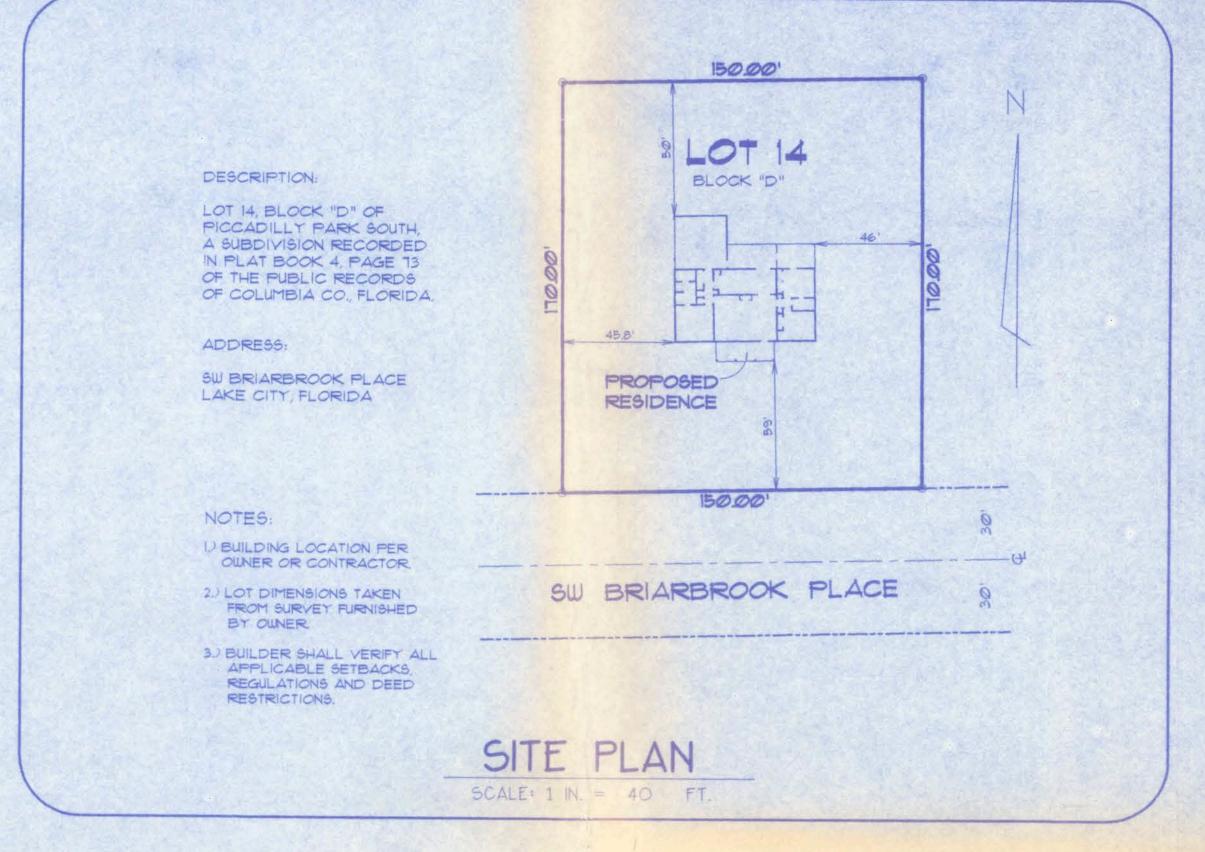


FLOOR PLAN SCALE: 1/4 IN. = 1 FT.

# Troiano Residence

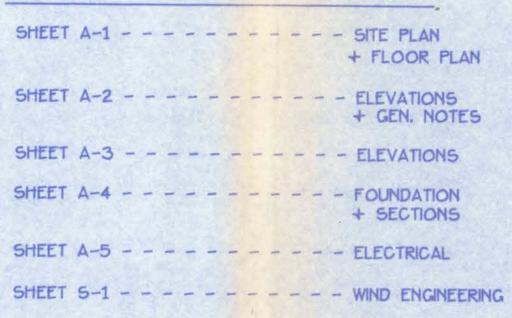


SWS = Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.

## AREA SUMMARY

| GARAGE | 484<br>192<br>203 | SF<br>SF<br>SF |
|--------|-------------------|----------------|
| ROOF   | 2776              | GE             |

## Index to Sheets



WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

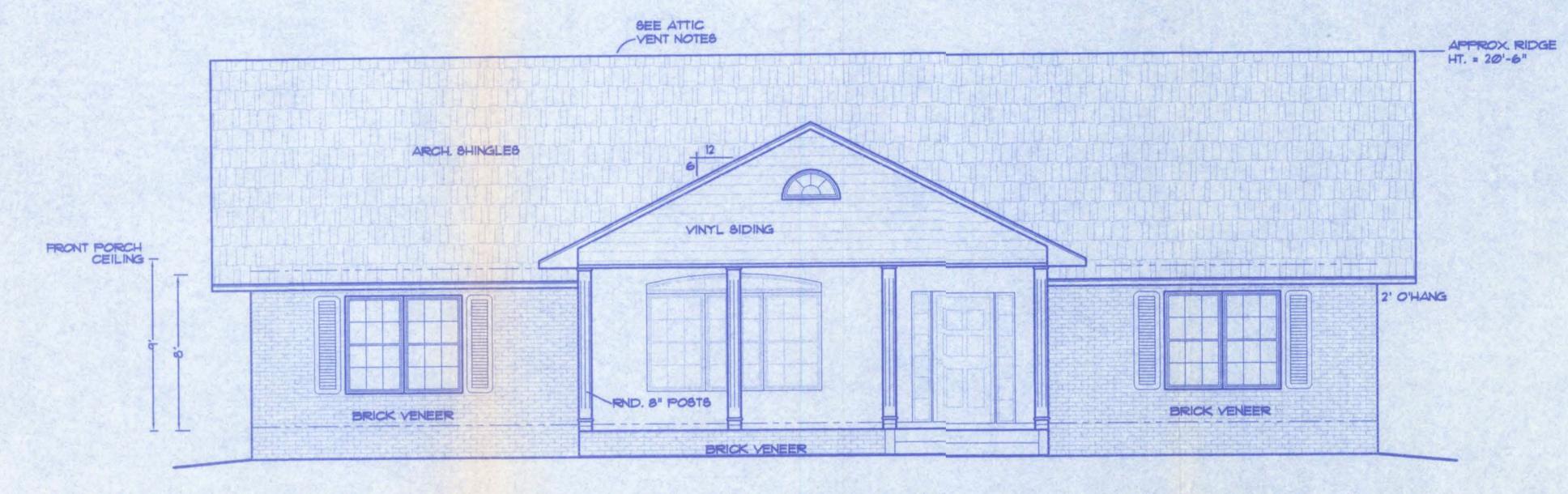
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scape of work, and builder responsibilities on sheet S-1 control.

LOT 14, BLK D PICCADILLY PARK SOUTH SUBDIVISION SW BRIARBROOK PLACE LAKE CITY, FLORIDA

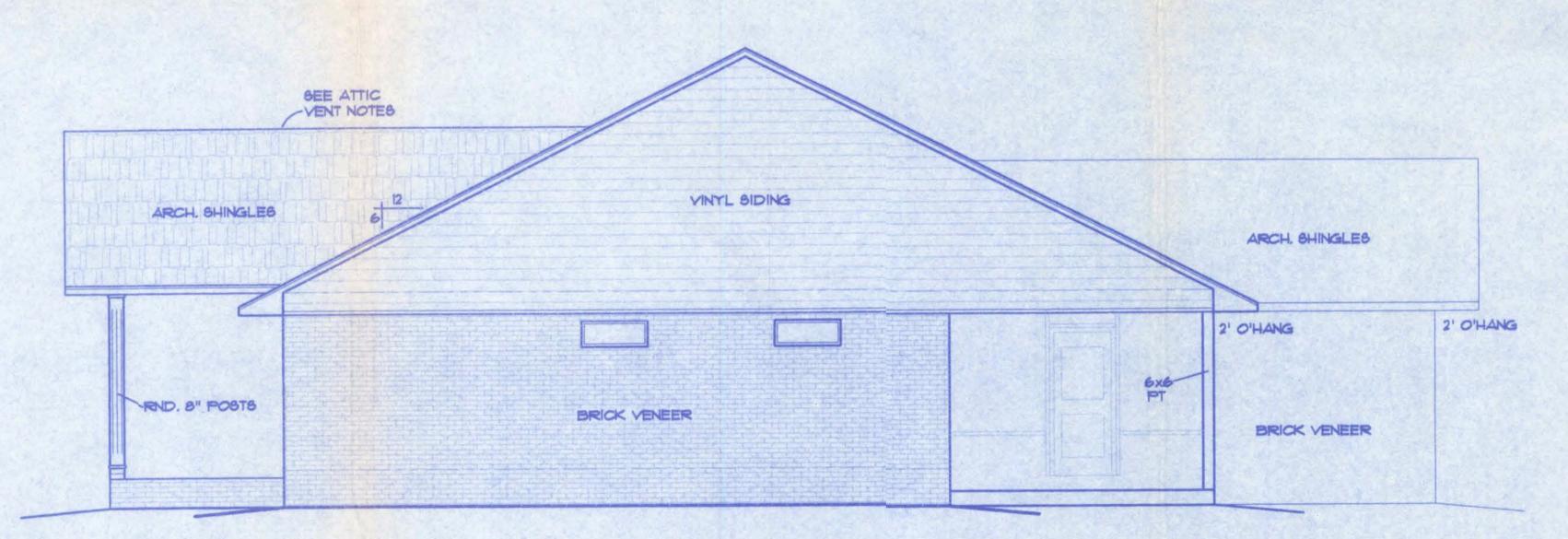






## FRONT ELEVATION

SCALE: 1/4 IN. = 1 FT.



RIGHT ELEVATION

SCALE: 1/4 IN. = 1 FT.

## GENERAL NOTES

- 1.) See 'Wind Load Detail Sheet S-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufactuer shall prepare Shop Drawings indicating Truss placement. Girder locations. Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

### ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion—resistant wire mesh, with 1 / 8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or comice vents with the balance of the required ventilation provided by eave or cornice vents.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

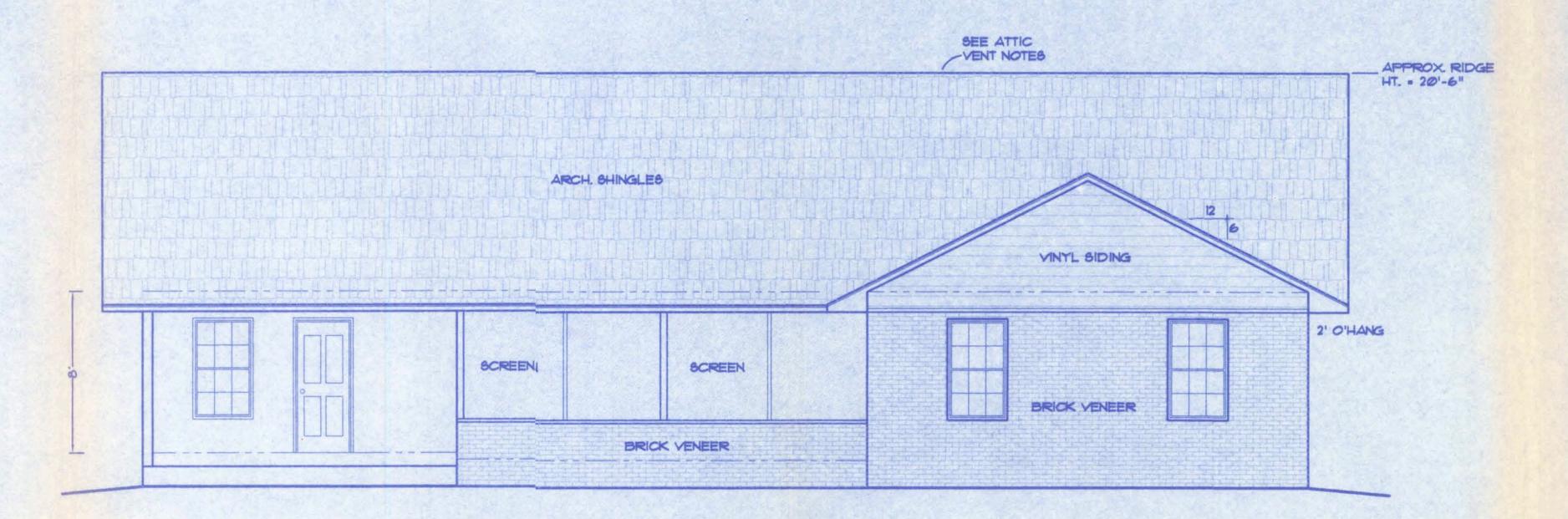
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DOCATION: LOT 14, BLK D PICCADILLY PARK SOUTH SUBDIVISION
SU BRIARBROOK PLACE
LAKE CITY, FLORIDA

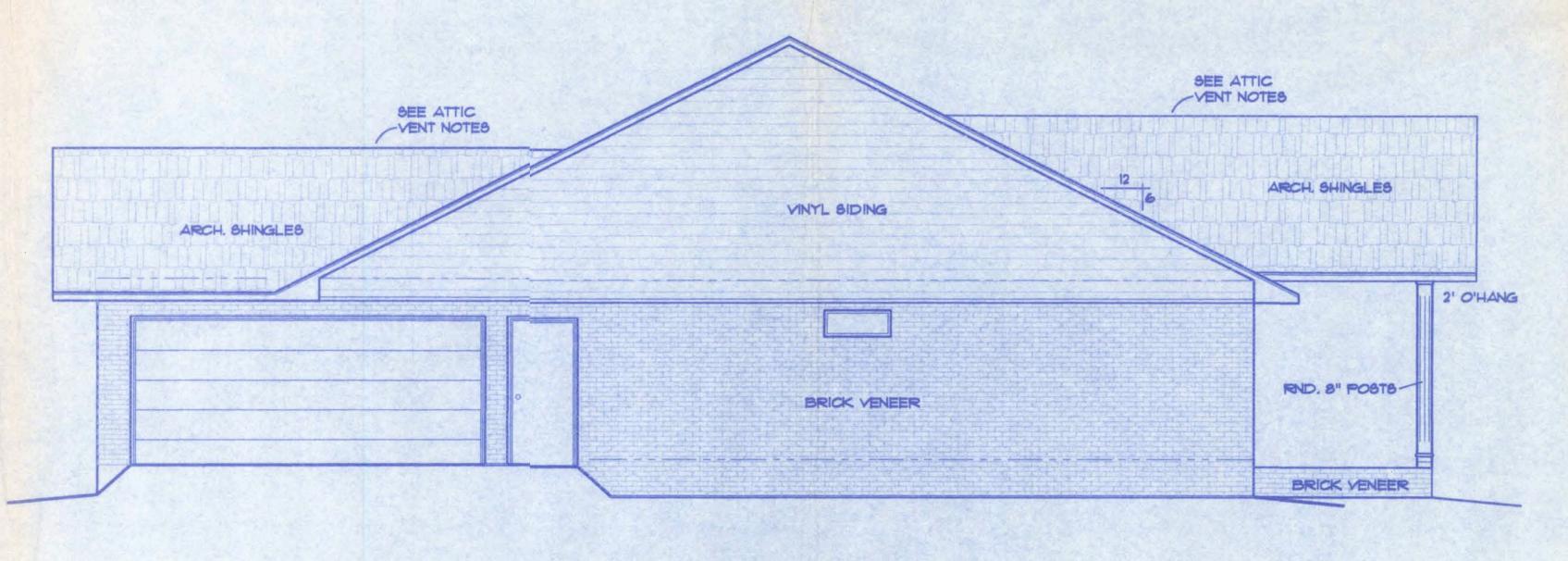
Job No.: 612078
Millie





## REAR ELEVATION

SCALE: 1/4 IN. = 1 FT.



#### ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, wit h 1 / 8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

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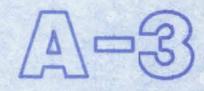
LEFT ELEVATION SCALE: 1/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disasway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

LOT 14, BLK D PICCADILLY PARK SOUTH SUBDIVISION SW BRIARBROOK PLACE LAKE CITY, FLORIDA

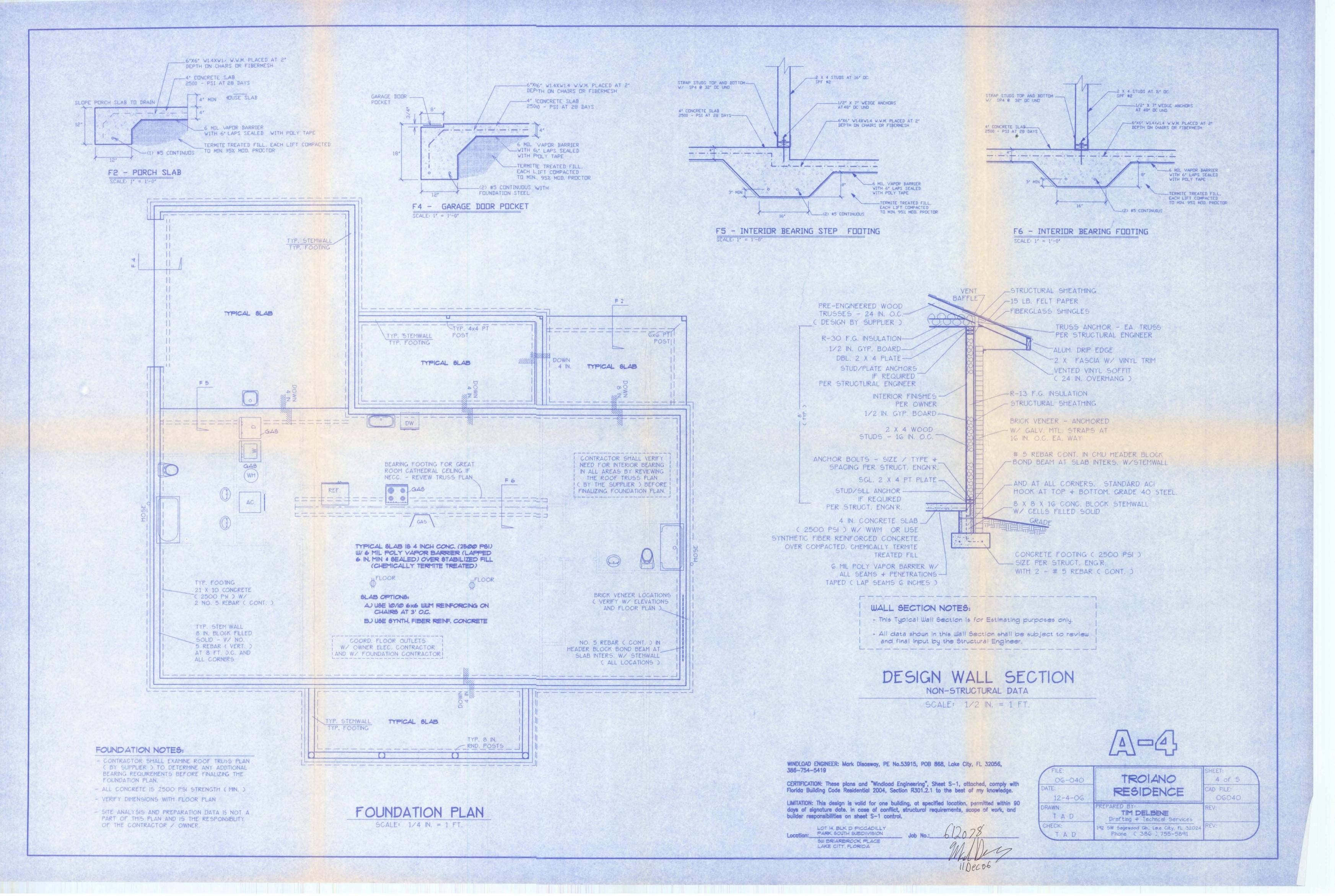


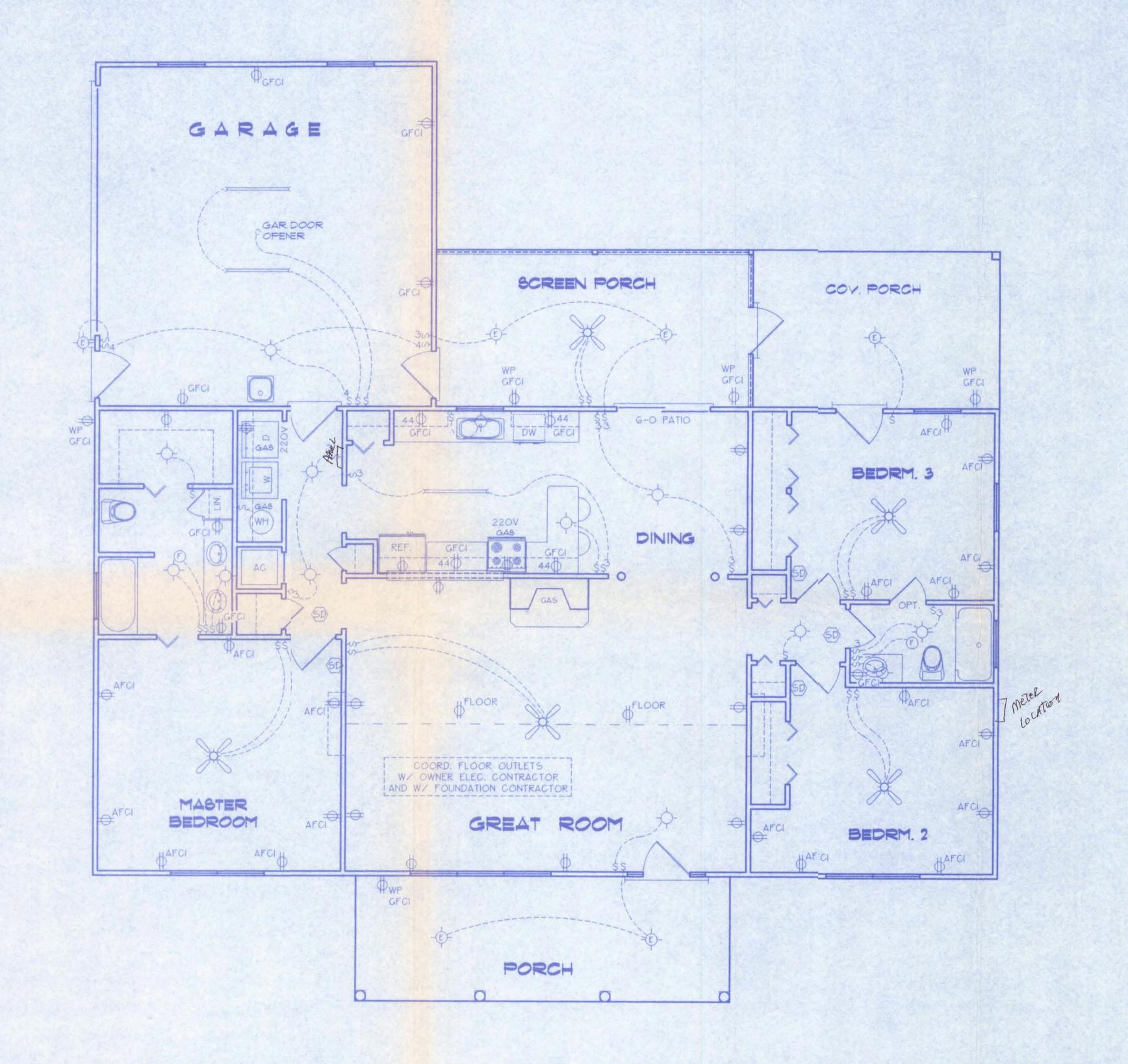
06-040 12-4-06 DRAWN: TAD CHECK:

TROIANO

3 of 5 AD FILE: 06040 TIM DELBENE
Drafting + Technical Services

192 SW Sagewood Gln. Late City. FL 32024 REV: Phone ( 386 ) 755-5891





ELECTRICAL PLAN
NOT TO SCALE

## ELECTRICAL SYMBOL LEGEND = FLOURESCENT LIGHTING FIXTURE. = CEILING LIGHT FIXTURE = EXTERIOR LIGHTING FIXTURE = LIGHT SWITCH. = THREE-WAY SWITCH. = 110 V. DUPLEX OUTLET. = SPECIAL HEIGHT 110 V. DUPLEX OUTLET = GROUND FAULT CIRC. OUTLET = ARC FAULT CIRC. OUTLET = 110 V. SINGLE ... = 220 VOLT OUTLET C 4 WIRE ) = FAN LOCATION C CEILING ) = FAN LOCATION C EXHAUST ) = SMOKE DETECTOR

#### ELECTRICAL PLAN NOTES

-WIRE ALL APPLIANCES. HVAC UNITS AND OTHER EQUIPMENT PER MANUE. SPECIFICATIONS.

-CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

-ALL INSTALLATIONS SHALL BE PER NATL ELECTRIC CODE.

-ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

-TELEPHONE. TELEVISION AND OTHER LOW VOLTAGE
DEVICES OR OUTLETS SHALL BE AS FER THE OWNER'S
DIRECTIONS. + IN ACCORDANGE W/ APPLICABLE
SECTIONS OF NEC-LATEST EDITION.

-ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

-ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD )
TO BE DETERMINED BY POWER COMPANY.

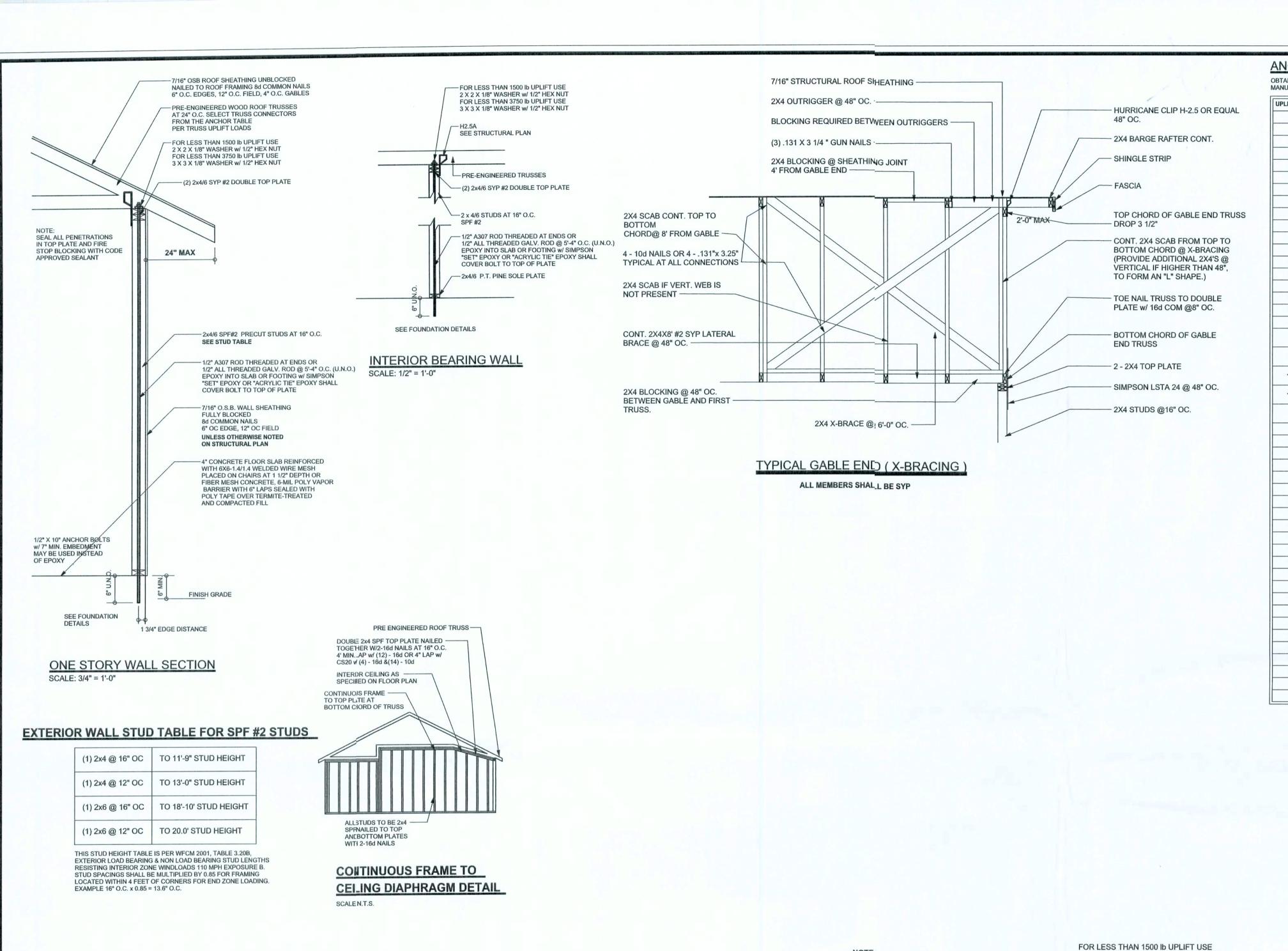
A-5

LOT 14, BLK D PICCADILLY PARK SOUTH SUBDIVISION SW BRIARBROOK PLACE LAKE CITY, FLORIDA

| FILE:<br>06-040 | TROIANO  | SHEET:<br>5 of 5   |
|-----------------|--|--------------------|
| DATE: 12-4-06   | KESIDENCE  | CAD FILE:<br>OGO4O |
| DRAWN:          | PREPARED BY:  TIM DELBENE  Drafting + Technical Services         | REV:               |
| CHECK:          | 192 SW Segewood Gh. Leke City FL 32024<br>Phone ( 38G ) 755-5891 | REV:               |

REVISIONS

SOFTPIAN



- NON-SUPPORTIVE

SUPPORTIVE

3 SIMPSON LSTA13'S (1-ONE SIDE, 2-ON-OPPOSITE SIDE) FA. NAILED WITH 14-1)d

SCALE: N.T3.

4-SIMPSON LST/18 -

(2-ONE SIDE,2-O) OTHER SIDE)

IF BEAM JOINT IS AT ----

INSTALL ONE SIMPSON

POST CONNECTION.

BEAMS

2) 2X12 SYP #2 MIN. -

SÉE STRUCTURAL PLAN

SIMPSON HUS412 MIN.

SCALE: N.T.S.

BEAM W/4-16d

BEAM MAY BE ATTACHED IN

**BEAM CORNER CONNECTION. DETAIL** 

— SIMPSON HUS412 MIN

SCALE: N.T.S.

SEE STRUCTURAL PLAN

SEE STRUCTURAL PLAN

(4)-2x4 SPF #2 NAILED

TOGETHER W/2-16d NAILS AT 16" O.C.

**BEAM MID-WALL CONNECTION DETAIL** 

MIN. (SEE STRUCTURAL PLAN)

SEE STRUCTURAL PLAN

2X4 LADDER BEAM

COLUMN

SUPPORTIVE POST TO BEAM

SUPPORTIVE CENTER POST TO BEAM DETAIL

DETAILFOR SINGLE BEAM

SUIPORTIVE BEAM-

SIMPSON H2.5A U.N.O.-

SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

**ANCHOR TABLE** 

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR*       | TO PLATES      | TO RAFTER/TRUSS | TO STUDS                             |
|-----------------|-----------------|------------------------|----------------|-----------------|--------------------------------------|
| < 420           | < 245           | H5A                    | 3-8d           | 3-8d            |                                      |
| < 455           | < 265           | H5                     | 4-8d           | 4-8d            |                                      |
| < 360           | < 235           | H4                     | 4-8d           | 4-8d            |                                      |
| < 455           | < 320           | Н3                     | 4-8d           | 4-8d            |                                      |
| < 415           | < 365           | H2.5                   | 5-8d           | 5-8d            |                                      |
| < 600           | < 535           | H2.5A                  | 5-8d           | 5-8d            |                                      |
| < 950           | < 820           | H6                     | 8-8d           | 8-8d            |                                      |
| < 745           | < 565           | H8                     | 5-10d, 1 1/2"  | 5-10d, 1 1/2"   |                                      |
| < 1465          | < 1050          | H14-1                  | 13-8d          | 12-8d, 1 1/2"   |                                      |
| < 1465          | < 1050          | H14-2                  | 15-8d          | 12-8d, 1 1/2"   |                                      |
| < 990           | < 850           | H10-1                  | 8-8d, 1 1/2"   | 8-8d, 1 1/2"    |                                      |
| < 760           | < 655           | H10-2                  | 6-10d          | 6-10d           |                                      |
| < 1470          | < 1265          | H16-1                  | 10-10d, 1 1/2" | 2-10d, 1 1/2"   |                                      |
| < 1470          | < 1265          | H16-2                  | 10-10d, 1 1/2" | 2-10d, 1 1/2"   |                                      |
| < 1000          | < 860           | MTS24C                 | 7-10d 1 1/2"   | 7-10d 1 1/2"    |                                      |
| < 1450          | < 1245          | HTS24                  | 12-10d 1 1/2"  | 12-10d 1 1/2"   |                                      |
| < 2900          | < 2490          | 2 - HTS24              |                |                 |                                      |
| < 2050          | < 1785          | LGT2                   | 14 -16d        | 14 -16d         |                                      |
|                 |                 | HEAVY GIRDER TIEDOWNS* |                |                 | TO FOUNDATION                        |
| < 3965          | < 3330          | MGT                    |                | 22 -10d         | 1-5/8" THREADED ROI<br>12" EMBEDMENT |
| < 10980         | < 6485          | HGT-2                  |                | 16 -10d         | 2-5/8" THREADED RO                   |
| < 10530         | < 9035          | HGT-3                  |                | 16 -10d         | 2-5/8" THREADED ROI<br>12" EMBEDMENT |
| < 9250          | < 9250          | HGT-4                  |                | 16 -10d         | 2-5/8" THREADED ROI<br>12" EMBEDMENT |
|                 |                 | STUD STRAP CONNECTOR*  |                |                 | TO STUDS                             |
| < 435           | < 435           | SSP DOUBLE TOP PLATE   | 3 -10d         |                 | 4 -10d                               |
| < 455           | < 420           | SSP SINGLE SILL PLATE  | 1 -10d         |                 | 4 -10d                               |
| < 825           | < 825           | DSP DOUBLE TOP PLATE   | 6 -10d         |                 | 8 -10d                               |
| < 825           | < 600           | DSP SINGLE SILL PLATE  | 2 -10d         |                 | 8 -10d                               |
| < 885           | < 760           | SP4                    |                |                 | 6-10d, 1 1/2"                        |
| < 1240          | < 1065          | SPH4                   |                |                 | 10-10d, 1 1/2"                       |
| < 885           | < 760           | SP6                    |                |                 | 6-10d, 1 1/2"                        |
| < 1240          | < 1065          | SPH6                   |                |                 | 10-10d, 1 1/2"                       |
| < 1235          | < 1165          | LSTA18                 | 14-10d         |                 |                                      |
| < 1235          | < 1235          | LSTA21                 | 16-10d         |                 |                                      |
| < 1030          | < 1030          | CS20                   | 18-8d          |                 |                                      |
| < 1705          | < 1705          | CS16                   | 28-8d          |                 |                                      |
|                 |                 | STUD ANCHORS*          | TO STUDS       |                 | TO FOUNDATION                        |
| < 1350          | < 1305          | LTT19                  | 8-16d          |                 | 1/2" AB                              |
| < 2310          | < 2310          | LTTI31                 | 18-10d, 1 1/2" |                 | 1/2" AB                              |
| < 2775          | < 2570          | HD2A                   | 2-5/8" BOLTS   |                 | 5/8" AB                              |
| < 4175          | < 3695          | HTT16                  | 18 - 16d       |                 | 5/8" AB                              |
| < 1400          | < 1400          | PAHD42                 | 16-16d         |                 |                                      |
| < 3335          | < 3335          | HPAHD22                | 16-16d         |                 |                                      |
| < 2200          | < 2200          | ABU44                  | 12-16d         |                 | 1/2" AB                              |
| < 2300          | < 2300          | ABU66                  | 12-16d         |                 | 1/2" AB                              |
| < 2320          | < 2320          | ABU88                  | 18 - 16d       |                 | 2-5/8" AB                            |

**GRADE & SPECIES TABLE** 

SYP #2

SYP #2

SYP #2

24F-V3 SP

TIMBERSTRAND

MICROLAM

PARALAM

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

DOOR WIDTH 3/8" x 4" LAG 16d (2) ROWS OF STAGGER 131 x 3 1/4" GN

5" O.C.

4" O.C.

3" O.C.

5" O.C.

4" O.C.

3" O.C.

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG

18" O.C.

16" O.C.

SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT

TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

GN PER TABLE BELOW:

11' - 15'

16' - 18'

2x6SYP #2 DOOR BUCK-

BRACKET. ----

2 X 2 X 1/8" WASHER

3 X 3 X 1/8" WASHER

CRIPPLES IF REQUIRED

(5) .131 x 3 1/4" GUN NAILS

INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

(SEE STRUCTURAL PLAN)

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL

TOE NAILED THRU SILL-

FOR LESS THAN 3750 Ib UPLIFT USE

— NAIL SHEATHING TO HEADER AND TOP

PLATE WITH 8d AT 3" O.C. FOR UPLIFT

— SP4/6 @ 48" O.C. (U.N.O.) /——(7) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU HEADER

INTO KING STUD

IF TRUSS TO WALL STRAPS ARE NAILED

TO THIE HEADER THE SP4/6 @ 48" O.C.

INTO KING STUD

(7) .1|31 x 3 1/4" GUN NAILS----

TOE NAILED THRU HEADER

-(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

SEE FOOTING DETAILS

ANCHOR BOLT

TYPICAL PORCH POST DETAIL

ARE NOT REQUIRED

Fb (psi) E (10<sup>6</sup> psi)

1.6

1.6

2.0

1200

1050

975

2900

2900

#### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT ANDREACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT I) THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WINDLOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET RAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3(00 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM ( 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BJILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CJTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOIN'S IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 'DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPFLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THER OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO

LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CML WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### **BUILDER'S RESPONSIBILITY**

| THE BUILDER    | R AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE  |
|----------------|---|
| SPECIFICALL    | Y NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.   |
| CONFIRM SITE ( | CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND  |
| BACKFILL HEIGH | IT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.   |
| PROVIDE MATE   | RIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY VITH FBCR 2004  |
| REQUIREMENTS   | FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.  |
| BELIEVE THE PL | TINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU AN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL ENGINEER IMMEDIATELY. |
| VERIFY THE TRI | JSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS  |

#### ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

#### **DESIGN DATA**

TRUSS SHEETS.

|                  | OADS PER   |          |  |                      |                   |             |               |                |       |                |
|------------------|--|----------|--|----------------------|-------------------|-------------|---------------|----------------|-------|----------------|
| MEAN F<br>ON UPF | SED SIMPL<br>ROOF HEIG<br>PER HALF C<br>AND UNOB | HT NOT E | R ESCAR  | IG LEAST<br>PMENT 60 | HORIZO<br>FT IN E | NTAL D      | IMEN<br>OFT I | SION           | OR 60 | 0 FT; N        |
| BUILDIN          | NG IS NOT I                                      | N THE HI | GH VELO  | CITY HUR             | RICANE            | ZONE        |               |                |       |                |
| BUILDIN          | NG IS NOT I                                      | N THE W  | IND-BORN   | NE DEBRIS            | REGIC             | N           |               |                |       |                |
| 1.) BA           | SIC WIND S                                       | SPEED =  | 110 MPH  |                      |                   |             |               |                |       |                |
| 2.) WI           | ND EXPOSI  | JRE = B  |  |                      |                   |             |               |                |       |                |
| 3.) WI           | ND IMPORT  | ANCE FA  | ACTOR =  | 1.0                  |                   |             |               |                |       |                |
|                  | ILDING CAT                                       |          |  |                      |                   |             |               |                |       |                |
|                  | OF ANGLE   |          |  |                      |                   |             |               |                |       |                |
| 1000             | AN ROOF H  | 1000     |  |                      |                   |             |               | <del></del>    |       |                |
|                  |  |          |  | IENT - NI            | /ENO              | OPED D      | LULD          | INIO           |       |                |
| -                | MRONENT  |          |  |                      |                   |             |               | ,              | Dace  | 0/011          |
| 8.) CO           | MPONENT  | S AND CL | ADDING   | DESIGN V             | אין טאוי          | ESSUR       | =5 (1.        | ABLE           | K301, | .2(2))         |
|                  | 7  |          |  |                      |                   | Zone        | Effec         | tive W         | ind A | ea (ft2)       |
| 4                |  |          |  |                      |                   |             |               | 10             |       | 100            |
|                  | XX   |          |  |                      |                   | 1           | -             | -              | -     | -18.1          |
| K                | 2 2  |          |  |                      |                   | 2           | 19.9          | -25.5          | 18.1  |                |
| 5                | 2  | 2 1      |  |                      |                   | 2 O'hg<br>3 | 10 0          | -40.6<br>-25.5 | 10 1  | -40.6<br>-21.8 |
| 3                | 4  | 1 2 3    | 5  |                      |                   | 3 O'hg      | 13.3          | -68.3          | 10.1  | -42.4          |
|                  | 1  | 13       | 4  |                      |                   | 4           | 21.8          | -23.6          | 18.5  |                |
| >2               |  | 55       |  |                      |                   | 5           |               | -29.1          |       | -              |
| 4                | PHE  | 1        |  |                      |                   | Doors       | & Wine        | dows           | 21.8  | -29.1          |
| 1                | 2/ 2   |          |  |                      |                   |             | st Cas        |                |       |                |
| (3)              | 1  |          |  |                      |                   |             | 5, 10         |                |       |                |
| 5                | 2  | 13       | The same of the sa |                      |                   | 8x7 Gar     |               |                | 19.5  | -              |
| 2                | 4  | 12/      | 4  |                      |                   | 16x7 Ga     | arage l       | Door           | 18.5  | -21.0          |
|                  |  | 3        | 57   |                      |                   |             |               |                |       |                |
|                  |  | 133      | -  |                      |                   |             |               |                |       |                |
|                  |  |          |  |                      |                   |             |               |                |       |                |
| DESIGN           | LOADS  |          |  |                      |                   |             |               |                |       | -              |
| FLOOR            | 40 PSF (A  | LL OTHE  | R DWELL  | ING ROO              | MS)               |             |               |                |       |                |
|                  | 30 PSF (S  |          |  |                      | -                 |             |               |                |       |                |
|                  | 30 PSF (A  |          | ,  |                      |                   |             |               |                | _     |                |
|                  |  |          |  | TORAGE,              | <3:12)            |             |               |                |       |                |
| ROOF             | 20 PSF (F  |          |  |                      |                   |             |               |                |       |                |
|                  | 16 PSF (4  |          |  |                      |                   |             |               |                | -     |                |
|                  | 12 PSF (1  |          |  | (R)                  |                   |             |               |                |       |                |
|                  | 0. (1  |          | JINE/ III  | ,                    |                   |             |               |                |       |                |

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

rmission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have amined this plan, and that the applicable rtions of the plan, relating to wind engin comply with section R301.2.1, florida building code residential 2004, to the best of my LIMITATION: This design is valid for one ouilding, at specified location. MARK DISOSWAY P.E. 53915

WINDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

ated dimensions supercede scaled

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not proceed without clarification

32056, 386-754-5419

Norton Home <u>Improvements</u>

Troiano Residence

ADDRESS: Lot 14 Block D Piccadilly Park South S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: December 08, 2006 DRAWN BY: STRUCTURAL BY

David Disosway

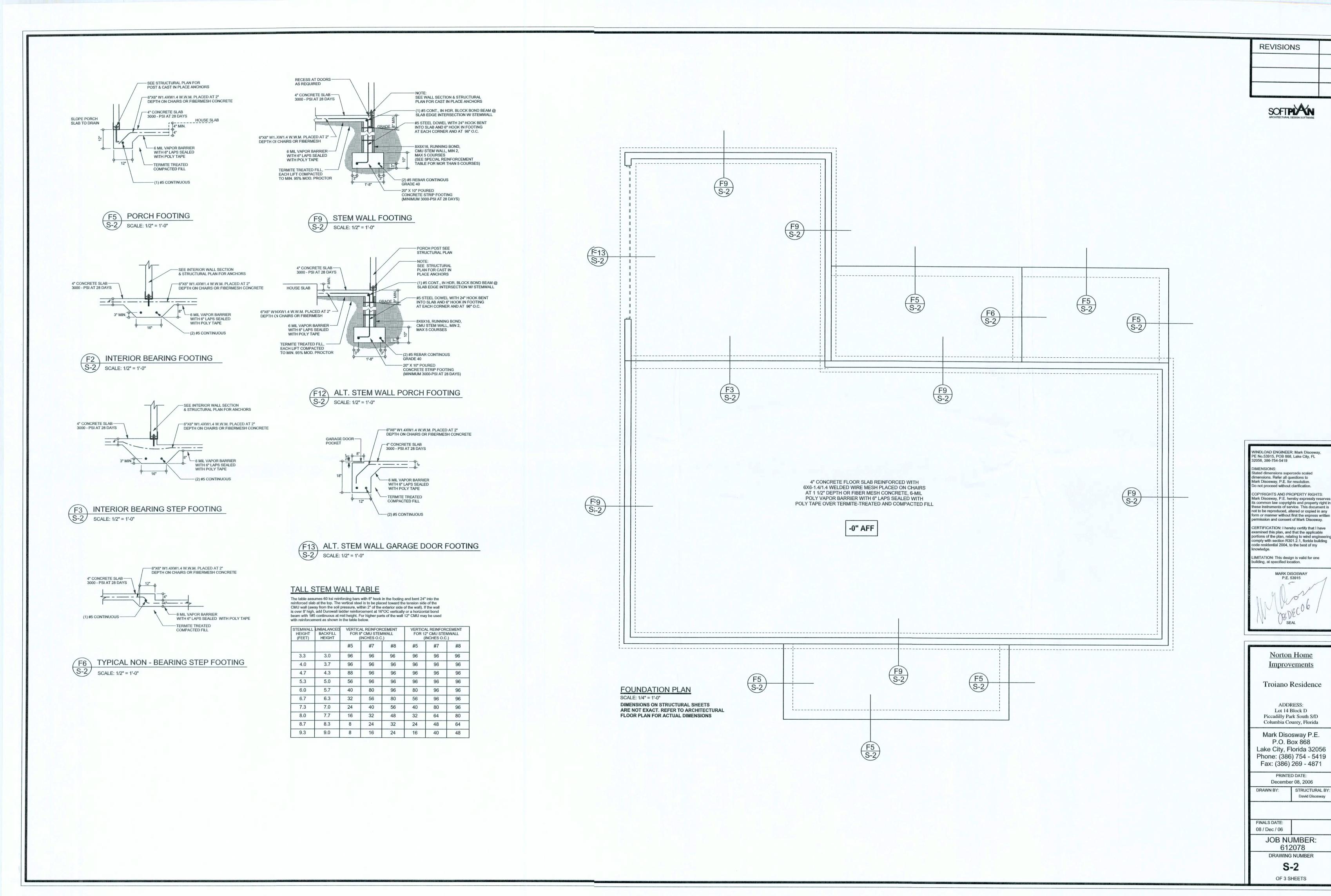
FINALS DATE: 08 / Dec / 06 JOB NUMBER:

612078

DRAWING NUMBER **S-1** 

OF 3 SHEETS

**GARAGE DOOR BUCK INSTALLATION DETAIL** 



MARK DISOSWAY P.E. 53915

Norton Home

<u>Improvements</u>

ADDRESS:

Lot 14 Block D

PRINTED DATE:

612078

**S-2** 

OF 3 SHEETS

STRUCTURAL BY

David Disosway

REVISIONS



#### STRUCTURAL PLAN NOTES

SWS = 8.0'

T02 TRUSS

**T02 TRUSS** 

T02 TRUSS

11 11 11 11 11

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

SWS = 4.0

12" EMBEDMENT-

12" EMBEDMENT-

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- ALL LOAD BEARING FRAME WALL HEADERS SN-2 SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

#### WALL LEGEND

—ABU (TYP.))

|| UPLIFIT

-1315 LB

UPLIFT

| (2) 2X/12X6',2J|1K

USE H2.5A (480Ib) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE

— ABU (TYP.)

SWS = 4.5'

T01 TRUSS

T01 TRUSS

T01 TRUSS

T01 TRUSS

**T01G TRUSS** 

-1315 LB UPLIFT

| SWS = 0.0' | 1ST FLOOR EXTERIOR WALL                                      |
|------------|--|
| SWS = 0.0' | 2ND FLOOR EXTERIOR   |
| IBW        | 1ST FLOOR INTERIOR BEARING WALLS<br>SEE DETAILS ON SHEET S-1 |
| IBW        | 2ND FLOOR INTERIOR BEARING WALLS<br>SEE DETAILS ON SHEET S-1 |

#### THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD -- INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

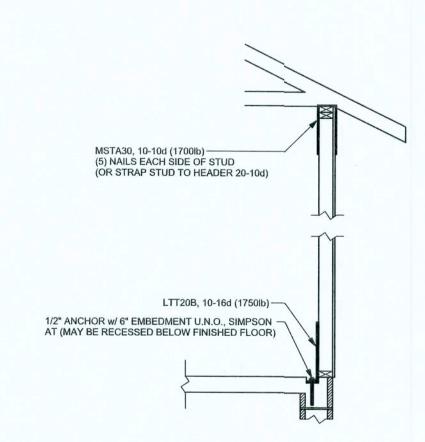
#### HEADER LEGEND

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) NUMBER OF KING STUDS (FULL LENGTH) ---NUMBER OF JACK STUDS (UNDER HEADER) -SPAN OF HEADER SIZE OF HEADER MATERIAL ---NUMBER OF PLIES IN HEADER

#### TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

|              | REQUIRED | ACTUAL |
|--------------|----------|--------|
| TRANSVERSE   | 34.5'    | 70.0'  |
| LONGITUDINAL | 32.5'    | 62.0'  |



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL SCALE: 1/2" = 1'-0"

> WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. o not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reser its common law copyrights and property right these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express writte permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineer comply with section R301.2.1, florida building code residential 2004, to the best of my LIMITATION: This design is valid for one building, at specified location. MARK DISOSWAY P.E. 53915

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PRINTED DATE: December 08, 2006 STRUCTURAL BY DRAWN BY: David Disosway

612078

S-3

OF 3 SHEETS

FINALS DATE: 08 / Dec / 06

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L219140

JOB NUMBER: DRAWING NUMBER