

DATE 11/07/2005

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000023825

APPLICANT HUGO ESCALANTE PHONE 386.288.8666
ADDRESS POB 280 FT. WHITE FL 32038
OWNER H&M CONSTRUCTION PHONE 386.288.8666
ADDRESS 269 SW PLATEAU GLEN LAKE CITY FL 32024
CONTRACTOR HUGO ESCALANTE PHONE 386.288.8666

LOCATION OF PROPERTY 47-S TO C-242,TR TO WISE DRIVE,TO GARDEN TERRACE,TR, GO TO PLATEAU,TR GO TO END, LOT ON L.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 96600.00

HEATED FLOOR AREA 1932.00 TOTAL AREA 2640.00 HEIGHT .00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03113-167 SUBDIVISION WISE ESTATES

LOT 37 BLOCK C PHASE UNIT TOTAL ACRES .66

000000882 CRC1326967
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
18"x32'MITERED 05-1007-N BLK JTH N
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: FINISHED FLOOR ELEVATION 100.0'. NEED ELEVATION LETTER BEFORE SLAB.
NOC ON FILE.

Check # or Cash 3348

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by
Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by
Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by
M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by
Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by
M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 485.00 CERTIFICATION FEE \$ 13.20 SURCHARGE FEE \$ 13.20
MISC. FEES \$.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 611.40
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.



Donald F. Lee & Associates, Inc.
Surveyors & Engineers

140 NW Ridgewood Avenue
Lake City, Florida 32055
(386) 755-6166
Fax (386) 755-6167
dfla@suwanneevalley.net

23825

Wednesday, December 28, 2005

TO: EWPL, Inc. – Hugo Escalante

CC: Columbia County Building Department

FROM: Tim Delbene, P.L.S. – Donald F. Lee & Associates, Inc.

RE: Lot 37, Block C, Wise Estates - Elevation check

This letter is to certify that the elevation was measured for the finished floor (at Stemwall) for a house under construction on the above referenced Lot in Wise Estates. The Elevations are as follows:

House Floor: 100.14 - Adjacent grades: 97.8 (lowest) & 98.2 (highest)

The property lies in Flood Zones "A" and "X" per Flood Insurance Rate Maps (FIRM). No base flood elevation (BFE) is established for this area. The project Engineer for Wise Estates subdivision, has set the minimum floor elevation for Lot 37, Block C at 100.0 feet (data per record plat).

Timothy A. Delbene, P.L.S.
Florida Cert. No. LS 5594

DATE: 12/28/2005

Donald F. Lee & Associates, Inc.

FEDERAL EMERGENCY MANAGEMENT AGENCY
NATIONAL FLOOD INSURANCE PROGRAM

O.M.B. No. 3067-0077
Expires December 31, 2005

ELEVATION CERTIFICATE

Important: Read the instructions on pages 1 - 7.

SECTION A - PROPERTY OWNER INFORMATION

BUILDING OWNER'S NAME EWPL, Inc. - Hugo Escalante		For Insurance Company Use: Policy Number	
BUILDING STREET ADDRESS (Including Apt., Unit, Suite, and/or Bldg. No.) OR P.O. ROUTE AND BOX NO. SW Plateau Glen		Company NAIC Number	
CITY Lake City	STATE FL	ZIP CODE 32025	
PROPERTY DESCRIPTION (Lot and Block Numbers, Tax Parcel Number, Legal Description, etc.) Lot 37, Block "C" - Wise Estates - Plat Bk 7, Pages 164-167			
BUILDING USE (e.g., Residential, Non-residential, Addition, Accessory, etc. Use a Comments area, if necessary.) Residential			
LATITUDE/LONGITUDE (OPTIONAL) (##° - ##' - ##" or ###.####)		HORIZONTAL DATUM: ☐ NAD 1927 ☐ NAD 1983	
		SOURCE: ☐ GPS (Type): ☐ USGS Quad Map ☐ Other: _____	

SECTION B - FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

B1. NFIP COMMUNITY NAME & COMMUNITY NUMBER Columbia County, Florida 120070		B2. COUNTY NAME Columbia		B3. STATE Florida	
B4. MAP AND PANEL NUMBER 120070 0175	B5. SUFFIX B	B6. FIRM INDEX DATE 1/6/1988	B7. FIRM PANEL EFFECTIVE/REVISED DATE 1/6/1988	B8. FLOOD ZONE(S) X	B9. BASE FLOOD ELEVATION(S) (Zone AO, use depth of flooding) Sec.D

B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in B9.
☐ FIS Profile ☐ FIRM ☐ Community Determined ☒ Other (Describe): No BFE

B11. Indicate the elevation datum used for the BFE in B9: ☐ NGVD 1929 ☐ NAVD 1988 ☒ Other (Describe): No BFE

B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? ☐ Yes ☒ No Designation Date _____

SECTION C - BUILDING ELEVATION INFORMATION (SURVEY REQUIRED)

C1. Building elevations are based on: ☐ Construction Drawings* ☒ Building Under Construction* ☐ Finished Construction
 *A new Elevation Certificate will be required when construction of the building is complete.

C2. Building Diagram Number 1 (Select the building diagram most similar to the building for which this certificate is being completed - see pages 6 and 7. If no diagram accurately represents the building, provide a sketch or photograph.)

C3. Elevations - Zones A1-A30, AE, AH, A (with BFE), VE, V1-V30, V (with BFE), AR, AR/A, AR/AE, AR/A1-A30, AR/AH, AR/AO
 Complete Items C3.-a-i below according to the building diagram specified in Item C2. State the datum used. If the datum is different from the datum used for the BFE in Section B, convert the datum to that used for the BFE. Show field measurements and datum conversion calculation. Use the space provided or the Comments area of Section D or Section G, as appropriate, to document the datum conversion.
 Datum NAVD 1988 Conversion/Comments per subdivision design benchmarks

Elevation reference mark used Local Does the elevation reference mark used appear on the FIRM? ☐ Yes ☒ No

o a) Top of bottom floor (including basement or enclosure) 100. 14 ft.(m)

o b) Top of next higher floor N/A. . ft.(m)

o c) Bottom of lowest horizontal structural member (V zones only) N/A. . ft.(m)

o d) Attached garage (top of slab) N/A. . ft.(m)

o e) Lowest elevation of machinery and/or equipment servicing the building (Describe in a Comments area) N/A. . ft.(m)

o f) Lowest adjacent (finished) grade (LAG) 97. 8 ft.(m)

o g) Highest adjacent (finished) grade (HAG) 98. 2 ft.(m)

o h) No. of permanent openings (flood vents) within 1 ft. above adjacent grade N/A

o i) Total area of all permanent openings (flood vents) in C3.h N/A sq. in. (sq. cm)

License Number, Embossed Seal,
Signature, and Date

Timothy A. Delbene
PLS# 5594
12/28/05

SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION

This certification is to be signed and sealed by a land surveyor, engineer, or architect authorized by law to certify elevation information.
 I certify that the information in Sections A, B, and C on this certificate represents my best efforts to interpret the data available.
 I understand that any false statement may be punishable by fine or imprisonment under 18 U.S. Code, Section 1001.

CERTIFIER'S NAME Timothy A. Delbene, PSM

LICENSE NUMBER LS 5594

TITLE Land Surveyor

COMPANY NAME Donald F. Lee & Associates, Inc.

ADDRESS

140 NW Ridgewood Avenue

SIGNATURE

Timothy A. Delbene

CITY

Lake City

STATE

FL

ZIP CODE

32055

DATE

12/28/2005

TELEPHONE

386-755-6166

IMPORTANT: In these spaces, copy the corresponding information from Section A.			For Insurance Company Use:
BUILDING STREET ADDRESS (Including Apt., Unit, Suite, and/or Bldg. No.) OR P.O. ROUTE AND BOX NO. SW Plateau Glen - Lot 37, Blk C Wise Estates			Policy Number
CITY Lake City	STATE FL	ZIP CODE 32025	Company NAIC Number

SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION (CONTINUED)

Copy both sides of this Elevation Certificate for (1) community official, (2) insurance agent/company, and (3) building owner.

COMMENTS

Foundation is under construction. Elevation is on stemwall.

Minimum Floor Elevation is 100.0 - per subdivision engineer and as shown on plat of record.

No Base Flood Elevation (BFE) is established in this area. Lot is in Flood Zones "X" and "A"

☐ Check here if attachments

SECTION E - BUILDING ELEVATION INFORMATION (SURVEY NOT REQUIRED) FOR ZONE AO AND ZONE A (WITHOUT BFE)

For Zone AO and Zone A (without BFE), complete Items E1 through E4. If the Elevation Certificate is intended for use as supporting information for a LOMA or LOMR-F, Section C must be completed.

E1. Building Diagram Number (Select the building diagram most similar to the building for which this certificate is being completed - see pages 6 and 7. If no diagram accurately represents the building, provide a sketch or photograph.)

E2. The top of the bottom floor (including basement or enclosure) of the building is ___ ft.(m) ___ in.(cm) ☐ above or ☐ below (check one) the highest adjacent grade. (Use natural grade, if available).

E3. For Building Diagrams 6-8 with openings (see page 7), the next higher floor or elevated floor (elevation b) of the building is ___ ft.(m) ___ in.(cm) above the highest adjacent grade. Complete items C3.h and C3.i on front of form.

E4. The top of the platform of machinery and/or equipment servicing the building is ___ ft.(m) ___ in.(cm) ☐ above or ☐ below (check one) the highest adjacent grade. (Use natural grade, if available).

E5. For Zone AO only: If no flood depth number is available, is the top of the bottom floor elevated in accordance with the community's floodplain management ordinance?

☐ Yes ☐ No ☐ Unknown. The local official must certify this information in Section G.

SECTION F - PROPERTY OWNER (OR OWNER'S REPRESENTATIVE) CERTIFICATION

The property owner or owner's authorized representative who completes Sections A, B, C (Items C3.h and C3.i only), and E for Zone A (without a FEMA-issued or community-issued BFE) or Zone AO must sign here. The statements in Sections A, B, C, and E are correct to the best of my knowledge.

PROPERTY OWNER'S OR OWNER'S AUTHORIZED REPRESENTATIVE'S NAME

ADDRESS

CITY

STATE

ZIP CODE

SIGNATURE

DATE

TELEPHONE

COMMENTS

☐ Check here if attachments

SECTION G - COMMUNITY INFORMATION (OPTIONAL)

The local official who is authorized by law or ordinance to administer the community's floodplain management ordinance can complete Sections A, B, C (or E), and G of this Elevation Certificate. Complete the applicable item(s) and sign below.

G1. ☐ The information in Section C was taken from other documentation that has been signed and embossed by a licensed surveyor, engineer, or architect who is authorized by state or local law to certify elevation information. (Indicate the source and date of the elevation data in the Comments area below.)

G2. ☐ A community official completed Section E for a building located in Zone A (without a FEMA-issued or community-issued BFE) or Zone AO.

G3. ☐ The following information (Items G4-G9) is provided for community floodplain management purposes.

G4. PERMIT NUMBER	G5. DATE PERMIT ISSUED	G6. DATE CERTIFICATE OF COMPLIANCE/OCCUPANCY ISSUED
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G7. This permit has been issued for: ☐ New Construction ☐ Substantial Improvement

G8. Elevation of as-built lowest floor (including basement) of the building is:

___ ft.(m)

Datum: ___

G9. BFE or (in Zone AO) depth of flooding at the building site is:

___ ft.(m)

Datum: ___

LOCAL OFFICIAL'S NAME

TITLE

COMMUNITY NAME

TELEPHONE

SIGNATURE

DATE

COMMENTS

☐ Check here if attachments

November 3, 2005

Building Department

Re: Permit 0510-92, Ewpl Inc / Hugo Escalate, Spec House, Lot #37, Wise Estates S/D, Columbia County, Florida

Dear Building Official:

Please accept this letter as addendum to the plans for the above referenced house to change all references to FBC 2001 to FBC 2004.

- The plan was drawn prior to the effective date for FBC 2004, 01 October 2005.
- Since the wind load requirements of FBC 2004 remain basically unchanged from FBC 2001 there are no structural changes required to this plan.

Mark Disosway
03/20/05

Mark Disosway, PE
Florida Registered Professional Engineer

Cc Ewpl Inc / Hugo Escalate (Builder)

Let 3p wise

COLUMBIA COUNTY BUILDING DEPARTMENT

**RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR
FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS
ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant

Plans Examiner



All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.



Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.



Site Plan including:

- a) Dimensions of lot
- b) Dimensions of building set backs
- c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- d) Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required

- a) Plans or specifications must state compliance with FBC Section 1606
- b) The following information must be shown as per section 1606.1.7 FBC
 - a. Basic wind speed (MPH)
 - b. Wind importance factor (I) and building category
 - c. Wind exposure - if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
 - d. The applicable internal pressure coefficient
 - e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional

See
NOTE
1

Elevations including:

- a) All sides
- b) Roof pitch
- c) Overhang dimensions and detail with attic ventilation
- d) Location, size and height above roof of chimneys
- e) Location and size of skylights
- f) Building height
- e) Number of stories



Floor Plan including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

Roof System:

- a) Truss package including:
 - 1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
 - 2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
 - 1. Rafter size, species and spacing
 - 2. Attachment to wall and uplift
 - 3. Ridge beam sized and valley framing and support details
 - 4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
 - 1. All materials making up wall
 - 2. Block size and mortar type with size and spacing of reinforcement
 - 3. Lintel, tie-beam sizes and reinforcement
 - 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
 - 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
 - 6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
 - 7. Fire resistant construction (if required)
 - 8. Fireproofing requirements
 - 9. Shoe type of termite treatment (termicide or alternative method)
 - 10. Slab on grade
 - a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
 - 11. Indicate where pressure treated wood will be placed
 - 12. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
 - a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
 - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

- a) Manual J sizing equipment or equivalent computation
- b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

*****Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.
2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
(386) 758-1058 (Toileet facilities shall be provided for construction workers)
4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is \$50.00
6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

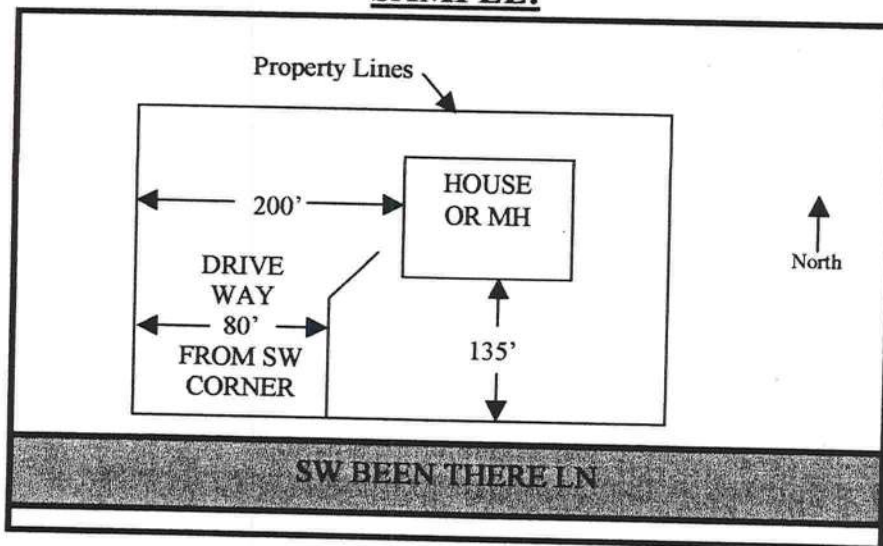
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
 - a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
 - b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
 - c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



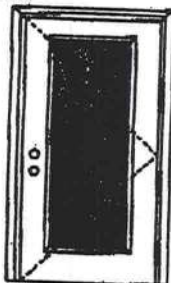
NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

X
Glazed Inswing Unit

COP-WL-EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'6".



Test Data Review Certificate #30284470 and COP/Time Report Validation Matrix #30284470-001 provides additional information - available from the ITA/WHI website (www.itala.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'6"

Design Pressure
+50.5/-50.5

(Inward water pressure special threshold design is used.)

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the action required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02 and MAD-WL-MAD041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



600 Series



622 Series

1/2 GLASS:



106 Series*



108, 109 Series*



120 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L Series*



157 Series*



108 Series



204 Series

*This glass is also used in the following door styles: 5-panel; 5-panel with scroll; 2x6-panel; 2x6-panel with scroll.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from
Masonite
Masonite International Corporation

X
Glazed Inswing Unit

COP-WI FN4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES: 3/4 GLASS:



404 Series



410 Series



450 Series

FULL GLASS:



100 Series

114, 120, 122
Series

152 Series



148 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balth

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



Test Data Review Certificate #0023447C and COP/Unit Report Validation Matrix #0023447C-001 provide additional information - available from the ITB/WI website (www.masonite.com). The Masonite website (www.masonite.com) or the Masonite technical center.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



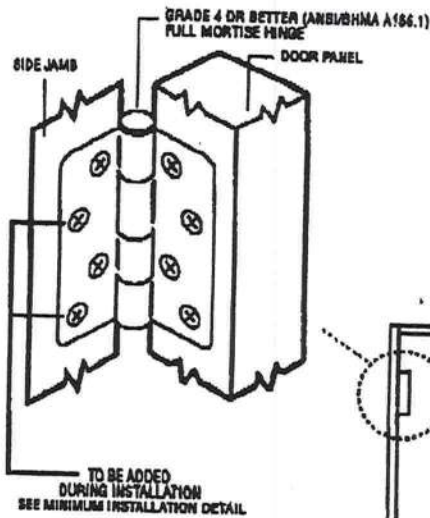
Exclusively from
Masonite
Masonite International Corporation

X
Unit

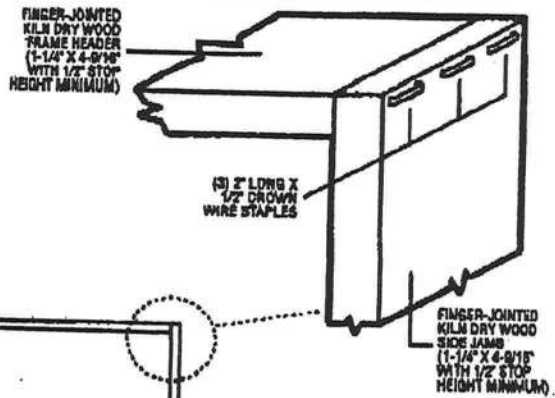
WAD-WI-MA0001-02

INSWING UNIT WITH SINGLE DOOR

TYPICAL HINGE ATTACHMENT



TYPICAL HEADER & SIDE JAMB ATTACHMENT

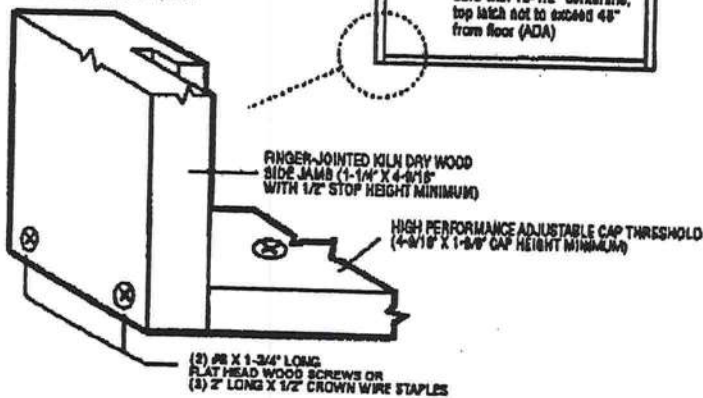


(3) FOR 7'0\"/>

Latching Hardware

- 6'0\"/>
 - Compliance requires double bore with 5-1/2\"/>
- 8'0\"/>
 - Compliance requires double bore with 10-1/2\"/>

TYPICAL THRESHOLD & SIDE JAMB ATTACHMENT

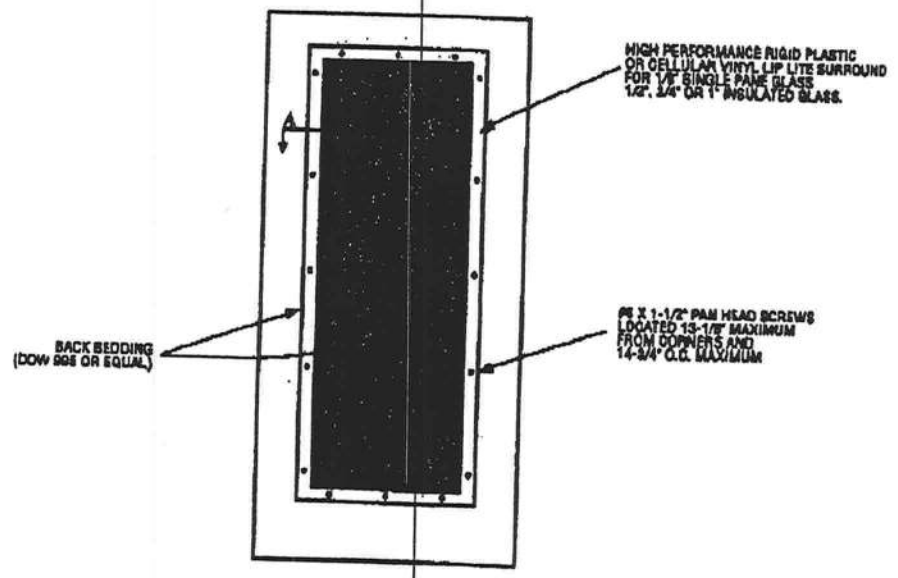


Test Data Review Certificate
#2028447A; #2028447B; #2028447C
and COP/Test Report Validation Matrix
#2028447A-001, 002, 003, 004;
#2028447B-001, 002, 003, 004;
#2028447C-001, 002, 003, 004
provides additional information
available from the ITR/WT website
(www.warrick.com), the Masonite
website (www.masonite.com) or the
Masonite technical specs.

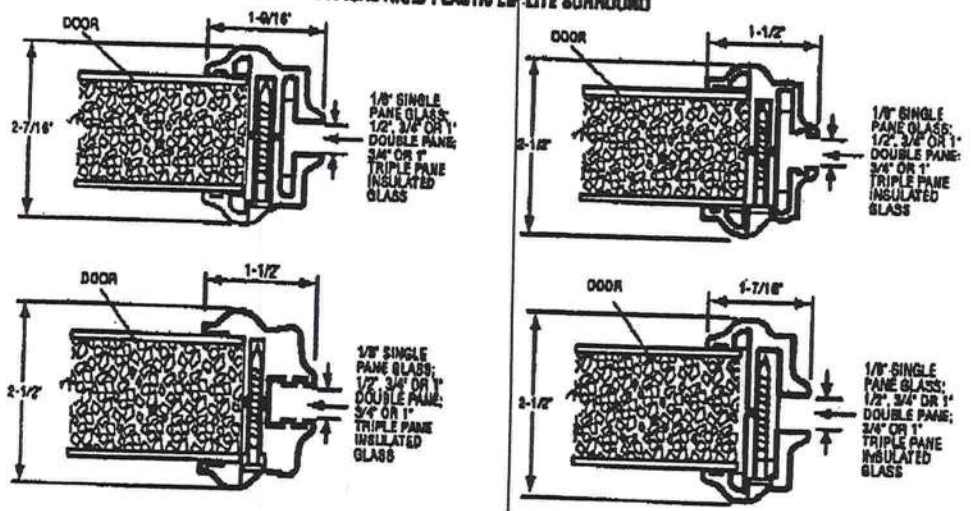
October 14, 2002
Our continuing program of product improvement means specifications,
designs and product details subject to change without notice.

Masonite

MAD-WI-MA0041-02
GLASS INSERT IN DOOR
OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP-LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Sanjco or approved validation service.

Warrick-Horsey Test Data Review Certificate #3028447A; #3028447B; #3028447C and COP/THI Report Validation Matrix #3028447A-001, 002, 003; #3028447B-001, 002, 003; #3028447C-001, 002, 003 provides additional information - available from the IFS/WH website (www.ifswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

JUNE 17, 2002
 Our continuing program of product improvement means specifications, design and product detail subject to change without notice.

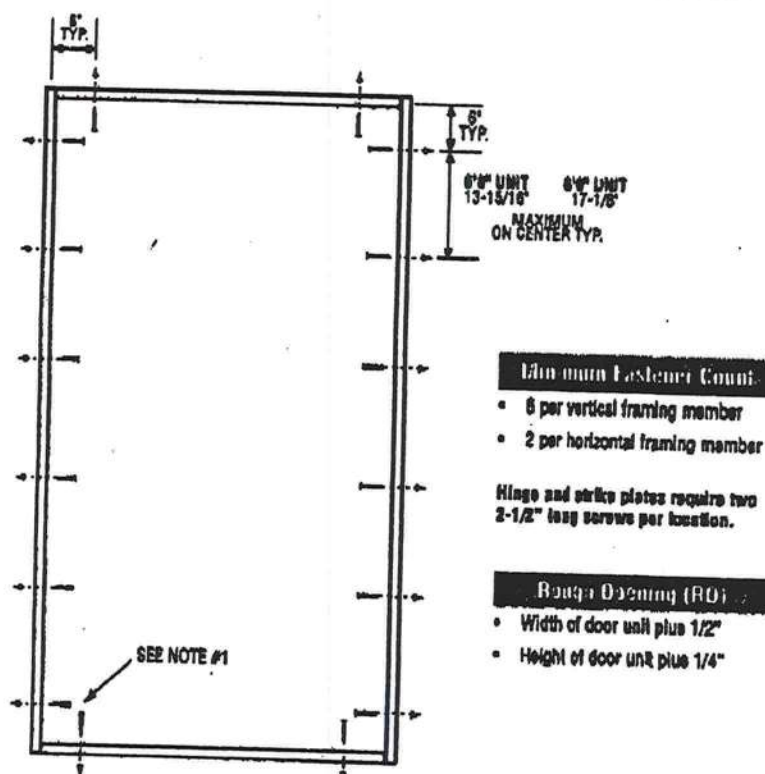
PREMIER Collection
Plasma Quality Series

Masonite
Masonite International Corporation

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Warbook Nursery Test Data Review Certificate #3026447A; #3026447B; #3026447C and CDR Test Report Validation Matrix additional information - available from the ITSA/WH website (www.itsaonline.com), the Muscarelli website (www.muscarelli.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246°, 0265°, 0261°, 0245, 0261° or 0266**
Compliance requires that 6" GRADE 1 (ANSI/BHMA A156.10) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/APA & PANDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2083
Our continuing program of product improvement makes specifications,
design and product detail subject to change without notice.

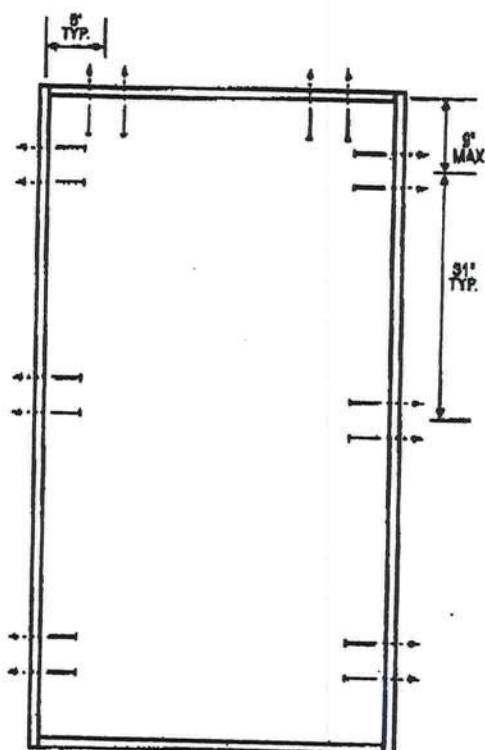


Masonite

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0\" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Hinge and strike plates require two 2-1/2\" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Interlock Survey Test Data Review Certificates #3028447A, #3028447B, #3028447C and COP/Text Report Validation Matrix #3028447A-001, 002, 003, 004; #3028447B-001, 002, 003, 004; #3028447C-001, 002, 003, 004 provides additional information - available from the ITB/WH website (www.steelcase.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0248*, 8285*, 3241*, 3248, 3291* or 3298**
Compliance requires that 8\" GRADE 1 (ANSI/BHMA A156.18) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

*Based on required Design Pressure - see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/APA NDS for southern pine lumber with a side member thickness of 1-1/4\" and achievement of minimum embedment of 1-1/4\".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 18, 2003
Our continuing program of product use/overhaul makes specifications, details and product detail subject to change without notice.

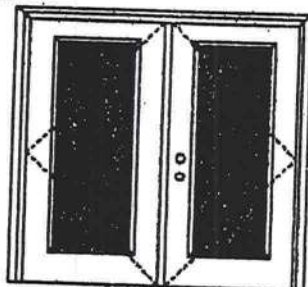
 **Masonite**

XX Glazed Outswing Unit

COP-WL-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #20284470
and COP/72nd Report Validation Matrix
#20284470-021 are also available
information - available from the IFMA/NA
website (www.ifma-na.com), the
Masonite website (www.masonite.com)
or the Masonite technical center.

Note:
Units of other sizes are covered by this
report as long as the panels used do not
exceed 3'0" x 6'8".

Double Door
Maximum unit size - 6'0" x 6'8"

Design Pressure

+50.5/-50.5

Limited water urbane special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national,
state or local building codes specify the action required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0012-02 and
MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 139 Series



135 Series



680 Series



622 Series

1/2 GLASS:



105 Series*



106, 140 Series*



129 Series*



200 Series*

12 R/L, 23 R/L, 24 R/L
Series*

147 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with sash; System 6-panel; System 6-panel with sash.

Entergy
Entry Systems

June 17, 2002
Determining program of product improvements makes specifications, design and product
cost subject to change without notice.



Exclusively from
Masonite
Masonite International Corporation

XX

Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS**APPROVED DOOR STYLES:****3/4 GLASS:**

404 Series



418 Series



430 Series

FULL GLASS:

100 Series



114, 120, 122 Series



132 Series



148 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested In Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



Test Data Review Certificate #3028447C
and COP/Test Report Verification Matrix
#3028447C-081 provide necessary
information - available from the ITSAWH
website (www.itsawh.com), the
Masonite website (www.masonite.com)
or the Masonite technical center

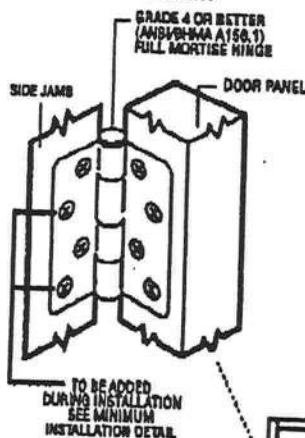
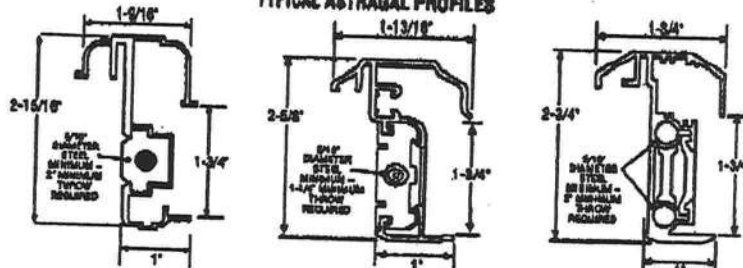
Entergy
Entry Systems

June 17, 2003

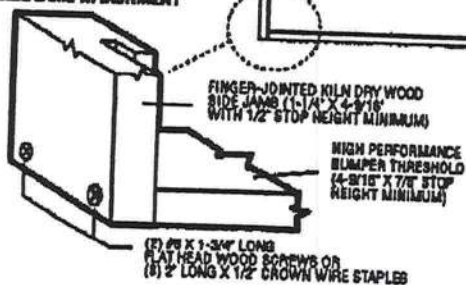
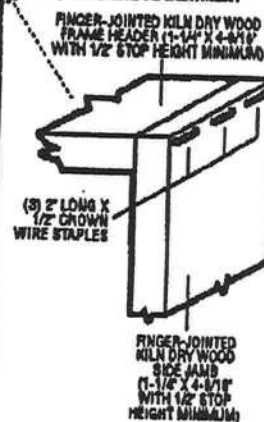
Our licensing program of product improvements matrix specifications, design and product
detail subject to change without notice.



Produced by
Masonite
Masonite International Corporation

XX
UnitMAD WL MA0012-02
**OUTSWING UNITS WITH
DOUBLE DOOR****TYPICAL HINGE ATTACHMENT****TYPICAL ASTRAGAL PROFILES**

ALUMINUM EXTRUDED ASTRAGAL (1.0" MINIMUM WALL THICKNESS) WITH ADDED REINFORCEMENT INSERTS AT TOP EXTENSION BOLT, BOTTOM EXTENSION BOLT AND CYLINDRICAL/DEADBOLT LATCHING LOCATIONS. ATTACH WITH #6 X 1" PAN HEAD SCREWS - LOCATE 1" FROM EACH END MINIMUM AND 2" O.C. MAXIMUM.

TYPICAL THRESHOLD & SIDE JAMB ATTACHMENT**TYPICAL HEADER & SIDE JAMB ATTACHMENT**

(3) FOR 7'0" HEIGHT OR SMALLER
(4) FOR HEIGHTS GREATER THAN 7'0"

- Latching Hardware**
- 6'8" Unit**
- Compliance requires double bore with 5-1/2" centerline, top latch not to exceed 48" from floor (ADA)
- 8'0" Unit**
- Compliance requires double bore with 10-1/2" centerline, top latch not to exceed 48" from floor (ADA)

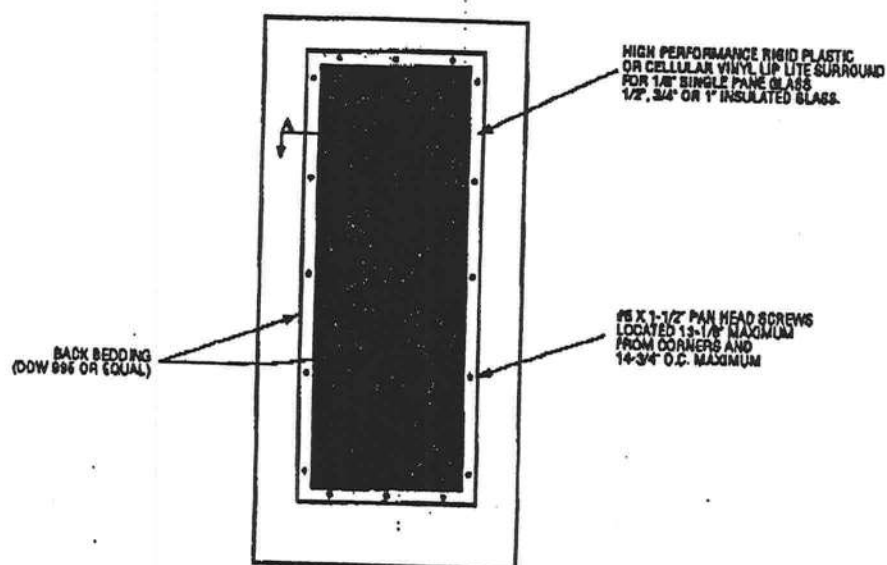
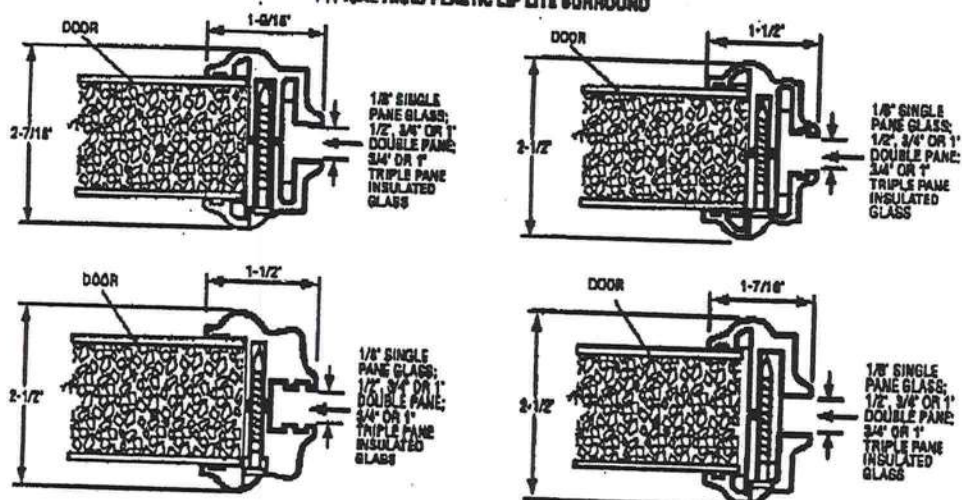
Watersmark History
W.H.

Test Data Review Certificate
#3026447A; #3026447B; #3026447C
and COPR Report Validation Matrix
COPR#447A-001, 002, 003, 004;
#3026447B-001, 002, 003, 004;
#3026447C-001, 002, 003, 004
provide additional information -
available from the ITS/WH website
(www.stormco.com), the Watersmark
website (www.watersmark.com) or the
Watersmark technical center.

October 14, 2002

Our continuing program of product improvement makes specifications, design and product
must subject to change without notice.

MAD-WI-MA0041-02

**GLASS INSERT IN DOOR
OR SIDELITE PANEL****SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND**

*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Masonite Warranty Test Data Review Certificates #3029447A; #3029447B; #3029447C and COP/Text Report Validation. Masonite #3029447A-Q01, Q02, Q03; #3029447B-Q01, Q02, Q03; #3029447C-Q01, Q02, Q03 provides additional information - available from the IF&W website (www.masonite.com), the Masonite website (www.masonite.com) or the Masonite technical dealer.

JUNE 17, 2002
Our continuing program of product improvement means specifications, design and product detail are subject to change without notice.

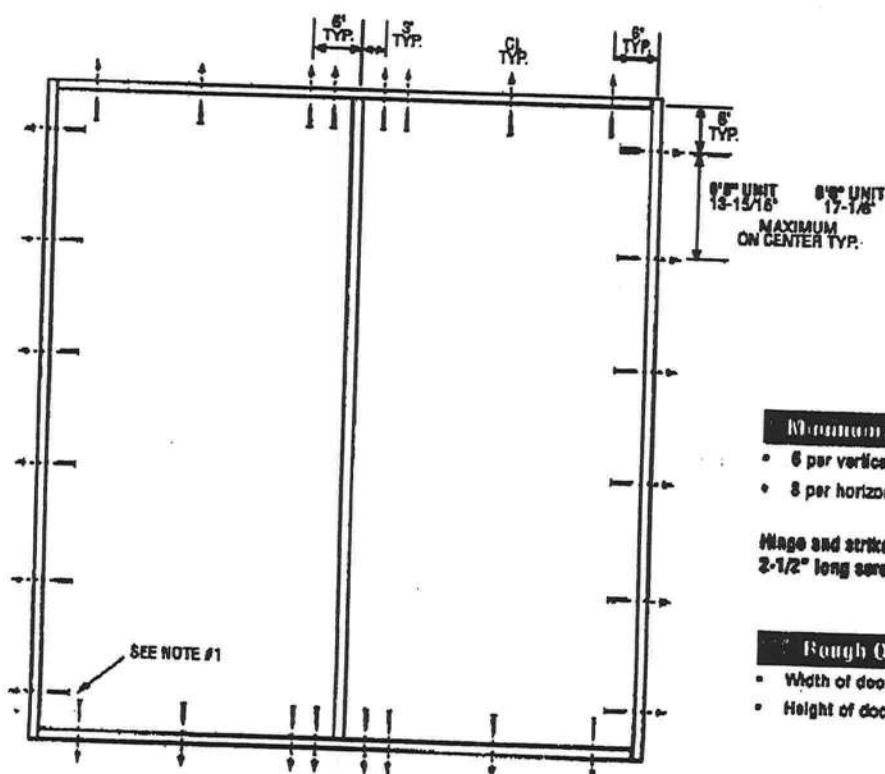
PRENDORF Masonite
Premium Quality Doors

Exclusively from
Masonite
Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Masonite Masonry TYPICAL CODE REVIEW CYCLES: #3025447A; #3025447B; #3025447C and COP/TEST REPORT VALIDATION MATRIX #3025447A-001, 002, 003, 004; #3025447B-001, 002, 003, 004; #3025447C-001, 002, 003, 004 provides additional information - available from the ITW/WH website (www.elcombe.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 8247*, 8287*, 8242*, 8247, 8282* or 8287**
Compliance requires that 6" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

*Based on required Design Pressure - see COP sheet for details.

Notes:

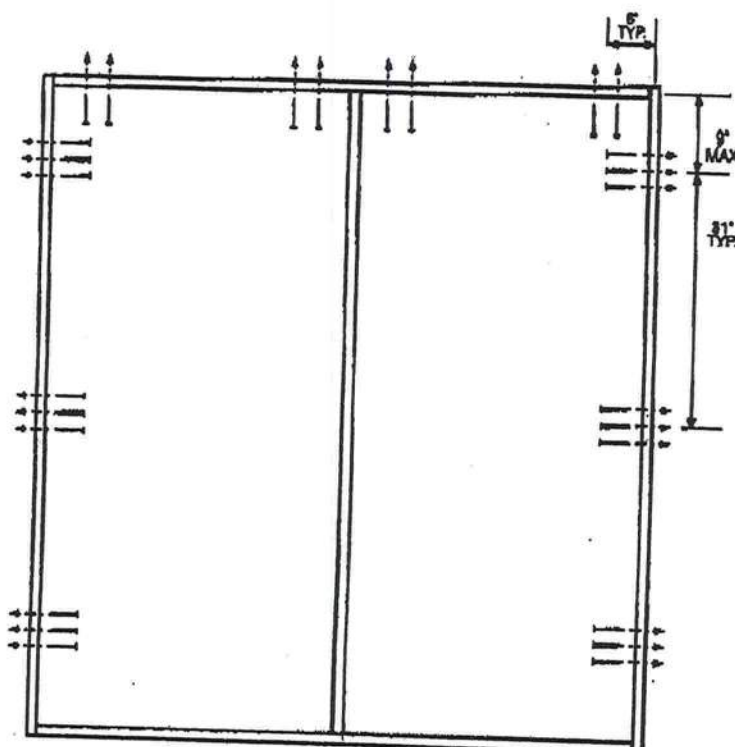
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANS/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 16, 2003
Our continuing program of product improvement makes specifications, designs and product data subject to change without notice.

Masonite

XX
Unit

MID-WL MADO02 U2

DOUBLE DOOR**Minimum Fastener Count**

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warrick Masonite This Data Review Certificate #3025447A, #3025447B, #3025447C and COP/TEST REPORT WARRICK MASONITE #3025447A-001, 002, 003, 004; #3025447B-001, 002, 003, 004; #3025447C-001, 002, 003, 004 provides additional information - available from the ITB/MSI website (www.itbmsi.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

*Based on required Design Pressure - see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/APA & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003
Our continuing program of product improvement makes specifications, design and product details subject to change without notice.



MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 SINGLE HUNG (FIN & FLANGE)
165 SINGLE HUNG (FIN & FLANGE)
BB165/740/744 FIXED (FIN & FLANGE)

- Test Reports
 - 165 Single Hung
 - #CTLA-787W (Fin)
 - #CTLA-787W-1 (Flange)
 - 740/744 Single Hung
 - #01-40351.03 (Fin)
 - #01-40351.04 (Flange)
 - 165/740/744 Fixed
 - #NCTL-310-0005-2.1 (Fin)
 - # NCTL-310-0005-5.1 (Flange)
 - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels



**AAMA/NWDA 101/LS2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744

TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft ²
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:baw



Allen N. Reeves
15 FEBRUARY 2002

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS,
EXPOSURE "B" (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST),
AND **WALL ZONE "5"** (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE*
DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT.
THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- | | |
|---------------|--------------------|
| • 2'- 6" WIDE | DP + 40.0 / - 55.4 |
| • 3'- 0" WIDE | DP + 40.0 / - 48.5 |
| • 4'- 0" WIDE | DP + 40.0 / - 40.3 |

THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND
AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT
IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT
MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO
MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686



DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.



Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance

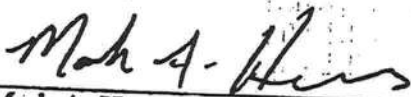
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.

* Exceeds L/175 for deflection, but meets all other test requirements.

4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:


Mark A. Hess
Technician

MAH:baw
01-40351.03


Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002



Test Specimen Description: (Continued)**Drainage:** Sloped sill**Reinforcement:** No reinforcement was utilized.**Installation:** The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.**Test Results:**

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft ²	0.30 cfm/ft ² max.
<i>Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/I.S. 2-97 for air infiltration.</i>			
2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.86"* 0.81"*	0.29" max. 0.29" max.
<i>Note: * Exceeds L/175 for deflection, but meets all other test requirements.</i>			
2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" <0.01"	0.20" max. 0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	
	Right stile	0.03"/6%	

Allen N. Reeves
15 FEBRUARY 2002

Test Specimen Description: (Continued)**Weatherstripping:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002





AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATT) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethtown, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

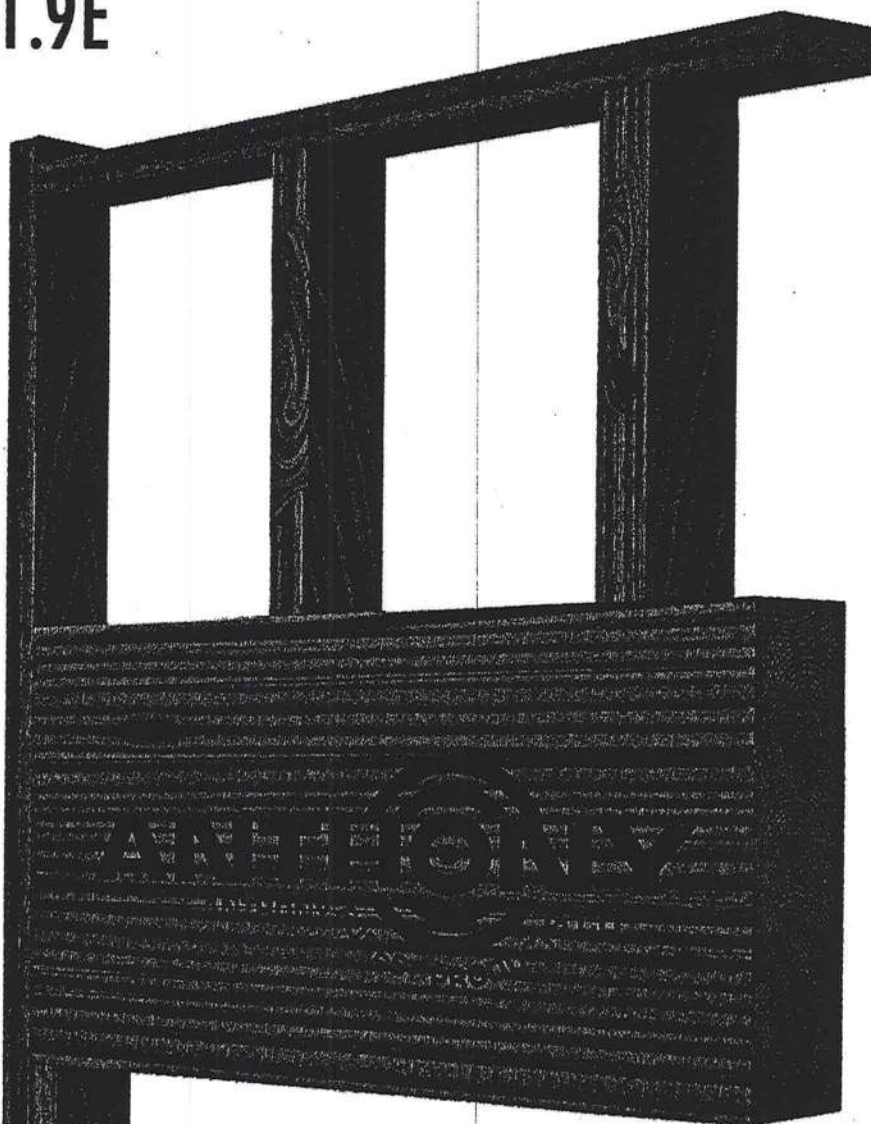
130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testatl.com



Allen M. Reeves

Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- ◆ Less Expensive than LVL or PSL
- ◆ Cambered or Non-cambered
- ◆ Lighter than Steel, LVL or PSL
- ◆ 3-1/2" Width to Match Framing
- ◆ Pre-Cut Lengths
- ◆ One Piece - No Nail Laminating
- ◆ Renewable Resource
- ◆ Lifetime Warranty

**Garage Header
Sizing Tables**

ANTHONY
ANTHONY FOREST PRODUCTS CO.

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY
HEADER SUPPORTING: 1/2 ROOF SPAN

SNOW LOAD AREAS USING DEFLECTION FACTOR = 1.5																	
9'-3"			16'-3"			18'-3"			9'-3"			16'-3"			18'-3"		
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	16-3/4
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	9-3/4	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8		9-3/4		
8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

NON-SNOW LOAD AREAS USING DEFLECTION FACTOR = 1.5																	
9'-3"			16'-3"			18'-3"			9'-3"			16'-3"			18'-3"		
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8
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8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8				

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.

2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.

3. Deflection is limited to L/240 for live load and L/180 for total load.

4. Headers are assumed to have continuous lateral support along top edge.

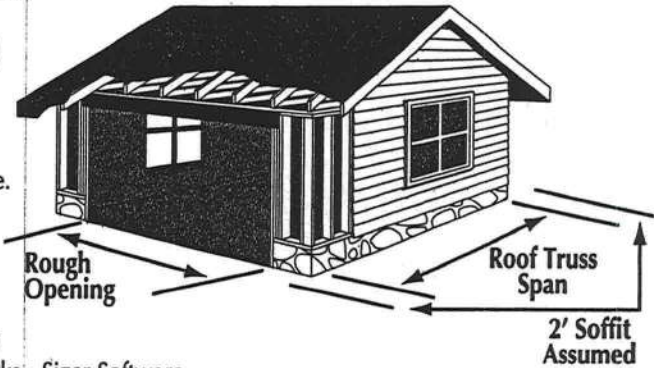
5. Bearing length based on full width bearing is indicated as follows:

Non-shaded sizes require two trimmers (3" bearing).

Shaded sizes require three trimmers (4.5" bearing).

Shaded & outlined sizes require four trimmers (6" bearing).

6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP 30F_b POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 115% SNOW LOAD AREA											
ROUGH OPENING SIZE	12"	14"	16"	18"	20"	22"	24"	26"	28"	30"	32"
8'0"	844	896	1216	1573							
8'6"	161	207	254	330	390	510	552	669	752	824	
9'0"	114	145	180	231	277	359	391	510	534	653	707 789

GARAGE HEADER SUPPORTING ROOF LOADS ONLY - 125% NON-SNOW LOAD AREA											
ROUGH OPENING SIZE	12"	14"	16"	18"	20"	22"	24"	26"	28"	30"	32"
8'0"	844	975	1322								
8'6"	161	207	254	330	390	510	552	724	752	897	
9'0"	114	145	180	231	277	359	391	510	534	699	693

GARAGE HEADER SUPPORTING ROOF, WALL AND FLOOR LOADS - 100% LOAD DURATION													
ROUGH OPENING SIZE	12"	14"	16"	18"	20"	22"	24"	26"	28"	30"	32"	34"	36"
8'0"	562	778	888	1056	1363	1367		1582					
8'6"	107	153	169	245	260	380	368	540	501	715	664	864	840
9'0"	76	107	120	171	185	267	261	380	356	521	471	684	609 813

NOTES:

- 1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
- 2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
- 3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
- 4. When no live load is listed, total load controls.
- 5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):	FLEXURAL STRESS (F _b) =	2600
	COMPRESSION PERP. TO GRAIN (F _{c⊥}) =	740
	HORIZONTAL SHEAR (F _v) =	225
	MODULUS OF ELASTICITY (MOE) =	1.9 x 10 ⁶

Section Size (inches)	7.7	9.0	10.4	11.7	12.9	14.2	15.5
Weight (lb/ft)	326	514	789	1115	1521	2014	2604
Moment Capacity (ft-lb)	8865	12015	15996	20145	24772	29877	35460
Load Capacity (lb)	3908	4550	5250	5892	6533	7175	7817

NOTES:

- 1. Beam weights are based on 38 pcf.
- 2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
- 3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
- 4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
- 5. The AITC NER 466 was used in calculating the above allowable design stresses for POWER HEADER®.

GARAGE HEADER COMPARISONS

Section Size (inches)	810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"***
	990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"***
	640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"
	765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"
	750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"
	900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

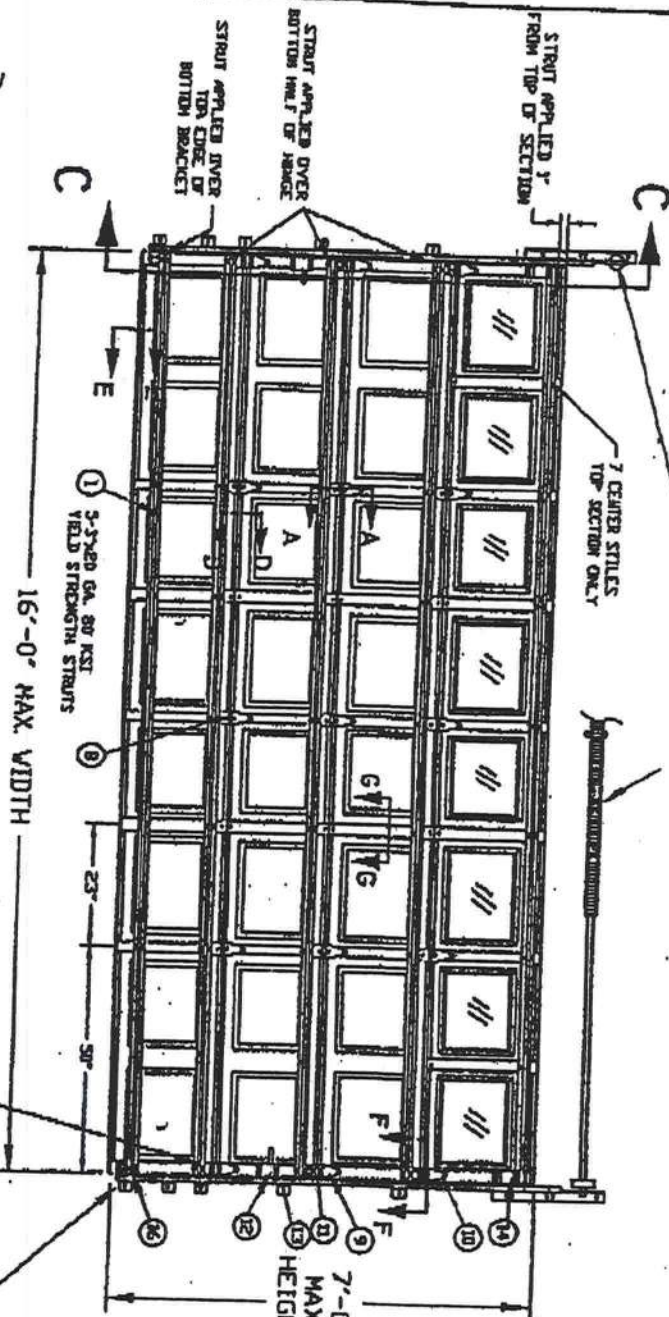
For more information on POWER HEADER®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

POWER HEADER® is a trademark of
Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: [http:// www.anthonyforest.com](http://www.anthonyforest.com)
e-mail: info@anthonyforest.com
© 2001 Anthony Forest Products Company

Distributed by:

- M.
1. TESTED TO POSITIVE AND NEGATIVE 20 PSF SECTION AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
 2. MAXIMUM SECTION HEIGHT - 27'
 3. SECTION HEIGHTS OF 24" AND 30" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS
 4. JOISTS MAY BE INSTALLED IN THE TOP SECTION, BUT NOT TESTED WITH 1/2" RIG GLASS OR EQUIVALENT, OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
 5. MINIMUM LENGTH OF HOLLOW STUD IS 5'-0" AS TESTED
 6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SCHEDULE
 7. STRUTS SECURED AT ALL LOCATIONS WITH TEST SCREWS
 8. QUANTITY OF SIDE LOCKS CAN BE AL OR G2 AS TESTED
 9. A 300# IN TYPE OF INSULATION IS OPTIONAL.

NOT PART OF VARIOUS SYSTEM
EXTENDED SPRING COUNTERBALANCE
TORSION SPRING COUNTERBALANCE



INSIDE ELEVATION

ALL ROLLER CARRIERS
AND HOOKS ARE 14 GA.

12 GA. JAMB BRACKETS, MAXIMUM SPACING = 19-1/2" WITH
LOWEST BRACKET APPROX. 3" FROM FLOOR, END BRACKET
NEAR THE HORIZONTAL C OF THE BOTTOM SECTION, AND 3RD
BRACKET NEAR THE TOP OF THE BOTTOM SECTION

SEC C-C

VERTICAL
TRACK, G6 GA.

The seal on this drawing only
certifies that the product(s)
illustrated and described herein
conform(s) to the installation(s)
and dimensions and configurations
of the door as tested.



REPORT NO. 2202

TEST REPORTS IN FILE VIDEO 10/19/00 400230

GALCO DOORS

SERIES 7400, EXTERIOR STEEL, 407 MIN GAS TESTED
SERIES 7023, EXTERIOR STEEL, 419 MIN GAS TESTED
SERIES 7024, EXTERIOR STEEL, 4024 MIN GAS TESTED
TESTED WITH VARIOUS

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CEN. STILE SPACING	STRUTS 80 KSI SIZE	VERTICAL BRACK.
16'	7'	23"	3"	5
				2 IN.

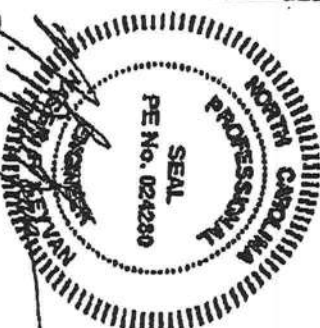


GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60038

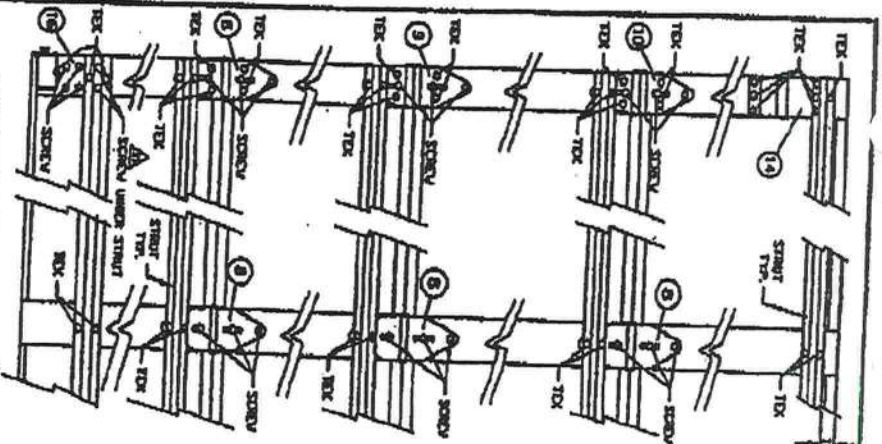
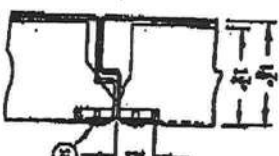
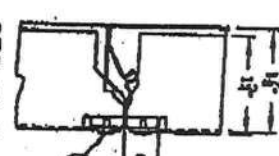
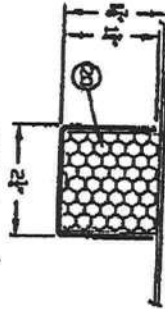
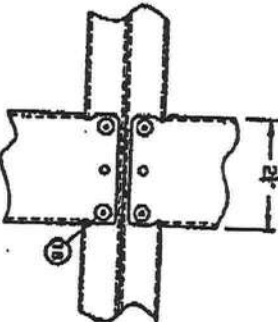
DESIGN LOAD +200 PSF & -200 PSF
TEST LOAD +300 PSF & -300 PSF
SCHEDULE 30
DOOR 11-20-00
REVISION (A) 11-20-00
16" x 7" MAX RAISED PANEL, STEEL DOOR - UNLOADED +20 PSF
PART NUMBER: V13220-1
PAGE 1 OF 2

REV.	DATE	BY	CHK.
A-1	11-20-00	bw	EC, CCM, BT

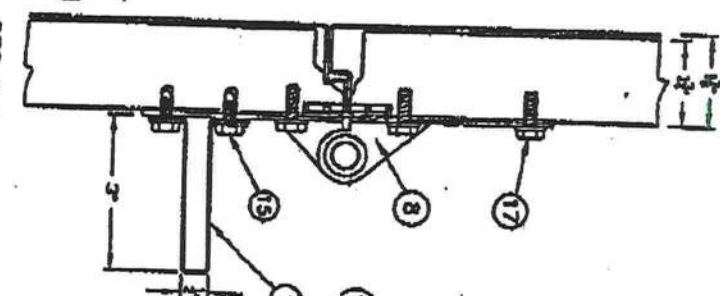
The seal on this drawing, only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.



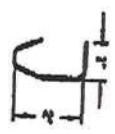
FASTENER ARRANGEMENT A

SEC. D-D
PAN ATTACHMENT
TO STILE
AS TESTEDSEC. D-D
PAN ATTACHMENT
TO STILE
OPTIMUMSEC. G-G
CENTER STILE
20 GA. GALV. VAMIZERPAN ATTACHMENT
TO STILE

SEC. A-A



REV.	DATE	BY	DESCRIPTION
1	11-27-03	WJ	REVISED TO ADD 1/2\"
2	11-27-03	WJ	REVISED TO ADD 1/2\"
3	11-27-03	WJ	REVISED TO ADD 1/2\"
4	11-27-03	WJ	REVISED TO ADD 1/2\"

TRACK
16 GA. CROSS PIN

SIDE LOCK



ROLLER IN BALLS

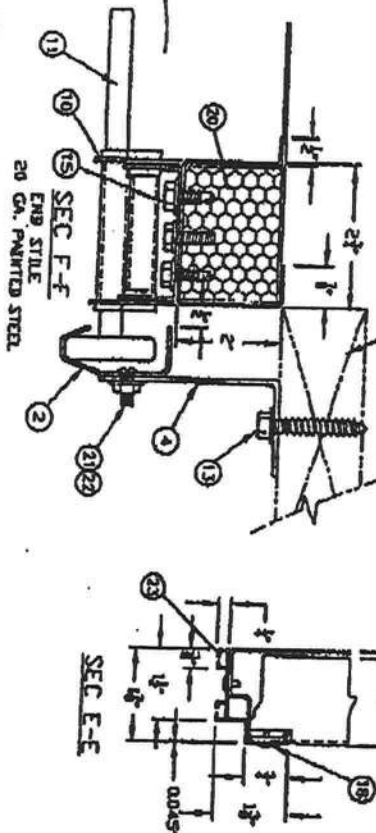


14. ROLLER IN BALLS

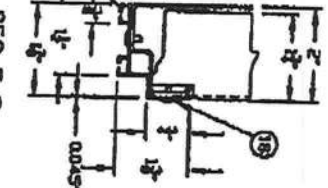
5-7/8\" GA. OR RES. TITLS
STRENGTH REDUCED
IF TEX SCREWS FOR HINGE
OR STILE LOCATION
ON FOR STIFF, HANDING



15. SCREW
1-20 x 3\"
HEX WASHER-HEAD
SCREW
1-20 x 3\"
HEX TEX SCREW
WITH R2
REDUCED POINT

20 GA. PRESSURE TREATED LUMBER
(GRADE #2 OR BETTER SOUTHERN PINE)END STILE
20 GA. PAINTED STEEL

SEC. E-E



GENERAL AMERICAN DOOR COMPANY
5000 BASELINE ROAD
MONTGOMERY, AL 36103

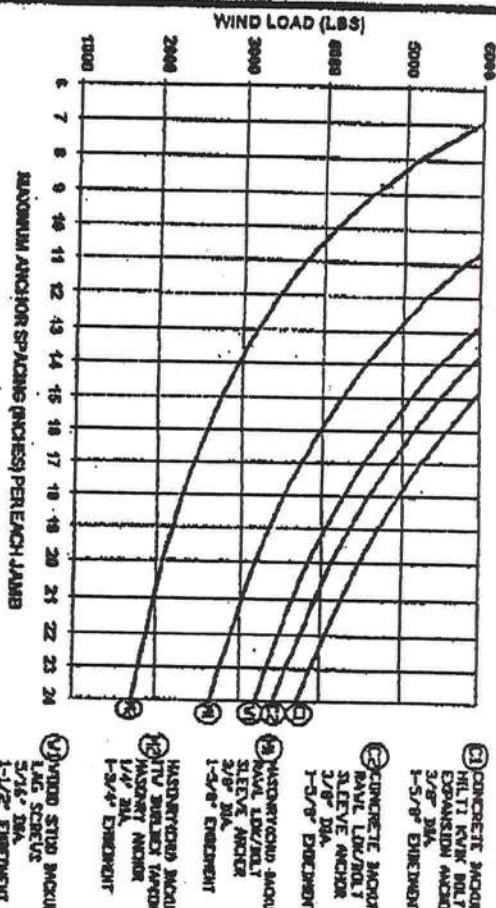
DATE: 11-27-03
REVISED BY: J. VILKINS
REVISED (3) 11-27-03

BY: X 7 MAX RANDED PANEL, STILE, ROOF-VAMIZER 220 PSF

PROD. 2 OF 2

REVISED TO ADD 1/2\"

WIND LOAD VS. ANCHOR SPACING



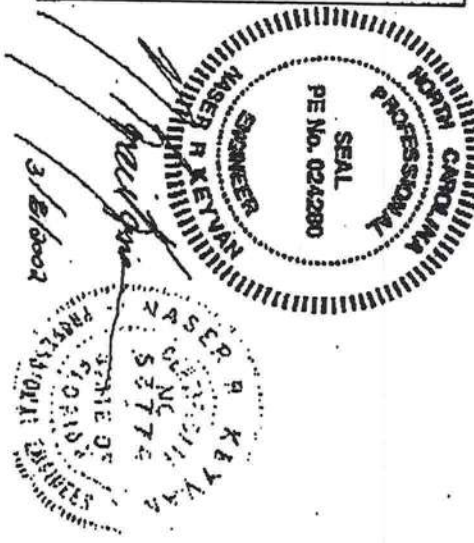
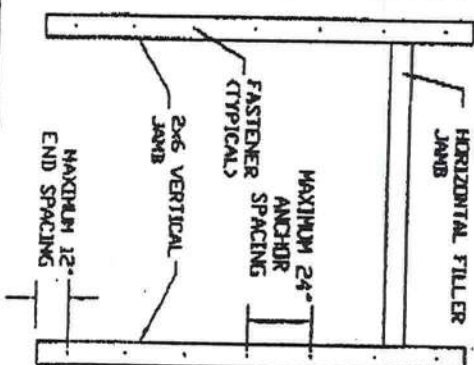
DESIGN (LBS) X GARAGE DOOR AREA (WIDTH-FT X HEIGHT-FT) = WIND LOAD (LBS)
LOAD FT²

MAXIMUM ANCHOR SPACING (INCHES) PER EACH JAMB

EXAMPLE:
30 LBS X 66 FT WIDE X 8 FT HIGH = 3960 LBS
FT²

- ① USE 22" SPACING
- ② USE 21" SPACING
- ③ USE 19" SPACING
- ④ USE 16" SPACING
- ⑤ USE 10" SPACING

SEE NOTE #1 FOR ADDITIONAL REQUIRED 2x6 WOOD JAMB ANCHORS



2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED GRADE #2 OR BETTER SOUTHERN PINE WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

- 1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.
- 2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SPECI. STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION SSTO 10, CURRENT EDITION.
- 3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, DIRECTIONS AND RECOMMENDATIONS.
- 4) WOOD FRAME BUILDINGS STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM GRADE OR BETTER WALL STUDS CONTINUOUS FROM FLOORING TO DOUBLE TOP PLATE.
- 5) REINFORCED CMU OR CONCRETE 2x6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS. OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON ENCASED MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2500 PSI. GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI. REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
- 6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.
- 7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4".
- 8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2x6 WALL STUDS.
- 9) WASHERS ARE REQUIRED ON ALL FASTENERS.
- 10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 10' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.
- 11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2x6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2x6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2x6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY	
3000 BASSETT ROAD MONTICHERY, IL 60538	
SALES NINE	SALES NINE
DATE 8-20-99	DATE 8-20-99
REVISION JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS	REVISION JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS
DATE 8-20-99	DATE 8-20-99
BY A10560	BY A10560



ELK



**PRESTIQUE®
HIGH DEFINITION®**



RAISED PROFILE™

**Prestique Plus *High Definition*
and Prestique Gallery Collection™**

Product size	13 1/4" x 39 3/4"	50-year limited warranty period:
Exposure	5 1/2"	non-prorated coverage for
Pieces/Bundle	16	shingles and application labor for
Bundles/Square	4/98.5 sq.ft.	the initial 5 years, plus an option
Squares/Pallet	11	for transferability*; prorated
		coverage for application labor and
		shingles for balance of limited
		warranty period; 5-year limited
		wind warranty*.

Raised Profile

Product size	13 1/4" x 38 3/4"	30-year limited warranty period:
Exposure	5 1/2"	non-prorated coverage for
Pieces/Bundle	22	shingles and application labor for
Bundles/Square	3/100 sq.ft.	the initial 5 years, plus an option
Squares/Pallet	16	for transferability*; prorated
		coverage for application labor and
		shingles for balance of limited
		warranty period; 5-year limited
		wind warranty*.

Prestique I *High Definition*

Product size	13 1/4" x 39 3/4"	40-year limited warranty period:
Exposure	5 1/2"	non-prorated coverage for
Pieces/Bundle	16	shingles and application labor for
Bundles/Square	4/98.5 sq.ft.	the initial 5 years, plus an option
Squares/Pallet	14	for transferability*; prorated
		coverage for application labor and
		shingles for balance of limited
		warranty period; 5-year limited
		wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/FLX™

Size: 12" x 12"
Exposure: 6 1/2"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Prestique *High Definition*

Product size	13 1/4" x 38 3/4"	30-year limited warranty period:
Exposure	5 1/2"	non-prorated coverage for
Pieces/Bundle	22	shingles and application labor for
Bundles/Square	3/100 sq.ft.	the initial 5 years, plus an option
Squares/Pallet	16	for transferability*; prorated
		coverage for application labor and
		shingles for balance of limited
		warranty period; 5-year limited
		wind warranty*.

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakeswood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood.
Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

*See actual limited warranty for conditions and limitations.
**Check for product availability.

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

Project Information for: L135114
Builder: HUGO ESCALANTE **Date:** 10/26/2005
Lot: LOT 37 WISE ESTATES **Start Number:** 1091
Subdivision: N/A
County or City: COLUMBIA COUNTY
Truss Page Count: 40

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2
Gravity **Wind** **Building Code:** FBC2004
Roof (psf): 42 Wind Standard: ASCE 7-02
Floor (psf): 55 Wind Speed (mph): 120

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)
ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 **Designer:** 28

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for vertical loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1026051091	10/26/2005				
2	CJ3	1026051092	10/26/2005				
3	CJ5	1026051093	10/26/2005				
4	EJ7	1026051094	10/26/2005				
5	EJ7A	1026051095	10/26/2005				
6	EJ7B	1026051096	10/26/2005				
7	EJ7G	1026051097	10/26/2005				
8	EJ7GA	1026051098	10/26/2005				
9	HJ9	1026051099	10/26/2005				
10	T01	1026051100	10/26/2005				
11	T01G	1026051101	10/26/2005				
12	T02	1026051102	10/26/2005				
13	T03	1026051103	10/26/2005				
14	T04	1026051104	10/26/2005				
15	T05	1026051105	10/26/2005				
16	T06	1026051106	10/26/2005				
17	T07	1026051107	10/26/2005				
18	T08	1026051108	10/26/2005				
19	T09	1026051109	10/26/2005				
20	T10	1026051110	10/26/2005				
21	T11	1026051111	10/26/2005				
22	T12	1026051112	10/26/2005				
23	T13	1026051113	10/26/2005				
24	T14	1026051114	10/26/2005				
25	T15	1026051115	10/26/2005				
26	T16	1026051116	10/26/2005				
27	T17	1026051117	10/26/2005				
28	T18	1026051118	10/26/2005				
29	T19	1026051119	10/26/2005				
30	T20	1026051120	10/26/2005				
31	T21	1026051121	10/26/2005				
32	T22	1026051122	10/26/2005				
33	T23	1026051123	10/26/2005				
34	T24	1026051124	10/26/2005				
35	T25	1026051125	10/26/2005				
36	T26	1026051126	10/26/2005				
37	T27	1026051127	10/26/2005				
38	T28	1026051128	10/26/2005				
39	T29	1026051129	10/26/2005				
40	T29G	1026051130	10/26/2005				

OCT 26 2005

Dwg.#1026051090

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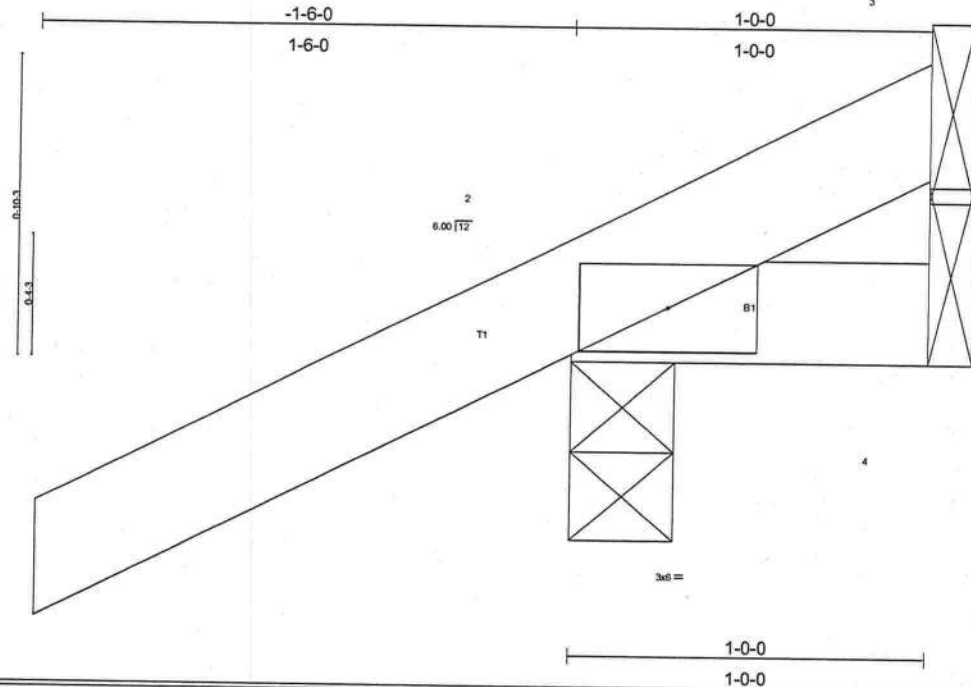
Name: **ESCALANTE, HUGO (Primary Name)**
EWPL INC (DBA Name)
Main Address: **P.O. BOX 280**
FORT WHITE, Florida 32038

License Information

License Type: **Certified Residential Contractor**
Rank: **Cert Residential**
License Number: **CRC1326967**
Status: **Current, Active**
Licensure Date: **11/24/2003**
Expires: **08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.19	l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(LL) -0.00 2 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) -0.00 2 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.00 3 n/a n/a		
				Weight: 6 lb	

BRACING

TOP CHORD	Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

(lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-220(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=73(load case 5)

TOP CHORD 1-2=0/35, 2-3=-45/41
BOT CHORD 2-4=0/0

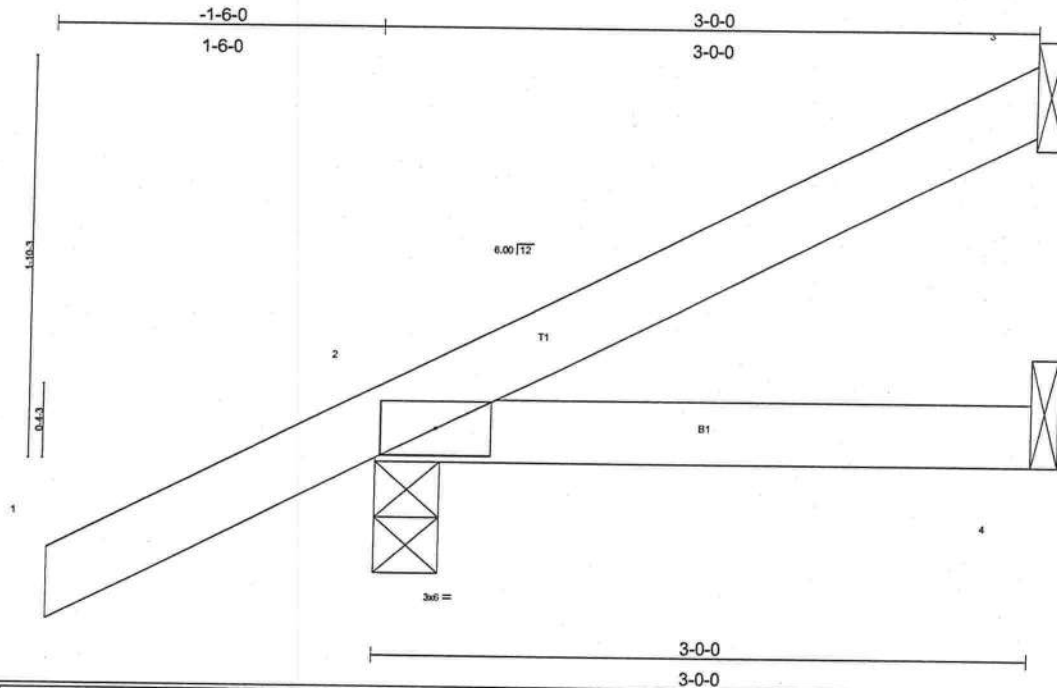
1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDF=4.2psf; BCDF=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Refer to girder(s) for truss to truss connections.

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2 and 40 lb uplift at joint 3.

OCTOBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job L135114	Truss CJ3	Truss Type MONO TRUSS	Qty 8	Ply 1	HUGO ESCALANTE-LOT 37 WISE ESTATES
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:21:26 2005 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.21	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.06	Vert(LL) -0.00 2-4 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.01 2-4 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
	Code FBC2004/TPI2002			Weight: 12 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=137(load case 5)
Max Uplift 3=-47(load case 5), 2=-187(load case 5)

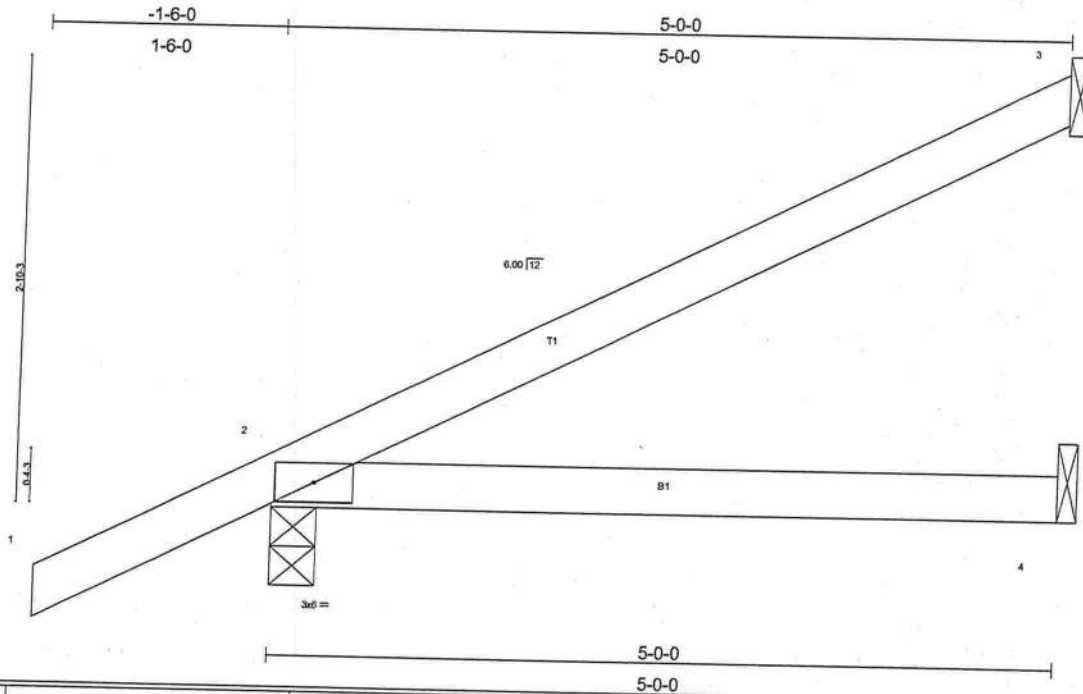
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-52/16
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02: 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed: MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 187 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Dwg.#1026051093
L135114	CJ5	MONO TRUSS	8	1	HUGO ESCALANTE-LOT 37 WISE ESTATES
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)
6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:21:30 2005 Page 1					



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.30	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.16	Vert(LL) -0.03 2-4 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.05 2-4 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
	Code FBC2004/TPI2002				
				Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=192(load case 5)
Max Uplift 3=124(load case 5), 2=197(load case 5)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-112/41
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 197 lb uplift at joint 2.

LOAD CASE(S) Standard

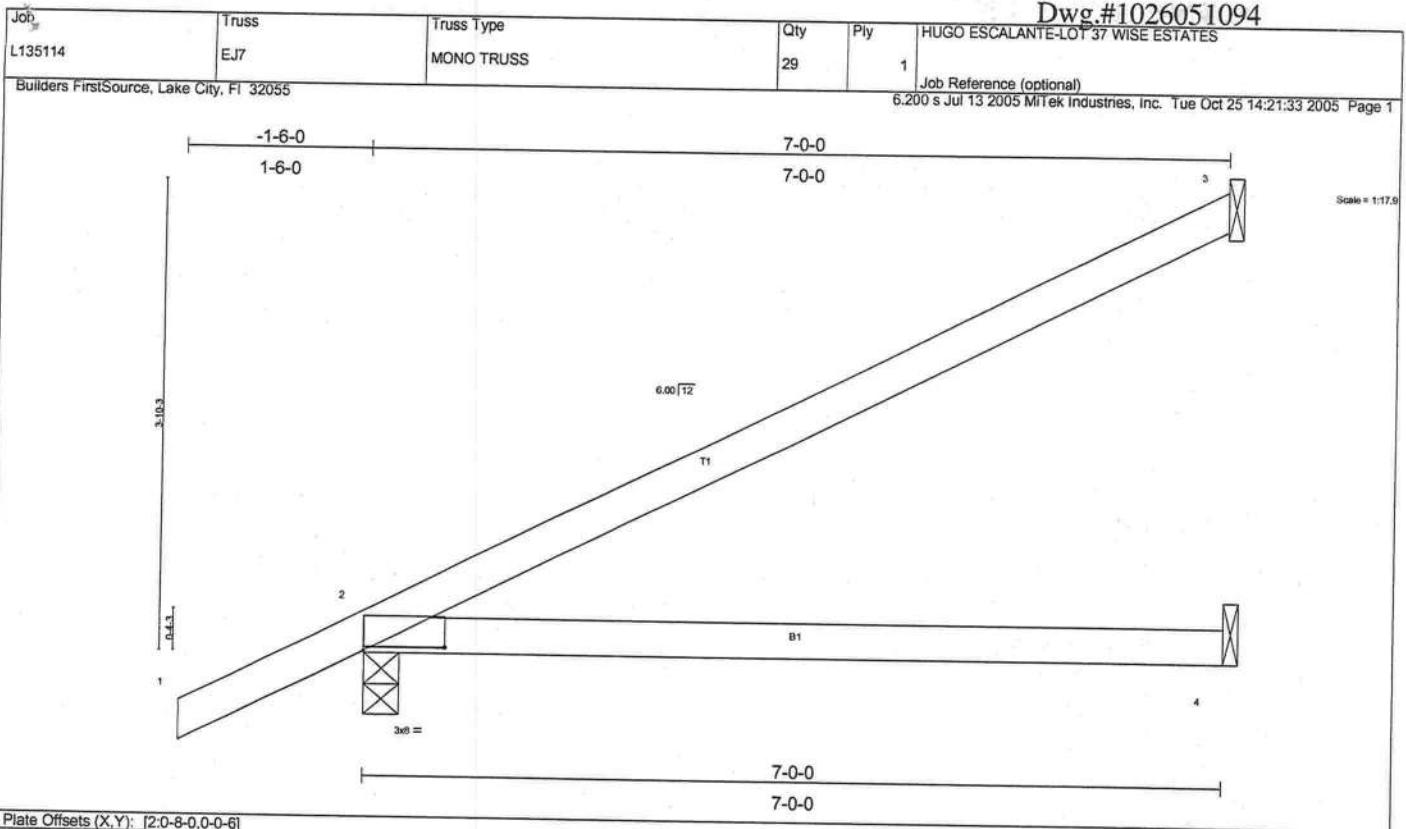


Plate Offsets (X,Y): [2-0-8-0,0-0-6]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.52	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 25 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=247(load case 5)
Max Uplift 3=170(load case 5), 2=217(load case 5), 4=1(load case 5)

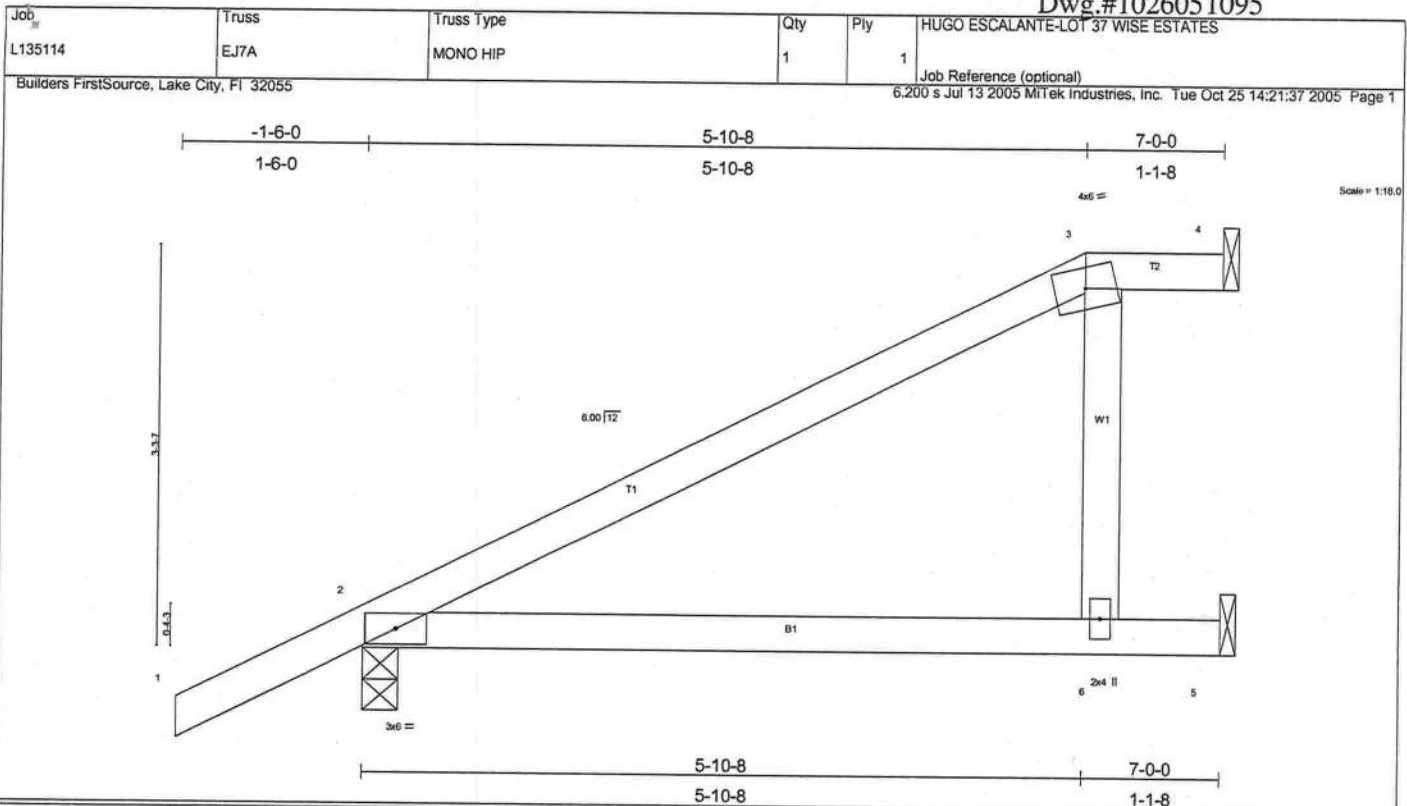
FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=142/59
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 3, 217 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.40	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.43	Vert(LL) 0.20 2-6 >404 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.08	Vert(TL) -0.25 2-6 >323 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 4 n/a n/a		
	Code FBC2004/TPI2002			Weight: 29 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(lb/size) 4=77/Mechanical, 2=385/0-3-8, 5=197/Mechanical
 Max Horz 2=219(load case 5)
 Max Uplift 4=13(load case 3), 2=-229(load case 5), 5=-160(load case 5)
 Max Grav 4=94(load case 10), 2=385(load case 1), 5=197(load case 1)

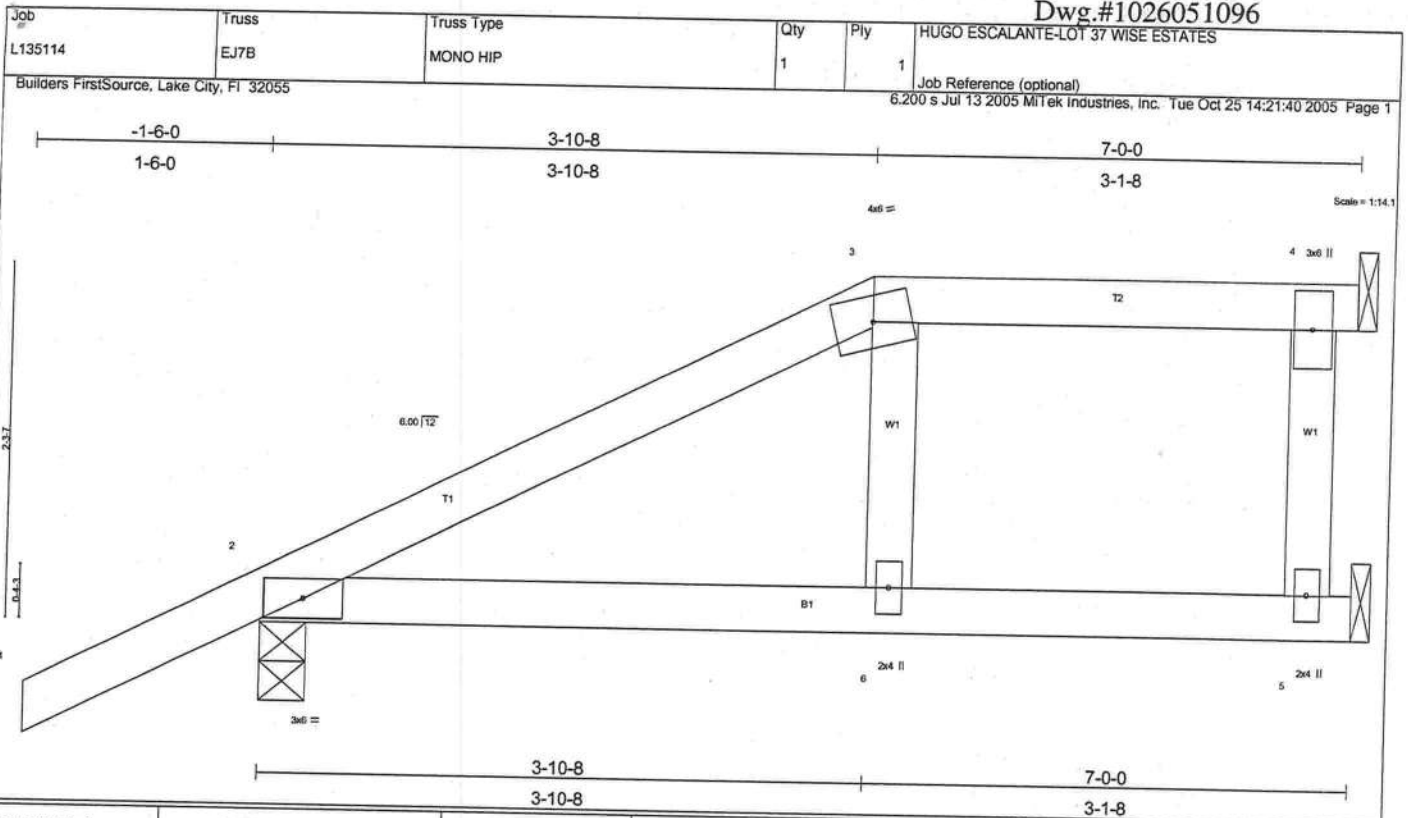
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-113/42, 3-4=0/1
 BOT CHORD 2-6=-15/5, 5-6=0/0
 WEBS 3-6=-112/302

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 4, 229 lb uplift at joint 2 and 160 lb uplift at joint 5.

LOAD CASE(S) Standard



LOADING (psf)		SPACING		CSI		DEFL				PLATES		GRIP	
TCLL	20.0	2-0-0		TC	0.34	in	(loc)	l/defl	L/d	MT20		244/190	
TCDL	7.0	Plates Increase	1.25	BC	0.44	Vert(LL)	0.19	2-6	>422				
BCLL	10.0	Lumber Increase	1.25	WB	0.04	Vert(TL)	-0.22	2-6	>355				
BCDL	5.0	Rep Stress Incr	YES	(Matrix)		Horz(TL)	0.11	4	n/a				
		Code FBC2004/TPI2002											
										Weight: 29 lb			

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=376/0-3-8, 4=138/Mechanical, 5=127/Mechanical
 Max Horz 2=164(load case 5)
 Max Uplift 2=-236(load case 5), 4=-83(load case 4), 5=-36(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/35, 2-3=-80/6, 3-4=-0/0
 BOT CHORD 2-6=-12/4, 5-6=0/0
 WEBS 3-6=-55/167, 4-5=0/0

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02: 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2, 83 lb uplift at joint 4 and 36 lb uplift at joint 5.
- Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

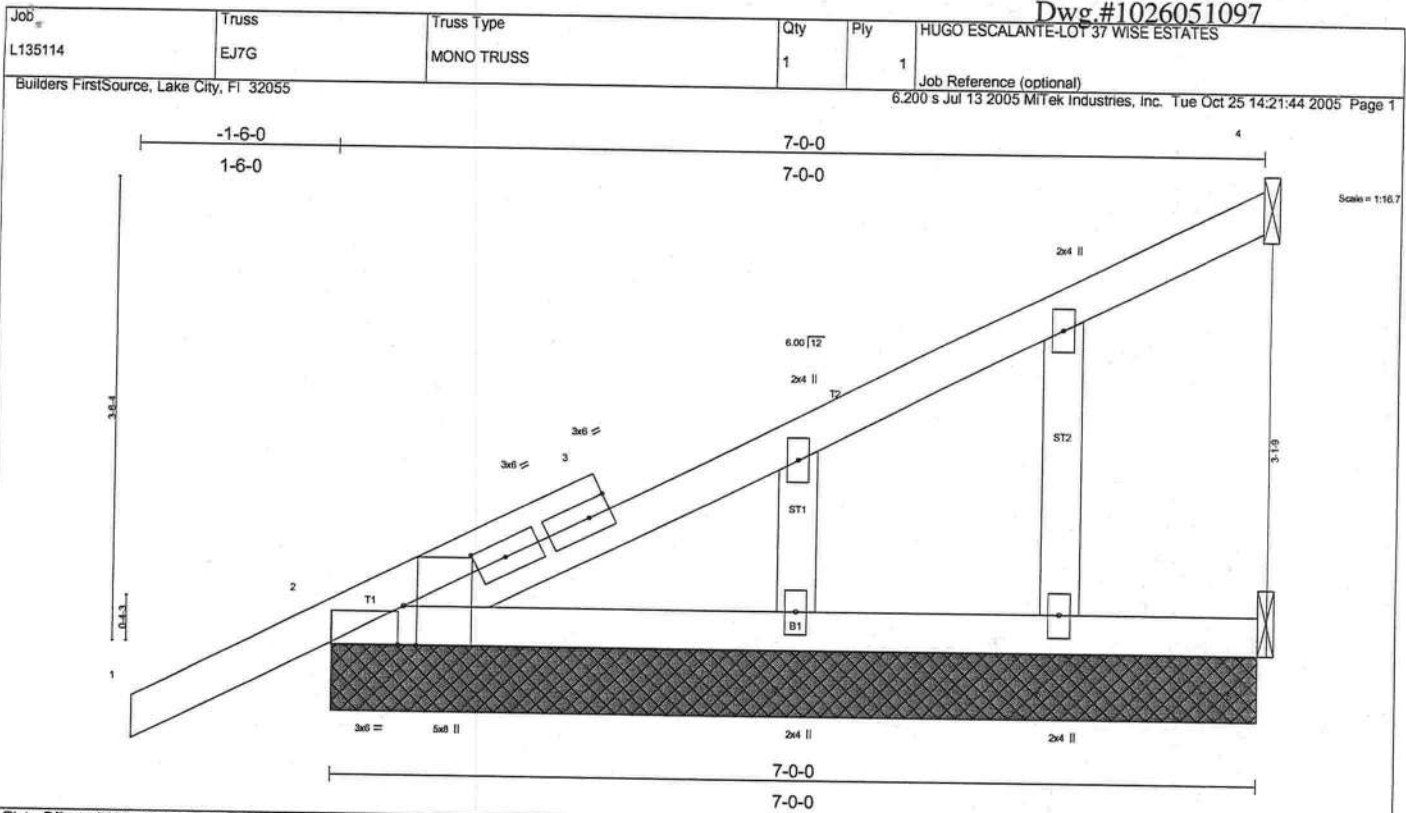


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.37	Vert(LL)	0.04	2-7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.43	Vert(TL)	0.03	2-7	>999	180		
BCCL 10.0	Rep Stress Incr	NO	WB 0.00	Horz(TL)	0.02	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 32 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.1D "Except"
 T1 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.1D
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/7-0-0, 4=146/Mechanical, 5=35/Mechanical, 7=243/7-0-0, 6=-27/7-0-0
 Max Horz 2=229(load case 5)
 Max Uplift 2=-175(load case 5), 4=-154(load case 5), 5=-12(load case 5), 7=-116(load case 5), 6=-27(load case 1)
 Max Grav 2=266(load case 1), 4=146(load case 1), 5=35(load case 1), 7=243(load case 1), 6=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/35, 2-3=-146/0, 3-4=-112/49
 BOT CHORD 2-7=0/0, 6-7=0/0, 5-6=0/0

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) Gable studs spaced at 2-0-0 oc.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 2, 154 lb uplift at joint 4, 12 lb uplift at joint 5, 116 lb uplift at joint 7 and 27 lb uplift at joint 6.

LOAD CASE(S) Standard

Plate Offsets (X,Y): [2-0-4-0-0-3-1]				4-10-0	
LOADING (psf) TCLL 20.0 TCCL 7.0 BCCL 10.0 BCCL 5.0		SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2004/TPI2002		CS1 TC 0.36 BC 0.13 WB 0.10 (Matrix)	
		DEFL in (loc) l/defl L/d Vert(LL) -0.00 1 n/r 120 Vert(TL) -0.01 1 n/r 90 Horz(TL) -0.00 6 n/a n/a		PLATES GRIP MT20 244/190 Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=198/7-0-0, 6=204/7-0-0, 7=464/7-0-0
Max Horz 2=99(load case 5)
Max Uplift 2=196(load case 5), 6=100(load case 3), 7=206(load case 4)

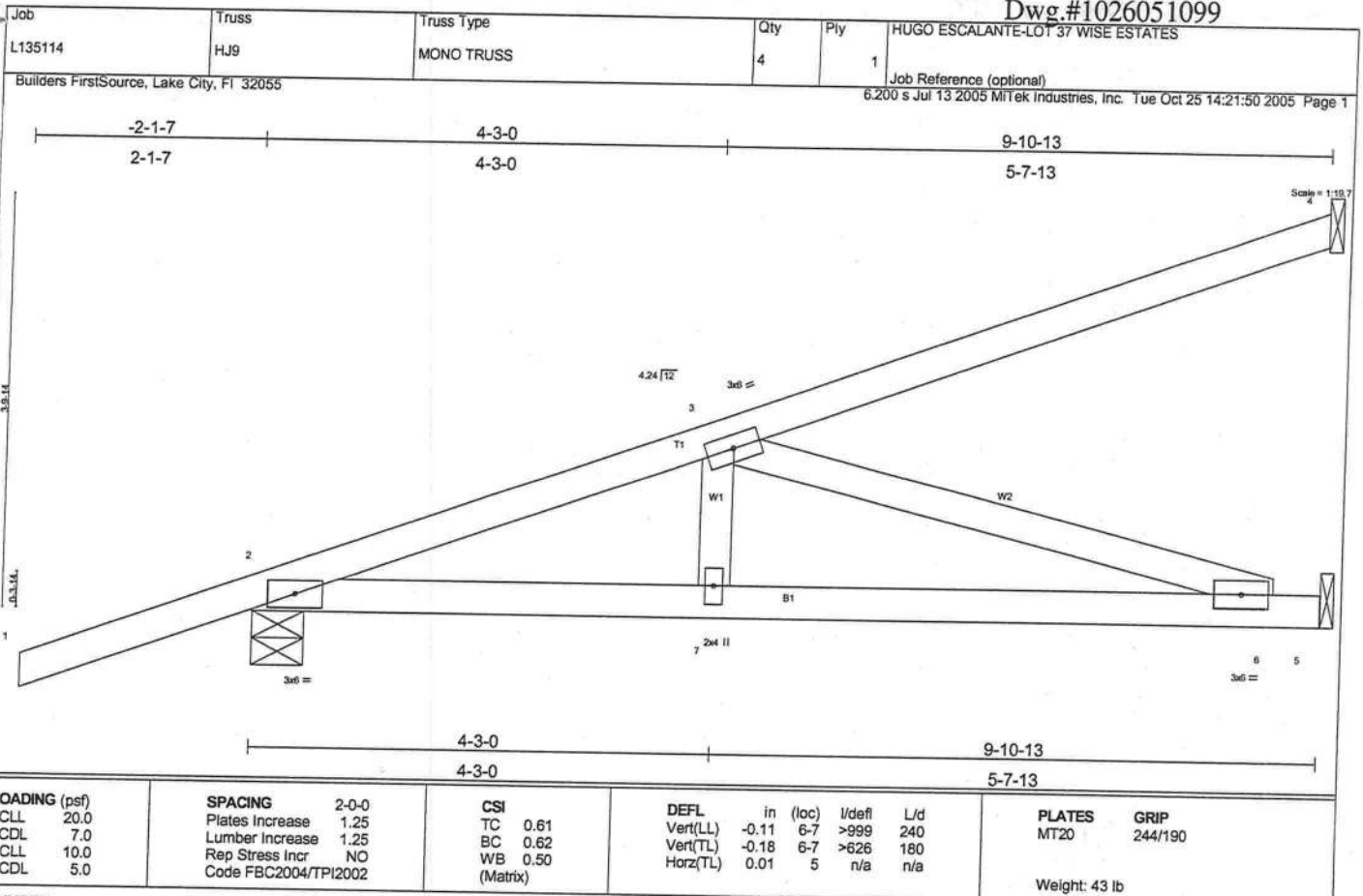
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=8/51, 2-3=77/28, 3-4=10/10, 4-5=0/0
BOT CHORD 2-7=70/60, 6-7=0/0
WEBS 4-7=338/393, 5-6=148/173

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); $h=14ft$; $TCDF=4.2psf$; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2'-0" oc.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 100 lb uplift at joint 6 and 206 lb uplift at joint 7.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-79(F=25), 4-5=-79(F=25), 2-6=-30

**LUMBER**

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-11-7 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS

(lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=386/Mechanical
 Max Horz 2=303(load case 2)
 Max Uplift 4=289(load case 2), 2=285(load case 2), 5=107(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=-934/246, 3-4=-124/65
 BOT CHORD 2-7=-475/869, 6-7=-475/869, 5-6=0/0
 WEBS 3-7=0/208, 3-6=-906/495

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 4, 285 lb uplift at joint 2 and 107 lb uplift at joint 5.
- 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

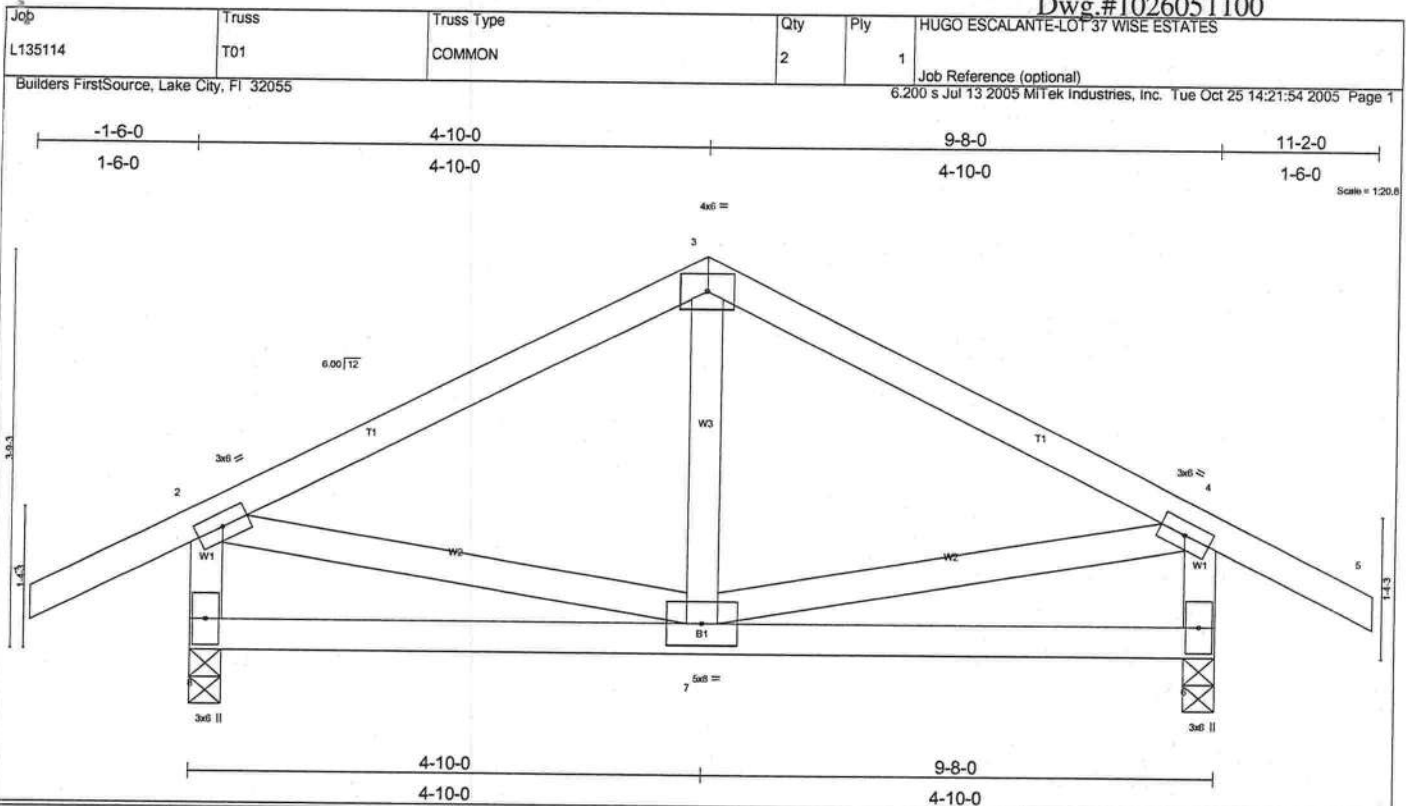
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.39	Vert(LL)	0.03	6-7	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.13	Vert(TL)	0.03	6-7	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.09	Horz(TL)	-0.00	6	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 56 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 8=483/0-3-8, 6=483/0-3-8
 Max Horz 8=87(load case 4)
 Max Uplift 8=417(load case 5), 6=417(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-380/578, 3-4=-380/578, 4-5=0/40, 2-6=-413/604, 4-6=-413/604
 BOT CHORD 7-8=-134/83, 6-7=-71/83
 WEBS 3-7=-223/67, 2-7=-219/222, 4-7=-219/222

NOTES

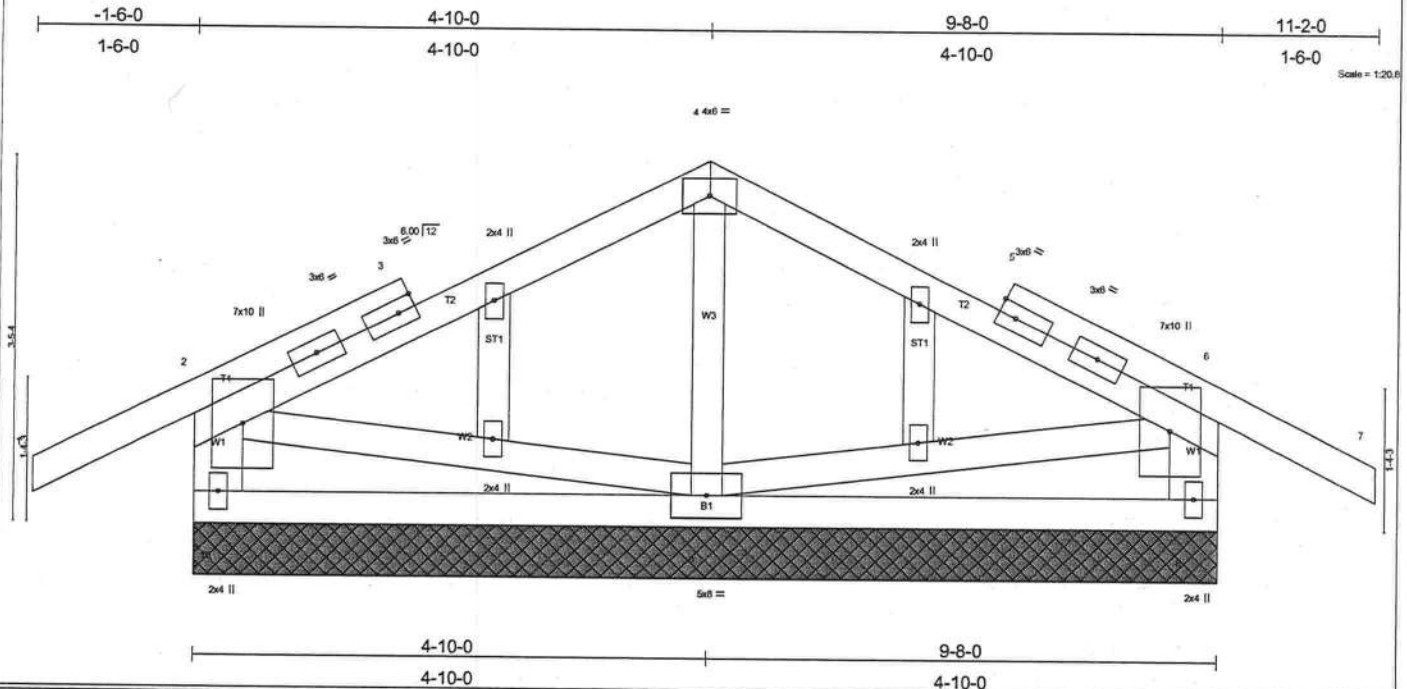
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 417 lb uplift at joint 8 and 417 lb uplift at joint 6.

LOAD CASE(S) Standard

Job:	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T01G	COMMON	1	1	
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:21:57 2005 Page 1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.30	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.11	Vert(LL) -0.01 7 n/r 120		
BCLL 10.0	Lumber Increase 1.25	WB 0.07	Vert(TL) -0.02 7 n/r 90		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.00 8 n/a n/a		
	Code FBC2004/TPI2002			Weight: 67 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 "Except"
 W1 2 X 6 SYP No.1D, W1 2 X 6 SYP No.1D
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=401/9-8-0, 8=401/9-8-0, 9=474/9-8-0

Max Horz 10=79(load case 4)
 Max Uplift 10=266(load case 5), 8=269(load case 6), 9=186(load case 5)
 Max Grav 10=405(load case 9), 8=405(load case 10), 9=474(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-6/51, 2-3=-118/80, 3-4=-41/76, 4-5=-41/76, 5-6=-118/80, 6-7=-6/51, 2-10=-338/386, 6-8=-338/386
 BOT CHORD 9-10=-109/151, 8-9=-36/151
 WEBS 4-9=-292/263, 2-9=-120/163, 6-9=-120/163

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 10, 269 lb uplift at joint 8 and 186 lb uplift at joint 9.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-79(F=-25), 2-4=-79(F=-25), 4-6=-79(F=-25), 6-7=-79(F=-25), 8-10=-30

[illegible]

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

TOP CHORD	Structural wood sheathing directly applied or 3-0-2 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-3-8 oc bracing.

REACTIONS (lb/size) 2=1841/0-3-8, 6=1841/0-3-8
Max Horz 2=-92(load case 5)
Max Uplift 2=-978(load case 4), 6=-978(load case 5)

TOP CHORD 1-2=0/35, 2-3=3339/1619, 3-4=3176/1615, 4-5=3176/1615, 5-6=3339/1619, 6-7=0/35
BOT CHORD 2-10=1368/2897, 9-10=1380/2930, 8-9=1327/2930, 6-8=1316/2897
WEBS 3-10=291/803, 3-9=315/472, 4-9=365/397, 5-9=316/472, 5-8=291/803

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDF=4.2psf; BCDF=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 978 lb uplift at joint 2 and 978 lb uplift at joint 6.
- 5) Girder carries hip end with 7-0 end setback.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 321 lb up at 14-4-0, and 539 lb down and 321 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(s) section, loads applied to the face of the truss are noted as front (F) or back (B).

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-113(F=-58), 5-7=-54, 2-10=-30, 8-10=-62(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) -0.18 7-9 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.47	Vert(TL) -0.30 7-9 >829 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.13	Horz(TL) 0.04 7 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 107 lb	

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-10-1 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 8-4-7 oc bracing.

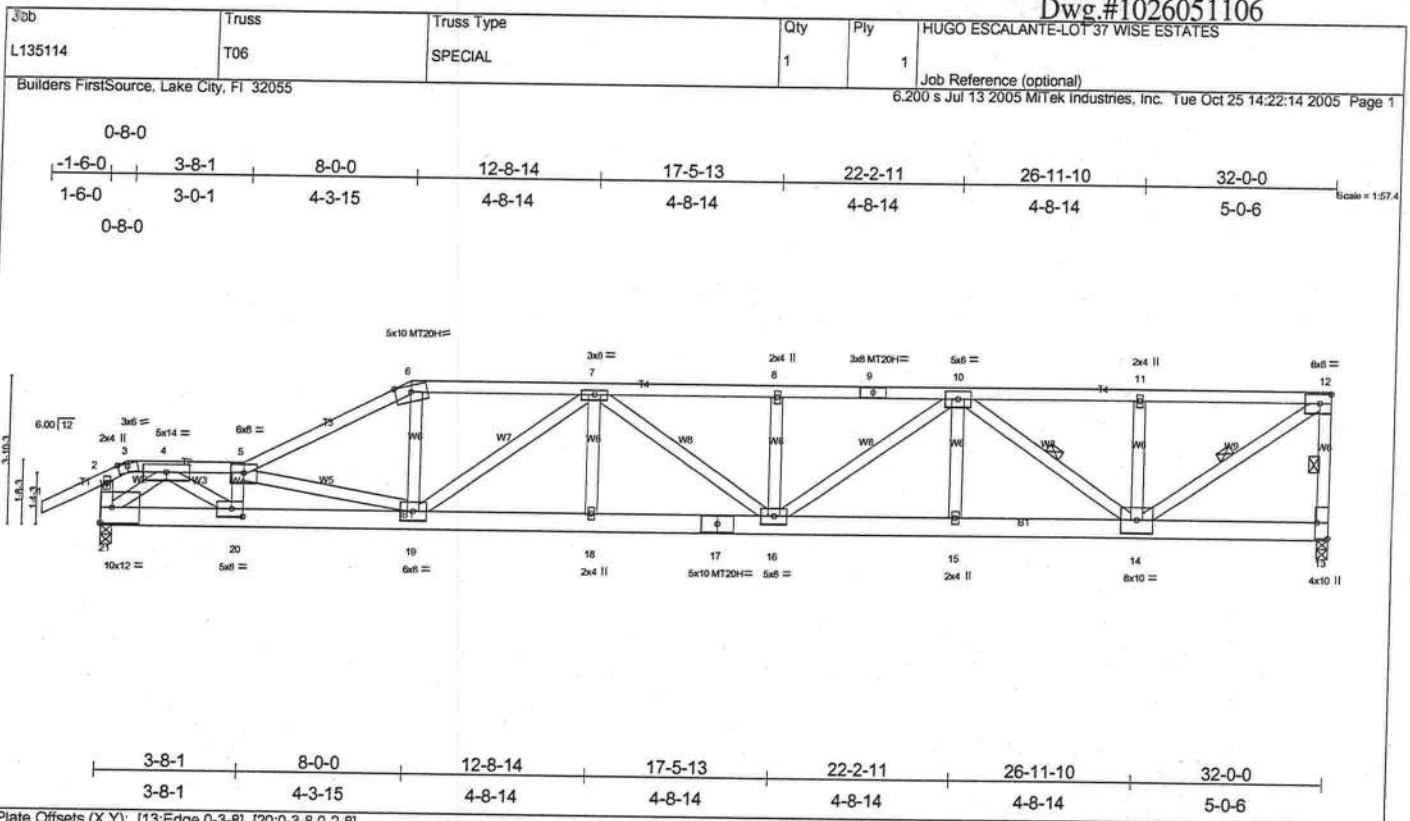
(lb/size) 2=973/0-3-8, 7=973/0-3-8
Max Horz 2=109(load case 6)
Max Uplift2=475(load case 5), 7=475(load case 6)

TOP CHORD 1-2=0/35, 2-3=1468/815, 3-4=1199/657, 4-5=1030/648, 5-6=1198/657, 6-7=1468/815, 7-8=0/35
BOT CHORD 2-11=568/1277, 10-11=304/1028, 9-10=304/1028, 7-9=568/1277
WEBS 3-11=291/300, 4-11=100/314, 5-11=102/106, 5-9=100/315, 6-9=293/300

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02: 120mph (3-second gust); $h=14$ ft; $C_{DOL}=4.2$ psf; $BCDL=3.0$ psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 475 lb uplift at joint 2 and 475 lb uplift at joint 7.

LOAD CASE(S) Standard

OCTOBER 26, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.87	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.93	Vert(LL) 0.47 16-18 >814	MT20H	187/143
BCLL 10.0	Lumber Increase 1.25	WB 0.84	Vert(TL) -0.74 16-18 >515		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.14 13 n/a		
	Code FBC2004/TPI2002				
				Weight: 208 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3 *Except*

W3 2 X 4 SYP No.2, W9 2 X 4 SYP No.2, W1 2 X 4 SYP No.1D, W2 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 4-4-11 oc bracing.

WEBS 1 Row at midpt 12-13, 10-14, 12-14

REACTIONS (lb/size) 13=2838/0-3-8, 21=2696/0-3-8

Max Horz 21=226(load case 4)

Max Uplift 13=1601(load case 3), 21=1421(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=258/154, 3-4=-214/142, 4-5=-5976/3114, 5-6=-5829/3177, 6-7=-5292/2935, 7-8=-6576/3693, 8-9=-6576/3693, 9-10=-6576/3693, 10-11=-3449/1939, 11-12=-3449/1939, 12-13=-2651/1587, 2-21=-237/227

BOT CHORD 20-21=-1554/2812, 19-20=-3335/6244, 18-19=-3636/6480, 17-18=-3636/6480, 16-17=-3636/6480, 15-16=-3162/5610, 14-15=-3162/5610, 13-14=-72/100

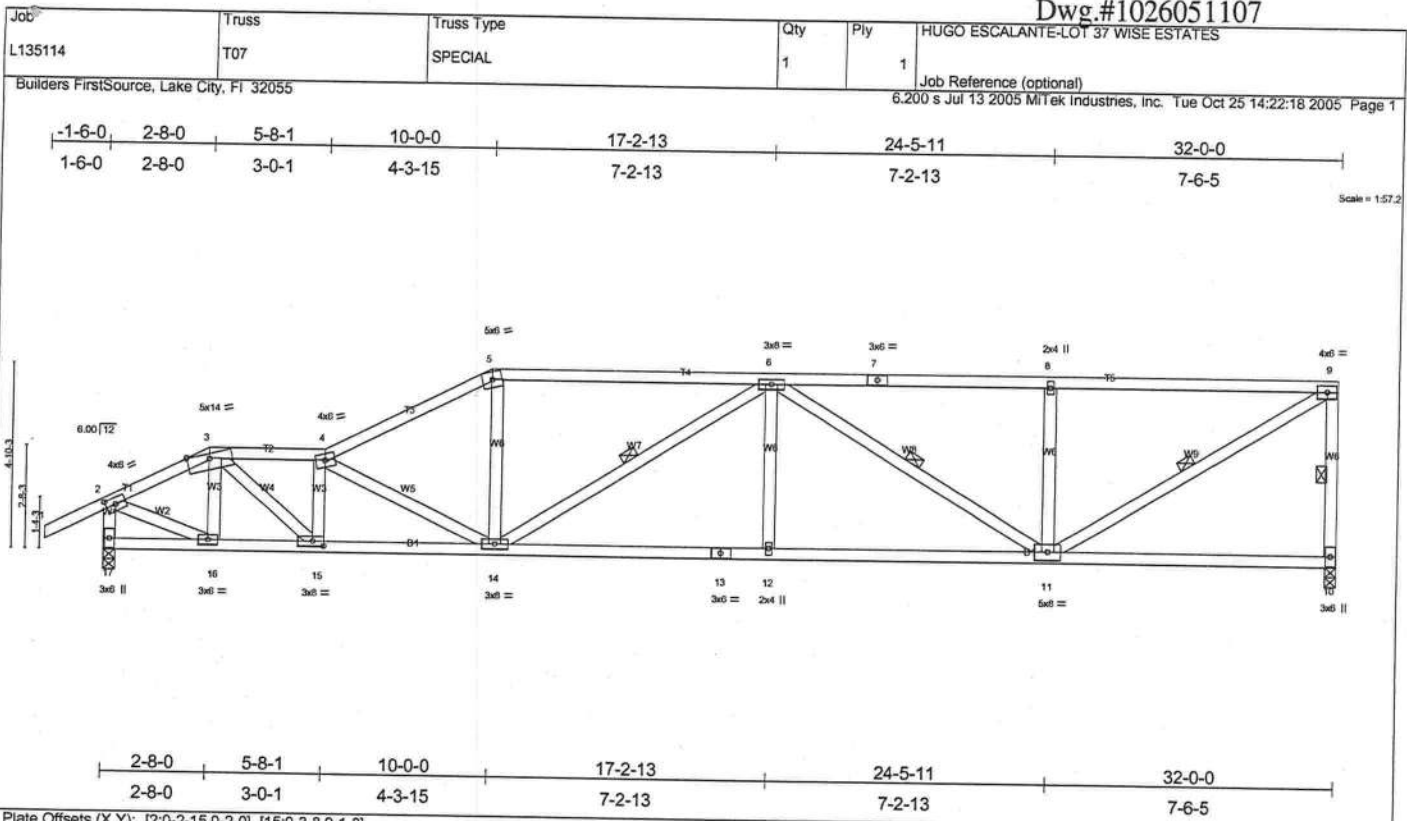
WEBS 4-20=-1998/3868, 5-20=-2401/1319, 5-19=-1086/592, 6-19=-1072/2149, 7-19=-1489/909, 7-18=0/295, 7-16=-91/119, 8-16=-521/519, 10-16=-659/1198, 10-15=0/335, 10-14=-2678/1516, 11-14=-545/554, 12-14=-2290/4108, 4-21=-3427/1790

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1601 lb uplift at joint 13 and 1421 lb uplift at joint 21.
- 5) Girder carries hip end with 0-0-0 right side setback, 8-0-0 left side setback, and 7-0-0 end setback.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 616 lb down and 397 lb up at 8-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
- Uniform Loads (plf)
- Vert: 1-2=-54, 2-3=-54, 3-5=-54, 5-6=-54, 6-12=-113(F=-59), 19-21=-30, 13-19=-62(F=-33)
- Concentrated Loads (lb)
- Vert: 19=616(F)



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.70	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.62	Vert(LL) -0.21 12-14 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.63	Vert(TL) -0.34 12-14 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.08 10 n/a n/a		
	Code FBC2004/TPI2002				
				Weight: 184 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-3-7 oc bracing.
 WEBS 1 Row at midpt 9-10, 6-14, 6-11, 9-11

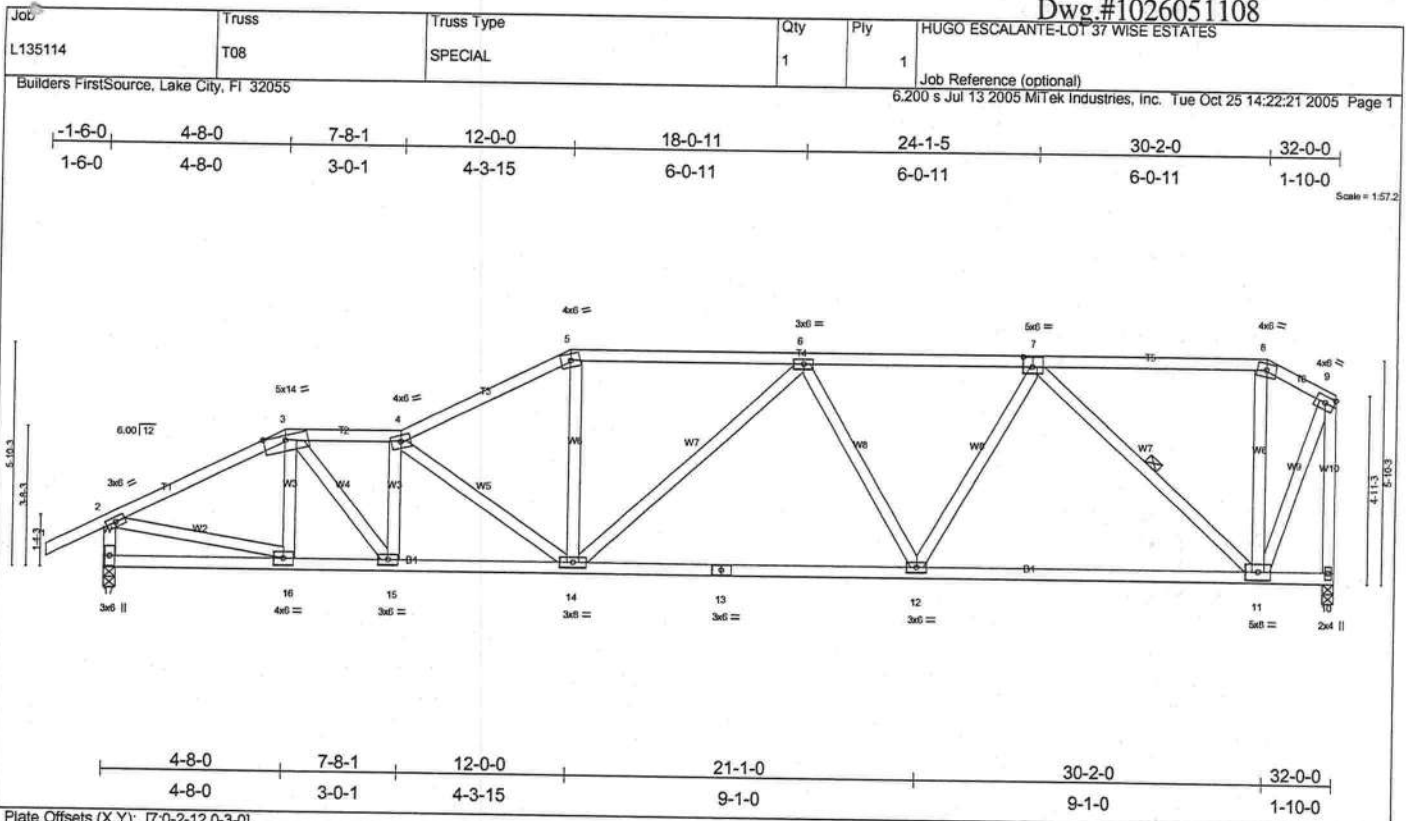
REACTIONS (lb/size) 10=1329/0-3-8, 17=1423/0-3-8
 Max Horz 17=283(load case 5)
 Max Uplift 10=619(load case 4), 17=613(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=1515/771, 3-4=2461/1281, 4-5=2293/1181, 5-6=2053/1135, 6-7=1733/901, 7-8=1733/901, 8-9=1733/901, 9-10=1218/698, 2-17=1373/825
 BOT CHORD 16-17=248/33, 15-16=829/1292, 14-15=1445/2516, 13-14=1257/2373, 12-13=1257/2373, 11-12=1257/2373, 10-11=34/68
 WEBS 3-16=458/255, 3-15=760/1522, 4-15=900/520, 4-14=548/358, 5-14=214/661, 6-14=380/269, 6-12=0/217, 6-11=756/421, 8-11=406/366, 9-11=1018/1957, 2-16=647/1394

NOTES

- 1) Wind: ASCE 7-02: 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 619 lb uplift at joint 10 and 613 lb uplift at joint 17.

LOAD CASE(S) Standard

**LOADING** (psf)

TCLL 20.0
TCCL 7.0
BCLL 10.0
BCDL 5.0

SPACING

2-0-0
Plates Increase 1.25
Lumber Increase 1.25
Rep Stress Incr YES
Code FBC2004/TP12002

CSI

TC 0.91
BC 0.66
WB 0.47
(Matrix)

DEFL

in (loc) l/defl L/d
Vert(LL) -0.21 12-14 >999 240
Vert(TL) -0.36 12-14 >999 180
Horz(TL) 0.07 10 n/a n/a

PLATES

MT20

GRIP

244/190

Weight: 202 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-14 oc bracing.
WEBS 1 Row at midpt 7-11

REACTIONS

(lb/size) 17=1423/0-3-8, 10=1329/0-3-8
Max Horz 17=305(load case 5)
Max Uplift 17=631(load case 5), 10=545(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-1791/910, 3-4=-2210/1195, 4-5=-2058/1090, 5-6=-1831/1048, 6-7=-1724/926, 7-8=-459/274, 8-9=-500/256, 2-17=-1340/832, 9-10=-1376/671
BOT CHORD 16-17=-316/130, 15-16=-940/1535, 14-15=-1292/2242, 13-14=-1018/1878, 12-13=-1018/1878, 11-12=-760/1416, 10-11=-5/4
WEBS 3-16=-225/177, 3-15=-519/1046, 4-15=-739/414, 4-14=-535/379, 5-14=-206/597, 6-14=-202/170, 6-12=-322/302, 7-12=-237/645, 7-11=-1317/739, 8-11=-56/119, 2-16=-649/1455, 9-11=-604/1254

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 631 lb uplift at joint 17 and 545 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 Mitek Industries, Inc. Tue Oct 25 14:22:25 2005 Page 1

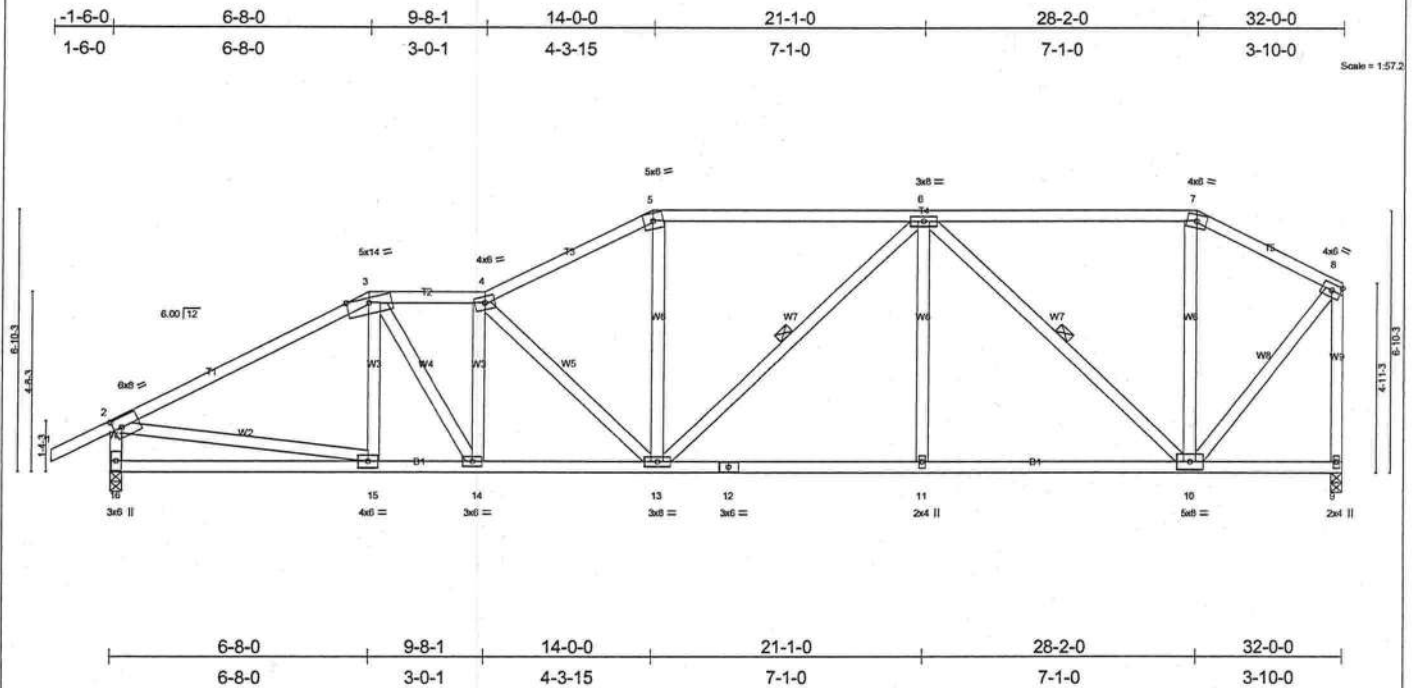


Plate Offsets (X,Y): [2:0-2-7, 0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCCL 20.0	Plates Increase	1.25	TC 0.83	Vert(LL)	-0.14 11-13	>999	240	MT20	244/190
TCCL 7.0	Lumber Increase	1.25	BC 0.52	Vert(TL)	-0.22 11-13	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.47	Horz(TL)	0.06 9	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 210 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-11-4 oc bracing.
 WEBS 1 Row at midpt 6-13, 6-10

REACTIONS

(lb/size) 16=1423/0-3-8, 9=1329/0-3-8
 Max Horz 16=321(load case 5)
 Max Uplift 16=648(load case 5), 9=479(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-1887/964, 3-4=-1994/1119, 4-5=-1825/1019, 5-6=-1623/987, 6-7=-719/476, 7-8=-830/462, 2-16=-1310/844, 8-9=-1290/709
 BOT CHORD 15-16=-412/271, 14-15=-959/1608, 13-14=-1147/2011, 12-13=-781/1497, 11-12=-781/1497, 10-11=-781/1497, 9-10=-8/7
 WEBS 3-15=-73/130, 3-14=-327/706, 4-14=-525/287, 4-13=-559/385, 5-13=-145/463, 6-13=-161/183, 6-11=0/218, 6-10=-1088/575, 7-10=0/93, 2-15=-559/1358, 8-10=-566/1125

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 648 lb uplift at joint 16 and 479 lb uplift at joint 9.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 Mitek Industries, Inc. Tue Oct 25 14:22:28 2005 Page 1

-1-6-0	4-5-12	8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0	33-6-0
1-6-0	4-5-12	4-2-4	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0	1-6-0

Scale = 1/56.7

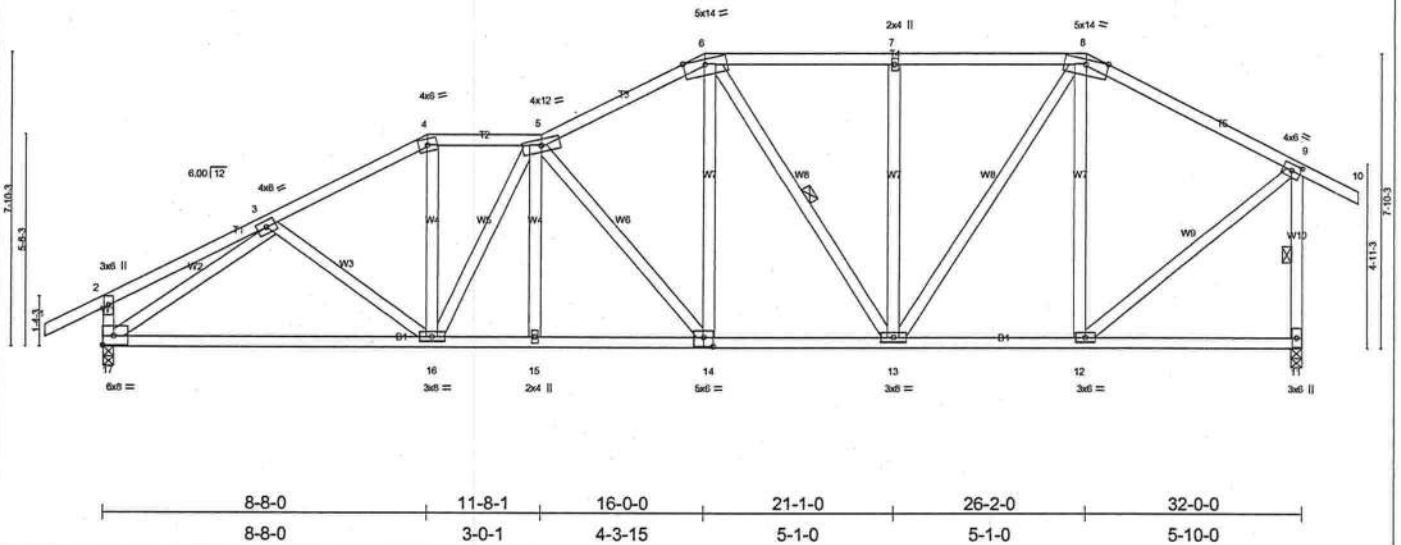


Plate Offsets (X,Y): [9:0-2-15,0-2-0], [14:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.68	Vert(LL)	-0.13 16-17	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.56	Vert(TL)	-0.23 16-17	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.78	Horz(TL)	0.06 11	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 229 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 7-1-2 oc bracing.
 WEBS 1 Row at midpt 6-13, 9-11

REACTIONS (lb/size) 17=1421/0-3-8, 11=1421/0-3-8
 Max Horz 17=297(load case 4)
 Max Uplift 17=674(load case 5), 11=565(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-335/151, 3-4=-1802/1023, 4-5=-1583/979, 5-6=-1611/1004, 6-7=-1264/887, 7-8=-1264/887, 8-9=-1024/684, 9-10=0/40, 2-17=-348/327, 9-11=-1335/865
 BOT CHORD 16-17=715/1467, 15-16=-786/1806, 14-15=-786/1805, 13-14=-558/1415, 12-13=-333/844, 11-12=-46/98
 WEBS 3-16=-58/162, 4-16=-262/561, 5-16=-476/290, 5-15=0/46, 5-14=-629/443, 6-14=-312/645, 6-13=-288/181, 7-13=-281/247, 8-13=-376/793, 8-12=-495/280, 3-17=-1538/913, 9-12=-410/1060

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 674 lb uplift at joint 17 and 565 lb uplift at joint 11.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T11	SPECIAL	1	1	

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:22:32 2005 Page 1

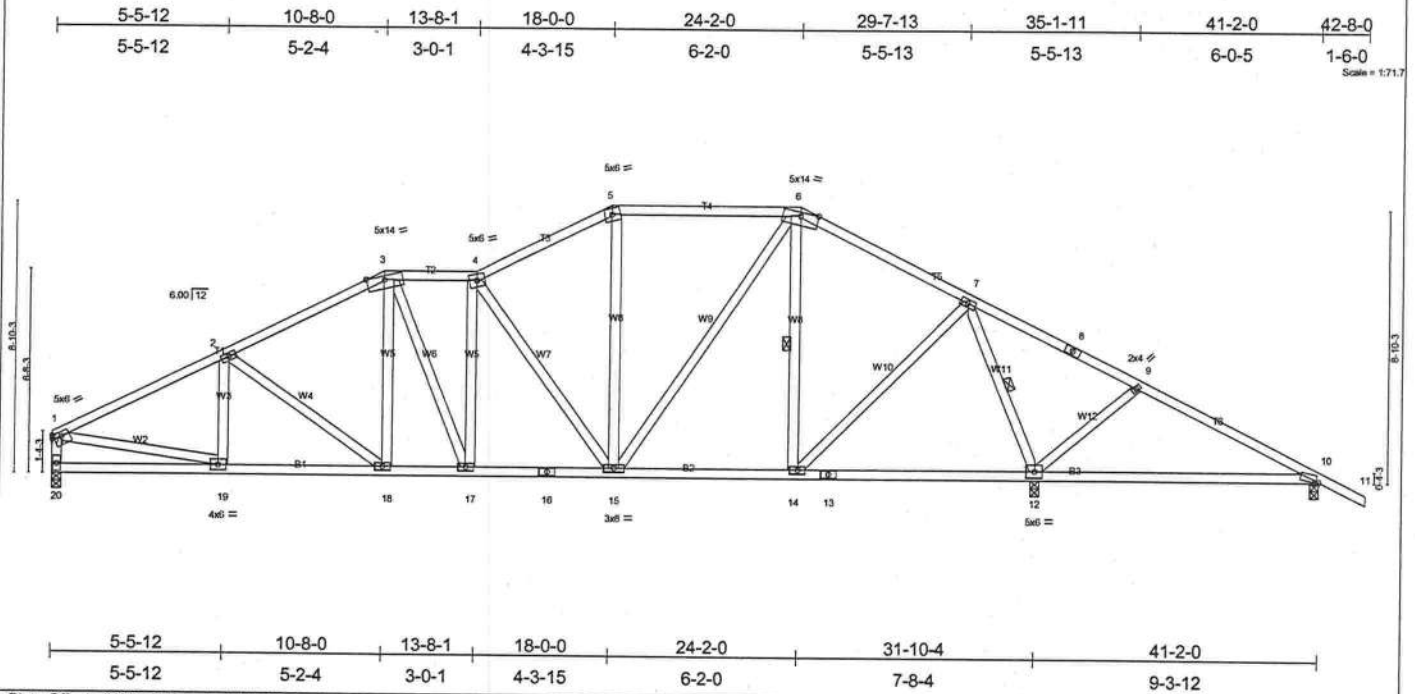


Plate Offsets (X,Y): [1:Edge,0-1-12], [10:0-1-5,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.52	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.46	Vert(LL) 0.41 10-12 >266 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.73	Vert(TL) 0.35 10-12 >312 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 12 n/a n/a		
	Code FBC2004/TPI2002				
				Weight: 260 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-3 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 6-14, 7-12

REACTIONS (lb/size) 20=1256/0-3-8, 12=2045/0-3-8, 10=221/0-3-8

Max Horz 20=209(load case 3)
 Max Uplift 20=539(load case 5), 12=913(load case 6), 10=345(load case 6)
 Max Grav 20=1256(load case 1), 12=2045(load case 1), 10=282(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1744/943, 2-3=-1589/943, 3-4=-1461/954, 4-5=-1238/836, 5-6=-1076/816, 6-7=-886/612, 7-8=-158/587, 8-9=-177/465, 9-10=-79/357,
 10-11=0/35, 1-20=-1167/676
 BOT CHORD 19-20=-195/213, 18-19=-652/1501, 17-18=-494/1368, 16-17=-511/1464, 15-16=-511/1464, 14-15=-114/728, 13-14=0/174, 12-13=0/174,
 10-12=-287/160
 WEBS 2-19=-118/170, 2-18=-177/204, 3-18=-104/235, 3-17=-111/228, 4-17=-121/128, 4-15=-681/476, 5-15=-97/225, 6-15=-346/648,
 6-14=-449/309, 7-14=-301/899, 7-12=-1633/892, 9-12=-351/440, 1-19=-600/1315

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02: 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All plates are 3x6 MT20 unless otherwise indicated.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 20, 913 lb uplift at joint 12 and 345 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:22:39 2005 Page 1

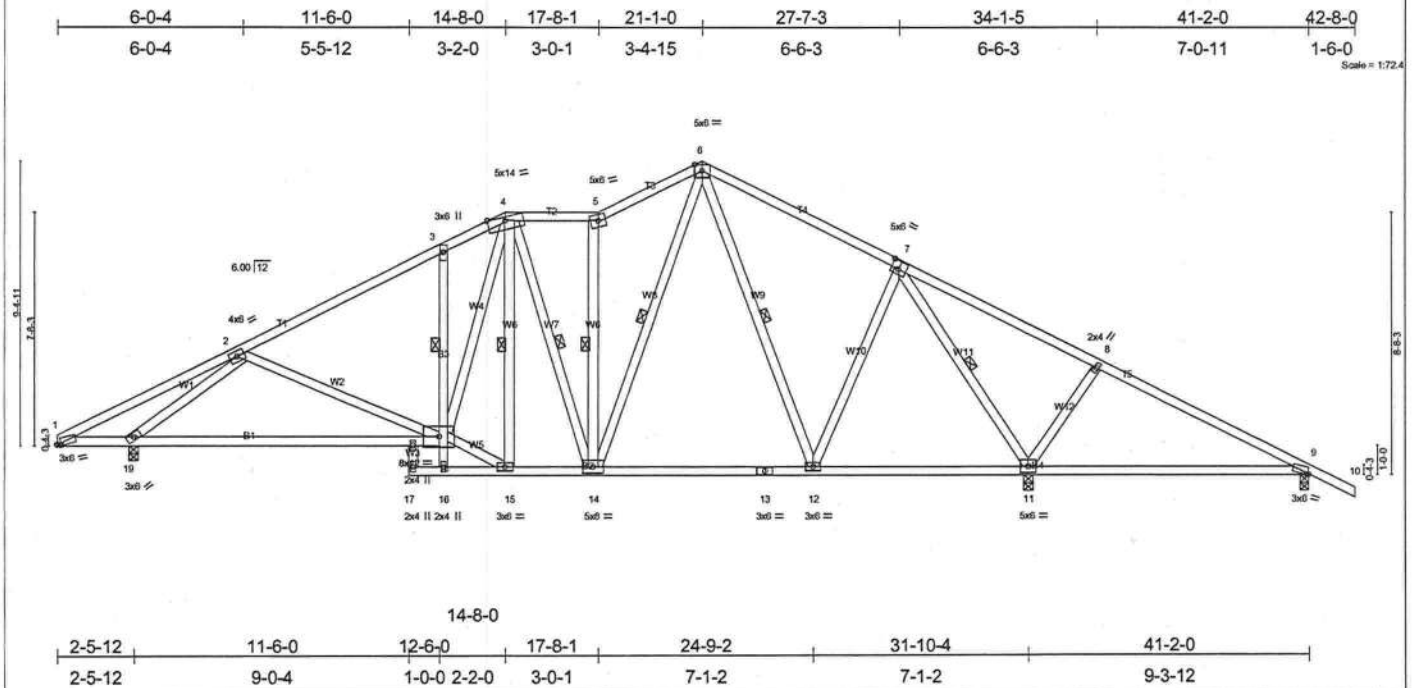


Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-3-0,0-3-0], [9:0-0-10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.40	Vert(LL)	0.45	9-11	>243	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.58	Vert(TL)	0.38	9-11	>290	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.64	Horz(TL)	0.05	11	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 273 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
 1 Row at midpt 3-18
 WEBS 1 Row at midpt 4-15, 4-14, 5-14, 6-14, 6-12, 7-11

REACTIONS (lb/size) 11=1892/0-3-8, 9=277/0-3-8, 19=1400/0-3-8

Max Horz 19=261(load case 6)
 Max Uplift 11=918(load case 6), 9=345(load case 6), 19=669(load case 5)
 Max Grav 11=1892(load case 1), 9=331(load case 10), 19=1400(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-301/91, 2-3=-1523/808, 3-4=-1435/955, 4-5=-994/747, 5-6=-1131/880, 6-7=-798/622, 7-8=-106/477, 8-9=-98/274, 9-10=0/35
 BOT CHORD 1-19=-14/339, 18-19=-562/1225, 16-18=0/64, 3-18=-239/341, 16-17=0/0, 15-16=-184/0, 14-15=-267/991, 13-14=-97/737, 12-13=-97/737,
 11-12=-5/453, 9-11=-211/208
 WEBS 2-18=0/186, 15-18=-167/1241, 4-18=-420/976, 4-15=-472/84, 4-14=-46/71, 5-14=-593/487, 6-14=-511/850, 6-12=-286/169, 7-12=-88/513,
 7-11=-1535/789, 8-11=-373/466, 2-19=-1609/1204

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 918 lb uplift at joint 11, 345 lb uplift at joint 9 and 669 lb uplift at joint 19.

LOAD CASE(S) Standard

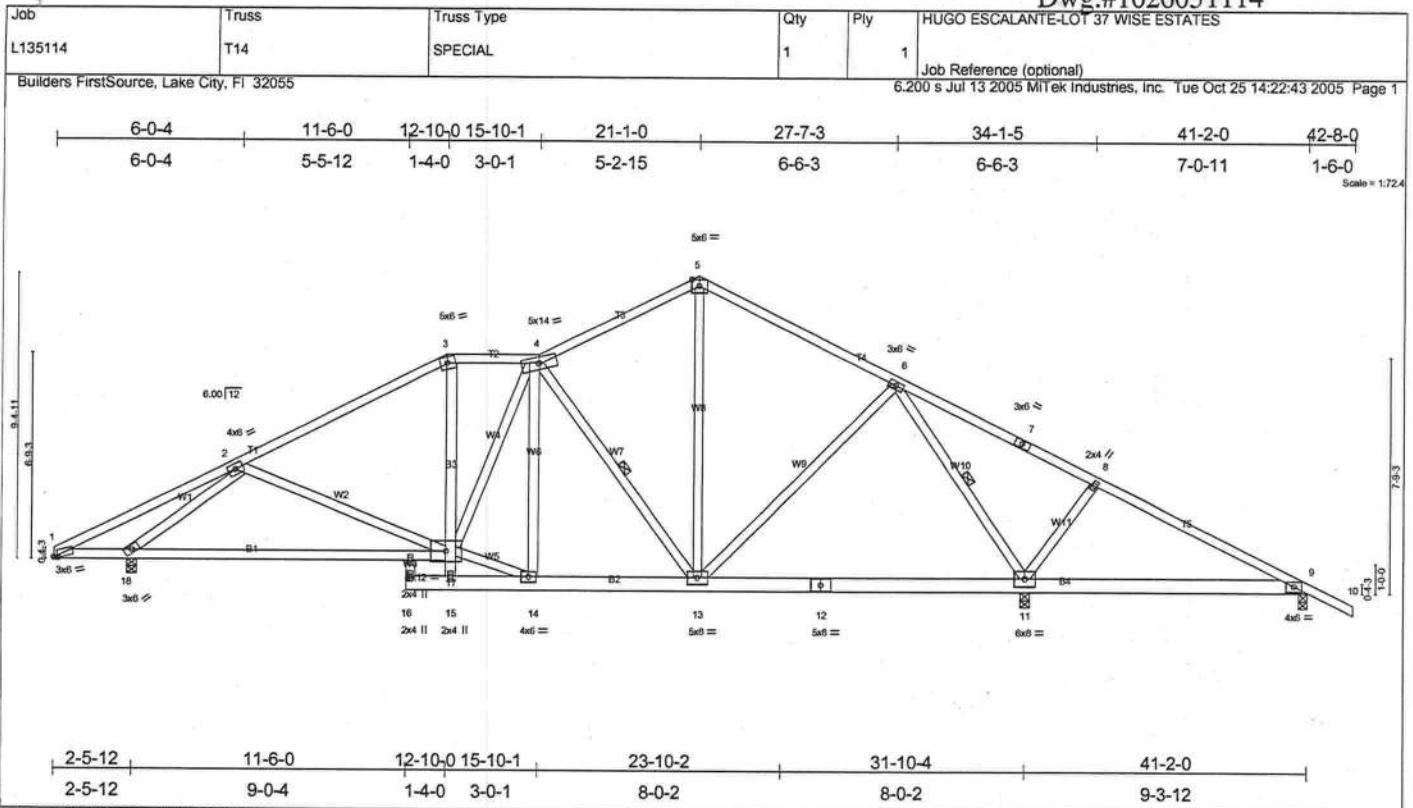


Plate Offsets (X,Y): [1:0-1-8,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.37	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.71	Vert(LL) 0.14 9-11 >778 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.64	Vert(TL) 0.13 9-11 >877 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 11 n/a n/a		
	Code FBC2004/TPI2002			Weight: 270 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D *Except*
 B1 2 X 4 SYP No.2, B3 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 4-13, 6-11

REACTIONS (lb/size) 11=1903/0-3-8, 9=271/0-3-8, 18=1405/0-3-8

Max Horz 18=-263(load case 6)
 Max Uplift 11=-914(load case 6), 9=-348(load case 6), 18=-668(load case 5)
 Max Grav 11=1903(load case 1), 9=329(load case 10), 18=1405(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-306/78, 2-3=-1527/801, 3-4=-1272/796, 4-5=-916/666, 5-6=-943/661, 6-7=-103/469, 7-8=-116/325, 8-9=-107/287, 9-10=0/39
 BOT CHORD 1-18=-3/343, 17-18=-565/1231, 15-17=0/98, 3-17=-71/354, 15-16=0/0, 14-15=-239/0, 13-14=-344/1140, 12-13=-18/441, 11-12=-18/441,
 9-11=-233/234
 WEBS 2-17=-4/195, 14-17=-205/1367, 4-17=-73/364, 4-14=-360/77, 4-13=-657/484, 5-13=-253/433, 6-13=-105/510, 6-11=-1482/831,
 8-11=-345/433, 2-18=-1602/1211

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone: cantilever left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 914 lb uplift at joint 11, 348 lb uplift at joint 9 and 668 lb uplift at joint 18.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L1351114	T15	SPECIAL	1	1	
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:22:46 2005 Page 1

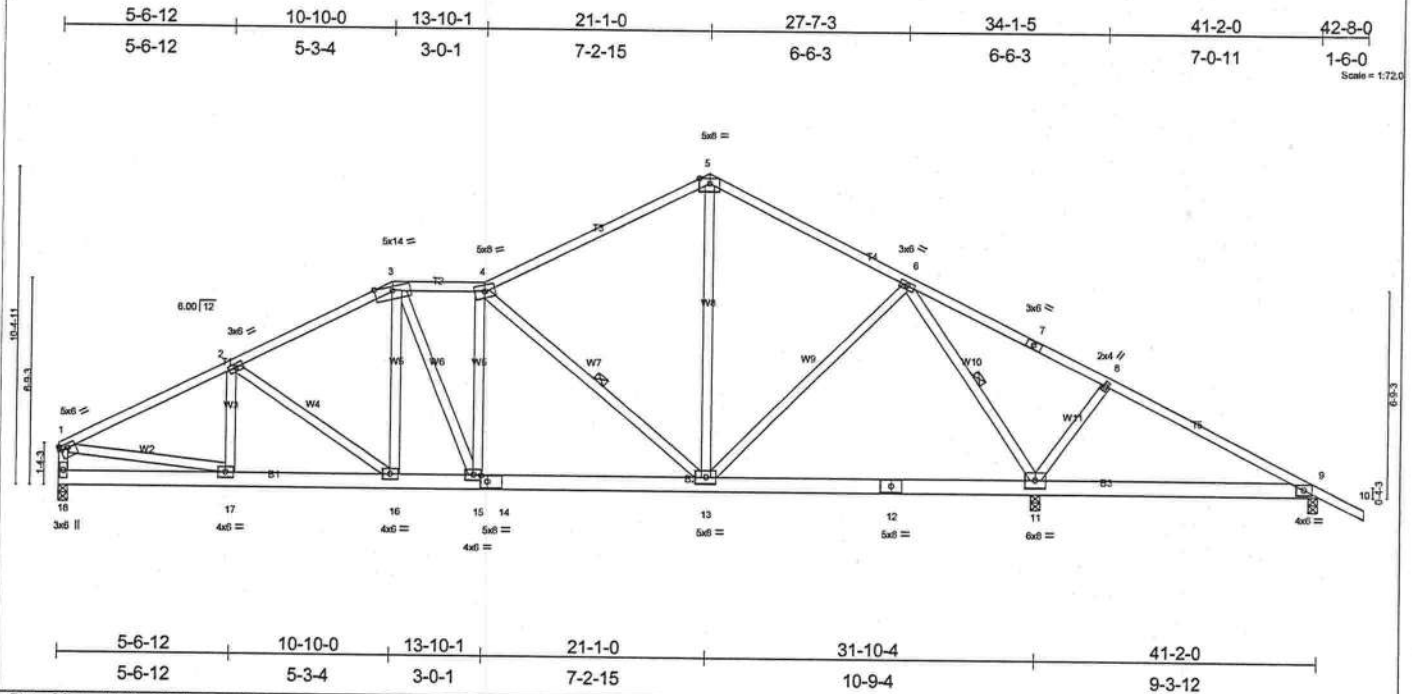


Plate Offsets (X,Y): [1:Edge,0-1-12], [14:0-2-7,0-2-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.55	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.24	Vert(LL) 0.14 9-11 >781 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.52	Vert(TL) 0.12 9-11 >882 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 11 n/a n/a		
	Code FBC2004/TPI2002				
				Weight: 276 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 4-13, 6-11

REACTIONS (lb/size) 18=1264/0-3-8, 11=2012/0-3-8, 9=247/0-3-8

Max Horz 18=-239(load case 6)
 Max Uplift 18=-549(load case 5), 11=-964(load case 6), 9=-328(load case 6)
 Max Grav 18=1264(load case 1), 11=2012(load case 1), 9=315(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1782/963, 2-3=-1613/956, 3-4=-1504/993, 4-5=-1033/702, 5-6=-1024/712, 6-7=-153/524, 7-8=-166/381, 8-9=-140/334, 9-10=0/39,
 1-18=-1153/666
 BOT CHORD 17-18=-274/293, 16-17=-700/1534, 15-16=-533/1387, 14-15=-580/1507, 13-14=-580/1507, 12-13=-28/460, 11-12=-28/460, 9-11=-275/280
 WEBS 2-17=-115/173, 2-16=-207/216, 3-16=-111/258, 3-15=-176/287, 4-15=-141/173, 4-13=-885/648, 5-13=-241/456, 6-13=-164/591,
 6-11=-1612/929, 8-11=-349/438, 1-17=-549/1263

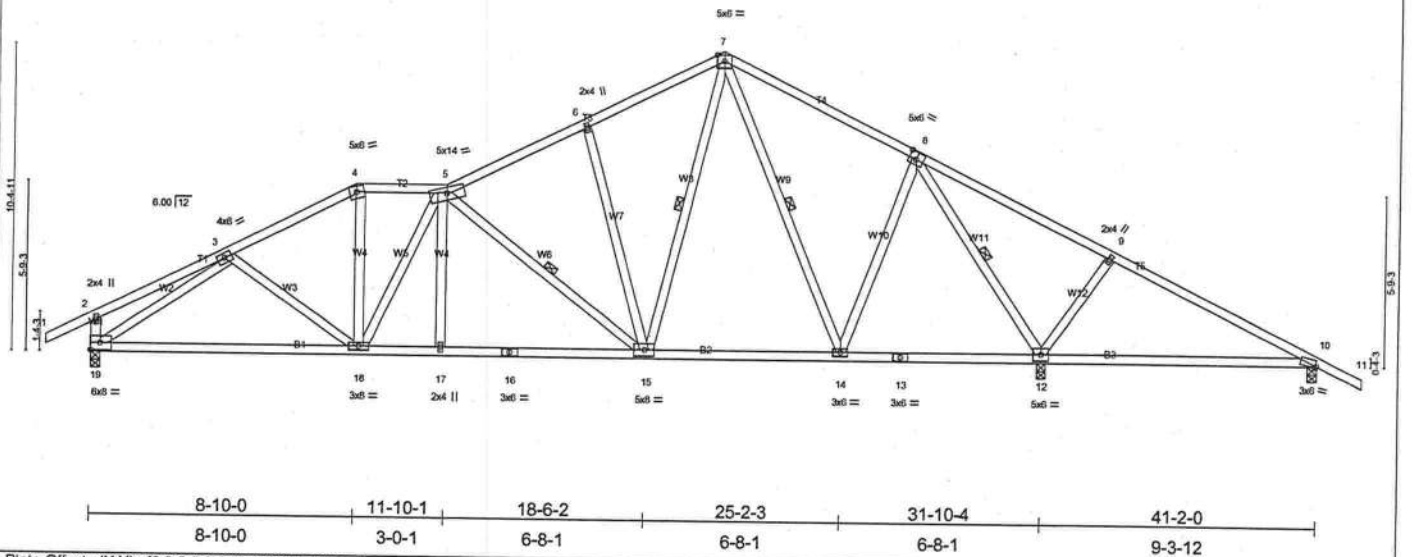
NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 18, 964 lb uplift at joint 11 and 328 lb uplift at joint 9.

LOAD CASE(S) Standard

Dwg.#1026051116

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES				
L135114	T16	SPECIAL	1	1					
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)						
			6.200 s Jul 13 2005 MITek Industries, Inc. Tue Oct 25 14:22:50 2005 Page 1						
1-6-0	4-6-12	8-10-0	11-10-1	16-5-8	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	4-6-12	4-3-4	3-0-1	4-7-8	4-7-8	6-6-3	6-6-3	7-0-11	1-6-0
Scale = 1/4"=1'-0"									



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.71	in (loc) l/defl L/d	MT20	244/190
BCDL 7.0	Plates Increase 1.25	BC 0.53	Vert(LL) 0.45 10-12 >246 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.75	Vert(TL) 0.37 10-12 >294 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 12 n/a n/a		
	Code FBC2004/TPI2002				
				Weight: 258 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 5-15, 7-15, 7-14, 8-12

REACTIONS (lb/size) 12=2020/0-3-8, 19=1353/0-3-8, 10=239/0-3-8
 Max Horz 19=-210(load case 6)
 Max Uplift 12=-964(load case 6), 19=-661(load case 5), 10=-327(load case 6)
 Max Grav 12=2020(load case 1), 19=1353(load case 1), 10=309(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-342/148, 3-4=-1676/956, 4-5=-1466/919, 5-6=-1267/817, 6-7=-1199/887, 7-8=-807/640, 8-9=-155/559, 9-10=-142/356,
 10-11=0/35, 2-19=-351/326
 BOT CHORD 18-19=-679/1384, 17-18=-659/1659, 16-17=-659/1657, 15-16=-659/1657, 14-15=-135/802, 13-14=-9/458, 12-13=-9/458, 10-12=-275/257
 WEBS 3-18=-11/119, 4-18=-240/514, 5-18=-416/241, 5-17=0/103, 5-15=-734/483, 6-15=-247/273, 7-15=-558/946, 7-14=-414/217, 8-14=-133/589,
 8-12=-1684/885, 9-12=-374/467, 3-19=-1423/860

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02: 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 964 lb uplift at joint 12, 661 lb uplift at joint 19 and 327 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L135114	Truss T17	Truss Type SPECIAL	Qty 1	Ply 1	HUGO ESCALANTE-LOT 37 WISE ESTATES
Builders FirstSource, Lake City, Fl 32055			Job Reference (optional)		

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:22:54 2005 Page 1

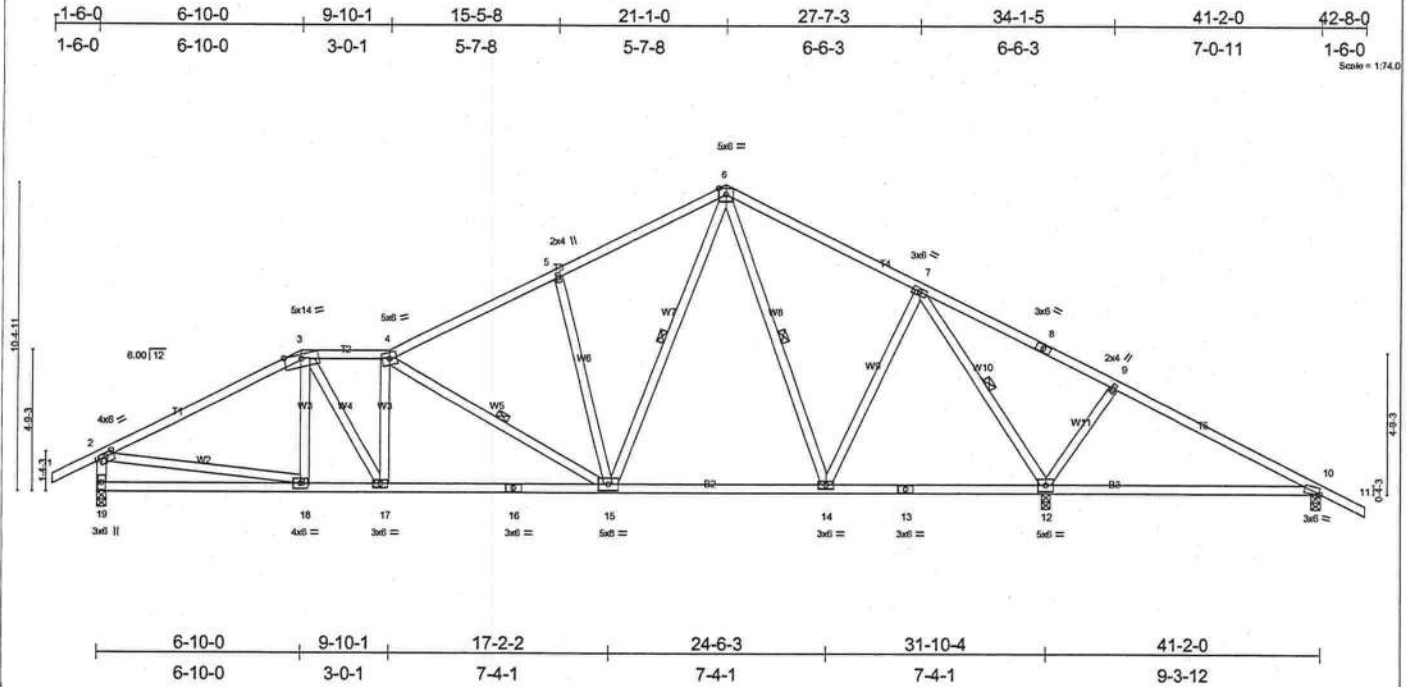


Plate Offsets (X,Y): [2-0-3-0-0-1-8], [10-0-0-10-Edge]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.59	Vert(LL) 0.45 10-12	>242 240
TCDL 7.0	Lumber Increase	1.25	BC 0.52	Vert(TL) 0.38 10-12	>287 180
BCLL 10.0	Rep Stress Incr	YES	WB 0.57	Horz(TL) 0.05 12	n/a n/a
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)		
Weight: 248 lb					

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 4-15, 6-15, 6-14, 7-12

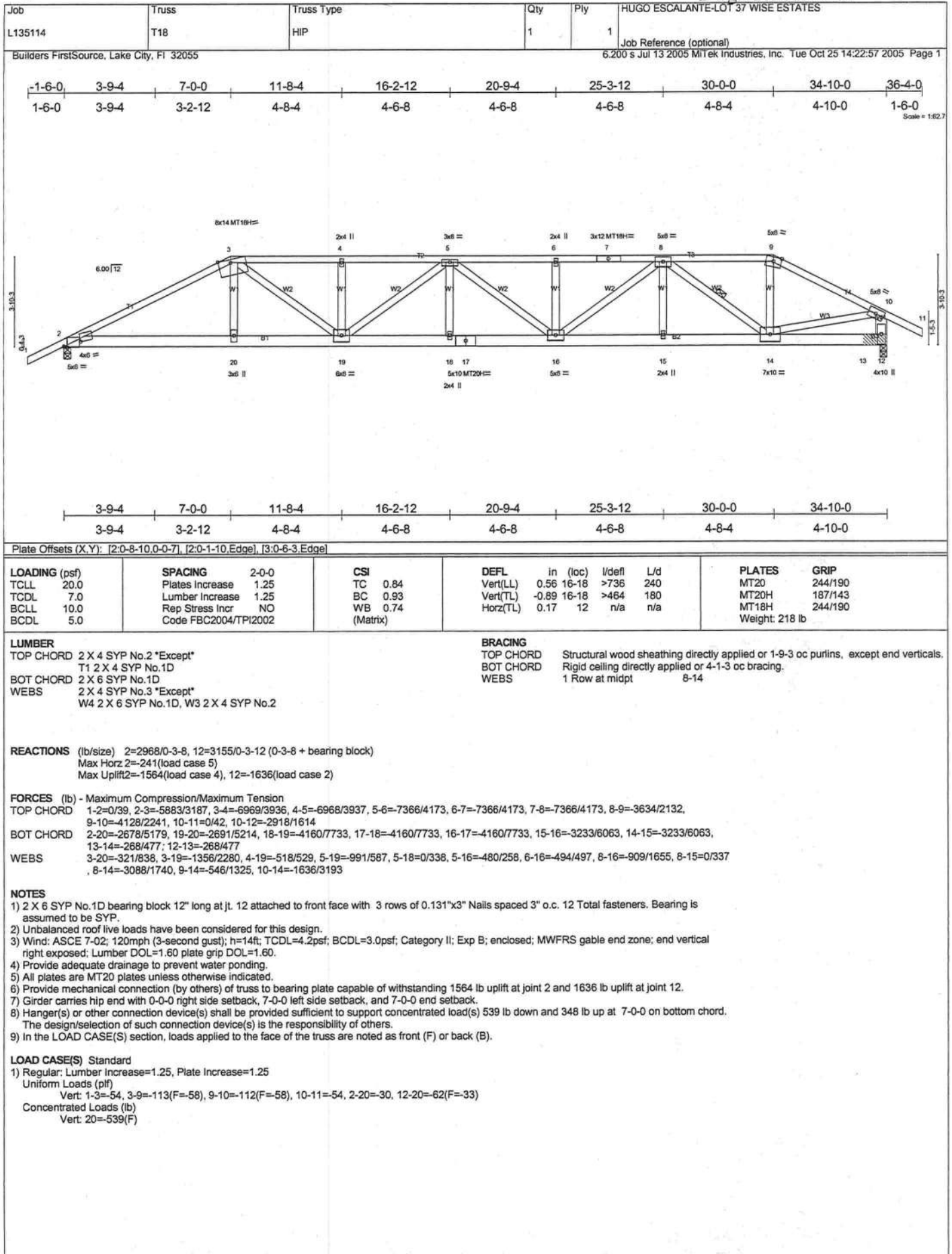
REACTIONS (lb/size) 19=1344/0-3-8, 12=2058/0-3-8, 10=209/0-3-8
 Max Horz 19=-210(load case 6)
 Max Uplift 19=659(load case 5), 12=974(load case 6), 10=319(load case 6)
 Max Grav 19=1344(load case 1), 12=2058(load case 1), 10=290(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-1753/944, 3-4=-1843/1098, 4-5=-1405/866, 5-6=-1334/965, 6-7=-823/642, 7-8=-159/622, 8-9=-181/478, 9-10=-158/419, 10-11=0/35, 2-19=-1227/833
 BOT CHORD 18-19=-268/290, 17-18=-654/1486, 16-17=-782/1856, 15-16=-782/1856, 14-15=-133/783, 13-14=-3/421, 12-13=-3/421, 10-12=-328/280
 WEBS 3-18=-77/113, 3-17=-324/663, 4-17=-404/308, 4-15=-764/492, 5-15=-307/357, 6-15=-610/992, 6-14=-375/219, 7-14=-144/607, 7-12=-1721/911, 9-12=-376/469, 2-18=-513/1214

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint 19, 974 lb uplift at joint 12 and 319 lb uplift at joint 10.

LOAD CASE(S) Standard



Job L135114	Truss T20	Truss Type HIP	Qty 1	Ply 1	HUGO ESCALANTE-LOT 37 WISE ESTATES Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MITek Industries, Inc. Tue Oct 25 14:23:05 2005 Page 1

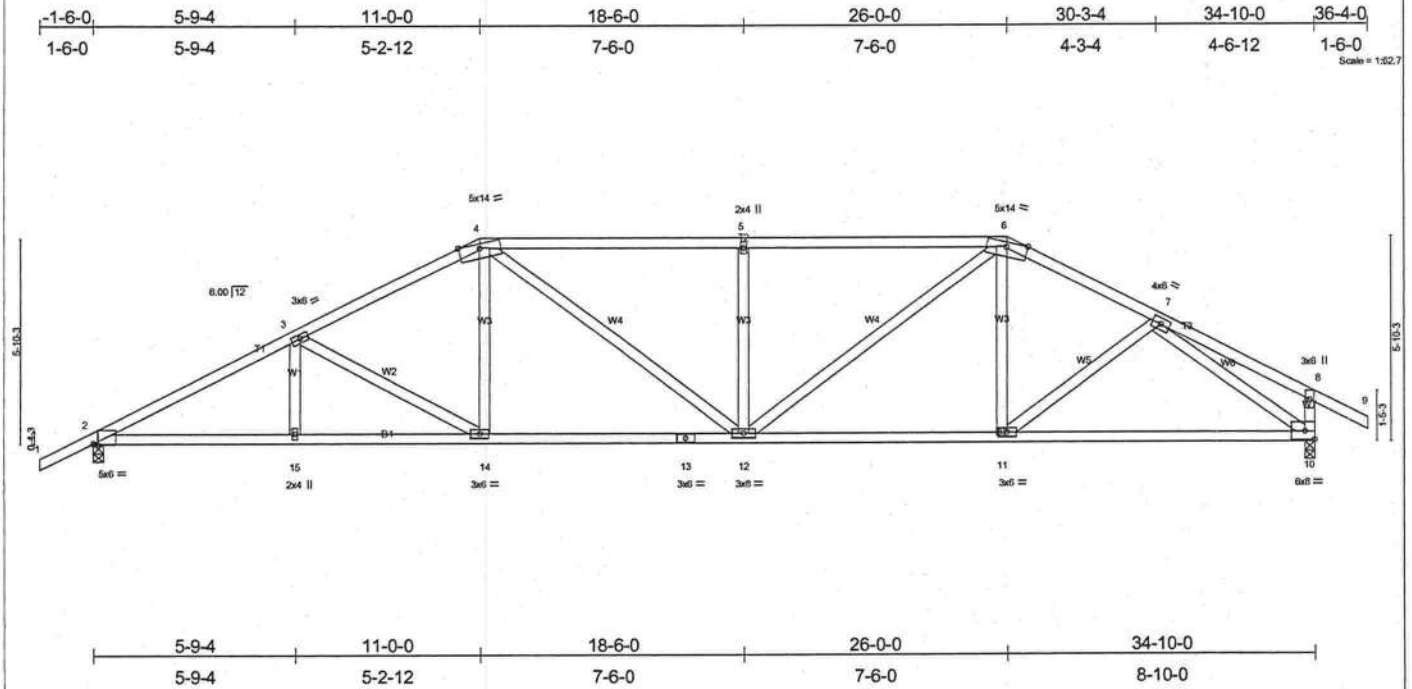


Plate Offsets (X,Y): [2:0-1:10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.68	Vert(LL)	-0.21	12-14	>999	240	
TCDL 7.0	Lumber Increase	1.25	BC 0.60	Vert(TL)	-0.35	12-14	>999	180	
BCLL 10.0	Rep Stress Incr	YES	WB 0.91	Horz(TL)	0.11	10	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 192 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-1-5 oc bracing.

REACTIONS

(lb/size) 2=1540/0-3-8, 10=1540/0-3-8
 Max Horz 2=135(load case 5)
 Max Uplift 2=672(load case 5), 10=644(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-2733/1390, 3-4=-2270/1241, 4-5=-2303/1341, 5-6=-2303/1341, 6-7=-1994/1106, 7-8=-336/172, 8-9=0/40, 8-10=-358/345
 BOT CHORD 2-15=-1073/2365, 14-15=-1073/2365, 13-14=-791/1990, 12-13=-791/1990, 11-12=-662/1754, 10-11=-695/1582
 WEBS 3-15=0/165, 3-14=-442/325, 4-14=-119/410, 4-12=-291/513, 5-12=-429/375, 6-12=-377/762, 6-11=-10/135, 7-11=-133/317, 7-10=-1700/954

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 672 lb uplift at joint 2 and 644 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:23:09 2005 Page 1

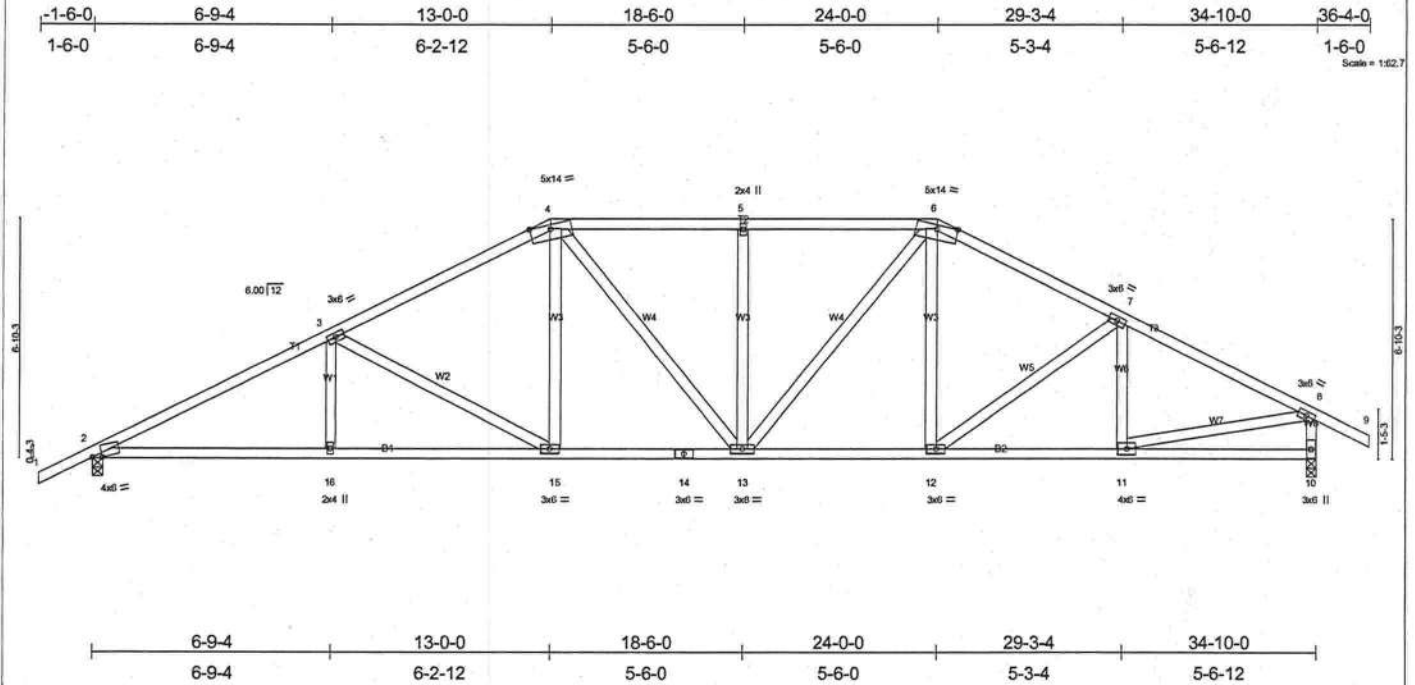


Plate Offsets (X,Y): [2.0-3.4,0.0-11]

LOADING (psf)	SPACING	2.0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.43	Vert(LL)	-0.16 15-16	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.27 15-16	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.56	Horz(TL)	0.10 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 204 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-5-13 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-8 oc bracing.

REACTIONS (lb/size) 2=1540/0-3-8, 10=1540/0-3-8
 Max Horz 2=151(load case 5)
 Max Uplift 2=691(load case 5), 10=665(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/35, 2-3=2705/1394, 3-4=2097/1185, 4-5=1920/1187, 5-6=1920/1187, 6-7=1911/1107, 7-8=1990/1068, 8-9=0/40, 8-10=1448/924
 BOT CHORD 2-16=1066/2337, 15-16=1066/2337, 14-15=687/1814, 13-14=687/1814, 12-13=606/1659, 11-12=730/1717, 10-11=34/162
 WEBS 3-16=0/220, 3-15=608/435, 4-15=172/462, 4-13=189/307, 5-13=303/269, 6-13=244/505, 6-12=78/233, 7-12=109/183, 7-11=196/203, 8-11=742/1592

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 691 lb uplift at joint 2 and 665 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:23:12 2005 Page 1

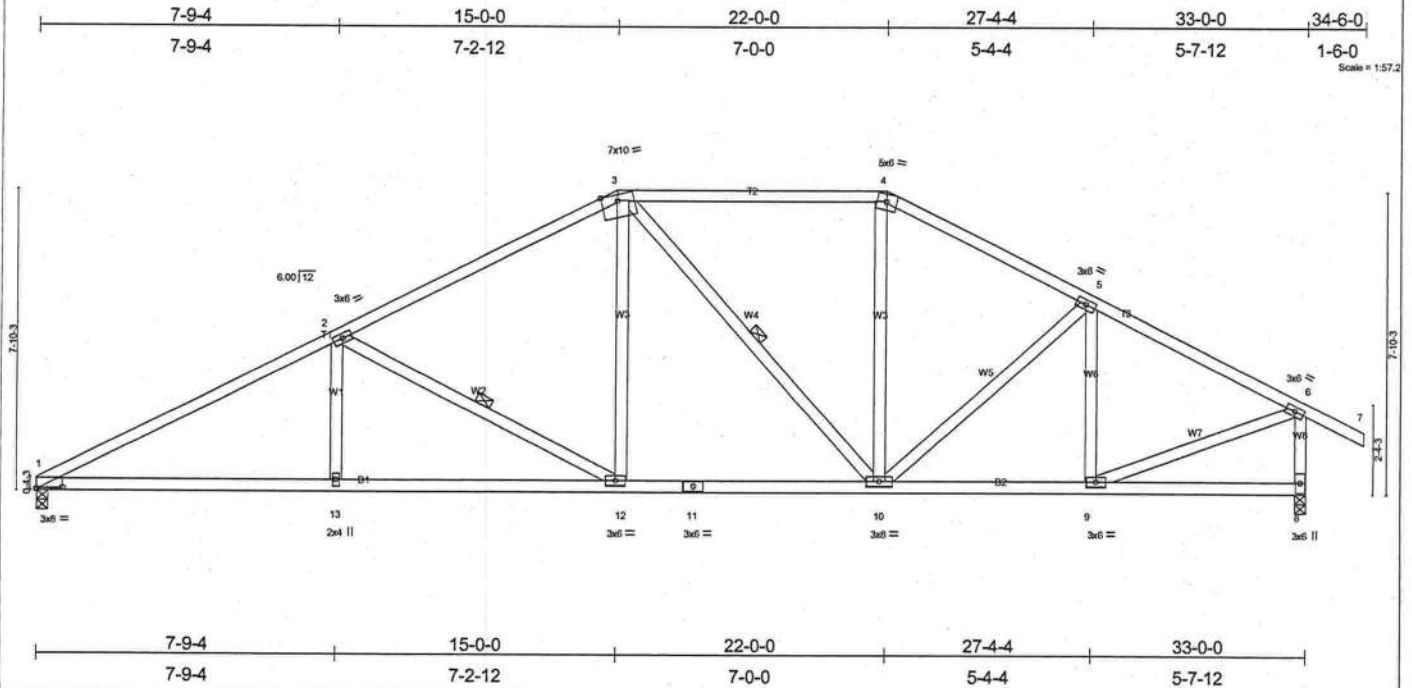


Plate Offsets (X,Y): [1:0-0-0-0-6]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.53	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.71	Vert(LL) -0.20 1-13 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.43	Vert(TL) -0.32 1-13 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.08 8 n/a n/a		
				Weight: 187 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-4-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-9-5 oc bracing.
 WEBS 1 Row at midpt 2-12, 3-10

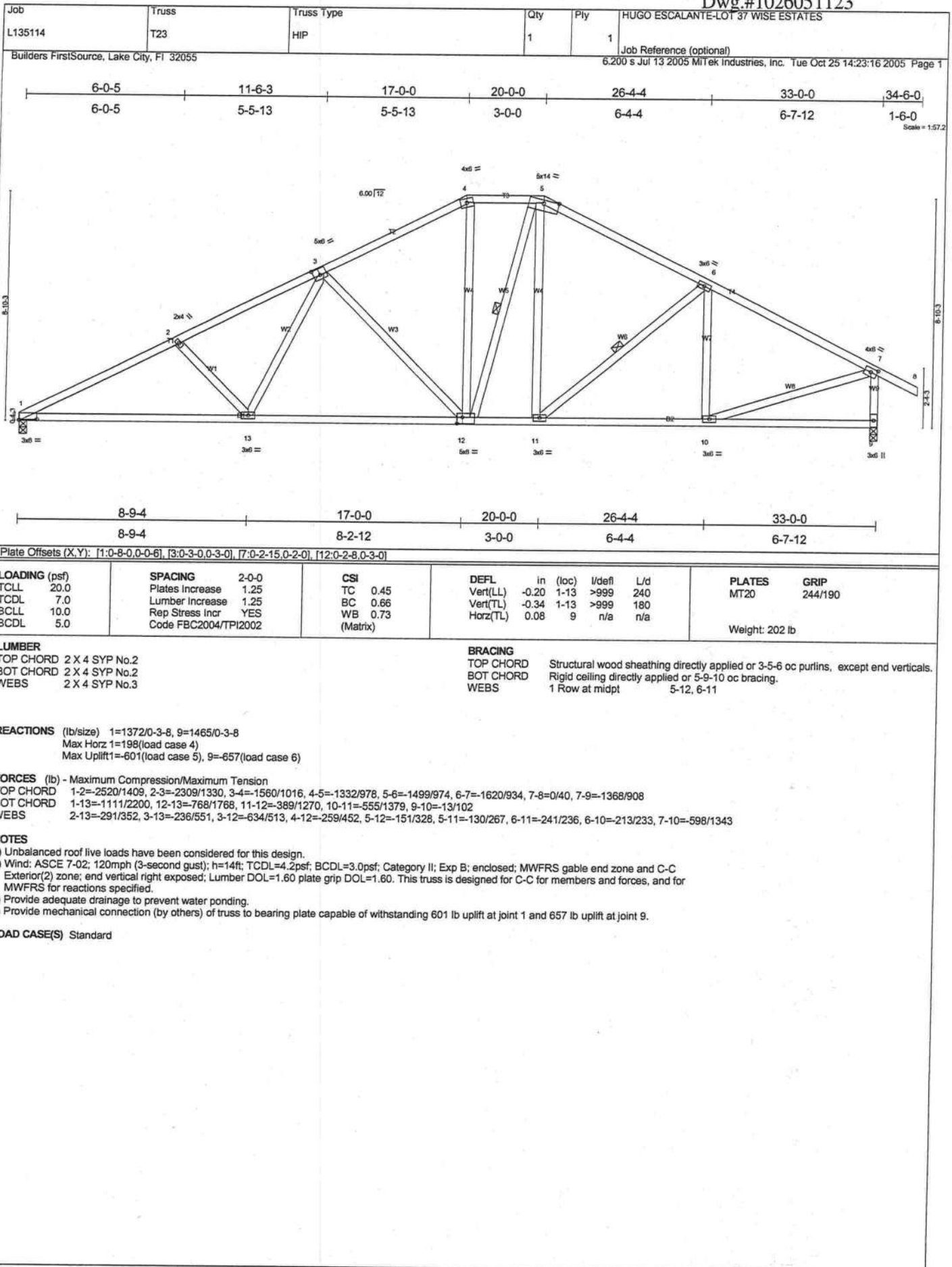
REACTIONS (lb/size) 1=1372/0-3-8, 8=1465/0-3-8
 Max Horz 1=181(load case 4)
 Max Uplift 1=566(load case 5), 8=640(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-2511/1342, 2-3=-1777/1055, 3-4=-1356/950, 4-5=-1568/977, 5-6=-1539/883, 6-7=0/40, 6-8=-1378/897
 BOT CHORD 1-13=-1025/2160, 12-13=-1025/2160, 11-12=-537/1518, 10-11=-537/1518, 9-10=-523/1314, 8-9=-18/77
 WEBS 2-13=0/267, 2-12=-740/558, 3-12=-211/552, 3-10=-346/175, 4-10=-135/335, 5-10=-98/168, 5-9=-322/252, 6-9=-597/1325

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 1 and 640 lb uplift at joint 8.

LOAD CASE(S) Standard



Job L135114	Truss T24	Truss Type COMMON	Qty 2	Ply 1	HUGO ESCALANTE-LOT 37 WISE ESTATES
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		

6.200 s Jul 13 2005 Mitek Industries, Inc. Tue Oct 25 14:23:19 2005 Page 1

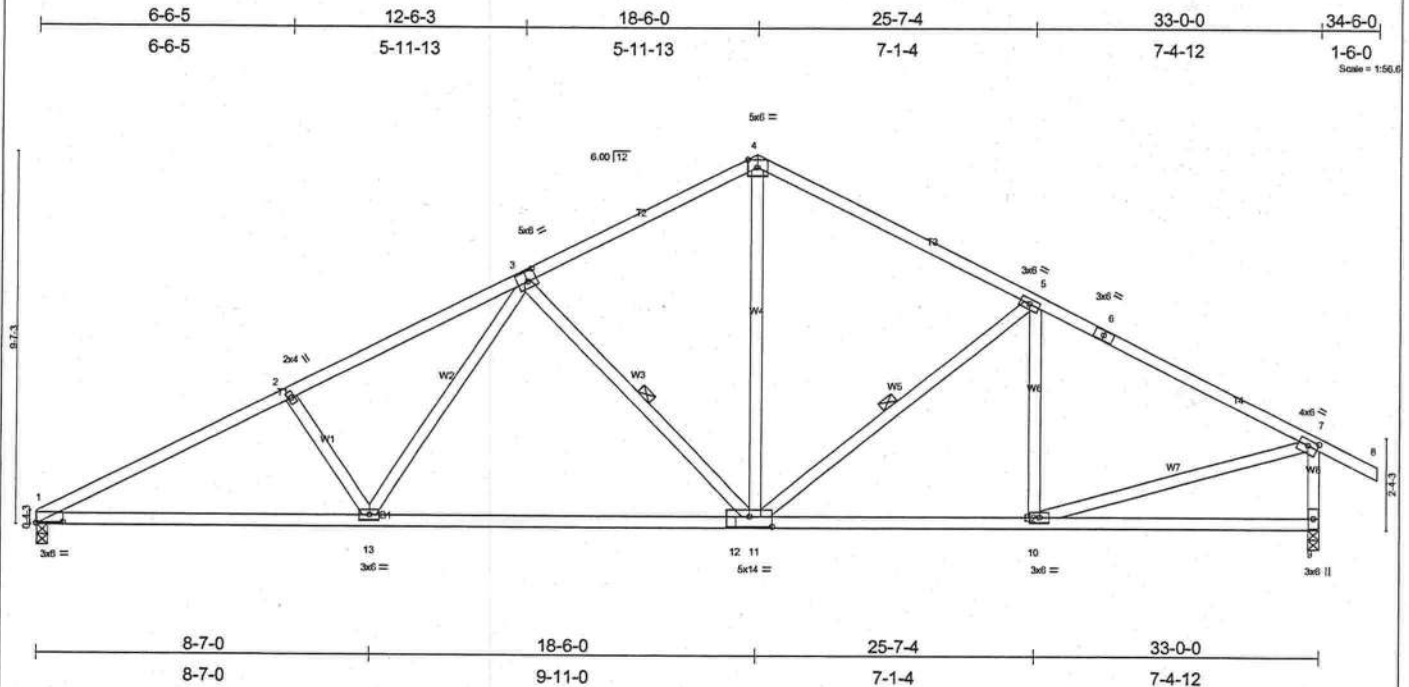


Plate Offsets (X,Y): [1:0-8-0-0-0-6], [3:0-2-12-0-3-0], [7:0-3-0-0-1-12], [12:0-7-0-0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.62	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.84	Vert(LL) -0.26 11-13 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.77	Vert(TL) -0.44 11-13 >886 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.08 9 n/a n/a		
					Weight: 183 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-6-13 oc bracing.
 WEBS 1 Row at midpt 3-11, 5-11

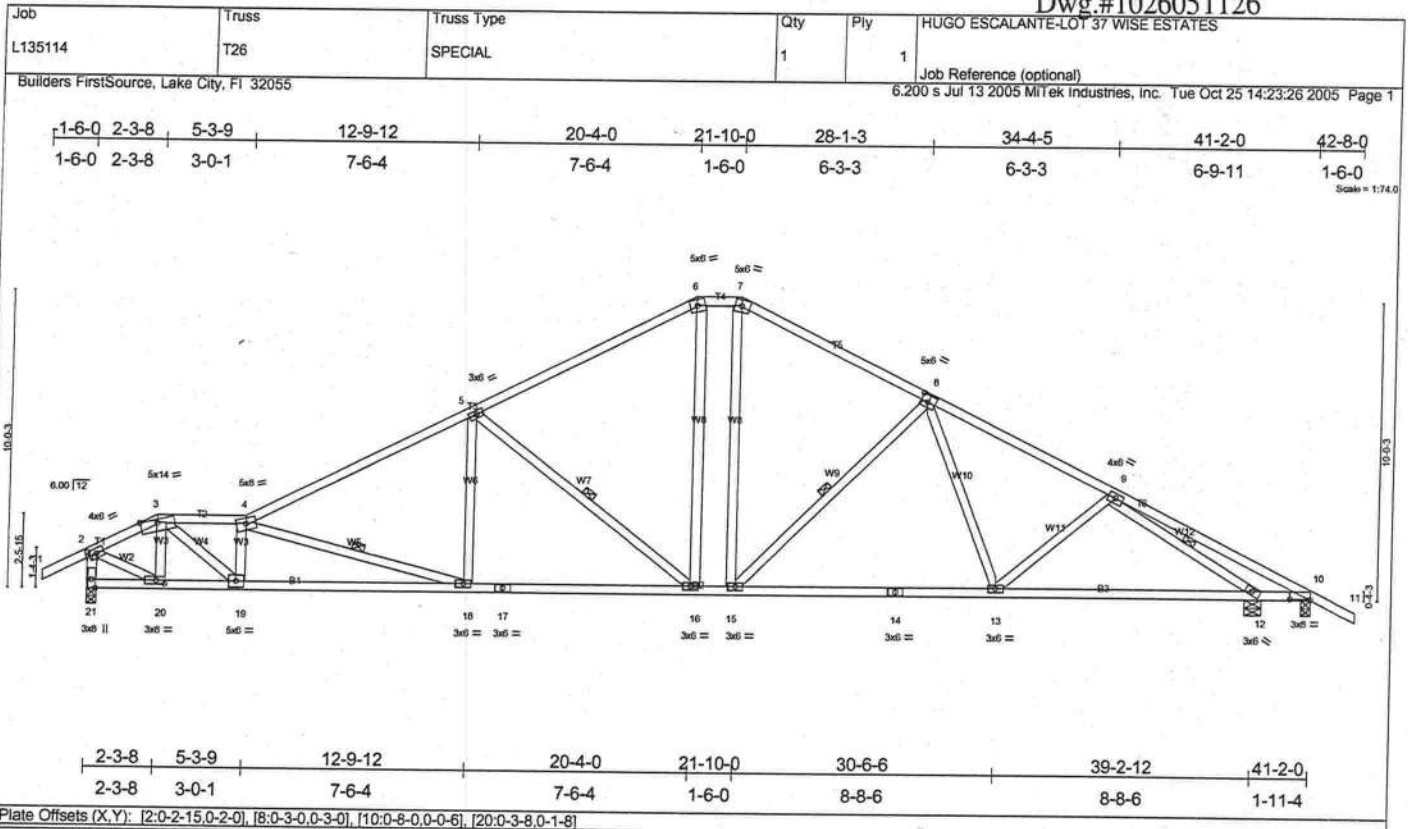
REACTIONS (lb/size) 1=1372/0-3-8, 9=1465/0-3-8
 Max Horz 1=211(load case 5)
 Max Uplift 1=585(load case 5), 9=670(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-2520/1398, 2-3=-2346/1379, 3-4=-1442/966, 4-5=-1463/960, 5-6=-1494/947, 6-7=-1651/933, 7-8=0/40, 7-9=-1350/921
 BOT CHORD 1-13=-1188/2195, 12-13=-807/1676, 11-12=-807/1676, 10-11=-648/1399, 9-10=-55/141
 WEBS 2-13=-304/381, 3-13=-324/666, 3-11=-669/537, 4-11=-506/868, 5-11=-303/287, 5-10=-171/219, 7-10=-617/1310

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02: 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 585 lb uplift at joint 1 and 670 lb uplift at joint 9.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.58	Vert(LL)	-0.31 16-18	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.81	Vert(TL)	-0.49 16-18	>959	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.74	Horz(TL)	0.14 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 251 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-2-14 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-9-3 oc bracing.
 WEBS 1 Row at midpt 4-18, 5-16, 8-15, 9-12

REACTIONS (lb/size) 21=1739/0-3-8, 12=1760/0-6-7, 10=113/0-3-8
 Max Horz 21=-204(load case 6)
 Max Uplift 21=814(load case 5), 12=627(load case 6), 10=-269(load case 6)
 Max Grav 21=1739(load case 1), 12=1760(load case 1), 10=118(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-1763/976, 3-4=-3327/1878, 4-5=-2818/1590, 5-6=-1970/1251, 6-7=-1682/1217, 7-8=-1948/1262, 8-9=-2450/1437, 9-10=-242/163, 10-11=0/35, 2-21=-1682/1052
 BOT CHORD 20-21=-139/209, 19-20=-688/1488, 18-19=-1733/3410, 17-18=-1103/2449, 16-17=-1103/2449, 15-16=-560/1682, 14-15=-877/2032, 13-14=-877/2032, 12-13=-1076/2109, 10-12=-1/138
 WEBS 3-20=-687/349, 3-19=-1294/2318, 4-19=-1256/851, 4-18=-1001/656, 5-18=-149/547, 5-16=-1000/694, 6-16=-328/609, 7-15=-376/635, 8-15=-574/463, 8-13=-96/315, 9-13=-27/230, 9-12=-2382/1347, 2-20=-829/1643

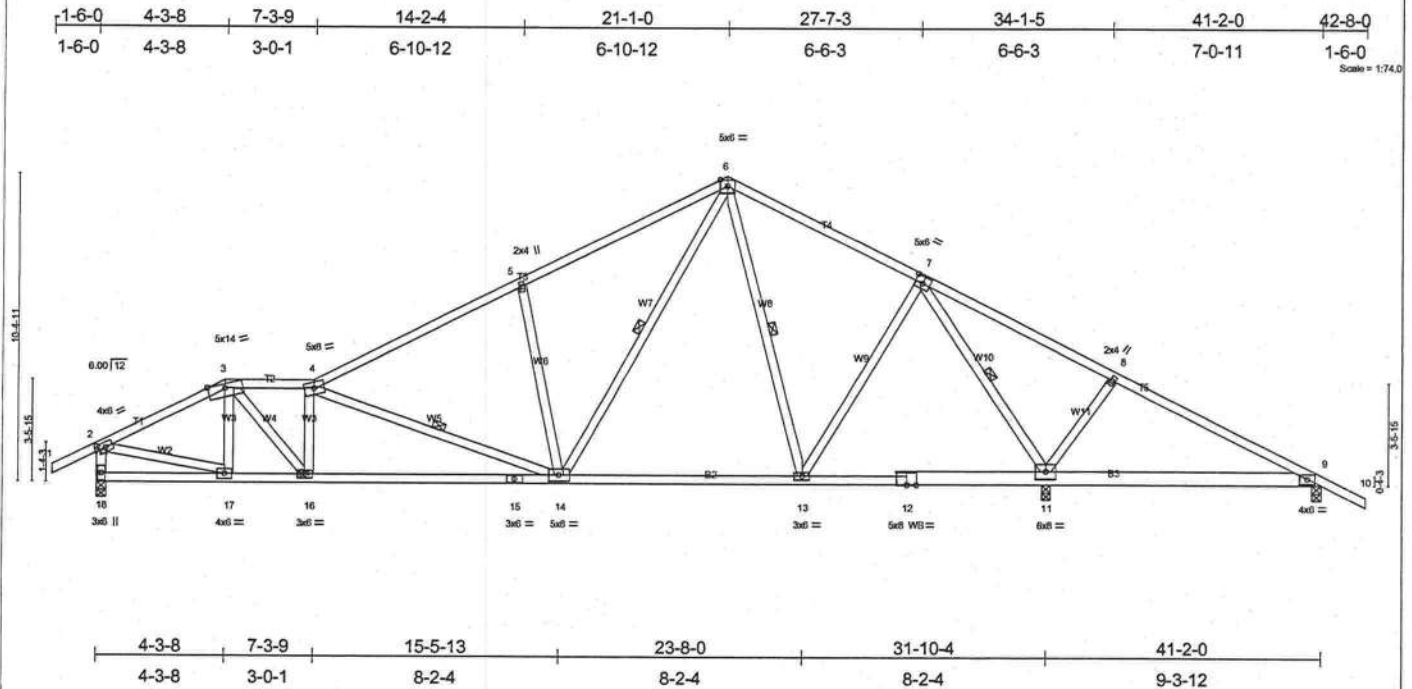
NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 814 lb uplift at joint 21, 627 lb uplift at joint 12 and 269 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L135114	Truss T27	Truss Type SPECIAL	Qty 1	Ply 1	HUGO ESCALANTE-LOT 37 WISE ESTATES
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Oct 25 14:23:30 2005 Page 1

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.43	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.62	Vert(LL) 0.14 9-11 >789 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.57	Vert(TL) 0.12 9-11 >918 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.05 11 n/a n/a		
				Weight: 251 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-1-4 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 4-14, 6-14, 6-13, 7-11

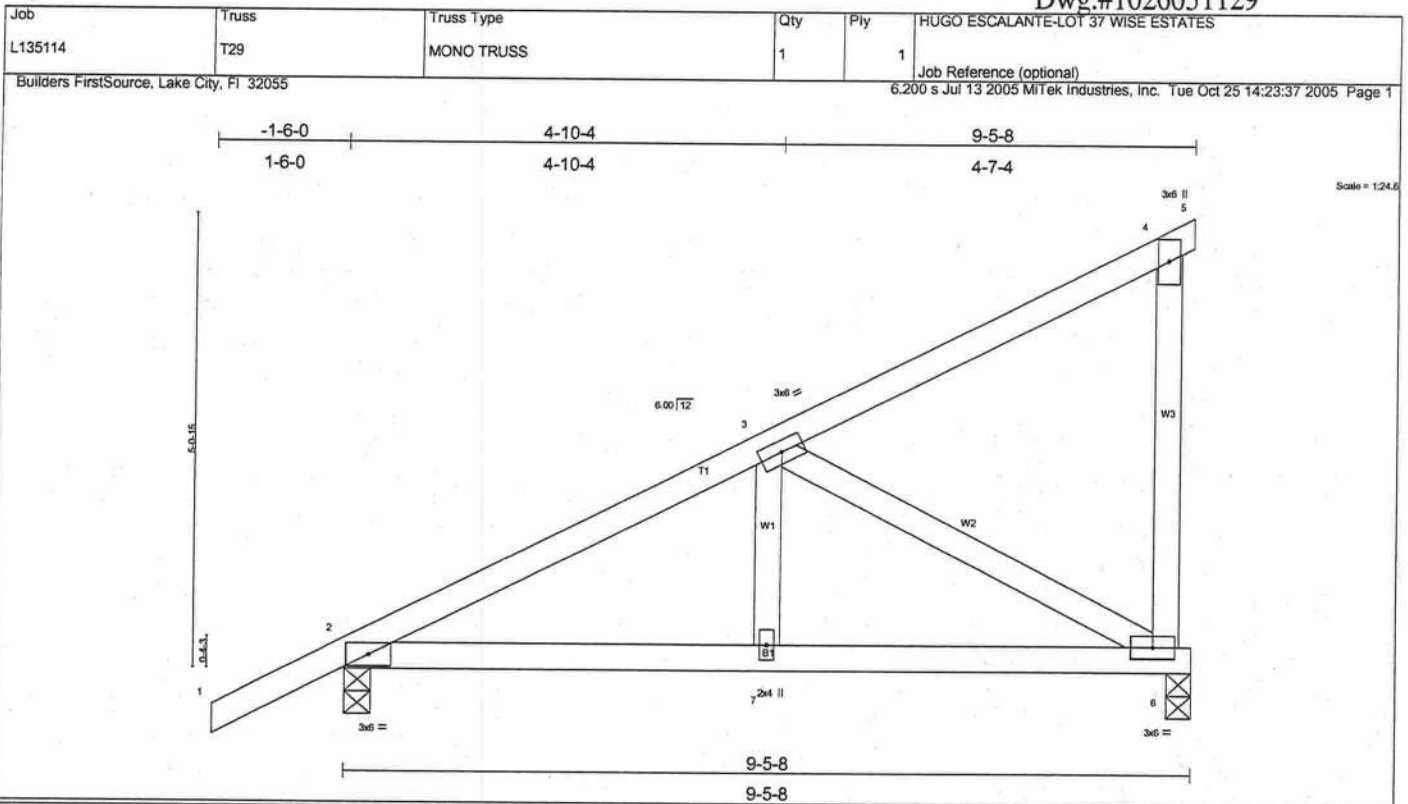
REACTIONS (lb/size) 18=1332/0-3-8, 11=2113/0-3-8, 9=166/0-3-8
 Max Horz 18=-212(load case 6)
 Max Uplift 18=-651(load case 5), 11=-995(load case 6), 9=-303(load case 6)
 Max Grav 18=1332(load case 1), 11=2113(load case 1), 9=268(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-1610/869, 3-4=-2127/1211, 4-5=-1585/920, 5-6=-1513/1062, 6-7=-836/636, 7-8=-266/715, 8-9=-234/510, 9-10=0/39, 2-18=-1247/817
 BOT CHORD 17-18=-166/215, 16-17=-637/1371, 15-16=-968/2157, 14-15=-968/2157, 13-14=-121/761, 12-13=0/374, 11-12=0/376, 9-11=-426/370
 WEBS 3-17=-283/164, 3-16=-596/1133, 4-16=-848/493, 4-14=-865/557, 5-14=-377/449, 6-14=-684/1077, 6-13=-342/233, 7-13=-174/633, 7-11=-1771/987, 8-11=-349/436, 2-17=-613/1305

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 18, 995 lb uplift at joint 11 and 303 lb uplift at joint 9.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.23	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.14	Vert(LL) 0.04 2-7 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.20	Vert(TL) 0.04 2-7 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.01 6 n/a n/a		
	Code FBC2004/TP12002			Weight: 49 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 W3 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.

REACTIONS

(lb/size) 6=375/0-3-8, 2=476/0-3-8
 Max Horz 2=315(load case 5)
 Max Uplift 6=385(load case 5), 2=374(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-512/540, 3-4=-81/27, 4-5=-2/0, 4-6=-104/147
 BOT CHORD 2-7=-723/407, 6-7=-723/407
 WEBS 3-6=-439/781, 3-7=-324/147

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 6 and 374 lb uplift at joint 2.

LOAD CASE(S) Standard

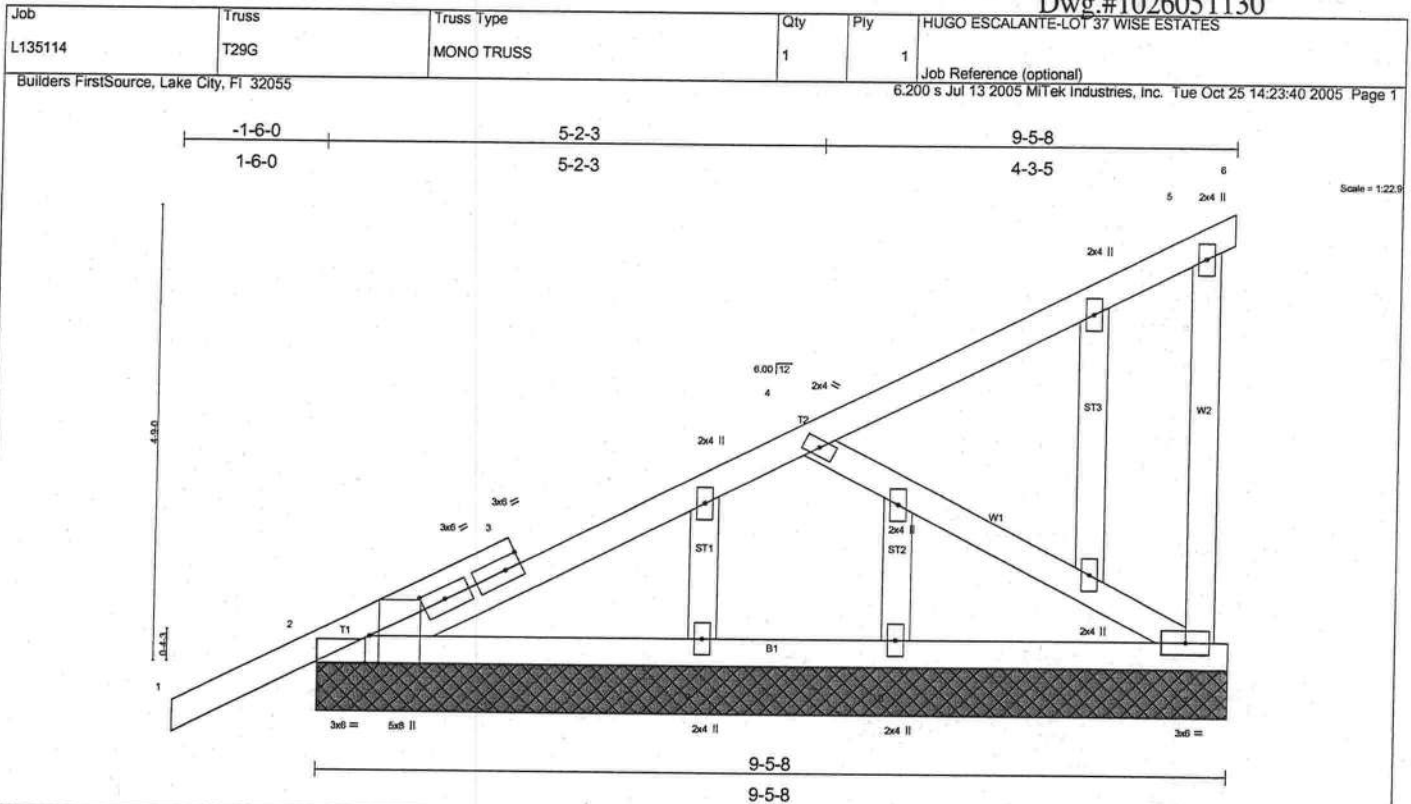


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.24	Vert(LL)	0.02	1	n/r	120	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.23	Vert(TL)	0.03	1	n/r	90		
BCLL 10.0	Lumber Increase 1.25	WB 0.18	Horz(TL)	-0.01	6	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TP12002								
								Weight: 56 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purfins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 9-0-2 oc bracing.

REACTIONS

(lb/size) 2=522/9-5-8, 6=-161/9-5-8, 7=582/9-5-8, 9=147/9-5-8, 8=52/9-5-8
 Max Horz 2=298(load case 5)
 Max Uplift 2=-313(load case 5), 6=-161(load case 1), 7=486(load case 5), 9=-12(load case 5)
 Max Grav 2=522(load case 1), 6=161(load case 5), 7=582(load case 1), 9=147(load case 1), 8=52(load case 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-6/51, 2-3=-506/240, 3-4=-450/238, 4-5=-92/54, 5-6=-82/100, 5-7=-323/433
 BOT CHORD 2-9=-462/402, 8-9=-462/402, 7-8=-462/402
 WEBS 4-7=-447/515

NOTES

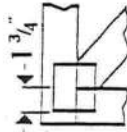
- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mitek "Standard Gable End Detail"
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 161 lb uplift at joint 6, 486 lb uplift at joint 7 and 12 lb uplift at joint 9.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

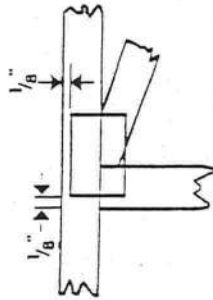
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-5=-79(F=-25), 5-6=-79(F=-25), 2-7=-30

Symbols

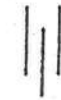
PLATE LOCATION AND ORIENTATION



- Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



- For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



- This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 x 4

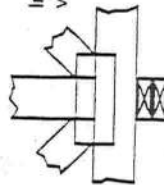
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



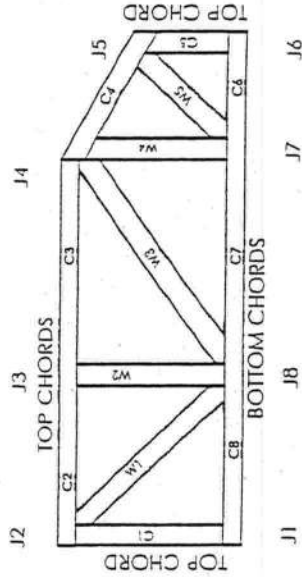
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILIR	960022-W, 970036-N
NER	561



MITek Engineering Reference Sheet: MIT-7473

General Safety Notes

Failure to Follow Could Cause Properly Damage or Personal Injury

- Provide copies of this truss design to the building designer, erection supervisor, properly owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length ($\pm 6"$ from adjacent joint.)
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size and location dimensions shown indicate minimum plating requirements.
- Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- Do not overload roof or floor trusses with stacks of construction materials.
- Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling, erection and installation of trusses.

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Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0510-92 Date Received 10-31-05 By G Permit # 23825/882
 Application Approved by - Zoning Official BLK Date 03-11-05 Plans Examiner AK 37H Date 11-4-05
 Flood Zone X PC PLAT Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Den.
 Comments Need letter of finished floor elevation 100.0' finished floor
EH needed before sign.

Applicants Name Hugo Escalante Phone 386-288-8666
 Address P.O. Box 280, Fort White, FL 32038
 Owners Name HBM Construction Phone 386-288-8666
 911 Address 269 S.W. Plateau Glen C.C. 32024
 Contractors Name Hugo Escalante (EWPL Inc) Phone 386-288-8666
 Address P.O. Box 280, Ft White, FL 32038
 Fee Simple Owner Name & Address N/A
 Bonding Co. Name & Address N/A
 Architect/Engineer Name & Address Daniel Shoheen, Lake City, FL 32038
 Mortgage Lenders Name & Address N/A
 Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy
 Property ID Number 24-45-16-03113-167 Estimated Cost of Construction 135,000
 Subdivision Name WISE ESTATES Lot 37 Block C Unit Phase
 Driving Directions 47 to 242 make right to WISE DR, make left at Gardner
Venue, go to Plateau make right to end lot on left
 Type of Construction New Single Family Residence Number of Existing Dwellings on Property 0
 Total Acreage 66 Acre Lot Size 66 Acre Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive
 Actual Distance of Structure from Property Lines - Front 65' Side 50' Side 50' Rear 75'
 Total Building Height 90'-6" Number of Stories 1 Heated Floor Area 1932 Sq Ft Roof Pitch 6-12'
5740' 2640'

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Hugo Escalante
 Owner/Builder or Agent (Including Contractor)

STATE OF FLORIDA
 COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
 this 31st day of October 2005.
 Personally known X or Produced Identification

Hugo Escalante
 Contractor Signature
 Contractors License Number CRC132867
 Competency Card Number
 NOTARY STAMP/SEAL



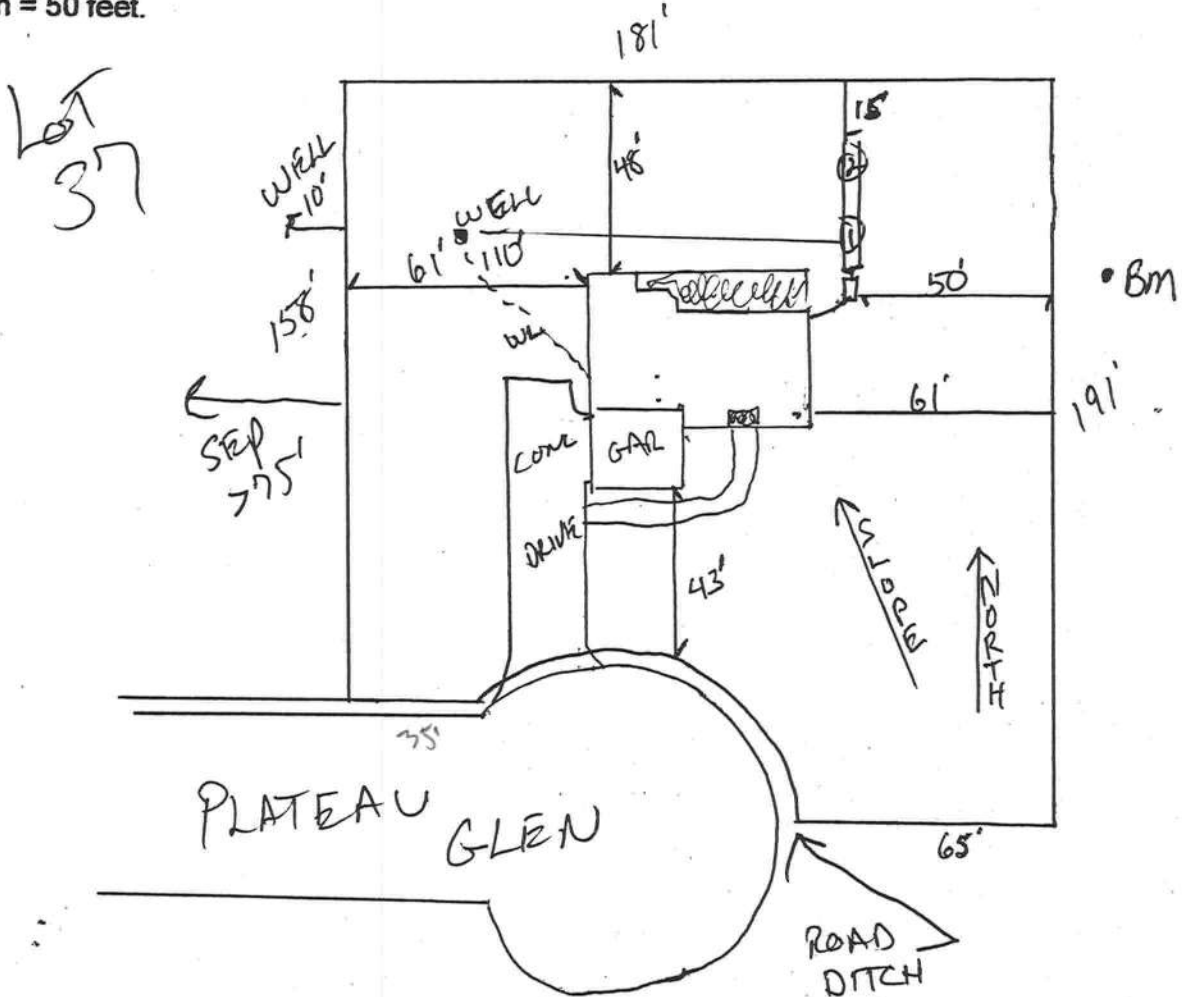
Garrie L. Revelle
 Notary Signature

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1007N

----- PART II - SITEPLAN -----

Scale: 1 inch = 50 feet.

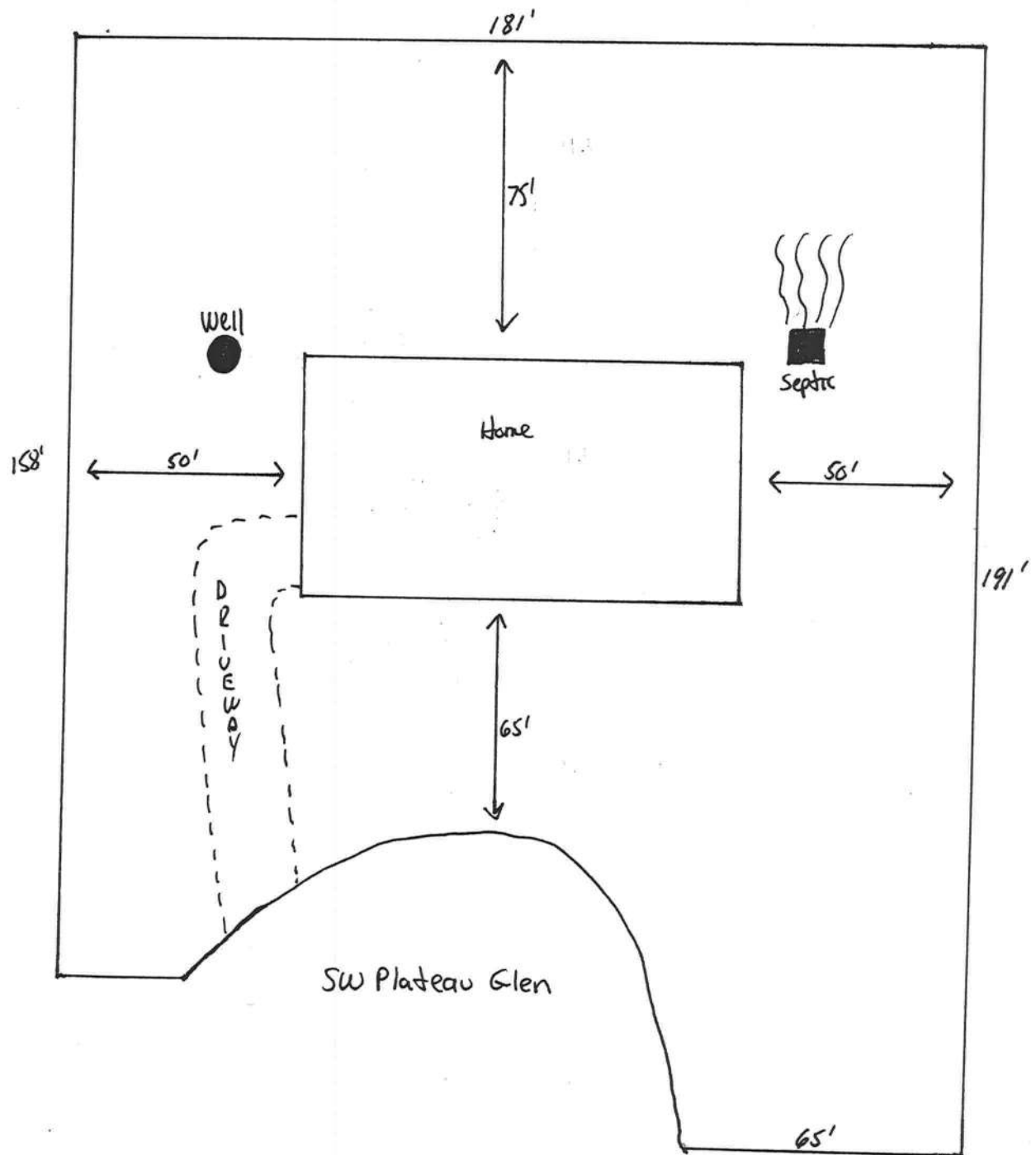


Notes: _____

Site Plan submitted by: Rock D F
Plan Approved ☒ Not Approved _____
By Mr S Land Columbia County Health Department
MASTER CONTRACTOR
Date 10-3-05

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Lot 37 Wise Estates
Parcel # 24-45-16-03113-167
269 S.W. Plateau Glen



R=1472.40'
L=109.20'
Δ=4°14'57"

R=1472.40'
L=163.58'
Δ=6°21'55"

R=1472.40'
L=172.98'
Δ=6°43'53"

DETAIL
N.T.S.

PARK

MINIMUM FLOOR ELEVATION

(as determined by Project Engineer)

BLOCK A

1 - 97.5	11 - 94.3
2 - 99.0	12 - 95.0
3 - 99.5	13 - 93.5
4 - 99.5	14 - 93.0
5 - 98.5	15 - 93.0
6 - 97.2	16 - 95.0
7 - 98.0	17 - 92.5
	18 - 93.2
	19 - 95.5
	20 - 96.5

BLOCK B

1 - 100.0	21 - 96.0
2 - 99.0	22 - 95.2
3 - 96.5	23 - 95.2
4 - 92.5	24 - 97.2
5 - 92.23	25 - 98.3
6 - 92.2	26 - 98.3
	27 - 97.3
	28 - 95.5
	29 - 93.0
	30 - 92.2

BLOCK C

1 - 97.3	31 - 93.0
2 - 98.5	32 - 95.0
3 - 99.3	33 - 96.5
4 - 99.5	34 - 98.5
5 - 99.0	35 - 100.5
6 - 97.0	36 - 100.5
7 - 98.0	37 - 100.0
8 - 99.0	38 - 99.5
9 - 96.3	39 - 95.0
10 - 94.3	40 - 97.2
	41 - 95.0
	42 - 93.2

NCE

Sec. 23
e survey

ND

SHEET 3 OF 4

PLAT DATE: 02/20/2004



Donald F. Lee and Associates, Inc.

SURVEYORS

ENGINEERS

140 Northwest Ridgewood Avenue, Lake City, Florida 32055

Phone: (386) 755-6166

FAX: (386) 755-6167

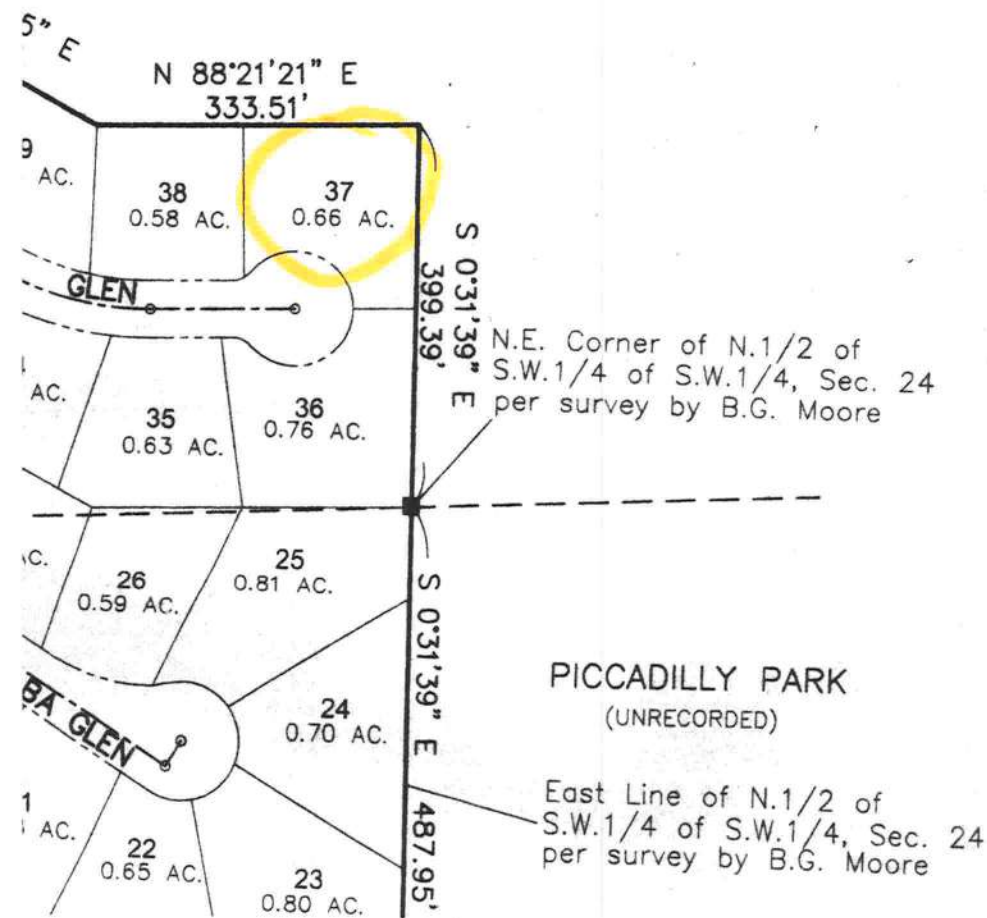
LB# 7042

WISE ESTATES

IN SECTIONS 23, 24, AND 26
TOWNSHIP 4 SOUTH, RANGE 16 EAST
COLUMBIA CO., FLORIDA



UNPLATTED LANDS
PETER GIEBEIG
60 ACRES±



NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 24-45-16-03113-167

1. Description of property: (legal description of the property and street address or 911 address)

Lot 37 Block C WISE Estates S/D WD 1018-2945

2. General description of improvement: New Single Family Residence

3. Owner Name & Address HBM Construction Corporation 10155 COLLINS AVE,
Suite 1004 Bgll Harbour, FL 33154 Interest in Property 100%

4. Name & Address of Fee Simple Owner (if other than owner): N/A

5. Contractor Name Hugo Escobedo (CWPL LLC) Phone Number 386-288-8666
Address P.O. BOX 280 Ford White FL 32038

6. Surety Holders Name N/A
Address N/A Inst: 2005027042 Date: 10/31/2005 Time: 11:19
Amount of Bond N/A MK DC, P. DeWitt Cason, Columbia County B: 1063 P: 1325

7. Lender Name N/A
Address N/A

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name Hugo Escobedo Phone Number 3862888666
Address P.O. BOX 280 Ford White, FL 32038

9. In addition to himself/herself the owner designates Hugo Escobedo of
Ford White, FL to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) -
(a) 7. Phone Number of the designee 386-288-8666

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording,
(Unless a different date is specified) _____

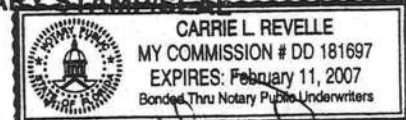
NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Hugo Escobedo
Signature of Owner

Sworn to (or affirmed) and subscribed before
day of October 31st, 2005

NOTARY STAMP/SEAL



Carrie L. Revelle
Signature of Notary

Columbia County Property Appraiser

DB Last Updated: 9/16/2005

2005 Proposed Values

Parcel: 24-4S-16-03113-167

Tax Record

Property Card

Interactive GIS Map

Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	H & M CONSTRUCTION CORPORATION
Site Address	PLATEAU
Mailing Address	10155 COLLINS AVE SUITE 1004 BALL HARBOUR, FL 33154
Brief Legal	LOT 37 BLOCK C WISE ESTATE S/D WD 1018-2245.

Use Desc. (code)	VACANT (000000)
Neighborhood	24416.00
Tax District	2
UD Codes	MKTA06
Market Area	06
Total Land Area	0.660 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	\$20,500.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (0)	\$0.00
XFOB Value	cnt: (0)	\$0.00
Total Appraised Value		\$20,500.00

Just Value	\$20,500.00
Class Value	\$0.00
Assessed Value	\$20,500.00
Exempt Value	\$0.00
Total Taxable Value	\$20,500.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
6/18/2004	1018/2245	WD	V	Q		\$92,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.660AC)	1.00/1.00/1.00/1.00	\$20,500.00	\$20,500.00

Columbia County Property Appraiser

DB Last Updated: 9/16/2005

1 of 1

Disclaimer

Columbia County Building Department Culvert Permit

Culvert Permit No.
000000882

DATE 11/07/2005 PARCEL ID # 24-4S-16-03113-167
APPLICANT HUGO ESCALANTE PHONE 386.288.8666
ADDRESS POB 280 FT. WHITE FL 32038
OWNER H&M CONSTRUCTION PHONE 386.288.8666
ADDRESS 269 S PLATEAU GLEN LAKE CITY FL 32038
CONTRACTOR HUGO ESCALANTE PHONE 386.288.8666
LOCATION OF PROPERTY 47-S TO C-242, TR TO WISE DRIVE, TO GARDNER TERRACE, TL, GO TO PLATEAU
, TR TOWARDS END, LOT ON L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT WISE ESTATES 37 C

SIGNATURE



INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
- b) the driveway to be served will be paved or formed with concrete.

Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALLATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave

Lake City, FL. 32025

Phone 386-752-6677

Fax 386-752-1477

37
Lot 37 WISEBuilding Permit # _____ Owner's Name Kapkin LLC

Well Depth _____ Ft. Casing Depth _____ Ft. Water Level _____ Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well SubmersiblePump Make Red Jacket Pump Model 100F21120G8 HP 1System Pressure (PSI) _____ On 30 Off 50 Average Pressure 30X 40Pumping System GPM at average pressure and pumping level 20 (GPM)Tank Installation: Precharged Bladder Make Challenger Model PC244 Size _____Tank Draw-down per cycle at system pressure 26.1 gallons**I HEREBY VERIFY THAT THIS WATER WELL SYSTEM HAS BEEN
INSTALLED AS PER THE ABOVE INFORMATION.**Linda Newcomb
SignatureLinda Newcomb
Print Name2609
License Number6-6-05
Date

Residential System Sizing Calculation

Summary

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

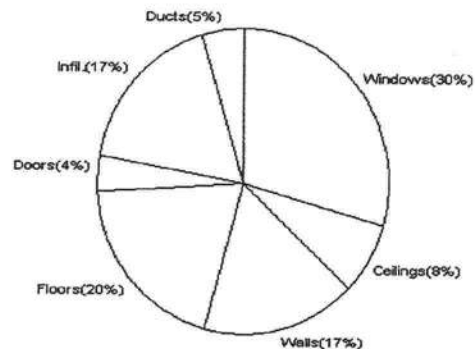
9/21/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)			
Winter design temperature	31 F	Summer design temperature	93 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	39 F	Summer temperature difference	18 F
Total heating load calculation	32409 Btuh	Total cooling load calculation	31653 Btuh
Submitted heating capacity	36000 Btuh	Submitted cooling capacity	36000 Btuh
Submitted as % of calculated	111.1 %	Submitted as % of calculated	113.7 %

WINTER CALCULATIONS

Winter Heating Load (for 1932 sqft)

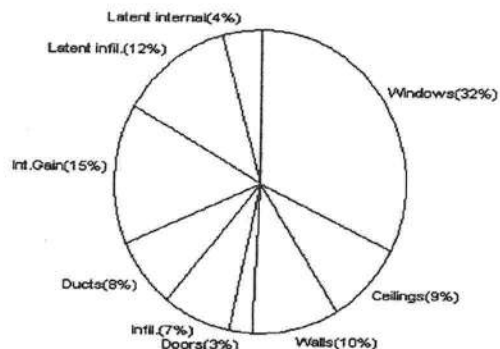
Load component		Load
Window total	339 sqft	9594 Btuh
Wall total	1892 sqft	5517 Btuh
Door total	80 sqft	1260 Btuh
Ceiling total	1932 sqft	2512 Btuh
Floor total	204 ft	6446 Btuh
Infiltration	129 cfm	5537 Btuh
Subtotal		30866 Btuh
Duct loss		1543 Btuh
TOTAL HEAT LOSS		32409 Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1932 sqft)

Load component		Load
Window total	339 sqft	10254 Btuh
Wall total	1892 sqft	3130 Btuh
Door total	80 sqft	798 Btuh
Ceiling total	1932 sqft	2743 Btuh
Floor total		0 Btuh
Infiltration	113 cfm	2236 Btuh
Internal gain		4800 Btuh
Subtotal(sensible)		23961 Btuh
Duct gain		2396 Btuh
Total sensible gain		26357 Btuh
Latent gain(infiltration)		3916 Btuh
Latent gain(internal)		1380 Btuh
Total latent gain		5296 Btuh
TOTAL HEAT GAIN		31653 Btuh



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: *[Signature]*

DATE: 9-21-05

Manual J Winter Calculations

Residential Load - Component Details (continued)

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

9/21/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Winter

Residential Load - Component Details

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

9/21/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	84.0	28.3	2377 Btuh
2	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
3	2, Clear, Metal, DEF	E	30.0	28.3	849 Btuh
4	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	SW	21.0	28.3	594 Btuh
6	2, Clear, Metal, DEF	S	70.0	28.3	1981 Btuh
7	2, Clear, Metal, DEF	N	16.0	28.3	453 Btuh
8	2, Clear, Metal, DEF	W	21.0	28.3	594 Btuh
9	2, Clear, Metal, DEF	N	12.5	28.3	354 Btuh
10	2, Clear, Metal, DEF	S	42.0	28.3	1189 Btuh
Window Total			339		9594 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	232	1.6	371 Btuh
2	Frame - Exterior	13.0	1660	3.1	5146 Btuh
Wall Total			1892		5517 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		20	17.9	359 Btuh
2	Wood - Adjac		20	9.2	184 Btuh
3	Wood - Exter		40	17.9	718 Btuh
Door Total			80		1260 Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1932	1.3	2512 Btuh
Ceiling Total			1932		2512 Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	204.0 ft(p)	31.6	6446 Btuh
Floor Total			204		6446 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	19320(sqft)	129	5537 Btuh
	Mechanical			0	0 Btuh
Infiltration Total				129	5537 Btuh

Totals for Heating	Subtotal	30866 Btuh
	Duct Loss(using duct multiplier of 0.05)	1543 Btuh
	Total Btuh Loss	32409 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

9/21/2005

Totals for Cooling	Subtotal	23961 Btuh
	Duct gain(using duct multiplier of 0.10)	2396 Btuh
	Total sensible gain	26357 Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	3916 Btuh
	Latent occupant gain (6 people @ 230 Btuh per person)	1380 Btuh
	Latent other gain	0 Btuh
	TOTAL GAIN	31653 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC
209 SW Paisley Ct.
Lake City, FL 32024-

Project Title:
THE NATHAN 4-BED

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 18.0 F

9/21/2005

Window	Type	Overhang		Window Area(sqft)			HTM		Load		
	Panes/SHGC/U/InSh/ExSh Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded			
1	2, Clear, DEF, N, N	N	1.5	7.5	84.0	0.0	84.0	22	22	1848	Btuh
2	2, Clear, DEF, N, N	N	6	3	12.5	0.0	12.5	22	22	275	Btuh
3	2, Clear, DEF, N, N	E	1.5	5.5	30.0	4.5	25.5	22	72	1936	Btuh
4	2, Clear, DEF, N, N	S	1.5	7	30.0	30.0	0.0	22	37	660	Btuh
5	2, Clear, DEF, N, N	SW	8	7.5	21.0	21.0	0.0	22	62	462	Btuh
6	2, Clear, DEF, N, N	S	8	8	70.0	35.0	35.0	22	37	2065	Btuh
7	2, Clear, DEF, N, N	N	1.5	6	16.0	0.0	16.0	22	22	352	Btuh
8	2, Clear, DEF, N, N	W	1.5	7.5	21.0	1.1	19.9	22	72	1456	Btuh
9	2, Clear, DEF, N, N	N	1.5	3	12.5	0.0	12.5	22	22	275	Btuh
10	2, Clear, DEF, N, N	S	1.5	8	42.0	42.0	0.0	22	37	924	Btuh
Window Total					339					10254 Btuh	
Walls	Type	R-Value			Area		HTM		Load		
1	Frame - Adjacent	13.0			232.0		1.0		241 Btuh		
2	Frame - Exterior	13.0			1660.0		1.7		2888 Btuh		
Wall Total					1892.0				3130 Btuh		
Doors	Type				Area		HTM		Load		
1	Wood - Exter				20.0		10.0		200 Btuh		
2	Wood - Adjac				20.0		10.0		200 Btuh		
3	Wood - Exter				40.0		10.0		399 Btuh		
Door Total					80.0				798 Btuh		
Ceilings	Type/Color	R-Value			Area		HTM		Load		
1	Under Attic/Dark	30.0			1932.0		1.4		2743 Btuh		
Ceiling Total					1932.0				2743 Btuh		
Floors	Type	R-Value			Size		HTM		Load		
1	Slab-On-Grade Edge Insulation	0.0			204.0 ft(p)		0.0		0 Btuh		
Floor Total					204.0				0 Btuh		
Infiltration	Type	ACH			Volume		CFM=		Load		
	Natural	0.35			19320		112.9		2236 Btuh		
	Mechanical						0		0 Btuh		
	Infiltration Total						113		2236 Btuh		
Internal gain	Occupants			Btuh/occupant		Appliance		Load			
	6			X 300 +		3000		4800 Btuh			

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

0510-92

Reference to: Build permit application Number:

Hugo Escalate Owner H & M Construction Corp. Lot 37 Block C
of Wise Estates

On the date of November 3, 2005 application 0510-92 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

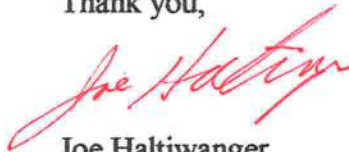
Please include application number 0510-92 when making reference to this application.

1. Application 0510-92 which was filed with the building department on the date of October 31, 2005 will be reviewed under the Florida Building Code 2004. The Wind Load design by Mr. Mark Disosway was design under the Florida Building Code 2001. The wind Load design should reflect the code sections of the Florida Building Code 2004 that relate to wind Load doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors. R309 design code requirements.
2. Please show compliance with sections R309 of the FRC-2004 R309.1 Opening protection. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel. 1.1 Duct penetration: Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a

minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage. R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

✓ 3. Please show compliance with section R322 of the FRC-2004: All new single-family houses, duplexes, triplexes, condominiums and townhouses shall provide at least one bathroom, located with maximum possible privacy, where bathrooms are provided on habitable grade levels, with a door that has a 29-inch (737 mm) clear opening. However, if only a toilet room is provided at grade level, such toilet rooms shall have a clear opening of not less than 29 inches (737 mm).

Thank you,



Joe Haltiwanger
Plan Examiner
Columbia County Building Department

COLUMBIA COUNTY FLORIDA DEPARTMENT OF BUILDING AND ZONING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03113-167

Building permit No. 000023825

Use Classification SFD/UTILITY

Fire: 17.76

Permit Holder HUGO ESCALANTE

Waste: 36.75

Owner of Building H&M CONSTRUCTION

Total: 54.51

Location: 269 SW PLATEAU GLEN

Date: 07/03/2006

Henry Dicker

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



Project Information for:		L135114
Builder:	Hugo Escalante	Date: 1/19/2006
Lot:	Lot 37	Start Number: 1048
Subdivision:	Wise Estates	
County or City:	Columbia County	
Truss Page Count:	41	

Truss Design Load Information (UNO)		Design Program: MiTek 5.2 / 6.2	
Gravity	Wind	Building Code:	FBC2004
Roof (psf): 42	Wind Standard: ASCE 7-02		
Floor (psf): 55	Wind Speed (mph): 120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)	
ESCALANTE, HUGO CRC 1326967	
Address:	P.O. BOX 280 FORT WHITE, FL. 32038
Designer:	32

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987	
Company:	Structural Engineering and Inspections, Inc. EB 9196
Address:	16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for vertical loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	119061048	1/19/2006	41	T29G	119061088	1/19/2006
2	CJ3	119061049	1/19/2006				
3	CJ5	119061050	1/19/2006				
4	EJ7	119061051	1/19/2006				
5	EJ7A	119061052	1/19/2006				
6	EJ7B	119061053	1/19/2006				
7	EJ7G	119061054	1/19/2006				
8	EJ7GA	119061055	1/19/2006				
9	HJ9	119061056	1/19/2006				
10	T01	119061057	1/19/2006				
11	T01G	119061058	1/19/2006				
12	T02	119061059	1/19/2006				
13	T03	119061060	1/19/2006				
14	T04	119061061	1/19/2006				
15	T05	119061062	1/19/2006				
16	T06	119061063	1/19/2006				
17	T07	119061064	1/19/2006				
18	T08	119061065	1/19/2006				
19	T09	119061066	1/19/2006				
20	T10	119061067	1/19/2006				
21	T11	119061068	1/19/2006				
22	T12	119061069	1/19/2006				
23	T13	119061070	1/19/2006				
24	T14	119061071	1/19/2006				
25	T15	119061072	1/19/2006				
26	T16	119061073	1/19/2006				
27	T17	119061074	1/19/2006				
28	T18	119061075	1/19/2006				
29	T19	119061076	1/19/2006				
30	T20	119061077	1/19/2006				
31	T21	119061078	1/19/2006				
32	T22	119061079	1/19/2006				
33	T23	119061080	1/19/2006				
34	T24	119061081	1/19/2006				
35	T24A	119061082	1/19/2006				
36	T25	119061083	1/19/2006				
37	T26	119061084	1/19/2006				
38	T27	119061085	1/19/2006				
39	T28	119061086	1/19/2006				
40	T29	119061087	1/19/2006				

REVISED

37 WISE

23825

JAN 19 2006

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[Term Glossary](#)[Online Help](#)**Licensee Details****Licensee Information**

Name: **ESCALANTE, HUGO (Primary Name)**
EWPL INC (DBA Name)
Main Address: **P.O. BOX 280**
FORT WHITE, Florida 32038

License Information

License Type: **Certified Residential Contractor**
Rank: **Cert Residential**
License Number: **CRC1326967**
Status: **Current, Active**
Licensure Date: **11/24/2003**
Expires: **08/31/2006**

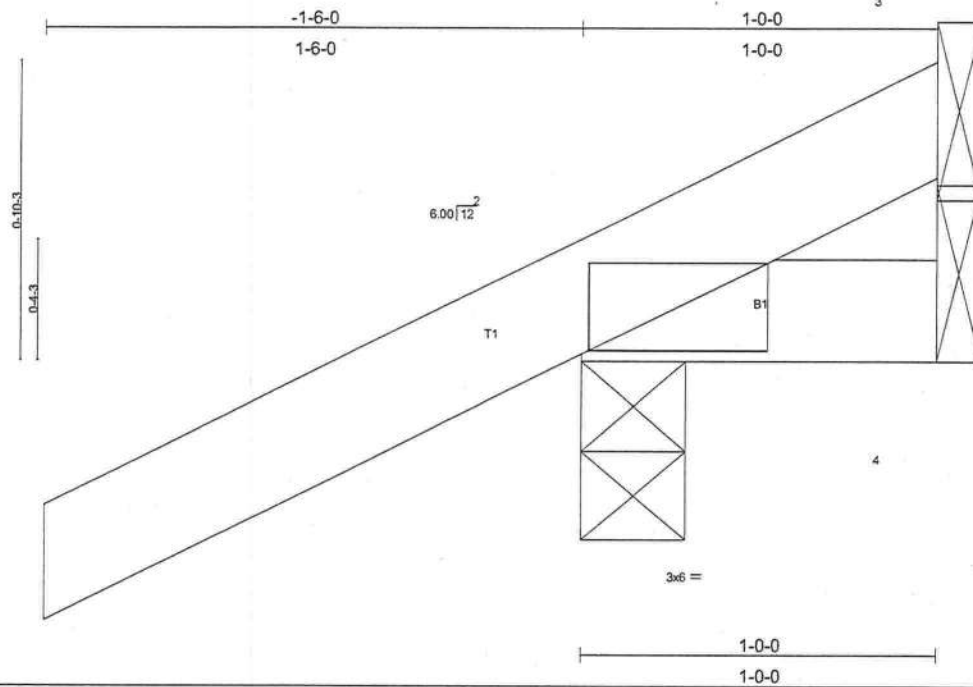
Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	CJ1	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MITek Industries, Inc. Wed Jan 18 16:23:51 2006 Page 1



Scale = 1/8"

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 6 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=40/Mechanical
 Max Horz 2=84(load case 5)
 Max Uplift 2=220(load case 5), 3=40(load case 1)
 Max Grav 2=189(load case 1), 4=14(load case 1), 3=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=45/41
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

NOTES

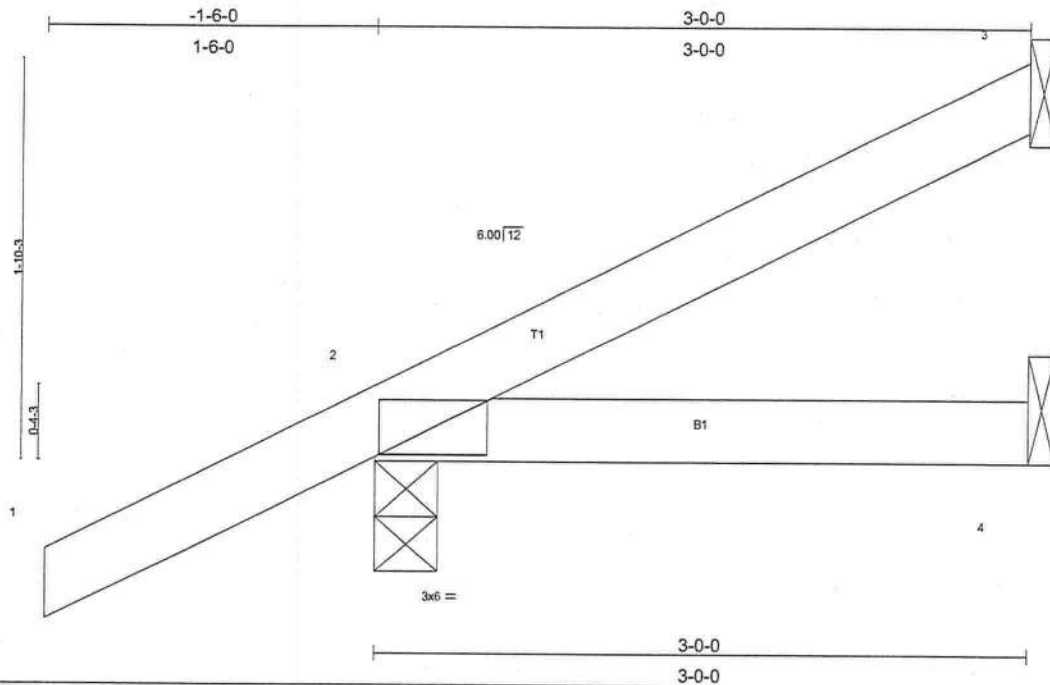
- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2 and 40 lb uplift at joint 3.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 36 WISE ESTATES
L135123	CJ3	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:29:10 2006 Page 1



Scale = 1:10.1

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.21	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 12 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical

Max Horz 2=137(load case 5)

Max Uplift 3=47(load case 5), 2=187(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=52/16

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

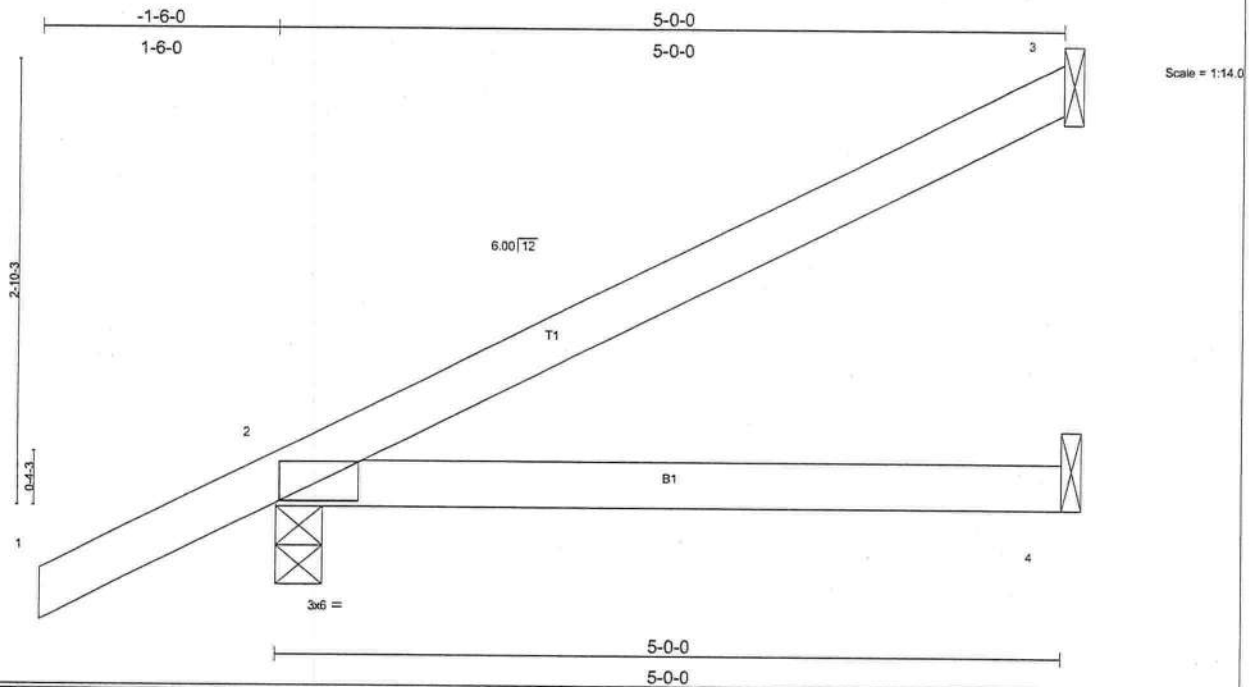
NOTES

1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) All bearings are assumed to be SYP No.2 crushing capacity of 565,000 psi

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 187 lb uplift at joint 2.

LOAD CASE(S) Standard



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) -0.03 2-4 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL) -0.05 2-4 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 18 lb	

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-124(load case 5), 2=-197(load case 5)

TOP CHORD 1-2=0/35, 2-3=-112/41
BOT CHORD 2-4=0/0

 $2 = 0.15$

- 1) Wind: ASCE 7-02; 120mph (3-second gust); $h=14ft$; $TCDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber $DOL=1.60$ plate grip $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 197 lb uplift at joint 2.

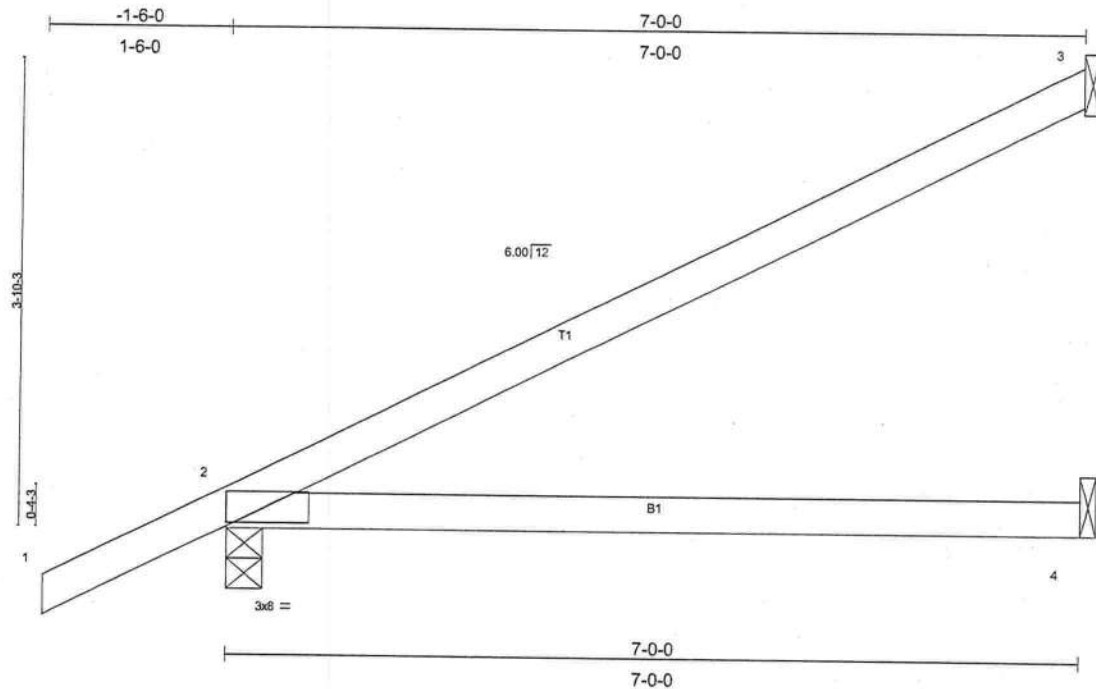
LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	EJ7	MONO TRUSS	29	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:23:53 2006 Page 1



Scale = 1:18.1

Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.52	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 25 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=247(load case 5)
Max Uplift 3=170(load case 5), 2=-217(load case 5), 4=-1(load case 5)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-142/59
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

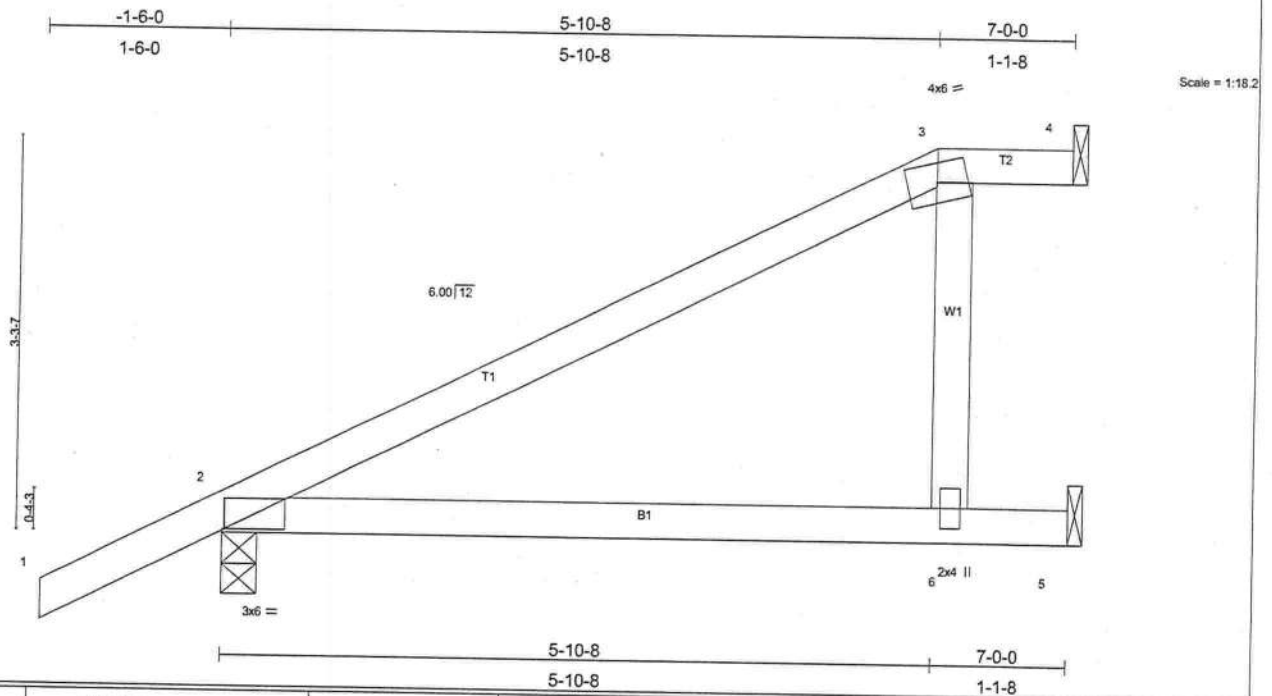
2 = 0.76

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 3, 217 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	EJ7A	MONO HIP	1	1	
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		
6.200 s Jul 13 2005 MITEK Industries, Inc. Wed Jan 18 16:23:53 2006 Page 1					



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.40	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.43	Vert(LL) 0.20 2-6 >404 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.08	Vert(TL) -0.25 2-6 >323 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 4 n/a n/a		
	Code FBC2004/TPI2002			Weight: 29 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=77/Mechanical, 2=385/0-3-8, 5=197/Mechanical
 Max Horz 2=219(load case 5)
 Max Uplift 4=13(load case 3), 2=229(load case 5), 5=160(load case 5)
 Max Grav 4=94(load case 10), 2=385(load case 1), 5=197(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/35, 2-3=-113/42, 3-4=-0/1
 BOT CHORD 2-6=-15/5, 5-6=0/0
 WEBS 3-6=-112/302

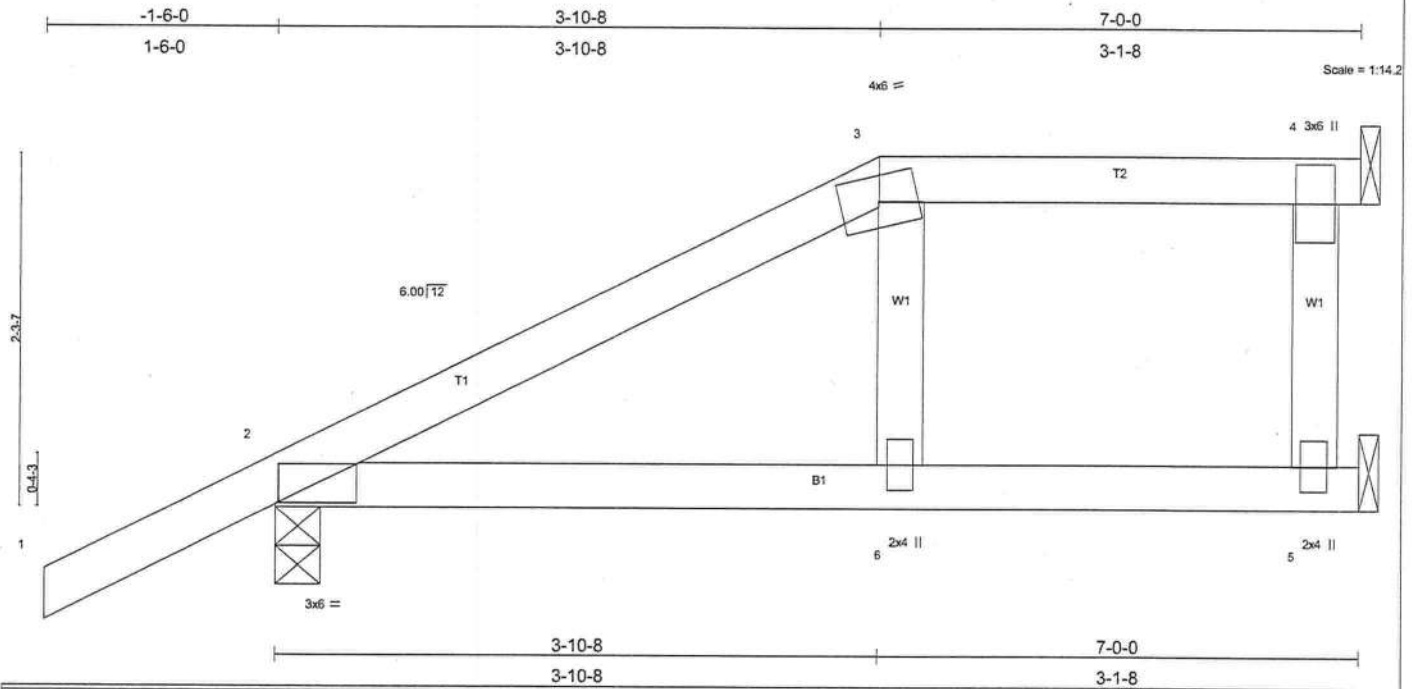
JOINT STRESS INDEX
 2 = 0.44, 3 = 0.42 and 6 = 0.17

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 4, 229 lb uplift at joint 2 and 160 lb uplift at joint 5.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	EJ7B	MONO HIP	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:23:54 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.34	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.44	Vert(LL) 0.19 2-6 >422 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.04	Vert(TL) -0.22 2-6 >355 180		
BCDL 5.0	Rep Stress incr YES	(Matrix)	Horz(TL) 0.11 4 n/a n/a		
	Code FBC2004/TPI2002			Weight: 29 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=376/0-3-8, 4=138/Mechanical, 5=127/Mechanical

Max Horz 2=164(load case 5)

Max Uplift 2=236(load case 5), 4=-83(load case 4), 5=-36(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-80/6, 3-4=0/0

BOT CHORD 2-6=-12/4, 5-6=0/0

WEBS 3-6=-55/167, 4-5=0/0

JOINT STRESS INDEX

2 = 0.25, 3 = 0.78, 4 = 0.00, 5 = 0.00 and 6 = 0.09

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2, 83 lb uplift at joint 4 and 36 lb uplift at joint 5.
- Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	EJ7GA	MONO HIP	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:23:55 2006 Page 1		

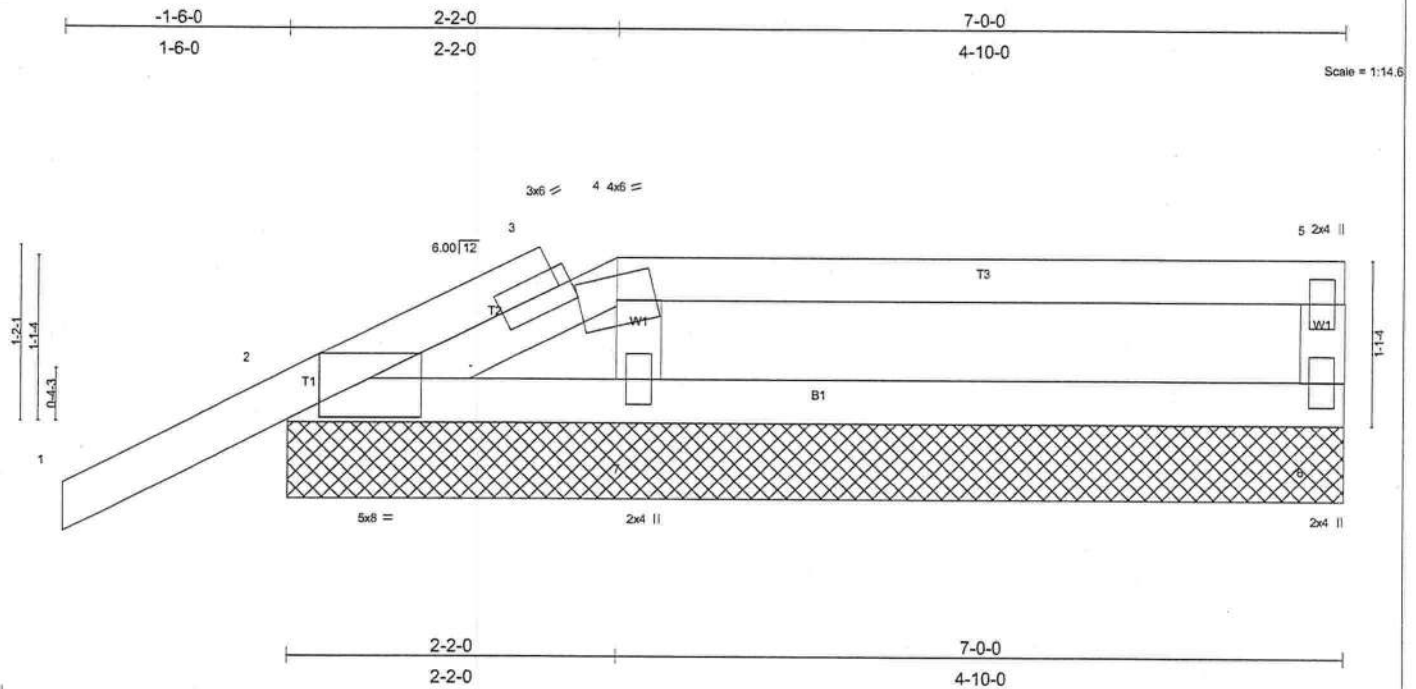


Plate Offsets (X,Y): [2'-0"-4'-0"-0'-3'-1"]									
LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.36	Vert(LL) -0.00	1	n/r	120		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.13	Vert(TL) -0.01	1	n/r	90			
BCLL 10.0	Rep Stress Incr NO	WB 0.10	Horz(TL) -0.00	6	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
								Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0"-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0"-0" oc bracing.

REACTIONS (lb/size) 2=198/7'-0"-0", 6=204/7'-0"-0", 7=464/7'-0"-0"

Max Horz 2=99(load case 5)

Max Uplift 2=196(load case 5), 6=100(load case 3), 7=206(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-8/51, 2-3=-77/28, 3-4=-10/10, 4-5=0/0

BOT CHORD 2-7=-70/60, 6-7=0/0

WEBS 4-7=-338/393, 5-6=-148/173

JOINT STRESS INDEX

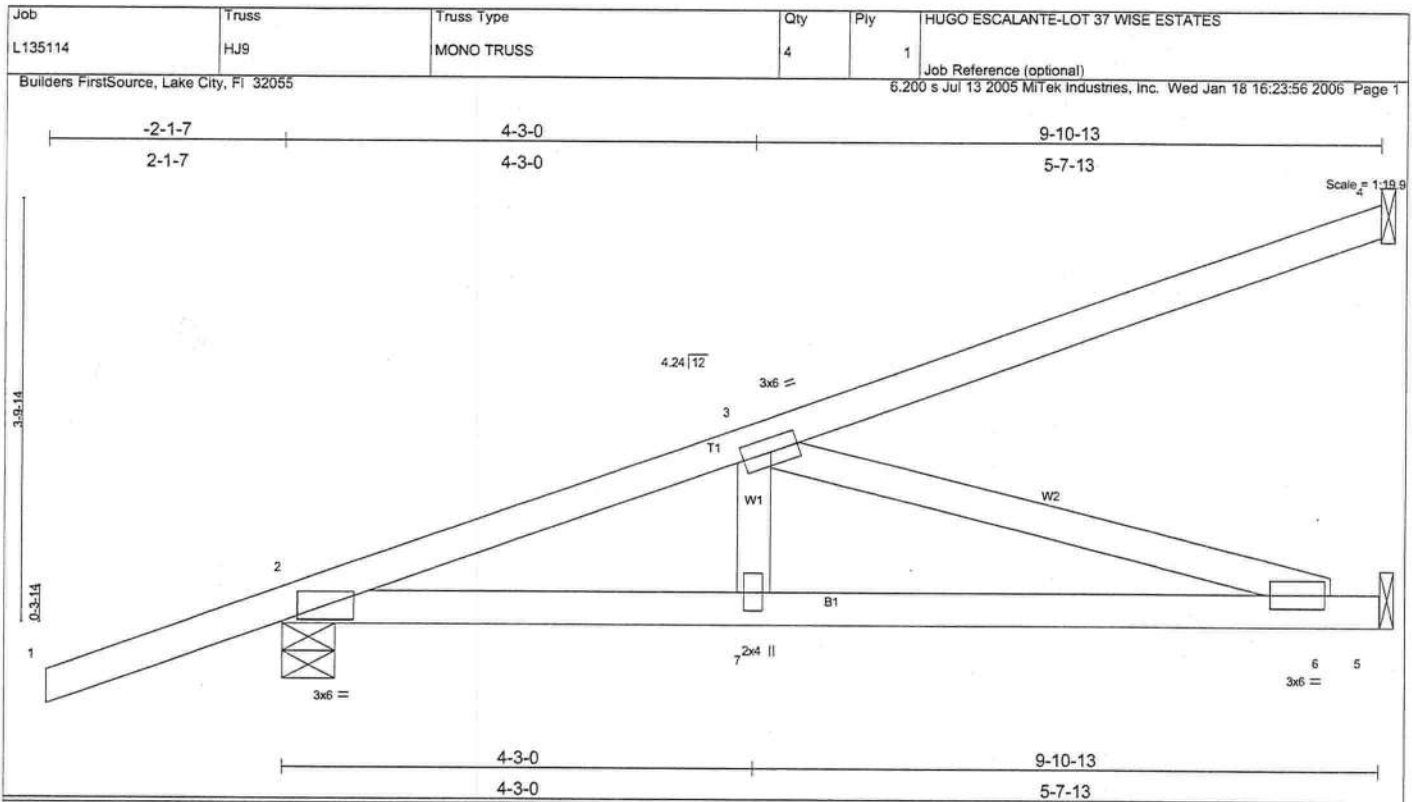
2 = 0.55, 3 = 0.00, 3 = 0.23, 4 = 0.74, 5 = 0.10, 6 = 0.10 and 7 = 0.22

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- Provide adequate drainage to prevent water ponding.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2'-0" oc.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 100 lb uplift at joint 6 and 206 lb uplift at joint 7.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-79(F=-25), 4-5=-79(F=-25), 2-6=-30



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2'-0"	TC 0.61	in (loc) l/defl L/d	MT20	244/190
TCCL 7.0	Plates Increase 1.25	BC 0.62	Vert(LL) -0.11 6-7 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.50	Vert(TL) -0.18 6-7 >626 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 5 n/a n/a		
	Code FBC2004/TPI2002			Weight: 43 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-11-7 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=386/Mechanical
 Max Horz 2=303(load case 2)
 Max Uplift 4=-289(load case 2), 2=-285(load case 2), 5=-107(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/37, 2-3=934/246, 3-4=-124/65
 BOT CHORD 2-7=475/869, 6-7=-475/869, 5-6=0/0
 WEBS 3-7=0/208, 3-6=-906/495

JOINT STRESS INDEX

2 = 0.72, 3 = 0.24, 6 = 0.26 and 7 = 0.15

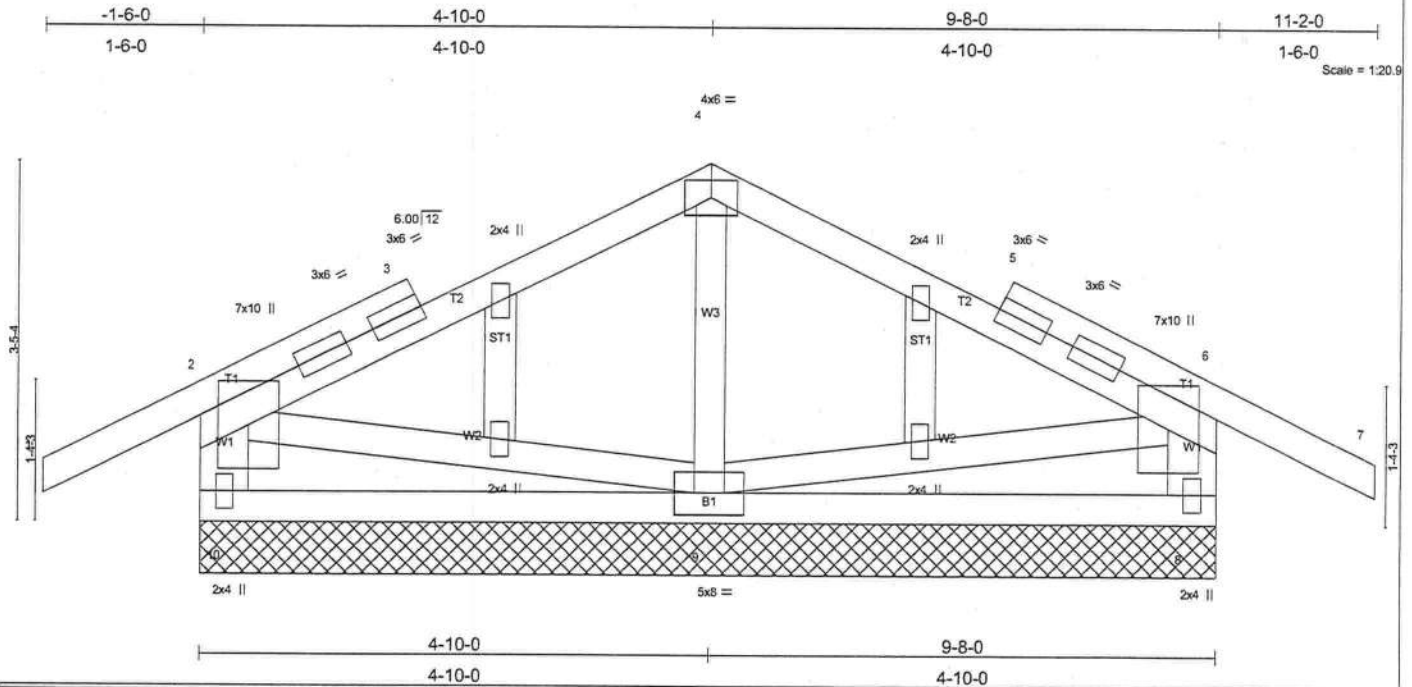
NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B: enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 4, 285 lb uplift at joint 2 and 107 lb uplift at joint 5.
- 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54
 Trapezoidal Loads (plf)
 Vert: 2=3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=22)

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T01G	COMMON	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:23:58 2006 Page 1		



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.11	Vert(TL)	-0.02	7	n/r	90		
BCLL 10.0	Rep Stress Incr	NO	WB 0.07	Horz(TL)	0.00	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 67 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 W1 2 X 6 SYP No.1D, W1 2 X 6 SYP No.1D
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 10=401/9-8-0, 8=401/9-8-0, 9=474/9-8-0
 Max Horz 10=79(load case 4)
 Max Uplift 10=266(load case 5), 8=269(load case 6), 9=186(load case 5)
 Max Grav 10=405(load case 9), 8=405(load case 10), 9=474(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-6/51, 2-3=-118/80, 3-4=-41/76, 4-5=-41/76, 5-6=-118/80, 6-7=-6/51, 2-10=-338/386, 6-8=-338/386
 BOT CHORD 9-10=-109/151, 8-9=-36/151
 WEBS 4-9=-292/263, 2-9=-120/163, 6-9=-120/163

JOINT STRESS INDEX

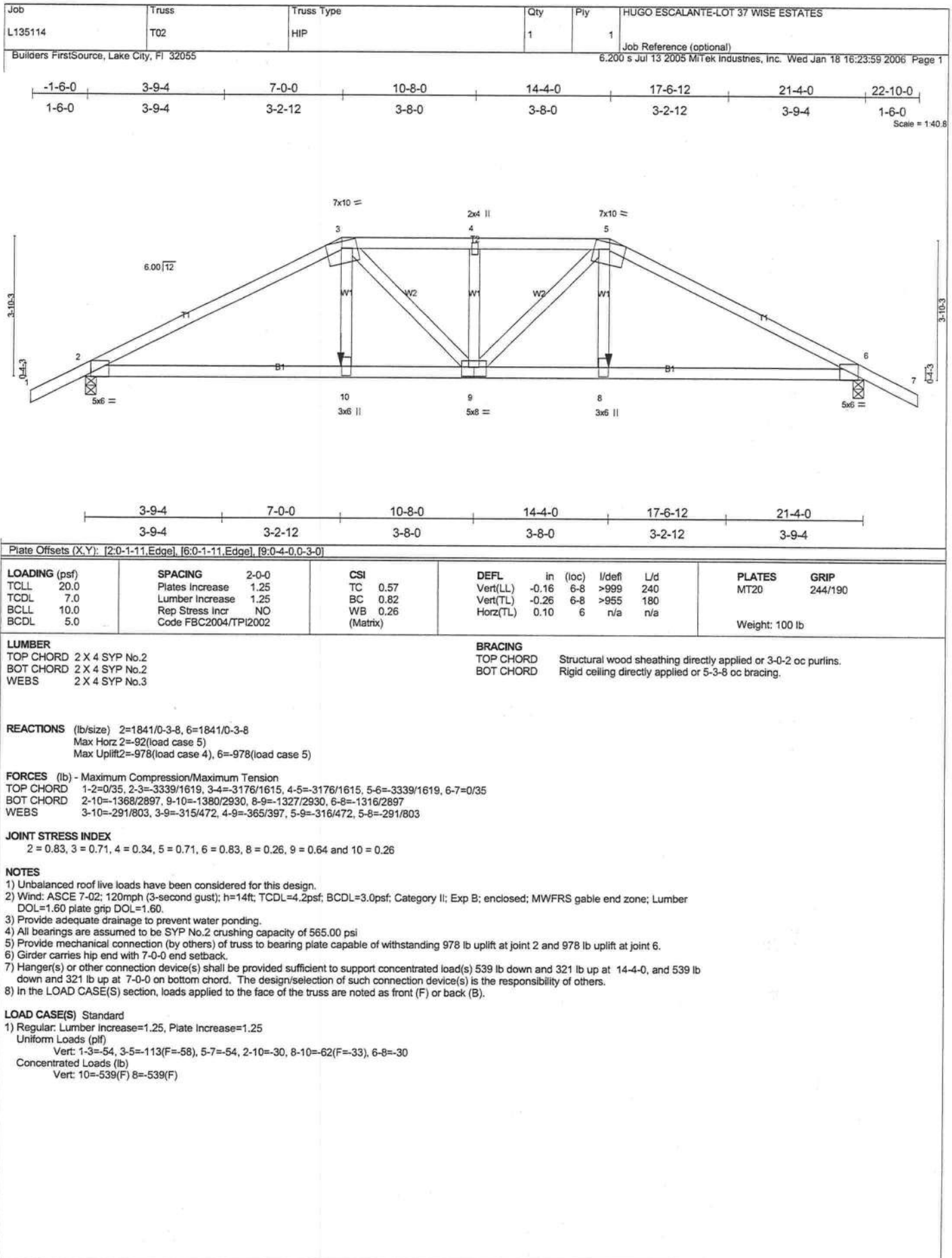
2 = 0.66, 3 = 0.00, 3 = 0.37, 3 = 0.37, 4 = 0.58, 5 = 0.00, 5 = 0.37, 5 = 0.37, 6 = 0.66, 8 = 0.52, 9 = 0.06, 10 = 0.52, 11 = 0.00, 12 = 0.00, 13 = 0.00 and 14 = 0.00

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 10, 269 lb uplift at joint 8 and 186 lb uplift at joint 9.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-79(F=-25), 2-4=-79(F=-25), 4-6=-79(F=-25), 6-7=-79(F=-25), 8-10=-30



Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T04	COMMON	2	1	
Builders FirstSource, Lake City, Fl 32055					Job Reference (optional)
					6,200 s Jul 13 2005 Mittek Industries, Inc. Wed Jan 18 16:24:00 2006 Page 1

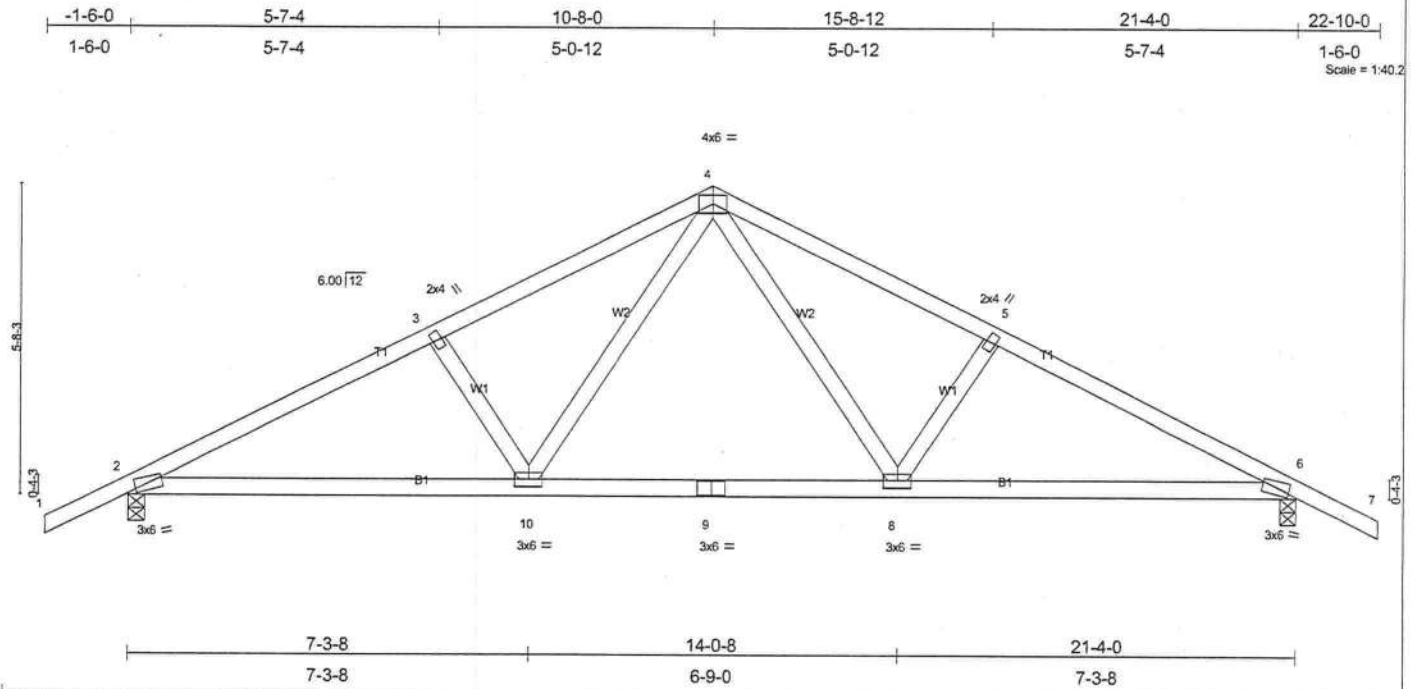


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]													
LOADING (psf)		SPACING 2-0-0		CSI		DEFL				PLATES		GRIP	
TCLL	20.0	Plates Increase 1.25		TC	0.36	in	(loc)	l/defl	L/d	MT20	244/190		
TCDL	7.0	Lumber Increase 1.25		BC	0.81	Vert(LL)	-0.19 8-10	>999	240				
BCLL	10.0	Rep Stress Incr NO		WB	0.26	Vert(TL)	-0.31 8-10	>821	180				
BCDL	5.0	Code FBC2004/TP12002		(Matrix)		Horz(TL)	0.05 6	n/a	n/a				
											Weight: 100 lb		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 4-5-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 7-3-10 oc bracing.

REACTIONS: (lb/size) 2=1141/0-3-8, 6=1141/0-3-8
Max Horz 2=123(load case 6)
Max Uplift2=568(load case 5), 6=568(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1888/1014, 3-4=-1732/1007, 4-5=-1732/1007, 5-6=-1888/1014, 6-7=0/35
BOT CHORD 2-10=-732/1619, 9-10=-383/1112, 8-9=-383/1112, 6-8=-732/1619
WEBS 3-10=-250/291, 4-10=-365/734, 4-8=-366/734, 5-8=-250/291

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.66, 5 = 0.34, 6 = 0.79, 8 = 0.56, 9 = 0.83 and 10 = 0.56

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.61. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 568 lb uplift at joint 2 and 568 lb uplift at joint 6.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T05	COMMON	4	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6,200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:00 2006 Page 1		

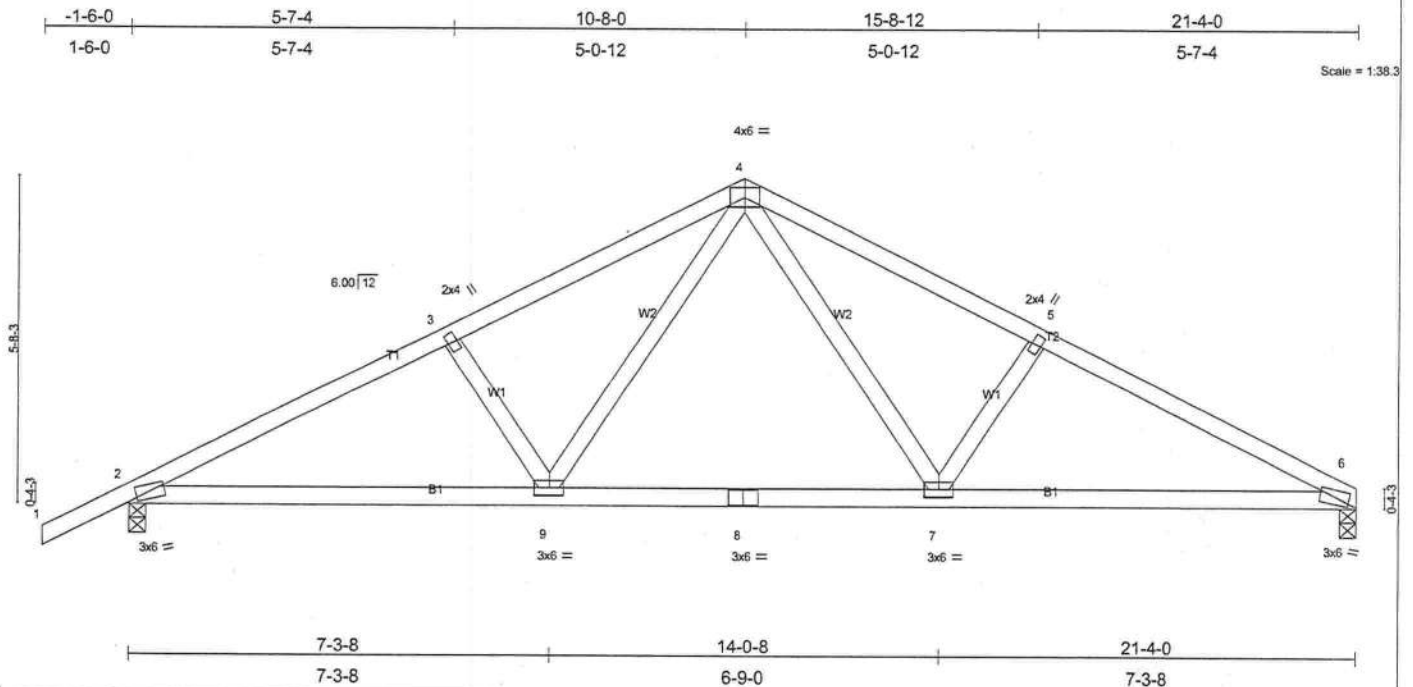


Plate Offsets (X,Y): [2,0-1-13,0-0-7], [6,0-1-13,0-0-7]											
LOADING (psf)		SPACING 2-0-0		CSI		DEFL				PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.36	in (loc)	l/def	L/d	MT20	244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(LL)	-0.19	7-9	>999	240	
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Vert(TL)	-0.30	7-9	>829	180	
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Horz(TL)	0.05	6	n/a	n/a	
									Weight: 97 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-4-6 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-7-13 oc bracing.

REACTIONS

(lb/size) 6=1049/0-3-8, 2=1145/0-3-8
Max Horz 2=144(load case 5)
Max Uplift 6=450(load case 6), 2=570(load case 5)

FORCES (lb)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1895/1028, 3-4=-1739/1020, 4-5=-1753/1046, 5-6=-1895/1055
BOT CHORD 2-9=813/1625, 8-9=-465/1119, 7-8=-465/1119, 6-7=-845/1642
WEBS 3-9=-250/292, 4-9=-361/734, 4-7=-402/753, 5-7=-260/311

JOINT STRESS INDEX

2 = 0.80, 3 = 0.34, 4 = 0.64, 5 = 0.34, 6 = 0.80, 7 = 0.57, 8 = 0.83 and 9 = 0.57

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); $h=14ft$; $CDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber $DOL=1.60$ plate grip $DOL=1.60$. This truss is designed for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 6 and 570 lb uplift at joint 2.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=50), 6-7=-30

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T06	SPECIAL	1	1	
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)
					6.200 s Jul 13 2005 Mitek Industries, Inc. Wed Jan 18 16:24:01 2006 Page 1

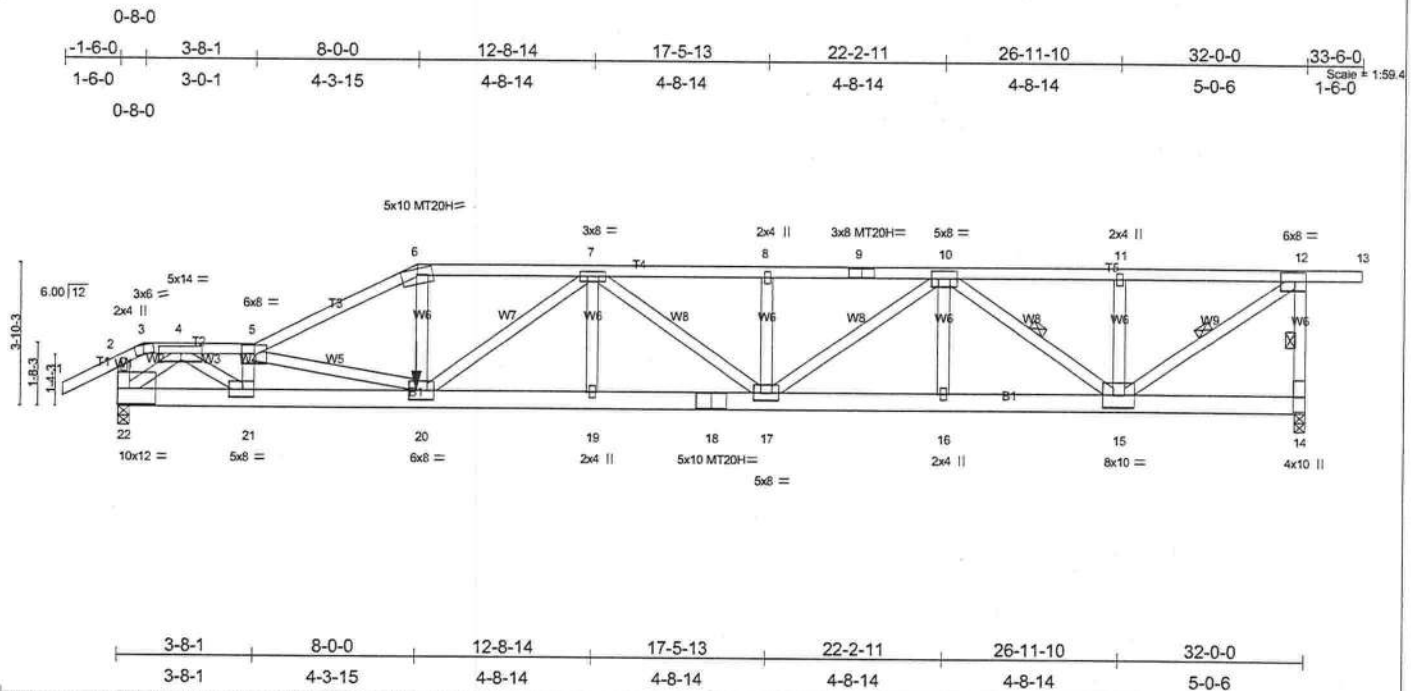


Plate Offsets (X,Y): [12:0-3,8,0-3,0], [14:Edge,0-3,8], [21:0-3,8,0-2,8]									
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	2-0-0	TC 0.86	Vert(LL) 0.47	17-19	>815	240	MT20	244/190	
TCOL 7.0	Plates Increase 1.25	BC 0.93	Vert(TL) -0.74	17-19	>516	180	MT20H	187/143	
BCLL 10.0	Rep Stress Incr NO	WB 0.83	Horz(TL) 0.14	14	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							Weight: 211 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-0-1 oc purlins, except end verticals.
BOT CHORD 2 X 6 SYP No.1D	BOT CHORD Rigid ceiling directly applied or 4-4-6 oc bracing.
WEBS 2 X 4 SYP No.3 "Except"	WEBS 1 Row at midpt 12-14, 10-15, 12-15
W3 2 X 4 SYP No.2, W9 2 X 4 SYP No.2, W1 2 X 4 SYP No.1D, W2 2 X 4 SYP No.2	

REACTIONS (lb/size) 14=2929/0-3-8, 22=2694/0-3-8
 Max Horz 22=212(load case 3)
 Max Uplift 14=1721(load case 3), 22=1425(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-258/153, 3-4=-213/141, 4-5=-597/13094, 5-6=-5822/3158, 6-7=-5286/2918, 7-8=-6561/3661, 8-9=-6561/3661, 9-10=-6561/3661, 10-11=-3428/1957, 11-12=-3428/1957, 12-13=0/0, 12-14=-2746/1707, 2-22=-237/227
 BOT CHORD 21-22=-1622/2810, 20-21=-3393/6238, 19-20=-3685/6468, 18-19=-3685/6468, 17-18=-3685/6468, 16-17=-3192/5592, 15-16=-3192/5592, 14-15=-93/83
 WEBS 4-21=-1986/3865, 5-21=-2399/1313, 5-20=-1087/597, 6-20=-1077/2146, 7-20=-1483/917, 7-19=0/295, 7-17=-98/115, 8-17=-521/519, 10-17=-671/1201, 10-16=0/334, 10-15=-2683/1530, 11-15=-535/546, 12-15=-2289/4104, 4-22=-3424/1799

JOINT STRESS INDEX
 2 = 0.44, 3 = 0.32, 4 = 0.95, 5 = 0.98, 6 = 0.99, 7 = 0.72, 8 = 0.34, 9 = 0.93, 10 = 0.68, 11 = 0.34, 12 = 0.79, 14 = 0.57, 15 = 0.97, 16 = 0.34, 17 = 0.56, 18 = 0.90, 19 = 0.34, 20 = 0.54, 21 = 0.88 and 22 = 0.58

NOTES

- 1) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1721 lb uplift at joint 14 and 1425 lb uplift at joint 22.
- 6) Girder carries hip end with 0-0-0 right side setback, 8-0-0 left side setback, and 7-0-0 end setback.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 616 lb down and 397 lb up at 8-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

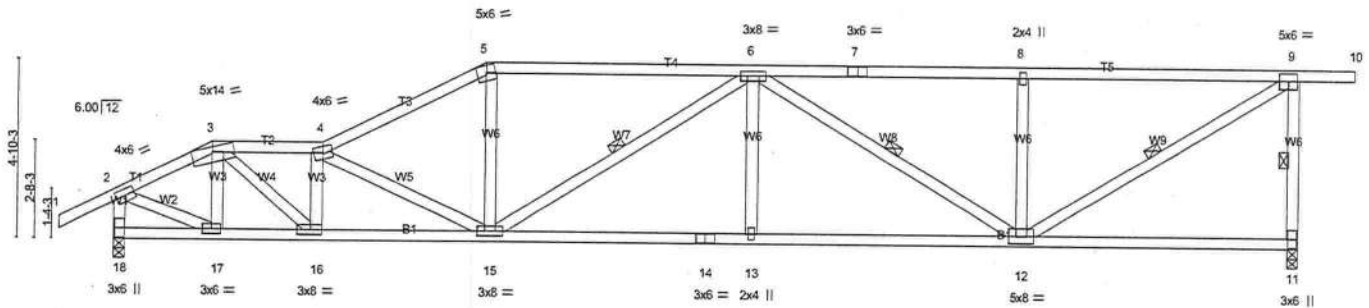
LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54, 2-3=-54, 3-5=-54, 5-6=-54, 6-12=-113(F=-58), 12-13=-54, 20-22=-30, 14-20=-62(F=-33)
 Concentrated Loads (lb)
 Vert: 20=-616(F)

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T07	SPECIAL	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:02 2006 Page 1		

-1-6-0	2-8-0	5-8-1	10-0-0	17-2-13	24-5-11	32-0-0	33-6-0
1-6-0	2-8-0	3-0-1	4-3-15	7-2-13	7-2-13	7-6-5	1-6-0

Scale = 1:59.4



2-8-0	5-8-1	10-0-0	17-2-13	24-5-11	32-0-0
2-8-0	3-0-1	4-3-15	7-2-13	7-2-13	7-6-5

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [16:0-3-8,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.60	Vert(LL)	-0.21	13-15	>999	240	
TCDL 7.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.34	13-15	>999	180	
BCLL 10.0	Rep Stress Incr	YES	WB 0.63	Horz(TL)	0.08	11	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 186 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-10-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-7-8 oc bracing.
 WEBS 1 Row at midpt 9-11, 6-15, 6-12, 9-12

REACTIONS (lb/size) 11=1421/0-3-8, 18=1421/0-3-8
 Max Horz 18=271(load case 4)
 Max Uplift 11=742(load case 4), 18=621(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-1512/787, 3-4=-2456/1310, 4-5=-2287/1211, 5-6=-2047/1162, 6-7=-1719/966, 7-8=-1719/966, 8-9=-1719/966, 9-10=0/0, 9-11=-1311/768, 2-18=-1371/836
 BOT CHORD 17-18=-252/81, 16-17=-654/1289, 15-16=-1273/2510, 14-15=-1138/2363, 13-14=-1138/2363, 12-13=-1138/2363, 11-12=-69/76
 WEBS 3-17=-457/260, 3-16=-781/1518, 4-16=-898/533, 4-15=-549/362, 5-15=-229/659, 6-15=-376/285, 6-13=0/216, 6-12=-762/399, 8-12=-399/351, 9-12=-987/1953, 2-17=-655/1391

JOINT STRESS INDEX
 2 = 0.71, 3 = 0.53, 4 = 0.56, 5 = 0.65, 6 = 0.57, 7 = 0.49, 8 = 0.34, 9 = 0.63, 11 = 0.40, 12 = 0.90, 13 = 0.34, 14 = 0.79, 15 = 0.57, 16 = 0.83, 17 = 0.79 and 18 = 0.28

NOTES

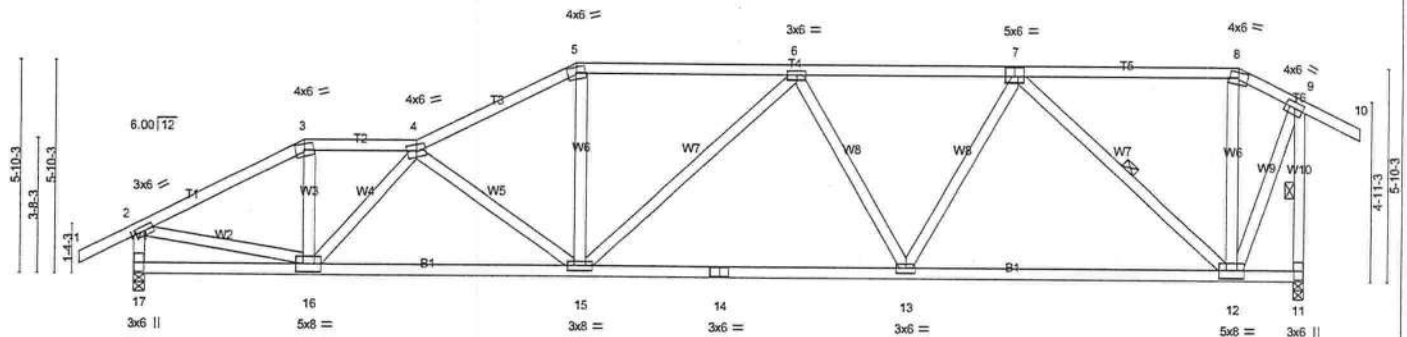
- 1) Wind: ASCE 7-02: 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 742 lb uplift at joint 11 and 621 lb uplift at joint 18.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T08	SPECIAL	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 Mitek Industries, Inc. Wed Jan 18 16:24:03 2006 Page 1		

-1-6-0	4-8-0	7-8-1	12-0-0	18-0-11	24-1-5	30-2-0	32-0-0	33-6-0
1-6-0	4-8-0	3-0-1	4-3-15	6-0-11	6-0-11	6-0-11	1-10-0	1-6-0

Scale = 1/60.2



4-8-0	7-8-1	12-0-0	21-1-0	30-2-0	32-0-0
4-8-0	3-0-1	4-3-15	9-1-0	9-1-0	1-10-0

Plate Offsets (X,Y): [7:0-2-12,0-3-0], [9:0-2-15,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.37	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.60	Vert(LL) -0.20 13-15 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.48	Vert(TL) -0.33 13-15 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.08 11 n/a n/a		
	Code FBC2004/TPI2002			Weight: 200 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 7-12, 9-11

REACTIONS

(lb/size) 17=1421/0-3-8, 11=1421/0-3-8
 Max Horz 17=262(load case 4)
 Max Uplift 17=643(load case 5), 11=597(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=1802/939, 3-4=1580/903, 4-5=2057/1142, 5-6=1831/1095, 6-7=1715/1011, 7-8=446/392, 8-9=504/434, 9-10=0/40, 2-17=-1351/849, 9-11=-1473/843
 BOT CHORD 16-17=-276/111, 15-16=-1060/2219, 14-15=-893/1871, 13-14=-893/1871, 12-13=-700/1406, 11-12=-51/113
 WEBS 3-16=-235/579, 4-16=-990/566, 4-15=-507/389, 5-15=-230/597, 6-15=-193/189, 6-13=-327/274, 7-13=-210/645, 7-12=-1318/711, 8-12=-55/115, 2-16=-660/1485, 9-12=-512/1234

JOINT STRESS INDEX

2 = 0.76, 3 = 0.43, 4 = 0.52, 5 = 0.79, 6 = 0.45, 7 = 0.63, 8 = 0.52, 9 = 0.74, 11 = 0.42, 12 = 0.71, 13 = 0.52, 14 = 0.84, 15 = 0.57, 16 = 0.71 and 17 = 0.30

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 643 lb uplift at joint 17 and 597 lb uplift at joint 11.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T09	SPECIAL	1	1	
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		
			6.200 s Jul 13 2005 Mitek Industries, Inc. Wed Jan 18 16:24:03 2006 Page 1		

-1-6-0	6-8-0	9-8-1	14-0-0	21-1-0	28-2-0	32-0-0	33-6-0
1-6-0	6-8-0	3-0-1	4-3-15	7-1-0	7-1-0	3-10-0	1-6-0
Scale = 1:60.2							

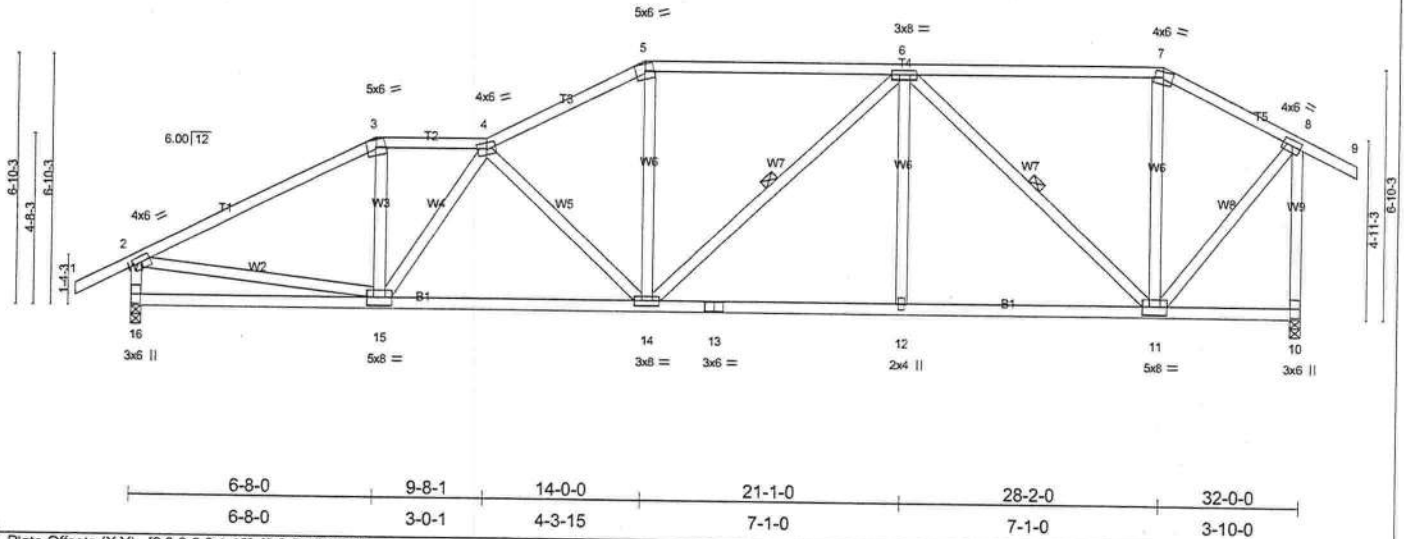


Plate Offsets (X,Y): [2:0-3:0,0-1-12], [8:0-2:15,0-2:0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.96	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.51	Vert(LL) -0.15 14-15 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.47	Vert(TL) -0.24 14-15 >999 180		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)	Horz(TL) 0.06 10 n/a n/a		
Weight: 207 lb					

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-4-5 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 6-14, 6-11

REACTIONS (lb/size) 16=1421/0-3-8, 10=1421/0-3-8
 Max Horz 16=279(load case 4)
 Max Uplift 16=660(load case 5), 10=540(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=1894/998, 3-4=1634/980, 4-5=1825/1071, 5-6=1622/1034, 6-7=709/569, 7-8=824/584, 8-9=0/40, 2-16=-1317/863, 8-10=-1385/856
 BOT CHORD 15-16=-335/254, 14-15=-912/1990, 13-14=-673/1489, 12-13=-673/1489, 11-12=-673/1489, 10-11=-42/117
 WEBS 3-15=-162/522, 4-15=-652/367, 4-14=-529/394, 5-14=-171/463, 6-14=-141/192, 6-12=0/210, 6-11=-1087/546, 7-11=-11/96, 2-15=-575/1382, 8-11=-453/1123

JOINT STRESS INDEX
 2 = 0.80, 3 = 0.61, 4 = 0.46, 5 = 0.63, 6 = 0.57, 7 = 0.69, 8 = 0.69, 10 = 0.43, 11 = 0.55, 12 = 0.34, 13 = 0.52, 14 = 0.57, 15 = 0.63 and 16 = 0.47

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 660 lb uplift at joint 16 and 540 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T10	SPECIAL	1	1	

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 Mitek Industries, Inc. Wed Jan 18 16:24:04 2006 Page 1

-1-6-0	4-5-12	8-8-0	11-8-1	16-0-0	21-1-0	26-2-0	32-0-0
1-6-0	4-5-12	4-2-4	3-0-1	4-3-15	5-1-0	5-1-0	5-10-0

Scale = 1:59.1

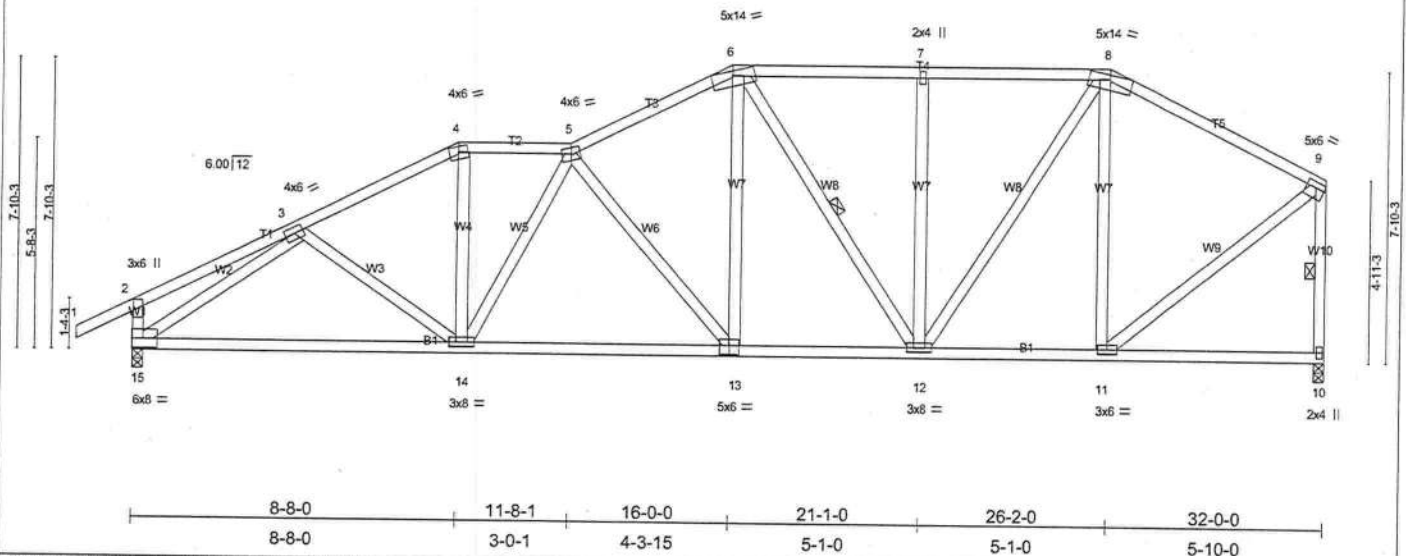


Plate Offsets (X,Y): [9:Edge,0-1-12], [13:0-3-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.65	Vert(LL)	-0.13 13-14	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.52	Vert(TL)	-0.22 13-14	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.79	Horz(TL)	0.06 10	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TP12002							
							Weight: 220 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-5 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-10-0 oc bracing.
 WEBS 1 Row at midpt 6-12, 9-10

REACTIONS

(lb/size) 15=1423/0-3-8, 10=1329/0-3-8
 Max Horz 15=328(load case 4)
 Max Uplift 15=672(load case 5), 10=460(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=324/153, 3-4=-1812/1009, 4-5=-1591/967, 5-6=-1620/987, 6-7=-1270/867, 7-8=-1270/867, 8-9=-1030/652, 2-15=-343/328, 9-10=-1242/722
 BOT CHORD 14-15=-785/1472, 13-14=-853/1801, 12-13=-583/1424, 11-12=-355/853, 10-11=-54/77
 WEBS 3-14=-57/166, 4-14=-255/565, 5-14=-446/289, 5-13=-610/447, 6-13=-309/654, 6-12=-295/183, 7-12=-281/245, 8-12=-381/790, 8-11=-495/316, 3-15=-1558/899, 9-11=-457/1059

JOINT STRESS INDEX

2 = 0.26, 3 = 0.46, 4 = 0.50, 5 = 0.49, 6 = 0.58, 7 = 0.34, 8 = 0.73, 9 = 0.70, 10 = 0.87, 11 = 0.64, 12 = 0.83, 13 = 0.58, 14 = 0.61 and 15 = 0.32

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 672 lb uplift at joint 15 and 460 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T11	SPECIAL	1	1	
Builders FirstSource, Lake City, FL 32055					
Job Reference (optional)					
6.200 s Jul 13 2005 MITek Industries, Inc. Wed Jan 18 16:24:05 2006 Page 1					

5-5-12	10-8-0	13-8-1	18-0-0	24-2-0	29-7-13	35-1-11	41-2-0	42-8-0
5-5-12	5-2-4	3-0-1	4-3-15	6-2-0	5-5-13	5-5-13	6-0-5	1-6-0
Scale = 1:73.1								

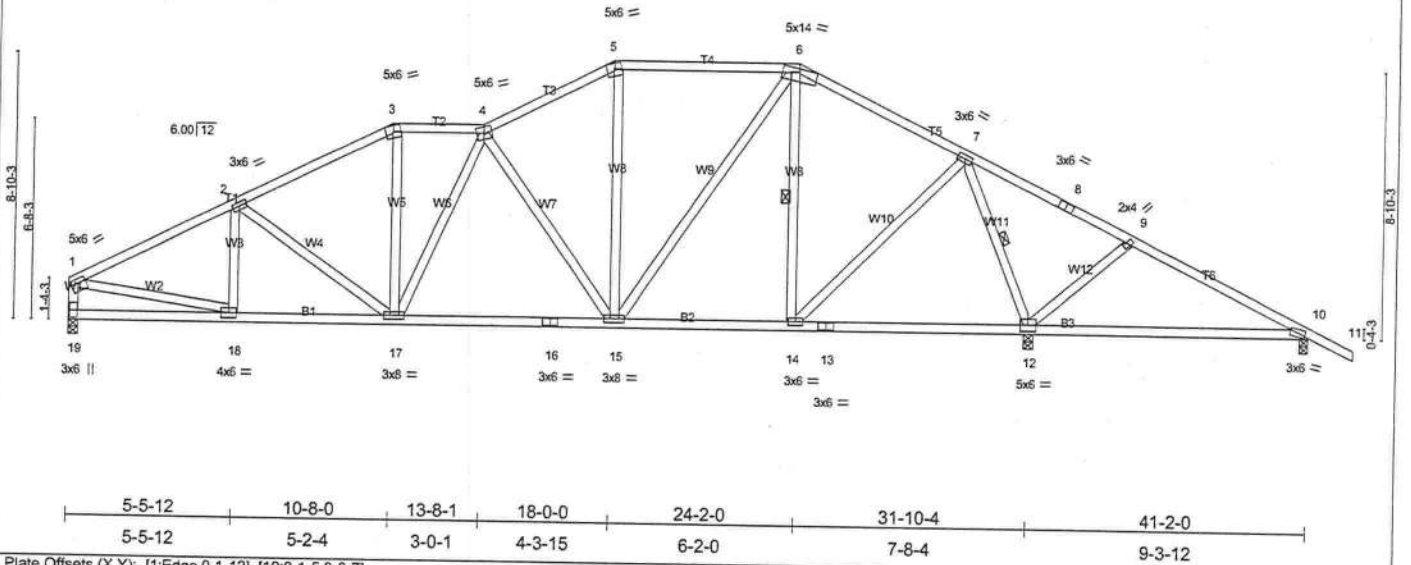


Plate Offsets (X,Y): [1:Edge,0-1-12], [10:0-1-5,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/def	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.53	Vert(LL)	0.41	10-12	>266	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.47	Vert(TL)	0.35	10-12	>311		
BCLL 10.0	Lumber Increase 1.25	WB 0.69	Horz(TL)	0.04	12	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TPI2002							
Weight: 251 lb								

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-8-4 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 6-14, 7-12

REACTIONS (lb/size) 19=1256/0-3-8, 12=2048/0-3-8, 10=219/0-3-8
 Max Horz 19=-209(load case 3)
 Max Uplift 19=-538(load case 5), 12=-912(load case 6), 10=-345(load case 6)
 Max Grav 19=1256(load case 1), 12=2048(load case 1), 10=281(load case 10)

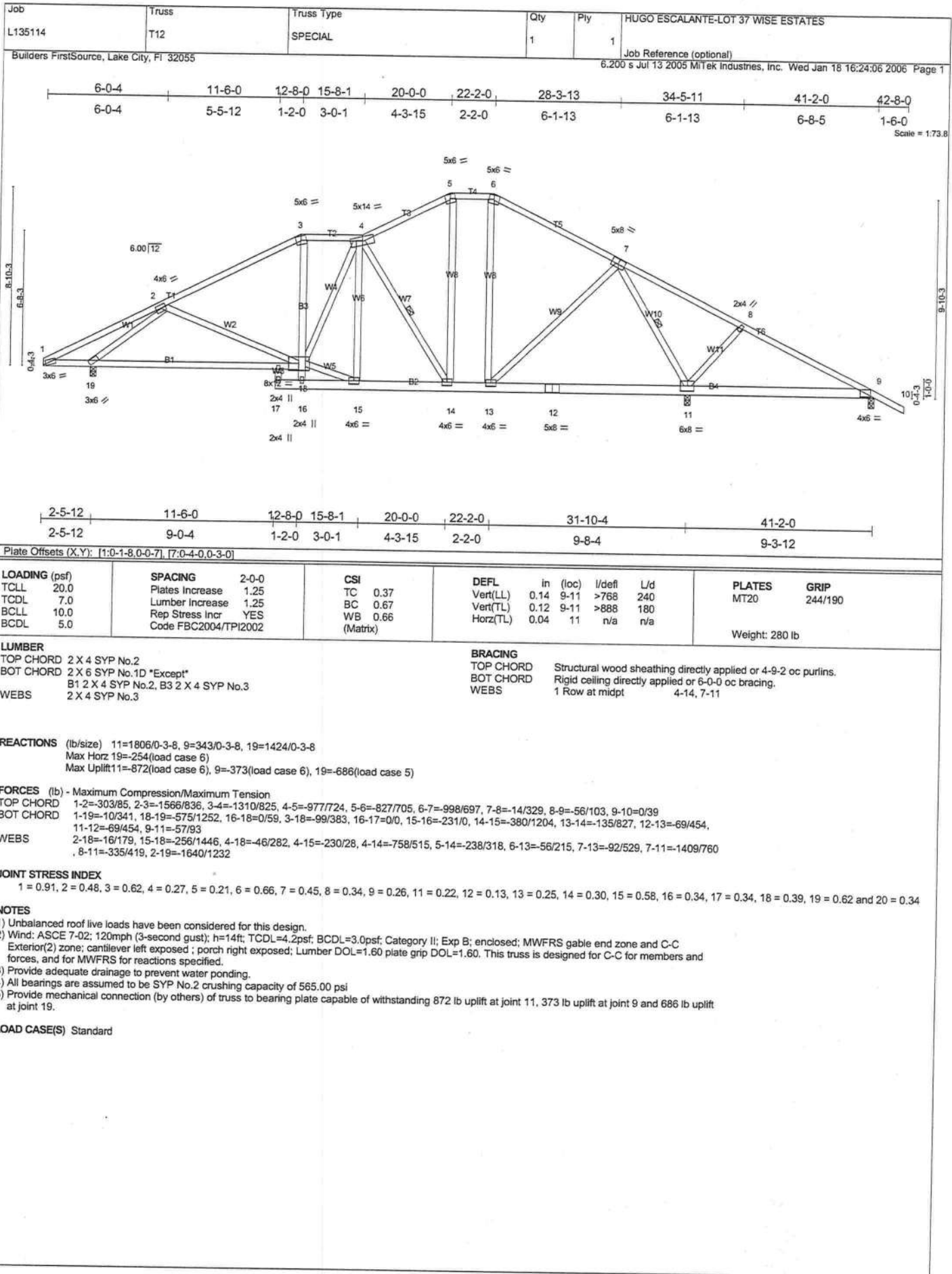
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-1738/945, 2-3=-1598/939, 3-4=-1382/910, 4-5=-1242/834, 5-6=-1080/814, 6-7=-883/613, 7-8=-159/591, 8-9=-178/469, 9-10=-80/361,
 10-11=0/35, 1-19=-1162/677
 BOT CHORD 18-19=-193/220, 17-18=-653/1495, 16-17=-515/1448, 15-16=-515/1448, 14-15=-114/725, 13-14=0/174, 12-13=0/174, 10-12=-291/161
 WEBS 2-18=-134/163, 2-17=-159/210, 3-17=-197/442, 4-17=-163/131, 4-15=-648/488, 5-15=-96/226, 6-15=-342/659, 6-14=-460/306,
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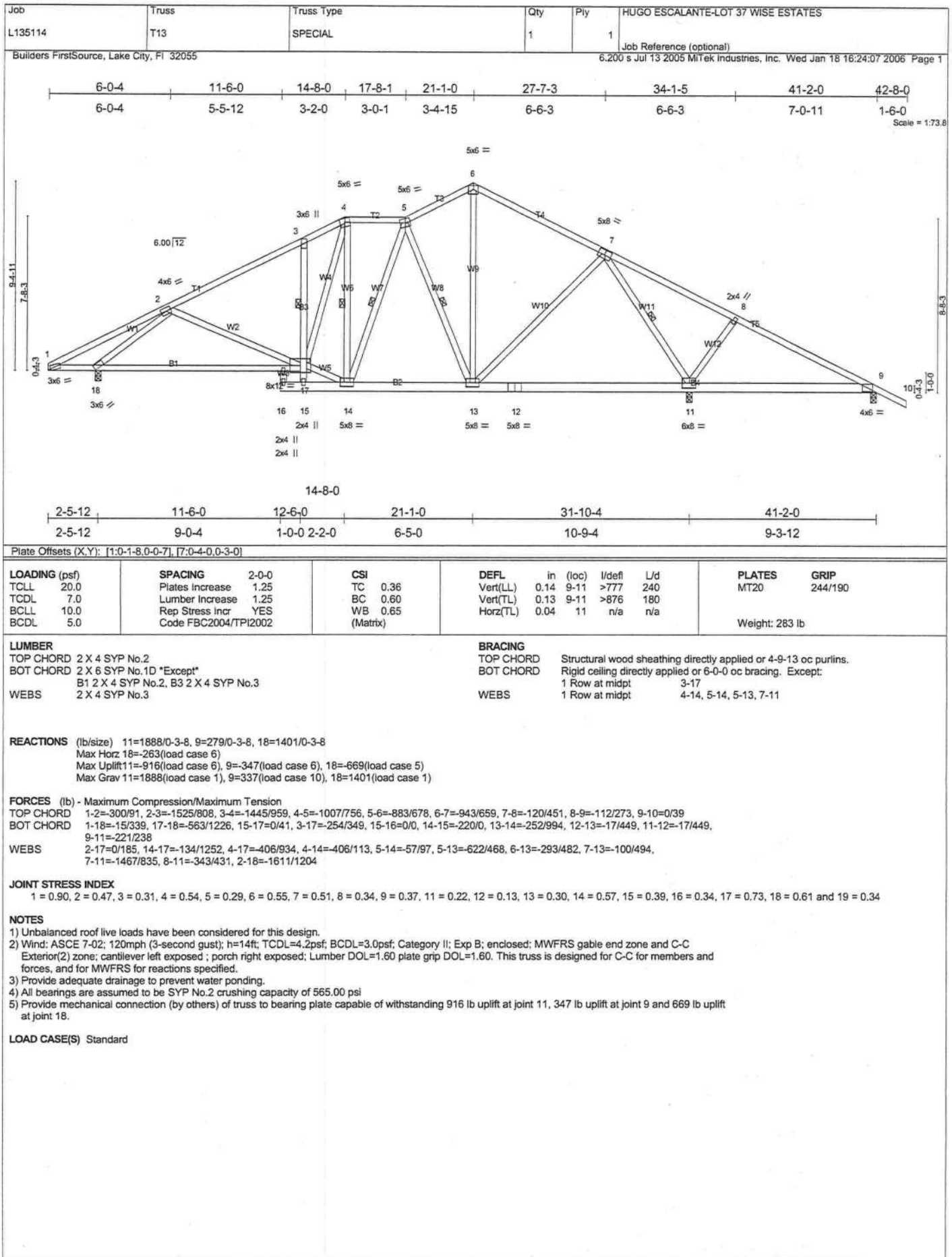
JOINT STRESS INDEX
 1 = 0.64, 2 = 0.41, 3 = 0.42, 4 = 0.31, 5 = 0.51, 6 = 0.81, 7 = 0.68, 8 = 0.24, 9 = 0.34, 10 = 0.91, 12 = 0.38, 13 = 0.15, 14 = 0.57, 15 = 0.69, 16 = 0.59, 17 = 0.63, 18 = 0.57 and 19 = 0.39

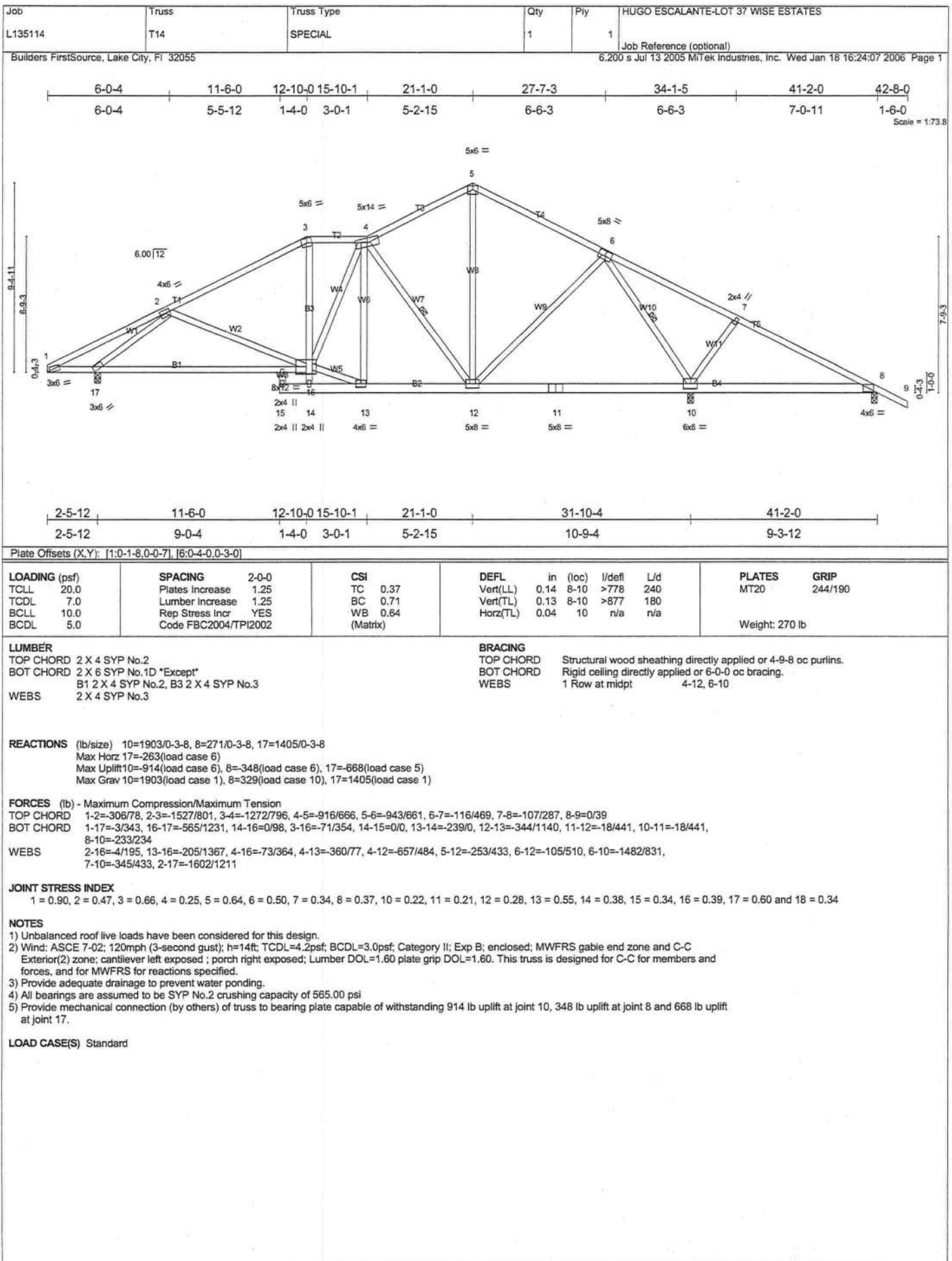
NOTES

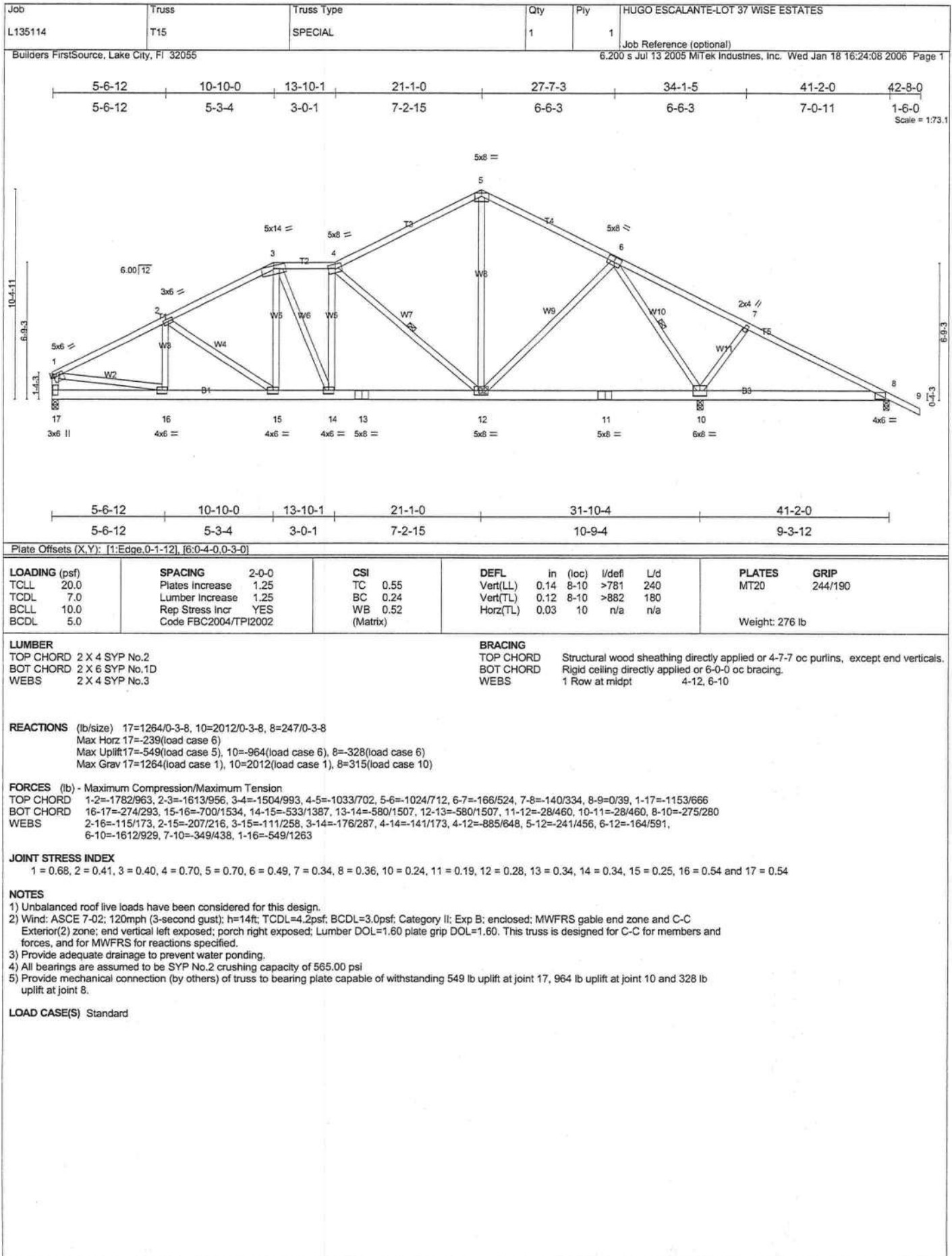
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 538 lb uplift at joint 19, 912 lb uplift at joint 12 and 345 lb uplift at joint 10.

LOAD CASE(S) Standard









Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T16	SPECIAL	1	1	Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:09 2006 Page 1

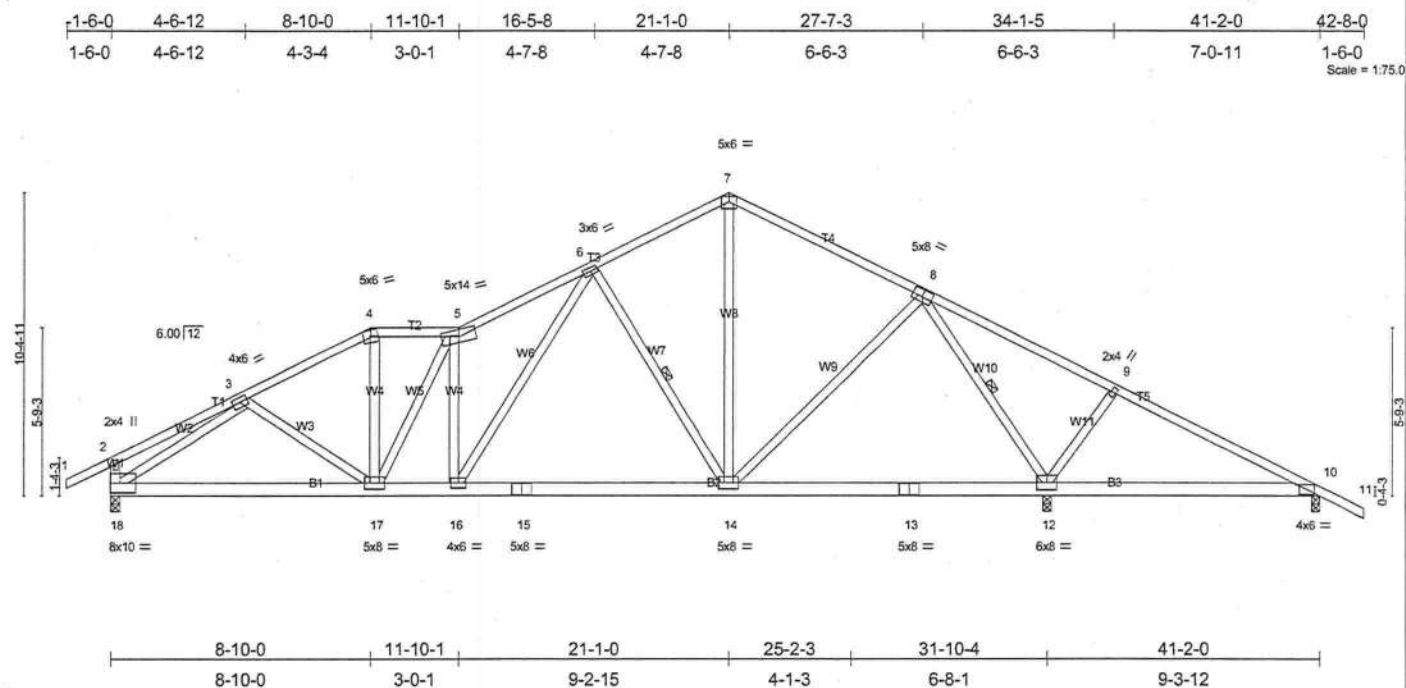


Plate Offsets (X,Y): [8-0-4-0,0-3-0]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d		PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	0.14 10-12 >775	240	MT20 244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	0.13 10-12 >880	180	
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.03 12 n/a	n/a	
BCDL	5.0	Code FBC2004/TP12002		(Matrix)					Weight: 279 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-4-15 oc purlins, except end verticals.
BOT CHORD 2 X 6 SYP No.1D	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
WEBS 2 X 4 SYP No.3	6-0-0 oc bracing: 10-12,
	1 Row at midpt 6-14, 8-12

REACTIONS (lb/size) 12=2029/0-3-8, 18=1351/0-3-8, 10=231/0-3-8
 Max Horz 18=-214(load case 6)
 Max Uplift 12=-969(load case 6), 18=-660(load case 5), 10=-324(load case 6)
 Max Grav 12=2029(load case 1), 18=1351(load case 1), 10=309(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=359/180, 3-4=1690/955, 4-5=-1478/918, 5-6=-1954/1252, 6-7=-984/718, 7-8=-1014/701, 8-9=-191/561, 9-10=-169/373,
 10-11=0/39, 2-18=357/341
BOT CHORD 17-18=-692/1411, 16-17=-662/1688, 15-16=-403/1214, 14-15=-403/1214, 13-14=-16/445, 12-13=-16/445, 10-12=-310/305
WEBS 3-17=-5/134, 4-17=-233/514, 5-17=-447/250, 5-16=-631/507, 6-16=-589/939, 6-14=-742/589, 8-14=-153/587, 8-12=-1644/950,
 9-12=-344/432, 3-18=-1425/833, 7-14=-325/524

JOINT STRESS INDEX
2 = 0.80, 3 = 0.42, 4 = 0.35, 5 = 0.47, 6 = 0.53, 7 = 0.56, 8 = 0.51, 9 = 0.34, 10 = 0.36, 12 = 0.25, 13 = 0.18, 14 = 0.28, 15 = 0.32, 16 = 0.50, 17 = 0.29 and 18 = 0.34

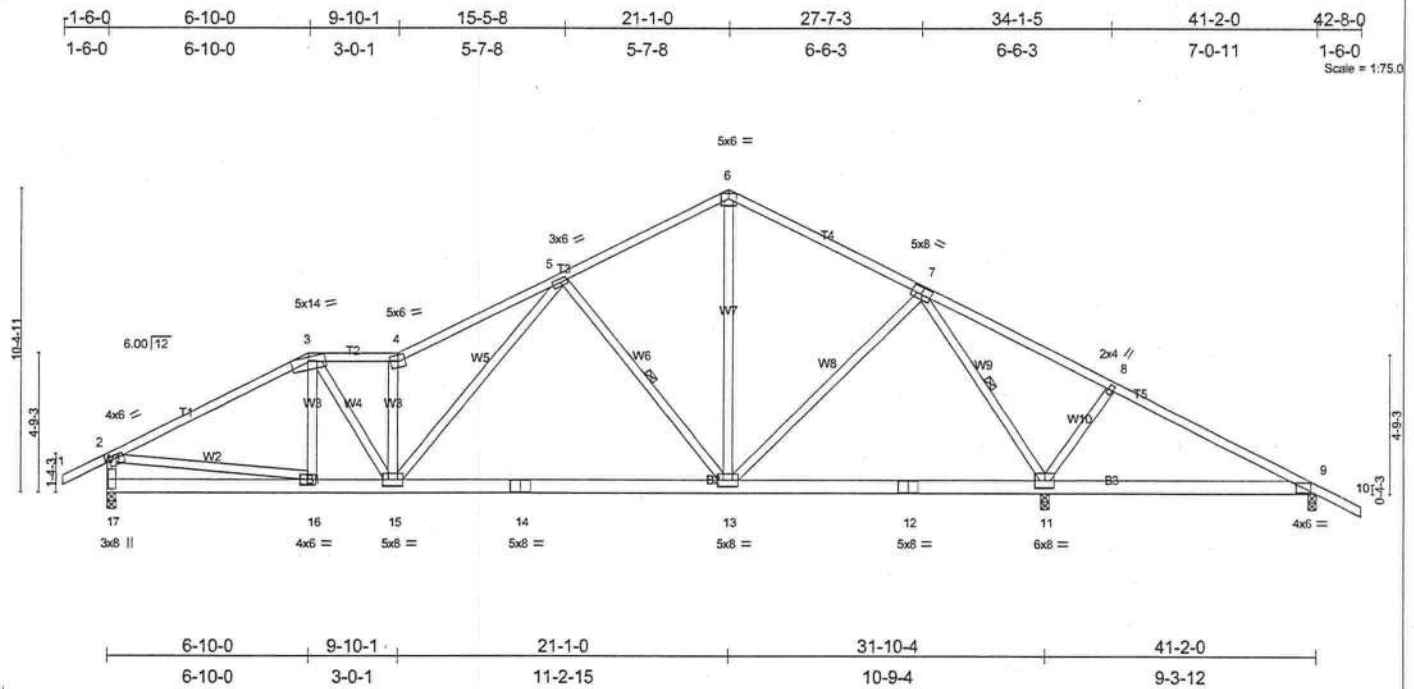
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); $h=14$ ft; $TCDL=4.2$ psf; $BCDL=3.0$ psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 969 lb uplift at joint 12, 660 lb uplift at joint 18 and 324 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T17	SPECIAL	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:10 2006 Page 1		

**LUMBER**

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-11 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
 WEBS 6-0-0 oc bracing: 9-11.
 1 Row at midpt 5-13, 7-11

REACTIONS

(lb/size) 17=1338/0-3-8, 11=2086/0-3-8, 9=188/0-3-8
 Max Horz 17=-214(load case 6)
 Max Uplift 17=-656(load case 5), 11=-989(load case 6), 9=-308(load case 6)
 Max Grav 17=1338(load case 1), 11=2086(load case 1), 9=281(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/40, 2-3=-1760/942, 3-4=-1884/1090, 4-5=-2205/1341, 5-6=-980/697, 6-7=-991/688, 7-8=-240/662, 8-9=-209/466, 9-10=0/39,
 2-17=-1208/817
 BOT CHORD 16-17=-316/356, 15-16=-655/1491, 14-15=-465/1280, 13-14=-465/1280, 12-13=0/395, 11-12=0/395, 9-11=-393/348
 WEBS 3-16=-130/105, 3-15=-307/720, 4-15=-1139/761, 5-15=-590/1032, 5-13=-755/619, 6-13=-290/486, 7-13=-170/630, 7-11=-1720/984,
 8-11=-346/434, 2-16=-456/1152

JOINT STRESS INDEX

2 = 0.81, 3 = 0.67, 4 = 0.53, 5 = 0.55, 6 = 0.61, 7 = 0.51, 8 = 0.34, 9 = 0.34, 11 = 0.26, 12 = 0.16, 13 = 0.30, 14 = 0.42, 15 = 0.50, 16 = 0.48 and 17 = 0.46

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B: enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 656 lb uplift at joint 17, 989 lb uplift at joint 11 and 308 lb uplift at joint 9.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T19	HIP	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:11 2006 Page 1		

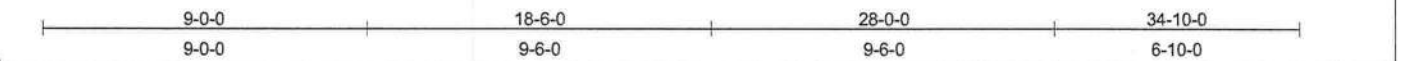
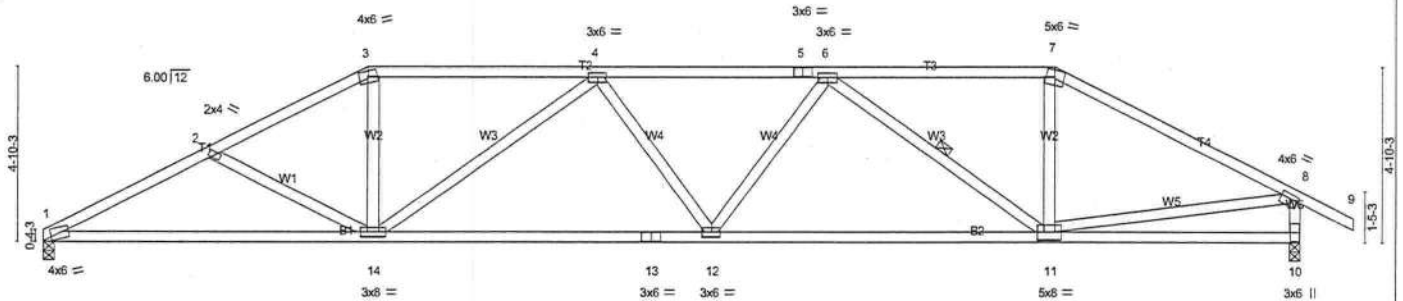
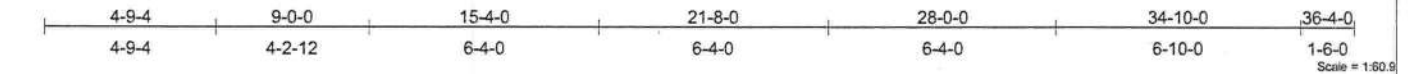


Plate Offsets (X,Y): [1:0-2-12,0-0-11], [8:0-3-0,0-1-8]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.53	Vert(LL)	-0.30 11-12	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.76	Vert(TL)	-0.50 11-12	>831	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.79	Horz(TL)	0.12 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
Weight: 178 lb									

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-5-7 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-9-1 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 6-11

REACTIONS (lb/size) 1=1449/0-3-8, 10=1542/0-3-8
 Max Horz 1=107(load case 6)
 Max Uplift 1=534(load case 5), 10=621(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=2706/1459, 2-3=2473/1306, 7-8=2084/1073, 8-9=0/40, 8-10=1447/913, 3-4=2193/1241, 4-5=2755/1491, 5-6=2755/1491, 6-7=1807/1051
 BOT CHORD 1-14=-1155/2371, 13-14=-1213/2724, 12-13=-1213/2724, 11-12=-1139/2587, 10-11=-85/217
 WEBS 2-14=-230/292, 3-14=-310/790, 4-14=-748/448, 4-12=0/108, 6-12=-29/317, 6-11=-1038/551, 7-11=-161/599, 8-11=-647/1596

JOINT STRESS INDEX
 1 = 0.87, 2 = 0.34, 3 = 0.79, 4 = 0.41, 5 = 0.59, 6 = 0.41, 7 = 0.73, 8 = 0.73, 10 = 0.42, 11 = 0.74, 12 = 0.41, 13 = 0.94 and 14 = 0.57

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 534 lb uplift at joint 1 and 621 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:12 2006 Page 1

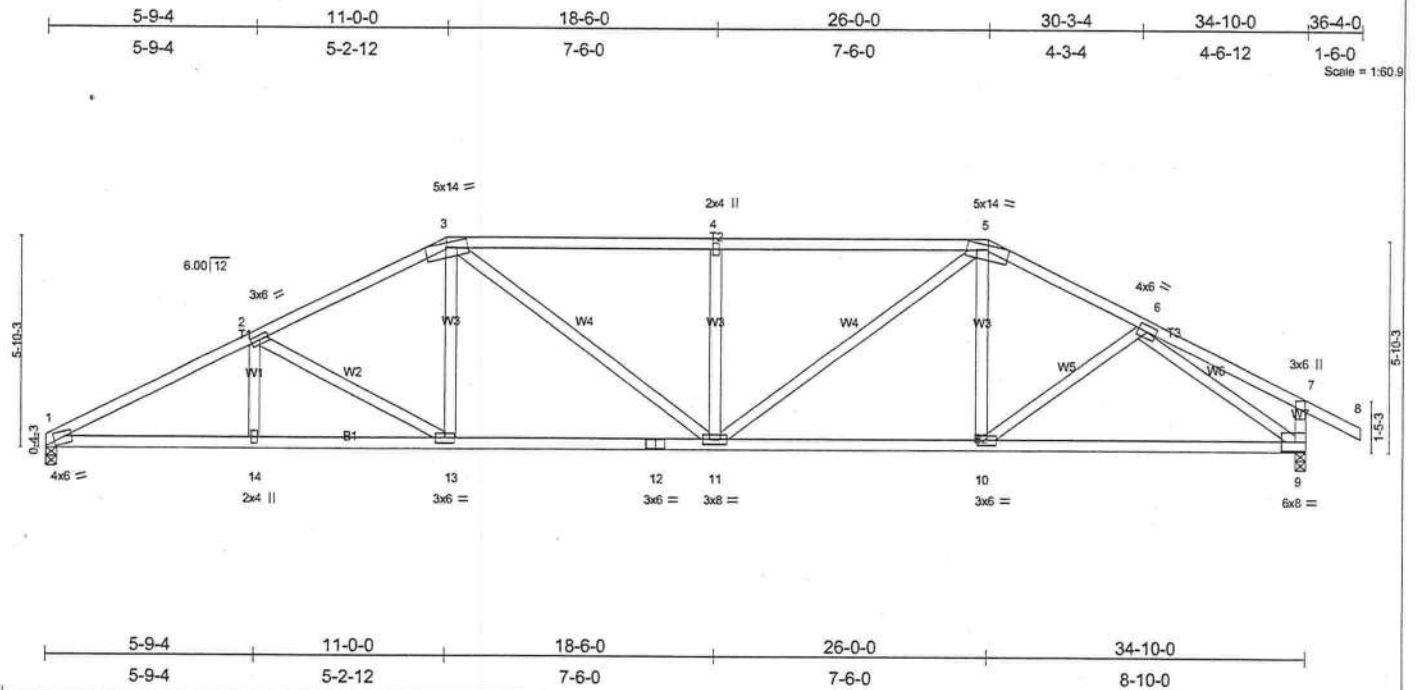


Plate Offsets (X,Y): [1:0-3-0,0-0-11]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.68	Vert(LL)	-0.22 11-13	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.61	Vert(TL)	-0.35 11-13	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.91	Horz(TL)	0.11 9	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)						
								Weight: 189 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-6-6 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-9-2 oc bracing.

REACTIONS (lb/size) 1=1449/0-3-8, 9=1542/0-3-8
 Max Horz 1=-123(load case 6)
 Max Uplift 1=-556(load case 5), 9=-645(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-2760/1443, 2-3=-2280/1260, 3-4=-2310/1353, 4-5=-2310/1353, 5-6=-1998/1113, 6-7=-336/173, 7-8=0/40, 7-9=-358/345
 BOT CHORD 1-14=-1127/2392, 13-14=-1127/2392, 12-13=-807/1998, 11-12=-807/1998, 10-11=-669/1757, 9-10=-701/1585
 WEBS 2-14=0/176, 2-13=-464/368, 3-13=-135/418, 3-11=-289/512, 4-11=-429/376, 5-11=-379/766, 5-10=-10/134, 6-10=-133/318, 6-9=-1703/960

JOINT STRESS INDEX
 1 = 0.88, 2 = 0.41, 3 = 0.92, 4 = 0.34, 5 = 0.93, 6 = 0.51, 7 = 0.31, 9 = 0.35, 10 = 0.35, 11 = 0.74, 12 = 0.70, 13 = 0.35 and 14 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 556 lb uplift at joint 1 and 645 lb uplift at joint 9.

LOAD CASE(S) Standard

Job L135114	Truss T22	Truss Type SPECIAL	Qty 1	Ply 1	HUGO ESCALANTE-LOT 37 WISE ESTATES
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		

6.200 s Jul 13 2005 Mittek Industries, Inc. Wed Jan 18 16:24:14 2006 Page 1

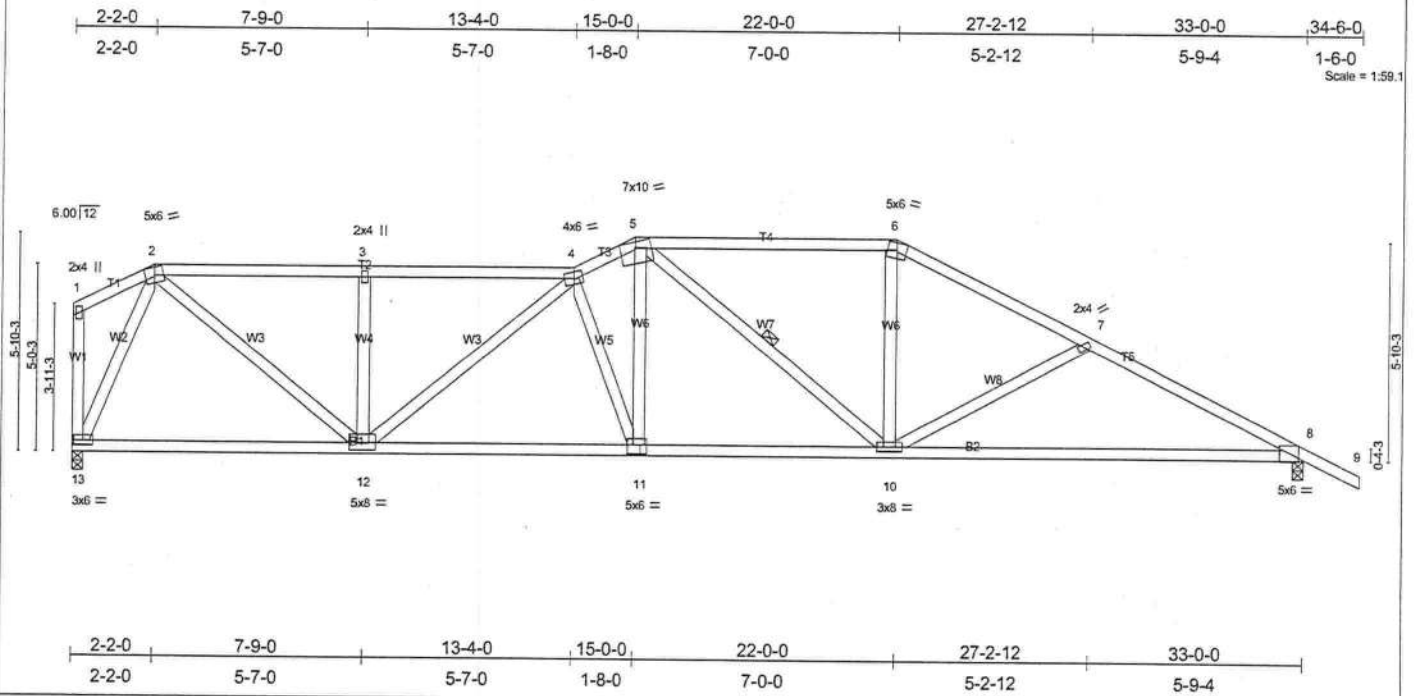


Plate Offsets (X,Y): [8:0-1-11,Edge], [11:0-2-4,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.50	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.85	Vert(LL) -0.40 8-10 >983 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.73	Vert(TL) -0.68 8-10 >578 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.09 8 n/a n/a		
	Code FBC2004/TP12002			Weight: 187 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B1 2 X 4 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-4-5 oc bracing.
 WEBS 1 Row at midpt 5-10

REACTIONS

(lb/size) 13=1372/0-3-8, 8=1465/0-3-8
 Max Horz 13=-288(load case 6)
 Max Uplift 13=-583(load case 5), 8=-640(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-58/18, 2-3=-1762/956, 3-4=-1762/956, 4-5=-2242/1245, 5-6=-1874/1083, 6-7=-2141/1123, 7-8=-2450/1304, 8-9=0/35, 1-13=-51/25
 BOT CHORD 12-13=-117/560, 11-12=-913/2284, 10-11=-767/2052, 8-10=-985/2143
 WEBS 2-12=-775/1573, 3-12=-331/289, 4-12=-683/409, 5-11=-371/830, 5-10=-355/164, 6-10=-183/602, 7-10=-332/354, 2-13=-1320/768,
 4-11=-708/435

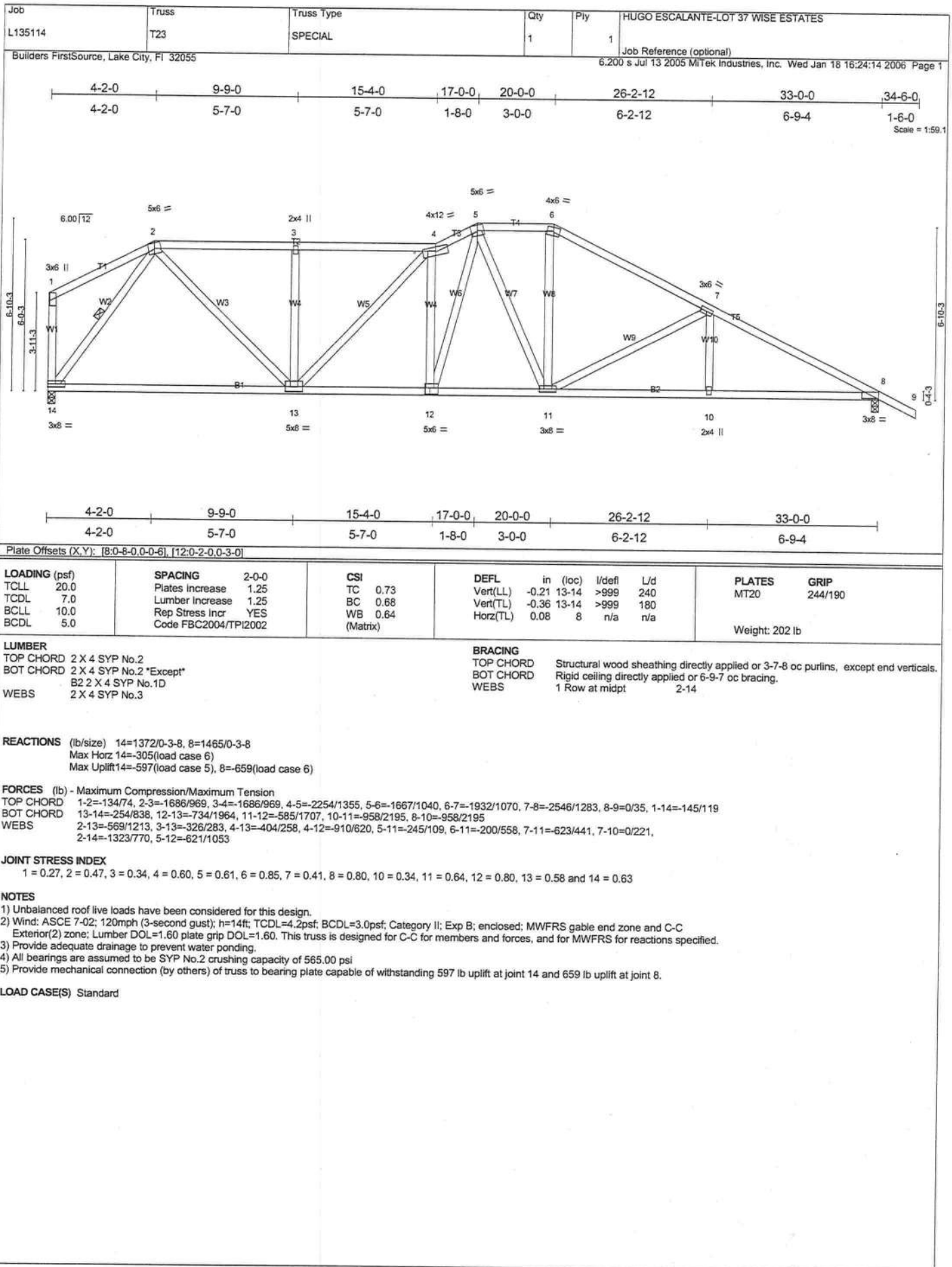
JOINT STRESS INDEX

1 = 0.44, 2 = 0.57, 3 = 0.34, 4 = 0.53, 5 = 0.66, 6 = 0.68, 7 = 0.34, 8 = 0.72, 10 = 0.57, 11 = 0.76, 12 = 0.74 and 13 = 0.58

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 583 lb uplift at joint 13 and 640 lb uplift at joint 8.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T24	SPECIAL	1	1	

Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 Mitek Industries, Inc. Wed Jan 18 16:24:15 2006 Page 1

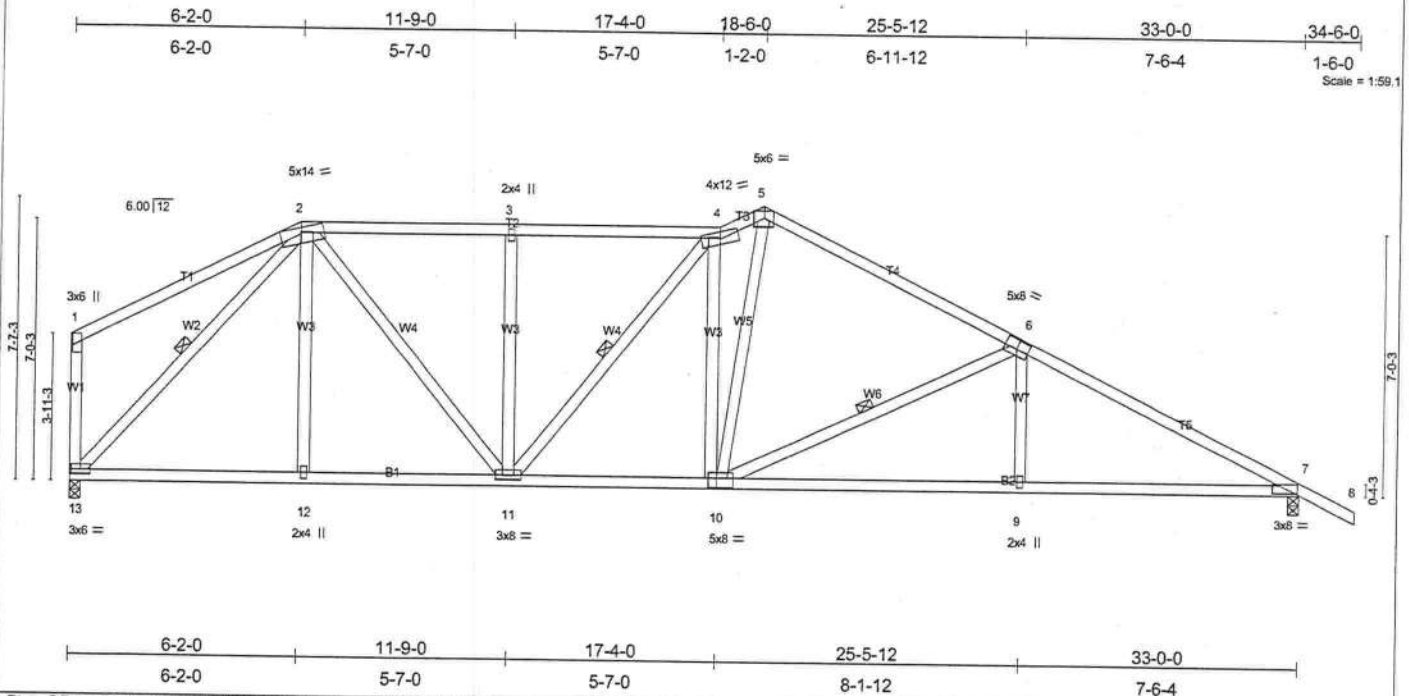


Plate Offsets (X,Y): [6:0-4-0,0-3-0], [7:0-8-8,0-0-10], [10:0-2-8,0-3-4]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.50	Ver(LL)	-0.20	9-10	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.71	Ver(TL)	-0.32	9-10	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.63	Horz(TL)	0.09	7	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 205 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-5-7 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-4-0 oc bracing.
 WEBS 1 Row at midpt 4-11, 6-10, 2-13

REACTIONS

(lb/size) 13=1372/0-3-8, 7=1465/0-3-8
 Max Horz 13=-317(load case 6)
 Max Uplift 13=-606(load case 5), 7=-671(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-142/161, 2-3=-1557/969, 3-4=-1557/969, 4-5=-1828/1119, 5-6=-1753/997, 6-7=-2512/1269, 7-8=0/35, 1-13=-191/218
 BOT CHORD 12-13=-316/1036, 11-12=-315/1040, 10-11=-568/1687, 9-10=-934/2153, 7-9=-934/2159
 WEBS 2-12=0/183, 2-11=-426/808, 3-11=-318/285, 4-11=-275/138, 4-10=-783/470, 5-10=-619/1238, 6-10=-746/514, 6-9=0/270, 2-13=-1458/703

JOINT STRESS INDEX

1 = 0.47, 2 = 0.34, 3 = 0.34, 4 = 0.41, 5 = 0.67, 6 = 0.68, 7 = 0.79, 9 = 0.34, 10 = 0.75, 11 = 0.82, 12 = 0.34 and 13 = 0.58

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 606 lb uplift at joint 13 and 671 lb uplift at joint 7.

LOAD CASE(S) Standard

Job L135114	Truss T24A	Truss Type HIP	Qty 1	Ply 1	HUGO ESCALANTE-LOT 37 WISE ESTATES
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		
6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:16 2006 Page 1					

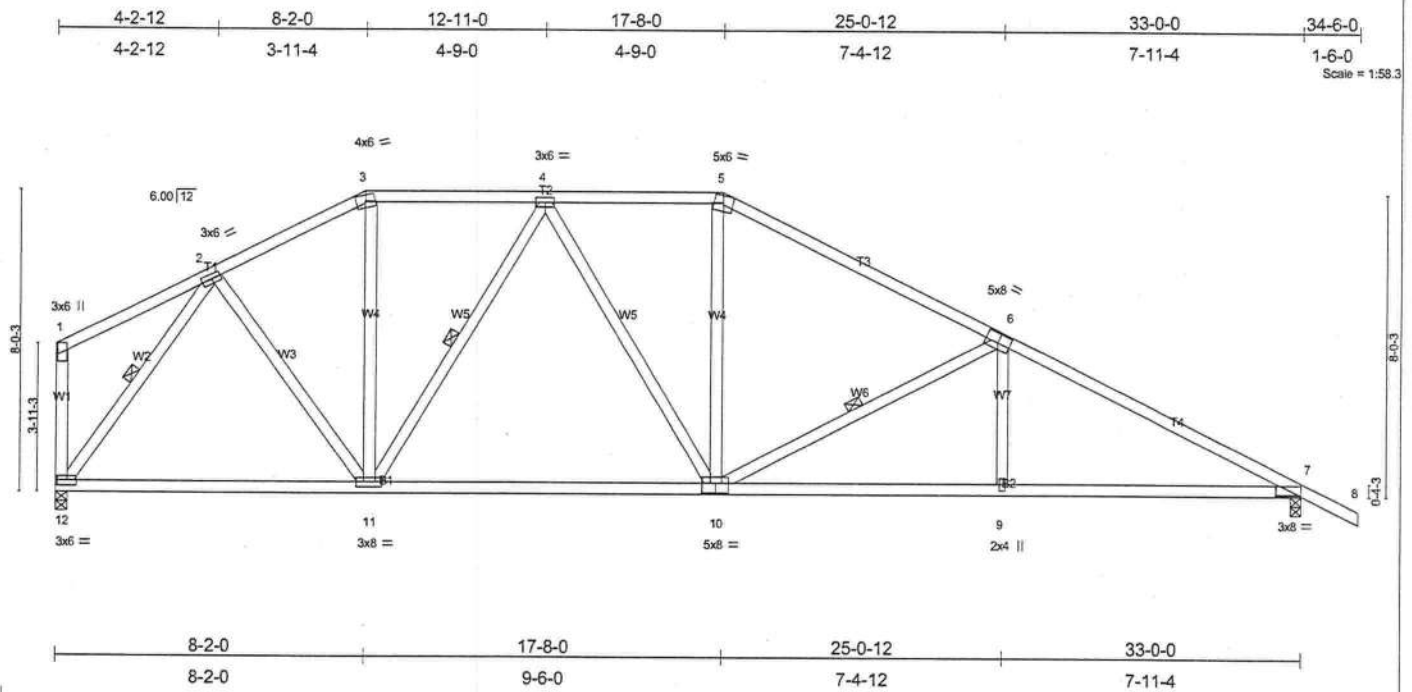


Plate Offsets (X,Y): [6:0-4-0-0-3-0], [7:0-8-0-0-0-6], [10:0-4-0-0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.53	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.69	Vert(LL) -0.20 10-11 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.36	Vert(TL) -0.34 10-11 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.09 7 n/a n/a		
	Code FBC2004/TPI2002			Weight: 196 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-4-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-9 oc bracing.
WEBS 1 Row at midpt 4-11, 6-10, 2-12

REACTIONS

(lb/size) 12=1372/0-3-8, 7=1465/0-3-8
Max Horz 12=-325(load case 6)
Max Uplift 12=-502(load case 5), 7=-677(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-111/84, 2-3=-1297/793, 3-4=-1127/767, 4-5=-1488/983, 5-6=-1750/995, 6-7=-2485/1267, 7-8=0/35, 1-12=-142/130
BOT CHORD 11-12=-211/839, 10-11=-393/1374, 9-10=-927/2128, 7-9=-927/2133
WEBS 2-11=-163/520, 3-11=-121/320, 4-11=-538/317, 4-10=-148/265, 5-10=-117/409, 6-10=-740/549, 6-9=0/255, 2-12=-1366/767

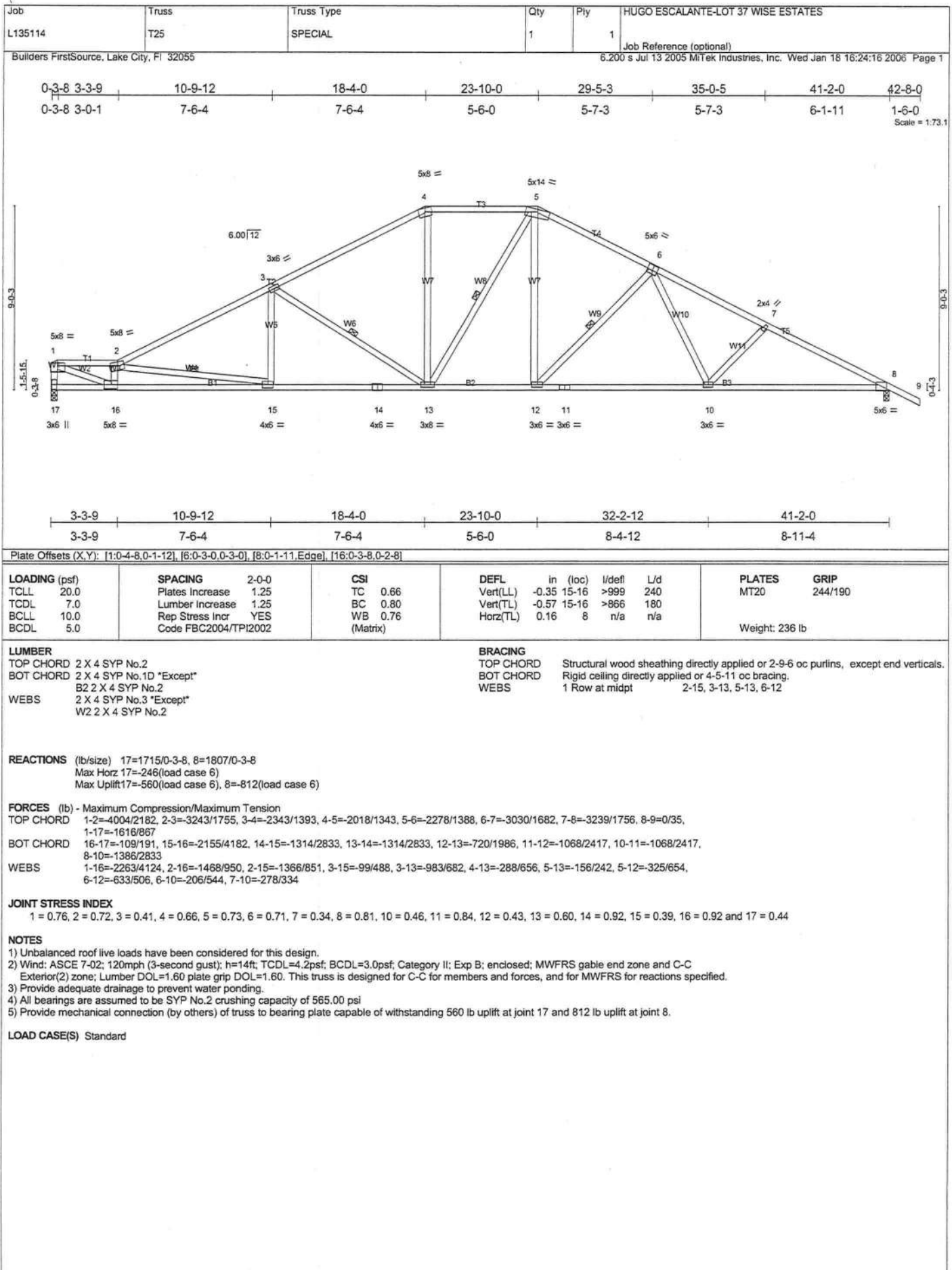
JOINT STRESS INDEX

1 = 0.25, 2 = 0.38, 3 = 0.56, 4 = 0.44, 5 = 0.73, 6 = 0.78, 7 = 0.78, 9 = 0.34, 10 = 0.61, 11 = 0.59 and 12 = 0.61

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 12 and 677 lb uplift at joint 7.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T28	SPECIAL	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:19 2006 Page 1		

1-6-0	6-3-8	9-3-9	15-2-4	21-1-0	27-7-3	34-1-5	41-2-0	42-8-0
1-6-0	6-3-8	3-0-1	5-10-12	5-10-12	6-6-3	6-6-3	7-0-11	1-6-0
Scale = 1:75.0								

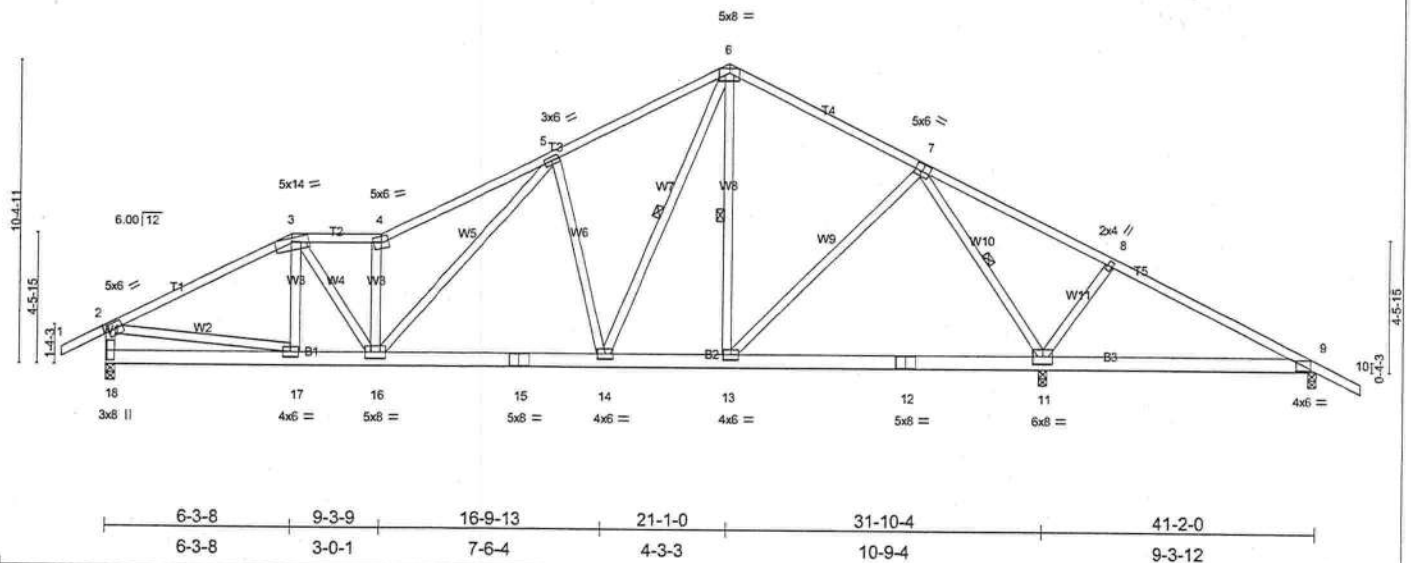


Plate Offsets (X,Y): [2:0-2-11,0-2-8], [7:0-3-0,0-3-0]																					
LOADING (psf)		SPACING		2-0-0		CSI		DEFL		in (loc)		l/defl		L/d		PLATES		GRIP			
TCLL 20.0		Plates Increase		1.25		TC 0.62		Vert(LL)		0.14		9-11		>781		240		MT20		244/190	
TCDL 7.0		Lumber Increase		1.25		BC 0.25		Vert(TL)		0.13		9-11		>878		180					
BCLL 10.0		Rep Stress Incr		YES		WB 0.87		Horz(TL)		0.02		11		n/a		n/a					
BCDL 5.0		Code FBC2004/TP12002				(Matrix)															
										Weight: 281 lb											

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-11-13 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
 6-0-0 oc bracing: 9-11.
 WEBS 1 Row at midpt 6-14, 6-13, 7-11

REACTIONS (lb/size) 18=1342/0-3-8, 11=2066/0-3-8, 9=203/0-3-8
 Max Horz 18=-214(load case 6)
 Max Uplift 18=-659(load case 5), 11=-980(load case 6), 9=-315(load case 6)
 Max Grav 18=1342(load case 1), 11=2066(load case 1), 9=288(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-1763/945, 3-4=-1919/1118, 4-5=-2254/1383, 5-6=-1377/992, 6-7=-987/696, 7-8=-213/620, 8-9=-181/424, 9-10=0/39,
 2-18=-1221/820
 BOT CHORD 17-18=-284/307, 16-17=-666/1499, 15-16=-486/1333, 14-15=-486/1333, 13-14=-123/812, 12-13=-10/409, 11-12=-10/409, 9-11=-356/324
 WEBS 3-17=-95/101, 3-16=-353/736, 4-16=-1187/811, 5-16=-599/968, 5-14=-710/621, 6-14=-664/924, 6-13=-199/203, 7-13=-163/606,
 7-11=-1675/965, 8-11=-345/434, 2-17=-505/1212

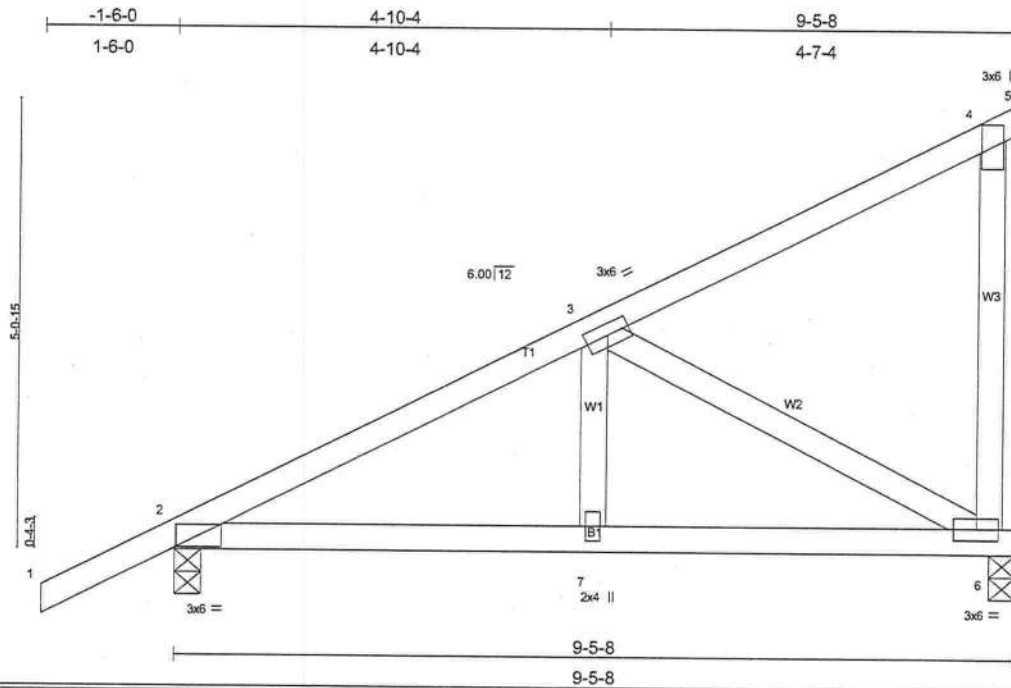
JOINT STRESS INDEX
 2 = 0.75, 3 = 0.53, 4 = 0.61, 5 = 0.59, 6 = 0.63, 7 = 0.75, 8 = 0.34, 9 = 0.36, 11 = 0.25, 12 = 0.19, 13 = 0.27, 14 = 0.51, 15 = 0.37, 16 = 0.47, 17 = 0.50 and 18 = 0.42

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 659 lb uplift at joint 18, 980 lb uplift at joint 11 and 315 lb uplift at joint 9.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	HUGO ESCALANTE-LOT 37 WISE ESTATES
L135114	T29	MONO TRUSS	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Wed Jan 18 16:24:19 2006 Page 1		



Scale = 1:24.8

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.23	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.14	Vert(LL) 0.04 2-7 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.20	Vert(TL) 0.04 2-7 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.01 6 n/a n/a		
	Code FBC2004/TP12002			Weight: 49 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 W3 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.

REACTIONS

(lb/size) 6=375/0-3-8, 2=476/0-3-8
 Max Horz 2=315(load case 5)
 Max Uplift 6=-385(load case 5), 2=-374(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-512/540, 3-4=-81/27, 4-5=-2/0, 4-6=-104/147
 BOT CHORD 2-7=-723/407, 6-7=-723/407
 WEBS 3-6=-439/781, 3-7=-324/147

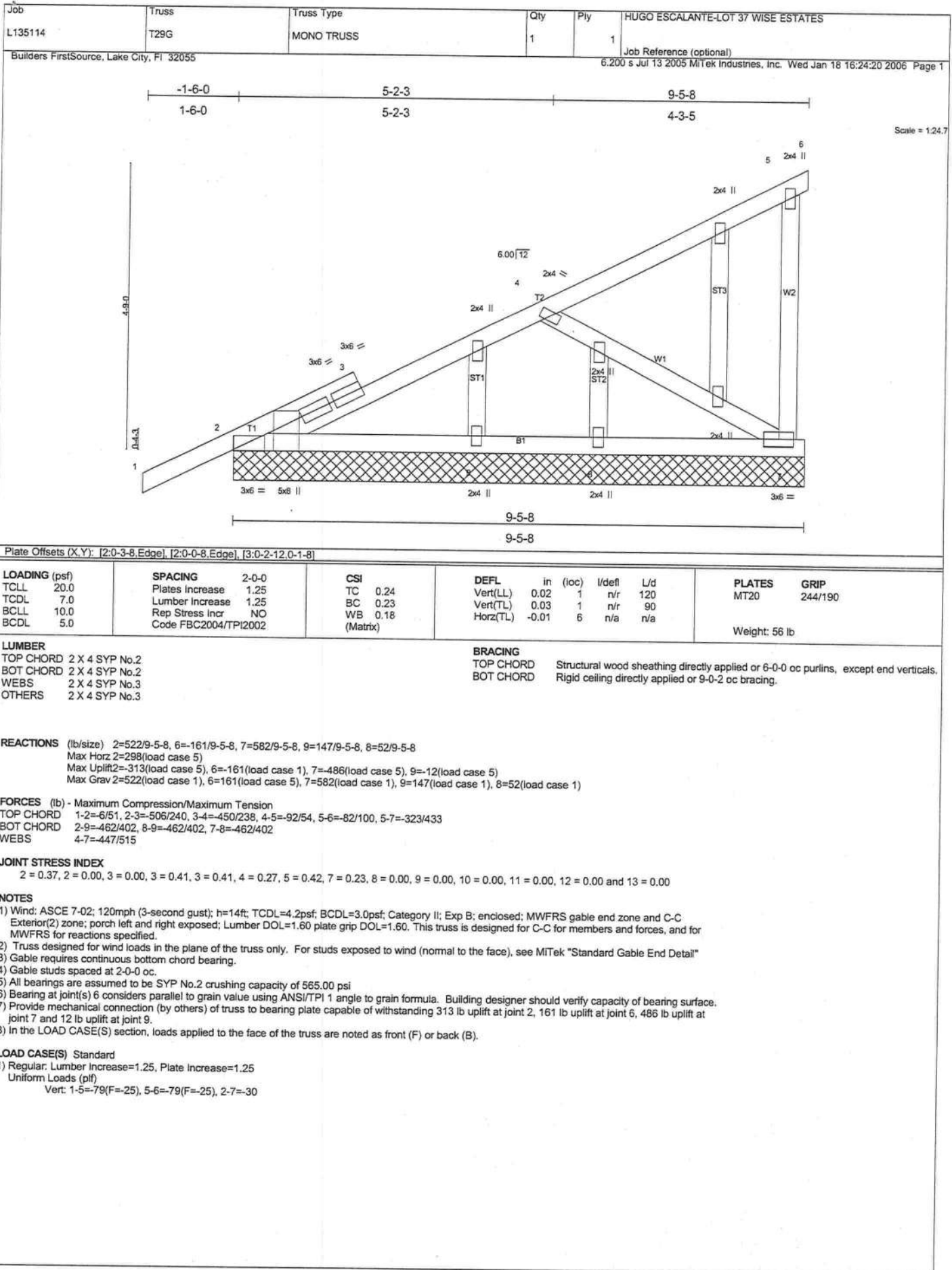
JOINT STRESS INDEX

2 = 0.41, 3 = 0.38, 4 = 0.30, 6 = 0.61 and 7 = 0.11

NOTES

- 1) Wind: ASCE 7-02: 120mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 6 and 374 lb uplift at joint 2.

LOAD CASE(S) Standard

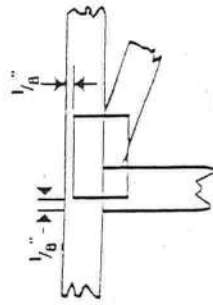


Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.



PLATE SIZE

4 X 4

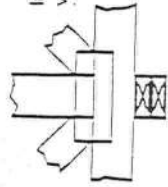
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



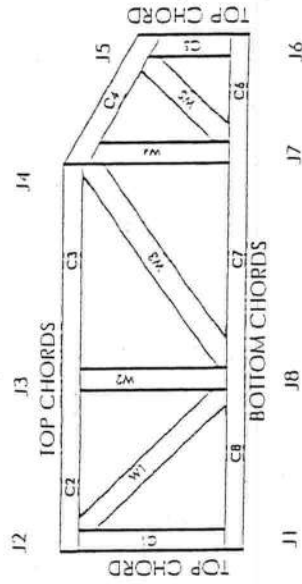
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILLIR	960022-W, 970036-H
IER	561



MITek Engineering Reference Sheet: MIT-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear lightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length ($\pm 6"$ from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing of 10 ft. spacing, or less. If no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

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Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: 1544A Ave

City: Lake City Phone: 752-1703

Site Location: Subdivision Wise Estates

Lot # 37C Block# Permit # 23825

Address 269 So Katcan Cir

Product used

Active Ingredient

% Concentration

☐ Premise Imidacloprid 0.1%

☐ Termidor Fipronil 0.12%

☒ Bora-Care Disodium Octaborate Tetrahydrate 23.0%

Type treatment:

☐ Soil

☒ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

Dwelling 2640 795 4

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

3-15-06 1250 F254 Gummy

Date

Time

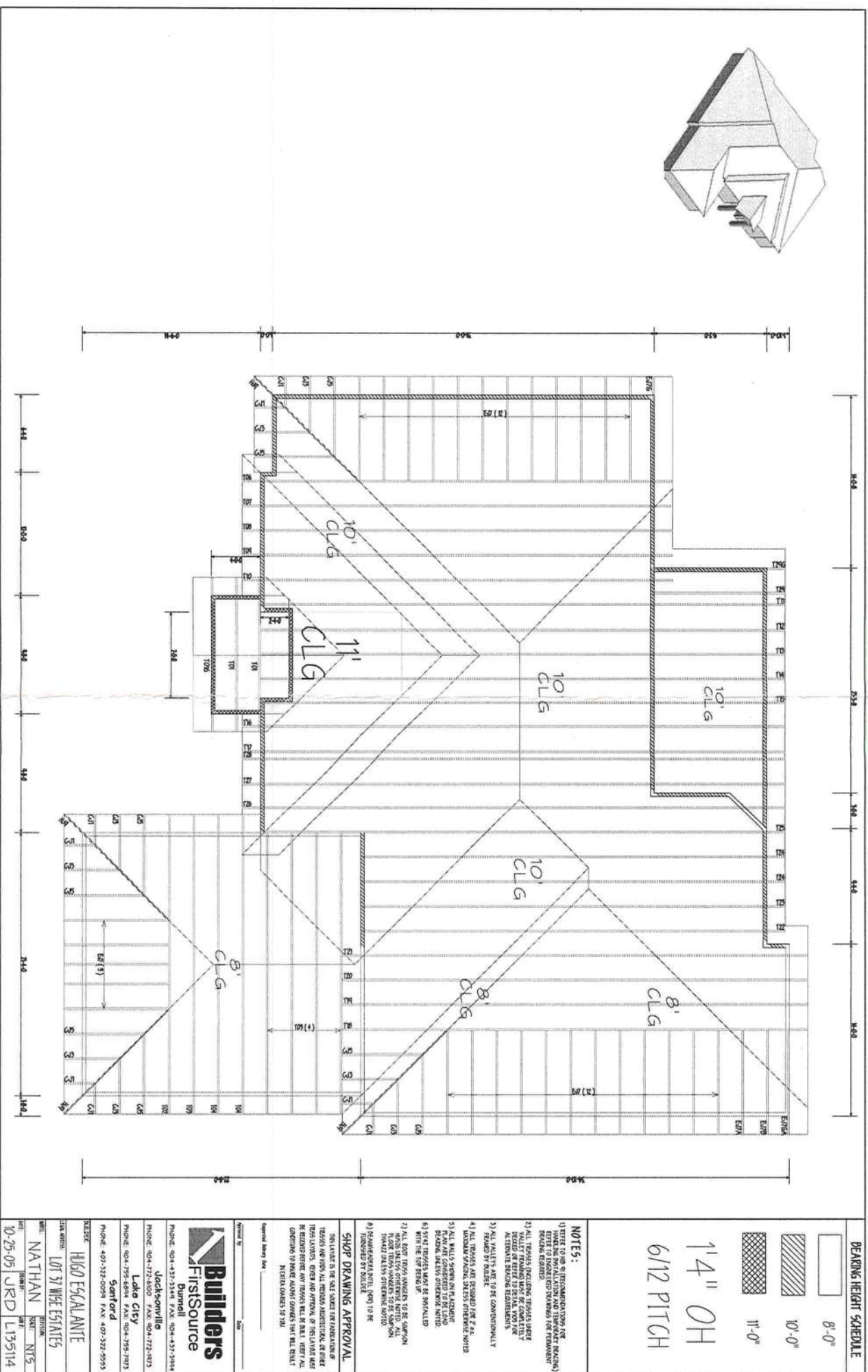
Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink



BEARING HEIGHT SCHEDULE

	8'-0"
	10'-0"
	11'-0"

1'4" OH
6/12 PITCH

NOTES:

- 1) REFER TO HD 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER FULLY PERMANENT BRACING) SHALL BE FULLY BRACED TO THE FOUNDATION OR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2 OC MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON R AGENT PLAN ARE CONSIDERED TO BE TEND BEARING, UNLESS OTHERWISE NOTED.
- 6) 5/42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL ROOF TRUSS HANGERS TO BE SHOWN UNLESS OTHERWISE NOTED. ALL TRUSS HANGERS TO BE SHOWN UNLESS OTHERWISE NOTED.
- 8) BEARING/DECKING (HPS) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VIDS. ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS, CENTER AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST DAMAGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Expend Sheet No. :
Approved By :
Date :



Burnell
PHONE: 904-437-3344 FAX: 904-437-3494
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 904-795-6894 FAX: 904-795-7473
Sanford
PHONE: 407-322-0099 FAX: 407-322-5953

BUILDER
HUGO ESCALANTE

LOT 37 WISE ESTATES

DATE: 10-25-05
DRAWN BY: JRD
SCALE: NTS
SHEET: L135114

