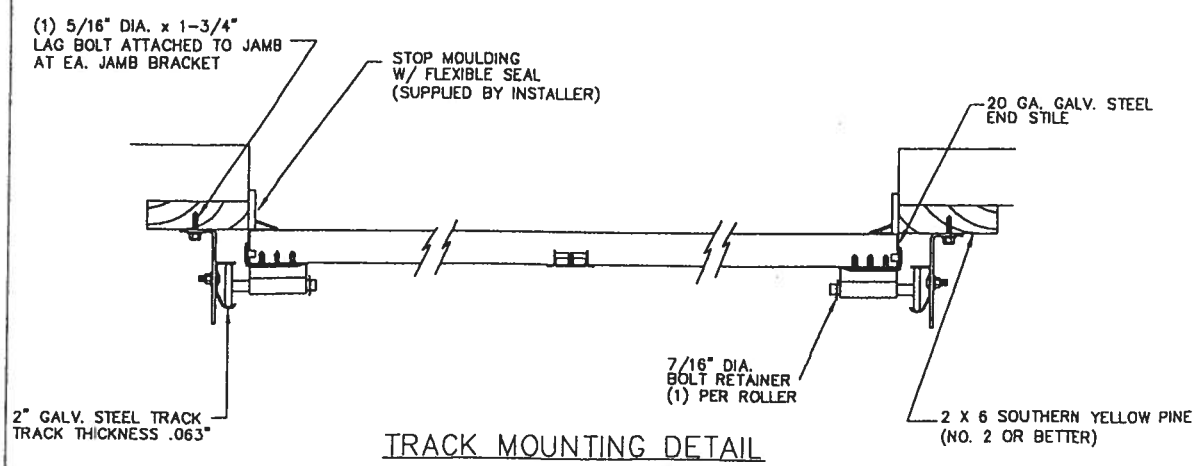


TYPICAL DURASAFE END HINGE S1/1
N.T.S.
NON GLAZED SECTION



TYPICAL DURASAFE CENTER HINGE S1/2
N.T.S.

SECTIO



TRACK MOUNTING DETAIL

WOOD JAMB A

RATED FOR 110 M

VERTICAL JAMB ATTACHMENT
5/16" X 3" LAG SCREWS STA

VERTICAL JAMB ATTACHMENT
HILTI KWIK BOLT 3/8" X 4" S
HILTI SLEEVE ANCHOR 3/8" X
ITW/RAMSET RED HEAD 3/8"

VERTICAL JAMB ATTACHMENT
HILTI SLEEVE ANCHOR 3/8" X
ITW/RAMSET TAPCON 1/4" X
FASTENERS (3" APART) AT 1

*LAGS AND BOLTS CAN BE COU
*PREPARATION OF WOOD JAMBS

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049

Architectural Testing

Single

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 450/650
TYPE: Aluminum Single Hung Window
RATING: H-C30 54 x 90; H-C45 52 x 72*

	Results	
Title of Test	**Test Specimen #1**	**Test Specimen #2**
Overall Design Pressure	30 psf	47 psf
Operating Force	20 lb max.	N/A
Air Infiltration	0.27 cfm/ft^2	N/A
Water Resistance	5.25 psf	6.0 psf
Structural Test Pressure	±45.0 psf	±70.5 psf
Deglazing	Passed	N/A
Forced Entry Resistance	Grade 10	N/A

Reference should be made to Report No. 01-37589.01 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Adam A. Fodor, Technician

AAF:tjp

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INCORPORATED
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-37589.01
Test Date: 06/29/00
Report Date: 09/11/00
Expiration Date: 06/29/04

Project Summary: Architectural Testing, Inc. (ATI) was contracted to witness tests on a Series/Model 450, aluminum single hung window at the MI Home Products in-plant test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1 H-C30 54 x 90; Test Specimen #2 H-C40 52 x 72*. Test specimen descriptions and results are reported herein.

General Note: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the minimum test size for the product type and class.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 450

Type: Aluminum Single Hung Window

Test Specimen #1 H-C30 54 x 90

Overall Size: 4' 6-1/2" wide by 7' 6-1/2" high

Sash Size: 4' 4" wide by 3' 9-3/4" high

Fixed Daylight Opening Size: 4' 1-1/2" wide by 3' 6-1/2" high

Screen Size: 4' 2-1/4" wide by 3' 8-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

Test Specimen Description: (Continued)

Test Specimen #2: H-C40 52 x 72*

Overall Size: 4' 4-1/4" wide by 6' 0" high

Sash Size: 4' 2" wide by 3' 0-1/2" high

Fixed Daylight Opening Size: 3' 11-1/2" wide by 2' 9-1/2" high

Screen Size: 4' 0" wide by 2' 11" high

The following descriptions apply to all specimens.

Finish: All aluminum was painted.

Glazing Details: The lites utilized 5/8" thick sealed insulating glass units fabricated from two sheets of 3/32" thick clear annealed glass and an Intercept™ spacer system. The sash was channel glazed with a flexible gasket. The fixed lite was interior glazed onto single-sided adhesive foam tape and secured with extruded PVC glazing beads.

Weatherstripping:

Description	Quantity	Location
0.210" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Stiles
0.300" diameter by 0.187" backed foam-filled vinyl bulb gasket	Row	Bottom rail
0.400' high by 1/2" square polypile dust plug	4	One on each sash corner

Frame Construction: The main frame was constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners. The fixed meeting rail was constructed of an extruded aluminum member with coped, butted and sealed ends fastened with two screws each.

Test Specimen Description: (Continued)

Sash Construction: The sash members were constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners fastened with one screw each.

Screen Construction: The screen was constructed of rolled aluminum members with plastic keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Plastic snap latch	1	Midspan of bottom rail
Block and tackle balance system	2	One per jamb
Plastic tilt latch	2	One on each end of sash meeting rail
Metal pivot bar	2	One on each end of bottom rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test unit was installed into the nominal 2" x 8" Spruce-Pine-Fir #2 wood test buck utilizing the integral nailing fin secured with 1" long galvanized roofing nails, 6" from each corner and every 18" on center. The nailing fin was also bedded in polyurethane. The exterior perimeter was blindstopped with wood members and secured with #8 x 3" screws every 24" on center.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-C30 54 x 90			
2.2.1.6.1	Operating Force	20 lbs	45 lbs max.
	Air Infiltration per ASTM E 283 (See Note #1) @ 1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Paragraph	Title of Test - Test Method	Results	Allowed
	Water Resistance per ASTM E 547 (with and without screen) WTP = 4.5 psf	No leakage	No leakage
2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail)		
	@ 45.0 psf (exterior)	0.03"	0.22" max.
	@ 45.0 psf (interior)	0.04"	0.22" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Meeting rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) WTP = 5.25 psf	No leakage	No leakage
Test Specimen #2: H-C40 52 X 72*			
Optional Performance			
4.3	Water Resistance per ASTM E 547 and 331 (with and without screen) WTP = 6.0 psf	No leakage	No leakage
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail) (Loads held for 33 seconds)		
	@ 47.0 psf (exterior)	0.04"	N/A
	@ 47.0 psf (interior)	0.03"	N/A
	(Loads held for 10 seconds)		
	@ 70.5 psf (exterior)	0.07"	0.21" max.
	@ 70.5 psf (interior)	0.04"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator.

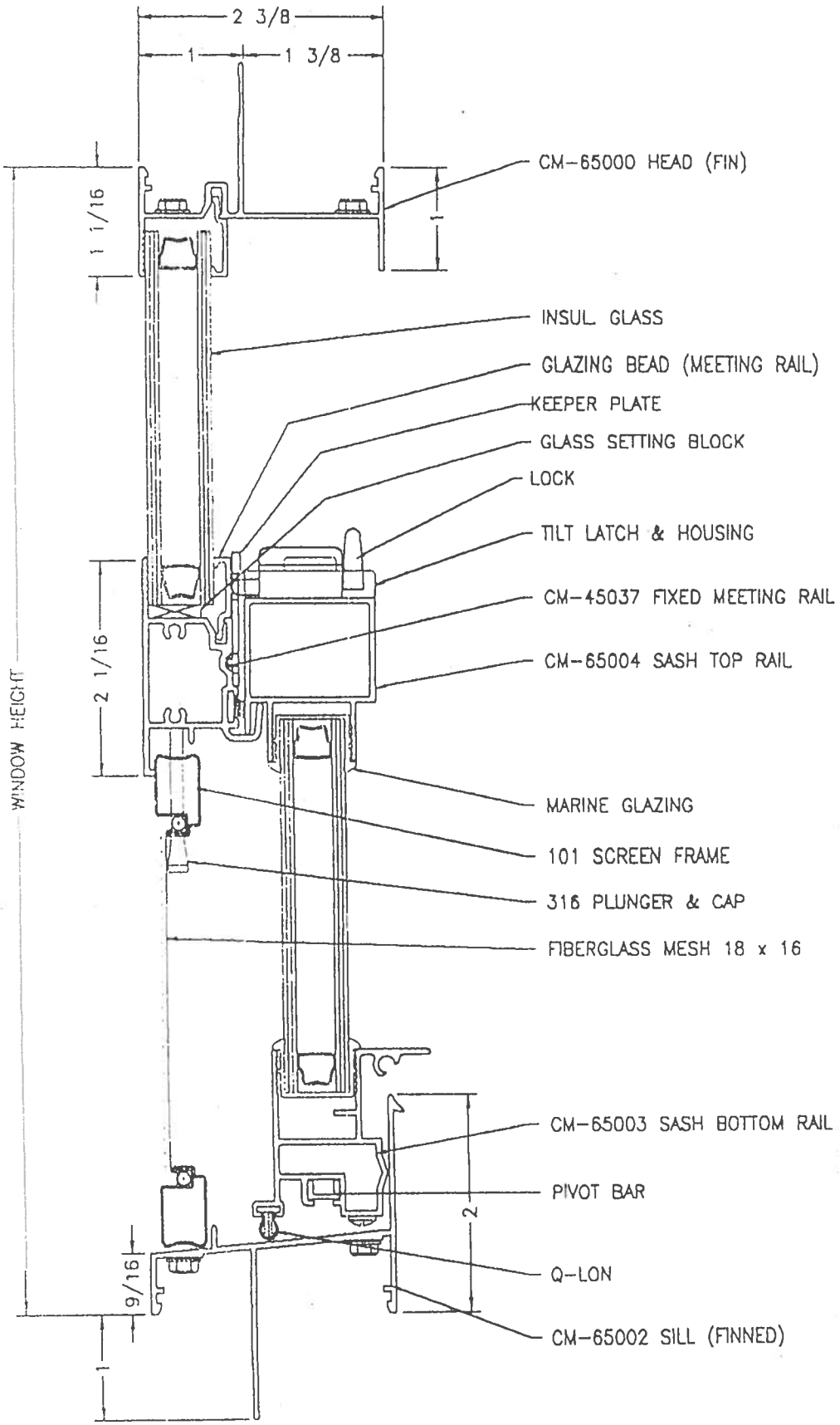
For ARCHITECTURAL TESTING, INC:

Adam A. Fodor
Technician

Bruce W. Croak
Director – Product/Physical Testing

AAF:
01-37589.01

650-AS1 A

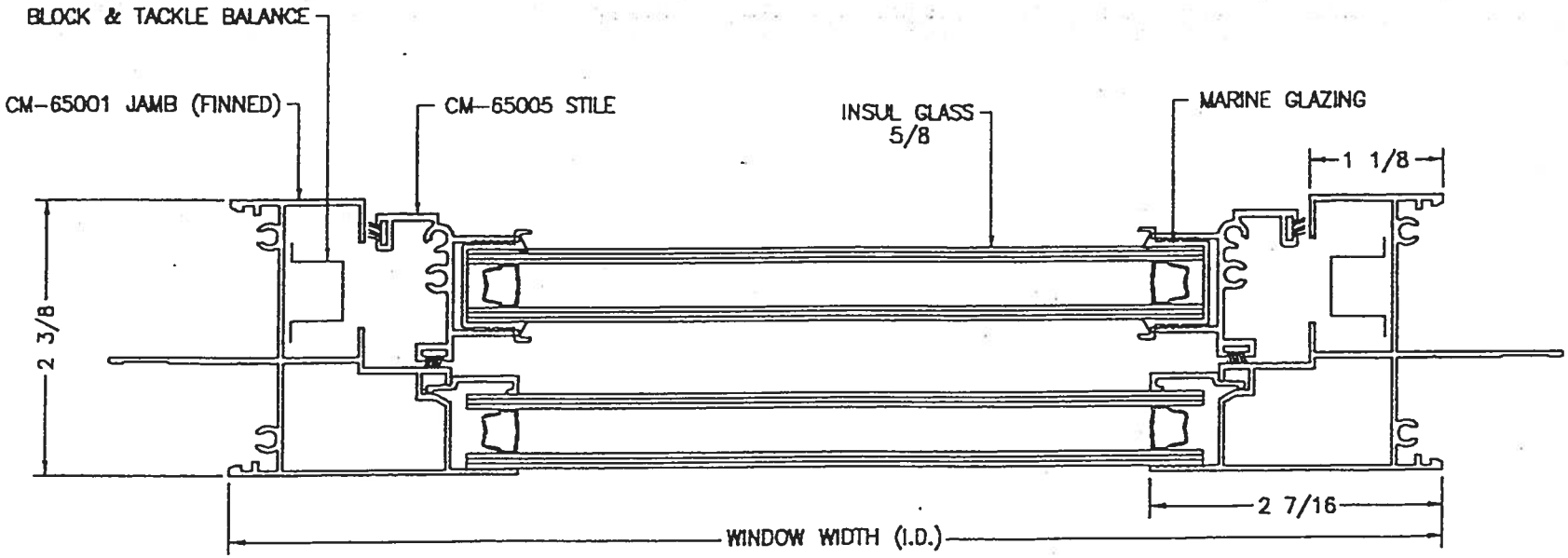


LTR	DESCRIPTION REVISIONS	BY	DATE

MI HOME PRODUCTS
650 WEST MARKET STREET • GRATZ, PA • 17030-0370

TITLE: 650 SH FIN MAIN FRAME VERTICAL CROSS SECTION

DRN	DATE	SCALE	DWG. NO.	REV.
V.M.R.	4-7-92	FULL	650-AS1	A

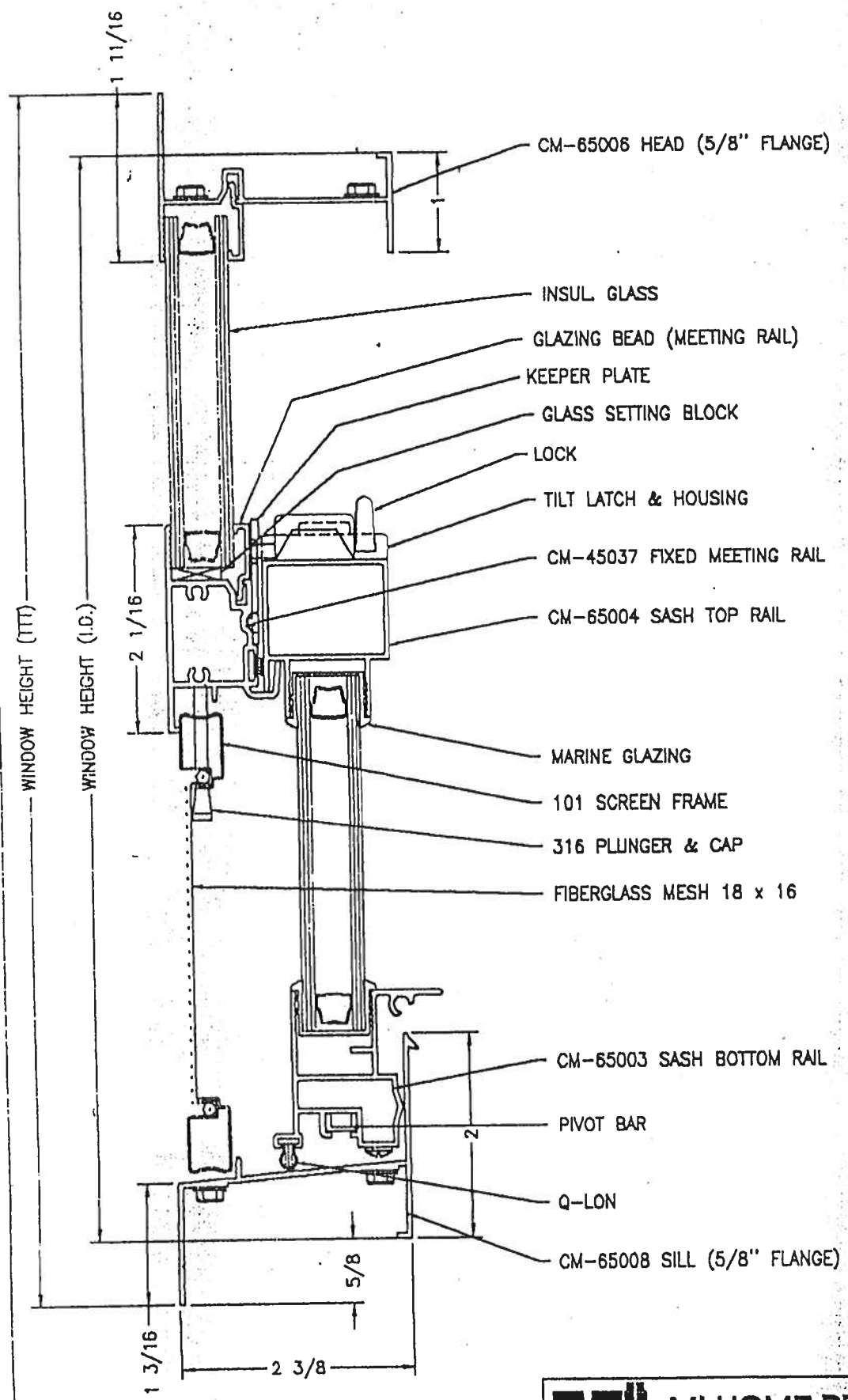


MI HOME PRODUCTS
650 WEST MARKET STREET • GRATZ, PA • 17030-0370

TITLE: 650 SH FIN MAIN FRAME INSULATED GLASS HORIZONTAL CROSS SECTION

DRAWN	DATE	SCALE	DRAWING NO.	REV
V.J.M.P.	4-7-92	FULL	650-AS2	B

650-AS3 A

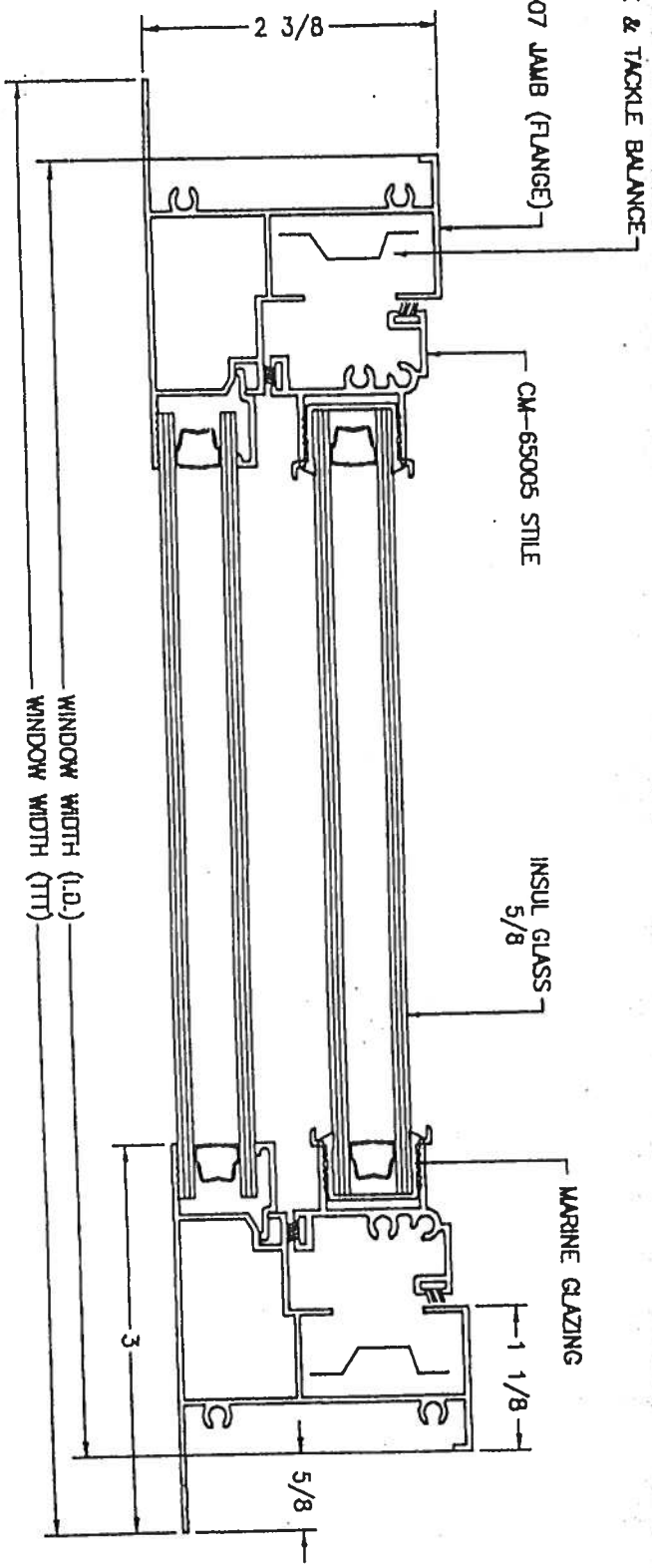


LTR	DESCRIPTION REVISIONS	BY	DATE

MI HOME PRODUCTS
650 WEST MARKET STREET • ORATZ, PA • 17030-0370

TITLE 650 SH L-FLANGE MAIN FRAME INSUL GLASS VERTICAL CROSS SECTION

DRN	DATE	SCALE	DWG. NO.	REV.
V.M.R.	7-15-82	FULL	650-AS3	A



650-AS4

MI HOME PRODUCTS
650 WEST MARKET STREET • GRATZ, PA • 17030-0370

TITLE: 650 SINGLE HUNG FLANGE FRAME HORIZONTAL ASSEMBLY

DRN	CHK	DATE	DESCRIPTION
V.M.R.		7-27-93	

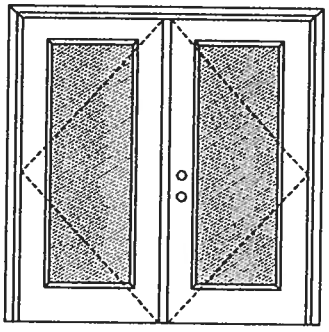
SCALE: FULL
DRAWING NO.: 650-AS4
REV.

XX
Glazed Outswing Unit

COP-WL-JH4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series

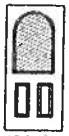

133, 135 Series


136 Series

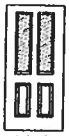

680 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors
Exclusively from Masonite®
Masonite International Corporation

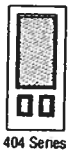
XX
Glazed Outswing Unit

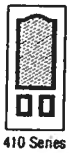
COP-WL-JH4162-02

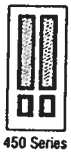
WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

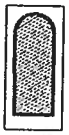
3/4 GLASS:

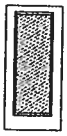

404 Series


410 Series

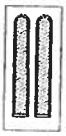

450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

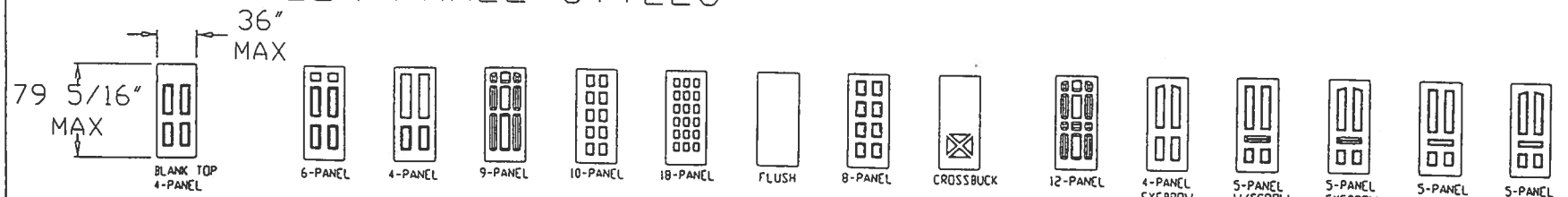
Johnson™ EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice

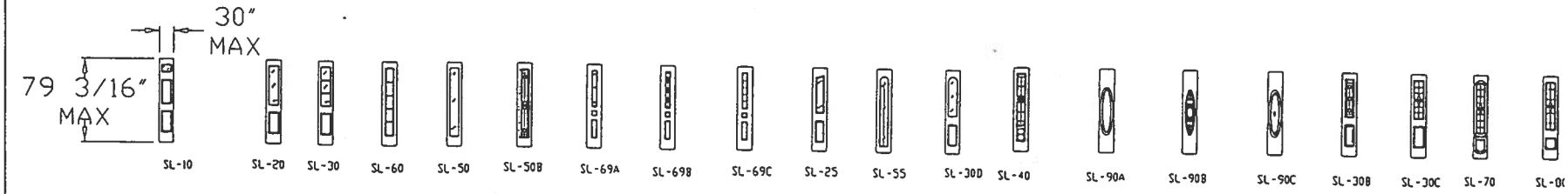
PREMDOR Collection Premium Quality Doors

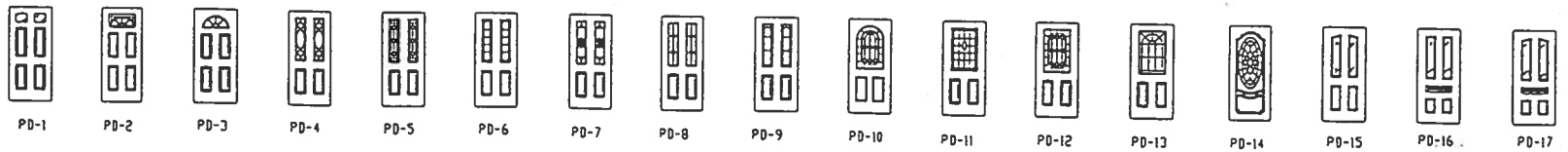
Exclusively from Masonite®
Masonite International Corporation

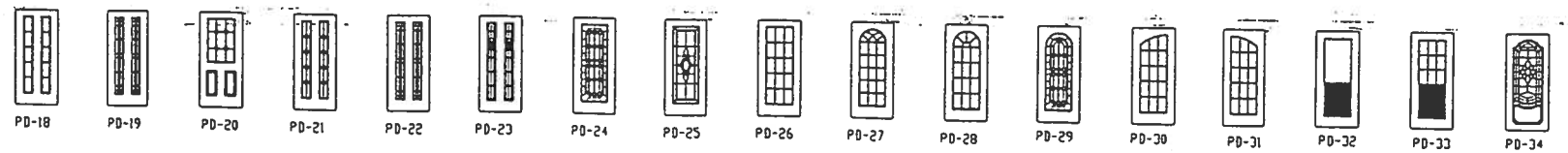
OTHER DOOR PANEL STYLES

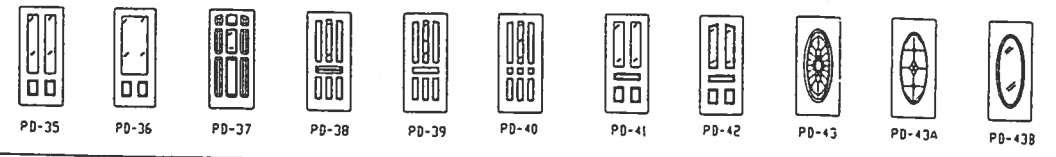


OTHER SIDELITE STYLES









APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY [signature]
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE

UNITS: UNLESS NOTED, FRAC : DEC : ANG :
EXTRUSIONS: UNLESS NOTED, STD COMM. TOL'S
ENGINEER:
DR BY J.D. DATE 1/15/01

LTR	REVISIONS	DATE

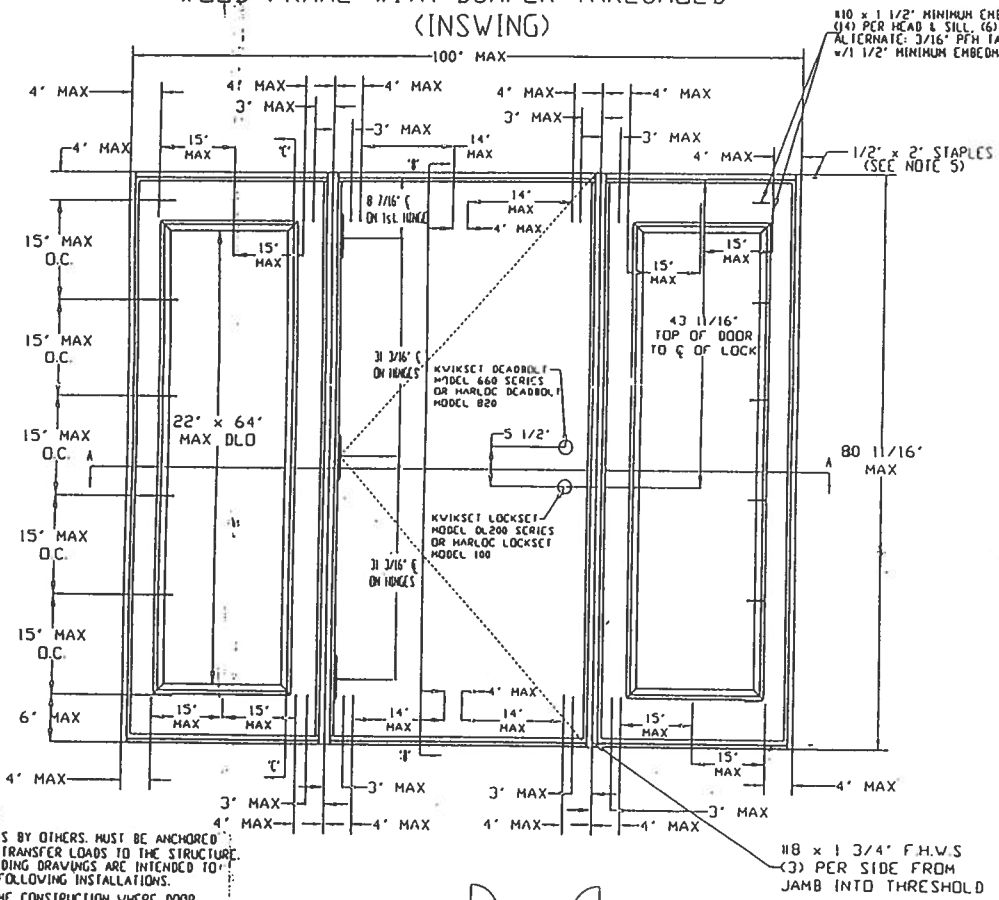
PART NAME: PREMDOOR DOOR OPTIONS
MAT'L: SCALE:

PREMDOOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS 66762

31-1020-EM-1
SHEET 6 OF 6

REVISION LETTER

PREMDOR (ENTERGY BRAND) STEEL EDGE SINGLE DOOR WITH SIDELITES IN WOOD FRAME WITH BUMPER THRESHOLD (INSWING)



NOTES:

1.) WOOD BUCKS BY OTHERS. MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO THE STRUCTURE.
2.) THE PRECEDING DRAWINGS ARE INTENDED TO QUALIFY THE FOLLOWING INSTALLATIONS.

A. WOOD FRAME CONSTRUCTION WHERE DOOR SYSTEM IS ANCHORED TO A MINIMUM TWO BY WOOD OPENING.

B. MASONRY OR CONCRETE CONSTRUCTION WHERE DOOR SYSTEM IS ANCHORED TO A MINIMUM TWO BY STRUCTURAL WOOD BUCK.

C. MASONRY OR CONCRETE CONSTRUCTION WHERE DOOR SYSTEM IS ANCHORED DIRECTLY TO CONCRETE OR MASONRY WITH OR WITHOUT A NON-STRUCTURAL ONE BY WOOD BUCK.

3. ALL ANCHORING SCREWS TO BE #10 WITH MINIMUM 1 1/2" EMBEDMENT INTO WOOD SUBSTRATE OR 3/16" PFH TAPCONS WITH 1 1/2" MINIMUM EMBEDMENT INTO MASONRY.
4. UNIT MUST BE INSTALLED WITH "MIAMI-DADE COUNTY APPROVED" SHUTTERS
5. THREE STAPLES PER SIDE JAMB INTO HEADER ON SIDELITES AND DOOR, THREE STAPLES PER JAMB INTO BASE ON SIDELITES.
6. LATEX SEALANT TO BE APPLIED AT SIDE BY SIDE JAMBS AND SIDELITES.
7. DOOR/SIDELITE HEADER, DOOR/SIDELITE JAMBS, AND SIDELITE BASE CORNERS ARE COPED AND BUTT JOINED.
8. DOORS SHALL BE PRE-PAINTED WITH A WATER-BASED EPOXY RUST INHIBITIVE PRIMER PAINT WITH A DRY FILM THICKNESS OF 0.8 TO 1.2 MIL.
9. FRAMES SHALL BE PRE-PAINTED WITH AN ACRYLIC LATEX WATER-BASED/ WATER-REDUCIBLE WHITE PRIMER WITH A DRY FILM THICKNESS OF 0.8 TO 1.2 MIL.

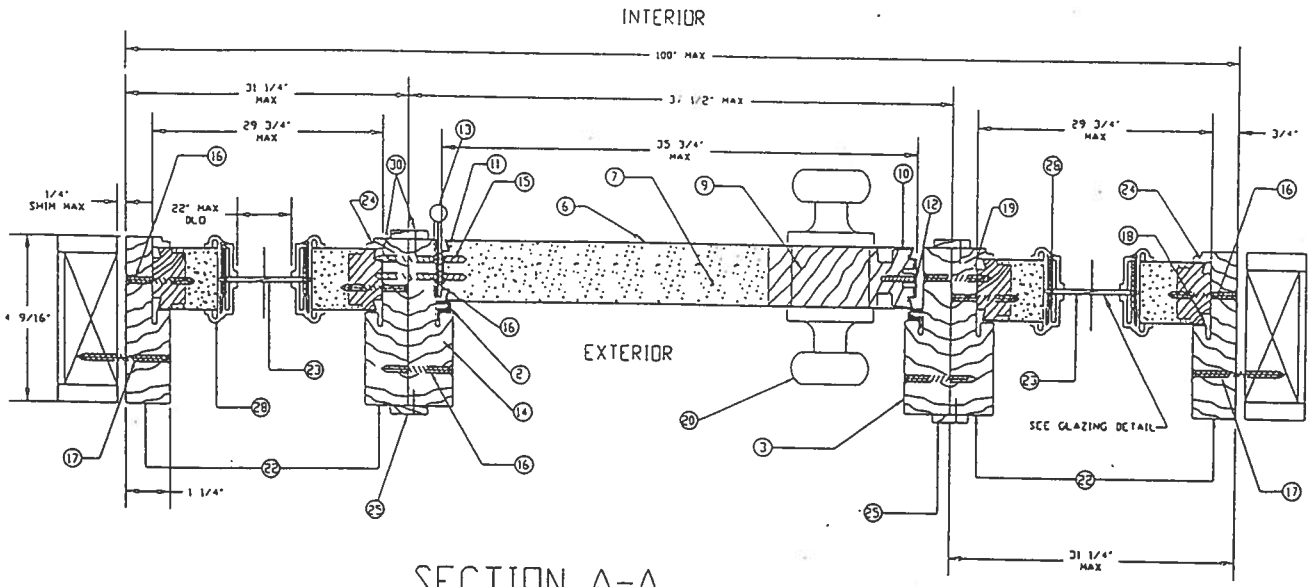
DESIGN PRESSURE RATINGS		
	WHERE WATER INFILTRATION REQUIREMENT IS NEEDED *	WHERE WATER INFILTRATION REQUIREMENT IS NOT NEEDED
Positive	NOT APPROVED *	+76.0 psf
Negative	NOT APPROVED *	-76.0 psf

* UNITS SHALL BE INSTALLED ONLY AT LOCATIONS PROTECTED BY A CANOPY OR OVERHANG SUCH THAT THE ANGLE BETWEEN THE EDGE OF CANOPY OR OVERHANG TO SILL IS LESS THAN 45 DEGREES UNLESS UNIT IS INSTALLED IN NON-HABITABLE AREAS WHERE THE UNIT AND THE AREA ARE DESIGNED TO ACCEPT WATER INFILTRATION.

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO 01-0314.16

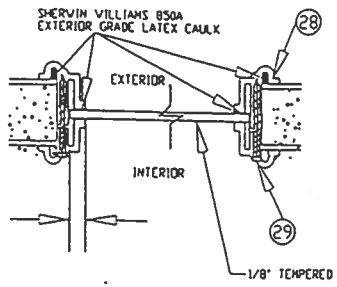
REVISIONS		
C	DADE COUNTY MODIFICATIONS	
B	ADD RATINGS & DRAWING	
A	ADD SCREWS FROM JAMB TO THRESH	
A	ADD NOTE 4 FOR STAPLES	

UNITS UNLESS NOTED OTHERWISE: DEC ± INCH ± FRACTIONS UNLESS NOTED OTHERWISE
ENGINEER
DRAWN BY: DATE 6-9-97
PREMDOR ENTRY SYSTEMS
31-1020-EM-1
SHEET 1 OF 6

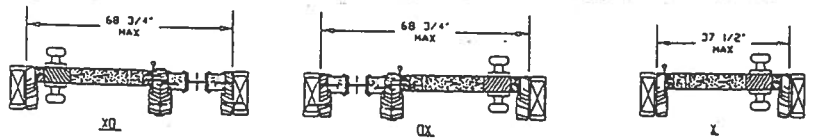


SECTION A-A
INSWING

GLAZING DETAIL



OTHER CONFIGURATIONS



APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO 01-0314.16

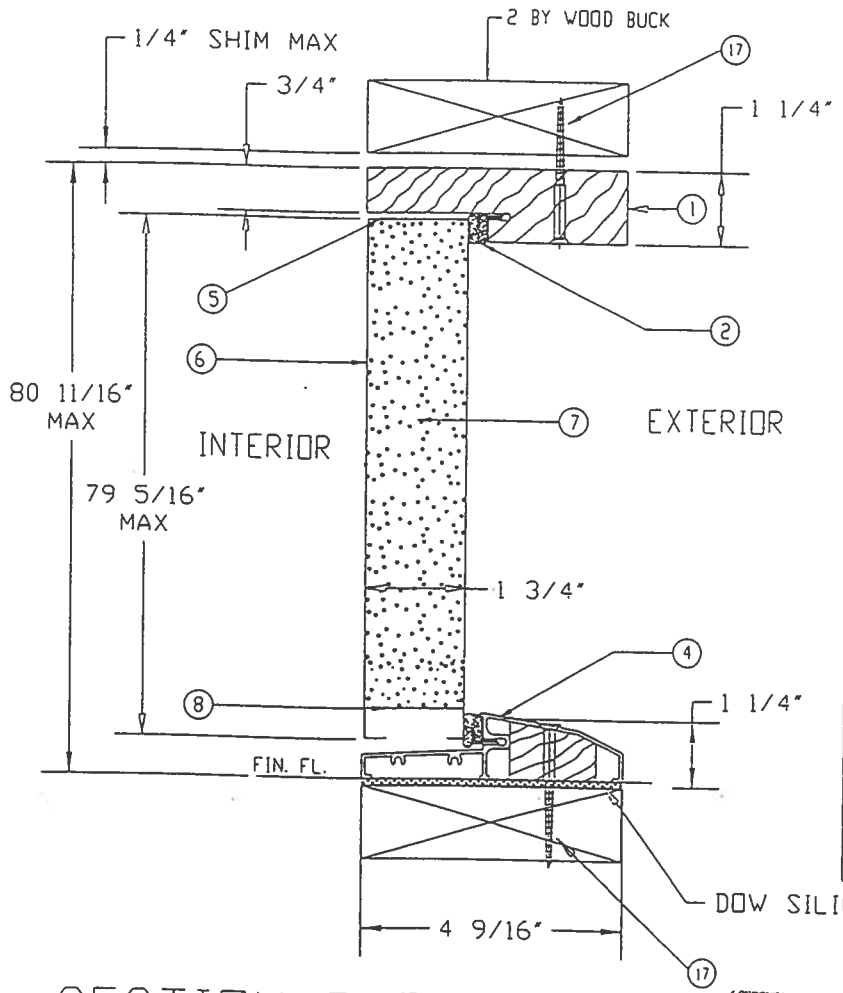
C	DADE COUNTY MODIFICATIONS	5/16/01	JB
B	ADDED PAGE 3 (DOOR OPTIONS)	10-11-00	RS
A	ADD SCREWS TO LITE FRAMES & ADD OTHER DOOR CONFIGURATIONS	7-10-97	RS
LTR	REVISIONS	DATE	BY

LOUIS UNLESS NOTED FRAC : DEC : ANG :
EXTRUDING UNLESS NOTED, SEE COMP. NO.'S
ENGINEER
DRN RS DATE 7-29-97
PART NAME ENTRY DOOR W/SIDELITES (4-9/16)
PART'S SCALE NTS

PREMDOR ENTRY SYSTEMS
100 E. JEFFERSON
PITTSBURG, CA 94565

31-1020-EM-1
SHEET 2 OF 6

REVISION LETTER C



SECTION B-B

MATERIALS LIST

ITEM NO.	DESCRIPTION	PART NUMBER	COMMENTS
1	WOOD HEAD JAMB	EM-14	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
2	COMPRESSION WEATHERSTRIP	EM-25	LOCKSCREEN BRAND LOXSEAL 9650 (BRONZE)
3	WOOD STRIKE JAMB	EM-12	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
4	ALUMINUM-BUMPER THRESHOLD	EM-15	PREMDOR BRAND OR EQUIVALENT - 1 1/4" x 4 9/16"
5	TOP CHANNEL	EM-08	PREMDOR BRAND - 1 11/16" - 20 GA STEEL
6	STEEL SKIN	26 ga. (.017 +.004 -.000)	MIN. YIELD STRENGTH 33,000 PSI MIN. THICKNESS PER ASTM A653 TEST REPORT IS KEY
7	POLYURETHANE FOAM CORE	BASF FOAM - DENSITY 2.0 TO 2.5 lbs/ft³	
8	BOTTOM CHANNEL	EM-07	PREMDOR BRAND - 1 11/16" - 20 GA STEEL
9	WOOD LOCK BLOCK	EM-09	4" X 9 1/2" MTL. TO BE PINE OR EQUIVALENT
10	STRIKE STILE	EM-06	PREMDOR BRAND - 1 11/16" - 20 GA STEEL
11	HINGE STILE	EM-05	PREMDOR BRAND - 1 11/16" - 20 GA STEEL
12	LOCK PREP FILLER PLATE	EM-10	PREMDOR BRAND - .050" THICK- MTL. TO BE POLYETHYLI
13	4"x4" HINGE	EM-16	HAGER BRAND HINGE OR EQUIVALENT - .097 THICK (STE
14	WOOD HINGE JAMB	EM-13	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
15	#10-24 x 1/2" F.H.M.S.		(4) SCREWS PER HINGE INTO DOOR
16	#10 X 2" F.H.W.S.		(5) SCREWS THROUGH HINGE JAMB INTO SIDELITE JAMB, 8" DOWN FROM MAX 18" O.C. THEREAFTER (10) SCREWS THROUGH STRIKE JAMB INTO SIDELITE JAMB, 4" DOWN FROM MAX 8" O.C. THEREAFTER (4) SCREWS THROUGH EACH HINGE INTO DOOR JAMB (6) SCREWS THROUGH EACH SIDELITE JAMB INTO SIDELITE, 4" DOWN FRO TOP, MAX 15" O.C. THEREAFTER
17	#10 F.H.W.S. W/MINIMUM 1 1/2" EMBEDMENT OR 3/16" PFH TAPCONS W/MINIMUM 1 1/2" EMBEDMENT		REFER TO ELEVATION VIEW, FOR # OF SCREWS USED AND LOCATION
18	SIDELITE WOOD STILE	EM-28	15/16" X 1 11/16" MTL. TO BE PINE OR EQUIVALENT
19	#8 x 2" F.H.W.S.		(2) SCREWS AT EACH STRIKE PLATE
20	LOCKSET		KWIKSET BRAND 200 LOCK OR HARLOC BRAND 100 LOCK
21	NOT USED ON THIS MODEL		
22	WOOD SIDELITE JAMB	EM-19	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
23	22" X 64" SINGLE PANEL GLASS	EM-20	TEMPERED GLASS IN POLYPROPYLENE FRAME- DC-1643 - (ODL 1/8" CLEAR TEMPERED GLASS
24	SIDELITE TRIM (WOOD)	EM-21	5/16 x 1/2" MTL. TO BE PINE OR EQUIVALEN
25	WOOD CASING	EM-22	1/8" x 1" MTL. TO BE PINE OR EQUIVALENT - ITEMS ARE MOLDINGS U FOR "SIDE BY SIDE JAMBS" AS MULLIONS
26	WOOD SIDELITE HEAD JAMB	EM-23	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
27	WOOD SIDELITE BASE	EM-24	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
28	POLYPROPYLENE LITE FRAME	DC-1643, ODL-2	HP Polypropylene by ODL
29	#6 X 1 1/2" PAN HEAD SCREWS		18 PER FRAME SCREW SPACING TO BE 3" IN FROM EACH CORNER AND TO EXCEED 14" OC THERE AFTER.
30	PIN NAIL		3/4" LONG NAIL, 4" IN FROM END, MAX 8" O.C. THEREAFTER, USED ON MULLIONS AND T

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 01-0314.10

LIMITS: UNLESS NOTED, FRAC : DEC : ANG :
EXTRUSIONS: UNLESS NOTED, STD. COMM. TOL'S.
ENGINEER:
DR. BY R.S. | DATE 7-29-97

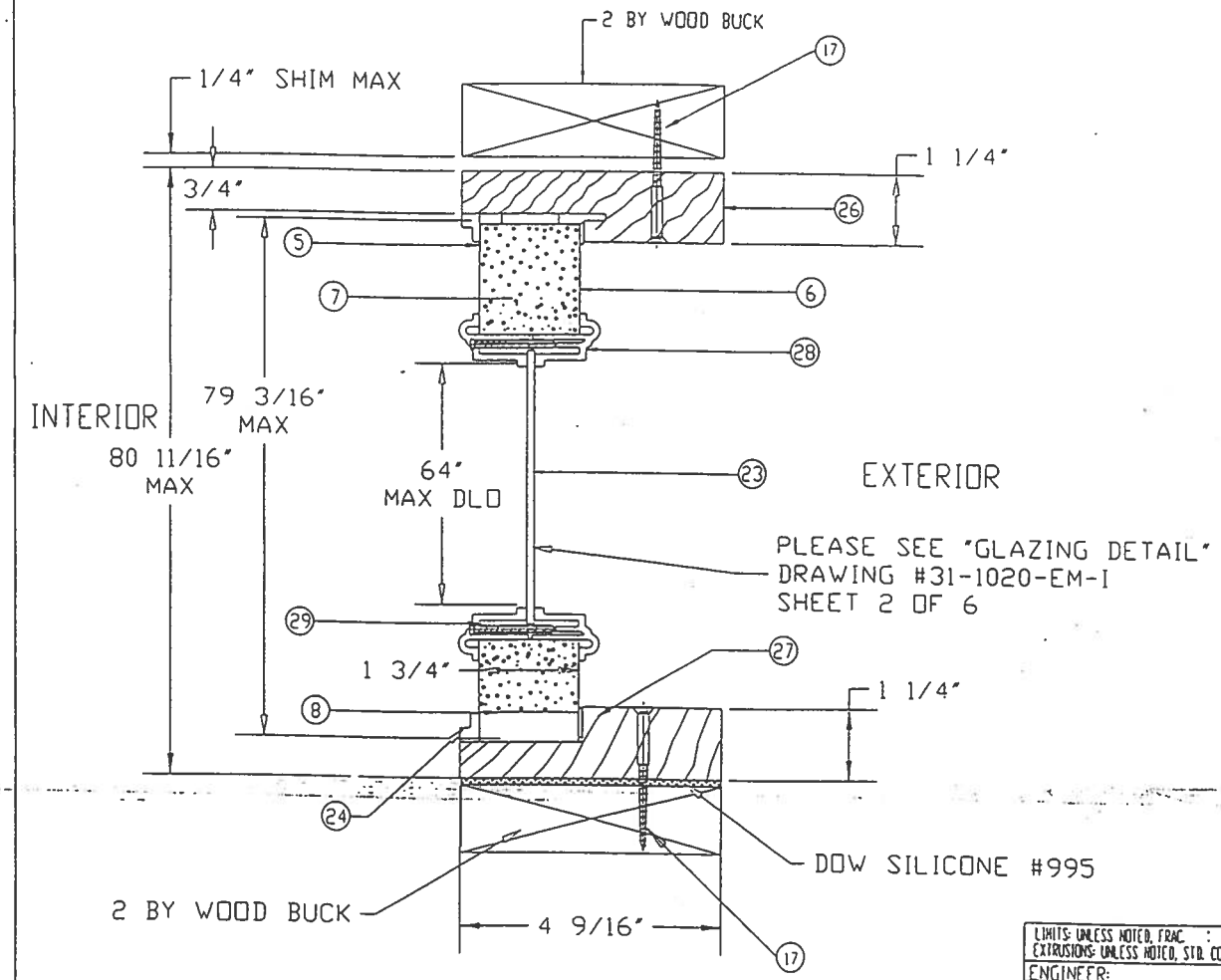
LTR	REVISIONS	DATE	BY
B	DADE COUNTY MODIFICATIONS	1/11/01	JD
A	ADDED PAGE 5 (DOOR OPTIONS)	10-1-98	RS

PART NAME: ENTERGY STEEL EDGE DOOR (B-B)
MAT'L: SCALE:

PREMDOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS. 66762

31-1020-EM-I
SHEET 3 OF 6

REVISION LETTER B



SECTION C-C

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 01-0314.16

LTR.	REVISIONS	DATE	BY
D	DADE COUNTY MODIFICATIONS	6/11/01	JD
C	MATERIAL WAS POLYSTYRENE	6-2-99	RS
B	ADDED PAGE 5 (DOOR OPTIONS)	10-1-98	RS
A	ADD SCREWS TO LITE FRAME & MATERIAL LIST	12-18-97	R.S.

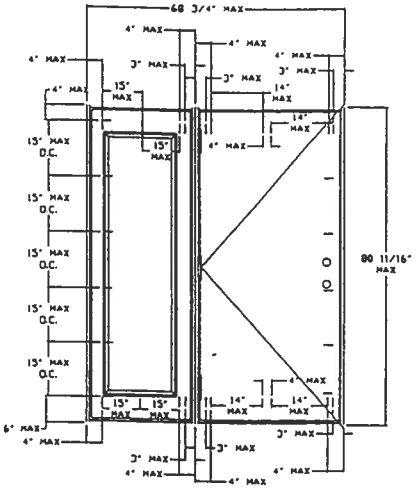
LIMITS: UNLESS NOTED, FRAC. : DEC. : ANG. :
EXTRUSIONS: UNLESS NOTED, STD. COMM. TOL'S
ENGINEER:
DR. BY R.S. | DATE 7-29-97
PART NAME: ENTERGY STEEL EDGE SIDELITE (C-C)
MAT'L: | SCALE:

PREMDOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS. 66762

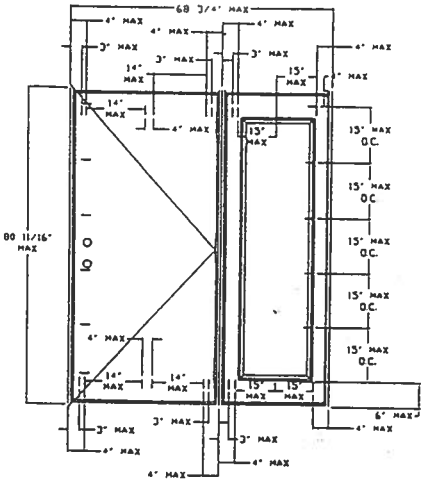
31-1020-EM-I
SHEET 4 OF 6

REVISION LETTER D

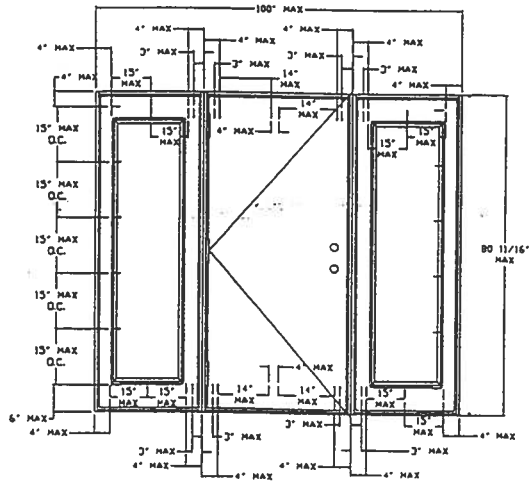
OTHER DOOR CONFIGURATIONS



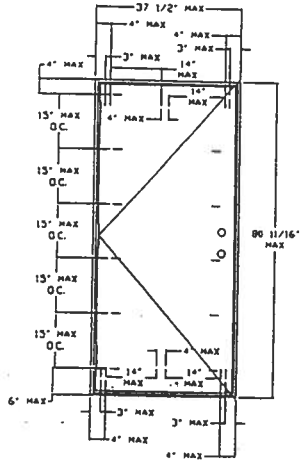
OX



XO



OXO



X

APPROVED AS COMPLYING WITH THE SOUTHERN BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO 01-0314.16

LIMITS: UNLESS NOTED, FRAC. : DEC. : ANG. :
EXTRUSIONS: UNLESS NOTED, STD. COMM. TOL'S
ENGINEER:
DR. BY J.D. DATE 1-11-01

LTR	REVISIONS	DATE	BY

PART NAME:
MAT'L:
SCALE:

PREMDOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS 66762

31-1020-EM-I
SHEET 5 OF 6

REVISION LETTER

Wendel Residence, Columbia County FL

Wind Load Analysis Requirements

(In Compliance with the 2004 Florida Building Code and Amendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

Description of New Residence:

Footprint: 63'x 57'4"' overall - "T" shaped with covered rear porch and inset front porch
Walls: 2x4-16" O.C. with 7/16" OSB sheathing minimum and brick veneer
and ½"gypsum wall board interior.
Roof Structure: Pre-engineered roof trusses and 7/16" OSB sheathing (min.)
Roof Type: Hip primarily with gables over living room and garage at front
(analyzed for 2' eave overhang and porch areas)
Foundation: footer with stemwall, with slab construction

Windload Data and Exposure:

Basic Wind Speed = 110 mph
Importance Factor = 1.0
Exposure category = B
Height and Exposure Adjustment Coefficient = 1.0
Residential Occupancy = Group R3
Analysis Method = FBC 1609.6 - Simplified Provisions for Low Rise Buildings
(see tables 1609.6A, 1609.6B, 1609.6C and 1609.6E for wind pressure values)
Mean roof height = 17'
Roof Cross Slope = 6:12
Eave Overhang= (Analyzed for 2' overhang and porches)
Wall Height = 9'
Shear Wall locations = exterior walls only(all walls 3' in length or greater)
Bracing method for gable locations = framing from wall to roof diaphragm(see attached detail)

Nailing Pattern Requirements:

Wall sheathing:	Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center around edges(including around doors and windows) and 6" on center interior. Full depth blocking shall be required at horizontal joints in sheathing.
Roof sheathing:	Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center at panel ends and overhang areas and 6" on center elsewhere.
Top wall plate:	Nail with 1-16d common nail 12" O.C.(average)

Strapping and Anchor Requirements:

truss to exterior wall plate and porch beam locations:	install one Simpson model H10 hurricane anchor at each common or tier truss. At jack trusses under 10' in length install one Simpson H2.5A. At hip trusses install Simpson model HCP.

Marty J. Humphries
4-19-06

wall strap tie requirements: at top and bottom of wall install one Simpson model SP4 at each side of each door and window 4' or less in width. At top and bottom of wall for windows and doors larger than 4' in width

wall strap ties(cont.): install two Simpson model SP4's each side of each opening. All other wall locations install SP4's top and bottom of wall 4' on center. At garage door opening install 3 full studs each side with 3 Simpson model SPH4's installed at top and bottom of wall.

Rear Porch Columns: Install Simpson model CB66 and Simpson model ECCQ46SDS2.5

Lookouts: Install one Simpson model H5 where lookouts connect to end gable truss.

Gable end: Install one LSTA18 - 4' on center connecting gable end truss to wall framing.

Gable End Bracing Requirements:

At each gable end install one 2x4 SPF 8' stud spaced 6' on center horizontal along top of bottom chord of trusses, nail with 2-12d nails at each truss including end truss. In addition, install a 2x4 brace extending from this stud at the gable end truss approx. 45 degrees to truss at roof sheathing, nail with 2 -12d nails where it crosses truss members and at ends. Gable end trusses shall be built to receive sheathing with vertical members 2' on center. Vertical members of gable end truss greater than 5' in height shall be stiffened with one 2x4 SPF nailed with 12d nails 8" on center to back of vertical member. (See attached detail)

Foundation Requirements:

Stemwall: Minimum size of footer shall be 10" x 21" wide with 3-#5 rebar continuous and 1-#5 vertical rebar 48" on center. All cells shall be filled with concrete. ½" anchor bolts with 2" washers shall be installed 3' on center and 9" from corners each way and at each side of door openings. (3000 psi concrete min.)(Note: foundation designed using an allowable bearing pressure of 1000 psf)

Header Requirements:

Windows & Doors: For windows or doors 4' or less in width the minimum header shall be 2 - #2 SYP 2x10's with ½" plywood/OSB between. For windows or doors greater than 4' in width minimum header shall be 2-#2 SYP 2x12's with ½" plywood/OSB between.

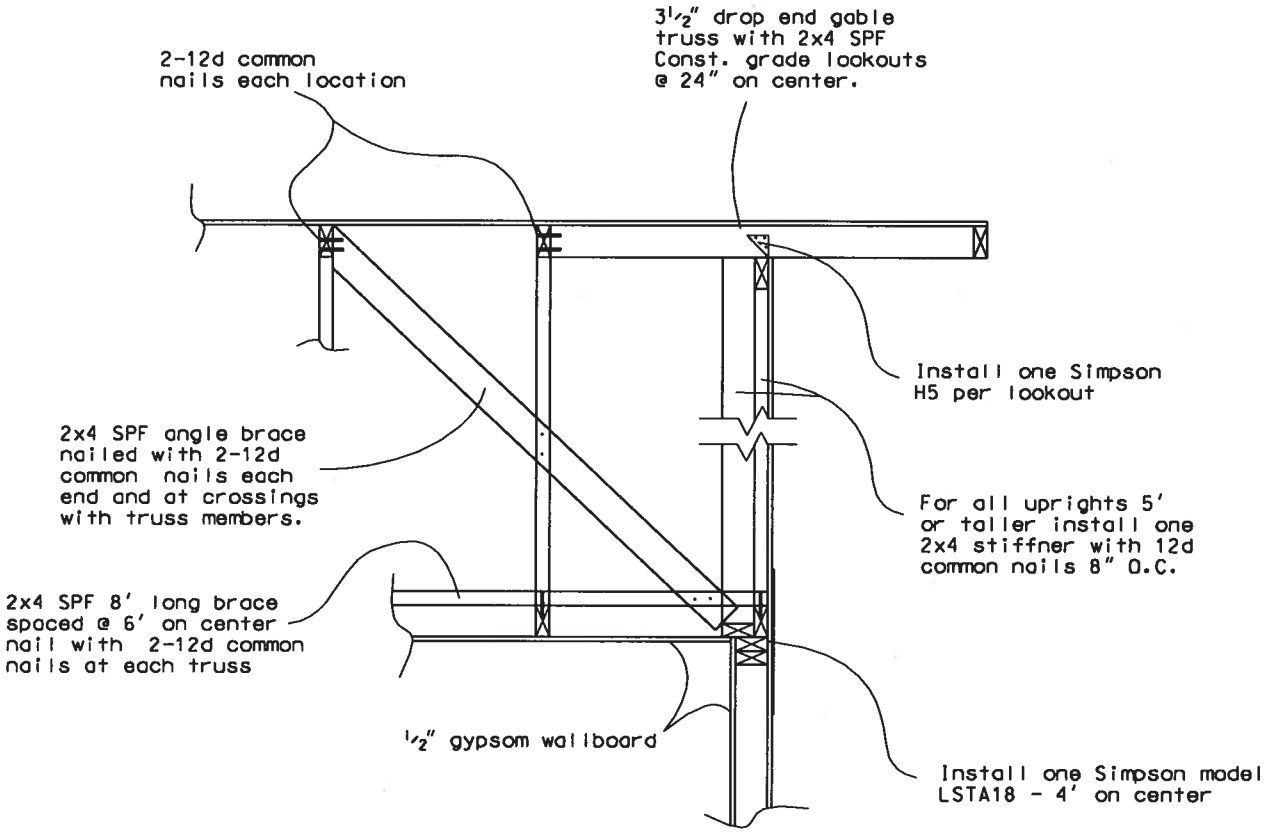
Front Porch Beam: Minimum header shall be 2-#2 SYP 2x12's or 3-#2 SYP 2x10's.

Rear Porch Beam: Rear porch beam comprised of 2 – 1.75"x 14" laminated veneer lumber beams and the beams shall be Louisiana Pacific-Gang Lam 2250 FB-1.5E or equivalent capacity beams. Nail beams together with 12d common nails 12" O.C. top and bottom.

Garage Header: Header for garage door opening shall be comprised of 2 – 1.75"x14" laminated veneer lumber beams and the beams shall be Louisiana Pacific-Gang Lam 2650 FB-1.9E or equivalent capacity beams. Nail beams together with 12d common nails 12" O.C. top and bottom.

Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

Mutty D. Hay
4-19-06



GABLE END BRACING DETAIL (N.T.S.)

Marty J. Humphries

4-19-06

Wendel Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071

NEW! The H2.5A is symetrically designed for easy installation, with higher uplift loads to meet new code requirements. A placement mark allows easy installation on double top plates.

NEW! The H5A has an installed cost benefit, as it only requires 6 nails, to meet lower uplift requirements.

The H connector series provides wind and seismic ties for trusses and rafters.

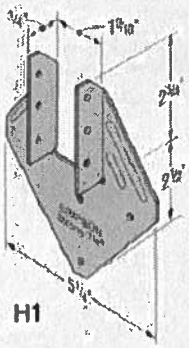
Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
Design Shear/Allowable Shear + Design Tension/Allowable Tension < 1.0.

MATERIAL: See table

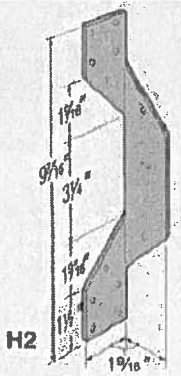
FINISH: Galvanized; H10-2, H11Z–Z-MAX. Other models available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- H1 can be installed with flanges facing outwards (reverse of drawing number 1). When installed inside a wall, a birdsmouth cut is required.
- H2.5, H3, H4, H5 and H6 ties are shipped in equal quantities of rights and lefts.
- Bend the H7 over the top of the truss. Install a minimum of four 8d nails into the truss, including two into the truss side.
- Hurricane Ties do not replace solid blocking.

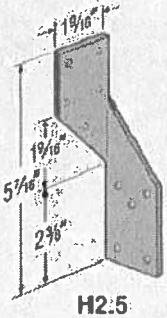
CODES: BOCA, ICBO, SBCCI NER-422, NER-393, NER-432; NER-499; City of L.A. RR 24818; Dade Co, FL 00-0512.11 (H10); 00-0926.01 (H1, H2.5, H3, H4, H5).



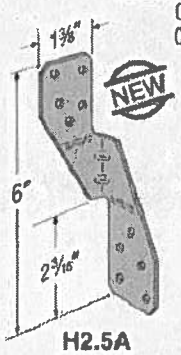
H1



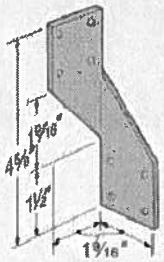
H2



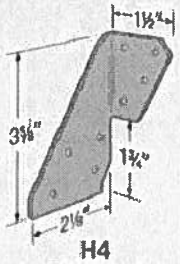
H2.5



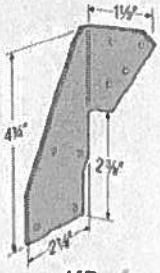
H2.5A



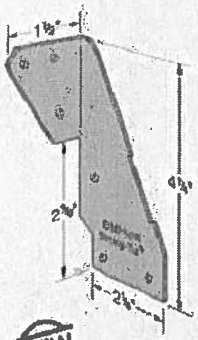
H3



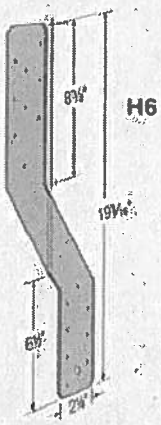
H4
U.S. Patent 4,714,372



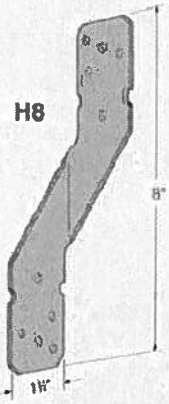
H5
U.S. Patent 4,714,372



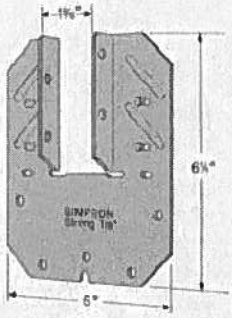
NEW
H5A
U.S. Patent 4,714,372



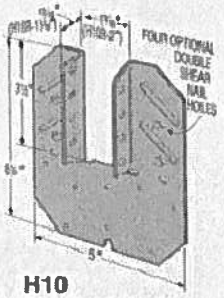
H7



H8

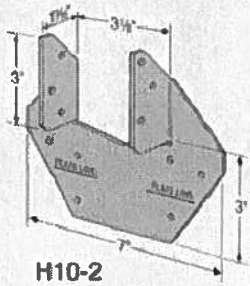


H9

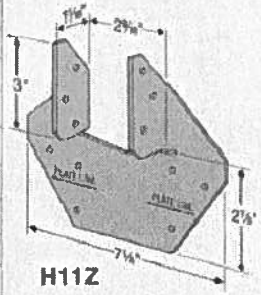


H10
(H10R similar)
U.S. Patents 4,480,941; Canada Patent 1,193,418

Model No.	Ga	Fasteners: To Rafters/ Truss	Fasteners: To Plates	Fasteners: To Studs	Uplift: Avg Ult	Doug-Fir Larch/So. Pine Allowable Loads[1,2]: Uplift (133)	Uplift (160)	Lateral (133/160) F_1	Lateral (133/160) F_2	Uplift Load with 8dx1½ Nails (133 & 160)	Spruce-Pine-Fir Allowable Loads[1,2]: Uplift (133)	Uplift (160)	Lateral (133/160) F_1	Lateral (133/160) F_2	Uplift Load with 8dx1½ Nails (133 & 160)
H1	18	6-8dx1½	4-8d	—	1958	490	585	485	165	455	400	400	415	140	370
H2	18	5-8d	—	5-8d	1040	335	335	—	—	335	230	230	—	—	230
H2.5	18	5-8d	5-8d	—	1300	415	415	150	150	415	365	365	130	130	365
NEW H2.5A	18	5-8d	5-8d	—	1793	600	600	110	110	480	520	535	110	110	480
H3	18	4-8d	4-8d	—	1433	455	455	125	160	415	320	320	105	140	290
H4	20	4-8d	4-8d	—	1144	360	360	165	160	360	235	235	140	135	235
H5	18	4-8d	4-8d	—	1485	455	465	115	200	455	265	265	100	170	265
NEW H5A	18	3-8d	3-8d	—	1500	350	420	115	180	290	245	245	100	120	170
H6	16	—	8-8d	8-8d	3983	915	950	650	—	—	785	820	560	—	—
H7	16	4-8d	2-8d	8-8d	2991	930	985	400	—	—	800	845	345	—	—
H8	18	5-10dx1½	5-10dx1½	—	2422	620	745	—	—	—	530	565	—	—	—
H9KT	18	4-SDS¼x1½	5-SDS¼x1½	—	2812	875	875	680	125	—	755	755	680	125	—
H10	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10R	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10-2	18	6-10d	6-10d	—	2447	760	760	455	395	—	655	655	390	340	—
H11Z	18	6-16dx2½	6-16dx2½	—	5097	830	830	525	760	—	715	715	450	655	—



H10-2



H11Z

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5); 390 lbs (H2.5A); 360 lbs (H4) and 310 lbs (H8).
4. The H9KT is sold in 20 piece packs with screws.
5. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
6. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

LSU/LSSU ADJUSTABLE LIGHT SLOPEABLE/SKEWABLE U HANGERS

SIMPSON Strong-Tie

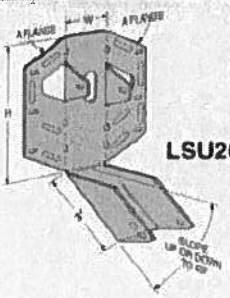
This series attach joists or rafters to headers, sloped up or down, and skewed left or right, up to 45°.

MATERIAL: See table.

FINISH: Galvanized

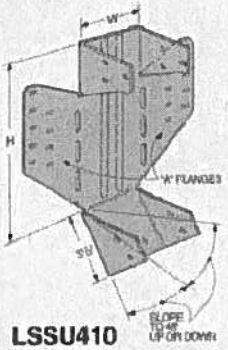
INSTALLATION:
- Use all specified fasteners. See General Notes.
- Attach the sloped joist at both ends so that the horizontal force developed by the slope is fully supported by the supporting members.
- Web stiffeners required for I-joist applications.

CODES: BOCA, ICBO, SBCCI NER-209, NER-421, NER-432. City of L.A. RR 24949, RR 25074 and RR 25076.



LSU26

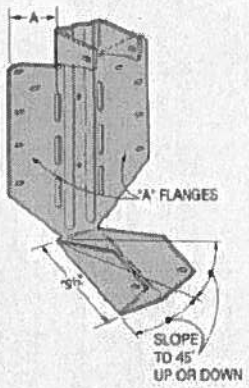
U.S. Patent 4,423,977 and Canada Patent 1,168,827



LSSU410 (LSSU210-2 similar)

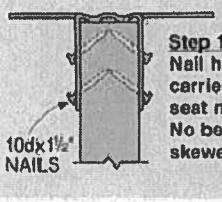
Joist Width	Model No.	Ga	Dimensions W	H	A	Fasteners Face	Joist	Allowable Loads DF/SP Species Header Uplift[2] (133)	Uplift[2] (160)	Floor (100)	Roof Snow (115)	Roof Const (125)	SPF Species Header Uplift[2] (133)	Floor (100)	Roof Snow (115)	Roof Const (125)
Sloped Only Hangers																
1½	LSU26	18	1⅜	4⅞	1½	6-10d	5-10dx1½	485	535	665	765	800	415	575	660	690
1½	LSSU28	18	1⅜	7⅛	1½	10-10d	5-10dx1½	485	535	1110	1275	1390	415	960	1105	1200
1½	LSSU210	18	1⅜	8½	1½	10-10d	7-10dx1½	730	875	1110	1275	1390	625	960	1105	1200
2½	LSSUH310	16	2⅜	8½	3½	18-16d	12-10dx1½	1150	1150	2395	2565	2565	990	2070	2215	2215
3	LSSU210-2	16	3⅜	8½	2⅞	18-16d	12-10dx1½	1150	1150	2395	2755	2990	990	2070	2380	2590
3½	LSSU410	16	3⅜	8½	2⅞	18-16d	12-10dx1½	1150	1150	2395	2755	2990	990	2070	2380	2590
Skewed Hangers or Sloped and Skewed																
1½	LSU26	18	1⅜	4⅞	1½	6-10d	5-10dx1½	485	535	665	765	800	415	575	660	690
1½	LSSU28	18	1⅜	7⅛	1½	9-10d	5-10x1¼	485	535	885	885	885	415	765	765	765
1½	LSSU210	18	1⅜	8½	1½	9-10d	7-10dx1½	730	785	995	1145	1205	625	860	995	1050
2½	LSSUH310	16	2⅜	8½	3½	14-16d	12-10dx1½	1150	1150	1600	1600	1600	990	1385	1385	1385
3	LSSU210-2	16	3⅜	8½	2⅞	14-16d	12-10dx1½	1150	1150	1825	1865	1865	990	1580	1610	1610
3½	LSSU410	16	3⅜		2⅞											

1. Roof loads are 125% of floor loads unless limited by other criteria. Floor loads may be adjusted for load durations according to the code provided they do not exceed those in the roof columns.
2. Uplift loads include a 33% and 60% increase for earthquake or wind loading; no further increase is allowed.

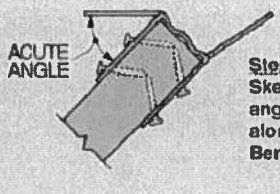


LSSU28

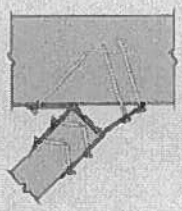
LSSU INSTALLATION SEQUENCE



Step 1
Nail hanger to slope-cut carried member, installing seat nail first.
No bevel necessary for skewed installation.



Step 2
Skew flange to form acute angle. Bend other flange back along centerline of slots.
Bend one time only.



Step 3
Attach hanger to the carrying member, acute angle side first. Install nails at an angle.

HCP HIP CORNER PLATES

The HCP connects a rafter or joist to double top plates at a 45° angle.

MATERIAL: 18 gauge.

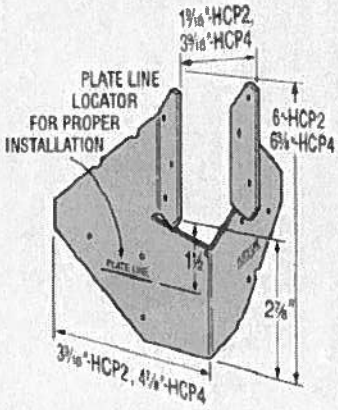
FINISH: HCP2–galvanized or Z-MAX; HCP4–galvanized.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- Attach HCP to double top plates; birdsmouth not required for table loads.
- Install rafter and complete nailing. Rafter may be sloped to 45°.

CODE: BOCA, ICBO, SBCCI NER-499.

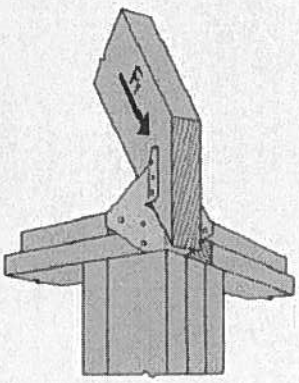
Member Size	Model No.	Fasteners To Rafters	Fasteners To Plates	Uplift Avg Ult	Doug-Fir-Larch/ So. Pine Allowable Loads[1] (133 & 160) Uplift	(133 & 160) F_1	Spruce-Pine-Fir Allowable Loads[1] (133 & 160) Uplift	(133 & 160) F_1
2x	HCP2	6-10dx1½	6-10dx1½	2017	605	300	520	260
4x	HCP4	8-10d	8-10d	3367	1000	265	860	230

1. Loads may not be increased for short-term loading.
2. The HCP can be installed on the inside and the outside of the wall with a flat bottom chord truss and achieve twice the load capacity.
3. Uplift loads include a 33% and 60% increase for earthquake or wind loading; no further increase allowed.



HCP2 (HCP4 similar)

U.S. Patent 5,380,115



Typical HCP Installation

Solid Sawn Connectors

Z2 clips secure 2x4 flat blocking between joists or trusses to support sheathing.

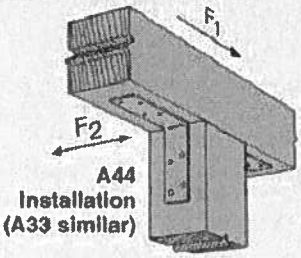
MATERIAL: Z clips–see table. A21 and A23–18 ga.; all other A angles–12 ga.

FINISH: Galvanized

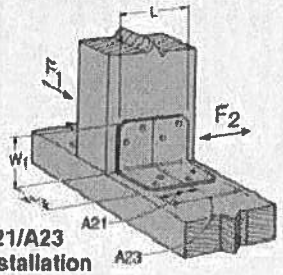
INSTALLATION: • Use all specified fasteners. See General Notes.
• Z clips do not provide lateral stability. Do not walk on stiffeners or apply load until diaphragm is installed and nailed to stiffeners.

CODES: BOCA, ICBO, SBCCI NER-421 (except A33, A44); City of L.A. RR 25076 (except A33, A44); Dade Co. FL 99-0623.04 (A21 and A23).

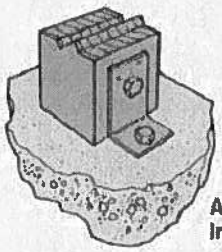
Model No.	Dimensions			Fasteners				Avg Ult F_2	Allowable Loads[1] DF/SP			
				Base		Post			(133)		(160)	
	W_1	W_2	L	Bolts	Nails	Bolts	Nails		F_1	F_2	F_1	F_2
A21	2	1½	1⅜	—	2-10dx1½	—	2-10dx1½	540	245	175	290	175
A23	2	1½	2¾	—	4-10dx1½	—	4-10dx1½	1767	485	485	585	565
A33	3	3	1½	—	4-10d	—	4-10d	2635	625	330	750	330
A44	4⅜	4⅜	1½	—	4-10d	—	4-10d	2490	625	295	750	295
A66	5⅞	5⅞	1⅛	2-⅞	—	2-⅞	—	N/A	N/A	N/A	N/A	N/A
A88	8	8	2	3-⅞	—	3-⅞	—	N/A	N/A	N/A	N/A	N/A
A24	3⅛	2	2½	1-½	—	1-½	2-10d	N/A	N/A	N/A	N/A	N/A
A311	11	3⅛	2	1-½	—	1-½	4-10d	N/A	N/A	N/A	N/A	N/A



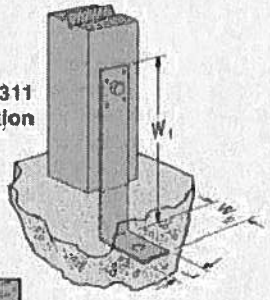
A44 Installation (A33 similar)



A21/A23 Installation



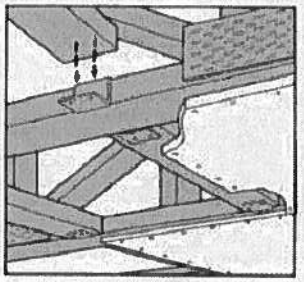
A24 Installation



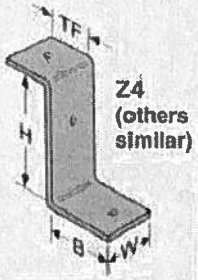
A311 Installation

Model No.	Ga	Dimensions				Fasteners[1] (Total)	Avg Ult	Allowable[2] Download (125)
		W	H	B	TF			
Z2	20	2⅝	1½	1⅜	1⅜	4-10dx1½	1507	465
Z4	12	1½	3⅛	2⅛	1¾	2-16d	1450	465
Z6	12	1½	5⅛	2	1⅜	2-16d	1517	485
Z28[3]	28	2⅝	1½	1⅜	1⅜	10dx1½	—	—
Z38[3]	28	2⅝	2⅛	1⅜	1⅜	10dx1½	—	—
Z44[3]	12	2½	3½	2	1⅜	4-16d	2800	865

1. Z28 and Z38 do not have nail holes. Fastener quantities are as required.
2. Allowable loads have been increased 25% for roof loading (Z clips), 33% and 60% for earthquake or wind loading (A angles); no further increase allowed; reduce for other load durations according to the code.
3. Z4 and Z6 loads apply with a nail into the top and a nail into the seat.



Typical Z2 Installation



Z4 (others similar)

SP/SPH/RSP4 STUD PLATE TIES

The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.

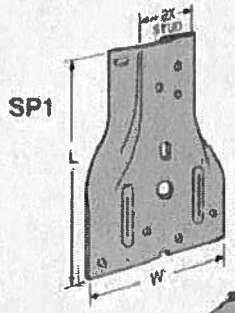
MATERIAL: SPH–18 gauge, all others–20 gauge **FINISH:** Galvanized

INSTALLATION: • Use all specified fasteners; see General Notes.
• SP–one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.

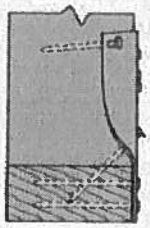
CODES: BOCA, ICBO, SBCCI NER-432, NER-443, NER-499, SBCCI 9603A; City of LA RR 25318 (RSP4); Dade Co. FL 99-0623.04 (SP1, SP2, SP4, SP6, SP8).

Model No.	Dimensions		Fasteners		Avg Ult	Allowable Uplift Loads DF/SP	
	W	L	Stud[1]	Plate		(133)[2]	(160)[2]
SP1	3½	5⅛	6-10d	4-10d	1950	585	585
SP2	3½	6⅝	6-10d	6-10d	3300	890	1065
SP3	4⅛	6⅝	6-10d	6-10d	3467	890	1065
SP4	3⅛	7¼	6-10dx1½	—	2917	735	885
SP5	4½	5⅛	6-10d	4-10d	1950	585	585
SP6	5⅛	7¼	6-10dx1½	—	2917	735	885
SP8	7⅛	8⅛	6-10dx1½	—	2917	735	885
SPH4	3⅛	8¼	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH6	5⅛	9¼	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH8	7⅛	8¼	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
RSP4 (1)	2⅛	4½	4-8dx1½	4-8dx1½	1032	315	315
RSP4 (2)	2⅛	4½	4-8dx1½	4-8dx1½	1445	450	450

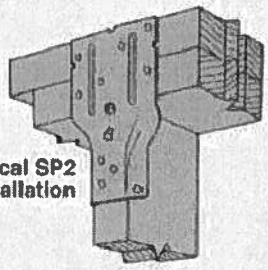
1. SP1, 2, 3 and SP5: drive one stud nail at an angle through the stud into the plate to achieve the table load (see illustration).
2. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
3. RSP4–see installation details (1) and (2) for reference.
4. RSP4 F2 is 280 lbs (installation 1) and 305 lbs (installation 2). F1 load is 210 lbs for both installations.
5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.
6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement



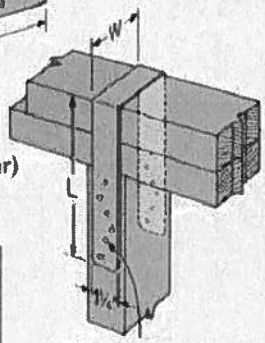
SP1



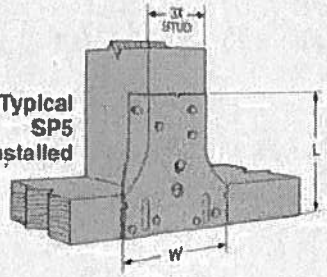
SP1 Nailing Profile



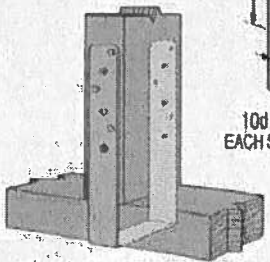
Typical SP2 Installation



Typical SPH Installation (SP4, 6, 8 similar)

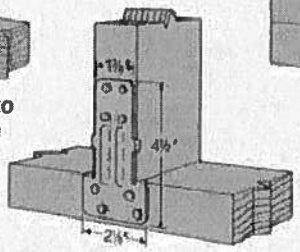


Typical SP5 Installed

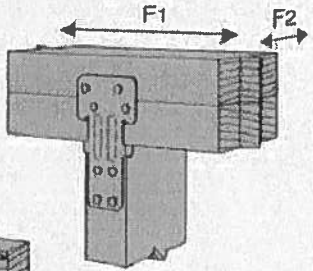


Typical SPH4 Stud to Single Bottom Plate

10d×1½" NAILS EACH SIDE OF STUD



(1) Typical RSP4 Stud to Single Bottom Plate



(2) Typical RSP4 Stud to Double Top Plate (see footnote 4)

RPS/ST/FHA/PS/HST/LSTA/LSTI/MST/MSTA/MSTC/MSTI

STRAP TIES

SIMPSON Strong-Tie CONNECTORS

The MSTC series has countersunk nail slots for a lower nailing profile. Coined edges ensure safer handling. The RPS meets UBC and City of Los Angeles code requirements for notching plates where plumbing, heating or other pipes are placed in partitions.

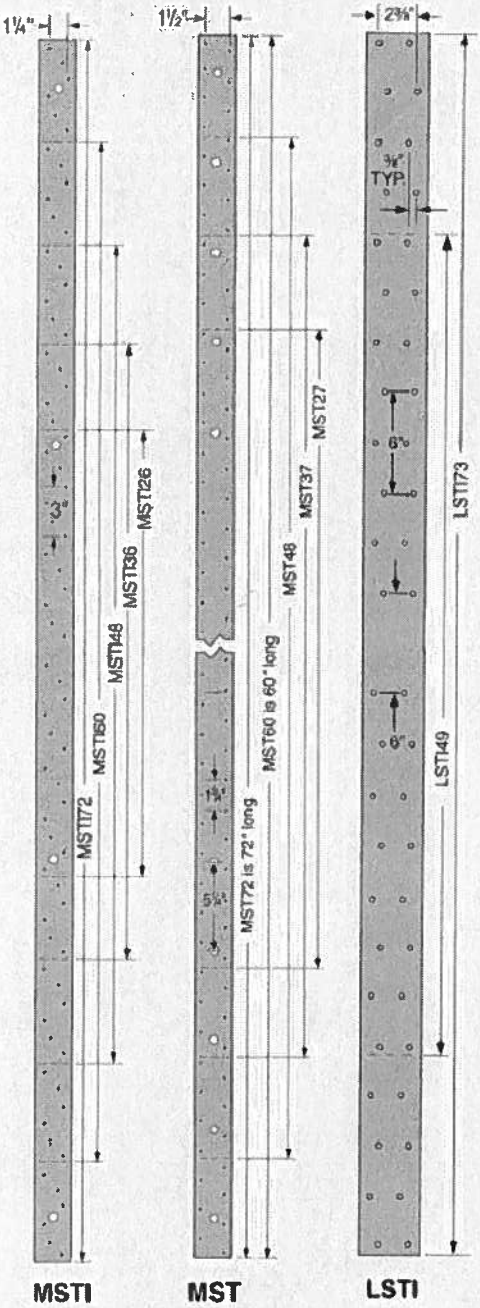
Install Strap Ties where plates or soles are cut, at wall intersections, and as ridge ties. LSTA and MSTA straps are engineered for use on 1½" members. The 3" center-to-center nail spacing reduces the possibility of splitting. For the MST, this may be a problem on lumber narrower than 3½"; either fill every nail hole with 10dx1½" nails or fill every other nail hole with 16d commons. Reduce the allowable load based on the size and quantity of fasteners used. The LSTI light strap ties are suitable where gun-nailing is necessary through diaphragm decking and wood chord open web trusses.

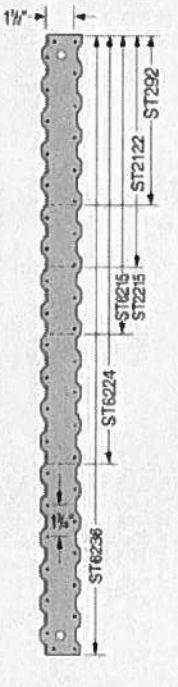
FINISH: HST–Simpson gray paint; PS–HDG; all others–galvanized. Some products are available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

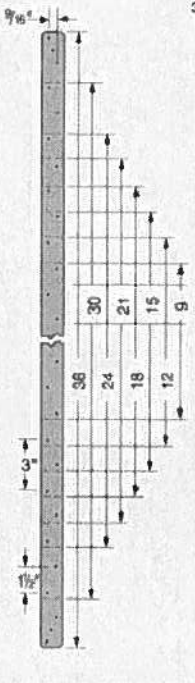
INSTALLATION: Use all specified fasteners. See General Notes.

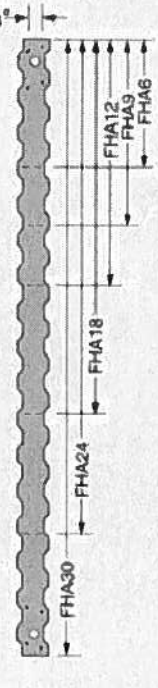
OPTIONS: Special sizes can be made to order. See also HCST.

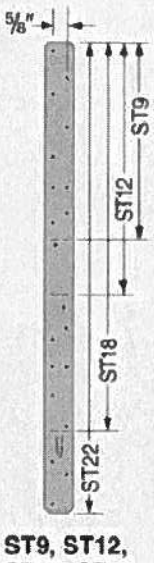
CODES: BOCA, ICBO, SBCCI NER-413, NER-443; ICBO 4935, 5357; Dade County, FL. 00-1023.05 (MSTA30, MSTA36, ST12, ST18, ST22); City of L.A. RR 25119, RR 25149, RR 25281.

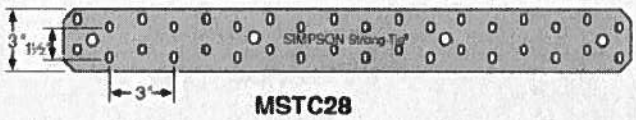


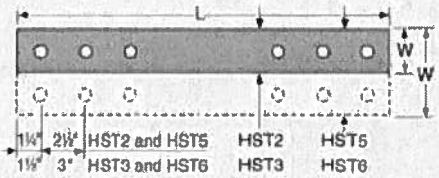


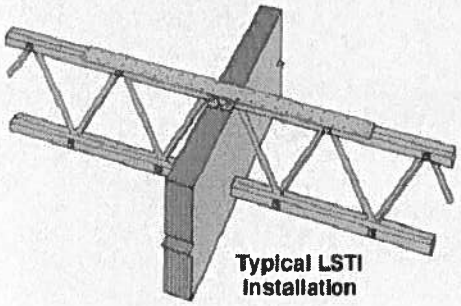


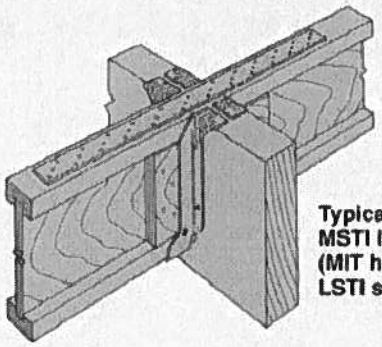


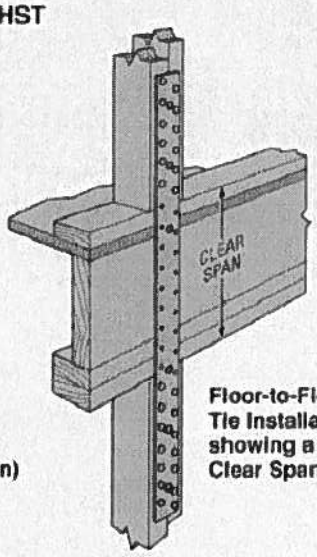








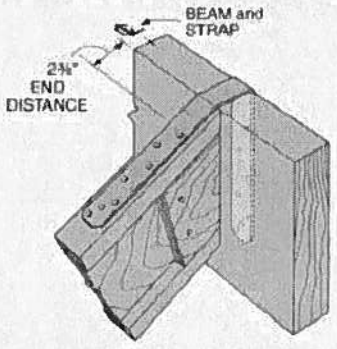




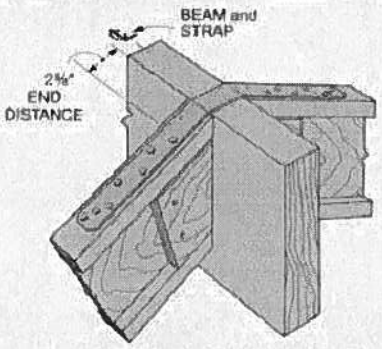
RPS/ST/FHA/PS/HST/LSTA/LSTI/MST/MSTA/MSTC/MSTI

STRAP TIES

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Allowable Tension Loads Floor (100)	(133)	(160)
RPS18	16	1½	18 5/16	12-16d	810	1080	1295
RPS22	16	1½	22 5/16	16-10d	905	1205	1445
RPS28	16	1½	28 5/16	12-16d	810	1080	1295
LSTA9	20	1¼	9	8-10d	450	605	725
LSTA12	20	1¼	12	10-10d	565	755	905
LSTA15	20	1¼	15	12-10d	680	905	1085
LSTA18	20	1¼	18	14-10d	790	1055	1265
LSTA21	20	1¼	21	16-10d	905	1205	1295
LSTA24	20	1¼	24	18-10d	1015	1295	1295
ST292	20	2 5/16	9 5/16	12-16d	790	1055	1130
ST2122	20	2 5/16	12 5/16	16-16d	1070	1425	1505
ST2115	20	¾	16 5/16	10-16d	450	600	600
ST2215	20	2 5/16	16 5/16	20-16d	1270	1695	1695
LSTA30	18	1¼	30	22-10d	1255	1670	1715
LSTA36	18	1¼	36	26-10d	1480	1715	1715
LSTI49	18	3¼	49	32-10dx1½	1455	1940	2330
LSTI73	18	3¼	73	48-10dx1½	2185	2910	3495
MSTA9	18	1¼	9	8-10d	455	610	730
MSTA12	18	1¼	12	10-10d	570	760	910
MSTA15	18	1¼	15	12-10d	685	910	1095
MSTA18	18	1¼	18	14-10d	800	1065	1275
MSTA21	18	1¼	21	16-10d	910	1215	1460
MSTA24	18	1¼	24	18-10d	1025	1370	1640
MSTA30	16	1¼	30	22-10d	1265	1685	2025
MSTA36	16	1¼	36	26-10d	1495	1995	2135
ST6215	16	2 5/16	16 5/16	20-16d	1330	1775	2130
ST6224	16	2 5/16	23 5/16	28-16d	1890	2520	2630
ST9	16	1¼	9	8-16d	530	705	850
ST12	16	1¼	11¾	10-16d	665	885	1065
ST18	16	1¼	17¾	14-16d	900	1200	1200
ST22	16	1¼	21¾	18-16d	1025	1370	1370
MSTC28	16	3	28¼	36-16d sinkers	2070	2760	3310
MSTC40	16	3	40¼	52-16d sinkers	2990	3985	4740
MSTC52	16	3	52¼	62-16d sinkers	3555	4740	4740
MSTC66	16	3	65¼	76-16d sinkers	4390	5855	5855
MSTC78	14	3	77¼	76-16d sinkers	4390	5855	5855
ST6236	16	2 5/16	33 5/16	40-16d	2575	3430	3430
FHA6	12	1 5/16	6⅜	8-16d	550	735	885
FHA9	12	1 5/16	9	8-16d	550	735	885
FHA12	12	1 5/16	11⅞	8-16d	550	735	885
FHA18	12	1 5/16	17¾	8-16d	550	735	885
FHA24	12	1 5/16	23⅞	8-16d	550	735	885
FHA30	12	1 5/16	30	8-16d	550	735	885
MSTI26	12	2 5/16	26	26-10dx1½	1130	1510	1810
MSTI36	12	2 5/16	36	36-10dx1½	1565	2090	2505
MSTI48	12	2 5/16	48	48-10dx1½	2135	2850	3420
MSTI60	12	2 5/16	60	60-10dx1½	2760	3680	4415
MSTI72	12	2 5/16	72	72-10dx1½	3310	4415	4725



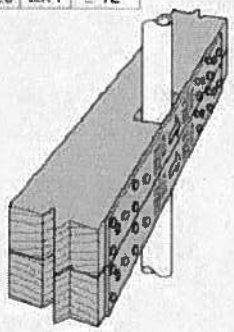
Typical LSTA Installation (hanger not shown)



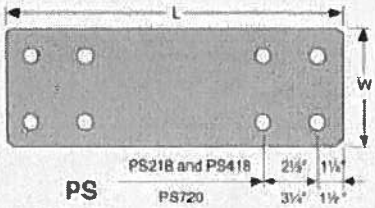
Typical LSTA Installation (hanger not shown)

Model No.	Plate	Notch Width
RPS18	2x4	≤ 5½"
RPS22	2x6	≤ 5½"
RPS28	2x4	≤ 12"

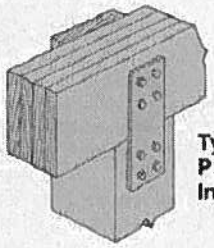
RPS



Typical RPS Installation



PS



Typical PS720 Installation

Model No.	Ga	Dimensions W	Dimensions L	Bolts Qty	Bolts Dia
PS218[5]	7	2	18	4	⅝
PS418[5]	7	4	18	4	⅝
PS720[5]	7	6¾	20	8	½

Floor-to-Floor Clear Span Table

Model No.	Clear Span	Fasteners (Total)	Allowable Tension Load (133)	(160)
MSTC28	18	12-16d sinker	920	1105
	16	16-16d sinker	1225	1470
MSTC40	18	28-16d sinker	2145	2575
	16	36-16d sinker	2455	2945
MSTC52	18	44-16d sinker	3375	4050
	16	48-16d sinker	3680	4415
MSTC66	18	64-16d sinker	5035	5855
	16	68-16d sinker	5350	5855
MSTC78	18	80-16d sinker	5855	5855
	16	80-16d sinker	5855	5855
MST37	18	20-16d	1905	2285
	16	22-16d	2100	2515
MST48	18	32-16d	3135	3765
	16	34-16d	3330	4000
MST60	18	46-16d	4785	5740
	16	48-16d	4990	5800
MST72	18	56-16d	5800	5800
	16	58-16d	5800	5800
MSTI36	18	14-10dx1½	810	975
	16	16-10dx1½	930	1115
MSTI48	18	26-10dx1½	1545	1855
	16	28-10dx1½	1660	1990
MSTI60	18	38-10dx1½	2330	2800
	16	40-10dx1½	2455	2945
MSTI72	18	50-10dx1½	3065	3680
	16	52-10dx1½	3190	3830

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Bolts Qty	Bolts Dia	Allowable Tension Loads Nails Floor (100)	Nails (133)	Nails (160)	Bolts[5] Floor (100)	Bolts[5] (133)	Bolts[5] (160)
MST27	12	2 5/16	27	30-16d	4	½	2070	2760	2790	1295	1725	2070
MST37	12	2 5/16	37½	42-16d	6	½	2860	3815	3815	1825	2435	2920
MST48	12	2 5/16	48	46-16d	8	½	3345	4460	4460	2225	2970	3560
MST60	10	2 5/16	60	56-16d	10	½	4350	5800	5800	2670	3565	4275
MST72	10	2 5/16	72	56-16d	10	½	4350	5800	5800	2670	3565	4275
HST2	7	2½	21¼	—	6	⅝	—	—	—	3130	4175	5005
HST5	7	5	21¼	—	12	⅝	—	—	—	6385	8510	10210
HST3	3	3	25⅝	—	6	¾	—	—	—	4645	6195	7435
HST6	3	6	25⅝	—	12	¾	—	—	—	9350	12465	14955

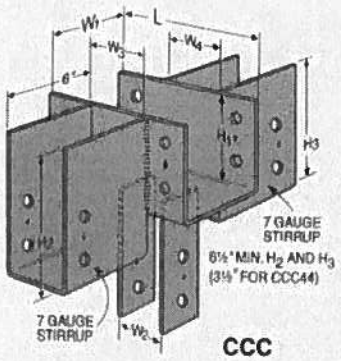
1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed. Floor loads may not be increased for other load durations.
2. 10dx1½" nails may be substituted where 16d sinkers are specified at 0.80 of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers (9 gauge x 3¼") or 10d commons may be substituted where 16d commons are specified at 0.84 of the table loads.
5. Allowable bolt loads are based on parallel-to-grain loading and these minimum member thicknesses: MST–2½"; HST2 and HST5–4"; HST3 and HST6–4½".
6. PS strap design loads must be determined by the building designer for each installation. Bolts are installed both perpendicular and parallel-to-grain.
7. Use half of the nails at each member being connected to achieve the listed loads.

CCC/CCT/ECCL ALTERNATE OPTIONS

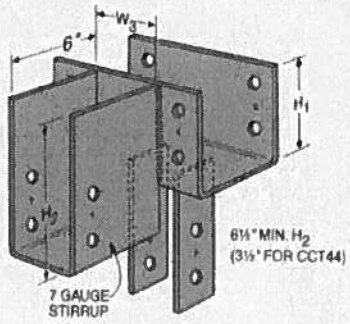
There are cost-effective alternatives for replacing column caps by using a combination of connectors. Here are some examples. Designer must specify the options required.

For more information, request Form T-CC and the Product Worksheet.
NOTE: The side cap will be welded flush with the top of the main cap.

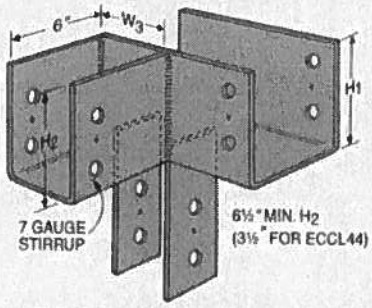
Side stirrups are available in different depths.



CCC

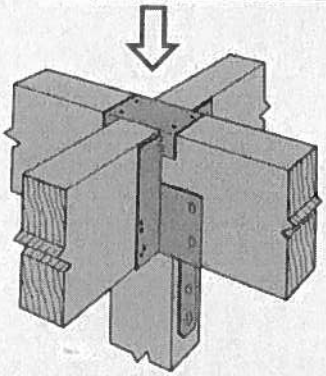


CCT

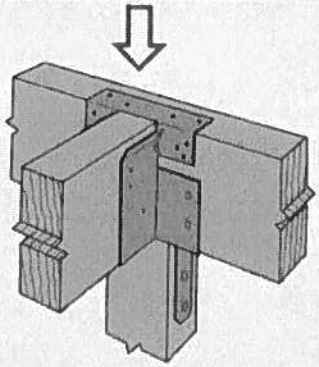


ECCLL (for left), shown above. Order ECCLR for right condition.

Instead of the column caps, consider these connector combinations.

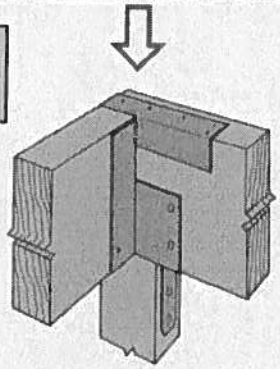


CC and WD



CC and GLT

Order each connector separately.

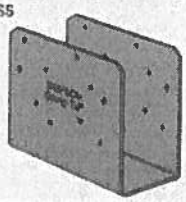


ECC and HW

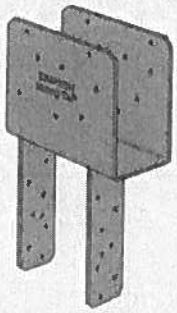
CCQ/ECCQ COLUMN CAPS

This design uses SDS screws to provide faster installation and maintain the wood cross section. The SDS screws provide for a lower profile compared to standard through bolts.
MATERIAL: CCQ5, ECCQ5, CCQ7, ECCQ7—3 gauge; all others—7 gauge.
FINISH: Simpson gray paint.
INSTALLATION: Fasteners provided. See General Notes.
- **Install Simpson's code-recognized SDS¼x2½ wood screws, which are provided with the column cap. (Lag screws will not achieve the same load.)**

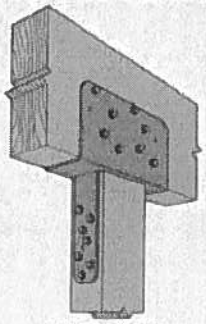
OPTIONS: Straps may be rotated. For end conditions, specify ECCQ.
CODE: Submitted to ICBO 10/99.



CCQ0Q46SDS2.5



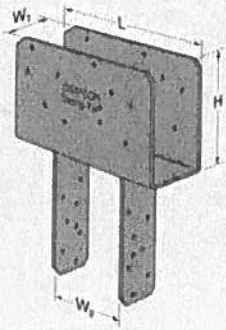
ECCQ46SDS2.5



CCQ46SDS2.5 Typical Installation

Model No.	Dimensions W1	W2	L CCQ	L ECCQ	H	No. of SDS¼" x 2½" Screws Beam	Post	Allowable Loads Uplift CCQ (133)	Uplift CCQ (160)	Uplift ECCQ 133/160	Down CCQ (100)	Down ECCQ (100)
CCQ3-4SDS2.5	3¼	3⅜	11	8½	7	16	14	5680	5680	3695	14765	6125
CCQ3-6SDS2.5	3¼	5½	11	8½	7	16	14	5680	5680	3695	19250	9625
CCQ44SDS2.5	3⅜	3⅜	11	8½	7	16	14	5680	5680	4040	15315	7655
CCQ46SDS2.5	3⅜	5½	11	8½	7	16	14	5955	7145	4040	24065	12030
CCQ48SDS2.5	3⅜	7½	11	8½	7	16	14	5955	7145	4040	24065	16405
CCQ5-4SDS2.5	5¼	3⅜	11	8½	7	16	14	5680	5680	4040	24215	10045
CCQ5-6SDS2.5	5¼	5½	11	8½	7	16	14	6270	7245	5535	31570	15785
CCQ5-8SDS2.5	5¼	7½	11	8½	7	16	14	6270	7245	5535	31570	21525
CCQ64SDS2.5	5½	3⅜	11	8½	7	16	14	5680	5680	4040	25990	12030
CCQ66SDS2.5	5½	5½	11	8½	7	16	14	5955	7145	4040	37815	18905
CCQ68SDS2.5	5½	7½	11	8½	7	16	14	5955	7145	4040	37815	25780
CCQ6-7.13SDS2.5	5½	7⅛	11	8½	7	16	14	5955	7145	4040	37815	24490
CCQ74SDS2.5	6⅛	3⅜	11	8½	7	16	14	5680	5680	4040	31895	13230
CCQ765DS2.5	6⅛	5½	11	8½	7	16	14	6270	7245	5535	41580	20790
CCQ77SDS2.5	6⅛	6⅛	11	8½	7	16	14	6270	7245	5535	41580	25515
CCQ78SDS2.5	6⅛	7⅛	11	8½	7	16	14	6270	7245	5535	41580	28350

1. Downloads are determined using Fc⊥ equal to: 560 psi for glulam sizes and 625 psi for all others; reduce where end bearing value of post, L/R of post, or other criteria are limiting.
2. **Spliced conditions must be detailed by the specifier to transfer tension loads between spliced members by means other than the column cap.**
3. **Uplift loads do not apply to splice conditions.**



CCQ46SDS2.5

Caps & Bases

LCB/CB COLUMN BASES

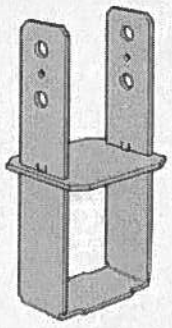
SIMPSON Strong-Tie

FINISH: LCB, CB44, CB46, CB66—galvanized; all other CB-Simpson gray paint or HDG.

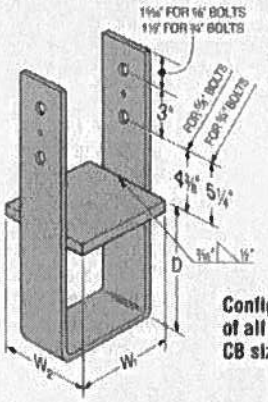
INSTALLATION: • Use all specified fasteners. See General Notes.
- For full loads, minimum side cover required is 3" for CB, 2" for LCB.
- Install all models with bottom of base plate flush with concrete.
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations *(such as fences or unbraced carports)*.

OPTIONS: • The LCB may be shipped unassembled; specify "Disassembled".
- LCB and CB are available in rough size. Other sizes available for CB specify W1 and W2 dimensions. Consult Simpson for bolt sizes and allowable loads. See PBS.

CODES: See page 10 for Code Listing Key Chart.



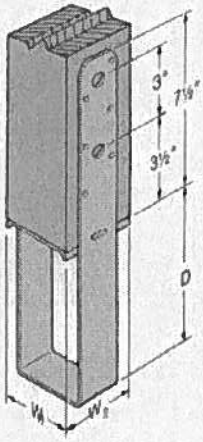
CB44
(CB46, CB66 similar)



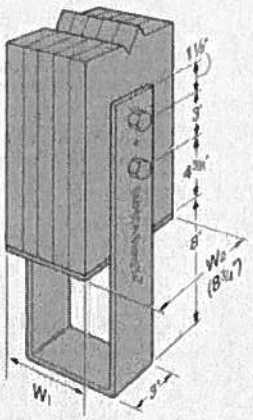
Configuration of all other CB sizes

Model No.	Nominal Column Size	Material: Strap	Material: Base (Ga)	Dimensions: W1	Dimensions: W2	Dimensions: D	Column Fasteners: Nails	Column Fasteners: Machine Bolts Qty	Column Fasteners: Machine Bolts Dia	Uplift Avg Ult	Allowable Uplift Loads: Nails (133)	Allowable Uplift Loads: Nails (160)	Allowable Uplift Loads: Bolts (133)	Allowable Uplift Loads: Bolts (160)	Code Ref.
LCB44	4x4	12 ga x2	16	3⅜	3½	6½	12-16d	2	½	17853	2255	2705	3545	4250	2, 40
CB44	4x4	7 ga x2	7	3⅜	3⅜	8	—	2	⅝	14350	—	—	4200	4200	2, 40, 82
LCB46	4x6	12 ga x2	16	3⅜	5½	6½	12-16d	2	½	17853	2255	2705	3530	4240	2, 40
CB46	4x6	7 ga x2	7	3⅜	5½	8	—	2	⅝	14350	—	—	4200	4200	2, 40, 82
CB48	4x8	7 ga x2	7	3⅜	7½	8	—	2	⅝	14350	—	—	4200	4200	
CB5-4.5	GLULAM	7 ga x3	7	4⅜	5⅛	8	—	2	⅝	14350	—	—	4200	4200	
CB5-6	GLULAM	7 ga x3	7	6⅛	5⅛	8	—	2	⅝	14350	—	—	4200	4200	
LCB66	6x6	12 ga x2	16	5½	5½	5½	12-16d	2	½	17853	2255	2705	3525	4230	2, 40
CB64	6x4	7 ga x3	7	5½	3⅜	8	—	2	⅝	14350	—	—	4200	4200	2, 40, 82
CB66	6x6	7 ga x3	7	5½	5½	8	—	2	⅝	14350	—	—	4200	4200	
CB6-7	6x	7 ga x3	7	5½	7	8	—	2	⅝	14350	—	—	4200	4200	
CB7½-4	PSL	3 ga x3	7	7½	3½	8	—	2	¾	20650	—	—	6650	6650	
CB7½-6	PSL	3 ga x3	7	7½	5½	8	—	2	¾	20650	—	—	6650	6650	
CB7½-7	PSL	3 ga x3	7	7½	7	8	—	2	¾	20650	—	—	6650	6650	
CB68	6x8	7 ga x3	7	5½	7½	8	—	2	⅝	14350	—	—	4200	4200	
CB7-6	GLULAM	3 ga x3	7	6⅛	6¾	8	—	2	¾	20650	—	—	6650	6650	
CB7-7.5	GLULAM	3 ga x3	7	7⅝	6¾	8	—	2	¾	20650	—	—	6650	6650	
CB7-9	GLULAM	3 ga x3	7	9⅛	6¾	8	—	2	¾	20650	—	—	6650	6650	
CB7-10.5	GLULAM	3 ga x3	7	10⅝	6¾	8	—	2	¾	20650	—	—	6650	6650	
CB86	8x6	3 ga x3	7	7½	5½	8	—	2	¾	20650	—	—	6650	6650	
CB88	8x8	3 ga x3	7	7½	7½	8	—	2	¾	20650	—	—	6650	6650	
CB9-6	GLULAM	3 ga x3	7	6⅛	8¾	8	—	2	¾	20650	—	—	6650	6650	
CB9-7.5	GLULAM	3 ga x3	7	7⅝	8¾	8	—	2	¾	20650	—	—	6650	6650	
CB9-9	GLULAM	3 ga x3	7	9⅛	8¾	8	—	2	¾	20650	—	—	6650	6650	
CB9-10.5	GLULAM	3 ga x3	7	10⅝	8¾	8	—	2	¾	20650	—	—	6650	6650	
CB1010	10x10	3 ga x3	3	9½	9½	8	—	2	¾	20650	—	—	6650	6650	
CB1012	10x12	3 ga x3	3	9½	11½	8	—	2	¾	20650	—	—	6650	6650	
CB1212	12x12	3 ga x3	3	11½	11½	8	—	2	¾	20650	—	—	6650	6650	

1. Uplift loads have been increased 33% and 60% for earthquake or wind loading, with no further increase allowed; reduce where other loads govern.
2. PSL is parallel strand lumber.
3. LCB nail or bolt loads do not combine.



LCB



CB9
(CB5, CB7 similar)
for glulam column

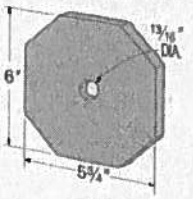
RP6 RETRO PLATE

The RP6 heavy steel plate fits on the outside of masonry buildings, helps tie the walls to the roof or floor structure with a ¾" diameter rod.

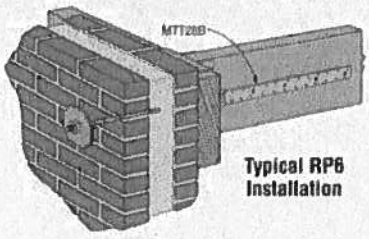
FINISH: Simpson gray paint. Optional hot-dip galvanized finish; see Corrosion-Resistance, page 5, and specify HDG.

MATERIAL: ⅜" steel

INSTALLATION: Use a ¾" diameter rod.



RP6



Typical RP6 Installation

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☐	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
N/A	☐	e) Location and size of skylights
☐	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- ☑ ☐ a) Rooms labeled and dimensioned
- ☑ ☐ b) Shear walls
- ☑ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☑ ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- N/A ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☑ ☐ f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- ☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☑ ☐ b) All posts and/or column footing including size and reinforcing
- N/A ☐ c) Any special support required by soil analysis such as piling
- ☑ ☐ d) Location of any vertical steel

Roof System:

- ☑ ☐ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- N/A ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- ☑ ☐ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☑ ☐ **b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

N/A ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

N/A ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

N/A ☐ b) Floor joist size and spacing

N/A ☐ c) Girder size and spacing

N/A ☐ d) Attachment of joist to girder

N/A ☐ e) Wind load requirements where applicable

☑ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☑ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

N/A ☐ **Disclosure Statement for Owner Builders**

☐ BANK ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☑ ☐ **Private Potable Water**

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
   (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

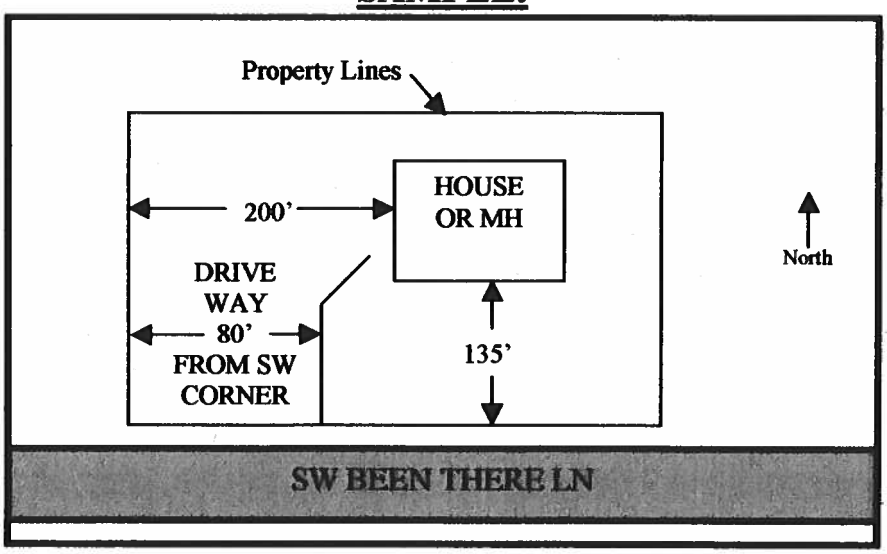
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

Notice of Treatment 12110

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: BAYAHOR
City Lake City Phone 7521103

Site Location: Subdivision
Lot # Block# Permit # 24452
Address 472 sw Sapphing Gln

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☑ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	3212	868	8
Plumbing Traps/lines	—	—	2

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

8/9/06 1100 F254
Date Time Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 09-5S-16-03499-005 **Building permit No.** 000024452

Use Classification SFD,UTILITY **Fire:** 118.69

Permit Holder GARY JOHNSON **Waste:** 184.25

Owner of Building PAUL & AUDREY WENDEL **Total:** 302.94

Location: 472 SW SAPLING GLEN, LAKE CITY, FL

Date: 11/22/2006

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 600B-97 — **Residential Component Prescriptive Method B** — **Department of Community Affairs** — **NORTH 1 2 3**

Compliance with Method B Chapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B-97 for single and multifamily residences of 3 stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptive measures listed in Table 6B-1 of this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C-97. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the Code.

PROJECT NAME: AND ADDRESS:	WENDEL JOB	BUILDER:	GARY JOHNSON CONST INC
		PERMITTING OFFICE:	CLIMATE ZONE: 1 ☐ 2 ☐ 3 ☑
OWNER:	PAUL WENDEL	PERMIT NO.:	24452 JURISDICTION NO.: 221000

GENERAL DIRECTIONS

1. New construction including additions which incorporates any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other non-vertical roof glass.
2. Choose one of the component packages "A" through "E" fromTable 6B-1 by which you intend to comply with the Code. Circle the column of the package you have chosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages", Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

	Please Print	CK
1. **Compliance package chosen (A-F)**	1.	
2. **New construction or addition**	2.	
3. **Single family detached or Multifamily attached**	3.	
4. **If Multifamily—No. of units covered by this submission**	4.	
5. **Is this a worst case?** (yes / no)	5.	
6. **Conditioned floor area** (sq. ft.)	6. 2197	
7. **Predominant eave overhang** (ft.)	7. 2	
8. **Glass type and area :**	Single Pane / Double Pane	
a. Clear glass	8a. ____ sq. ft. / 275 sq. ft.	
b. Tint, film or solar screen	8b. ____ sq. ft. / ____ sq. ft.	
9. **Percentage of glass to floor area**	9. 12 %	
10. **Floor type, area or perimeter, and insulation:**		
a. Slab on grade (R-value)	10a. R= ____ ____ lin. ft.	
b. Wood, raised (R-value)	10b. R= ____ ____ sq. ft.	
c. Wood, common (R-value)	10c. R= ____ ____ sq. ft.	
d. Concrete, raised (R-value)	10d. R= ____ ____ sq. ft.	
e. Concrete, common (R-value)	10e. R= ____ ____ sq. ft.	
11. **Wall type, area and insulation:**		
a. Exterior: 1. Masonry (Insulation R-value)	11a-1 R= ____ ____ sq. ft.	
2. Wood frame (Insulation R-value)	11a-2 R= 13 ____ sq. ft.	
b. Adjacent: 1. Masonry (Insulation R-value)	11b-1 R= ____ ____ sq. ft.	
2. Wood frame (Insulation R-value)	11b-2 R= ____ ____ sq. ft.	
12. **Ceiling type, area and insulation:**		
a. Under attic (Insulation R-value)	12a. R= 30 ____ sq. ft.	
b. Single assembly (Insulation R-value)	12b. R= ____ ____ sq. ft.	
13. **Air Distribution System: Duct insulation, location**	13. R= ____	
14. **Cooling system**	14a. Type: CENTRAL	
(Types: central, room unit, package terminal A.C., gas, none)	14b. SEER/EER: 12	
	14c. Capacity: ____	
15. **Heating system:**	15a. Type: HEAT PUMP	
(Types: heat pump, elec. strip, nat. gas, L.P. gas, gas h.p., room or PTAC, none)	15b. HSPF/COP/AFUE:	
	15c. Capacity: 7.7	
16. **Hot water system:**	16a. Type: ELECTRIC	
(Types: elec., nat. gas, L.P. gas, solar, heat rec., ded. heat pump, other, none)	16b. EF: .90	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code

PREPARED BY: Gary Johnson DATE: 3/27/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code

OWNER AGENT: _____ DATE: _____

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

Revised 1998

TABLE 6B-1 **MINIMUM REQUIREMENTS** **Climate Zones 1 2 3**

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION					TO BE INSTALLED
		A	B	C	D	E	
GLASS	Max.%of glass to Floor Area	15%	15%	20%	20%	25%	12 %
	Type	Double Clear (DC)	Double Tint (DT)	Double Tint (DT)	Double Clear (DC)	Double Tint (DT)	DC: ☑ DT: ☐
	Overhang	1'4"	2'	2'	2'	2'	2.75 FEET
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE.					EXT: R = ___ ADJ: R = ___ COM: R = ___
	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD FRAME WALLS R-11					EXT: R = 13 ADJ: R = ___ COM: R = ___
CEILINGS		R-30	R-38	R-30	R-38	R-30	UNDER ATTIC: R = 30 COMMON: R = ___
		(NO SINGLE ASSEMBLY CEILINGS ALLOWED)					
FLOORS	Slab-On-Grade	R-0					R = 0
	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)					R = ___
	Raised Concrete	R-7					R = ___
DUCTS		R-6	R-6	R-6	COND.	R-6	R = 6 COND. ☐
SPACE COOLING (SEER)		11.5	10.5	12.0	10.5	10.0*	SEER = 12
HEAT	Elect. (HSPF)	7.7	7.1	8.0	7.1	6.8*	COP= 7.7
	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)					AFUE = ___
HOT WATER SYSTEM	Electric Resistance**	EF .90	EF .90	NOT ALLOWED (SEE BELOW)	EF .90	NOT ALLOWED (SEE BELOW)	EF = .90
	Gas & Oil **	MINIMUM EF OF .54				NATURAL GAS ONLY (SEE BELOW)	EF = ___
	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.					DHP: ☐ EF= ___ HRU: ☐ SOLAR: ☐ EF= ___

* Single package units minimum SEER=9.7, HSPF = 6.6.

** Minimum efficiencies for gas and electric hot water systems apply to to 40 gallon water heaters. Refer to Table 6-12 for minimum Code efficiencies for oil water heaters and other sizes.

DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions: 1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multi-story house.

Wall, Ceiling and Floor Insulation Values: The R-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The R-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multifamily building. "Adjacent" components separate conditioned space from unconditioned but enclosed space. "Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "COND" indicates that the ducts must be installed within the conditioned space; that is, the ductwork shall be located on the conditioned side of the insulation. Ducts in conditioned space are acceptable for any prescriptive package.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTAC's equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of section 608.1.ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed", an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options". See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having an EF of .88 or greater, or natural gas systems with EF .54 or greater may be used in conjunction with these systems.

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Multi-story Houses	606.1	Air barrier on perimeter of floor cavity between floors.	✓
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	N/A
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	✓
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1SVO487-Z0121100422

Truss Fabricator:	Anderson Truss Company
Job Identification:	6-112--GARY JOHNSON Wendell -- , **
Truss Count:	36
Model Code:	Florida Building Code 2004
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.24.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 32.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Notes:

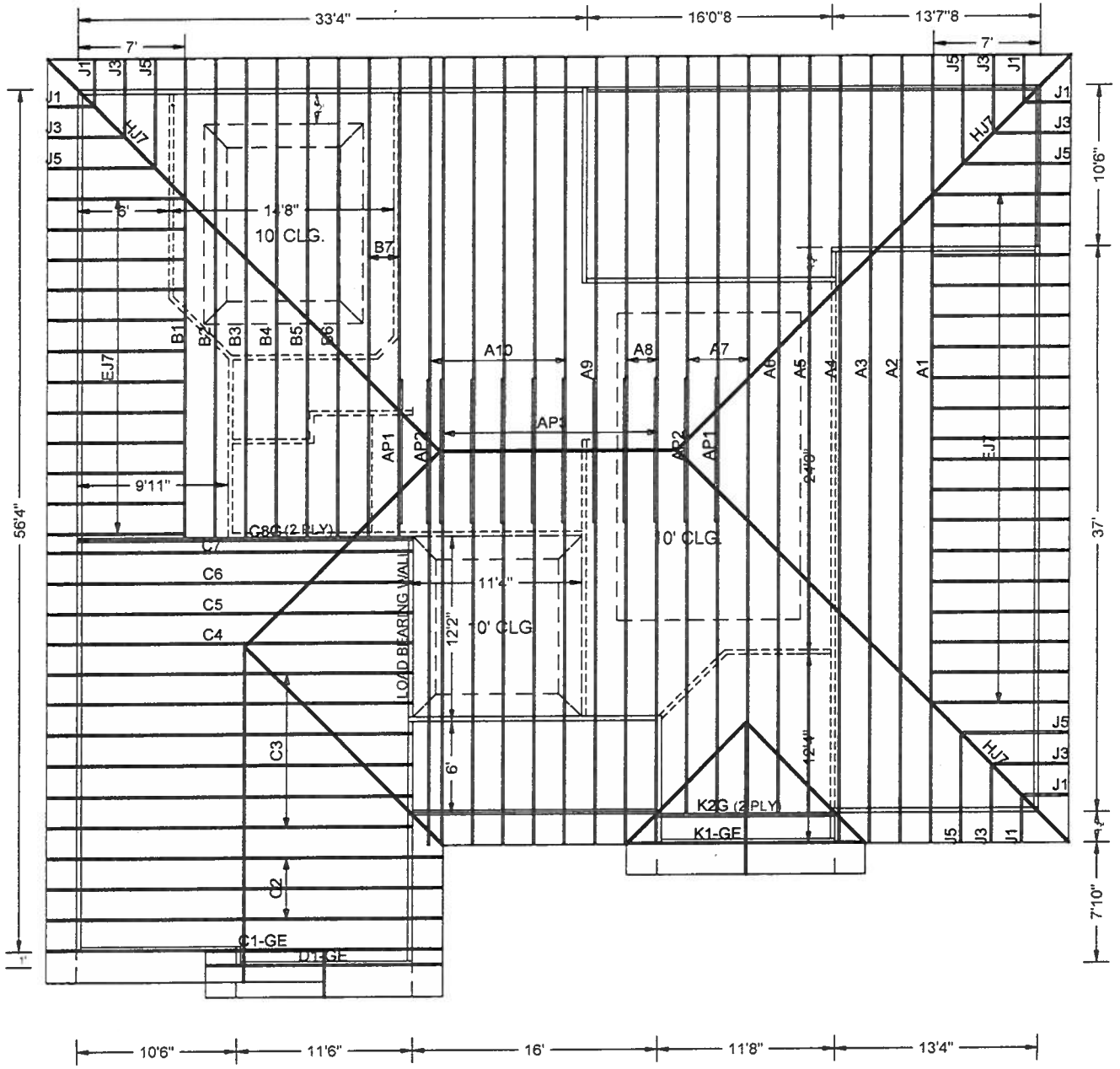
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: CNBRGBLK-BRCLBSUB-A11015EE-GBLLETIN-PIGBACKB-

Seal Date: 03/21/2006

-Truss Design Engineer-
Arthur R. Fisher
Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

#	Ref Description	Drawing#	Date
1	53340--A1	06080010	03/21/06
2	53341--A2	06080011	03/21/06
3	53342--A3	06080012	03/21/06
4	53343--A4	06080013	03/21/06
5	53344--A5	06080014	03/21/06
6	53345--A6	06080015	03/21/06
7	53346--A7	06080016	03/21/06
8	53347--A8	06080017	03/21/06
9	53348--A9	06080018	03/21/06
10	53349--A10	06080019	03/21/06
11	53350--B1	06080020	03/21/06
12	53351--B2	06080021	03/21/06
13	53352--B3	06080022	03/21/06
14	53353--B4	06080023	03/21/06
15	53354--B5	06080024	03/21/06
16	53355--B6	06080025	03/21/06
17	53356--B7	06080026	03/21/06
18	53357--C1-GE	06080027	03/21/06
19	53358--C2	06080001	03/21/06
20	53359--C3	06080002	03/21/06
21	53360--C4	06080003	03/21/06
22	53361--C5	06080004	03/21/06
23	53362--C6	06080005	03/21/06
24	53363--C7	06080006	03/21/06
25	53364--C8G	06080028	03/21/06
26	53365--D1-GE	06080029	03/21/06
27	53366--HJ7	06080030	03/21/06
28	53367--EJ7	06080007	03/21/06
29	53368--J5	06080008	03/21/06
30	53369--J3	06080009	03/21/06
31	53370--J1	06080031	03/21/06
32	53371--K1-GE	06080032	03/21/06
33	53372--K2G	06080033	03/21/06
34	53373--AP1	06080034	03/21/06
35	53374--AP2	06080035	03/21/06
36	53375--AP3	06080036	03/21/06



3/20/06

#6-112 GARY JOHNSON - WENDELL

Scale: 3/32" = 1'

PAGE NO: 1 OF 1

(o 112--GARY JOHNSON WendeTI -- , ** - A1)

Top chord 2x6 SP #1 Dense :T1, T5 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3 :W5, W7 2x4 SP #2 Dense:

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

#1 hip supports 7-0-0 jacks with no webs.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

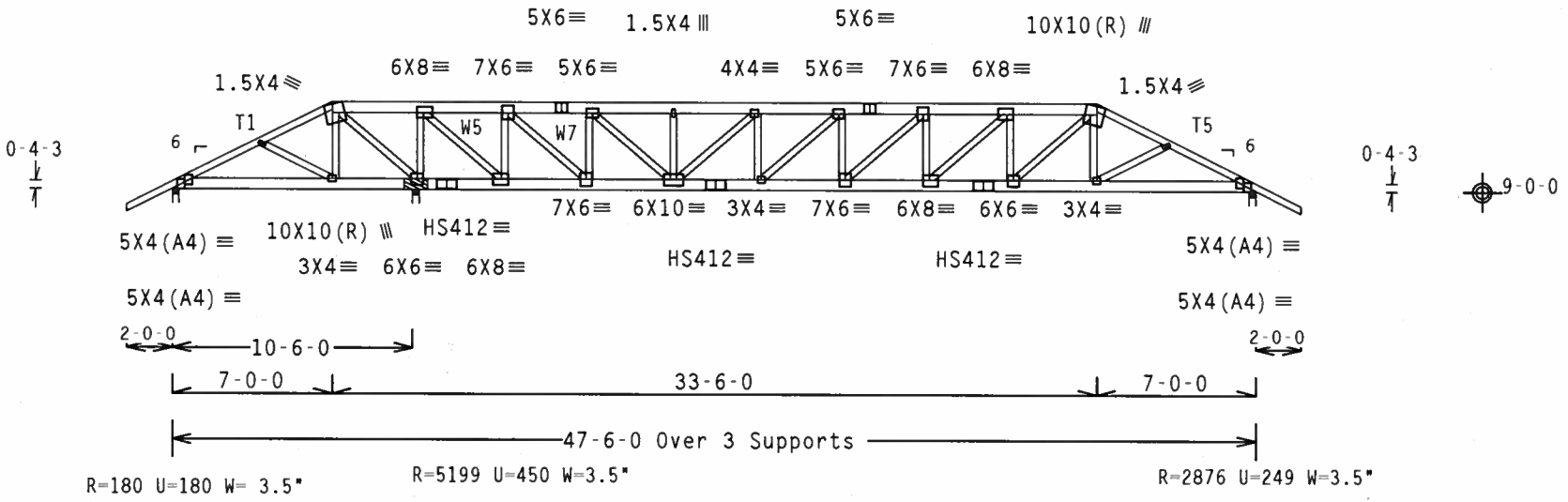
Bearing blocks: Nail type: 10d_Box_or_Gun_(0.128"x3",_min.)_nails

BRG	X-LOC	#BLOCKS	LENGTH/BLK	#NAILS/BLK	WALL PLATE
1	10.500'	1	12"	12	Match Truss

Bearing block to be same size and species as bottom chord.
Refer to drawing CNBRGBLK1103 for additional information.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53340
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080010
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3321
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - A2)

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #1 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

(B) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

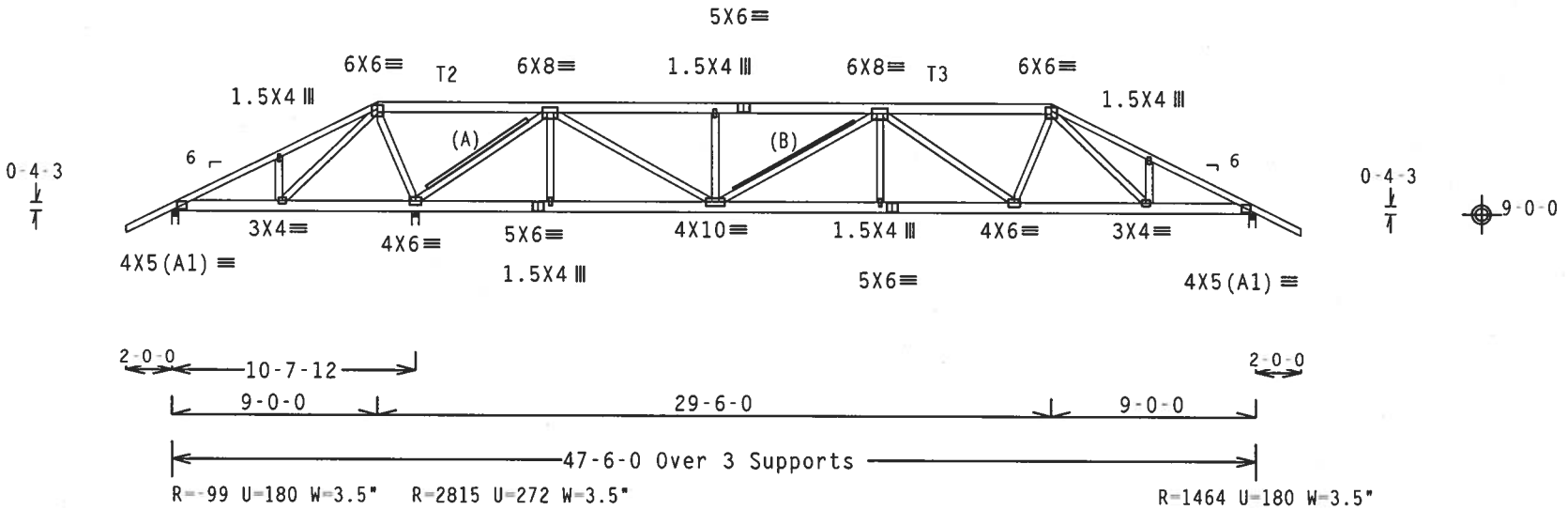
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 06

TC LL	20.0 PSF	REF	R487-- 53341
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080011
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3331
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON wendell -- , ** - A3)

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #1 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

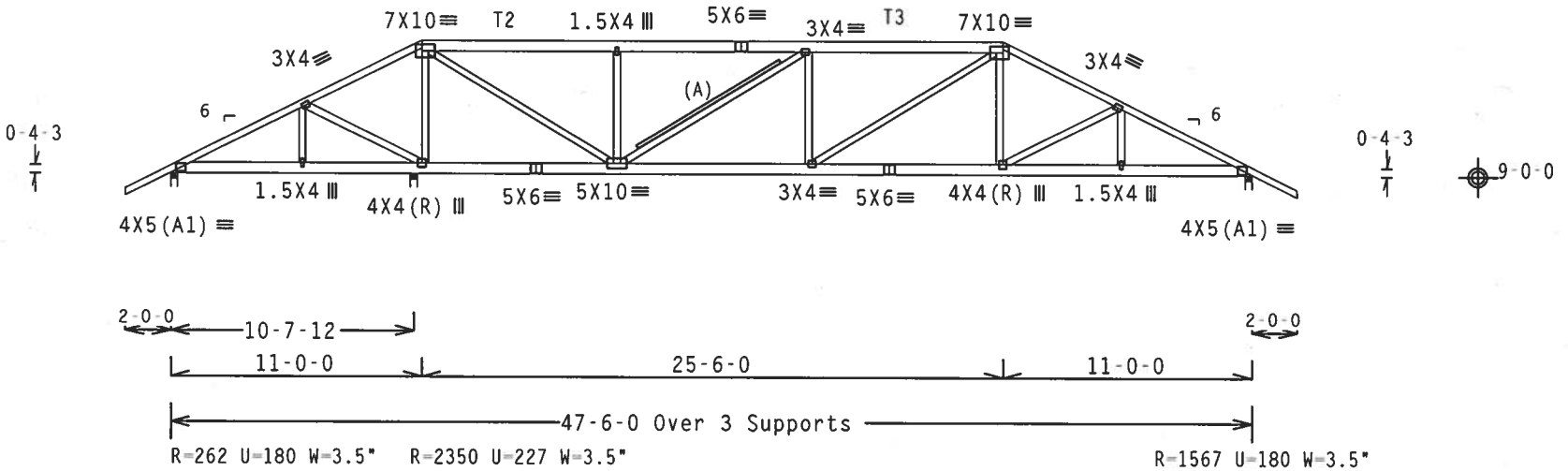
(A) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 21 '06

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53342
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080012
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	3337
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON wendell -- , ** - A4)

Top chord 2x6 SP #1 Dense :T1, T6 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

(B) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

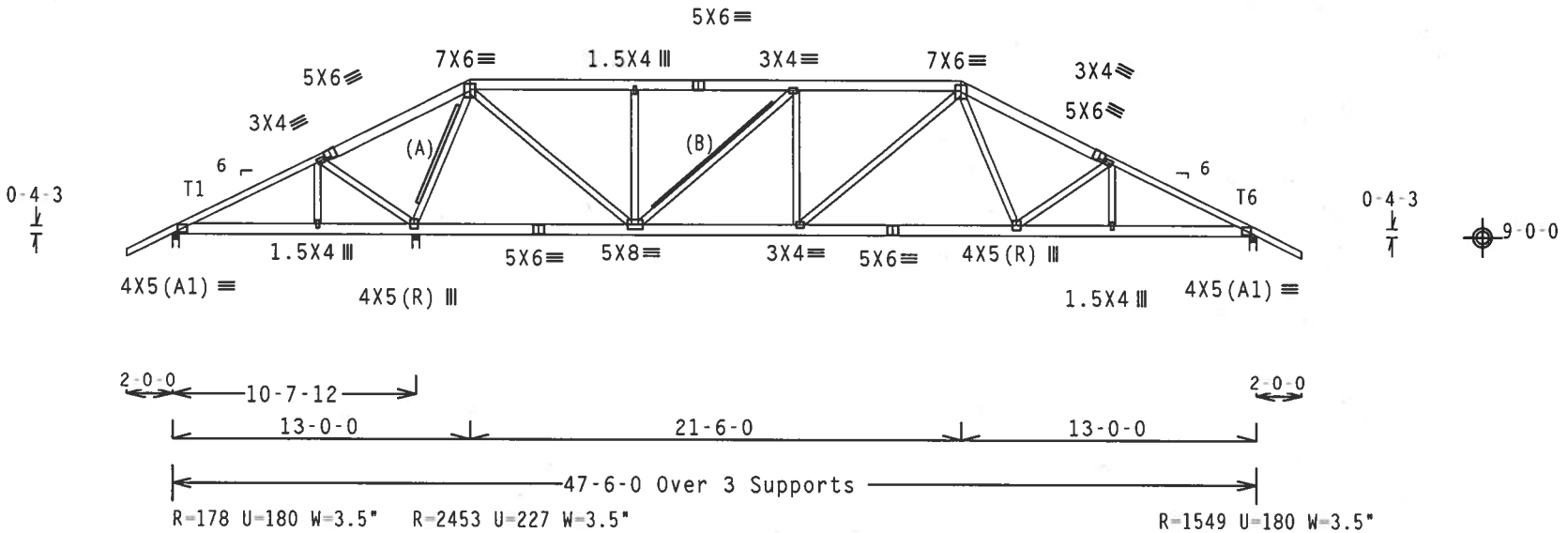
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W. K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53343
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080013
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3348
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV04[illegible]201

(0-112--GARY JOHNSON Wendell--- , **--A5)

Top chord 2x6 SP #1 Dense :T1, T6 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

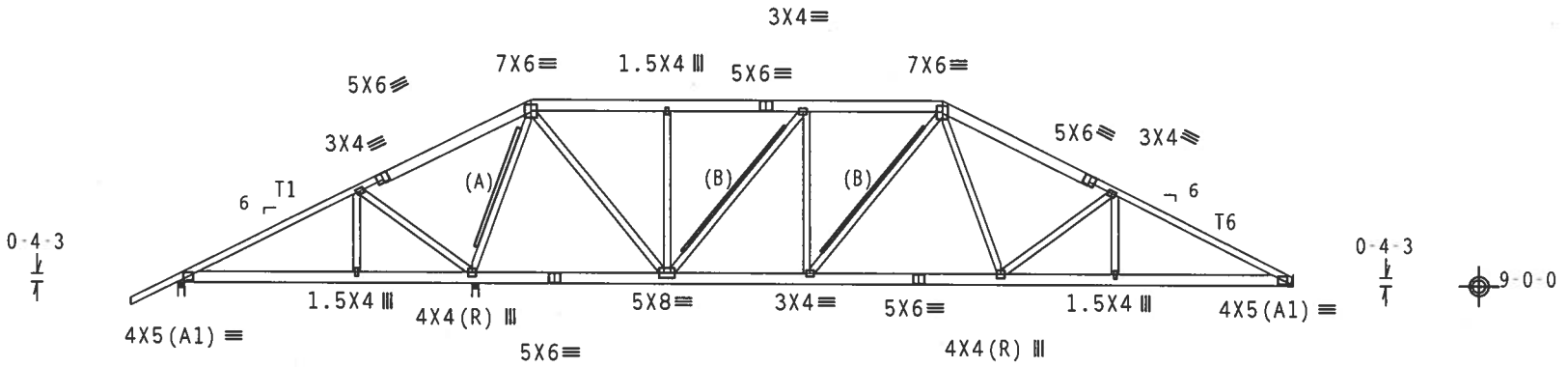
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

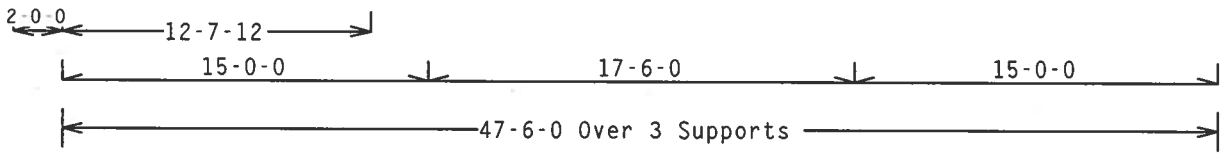
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(B) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(A) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=331 U=180 W=3.5" R=2387 U=226 W=3.5"

R=1327 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
...der is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1...TY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 2...06

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53344
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080014
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	3360
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON wendell , ** A7)

Top chord 2x6 SP #1 Dense :T1, T5 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

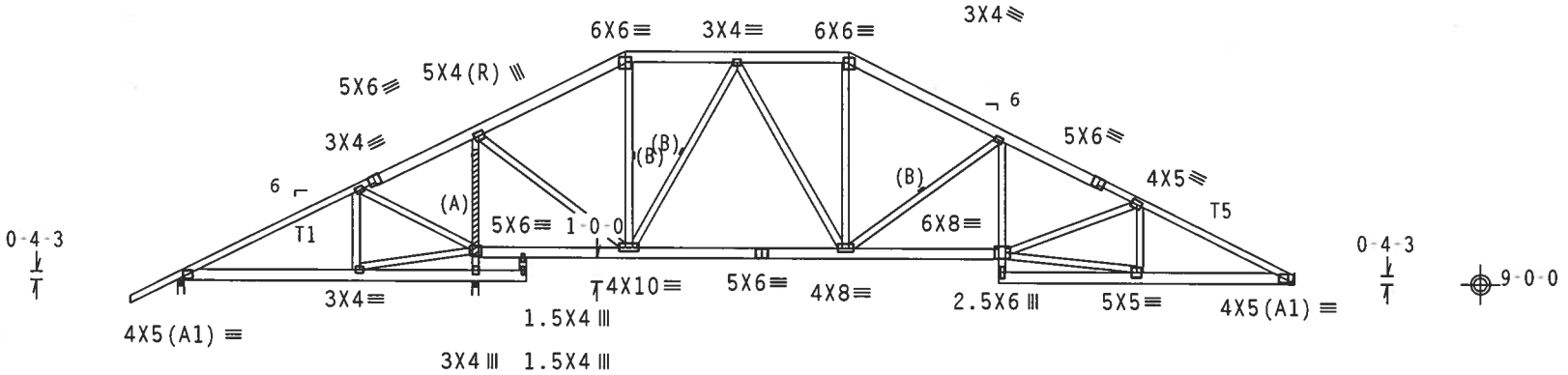
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

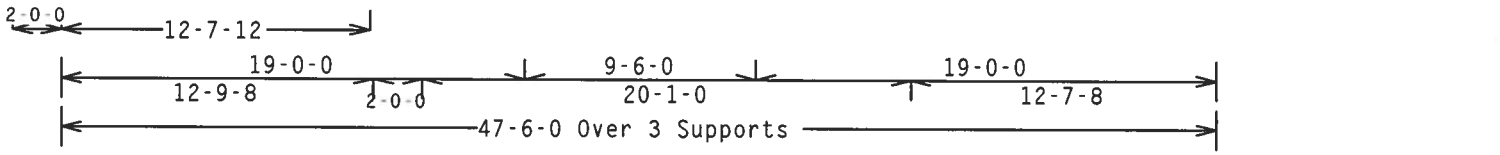
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

(B) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

SEE DWGS TCFILLER1103 AND BCFILLER1103 FOR FILLER DETAILS. LATERALLY BRACE BOTTOM CHORD ABOVE FILLER AT 24" O.C,AND TOP CHORD UNDER FILLER AT 24" OC INCLUDING A LATERAL BRACE AT CHORD ENDS.





R=398 U=180 W=3.5" R=2307 U=207 W=3.5"

R=1341 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
...der is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.12 ...TY:3 FL/-/4/-/-/R/- Scale =.125"/Ft.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53346
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080016
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	3414
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON WENDELL -- , ** - A8)

Top chord 2x6 SP #1 Dense :T1, T5 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

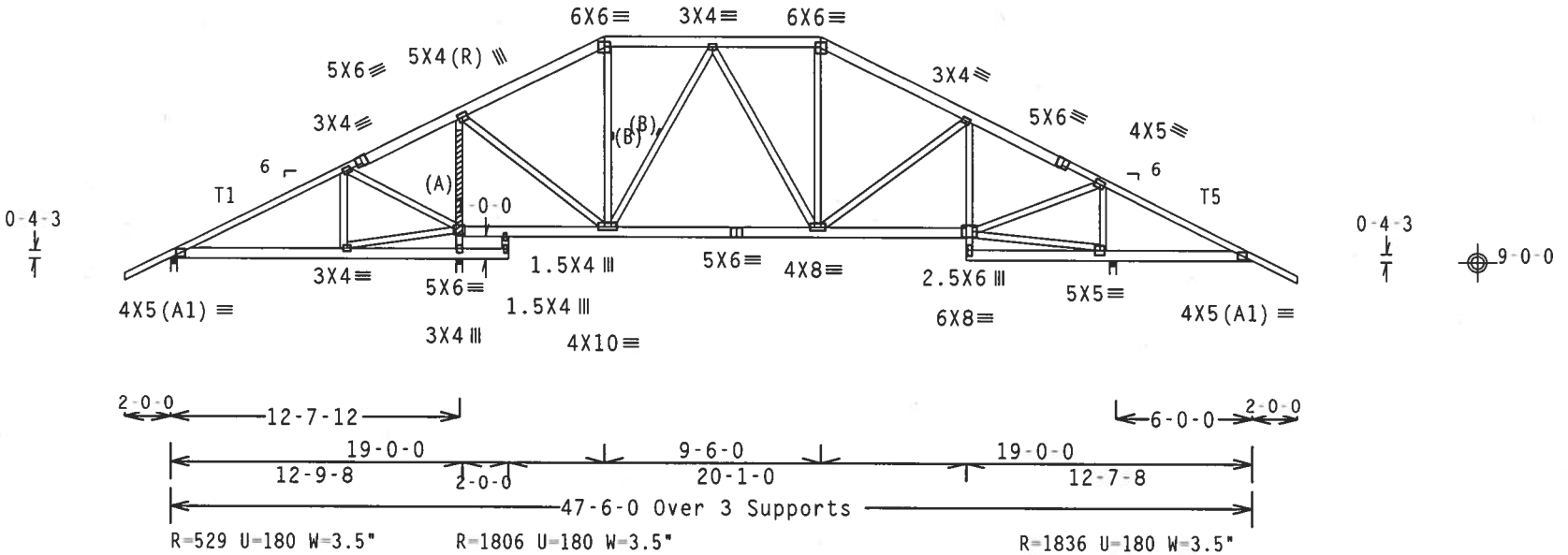
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SEE DWGS TCFILLER1103 AND BCFILLER1103 FOR FILLER DETAILS. LATERALLY BRACE BOTTOM CHORD ABOVE FILLER AT 24" O.C,AND TOP CHORD UNDER FILLER AT 24" OC INCLUDING A LATERAL BRACE AT CHORD ENDS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(B) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:2 FL/-/4/-/-/R/-

Scale =.125"/Ft.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 21 '06

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53347
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080017
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3408
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6 112--GARY JOHNSON WENDELL , ** - A9)

Top chord 2x6 SP #1 Dense :T1, T5 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

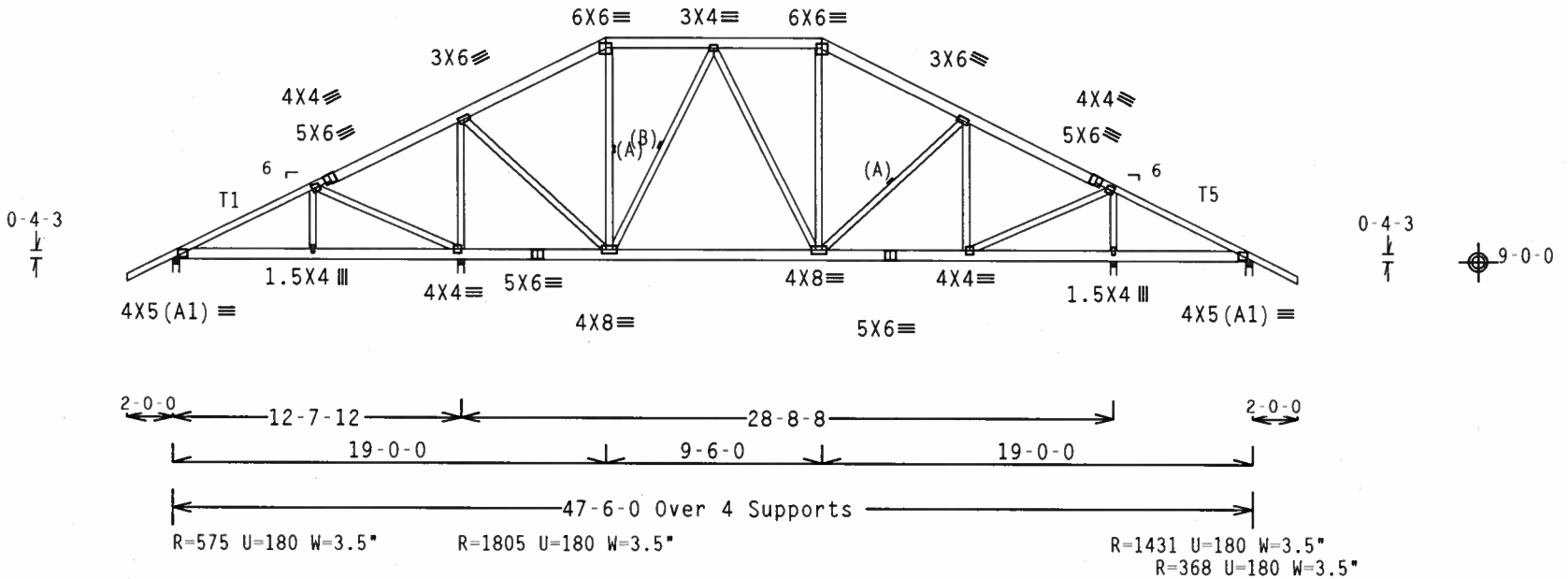
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(B) Continuous lateral bracing equally spaced on member. Or 2x4 SP #3 or better "T" brace. 80% length of web member. Attached with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF R487-- 53348
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080018
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 3425
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(6-112--GARY JOHNSON Wendell - - , ** - A10)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense :B3, B4, B5 2x4 SP #2 Dense:
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

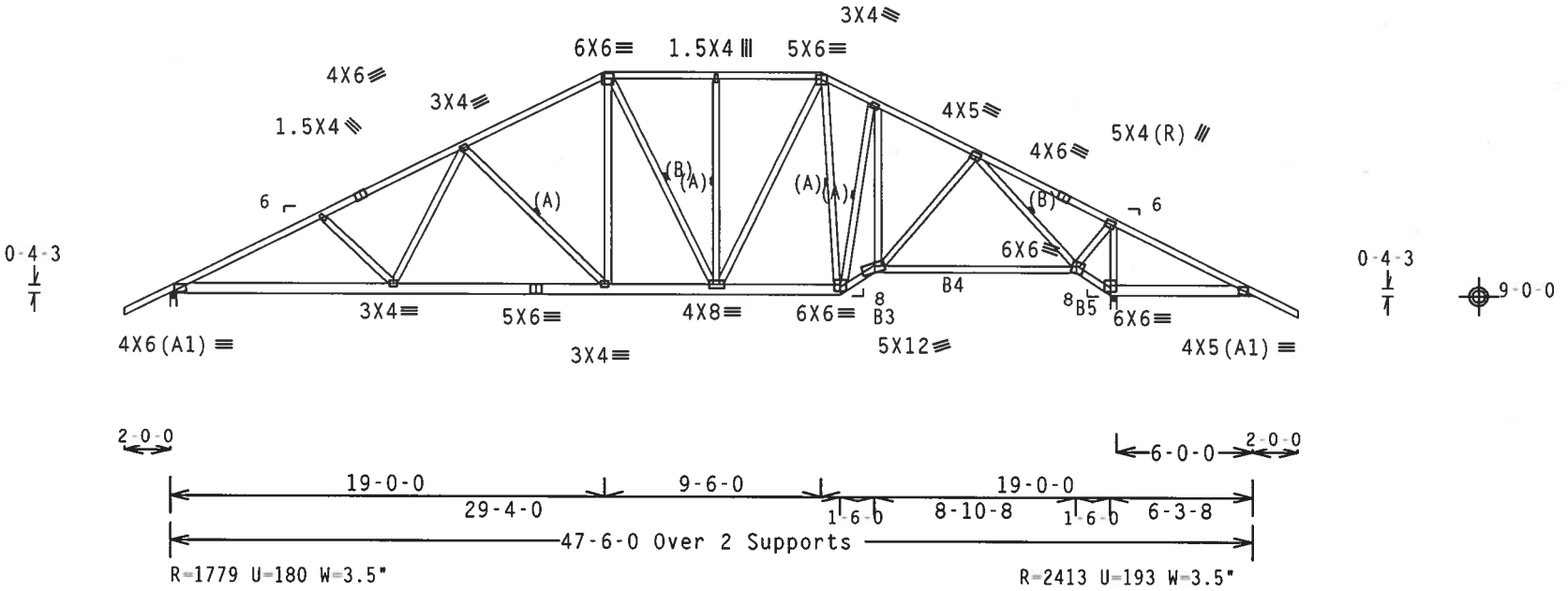
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(B) Continuous lateral bracing equally spaced on member. Or 2x4 SP #3 or better "T" brace. 80% length of web member. Attached with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:6 FL/-/4/-/-/R/- Scale =.125"/Ft.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 21 '06

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0	PSF	REF R487-- 53349
TC DL	10.0	PSF	DATE 03/21/06
BC DL	10.0	PSF	DRW HCUSR487 06080019
BC LL	0.0	PSF	HC-ENG JB/AF
TOT.LD.	40.0	PSF	SEQN- 3437
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1SV0487_Z01

(6-112--GARY JOHNSON wendell -- , ** - B1)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3 :W11 2x4 SP #2 Dense:

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

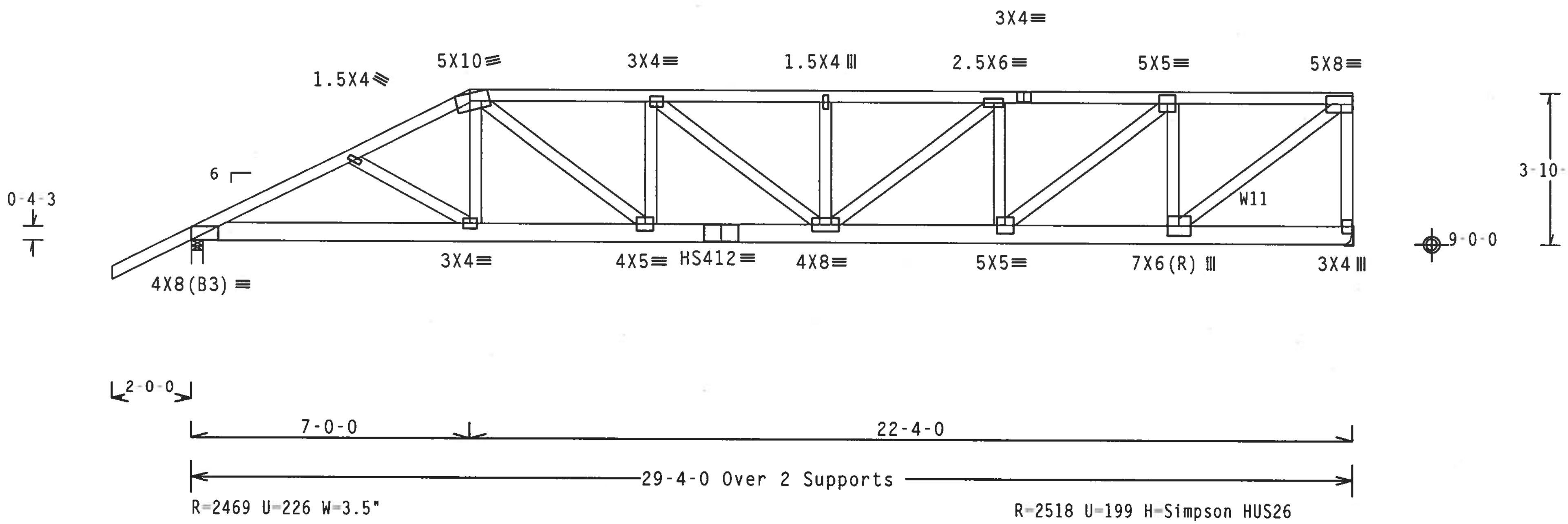
#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.



R=2518 U=199 H=Simpson HUS26
w/ (6) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

PLT TYP. 20 Gauge HS,Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/S/K) ASTM A653 GRADE 40/60 (W. K/H.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 21 '06

TC LL	20.0 PSF	REF R487-- 53350
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080020
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 3625
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - 82)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

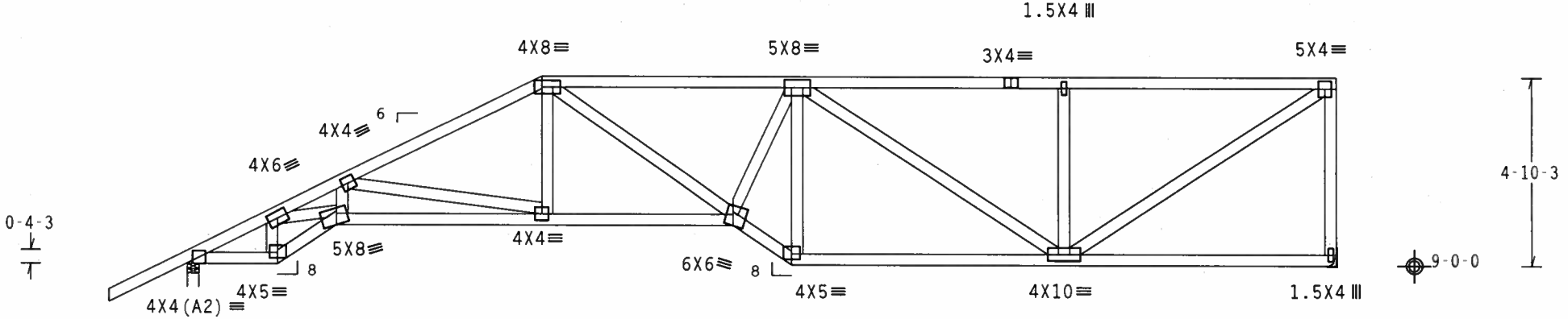
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

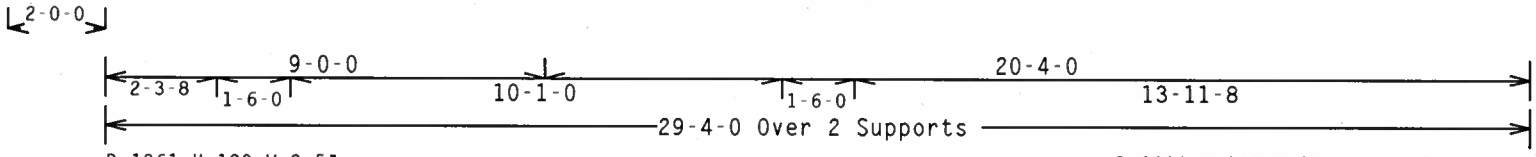
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.





R=1361 U=180 W=3.5"

R=1200 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.12 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF R487-- 53351
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080021
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 3636
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - 83)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

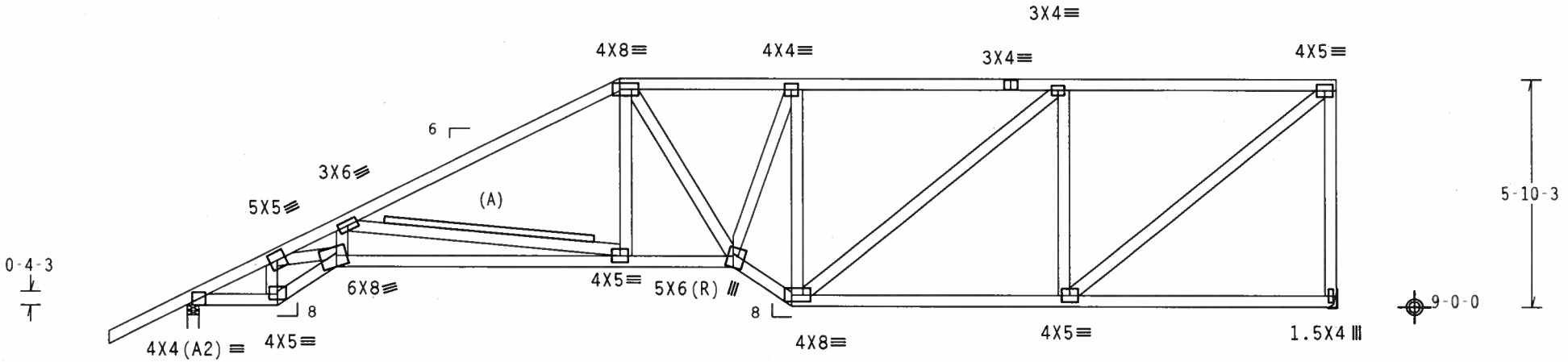
(A) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

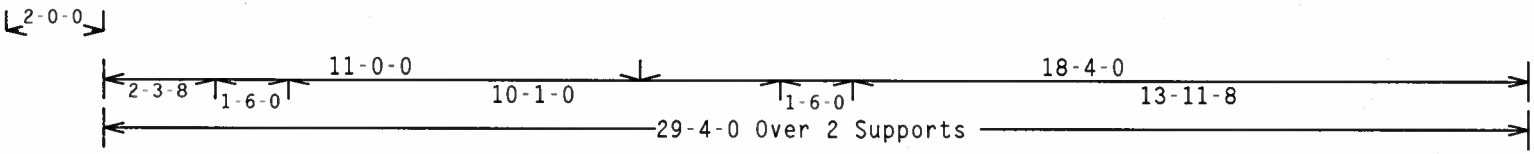
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=1361 U=180 W=3.5"

R=1200 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/-

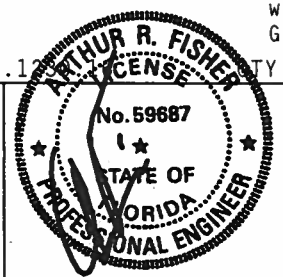
PLT TYP. Wave

Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53352
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080022
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3645
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - 84)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

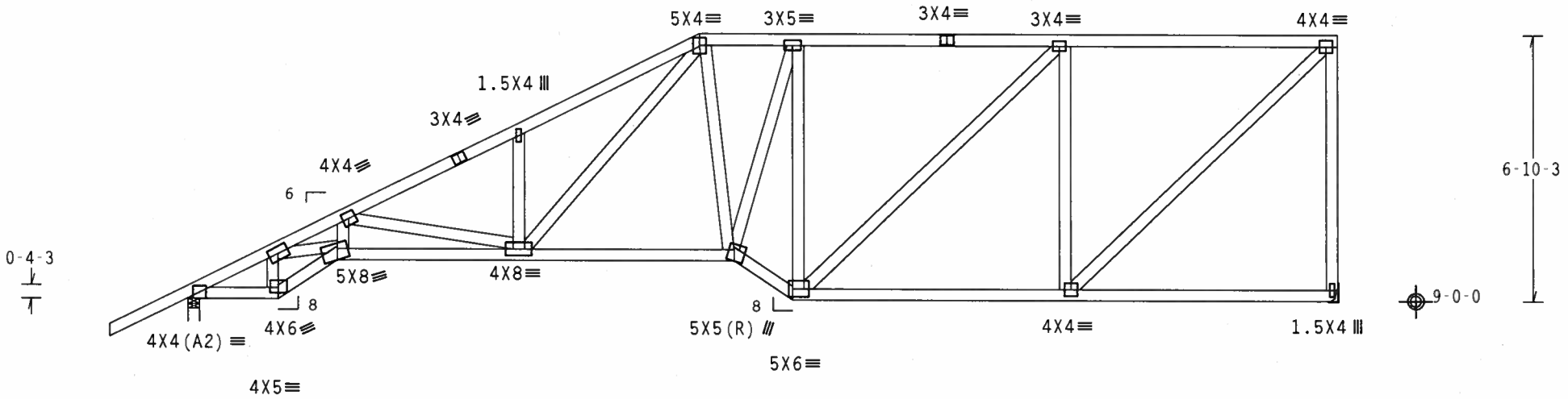
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

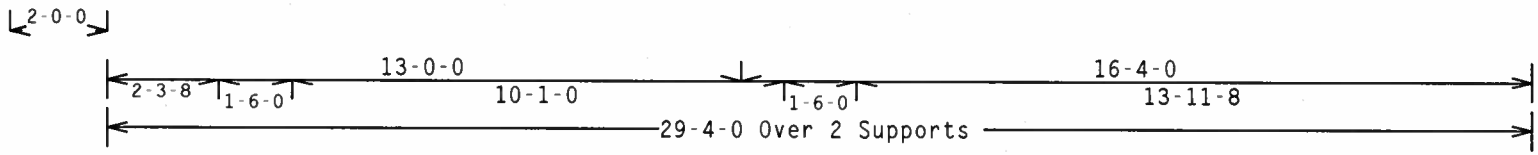
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.





R=1361 U=180 W=3.5"

R=1200 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
/ (14) 10d Common, 0.148"x3.0" nails in Girder
er is (1)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 [illegible] Y:1 FL/-/4/-/-/R/-

Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF R487-- 53353
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080023
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 3654
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(o 112--GARY JOHNSON Wendell -- , ** 65)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

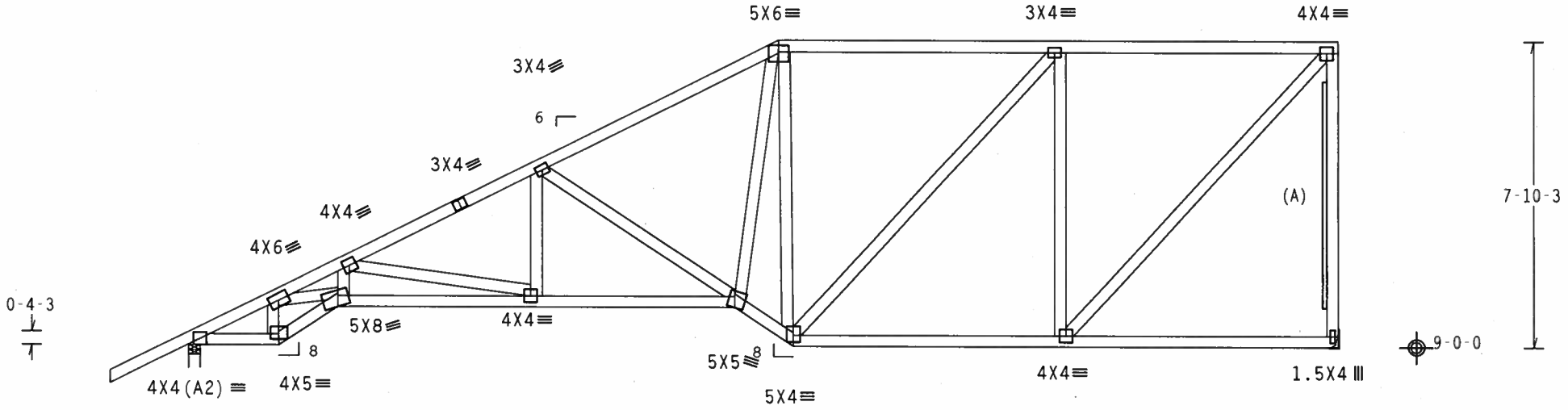
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

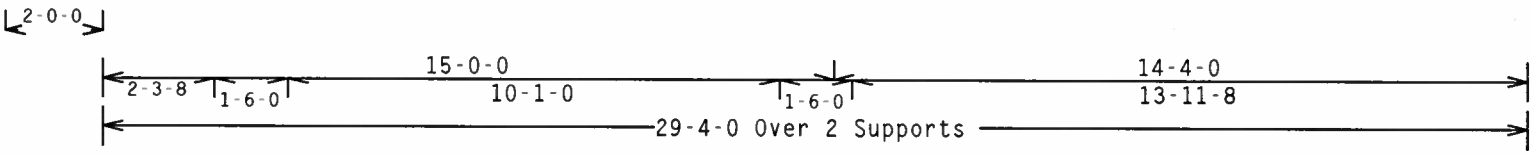
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=1361 U=180 W=3.5"

R=1200 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
...der is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1... ...QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar 21 06

TC LL	20.0 PSF	REF R487-- 53354
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080024
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 3664
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(6-112--GARY JOHNSON wendell -- , ** - B6)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(B) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

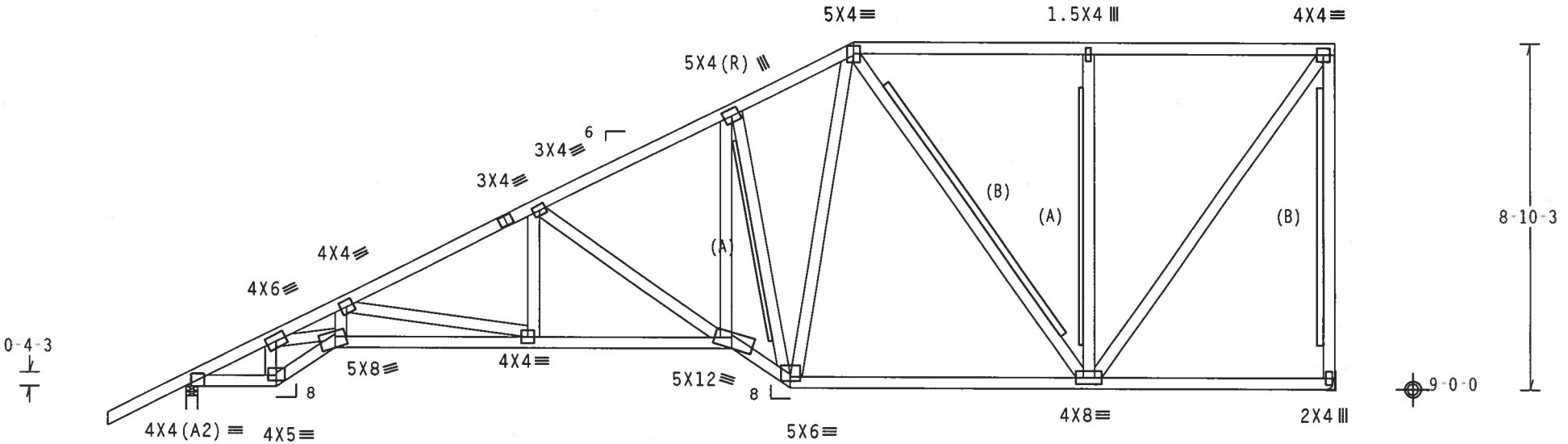
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

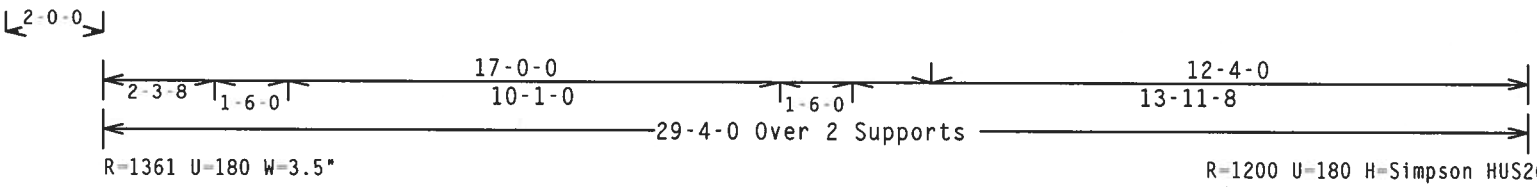
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.





R=1361 U=180 W=3.5"

R=1200 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53355
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080025
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	3678
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(o 112 - GARY JOHNSON Wendell - - , ** - 87)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

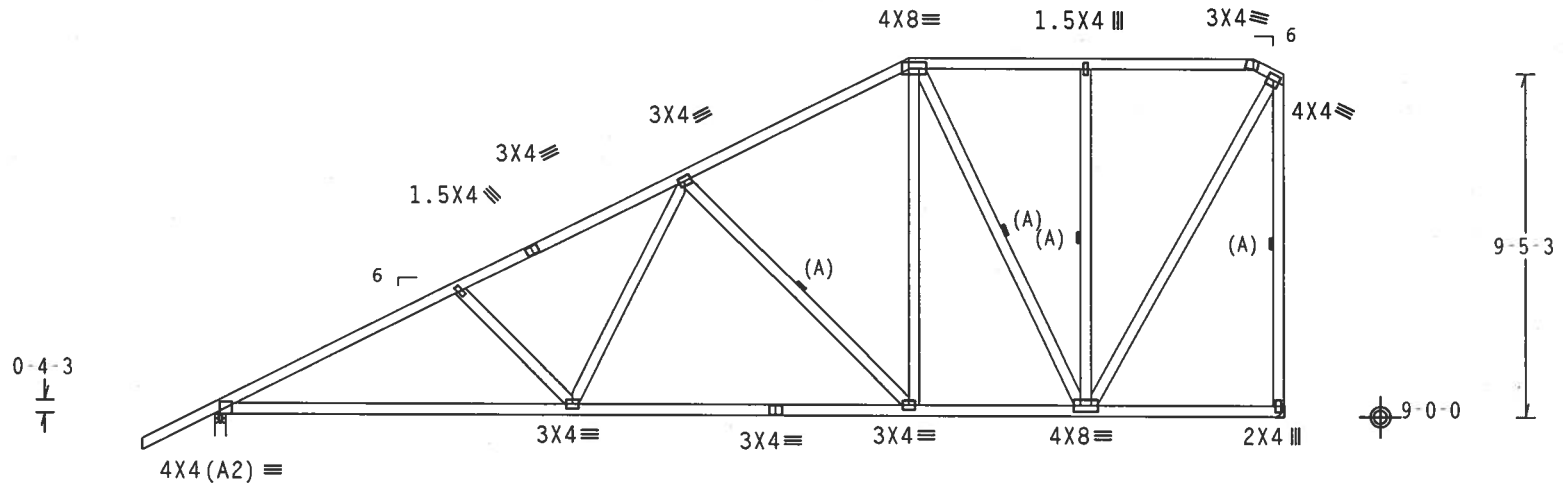
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

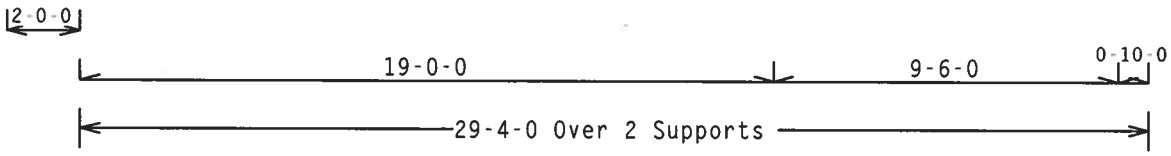
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=1353 U=180 W=3.5"

R=1197 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
[illegible] is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 [illegible] QTY:2 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 21 '06

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53356
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080026
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3687
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - C1-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	62 PLF	at	-2.00	to	62 PLF	at 5.69
TC - From	92 PLF	at	5.69	to	134 PLF	at 11.00
TC - From	134 PLF	at	11.00	to	134 PLF	at 11.50
TC - From	104 PLF	at	11.50	to	104 PLF	at 24.00
BC - From	20 PLF	at	0.00	to	20 PLF	at 22.00
BC - From	4 PLF	at	22.00	to	4 PLF	at 24.00

+ MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

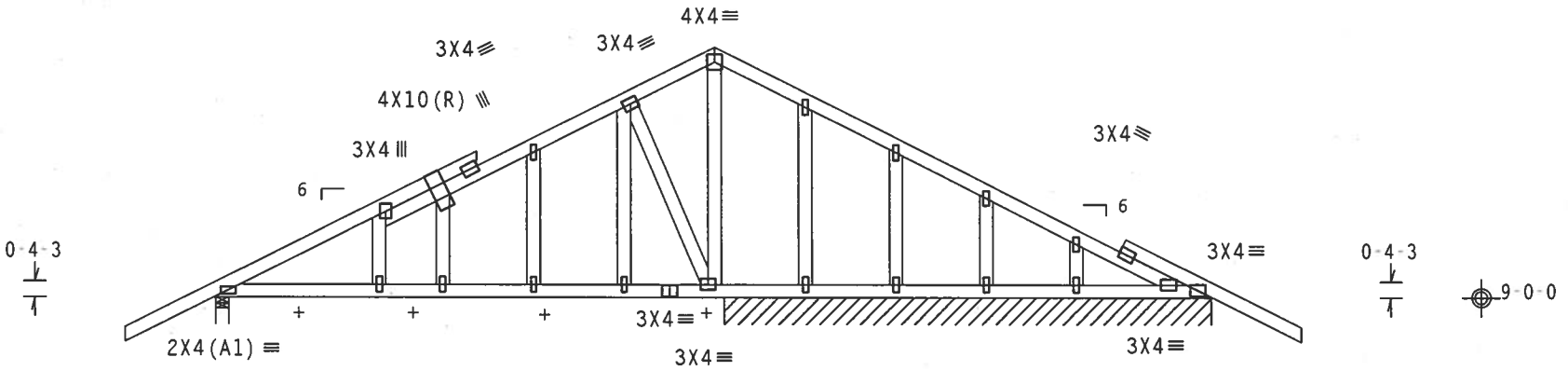
See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

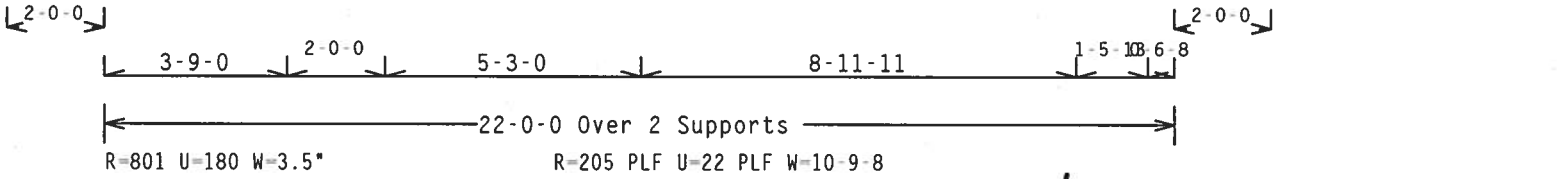
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.





Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1[illegible] QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 21 '06

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53357
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080027
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	3707 REV
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

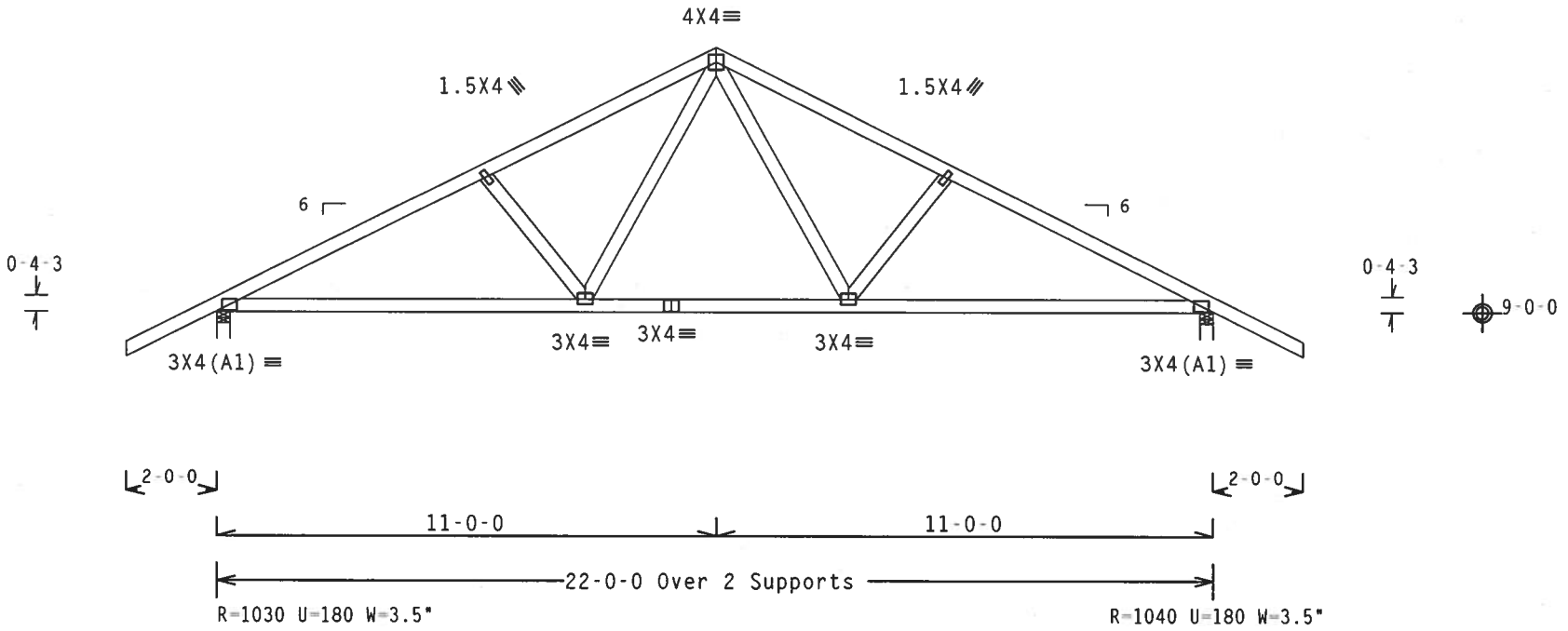
(6-112--GARY JOHNSON Wendell -- , ** - C2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:3 FL/-/4/-/-/R/- Scale =.25"/Ft.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
March 21 '06

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53358
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080001
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	3564
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

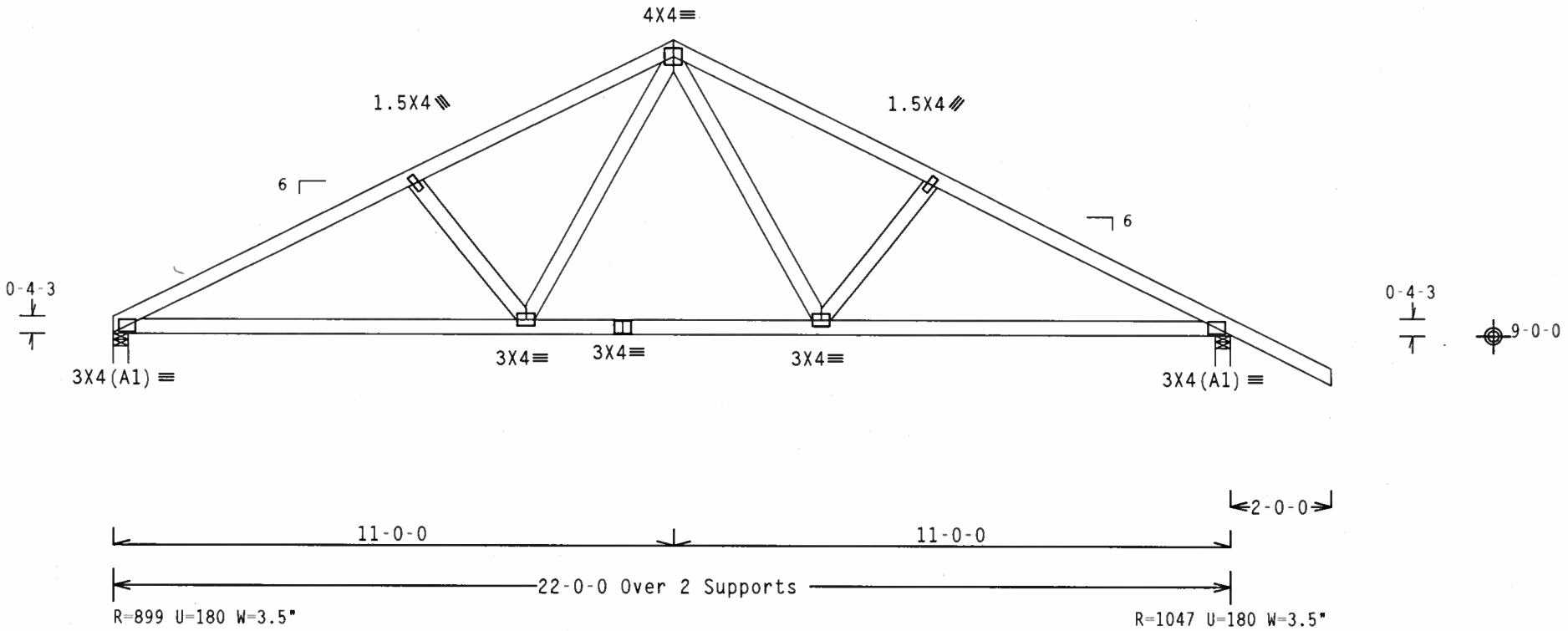
(6-112--GARY JOHNSON Wendell [illegible], **[illegible]C3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.1[illegible] QTY:4 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN. MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53359
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080002
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	3559
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - C4)

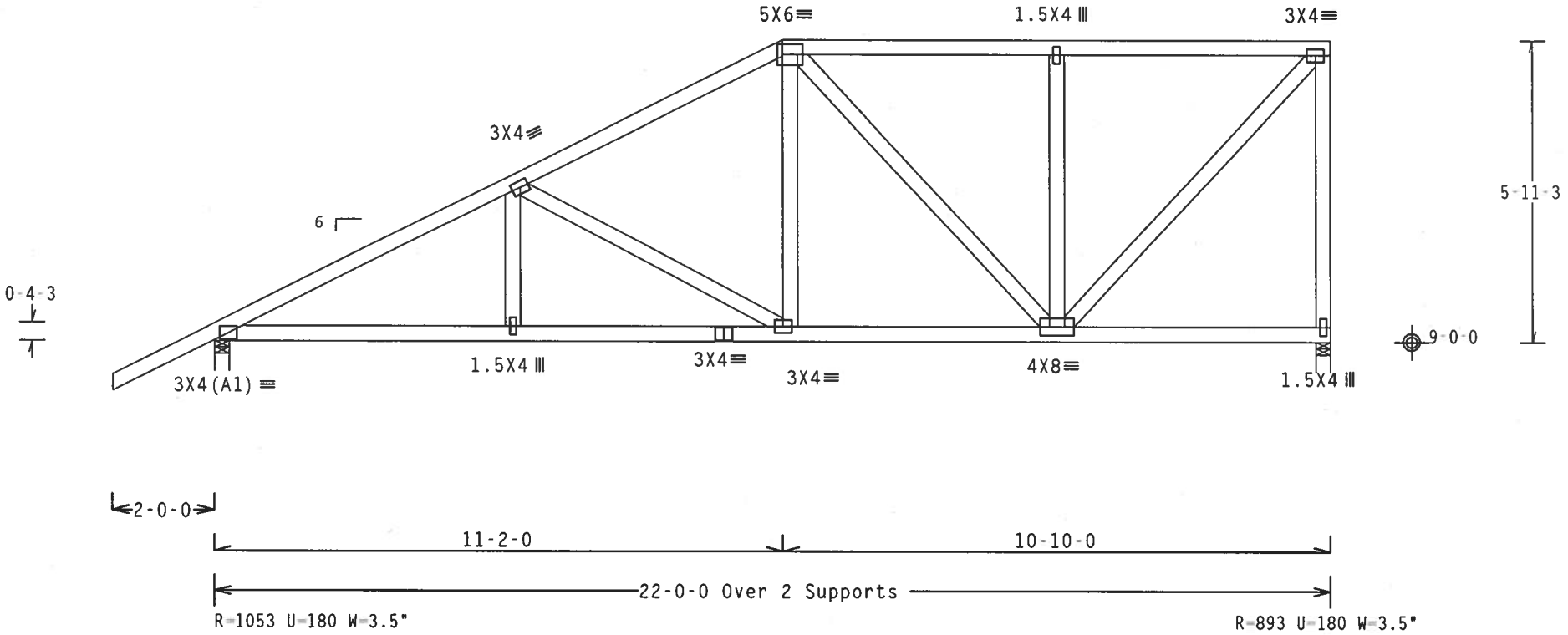
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar 21 '06

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53360
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080003
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	3588
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - C5)

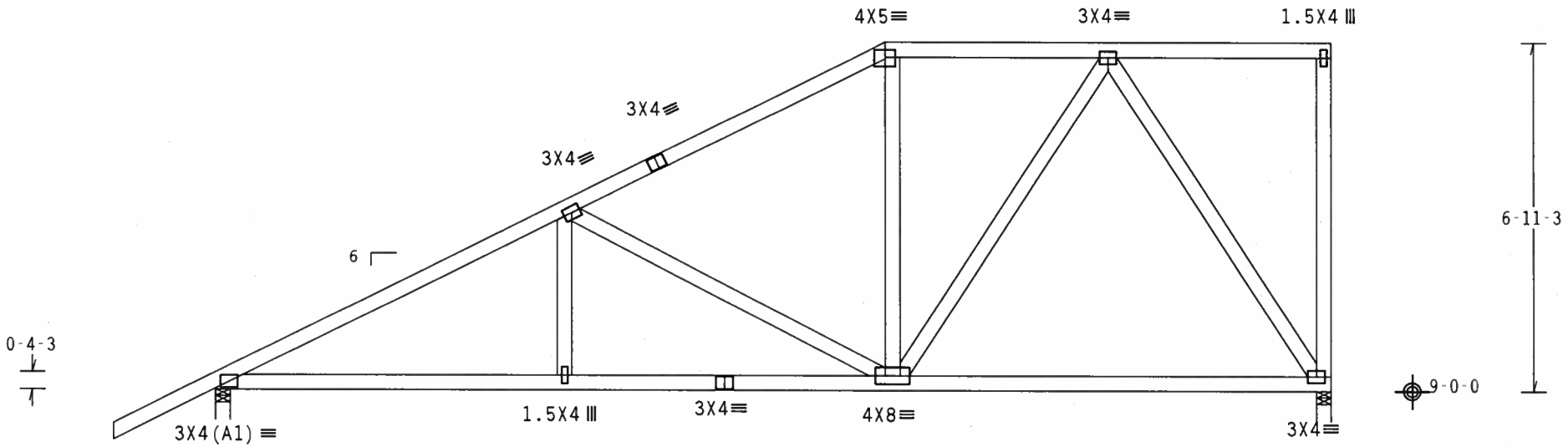
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

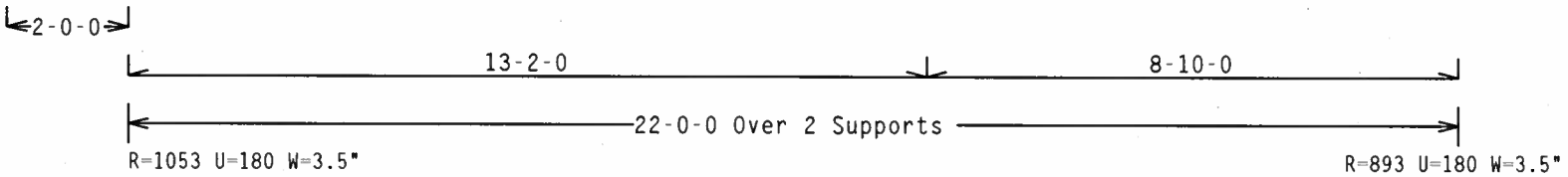
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN. MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF R487-- 53361
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080004
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 3594
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** C6)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

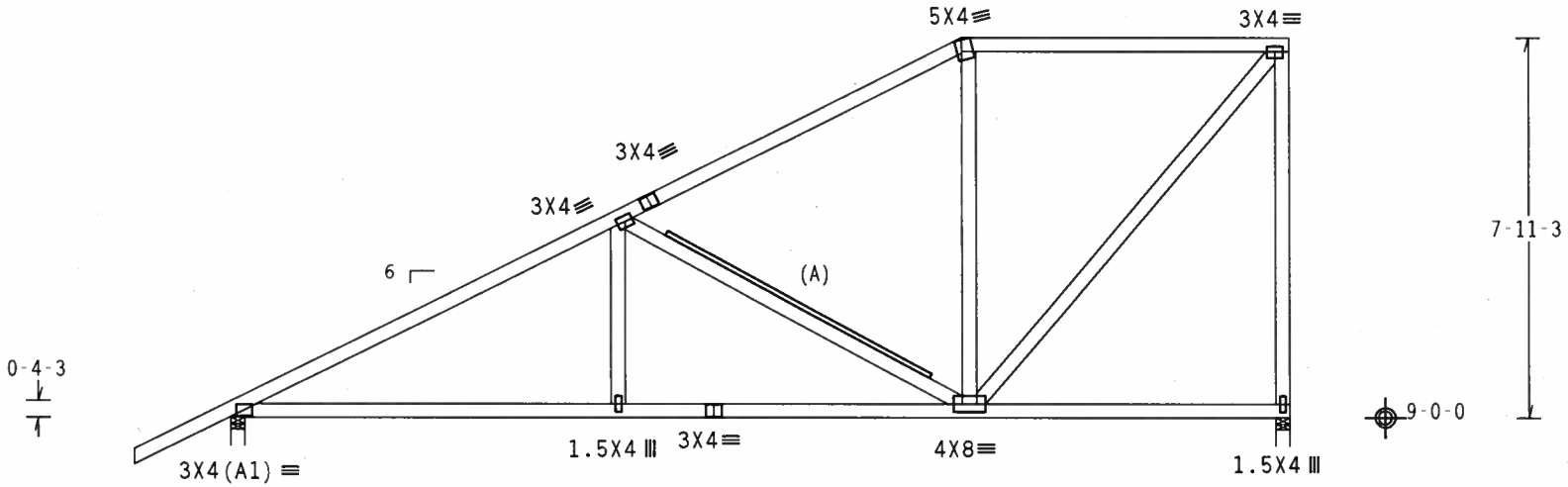
(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

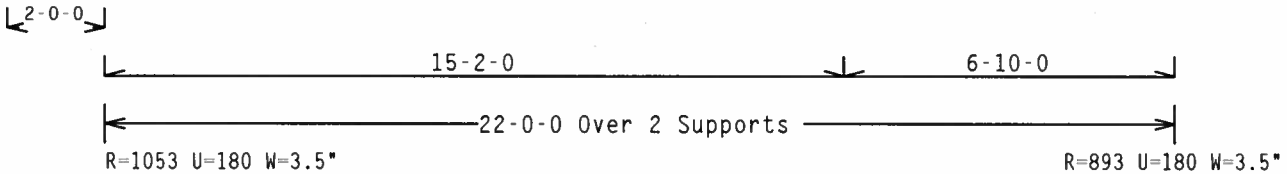
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.





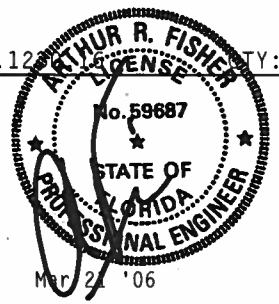
Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.12 TY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R487-- 53362
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080005
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 3602
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(6-112--GARY JOHNSON wendell--, **--C7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

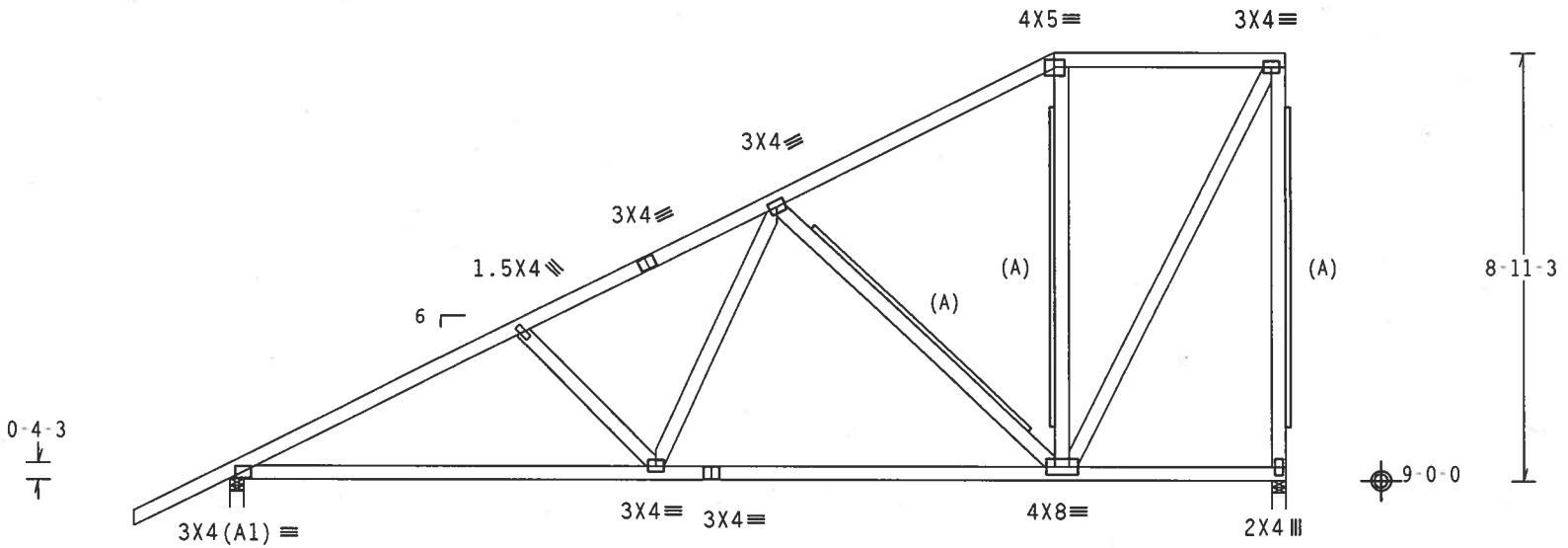
(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

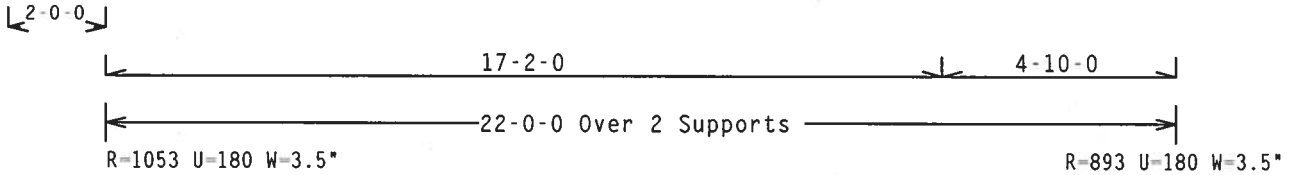
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN. MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R487-- 53363
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080006
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	3610
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell ---, ** --C8G)

Top chord 2x4 SP #2 Dense
Bot chord 2x8 SP SS
Webs 2x4 SP #3 :W6 2x4 SP #2 Dense:

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 62 PLF at 0.00 to 62 PLF at 22.00
BC - From 20 PLF at 0.00 to 20 PLF at 22.00
BC - 2518 LB Conc. Load at 7.06
BC - 1200 LB Conc. Load at 9.06, 11.06, 13.06, 15.06, 17.06
BC - 1197 LB Conc. Load at 19.06, 21.06

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (10d_Box_or_Gun_(0.128"x3",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

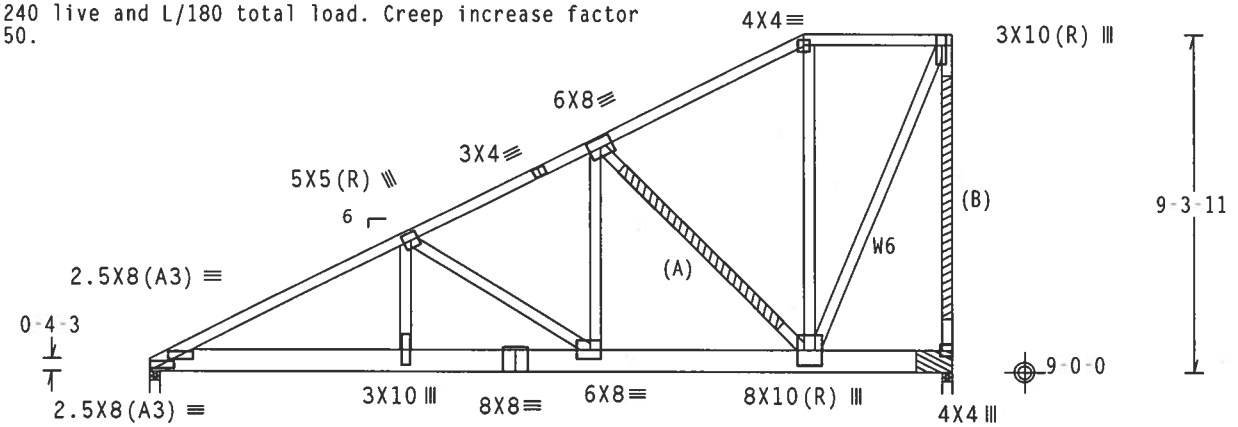
Bearing blocks: Nail type: 10d_Box_or_Gun_(0.128"x3",_min.)_nails

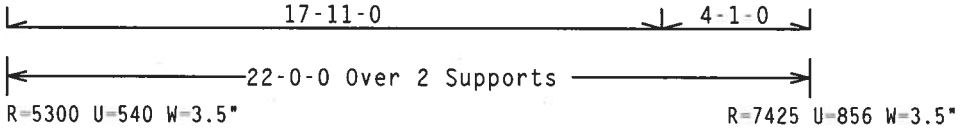
BRG	X-LOC	#BLOCKS	LENGTH/BLK	#NAILS/BLK	WALL PLATE
2	21.708'	1	12"	13	Match Truss

Bearing block to be same size and species as bottom chord.
Refer to drawing CNBRGBLK1103 for additional information.

Right end vertical not exposed to wind pressure.

(B) (2) #3 or better scab braces. Same size & 80% length of web member. Attach one to each face w/10d Box or Gun (0.128"x3",min.)nails @ 6" OC.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 22 '06

TC LL	20.0 PSF	REF	R487-- 53364
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080028
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	3697
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6 112--GARY JOHNSON Wendell -- , ** - 01-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
:Stack Chord SC2 2x4 SP #2 Dense:

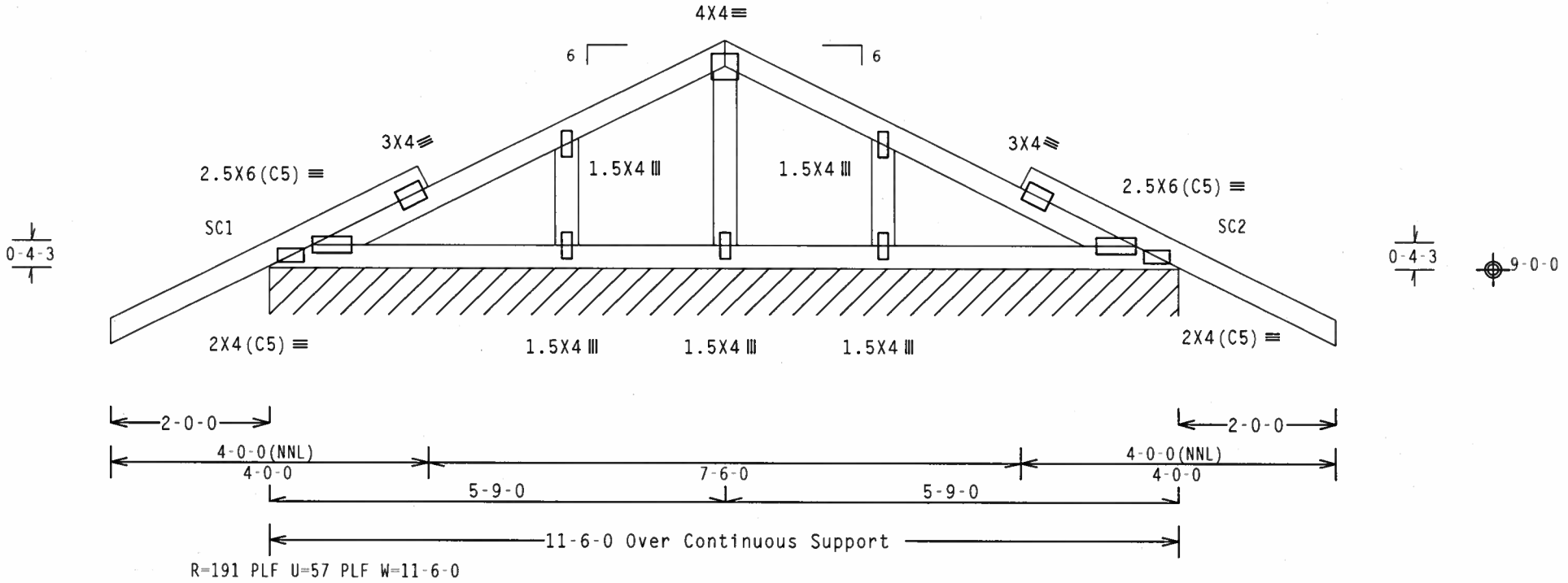
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN. MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR A. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF R487-- 53365
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080029
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 3712
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(0-11 2--GARY JOHNSON wendell -- , ** - HJ7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

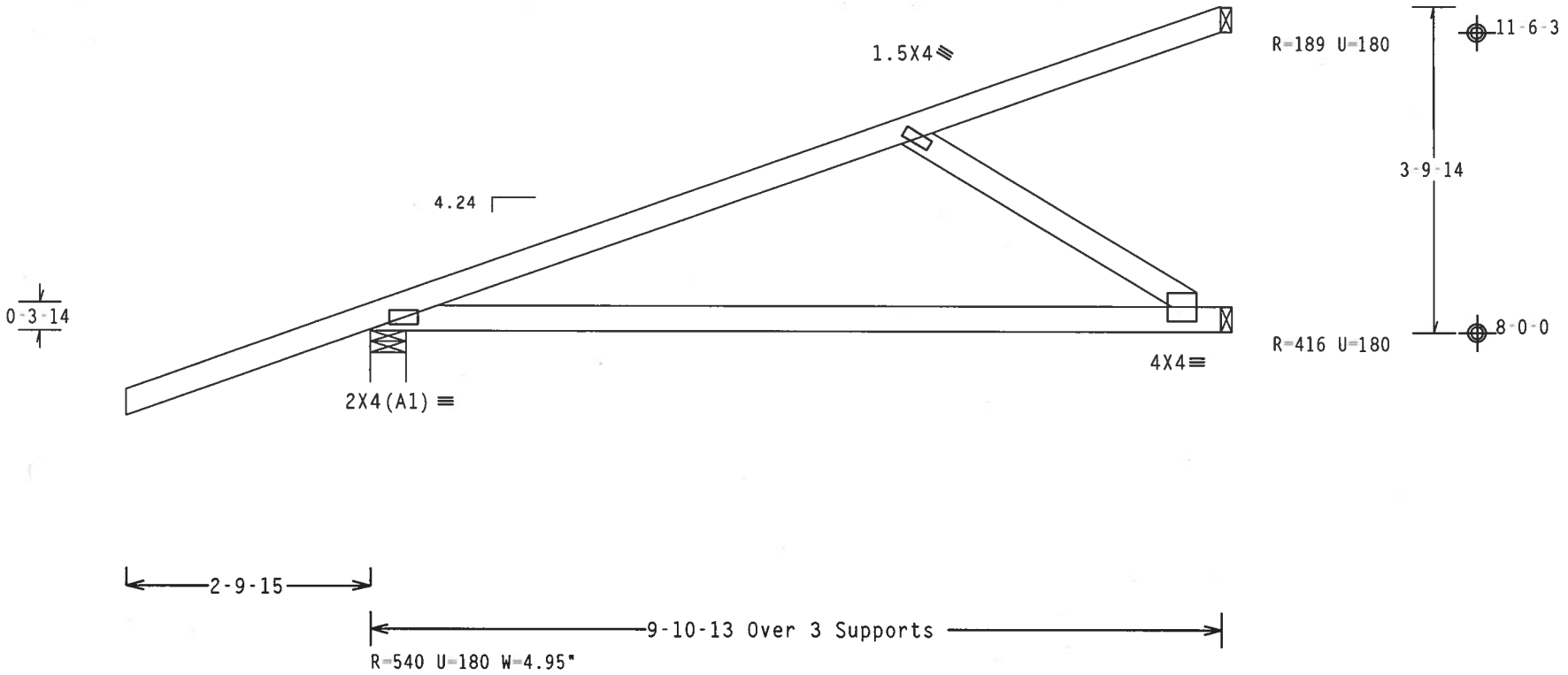
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:3 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE NO.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53366
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080030
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3569
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - EJ7)

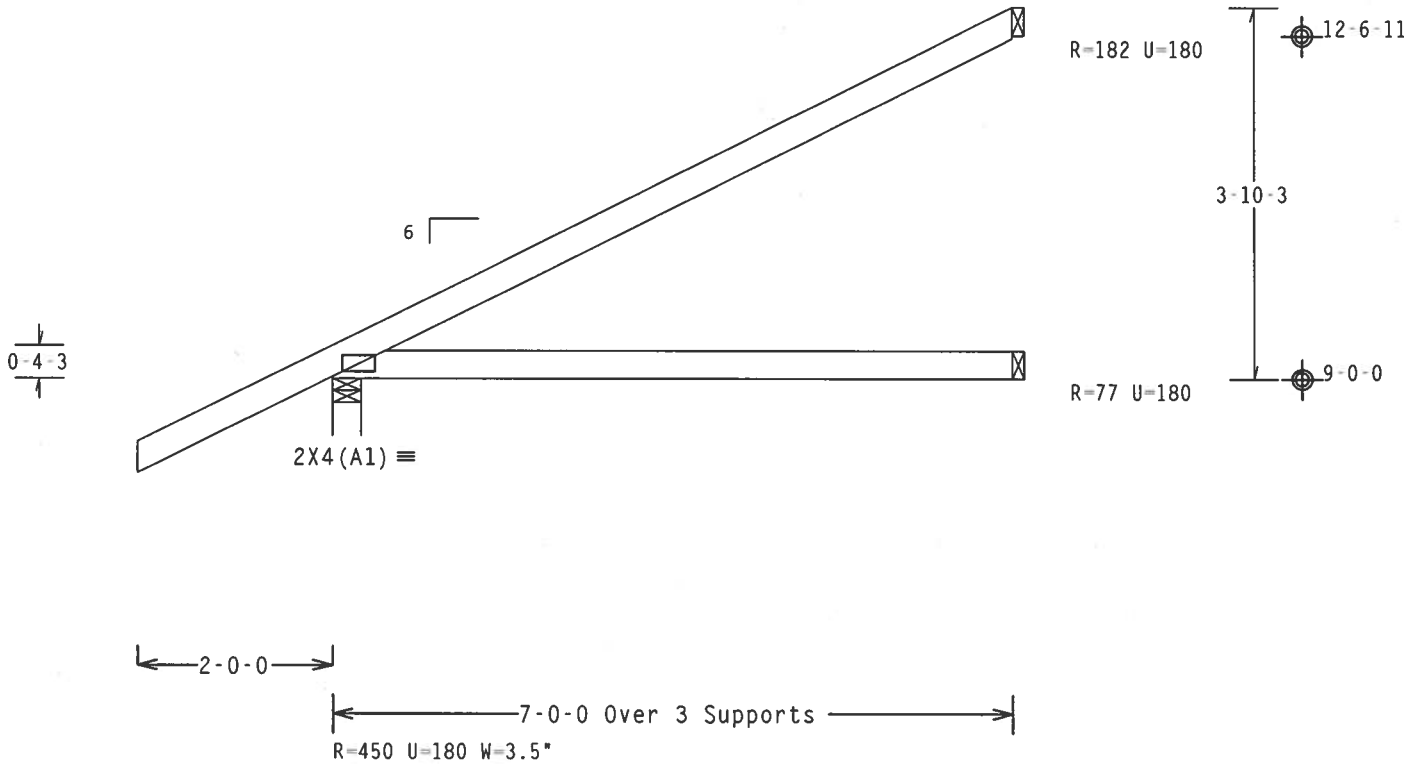
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:36 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN. MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR B. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 22 '06

TC LL	20.0 PSF	REF R487-- 53367
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080007
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 3280
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(6-112--GARY JOHNSON wendell [illegible], **-35)

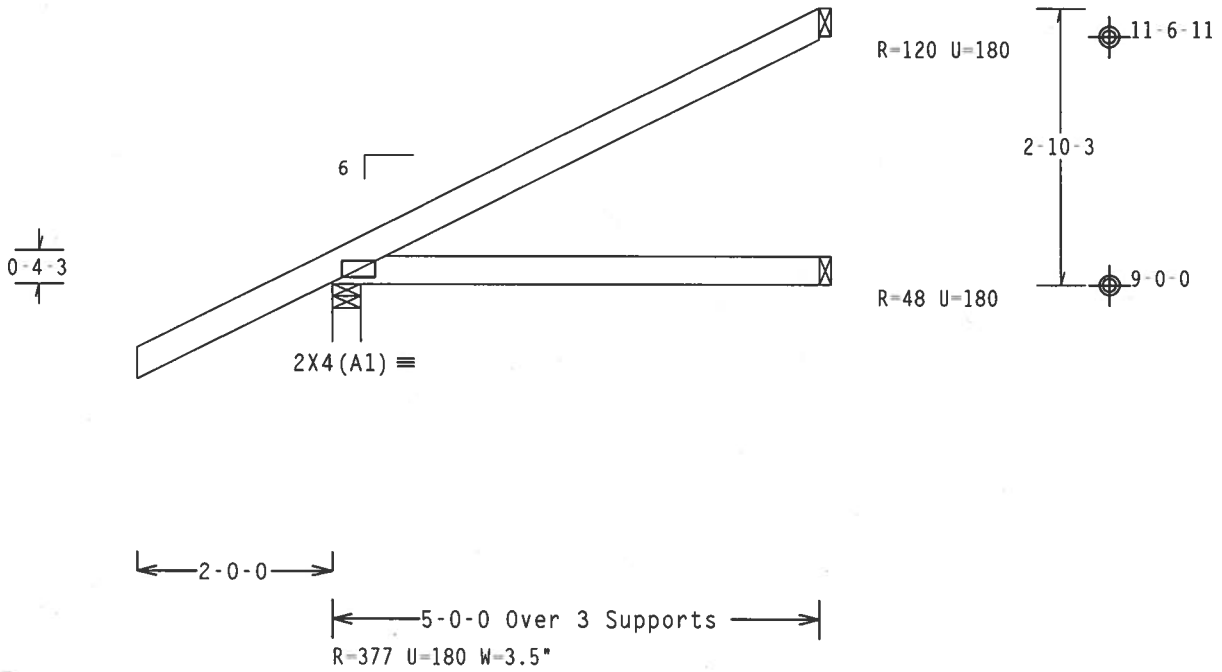
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1 [illegible] TY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53368
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080008
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	3284
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(B-112--GARY JOHNSON wendeTI -- , ** - J3)

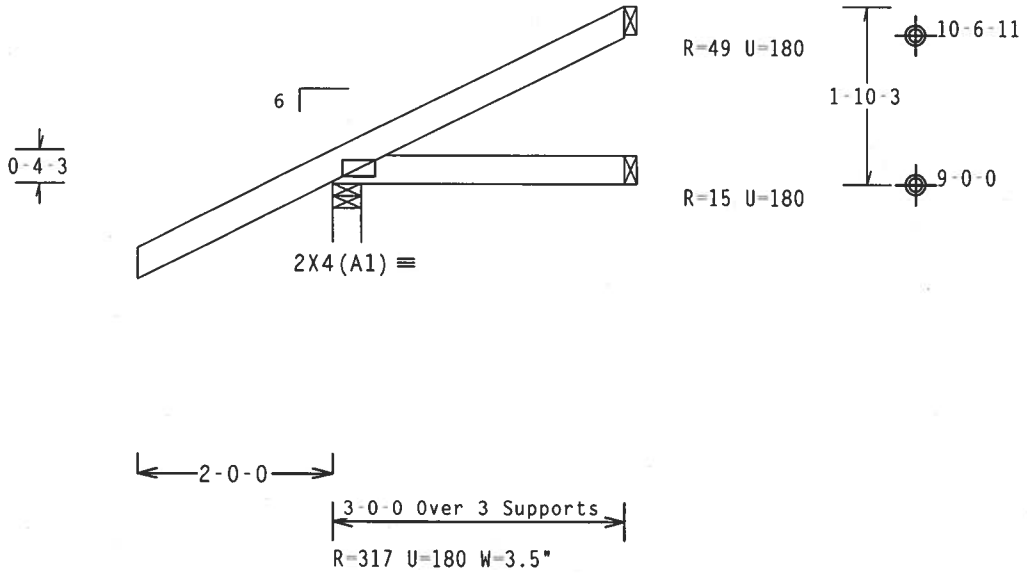
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

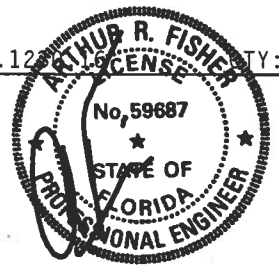


Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.12 TY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53369
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080009
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	3287
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , **- J1)

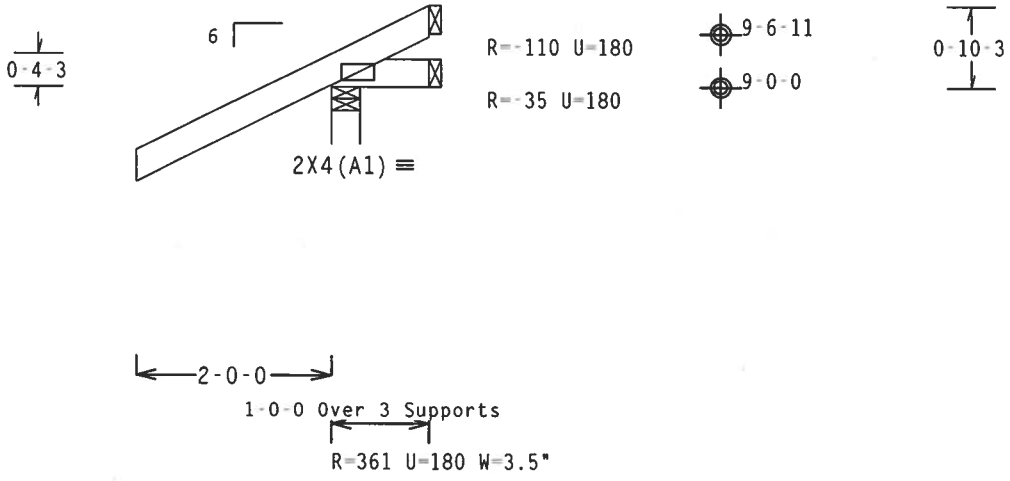
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53370
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080031
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3289
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell -- , ** - K1-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
:Stack Chord SC2 2x4 SP #2 Dense:

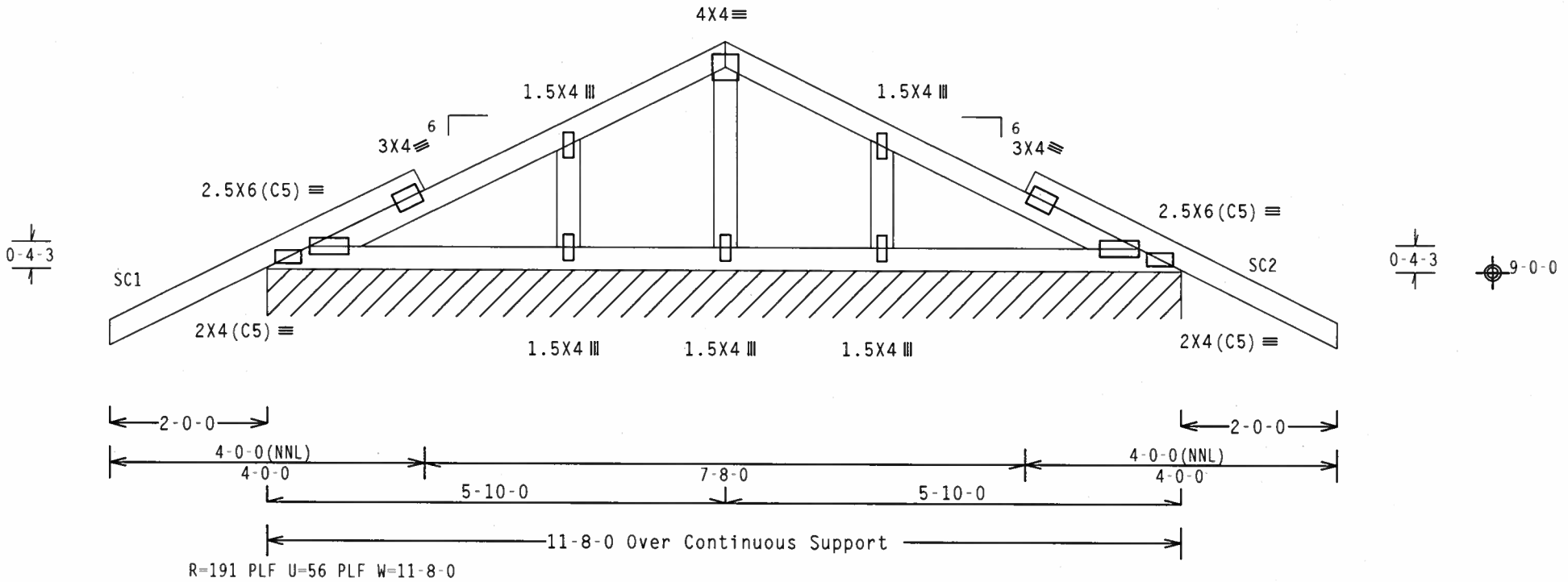
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF R487-- 53371
TC DL	10.0 PSF	DATE 03/21/06
BC DL	10.0 PSF	DRW HCUSR487 06080032
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 3717
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(0 112--GARY JOHNSON wendell , ** K2G)

Top chord 2x4 SP #2 Dense
Bot chord 2x8 SP #1 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 62 PLF at 0.00 to 62 PLF at 11.67
BC - From 20 PLF at 0.00 to 20 PLF at 11.67
BC - 1327 LB Conc. Load at 1.73
BC - 1340 LB Conc. Load at 3.73
BC - 1345 LB Conc. Load at 5.73, 7.73, 9.73

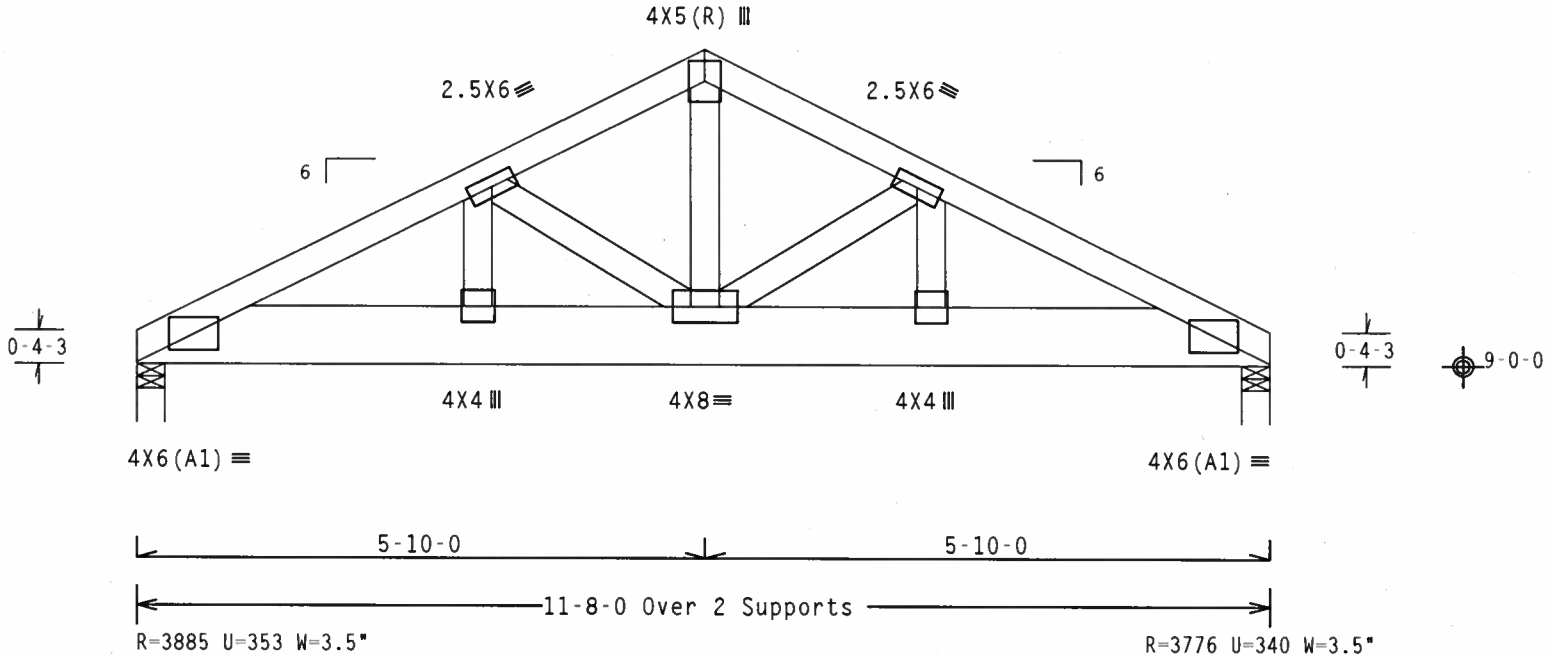
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (10d_Box_or_Gun_(0.128"x3",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 5.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.1 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No. 39687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21,06

TC LL	20.0 PSF	REF	R487-- 53372
TC DL	10.0 PSF	DATE	03/21/06
BC DL	10.0 PSF	DRW	HCUSR487 06080033
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN-	3722
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell , ** AP1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

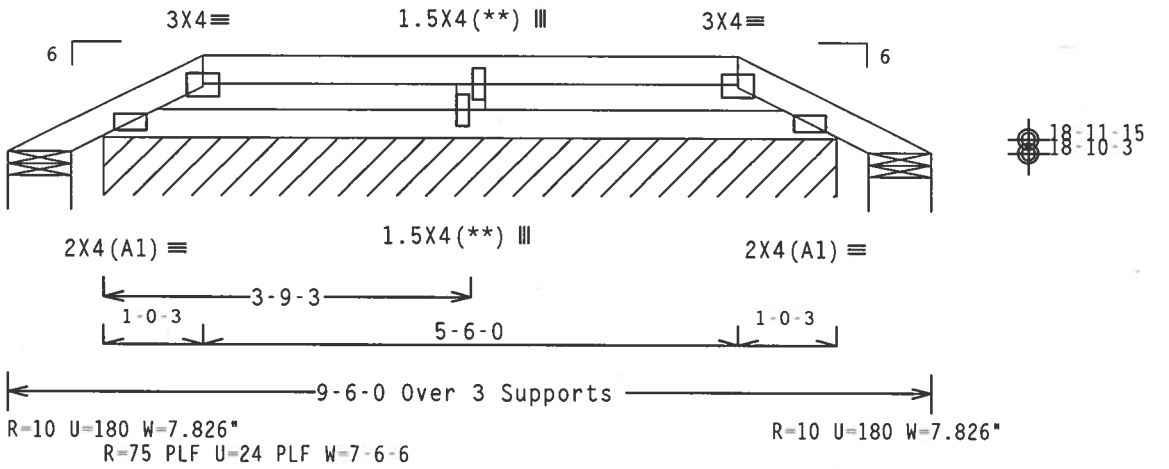
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.35 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24"O.C., UNLESS OTHERWISE SPECIFIED

(**) Plate relocated as shown.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.12 [illegible] Y:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Mar 21 06

TC LL	20.0 PSF	REF	R487-- 53373
TC DL	10.0 PSF	DATE	03/21/06
BC DL	2.0 PSF	DRW	HCUSR487 06080034
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	32.0 PSF	SEQN-	3455
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

(6-112--GARY JOHNSON Wendell , **- AP2)

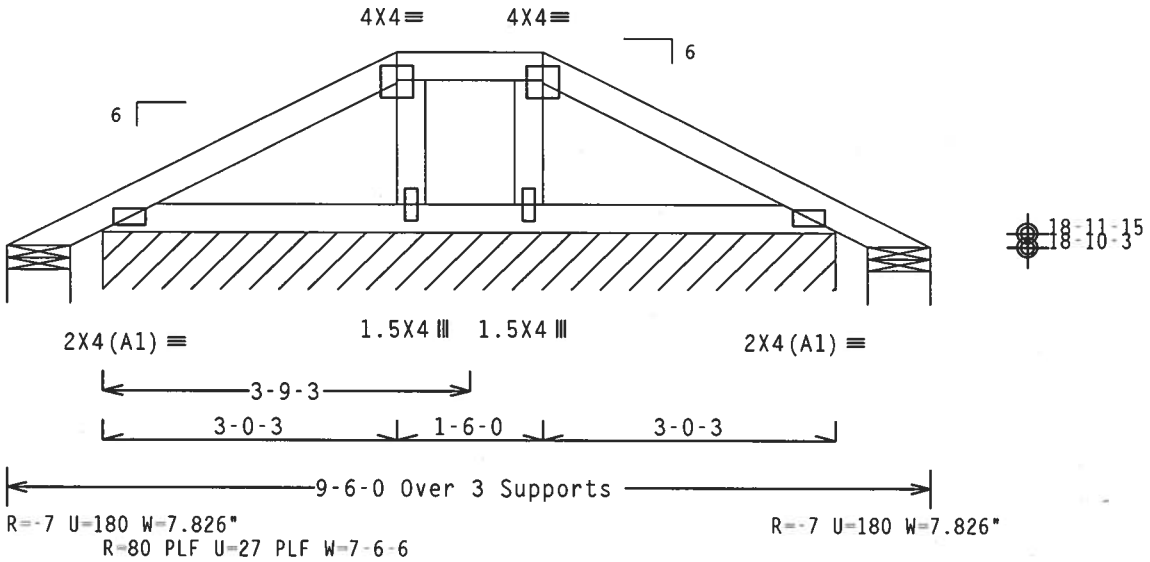
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24"O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 19.85 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.24.12 QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER
LICENSE
No.59687
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
Mar 22, 06

TC LL	20.0 PSF	REF R487-- 53374
TC DL	10.0 PSF	DATE 03/21/06
BC DL	2.0 PSF	DRW HCUSR487 06080035
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	32.0 PSF	SEQN- 3450
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1SV0487_Z01

(0 112--GARY JOHNSON Wendell , ** AP3)

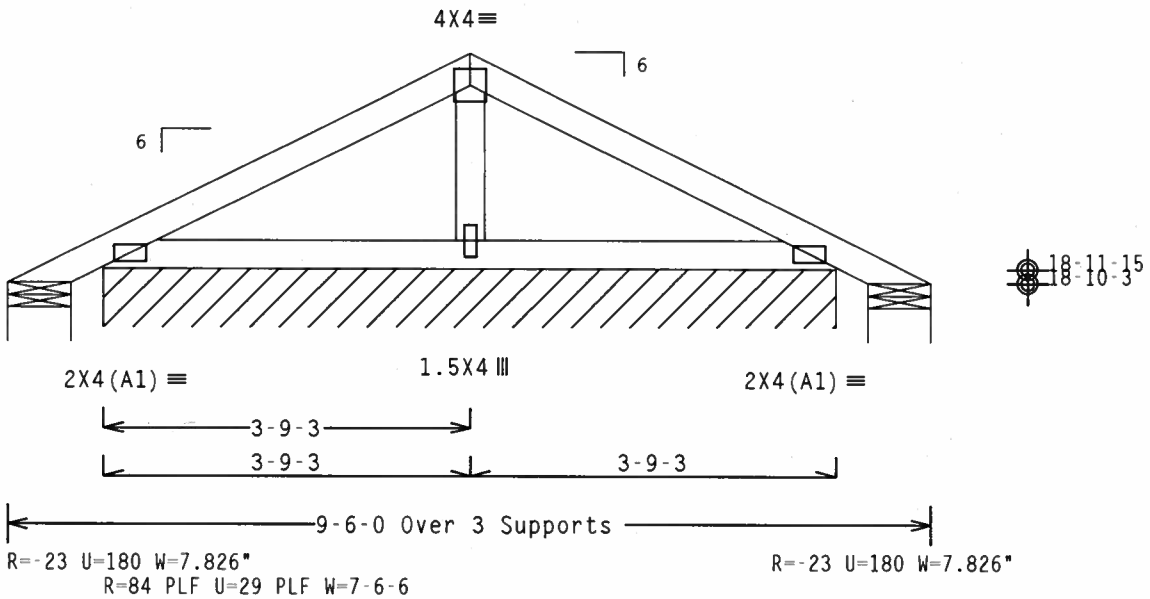
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24"O.C., UNLESS OTHERWISE SPECIFIED

110 mph wind, 20.03 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1 TY:8 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No. 59637 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar 21 '06

TC LL	20.0 PSF	REF	R487-- 53375
TC DL	10.0 PSF	DATE	03/21/06
BC DL	2.0 PSF	DRW	HCUSR487 06080036
BC LL	0.0 PSF		HC-ENG JB/AF
TOT.LD.	32.0 PSF	SEQN-	3443
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1SV0487_Z01

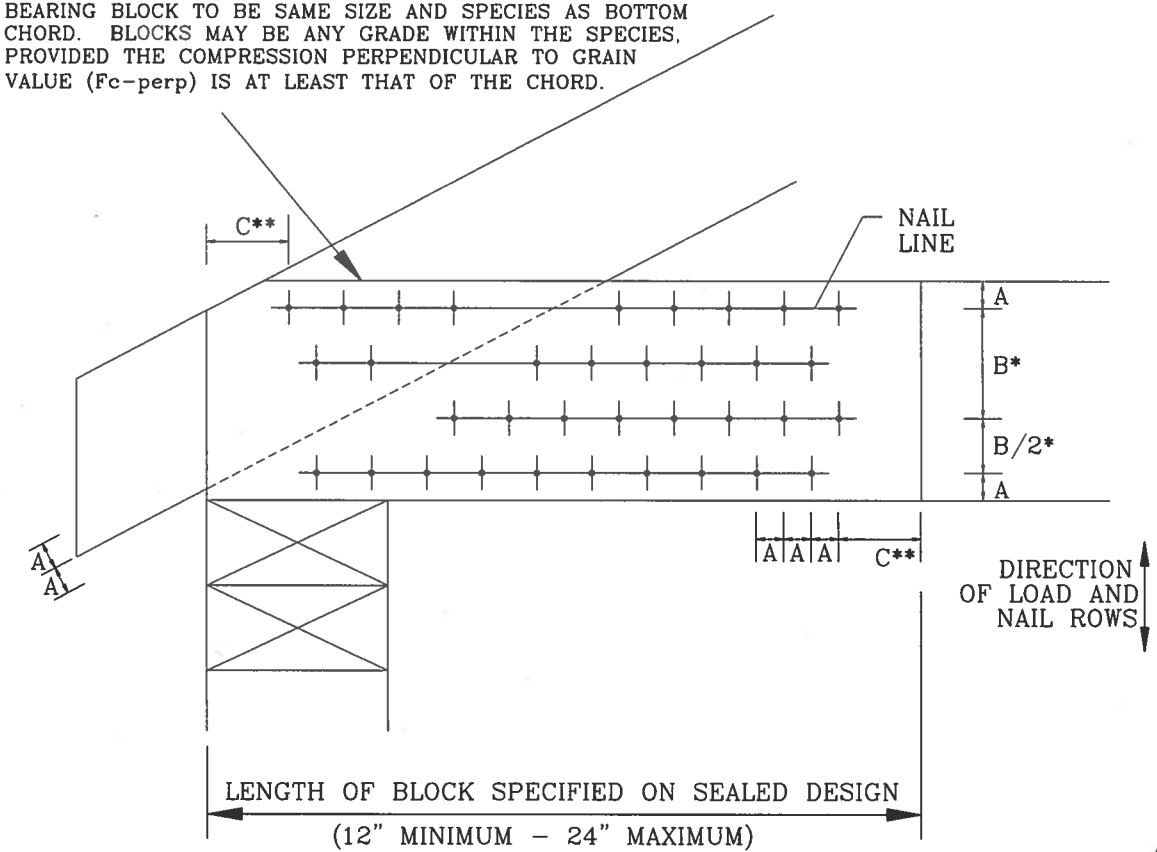
BEARING BLOCK NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

A – EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B – SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C – END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
- • SPACING MAY BE REDUCED BY 50%
- •• SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (Fc-perp) IS AT LEAST THAT OF THE CHORD.



MAXIMUM NUMBER OF NAIL LINES PARALLEL TO GRAIN

NAIL TYPE	CHORD SIZE 2X4	2X6	2X8	2X10	2X12
8d BOX (0.113"X2.5")	3	6	9	12	15
10d BOX (0.128"X3")	3	5	7	10	12
12d BOX (0.128"X3.25")	3	5	7	10	12
16d BOX (0.135"X3.5")	3	5	7	10	12
20d BOX (0.148"X4")	2	4	5	6	8
8d COMMON (0.131"X2.5")	3	5	7	10	12
10d COMMON (0.148"X3")	2	4	6	8	10
12d COMMON (0.148"X3.25")	2	4	6	8	10
16d COMMON (0.162"X3.5")	2	4	6	8	10
0.120"X2.5" GUN	3	6	8	11	14
0.131"X2.5" GUN	3	5	7	10	12
0.120"X3.0" GUN	3	6	8	11	14
0.131"x3.0" GUN	3	5	7	10	12

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**
8d BOX (0.113"X2.5")	3/4"	1 3/8"	1 3/4"
10d BOX (0.128"X3")	7/8"	1 5/8"	2"
12d BOX (0.128"X3.25")	7/8"	1 5/8"	2"
16d BOX (0.135"X3.5")	7/8"	1 5/8"	2 1/8"
20d BOX (0.148"X4")	1"	1 7/8"	2 1/4"
8d COMMON (0.131"X2.5")	7/8"	1 5/8"	2"
10d COMMON (0.148"X3")	1"	1 7/8"	2 1/4"
12d COMMON (0.148"X3.25")	1"	1 7/8"	2 1/4"
16d COMMON (0.162"X3.5")	1'	2"	2 1/2"
0.120"X2.5" GUN	3/4"	1 1/2"	1 7/8"
0.131"X2.5" GUN	7/8"	1 5/8"	2"
0.120"X3.0" GUN	3/4"	1 1/2"	1 7/8"
0.131"x3.0" GUN	7/8"	1 5/8"	2"

THIS DRAWING REPLACES DRAWING B139 AND CNBRGBLK0699

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No. 39687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Mar

REF	BEARING BLOCK
DATE	11/26/03
DRWG	CNBRGBLK1103
-ENG	SJP/KAR

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

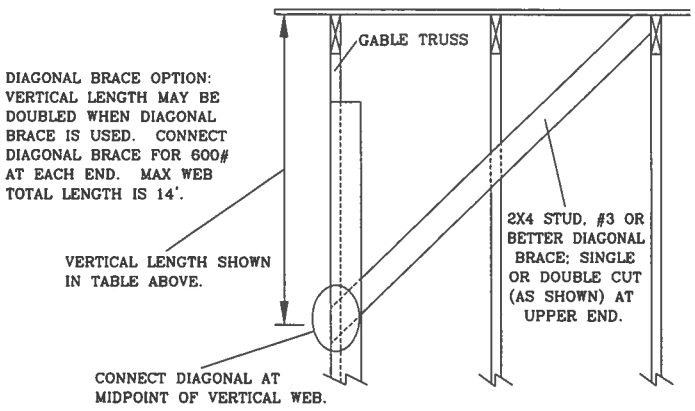
PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

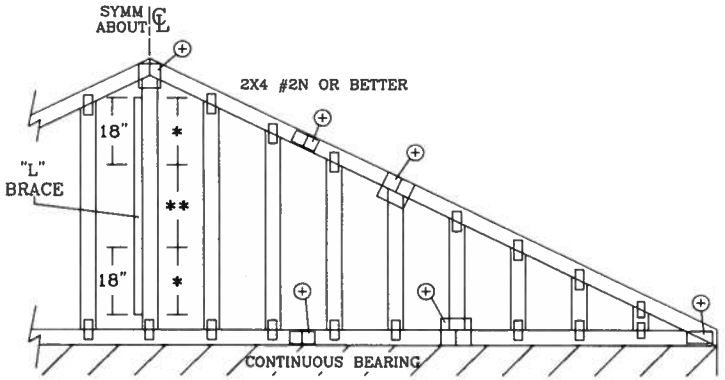
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 600# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

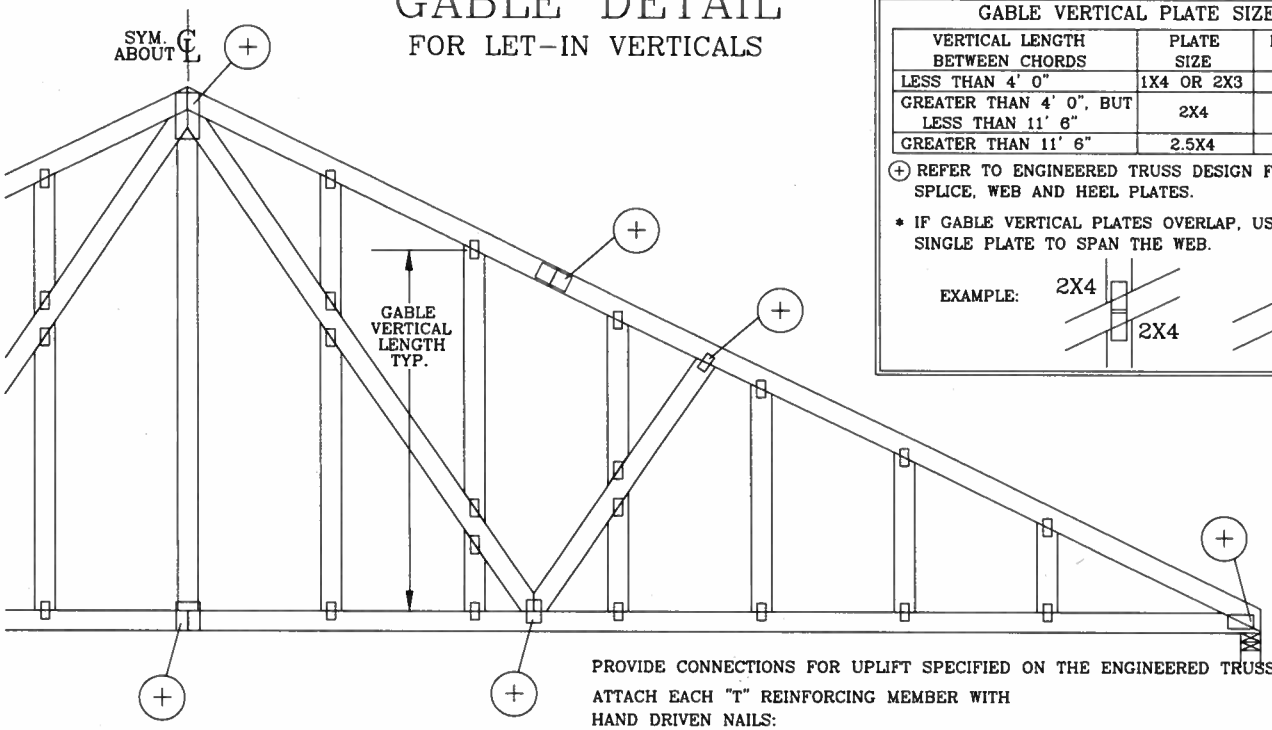
Mar 31

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	04/15/05
DRWG	A11015EE0405
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS

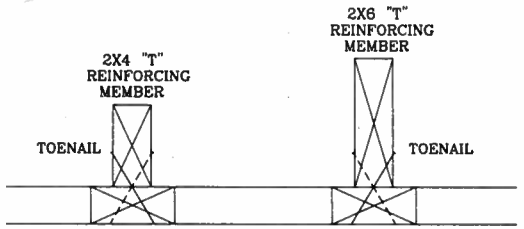


GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:



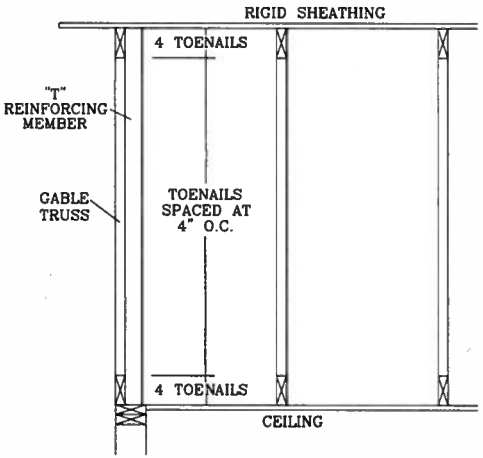
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.
GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN1103, A10015EN1103, A09015EN1103, A08015EN1103, A07015EN1103
A11030EN1103, A10030EN1103, A09030EN1103, A08030EN1103, A07030EN1103
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC1103, A12015EC1103, A11015EC1103, A10015EC1103, A08515EC1103
A13030EC1103, A12030EC1103, A11030EC1103, A10030EC1103, A08530EC1103
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0405, A12015EE0405, A11015EE0405, A10015EE0405, A08515EE0405,
A13030EE0405, A12030EE0405, A11030EE0405, A10030EE0405, A08530EE0405

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Mar

ALPINE
ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

REF	LET-IN VERT
DATE	04/14/05
DRWG	GBLLETIN0405
-ENG	DLJ/KAR

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

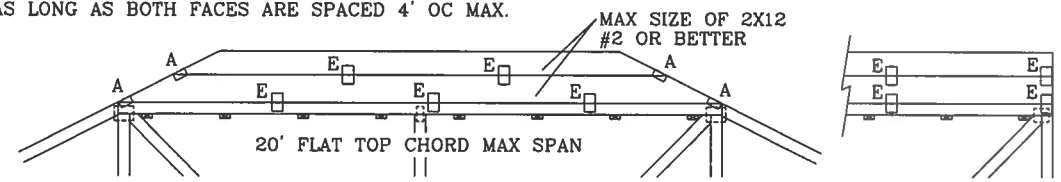
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

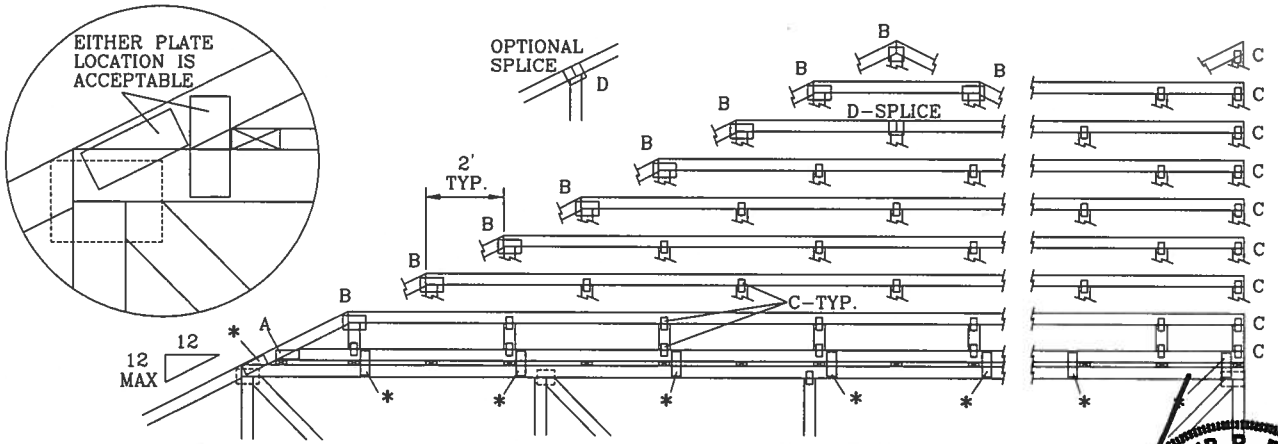
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

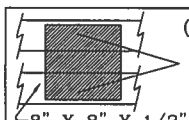
130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.



(4) 6d BOX (0.099"X 2.",MIN) NAILS.

8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET.

(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

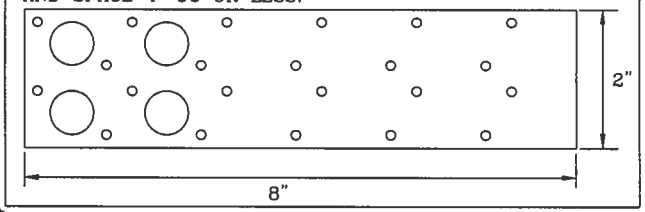
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



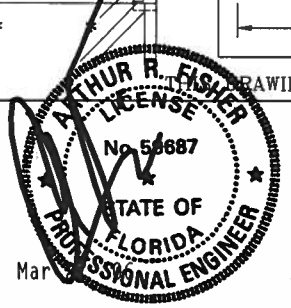
THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



Mar

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF	PIGGYBACK
DATE	04/14/05
DRWG	PIGBACKB0405
-ENG	DLJ/KAR