

DATE 07/31/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT

000027223

APPLICANT PATRICK WILSON PHONE 352 206-5459
ADDRESS 6800 SOUTHPOINT PARKWAY JACKSONVILLE FL 32024
OWNER MARONDA HOMES PHONE 904 296-1490
ADDRESS 228 SW TIMBER RIDGE DRIVE LAKE CITY FL 32024
CONTRACTOR THEODORE BROCK PHONE 407 227-3504
LOCATION OF PROPERTY 90W, TL ON 247S, TR ON 252B, TL ON TIMBER RIDGE,
3RD LOT ON LEFT
TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 154800.00
HEATED FLOOR AREA 2527.00 TOTAL AREA 3096.00 HEIGHT STORIES 1
FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB
LAND USE & ZONING RSF-2 MAX. HEIGHT
Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 10-4S-16-02856-122 SUBDIVISION TIMBERLANDS
LOT 22 BLOCK PHASE UNIT TOTAL ACRES 0.50
000001646 CBC1256382
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
CULVERT 08-458 BK WR Y
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident
COMMENTS: ELEVATION CONFIRMATION LETTER REQUIRED BEFORE SLAB, MFE AT 96'
PER PLAT
 Check # or Cash 918690

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic
 date/app. by date/app. by date/app. by
Under slab rough-in plumbing Slab Sheathing/Nailing
 date/app. by date/app. by date/app. by
Framing Rough-in plumbing above slab and below wood floor
 date/app. by date/app. by
Electrical rough-in Heat & Air Duct Peri. beam (Lintel)
 date/app. by date/app. by date/app. by
Permanent power C.O. Final Culvert
 date/app. by date/app. by date/app. by
M/H tie downs, blocking, electricity and plumbing Pool
 date/app. by date/app. by
Reconnection Pump pole Utility Pole
 date/app. by date/app. by date/app. by
M/H Pole Travel Trailer Re-roof
 date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 775.00 CERTIFICATION FEE \$ 15.48 SURCHARGE FEE \$ 15.48
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 905.96
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Harry, we have the original in file 27223

FIELD DENSITY WORKSHEET

CLIENT Maranda Homes DATE 1-9-09
 PROJECT NO. _____
 PROJECT NAME Timberlands Lot 22 PERMIT NO. _____
 EARTH CONTRACTOR _____ TESTED BY S.L.
 COMPACTION REQUIREMENT (%) 95 ☐ Standard Proctor ☐ Modified Proctor _____ FIELD CONTACT
 TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☐ Building Footing ☐ Other _____

TEST LOCATION	LAB PROCTOR		TEST DEPTH	PROBE DEPTH	% MOIST.	WET DENSITY (PCF)	DRY DENSITY (PCF)	% COMP.
	DENS.	OMC						
	105.8	10.7	F/G	12"				
8' SE of NW corner of Pad					6.0	109.8	103.6	97.9
Center of Pad					5.4	109.6	104.0	98.3
8' NW of SE corner of Pad					5.8	109.8	103.8	98.1

REMARKS _____

- * Density failed to meet minimum project requirement
- ** Retest indicates minimum density requirement was obtained.
- () Client is aware of unsatisfactory test results.

27223

FIELD DENSITY WORKSHEET

CLIENT Maronda Homes DATE 1-9-09
 PROJECT NAME Timberlands Lot 22 PROJECT NO. _____
 EARTH CONTRACTOR _____ PERMIT NO. _____
 COMPACTION REQUIREMENT (%) 95 ☐ Standard Proctor ☐ Modified Proctor FIELD CONTACT _____
 TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☐ Building Footing ☐ Other _____

TEST LOCATION	LAB PROCTOR		TEST DEPTH	PROBE DEPTH	% MOIST.	WET DENSITY (PCF)	DRY DENSITY (PCF)	% COMP.
	DENS.	OMC						
	105.8	10.7	F/G	12"				
8' SE of NW corner of Pad					6.0	109.8	103.6	97.9
Center of Pad					5.4	109.6	104.0	98.3
8' NW of SE corner of Pad					5.8	109.8	103.8	98.1

REMARKS _____

- * Density failed to meet minimum project requirement
- ** Retest indicates minimum density requirement was obtained.
- () Client is aware of unsatisfactory test results.

Columbia County Building Permit Application

For Office Use Only Application # 0807-07 Date Received 7/2 By TW Permit # 27223/1646
Zoning Official BLK Date 17.07.08 Flood Zone X per plot FEMA Map # N/A Zoning RSF-2
Land Use Res. Low Dev Elevation N/A MFE 96.0 ft River N/A Plans Examiner (signature) Date 7/15
Comments Elevation Confirmation Letter Required at Slab
☒ NOC ☐ EH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel # _____
☐ Dev Permit # _____ ☐ In Floodway ☒ Letter of Authorization from Contractor
☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. _____ Fax (904)-332-6367
Name Authorized Person Signing Permit Theodore C. Brock / Patrick Wilson Phone (904)-296-1490

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490

911 Address 228 SW Timber Ridge Dr. Lake City, FL 32024

Contractors Name Theodore C. Brock Phone (407)-227-3504

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771

Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 10-45-10-02850-122 Estimated Cost of Construction \$116,100.00

Subdivision Name Timberlands Lot 22 Block 1 Unit _____ Phase _____

Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge, 3rd lot on left.

Number of Existing Dwellings on Property 0

Construction of Residential Single Family Dwelling Total Acreage .50 Lot Size _____

Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height N/A

Actual Distance of Structure from Property Lines - Front 50.0' Side 32.0' Side 37.0' Rear 53.0'

Number of Stories 1 Heated Floor Area 2527 Total Floor Area 3096 Roof Pitch 12/6

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Send email 7/17/08

905.96

Columbia County Building Permit Application

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

Steve Hogg

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Theodore C. Brock

Contractor's License Number

CPC 1256382

Columbia County

Competency Card Number

Affirmed under penalty of perjury to by the Contractor and subscribed before me this

2

day of

July

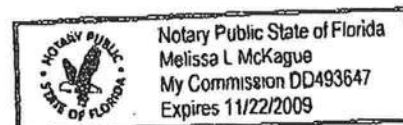
2008

Personally known XXX or Produced Identification

State of Florida Notary Signature (For the Contractor)

Melissa L. McKague

SEAL:



**UNANIMOUS WRITTEN CONSENT
OF THE BOARD OF DIRECTORS
OF
MARONDA HOMES, INC. OF FLORIDA**

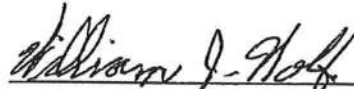
The undersigned, constituting the entire Board of Directors of Maronda Homes, Inc. of Florida (the "Corporation") and waiving all requirements of notice, consents to the corporate actions specified below and adopts the following resolutions by written consent, without a meeting, pursuant to the Florida Business Corporation Act:

RESOLVED, that the following persons be and are hereby authorized, empowered, and designated to enter into agreements to buy and sell individual lots and houses to individual purchasers, to execute deeds to effectuate the buying or selling of individual lots, and to enter into short-term leases not to exceed two (2) years for such individual houses.

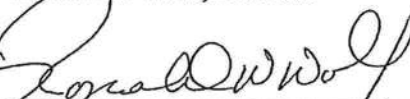
I FURTHER CERTIFY, that the following persons hold the following offices with the Corporation:

William J. Wolf	C.E.O.
Wayne Von Dreele	President
Ronald W. Wolf	Vice President
Ronald W. Wolf	Treasurer
Ronald W. Wolf	Secretary
Scott C. Howard	Vice President
Thomas Greenawalt	Vice President
William Rousch	Vice President
P. Thomas Prior	Vice President
Richard A. Brown	Vice President
Steven R. Hogg	Vice President
Dominick Donato	Vice President
Peter Chun	Vice President
George Friedman	Vice President
Mark Bowes	Vice President
Gregg S. Carlson	Vice President
Chad Johannesen	Vice President
William Berryhill	Vice President
John P. Mills	Vice President
John Williams	Vice President

IN WITNESS WHEREOF, the undersigned has executed this consent effective as of the 16th day of January 2008.



William J. Wolf, Director



Ronald W. Wolf, Director

This Instrument Prepared by and Return to :

Amy Weap
SOUTHERN TITLE HOLDING
COMPANY, LLC.
3943 BAY MEADOWS ROAD
JACKSONVILLE, Florida 32217

as a necessary incident to the fulfillment of conditions
contained in a title insurance commitment issued by it.

Property Appraisers Parcel LD. (Folio) Number(s):
R02856-000

Grantee(s) LD.#(s):
File No: JX0812085

Inst: 200812010775 Date: 6/6/2008 Time: 1:05 PM

Doc Stamp-Deed: 6293.00

DC, P. DeWitt Cason, Columbia County Page 1 of 1 8:1151 P:2385

WARRANTY DEED
(CORPORATION)

This Warranty Deed Made this 27th day of May, 2008, by RML HOLDINGS INC., A FLORIDA CORPORATION, and having its place of business at 703 NW BLACKBERRY CIRCLE, LAKE CITY, Florida 32055, hereinafter called the grantor,

to MARONDA HOMES, INC. OF FLORIDA, A FLORIDA CORPORATION, whose post office address is: 11200 ST. JOHNS INDUSTRIAL PARKWAY, JACKSONVILLE, FLORIDA 32246, hereinafter called the grantee,

\$899,000.00

WITNESSETH: That said grantor, for and in consideration of the sum of ~~\$48,000~~ Dollars and other valuable considerations, receipt whereof is hereby acknowledged, by these presents grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz: LOTS 1, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 24, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, AND 41, OF TIMBERLANDS, PHASE 1, ACCORDING TO PLAT THEREOF AS RECORDED IN PLAT BOOK 9, PAGE 26 AND 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining. To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007, reservations, restrictions and easements of record, if any.
(Wherever used herein the terms "grantor" and "grantee" included all the parties to this instrument, and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation.)

In Witness Whereof, the Grantor has caused these presents to be executed in its name, and its corporate seal to be hereunto affixed, by its proper officers thereunto duly authorized, the day and year first above written.

Signed, sealed and delivered in our presence:

ATTEST:

Secretary

RML HOLDINGS INC.

Witness Signature: Worth D. Morris
Printed Name: WORTH D. MORRIS

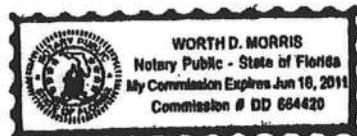
Witness Signature: Jodym. Goble. AWP
Printed Name: Jody M. Goble

BY: Robert R. Lardizabal
ROBERT R. LARDIZABAL, PRESIDENT

STATE OF FLORIDA
COUNTY OF DUVAL

The foregoing instrument was acknowledged before me this 28th day of May, 2008, by ROBERT R. LARDIZABAL as PRESIDENT of RML HOLDINGS INC., A FLORIDA CORPORATION, on behalf of the corporation. He/she is personally known to me or who has produced driver license(s) as identification.

My Commission Expires:



Worth D. Morris
Printed Name: WORTH D. MORRIS
Notary Public
Serial Number



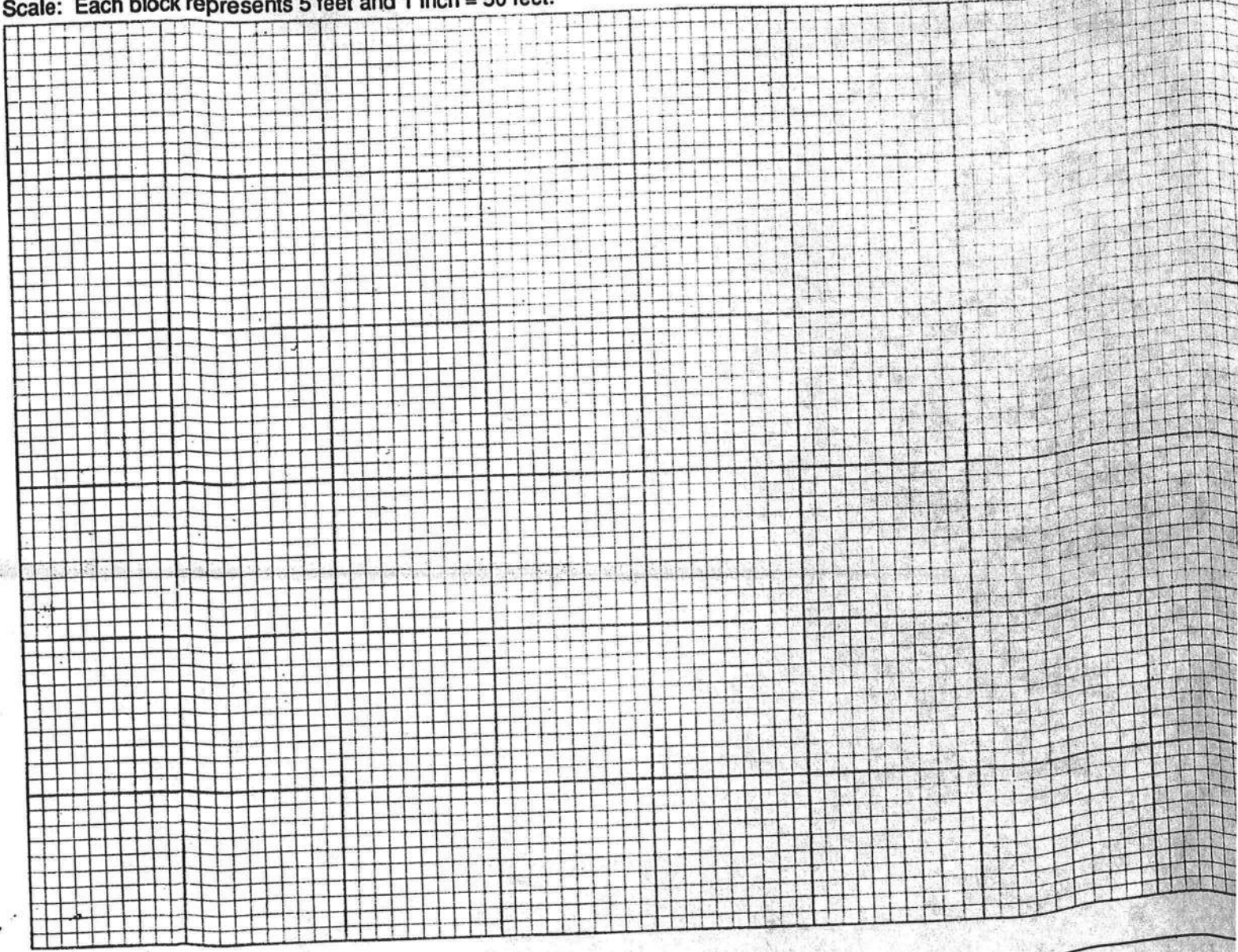
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 08-458

PART II - SITE PLAN

Lot 22

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:

See Site Plan

Site Plan submitted by: Patricia Valen 6/25/08

Signature

Plan Approved X

Not Approved

By

Salhi Ford ESII

Columbia CHD

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

STATE OF FLORIDA

LEGAL DESCRIPTION:

LOT TWENTY-TWO (22) OF "TIMBERLANDS, PHASE 1"
AS PER PLAT THEREOF, AS RECORDED IN PLAT
BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF
COLUMBIA COUNTY, FLORIDA.



PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA

S.W. TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

CERTIFIED TO:

- 1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT
25', REAR 15', SIDE 10'

SURVEYOR NOTES:

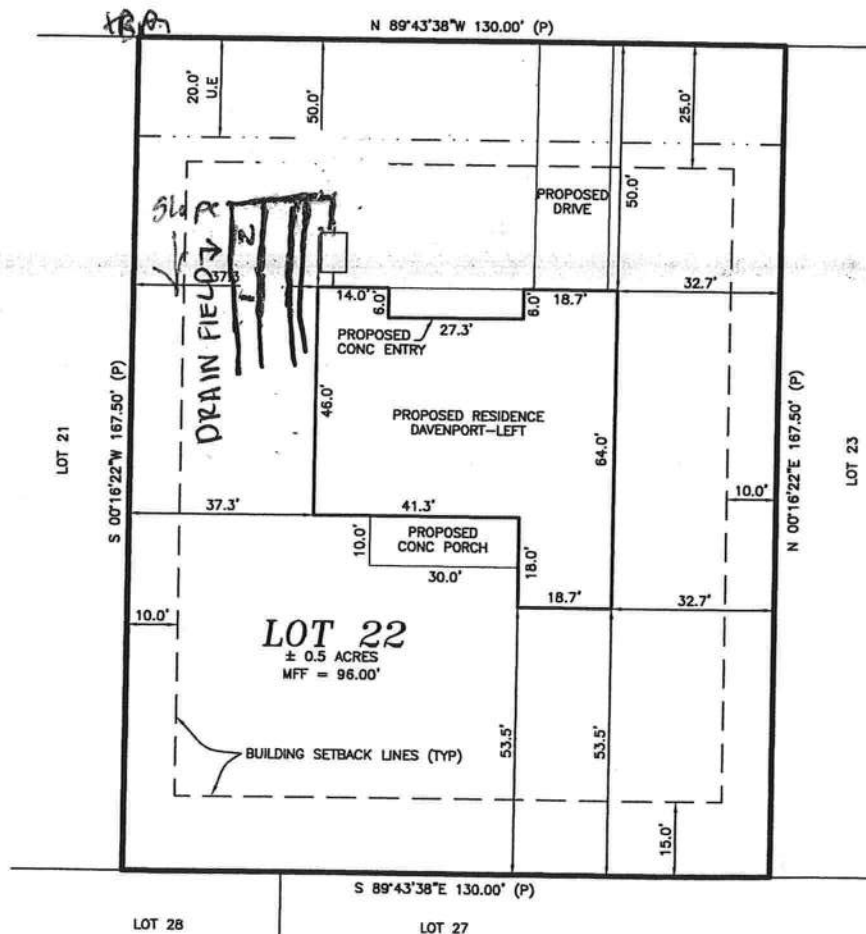
- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS, TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY CLAIMS ARE NOT CERTIFIED HEREON.



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

22/1 TM

Project Name: DAVENPORT GAINESVILLE	Builder: MARONDA HOMES
Address: 228 SW Timber Ridge Dr.	Permitting Office: Columbia
City, State: Lake City, FL 32045	Permit Number: 27223
Owner: ELECTRIC	Jurisdiction Number: 221000
Climate Zone: North	

1. New construction or existing New ☐	12. Cooling systems
2. Single family or multi-family Single family ☐	a. Central Unit Cap: 47.5 kBtu/hr
3. Number of units, if multi-family 1 ☐	SEER: 13.00 ☐
4. Number of Bedrooms 4 ☐	b. N/A ☐
5. Is this a worst case? Yes ☐	c. N/A ☐
6. Conditioned floor area (ft²) 2527 ft² ☐	13. Heating systems
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)	a. Electric Heat Pump Cap: 47.5 kBtu/hr
a. U-factor: Description Area	HSPF: 8.00 ☐
(or Single or Double DEFAULT) 7a(Sngle Default) 237.0 ft² ☐	b. N/A ☐
b. SHGC:	c. N/A ☐
(or Clear or Tint DEFAULT) 7b. (Clear) 237.0 ft² ☐	14. Hot water systems
8. Floor types	a. Electric Resistance Cap: 50.0 gallons
a. Slab-On-Grade Edge Insulation R=0.0, 211.0(p) ft ☐	EF: 0.90 ☐
b. N/A ☐	b. N/A ☐
c. N/A ☐	c. Conservation credits
9. Wall types	(HR-Heat recovery, Solar
a. Concrete, Int Insul, Exterior R=4.1, 1173.0 ft² ☐	DHP-Dedicated heat pump)
b. Frame, Wood, Adjacent R=13.0, 326.0 ft² ☐	15. HVAC credits PT, ☐
c. N/A ☐	(CF-Ceiling fan, CV-Cross ventilation,
d. N/A ☐	HF-Whole house fan,
e. N/A ☐	PT-Programmable Thermostat,
10. Ceiling types	MZ-C-Multizone cooling,
a. Under Attic R=19.0, 2650.0 ft² ☐	MZ-H-Multizone heating)
b. N/A ☐	
c. N/A ☐	
11. Ducts	
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 250.0 ft ☐	
b. N/A ☐	

Glass/Floor Area: 0.09

Total as-built points: 31338

Total base points: 32964

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY:**DATE:** Wayne Campbell 06/17/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Melissa McKame**DATE:** Melissa McKame 06/17/08

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

**BUILDING OFFICIAL:** _____**DATE:** _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT								
GLASS TYPES												
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points					
.18	2527.0	18.59	8456.0	1.Single, Clear	W	1.0	6.0	71.0	43.84	0.97	3020.0	
				2.Single, Clear	S	1.0	6.0	16.0	40.81	0.94	614.0	
				3.Single, Clear	E	1.0	6.0	78.0	47.92	0.97	3618.0	
				4.Single, Clear	W	1.0	8.0	40.0	43.84	0.99	1734.0	
				5.Single, Clear	N	1.0	6.0	32.0	21.73	0.97	677.0	
				As-Built Total:				237.0		9663.0		
WALL TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points					
Adjacent	326.0	0.70	228.2	1. Concrete, Int Insul, Exterior	4.1		1173.0	1.13	1331.4			
Exterior	1173.0	1.70	1994.1	2. Frame, Wood, Adjacent	13.0		326.0	0.60	195.6			
Base Total:				As-Built Total:				1499.0		1527.0		
DOOR TYPES Area X BSPM = Points				Type	Area X SPM = Points							
Adjacent	18.0	2.40	43.2	1.Exterior Wood				20.0	6.10	122.0		
Exterior	38.0	6.10	231.8	2.Adjacent Wood				18.0	2.40	43.2		
				3.Exterior Wood				18.0	6.10	109.8		
Base Total:				As-Built Total:					56.0		275.0	
CEILING TYPES Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points					
Under Attic	2527.0	1.73	4371.7	1. Under Attic	19.0		2650.0	2.34 X 1.00		6201.0		
Base Total:				As-Built Total:					2650.0		6201.0	
FLOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points					
Slab	211.0(p)	-37.0	-7807.0	1. Slab-On-Grade Edge Insulation	0.0		211.0(p)	-41.20		-8693.2		
Raised	0.0	0.00	0.0									
Base Total:				As-Built Total:					211.0		-8693.2	
INFILTRATION Area X BSPM = Points				Area X SPM = Points								
	2527.0	10.21	25800.7									
								2527.0	10.21	25800.7		

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature


Theodore C. Brock

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)


Theodore C. Brock

Contractor's License Number

Columbia County

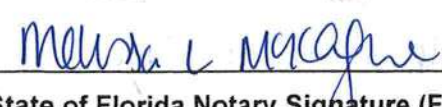
Competency Card Number

CB012510382

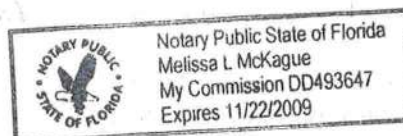
Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10 day of June 2008.

Personally known XXX or Produced Identification _____

State of Florida Notary Signature (For the Contractor)


Melissa L. McKague

SEAL:



SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 33318.7				Summer As-Built Points: 34773.4						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
33318.7	0.3250		10828.6	(sys 1: Central Unit 47500btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 34773	1.00	(1.08 x 1.147 x 0.86)	0.260	0.950		9206.6
33318.7	0.3250		10828.6	34773.4	1.00	1.072	0.260	0.950		9206.6

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT								
GLASS TYPES												
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points					
.18	2527.0	20.17	9175.0	1.Single, Clear	W	1.0	6.0	71.0	28.84	1.01	2063.0	
				2.Single, Clear	S	1.0	6.0	16.0	20.24	1.03	332.0	
				3.Single, Clear	E	1.0	6.0	78.0	26.41	1.02	2093.0	
				4.Single, Clear	W	1.0	8.0	40.0	28.84	1.00	1157.0	
				5.Single, Clear	N	1.0	6.0	32.0	33.22	1.00	1063.0	
				As-Built Total:				237.0		6708.0		
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM		= Points			
Adjacent	326.0	3.60	1173.6	1. Concrete, Int Insul, Exterior	4.1		1173.0	6.42	7530.7			
Exterior	1173.0	3.70	4340.1	2. Frame, Wood, Adjacent	13.0		326.0	3.30	1075.8			
Base Total:				As-Built Total:				1499.0		8606.5		
DOOR TYPES Area X BWPM = Points				Type			Area X WPM		= Points			
Adjacent	18.0	11.50	207.0	1.Exterior Wood			20.0	12.30	246.0			
Exterior	38.0	12.30	467.4	2.Adjacent Wood			18.0	11.50	207.0			
				3.Exterior Wood			18.0	12.30	221.4			
Base Total:				As-Built Total:				56.0		674.4		
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM		= Points			
Under Attic	2527.0	2.05	5180.4	1. Under Attic	19.0		2650.0	2.70 X 1.00	7155.0			
Base Total:				As-Built Total:				2650.0		7155.0		
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM		= Points			
Slab	211.0(p)	8.9	1877.9	1. Slab-On-Grade Edge Insulation	0.0		211.0(p)	18.80	3966.8			
Raised	0.0	0.00	0.0									
Base Total:				As-Built Total:				211.0		3966.8		
INFILTRATION Area X BWPM = Points								Area X WPM		= Points		
								2527.0		-0.59 -1490.9		

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE			AS-BUILT						
Winter Base Points: 20930.4			Winter As-Built Points: 25619.7						
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
			(sys 1: Electric Heat Pump 47500 btuh ,EFF(8.0) Ducts:Unc(S),Con(R),Int(AH),R6.0						
			25619.7	1.000	(1.060 x 1.169 x 0.88)	0.426	0.950	11357.7	
20930.4	0.5540	11595.5	25619.7	1.00	1.095	0.426	0.950	11357.7	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
WATER HEATING				Tank	EF	Number of	X	Tank X	Multiplier X Credit = Total
Number of	X	Multiplier	=	Volume		Bedrooms		Ratio	Multiplier
Bedrooms									
4		2635.00	10540.0	50.0	0.90	4		1.00	2693.56 1.00 10774.2
				As-Built Total:					10774.2

CODE COMPLIANCE STATUS

BASE						AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
10829		11595		10540	32964	9207		11358		10774	31338

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC-rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.7

The higher the score, the more efficient the home.

ELECTRIC, , ,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 47.5 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	2527 ft ²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 47.5 kBtu/hr
(or Single or Double DEFAULT) 7a(Sngle Default)	237.0 ft ²		HSPF: 8.00
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT) 7b. (Clear)	237.0 ft ²		
8. Floor types		c. N/A	
a. Slab-On-Grade Edge Insulation	R=0.0, 211.0(p) ft		
b. N/A		14. Hot water systems	
c. N/A		a. Electric Resistance	Cap: 50.0 gallons
9. Wall types			EF: 0.90
a. Concrete, Int Insul, Exterior	R=4.1, 1173.0 ft ²	b. N/A	
b. Frame, Wood, Adjacent	R=13.0, 326.0 ft ²	c. Conservation credits	
c. N/A		(HR-Heat recovery, Solar	
d. N/A		DHP-Dedicated heat pump)	
e. N/A		15. HVAC credits	PT,
10. Ceiling types		(CF-Ceiling fan, CV-Cross ventilation,	
a. Under Attic	R=19.0, 2650.0 ft ²	HF-Whole house fan,	
b. N/A		PT-Programmable Thermostat,	
c. N/A		MZ-C-Multizone cooling,	
11. Ducts		MZ-H-Multizone heating)	
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 250.0 ft			
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

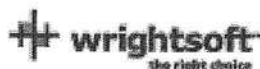
Builder Signature: Melvin McGee Date: 06/17/08

Address of New Home: 228 SW Timber Ridge Dr City/FL Zip: Lake City, FL 32815



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5.2)



Project Summary

Entire House

Maronda Homes

Job:
Date:
By: G. Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-3913 Fax: 407-321-0064

Project Information

For: DAVENPORT

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	34215 Btuh
Ducts	5655 Btuh
Central vent (100 cfm)	4048 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	43918 Btuh

Sensible Cooling Equipment Load Sizing

Structure	19692 Btuh
Ducts	12208 Btuh
Central vent (100 cfm)	1860 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	32747 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	2527	2527
Volume (ft³)	20216	20216
Air changes/hour	0.32	0.16
Equiv. AVF (cfm)	108	54

Latent Cooling Equipment Load Sizing

Structure	1895 Btuh
Ducts	2875 Btuh
Central vent (100 cfm)	3516 Btuh
Equipment latent load	8286 Btuh

Equipment total load	41034 Btuh
Req. total capacity at 0.76 SHR	3.6 ton

Heating Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Model N4H348AKA

Efficiency	8.3 HSPF
Heating input	0 Btuh @ 47°F
Heating output	0 °F
Temperature rise	0 cfm
Actual air flow	0.000 cfm/Btuh
Air flow factor	0.60 in H2O
Static pressure	
Space thermostat	

Cooling Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Cond N4H348AKA
Coil FSM4X4800A

Efficiency	13 SEER
Sensible cooling	35340 Btuh
Latent cooling	11160 Btuh
Total cooling	46500 Btuh
Actual air flow	1460 cfm
Air flow factor	0.046 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.80

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



wrightsoft

Right-Suite Residential 6.0.90 RSR21115

C:\Documents and Settings\gcarmack\My Documents\Man J Calcs 2007\GAINESVILLE\DAVENPORT 13.mp Cal

2008-Jun-12 11:19:55

Page 1

Project Information

For: DAVENPORT

Design Conditions

Location:

Gainesville, FL, US
Elevation: 151 ft
Latitude: 30°N

Outdoor:

Dry bulb (°F)
Daily range (°F)
Wet bulb (°F)
Wind speed (mph)

Heating

33
-
-
15.0

Cooling

92
19 (M)
77
7.5

Indoor:

Indoor temperature (°F)
Design TD (°F)
Relative humidity (%)
Moisture difference (gr/lb)

Heating

70
37
30
10.6

Cooling

75
17
50
52.0

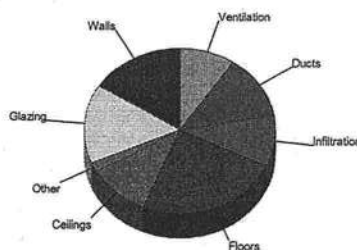
Infiltration:

Method
Construction quality
Fireplaces

Simplified
Average
0

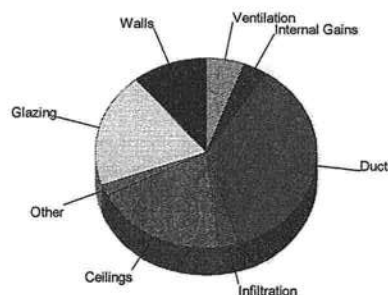
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.4	7021	16.0
Glazing	47.0	6814	15.5
Doors	14.4	808	1.8
Ceilings	1.8	4607	10.5
Floors	17.4	10602	24.1
Infiltration	2.4	4364	9.9
Ducts		5655	12.9
Piping		0	0.0
Humidification		0	0.0
Ventilation		4048	9.2
Adjustments		0	
Total		43918	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.3	3684	10.9
Glazing	45.6	6613	19.6
Doors	11.4	637	1.9
Ceilings	2.6	6555	19.4
Floors	0.0	0	0.0
Infiltration	0.6	1003	3.0
Ducts		12208	36.2
Ventilation		1860	5.5
Internal gains		1200	3.6
Blower		0	0.0
Adjustments		0	
Total		33760	100.0



Overall U-value = 0.163 Btuh/ft²-°F

Data entries checked.

Duct System Summary

Entire House

Maronda Homes

Job:
Date:
By: G. Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-3913 Fax: 407-321-0064

Project Information

For: DAVENPORT

	Heating	Cooling
External static pressure	0.60 in H ₂ O	0.60 in H ₂ O
Pressure losses	0.00 in H ₂ O	0.00 in H ₂ O
Available static pressure	0.60 in H ₂ O	0.60 in H ₂ O
Supply / return available pressure	0.46 / 0.14 in H ₂ O	0.46 / 0.14 in H ₂ O
Lowest friction rate	0.100 in/100ft	0.100 in/100ft
Actual air flow	1460 cfm	1460 cfm
Total effective length (TEL)	26 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER BDR	h 626.5	240	240	0.100	8	x0	VIFx	5.0	0.0	st5
MASTER BATH	h 294.7	75	75	0.100	6	x0	VIFx	10.0	0.0	st5
MASTER TOILET	h 111.6	25	25	0.100	4	x0	VIFx	20.0	0.0	st5
LIVING ROOM	h 310.7	155	155	0.100	7	x0	VIFx	5.0	0.0	st4
FAMILY ROOM	c 413.9	165	165	0.100	7	x0	VIFx	5.0	0.0	st4
DINING ROOM	h 434.1	125	125	0.100	6	x0	VIFx	5.0	0.0	st1
NOOK	h 265.3	125	125	0.100	6	x0	VIFx	20.0	0.0	st1
KITCHEN	c 262.7	100	100	0.100	6	x0	VIFx	10.0	0.0	st1
BDR #4	h 325.2	130	130	0.100	6	x0	VIFx	10.0	0.0	ST2
bDR #3	h 198.4	90	90	0.100	6	x0	VIFx	5.0	0.0	ST2
BATH	h 86.7	20	20	0.100	4	x0	VIFx	15.0	0.0	ST2
BDR #2	c 410.9	160	160	0.100	7	x0	VIFx	15.0	0.0	ST2
UTILITY	h 90.7	50	50	0.100	4	x0	VIFx	10.0	0.0	ST3

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	101.0	101.0	0.100	590	16	0 x 0	VinIFlx	st4 st1
ST2	Peak AVF	400	400	0.100	517	10	0 x 0	VinIFlx	
ST3	Peak AVF	50	50	0.100	284	4	0 x 0	VinIFlx	
st5	Peak AVF	660	660	0.100	562	14	0 x 0	VinIFlx	
st4	Peak AVF	340	340	0.100	683	12	0 x 0	VinIFlx	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1460	1460	6.0	2.308	669	20	x0 0		VIFx	

NOTICE OF COMMENCEMENT

Tax Parcel Identification Number 10-45-10-02856-122

County Clerk's Office Stamp or Seal

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): 22/1 Timberlands
a) Street (job) Address: 228 SW Timber Ridge Drive
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
a) Name and address: Maronda Homes Inc of FL 10800 Southpoint Pkwy #300 Jax FL 32216
b) Name and address of fee simple titleholder (if other than owner):
c) Interest in property:
4. Contractor Information
a) Name and address: Maronda Homes Inc of FL 10800 Southpoint Pkwy #300 Jax FL 32216
b) Telephone No.: (904) 296-1490 Fax No. (Opt.): (904) 332-0375
5. Surety Information
a) Name and address:
b) Amount of Bond:
c) Telephone No.: Fax No. (Opt.):
6. Lender
a) Name and address:
b) Phone No.:
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
a) Name and address: Southern Title Holding Co LLC 3945 Bay Meadows Rd Jax FL 32217
b) Telephone No.: (904) 739-2205 Fax No. (Opt.):
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name and address:
b) Telephone No.: Fax No. (Opt.):
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

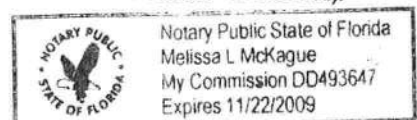
STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager
Theodore C. Brock
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 10 day of June, 20 08, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney
fact) for Maronda Homes Inc of Florida (name of party on behalf of whom instrument was executed).

Personally Known ☒ OR Produced Identification _____ Type _____

Notary Signature Melissa L McKague Notary Stamp or Seal:



11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
Signature of Natural Person Signing (in line #10 above.)

Columbia County Building Department Culvert Permit

Culvert Permit No.
000001646

DATE 07/31/2008 PARCEL ID # 10-4S-16-02856-122

APPLICANT PATRICK WILSON PHONE 352 206-5459

ADDRESS 6800 SOOUTHPOINT PARKWAY JACKSONVILLE FL 32216

OWNER MARONDA HOMES PHONE 904 296-1490

ADDRESS 228 SW TIMBER RIDGE DRIVE LAKE CITY FL 32024

CONTRACTOR THEODORE BROCK PHONE 407 227-3504

LOCATION OF PROPERTY 90W, TL 247S, TR ON 252B, TL ON TIMBER RIDGE, 3RD LOT ON
LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBERLANDS 22

SIGNATURE



INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



**COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST
FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006
Supplements and One (1) and Two (2) Family Dwellings**

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE- AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS:

- ☒ Two (2) complete sets of plans containing the following:
- ☒ All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- ☒ Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- ☒ Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

Site Plan information including:

- ☒ Dimensions of lot or parcel of land
- ☒ Dimensions of all building set backs
- ☒ Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- ☒ Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required:

- ☒ Plans or specifications must meet state compliance with FRC Chapter 3
- ☒ The following information must be shown as per section FRC
- ☒ Basic wind speed (3-second gust), miles per hour
- ☒ Wind importance factor and nature of occupancy
- ☐ Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- ☒ The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specically designed by the registered design professional.

Elevations Drawing including:

- ☒ All side views of the structure
- ☒ Roof pitch
- ☒ Overhang dimensions and detail with attic ventilation
- ☒ Location, size and height above roof of chimneys
- ☒ Location and size of skylights with Florida Product Approval
- ☒ Number of stories
- ☒ e) Building height from the established grade to the roofs highest peak

Floor Plan including:

- Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
- All exterior and interior shear walls indicated
- Shear wall opening shown (Windows, Doors and Garage doors)
- Emergency escape and rescue opening in each bedroom (net clear opening shown)
- Safety glazing of glass where needed
- Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
- Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
- Plans must show and identify accessibility of bathroom (see FRC 322)

All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

Foundation Plans Per FRC 403:

- a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling.
- d) Assumed load-bearing value of soil _____ (psf)
- e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

CONCRETE SLAB ON GRADE Per FRC R506

- Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports

PROTECTION AGAINST TERMITES Per FRC 320:

- Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606

- Show all materials making up walls, wall height, and Block size, mortar type
- Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

- Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers
- Girder type, size and spacing to load bearing walls, stem wall and/or piers
- Attachment of joist to girder
- Wind load requirements where applicable
- Show required under-floor crawl space
- Show required amount of ventilation opening for under-floor spaces
- Show required covering of ventilation opening.
- Show the required access opening to access to under-floor spaces
- Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- Show Draft stopping, Fire caulking and Fire blocking
- Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- Provide live and dead load rating of floor framing systems (psf).

WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- Fastener schedule for structural members per table R602.3 (1) are to be shown.
- Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- Indicate where pressure treated wood will be placed.
- Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

ROOF SYSTEMS:

- Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- Provide dead load rating of trusses

Conventional Roof Framing Layout Per FRC 802:

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- Valley framing and support details
- Provide dead load rating of rafter system.

ROOF SHEATHING FRC Table R602,3(2) FRC 803

- Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

- Include all materials which will make up the roof assemblies covering; with Florida Product Approval numbers for each component of the roof assemblies covering.

FBC Chapter 13 Florida Energy Efficiency Code for Building Construction

- Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

HVAC information shown

- Manual J sizing equipment or equivalent computation
- Exhaust fans locations in bathrooms

Plumbing Fixture layout shown

- All fixtures waste water lines shall be shown on the foundation plan

Electrical layout shown including:

- Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- Ceiling fans
- Smoke detectors
- Service panel, sub-panel, location(s) and total ampere ratings

- On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- Appliances and HVAC equipment and disconnects
- Arc Fault Circuits (AFCI) in bedrooms
- Notarized Disclosure Statement for Owner Builders
- Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice Of Commencement is required to be filed with the building department Before Any Inspections Will Be Done.

Private Potable Water

- Size of pump motor
- Size of pressure tank
- Cycle stop valve if used

CITY

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- City Approval: If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.** A development permit will also be required. The permit cost is \$50.00.
- Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

PRODUCT APPROVAL SPECIFICATION SHEET

Location: _____ **Project Name:** _____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

Location

Permit # (FOR STAFF USE ONLY)

Maronda Systems

Maronda Systems 4005 Maronda Way Sanford FL 32771 (407) 321-0064 Fax (407) 321-3913
 Engineer/Architect of Record: Tomas Ponce, P.E. 367 Medallion PL. Chuluota, FL 32766 FL PE # 50068,
 Design Criteria: TPI Design: Matrix Analysis MiTek software

PLAN JOB #	LOT	ADDRESS	DIV/SUB	MODEL	DAVENPORT A-M
9TM02201	22-1	228 SW TIMBER RIDGE DR	JAX-9TM	DVPM4 LEFT	

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone. Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek/Trenco.
 I, Tomas Ponce, P.E. the Architect/Engineer of Record for the above referenced lot Have reviewed the package and confirmed that it matches the physical and structural Parameters found on the set of permit drawings.

Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. Dwgs:
Layout	10/18/07	✓	S1	11/13/07	✓	46
V	07/27/05		T1	11/13/07		
HIP	11/02/06		T2	11/13/07		
B1	11/13/07		V1S	11/13/07		
BGRD1	01/10/08		V2S	11/13/07		
G1	11/13/07		V3S	11/13/07		
G2	11/13/07		V4S	11/13/07		
GRD1	11/13/07		V6	11/13/07		
GRD2	11/13/07		V7	11/13/07		
H1S	11/13/07		V8	11/13/07	✓	
H2S	11/13/07					
H3S	11/13/07					
H4S	11/13/07					
H5S	11/13/07					
H6	11/13/07					
HGRD1	11/13/07					
HS1	11/13/07					
HS2	11/13/07					
HS3	11/13/07					
HS4	11/13/07					
HS5	11/13/07					
HS6S	11/13/07					
HS7S	11/13/07					
HS8S	11/13/07					
HSGRD1S	11/13/07					
J	11/13/07					
J1	11/13/07					
J2	11/13/07		INV #	DESC	QNTY	
J3	11/13/07		50060.0048	THD26	15	
JGRD1	11/13/07		50060.0047	THD28		
JS	11/13/07		50060.0050	THDH28-3	1	
JS1	11/13/07		50060.0110	JUS26		
JS2	11/13/07		50060.0058	THJ26	3	
JS3	11/13/07		50060.0049	THD28-2	2	
JSA	11/13/07					
JSGRD1	11/13/07	✓				
			SEAT PLATES		150	

Roof Loads-
TC Live: 16.0 psf
TC Dead: 7.0 psf
BC Live: 10.0 psf
BC Dead: 10.0 psf
Total 43.0 psf
DurFac- Lbr: 1.25
DurFac- Plt: 1.25
O.C. Spacing: 24.0"

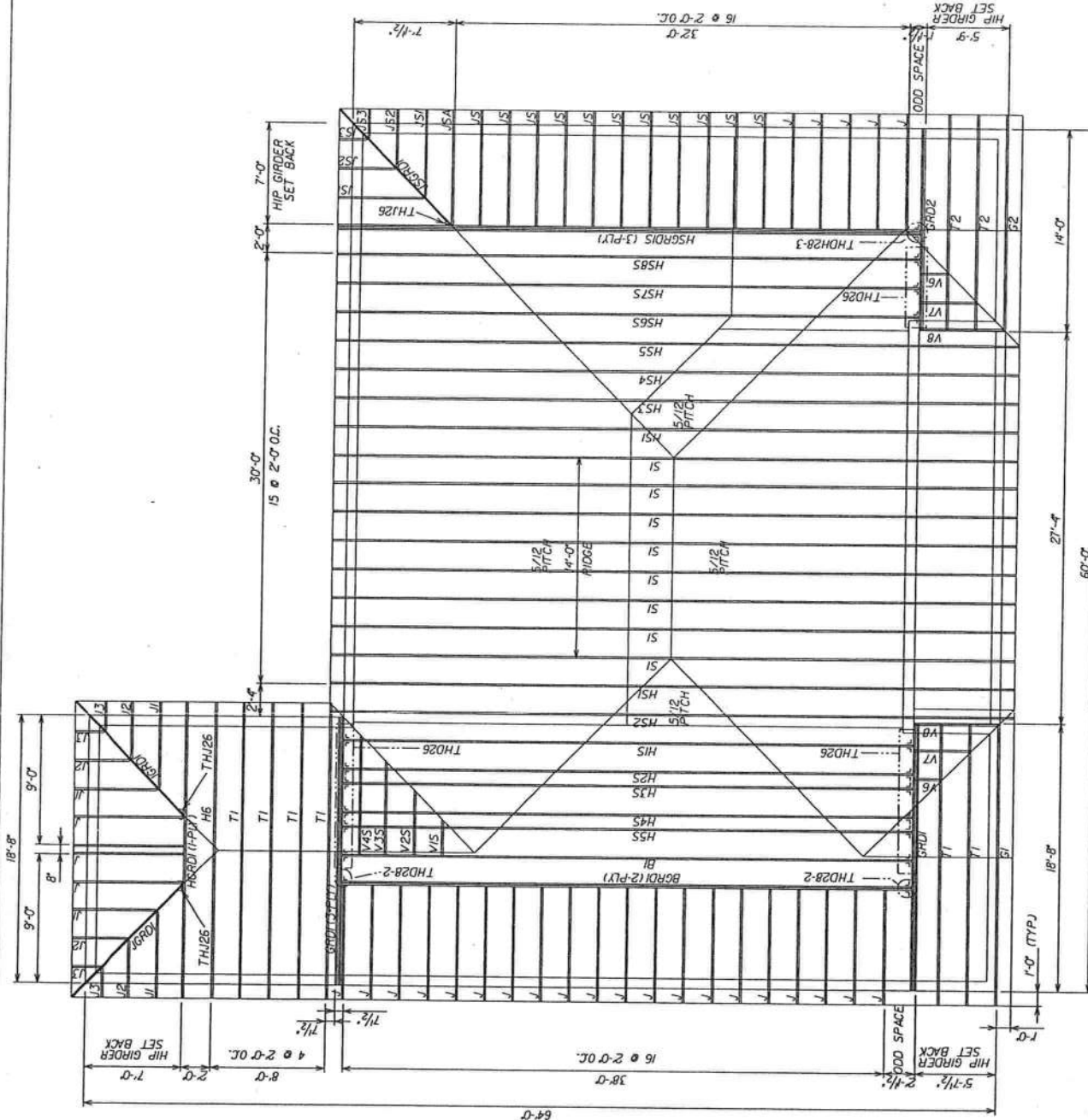


[Signature]
 JUN 05 2008

DATE:

HARDWARE LEGEND

- 1 HUS26
- 2 HUS28
- 3 JUS26
- 4 MP6F
- 5 MPA1 & MPA1F
- 6 SKH26 L/R
- 7 SKHH26 L/R
- 8 SUS26
- 9 SUS28
- 10 THD26
- 11 THD28
- 12 THD28-2
- 13 THD28-3
- 14 THD48
- 15 THJ26**
- 16 LTW12



JUN 05 2008

HARDWARE MANUFACTURED
BY USP

* HARDWARE MANUFACTURED
BY SIMPSON

** HARDWARE MANUFACTURED
BY CLEVELAND

DAVENPORT ELEVATION "A", "M"- FL

GARAGE : LEFT

DESIGNER: WCA
CHECKER: MIKE

SCALE: 1/8" = 1'-0"
DATE: 10/18/2007

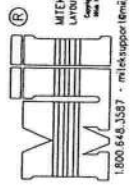
LOADING-FBC2004/TPI2002

TC LIVE	16.00	SNOW LOAD	0.00
TC DEAD	7.00	LUMBER DOL	1.25
BC LIVE	10.00	PLATE DOL	1.25
BC DEAD	10.00	WIND	125
TOTAL	43.00	SPACING	2'-0"

COPYRIGHT © 2007 MARONDA HOMES

Maronda Homes

(407) 321-0064 4005 MARONDA WAY SANFORD, FLORIDA



1.800.646.3387 • mitek@itek.com



Trenco

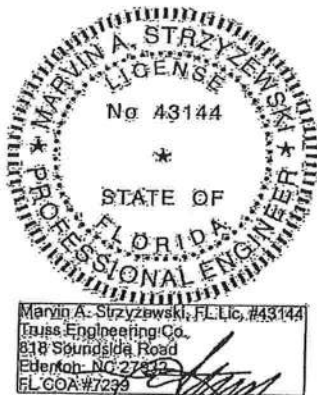
818 Soundside Rd
Edenton, NC 27932

Re: DAVENPORT
DAVENPORT_125

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Pages or sheets covered by this seal: E4513785 thru E4513864

My license renewal date for the state of Florida is February 28, 2009.



November 13, 2007

Strzyzewski, Marvin

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.
Engineering services provided by Truss Engineering Company.

RE: DAVENPORT - DAVENPORT_125

Trenco

818 Soundside Rd
Edenton, NC 27932

Site Information:

Project Customer: Maronda Homes Project Name: Maronda Homes
Lot/Block: Sanford Subdivision: Sanford
Address: Sanford
City: Sanford State: Florida

Name Address and License # of Structural Engineer of Record, if there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 7.0
Wind Code: ASCE 7-02 Wind Speed: 125 mph Floor Load: N/A psf
Roof Load: 43.0 psf

This package includes 1 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date
1	E4606947	BGRD1	1/10/08

The truss drawing(s) referenced above have been prepared by
TRENCO under my direct supervision based on the parameters
provided by Maronda Homes-Pittsburgh, PA.

Truss Design Engineer's Name: Strzyzewski, Marvin
My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of
professional engineering responsibility solely for the truss
components shown. The suitability and use of this component
for any particular building is the responsibility of the building
designer, per ANSI/TPI-1 Chapter 2.



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

January 10, 2008

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

- ⚠ The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

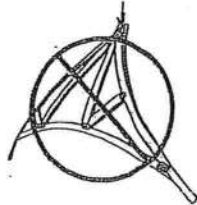
El resultado de un manejo, levantamiento, instalación, restricción y arrioste incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

- ⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

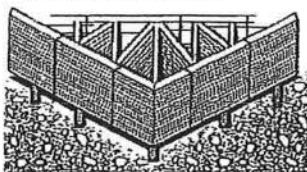
HANDLING — MANEJO

- ⚠ Avoid lateral bending. — Evite la flexión lateral.



- ⚠ The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



- ✓ If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

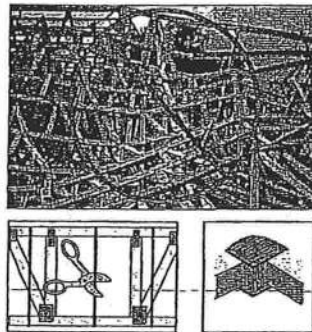
Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostres de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenamiento de los trusses en área de trabajo.

NOTAS GENERALES

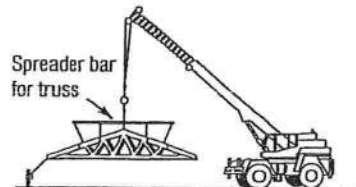
Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arrioste diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arrioste temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arrioste de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arrioste Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.



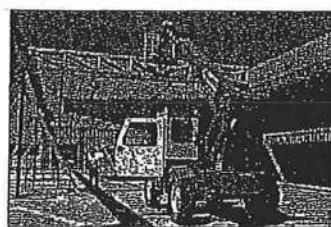
- ⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



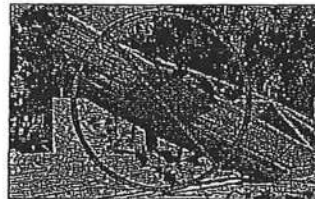
- ✓ Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



- ⚠ Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



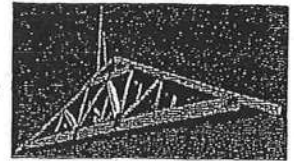
- ⚠ Do not store on uneven ground.

No almacene en tierra desigual.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

- ⚠ Warning! Don't overload the crane.
¡Advertencia! No sobrecarga la grúa!
- ⚠ Never use banding alone to lift a bundle.
Do not lift a group of individually banded bundles.
Nunca use sólo los empaques para levantar un paquete.
No levante un grupo de empaques individuales.



- ✓ A single lift point may be used for bundles with trusses up to 45'.
- Two lift points may be used for bundles with trusses up to 60'.
- Use at least 3 lift points for bundles with trusses greater than 60'.

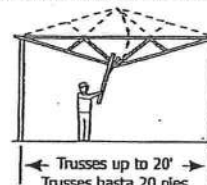
Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.
Puede usar dos puntos de levantar para paquetes más de 60 pies.
Use por lo menos tres puntos de levantar para paquetes más de 60 pies.

- ⚠ Warning! Do not over load supporting structure with truss bundle.
¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

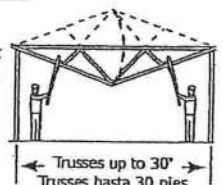
- ✓ Place truss bundles in stable position.
Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

- ✓ Trusses 20' or less, support at peak.
Levante del pico los trusses de 20 pies o menos.



- ✓ Trusses 30' or less, support at quarter points.
Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALES

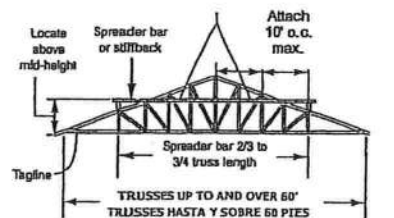
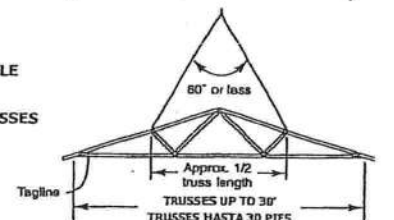
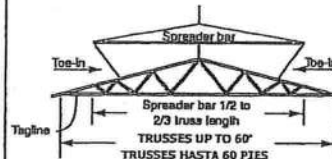
- ✓ Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalada y el truss está asegurado en los soportes.

- ⚠ Warning! Using a single pick-point at the peak can damage the truss.
¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



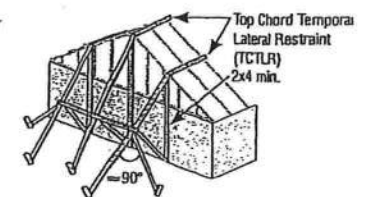
TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

- ⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

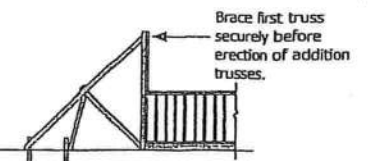
Vea el resumen BCSI B2 - Restricción/Arrioste Temporal y Instalación de los Trusses para más información.

- ✓ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).



- ⚠ Do not walk on unbraced trusses.
No camine en trusses sueltos.



Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

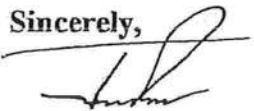
From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

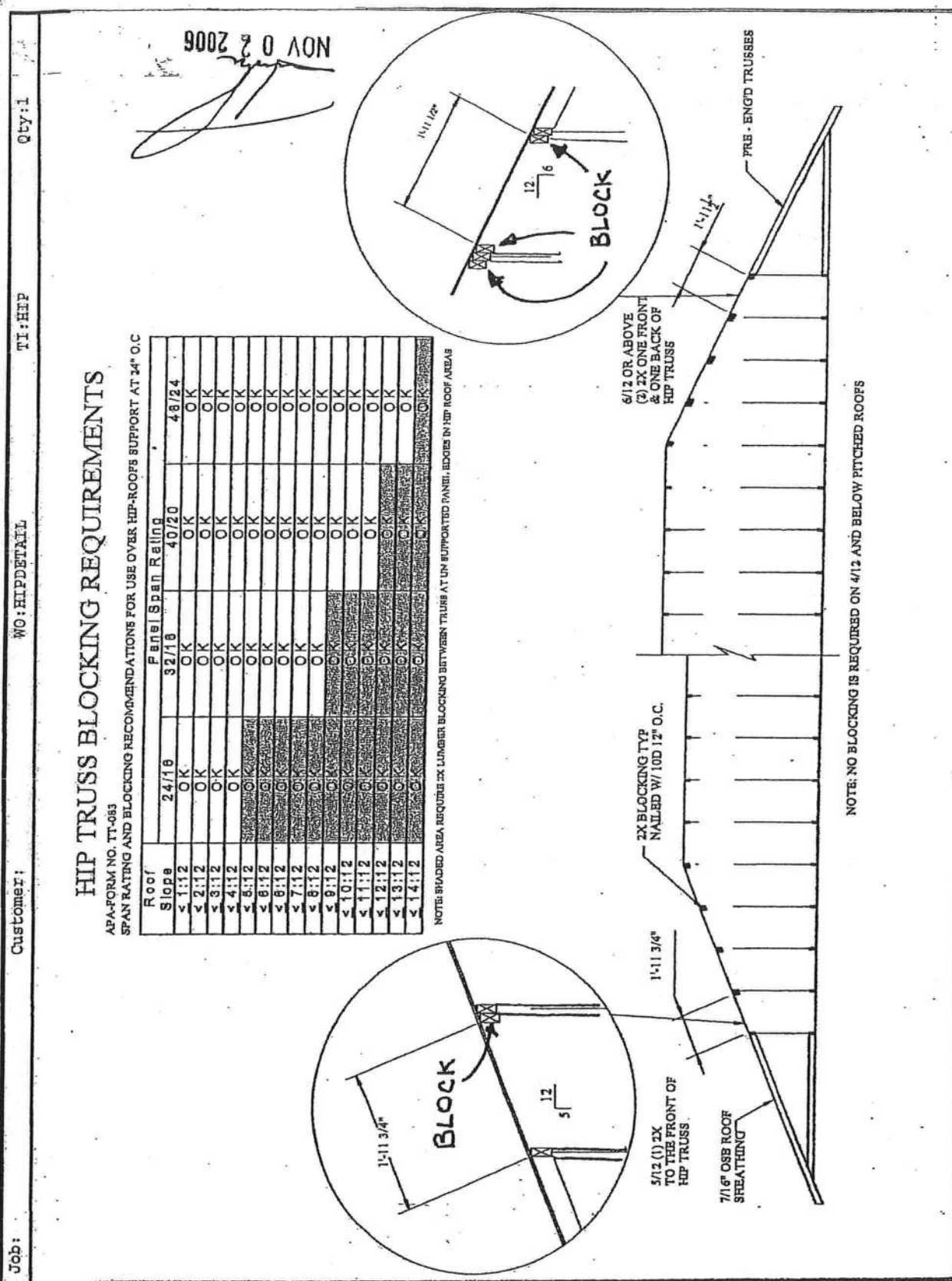
Sincerely,



Tomas Ponce, P.E.

Date: 11/1/06

Maronda Systems 4005 MARONDA WAY Sanford, FL 32711 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 1005 VANNESSA DR. OVIEDO FL 32766 Design: Matrix Analysis	WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO BRACING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wheel bracing, portal bracing or similar bracing which is a part of the building design and which must be provided by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to provide lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the second floor may be required. (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 383 D'Oenchio Drive, Madison, Wisconsin 53719). Profile Path: C:\TEE-LOK\Work\Jobs\MARONDA SYSTEMS\VT.prx <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 30%;">Eng Job:</td> <td>WO: VALLEY SET</td> </tr> <tr> <td>Dwg:</td> <td>TI: V</td> </tr> <tr> <td>Dsgnr:</td> <td>7/27/2005</td> </tr> <tr> <td style="text-align: center;">Chk:</td> <td></td> </tr> <tr> <td>TC Live</td> <td>15.0 psf</td> </tr> <tr> <td>TC Dead</td> <td>7.0 psf</td> </tr> <tr> <td>BC Live</td> <td>10.0 psf</td> </tr> <tr> <td>BC Dead</td> <td>2.0 psf</td> </tr> <tr> <td>TOTAL</td> <td>35.0 psf</td> </tr> </table>	Eng Job:	WO: VALLEY SET	Dwg:	TI: V	Dsgnr:	7/27/2005	Chk:		TC Live	15.0 psf	TC Dead	7.0 psf	BC Live	10.0 psf	BC Dead	2.0 psf	TOTAL	35.0 psf
Eng Job:	WO: VALLEY SET																		
Dwg:	TI: V																		
Dsgnr:	7/27/2005																		
Chk:																			
TC Live	15.0 psf																		
TC Dead	7.0 psf																		
BC Live	10.0 psf																		
BC Dead	2.0 psf																		
TOTAL	35.0 psf																		



DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and NDS-90 and shall be incorporated as part of the building design by the Building Designer. When reviewed for approval by the Building Designer, the Building Designer shall be checked to be sure that the design is in accordance with the local building codes, local climate record for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0\"/>

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over bunnies, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with joints fully embedded and shall be sym. about the joint unless otherwise shown. A 3/4\"/>

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and buckling. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Precautionary devices shall be sought and used. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No live loads shall be applied to the weight of the trusses shall be applied to trusses until after all bracing and erection is completed.

Maronda Systems 4005 MARONDA WAY Sanford, FL 32771 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 367 Medallion Pl Chuluota, FL 32766		WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. ERECTING WARNING: Bracing shown on this drawing is not erection bracing. Erection bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Erection bracing is to be provided by members only to reduce bracing length. Provision must be made to ensure that erection bracing is installed at the locations determined by the building designer. Additional bracing of the overall truss system may be required. (See HHS-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plus Institute, 177 E. Keweenaw St., D'Oroville Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Seaside Rd., Edenton, NC 27932	
Customer: WO: HIPDETAIL Job: TI: HIP Qty: 1		Eng Job: WO: HIPDETAIL Dwg: TI: HIP Design: 11/2/2006	
APR-2006 NOV 0 2 2006		Chk: Lbr DF: 1.25 Plt DF: 1.25 O.C.: 2-0-0 TPI-02/FBC-04 Code: FLA v4.7.32-0	
SPAN RATING AND BLOCKING RECOMMENDATIONS FOR USE OVER HIP-ROOFS SUPPORT AT 24\"/> 		TOTAL 43.0 psf	

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513785
DAVENPORT	81	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:17:33 2007 Page 1

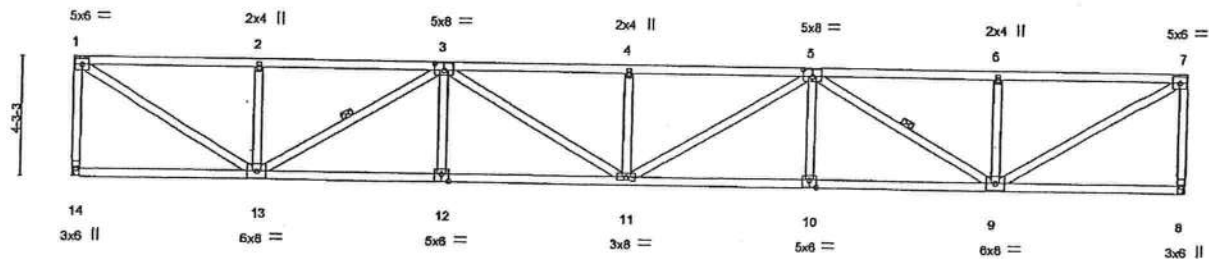
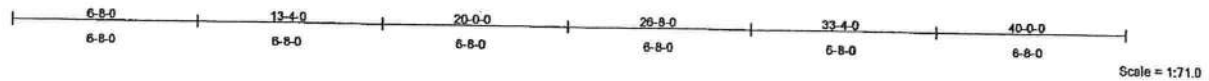


Plate Offsets (X,Y): [3:0-4-0-0-3-0], [5:0-4-0-0-3-0], [10:0-3-0-0-3-0], [12:0-3-0-0-3-0]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 16.0	Plates Increase	1.25	TC 0.70	Vert(LL)	-0.40 11-12	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.83	Vert(TL)	-0.80 11-12	>597
BCLL 10.0	Rep Stress Incr	YES	WB 0.86	Horz(TL)	0.18 8	n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)			
						Weight: 223 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-2 oc purfins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-0 oc bracing.
WEBS 1 Row at midpt 3-13, 5-9

REACTIONS (lb/size) 14=1707/Mechanical, 8=1707/Mechanical
Max Uplift 14=418(LC 4), 8=418(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-14=-1577/646, 1-2=-2415/872, 2-3=-2415/872, 3-4=-4336/1565, 4-5=-4336/1565, 5-6=-2415/872, 6-7=-2415/872, 7-8=-1577/646
BOT CHORD 13-14=-25/68, 12-13=-1393/3856, 11-12=-1393/3856, 10-11=-1393/3856, 9-10=-1393/3856, 8-9=-25/68
WEBS 1-13=-992/2749, 2-13=-307/297, 3-13=-1677/606, 3-12=0/266, 3-11=-201/559, 4-11=-308/288, 5-11=-201/559, 5-10=0/266, 5-9=-1677/606, 6-9=-307/297, 7-9=-992/2749

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 418 lb uplift at joint 14 and 418 lb uplift at joint 8.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-87 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncillo Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4606947
DAVENPORT	BGRD1	SPECIAL	1	2	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Jan 10 09:25:26 2008 Page 1

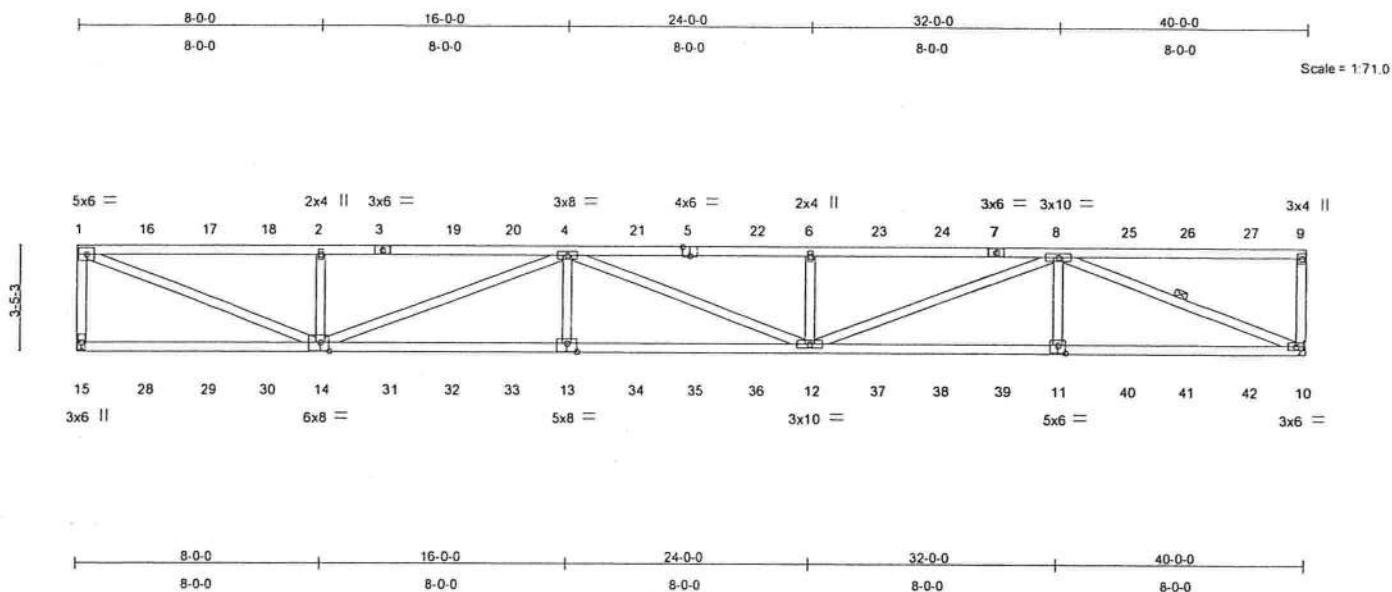


Plate Offsets (X,Y): [5:0-3:0:Edge], [11:0-3:0:0-3:0], [13:0-4:0:0-3:4], [14:0-3:8:0-3:4]

LOADING (psf)	SPACING	1-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.83	Vert(LL)	0.75 12-13	>635	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.82	Vert(TL)	-1.10 12-13	>432	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.87	Horz(TL)	0.18 10	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 410 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-14 oc purtins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-2-0 oc bracing.
WEBS 1 Row at midpt 8-10

REACTIONS (lb/size) 15=3030/Mechanical, 10=2848/Mechanical
Max Uplift 15=1833(LC 9), 10=1697(LC 9)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-15=-2676/1670, 1-16=-6079/3645, 16-17=-6079/3645, 17-18=-6079/3645, 2-18=-6079/3645, 2-3=-6079/3645, 3-19=-6079/3645, 19-20=-6079/3645, 4-20=-6079/3645, 4-21=-9072/5440, 5-21=-9072/5440, 5-22=-9072/5440, 6-22=-9072/5440, 6-23=-9072/5440, 23-24=-9072/5440, 7-24=-9072/5440, 7-8=-9072/5440, 8-25=-228/137, 25-26=-228/137, 26-27=-228/137, 9-27=-228/137, 9-10=-250/195
BOT CHORD 15-28=-137/229, 28-29=-137/229, 29-30=-137/229, 14-30=-137/229, 14-31=-5446/9081, 31-32=-5446/9081, 32-33=-5446/9081, 13-33=-5446/9081, 13-34=-5446/9081, 34-35=-5446/9081, 35-36=-5446/9081, 12-36=-5446/9081, 12-37=-3644/6078, 37-38=-3644/6078, 38-39=-3644/6078, 11-39=-3644/6078, 11-40=-3644/6078, 40-41=-3644/6078, 41-42=-3644/6078, 10-42=-3644/6078
WEBS 1-14=-3778/6301, 2-14=-569/466, 4-14=-3225/1934, 4-13=-243/603, 4-12=-9/6, 6-12=-574/463, 8-12=-1930/3216, 8-11=-235/595, 8-10=-6301/3777

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1833 lb uplift at joint 15 and 1697 lb uplift at joint 10.

Continued on page 2



January 10, 2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Ondra Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4606947
DAVENPORT	BGRD1	SPECIAL	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Jan 10 09:25:26 2008 Page 2

NOTES

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 98 lb down and 83 lb up at 0-1-12, 98 lb down and 83 lb up at 2-2-4, 98 lb down and 83 lb up at 4-2-4, 98 lb down and 83 lb up at 6-2-4, 98 lb down and 83 lb up at 8-2-4, 98 lb down and 83 lb up at 10-2-4, 98 lb down and 83 lb up at 12-2-4, 98 lb down and 83 lb up at 14-2-4, 98 lb down and 83 lb up at 16-2-4, 98 lb down and 83 lb up at 18-2-4, 98 lb down and 83 lb up at 20-2-4, 98 lb down and 83 lb up at 22-2-4, 98 lb down and 83 lb up at 24-2-4, 98 lb down and 83 lb up at 26-2-4, 98 lb down and 83 lb up at 28-2-4, 98 lb down and 83 lb up at 30-2-4, 98 lb down and 83 lb up at 32-2-4, 98 lb down and 83 lb up at 34-2-4, and 98 lb down and 83 lb up at 36-2-4, and 98 lb down and 83 lb up at 38-2-4 on top chord, and 110 lb down and 72 lb up at 0-1-12, 110 lb down and 72 lb up at 2-2-4, 110 lb down and 72 lb up at 4-2-4, 110 lb down and 72 lb up at 6-2-4, 110 lb down and 72 lb up at 8-2-4, 110 lb down and 72 lb up at 10-2-4, 110 lb down and 72 lb up at 12-2-4, 110 lb down and 72 lb up at 14-2-4, 110 lb down and 72 lb up at 16-2-4, 110 lb down and 72 lb up at 18-2-4, 110 lb down and 72 lb up at 20-2-4, 110 lb down and 72 lb up at 22-2-4, 110 lb down and 72 lb up at 24-2-4, 110 lb down and 72 lb up at 26-2-4, 110 lb down and 72 lb up at 28-2-4, 110 lb down and 72 lb up at 30-2-4, 110 lb down and 72 lb up at 32-2-4, 110 lb down and 72 lb up at 34-2-4, and 110 lb down and 72 lb up at 36-2-4, and 110 lb down and 72 lb up at 38-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-9=-23, 10-15=-20

Concentrated Loads (lb)

Vert: 15=-110(F) 1=-98(F) 3=-98(F) 14=-110(F) 2=-98(F) 4=-98(F) 13=-110(F) 6=-98(F) 12=-110(F) 11=-110(F) 8=-98(F) 5=-98(F) 7=-98(F) 16=-98(F) 17=-98(F) 18=-98(F) 19=-98(F) 20=-98(F) 21=-98(F) 22=-98(F) 23=-98(F) 24=-98(F) 25=-98(F) 26=-98(F) 27=-98(F) 28=-110(F) 29=-110(F) 30=-110(F) 31=-110(F) 32=-110(F) 33=-110(F) 34=-110(F) 35=-110(F) 36=-110(F) 37=-110(F) 38=-110(F) 39=-110(F) 40=-110(F) 41=-110(F) 42=-110(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513787
DAVENPORT	G1	GABLE	1	1	Job Reference (optional)	
Maronde Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:35 2007 Page 1

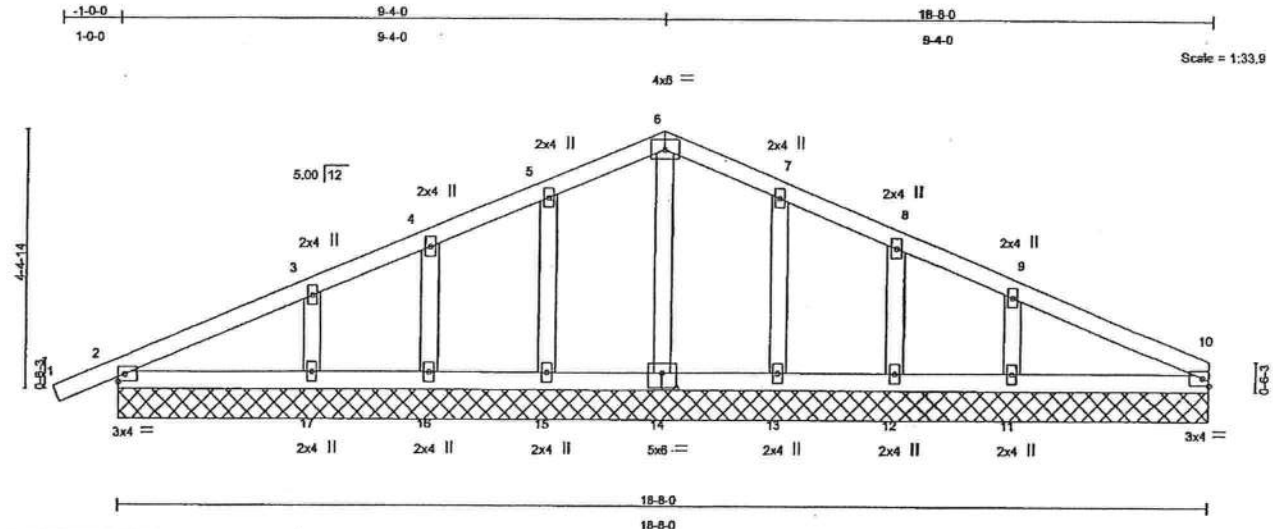


Plate Offsets (X,Y): [14.0-3.0-0.3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.10	Vert(LL)	0.00	1	n/r	120	MT20
TCOL 7.0	Lumber Increase	1.25	BC 0.09	Vert(TL)	0.01	1	n/r	120	244/190
BCLL 10.0	Rep Stress Incr	NO	WB 0.03	Horz(TL)	0.00	10	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 85 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 10=121/18-8-0, 2=179/18-8-0, 14=160/18-8-0, 15=183/18-8-0, 16=132/18-8-0, 17=277/18-8-0, 13=185/18-8-0, 12=125/18-8-0, 11=293/18-8-0
Max Horz 2=93(LC 6)
Max Uplift 10=32(LC 7), 2=115(LC 6), 15=102(LC 6), 16=82(LC 6), 17=151(LC 6), 13=104(LC 7), 12=71(LC 7), 11=176(LC 7)
Max Grav 10=121(LC 1), 2=179(LC 1), 14=160(LC 1), 15=187(LC 10), 16=132(LC 1), 17=277(LC 10), 13=188(LC 11), 12=125(LC 1), 11=293(LC 11)

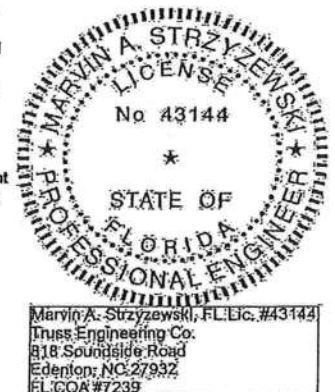
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/10, 2-3=-76/34, 3-4=-32/89, 4-5=-24/122, 5-6=-26/179, 6-7=-26/180, 7-8=-24/120, 8-9=-33/70, 9-10=-46/33
BOT CHORD 2-17=0/83, 16-17=0/83, 15-16=0/83, 14-15=0/83, 13-14=0/83, 12-13=0/83, 11-12=0/83, 10-11=0/83
WEBS 6-14=-83/0, 5-15=-101/137, 4-16=-72/119, 3-17=-145/197, 7-13=-101/139, 8-12=-69/112, 9-11=-153/213

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCOL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2'-0" oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 10, 115 lb uplift at joint 2, 102 lb uplift at joint 15, 82 lb uplift at joint 16, 151 lb uplift at joint 17, 104 lb uplift at joint 13, 71 lb uplift at joint 12 and 176 lb uplift at joint 11.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

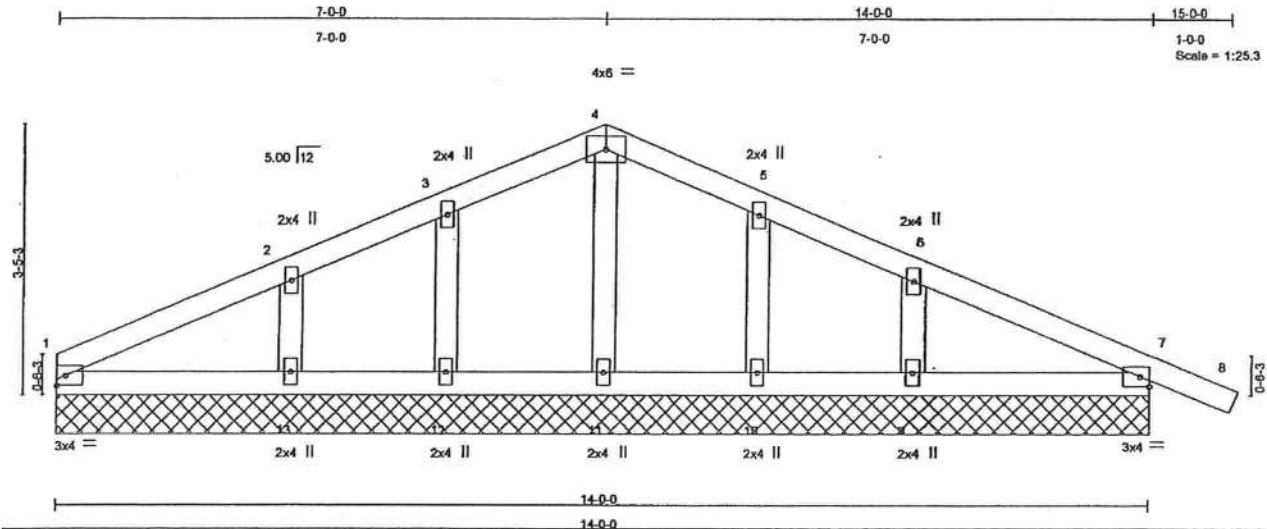
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE NIS-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Tracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, 058-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
AMTEK AMBIO

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513788
DAVENPORT	G2	GABLE	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:17:37 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.10	Vert(LL)	0.00	8	n/r	120	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.07	Vert(TL)	0.00	8	n/r	120	244/190
BCLL 10.0	Rep Stress Incr	NO	WB 0.03	Horz(TL)	0.00	7	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 50 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS (lb/size) 1=109/14-0-0, 7=169/14-0-0, 11=180/14-0-0, 12=137/14-0-0, 13=266/14-0-0, 10=143/14-0-0, 9=249/14-0-0

Max Horz 1=-76(LC 7)

Max Uplift 1=-39(LC 7), 7=-138(LC 7), 12=-78(LC 6), 13=-161(LC 6), 10=-88(LC 7), 9=-131(LC 7)

Max Grav 1=109(LC 1), 7=169(LC 1), 11=180(LC 1), 12=141(LC 10), 13=266(LC 1), 10=147(LC 11), 9=249(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-48/42, 2-3=-32/88, 3-4=-28/151, 4-5=-26/152, 5-6=-31/98, 6-7=-37/26, 7-8=0/10

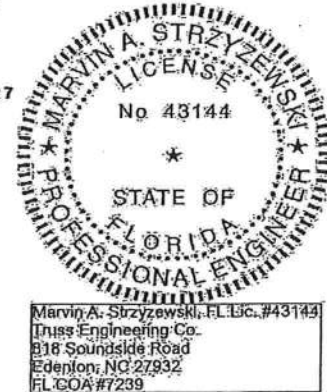
BOT CHORD 1-13=0/75, 12-13=0/75, 11-12=0/75, 10-11=0/75, 9-10=0/75, 7-9=0/75

WEBS 4-11=-92/23, 3-12=-79/122, 2-13=-138/210, 5-10=-82/129, 6-9=-131/193

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP1 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2'-0-0 oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1, 138 lb uplift at joint 7, 78 lb uplift at joint 12, 161 lb uplift at joint 13, 88 lb uplift at joint 10 and 131 lb uplift at joint 9.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
318 Soundside Road
Edenboro, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Edenboro, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513789
DAVENPORT	GRD1	COMMON	2	3	Job Reference (optional)	
Meranda Homes Inc., Sanford, FL						

7.010 s Oct 18 2007 MTEK Industries, Inc. Tue Nov 13 08:17:37 2007 Page 1

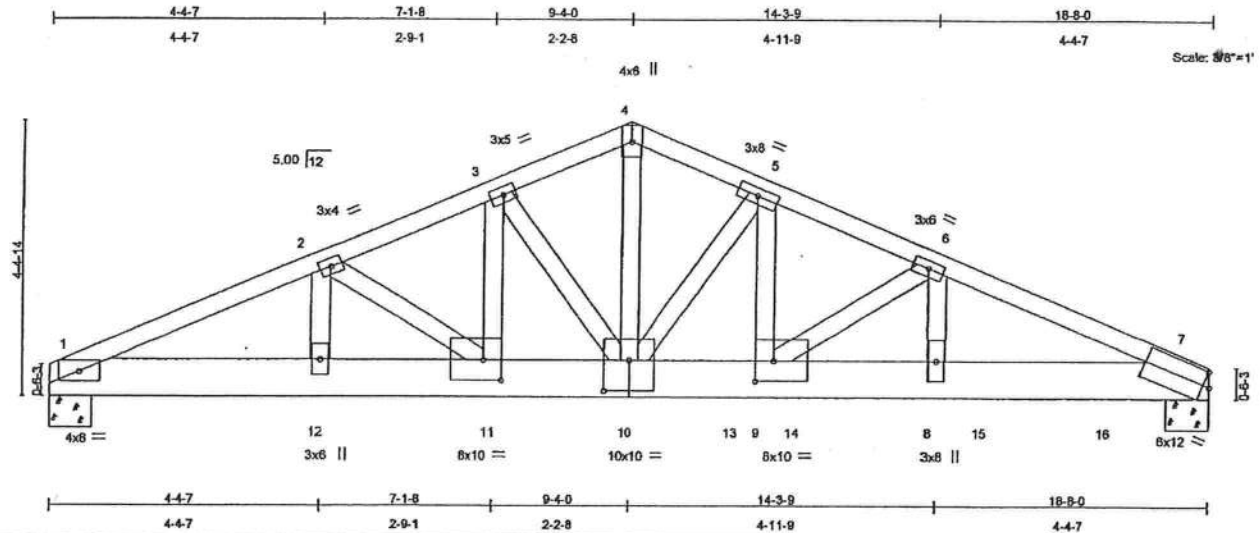


Plate Offsets (X,Y): [7:Edge,0-2-14], [9:0-3-8,0-4-0], [10:0-5-0,0-6-0], [11:0-3-8,0-4-0]

LOADING (psf)	SPACING	1-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.57	Vert(LL)	-0.16	9	>999	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.87	Vert(TL)	-0.29	9	>736	180	244/190
BCLL 10.0	Rep Stress Incr	NO	WB 0.57	Horz(TL)	0.06	7	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 371 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.2 *Except*
3-11 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=6166/0-8-6, 7=10143/0-8-6
Max Horz 1=-30(LC 6)
Max Uplift 1=-2521(LC 5), 7=-3632(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

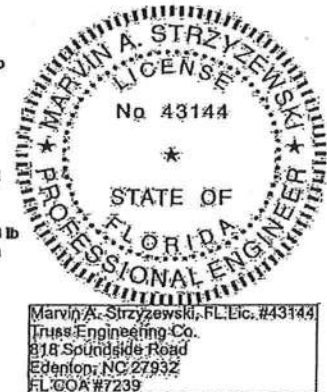
TOP CHORD 1-2=-13584/5585, 2-3=-14213/5864, 3-4=-12490/4958, 4-5=-12494/4957, 5-6=-15481/5858, 6-7=-19008/6918
BOT CHORD 1-12=-5069/12296, 11-12=-5069/12296, 10-11=-5386/13101, 10-13=-5380/14279, 9-13=-5360/14279,
8-14=-6252/17227, 8-14=-6252/17227, 8-15=-6252/17227, 15-16=-6252/17227, 7-16=-6252/17227
WEBS 2-12=-375/177, 4-10=-3664/9255, 5-10=-4577/1407, 6-8=-1210/3974, 3-11=-1521/2862, 2-11=-418/1024,
3-10=-2579/1412, 5-9=-1506/4936, 6-9=-3574/1088

NOTES

- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 4 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2521 lb uplift at joint 1 and 3632 lb uplift at joint 7.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 4002 lb down and 2065 lb up at 7-1-8, 1826 lb down and 789 lb up at 9-0-12, 1910 lb down and 622 lb up at 11-0-12, 1932 lb down and 608 lb up at 12-0-12, 1943 lb down and 627 lb up at 14-0-12, and 1945 lb down and 637 lb up at 15-0-12, and 1976 lb down and 656 lb up at 17-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2



Marvin A. Strzyzewski, FL E.C. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL GOA #7239

November 13, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult: ANSI/TPI1 Quality Criteria, D58-89 and ECSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513789
DAVENPORT	GRD1	COMMON	2	3	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:37 2007 Page 2

LOAD CASE(S) Standard

1) Regular; Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-23, 4-7=-23, 1-7=-20

Concentrated Loads (lb)

Vert: 10=-1826(F) 8=-1943(F) 11=-4002(F) 13=-1910(F) 14=-1932(F) 15=-1945(F) 16=-1976(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ICS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

TRENCO
 ENGINEERING BY
 A Mitek Alliance

818 Soundside Road
 Eden, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513790
DAVENPORT	GRD2	COMMON	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MitTek Industries, Inc. Tue Nov 13 08:17:38 2007 Page 1

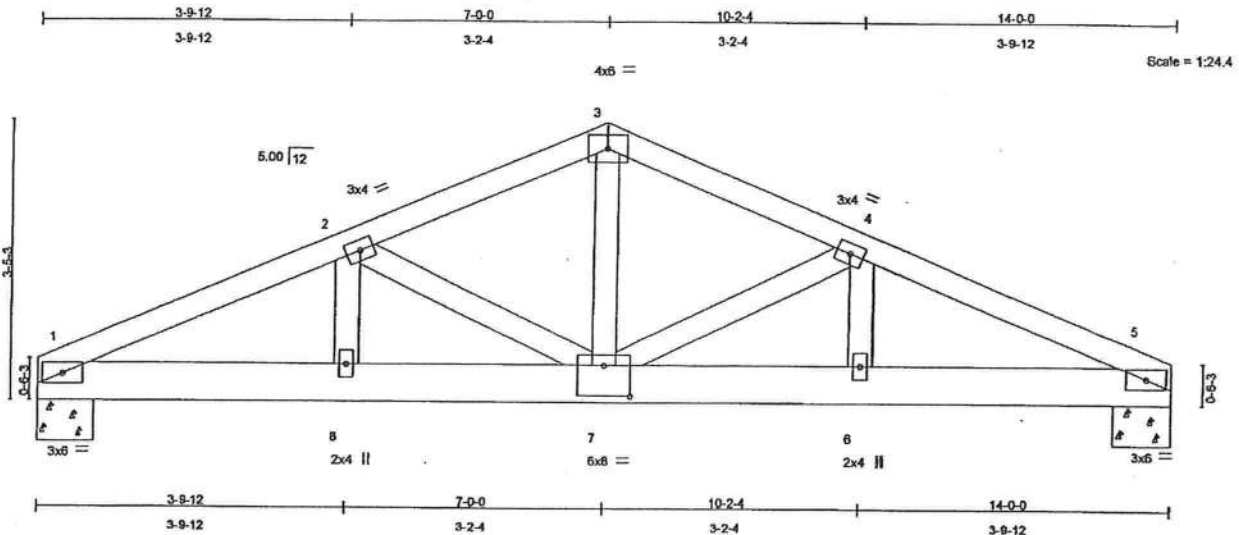


Plate Offsets (X,Y): [7-0 4-0 0-4-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.16	Vert(LL)	-0.06	7	>999	240	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.41	Vert(TL)	-0.11	7	>999	180	
BCLL 10.0	Rep Stress Incr	NO	WB 0.34	Horz(TL)	0.03	5	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						

Weight: 147 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=2430/0-8-6, 5=2430/0-8-6
Max Horz 1=48(LC 5)
Max Uplift 1=807(LC 5), 5=807(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=5038/1699, 2-3=5098/1792, 3-4=5098/1792, 4-5=5038/1700
BOT CHORD 1-8=1528/4479, 7-8=1528/4479, 6-7=1481/4479, 5-6=1481/4479
WEBS 2-8=0/72, 2-7=154/360, 3-7=1276/3707, 4-7=156/360, 4-8=0/73

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-3-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 807 lb uplift at joint 1 and 807 lb uplift at joint 5.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3714 lb down and 1394 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=48, 3-5=48, 1-5=40
Concentrated Loads (lb)
Vert: 7=3714(F)



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MitTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MitTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513802
DAVENPORT	HIS	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:47 2007 Page 1

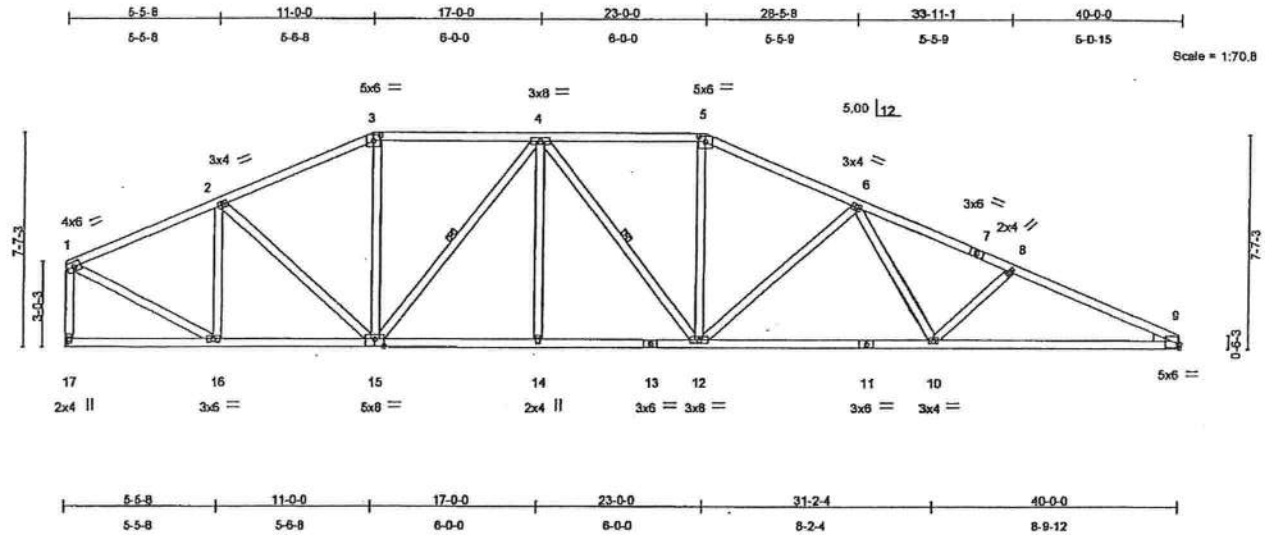


Plate Offsets (X,Y): [3.0-3.0-0-2-4] [5.0-3.0-0-2-4] [9.0-0-1.0-1-11] [15.0-4.0-0-3.0]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 16.0	Plates Increase	1.25	TC 0.59	Vert(LL)	-0.26 10-12	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.91	Vert(TL)	-0.54 10-12	>889
BCLL 10.0	Rep Stress Incr	YES	WB 0.56	Horz(TL)	-0.15 17	n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)			
						Weight: 236 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2
 WEDGE
 Right 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-8-8 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 5-9-8 oc bracing.
 WEBS 1 Row at midpt 4-12, 4-15

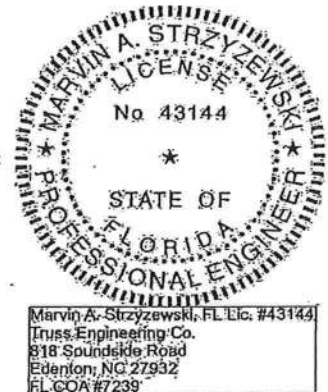
REACTIONS (lb/size) 9=1711/Mechanical, 17=1711/Mechanical
 Max Horz 9=184(LC 7)
 Max Uplift 9=308(LC 7), 17=281(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 5-8=2548/946, 6-7=3318/1112, 7-8=3371/1095, 8-9=3545/1185, 3-4=1895/782, 4-5=2323/922, 1-2=1799/620, 2-3=2091/795, 1-17=1603/584
 BOT CHORD 18-17=287/1, 15-16=515/1617, 14-15=718/2305, 13-14=718/2305, 12-13=718/2305, 11-12=934/2788, 10-11=934/2788, 9-10=1137/3198
 WEBS 8-10=191/234, 6-10=67/576, 6-12=648/304, 5-12=125/690, 4-12=129/87, 4-14=0/218, 4-15=726/235, 3-15=87/521, 2-15=72/431, 2-16=594/329, 1-16=550/1744

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 306 lb uplift at joint 9 and 281 lb uplift at joint 17.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCES PAGE M1-1473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to truss stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513603
DAVENPORT	H2S	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 18 2007 MITek Industries, Inc. Tue Nov 13 08:17:48 2007 Page 1

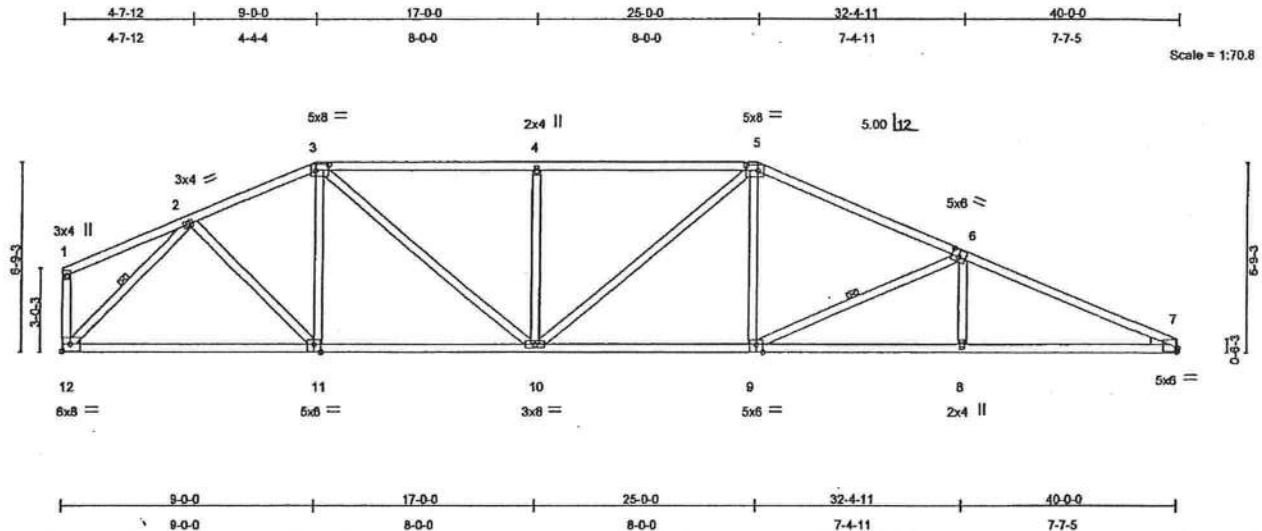


Plate Offsets (X,Y): [3-0-5-12-0-2-8] [5-0-5-4-0-2-4] [6-0-3-0-0-3-0] [7-0-0-1-0-1-1] [8-0-3-0-0-3-4] [11-0-3-0-0-3-4]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	L/d
TCLL 16.0	Plates Increase	1.25	TC 0.89	Vert(LL)	-0.26 9-10	>999 240
TCDL 7.0	Lumber Increase	1.25	BC 0.93	Vert(TL)	-0.54 9-10	>880 180
BCLL 10.0	Rep Stress Incr	YES	WB 0.56	Horz(TL)	-0.16 12	n/a n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)			
						Weight: 218 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 6-9, 2-12

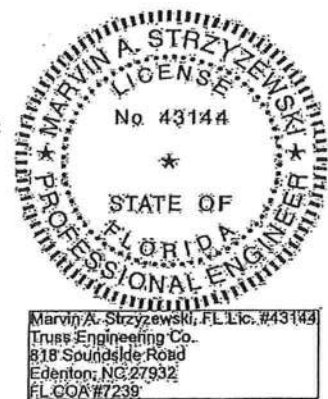
REACTIONS (lb/size) 7=1711/Mechanical, 12=1711/Mechanical
Max Horz 7=171(LC 7)
Max Uplift 7=290(LC 7), 12=306(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 5-6=2797/1008, 6-7=3576/1172, 3-4=2636/1039, 4-5=2636/1039, 1-2=-187/52, 2-3=2079/777, 1-12=-177/100
BOT CHORD 11-12=498/1421, 10-11=588/1892, 9-10=819/2534, 8-9=1112/3222, 7-8=1112/3222
WEBS 6-8=0/334, 6-9=773/323, 5-9=37/622, 5-10=72/290, 4-10=392/356, 3-10=360/1012, 3-11=160/201, 2-11=134/717, 2-12=1873/721

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 290 lb uplift at joint 7 and 306 lb uplift at joint 12.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MYTEK REFERENCE PAGE NO. 7473 BEFORE USE.
Design valid for use only with Mytek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCSIT Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513804
DAVENPORT	HIS	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 18 2007 MITek Industries, Inc. Tue Nov 13 08:17:48 2007 Page 1

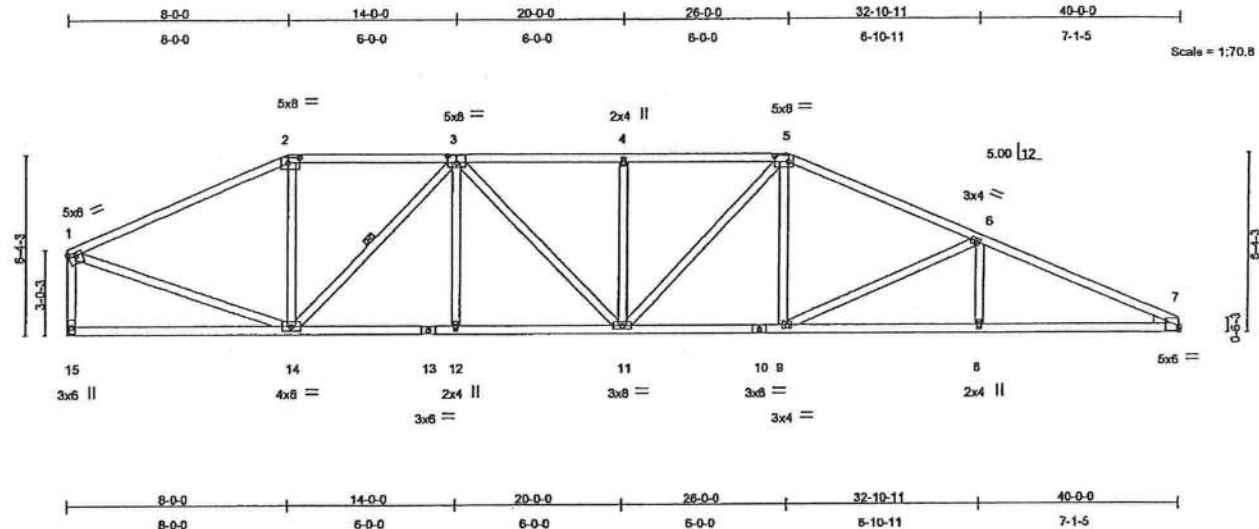


Plate Offsets (X,Y): [1:0-3-0-0-1-8], [2:0-5-0-0-2-4], [3:0-4-0-0-3-0], [5:0-5-12-0-2-8], [7:0-0-1-0-1-11]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 18.0	Plates Increase	1.25	TC 0.58	Ver(LL)	-0.24	8-9 >999
TCDL 7.0	Lumber Increase	1.25	BC 0.89	Ver(TL)	-0.50	8-9 >962
BCLL 10.0	Rep Stress Incr	YES	WB 0.64	Horz(TL)	-0.16	15 n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)			
						Weight 224 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-8-13 oc bracing.
WEBS 1 Row at midpt 3-14

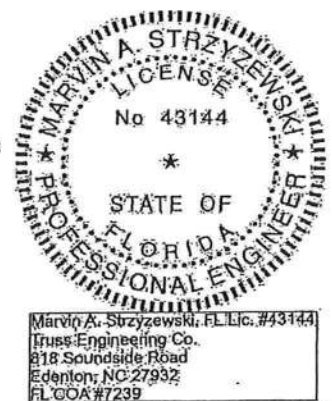
REACTIONS (lb/size) 7=1711/Mechanical, 15=1711/Mechanical
Max Horz 7=-165(LC 7)
Max Uplift 7=-285(LC 4), 15=-319(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 5-6=-2878/1039, 8-7=-3803/1188, 2-3=-1884/755, 3-4=-2849/1105, 4-5=-2849/1105, 1-2=-2079/747, 1-15=-1545/618
BOT CHORD 14-15=-83/161, 13-14=-860/2586, 12-13=-860/2586, 11-12=-860/2586, 10-11=-861/2611, 9-10=-861/2611, 8-9=-1132/3246, 7-8=-1132/3246
WEBS 6-8=0/318, 8-9=-714/298, 5-9=-44/548, 5-11=-128/467, 4-11=-288/255, 3-11=-129/401, 3-12=0/223, 3-14=-1102/372, 2-14=0/524, 1-14=-571/1791

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCFL=4.2psf; BCDL=8.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at Joint 7 and 319 lb uplift at Joint 15.

LOAD CASE(S) Standard



November 13, 2007

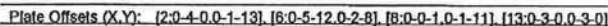
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE M1-7493 BEFORE USE.
Design valid for use only with MITTEK connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-87 and BC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

TRENCO
A MITTEK Alliance

818 Soundside Road
Edenton, NC 27932

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MiTek Industries, Inc. Tue Nov 13 08:17:49 2007 Page 1



LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Right: 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 2-11-12 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-8-12 oc bracing.
WEBS	1 Row at midpt 3-14

REACTIONS (lb/size) 8=1711/Mechanical, 15=1711/Mechanical
Max Horz 8=152(LC 7)
Max Uplift 8=303(LC 4), 15=344(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 6-7=307/41107, 7-8=3825/1212, 2-3=1729/692, 3-4=3325/1265, 4-5=3325/1265, 5-6=3325/1265, 1-2=1908/692, 1-15=1599/615

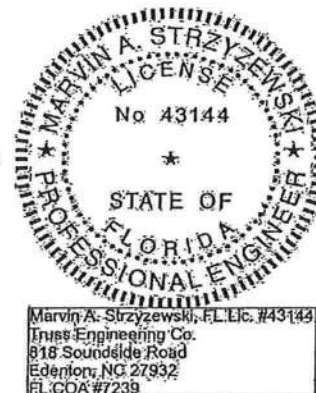
BÓT CHORD 14-16=28/72, 13-14=1023/2963, 12-13=1023/2963, 11-12=958/2806, 10-11=958/2806, 9-10=1160/3263, 8-9=1180/3263

WEBS 7-8=0/255, 7-10=-518/223, 8-10=-12/485, 8-12=-235/748, 5-12=-348/317, 3-12=-157/467, 3-13=0/292, 3-14=-1559/549, 2-14=0/456, 1-14=-608/1813

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.80. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at Joint 8 and 344 lb uplift at Joint 15.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7493 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Mitek design. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSII/TPI Quality Criteria, DSR-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 O'Connor Drive, Modesto, WI 53719.

ENGINEERING BY
TRENCO
A MILCO AFFILIATE

818 Soundside Road
Eden, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513008
DAVENPORT	HSS	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:50 2007 Page 1

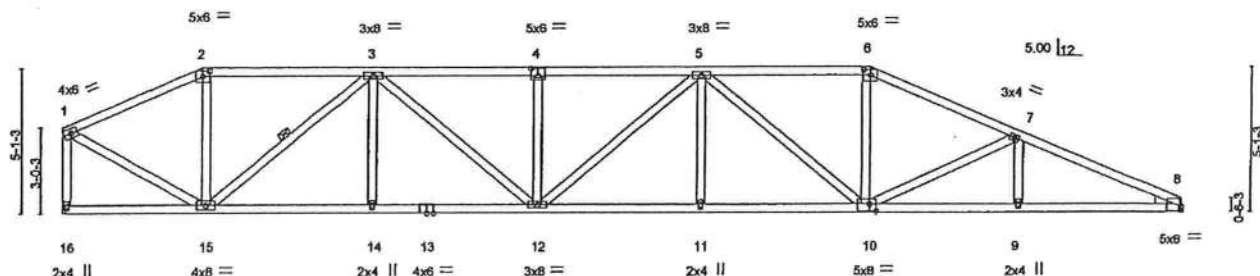
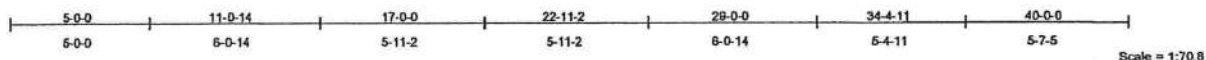


Plate Offsets (X,Y): [2-0-3-0-0-2-4], [4-0-3-0-0-3-0], [6-0-3-0-0-2-4], [8-0-0-1-0-1-11], [10-0-2-12-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/def	L/d	PLATES	GRIP
TCCL 16.0	Plates Increase	1.25	TC 0.36	Vert(LL)	-0.31	11-12	>999	240	MT20
TCCL 7.0	Lumber Increase	1.25	BC 0.78	Vert(TL)	-0.62	11-12	>773	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.78	Horz(TL)	-0.19	16	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						Weight: 222 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-8-12 oc bracing.
WEBS 1 Row at midpt 3-15

REACTIONS (lb/size) 8=1711/Mechanical, 16=1711/Mechanical
Max Horz 8=-146(LC 7)
Max Uplift 8=312(LC 4), 16=356(LC 5)

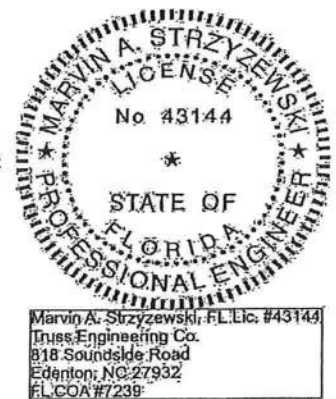
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 6-7=-3162/1137, 7-8=-3638/1224, 2-3=-1800/638, 3-4=-3504/1324, 4-5=-3504/1324, 5-6=-2878/1094, 1-2=-1759/642, 1-16=-1616/615
BOT CHORD 15-16=-20/52, 14-15=-1003/2871, 13-14=-1003/2871, 12-13=-1003/2871, 11-12=-1232/3515, 10-11=-1232/3515, 9-10=-1176/3271, 8-9=-1176/3271
WEBS 7-9=0/229, 7-10=-438/200, 8-10=-199/928, 5-10=-904/296, 5-11=0/241, 5-12=-29/59, 4-12=-269/251, 3-12=-290/827, 3-14=0/240, 3-15=-1682/582, 2-15=-10/451, 1-15=-597/1755

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 8 and 356 lb uplift at joint 16.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MIITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 563 D'Oroffo Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513807
DAVENPORT	H8	HIP	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 MTEK Industries, Inc. Tue Nov 13 08:17:50 2007 Page 1

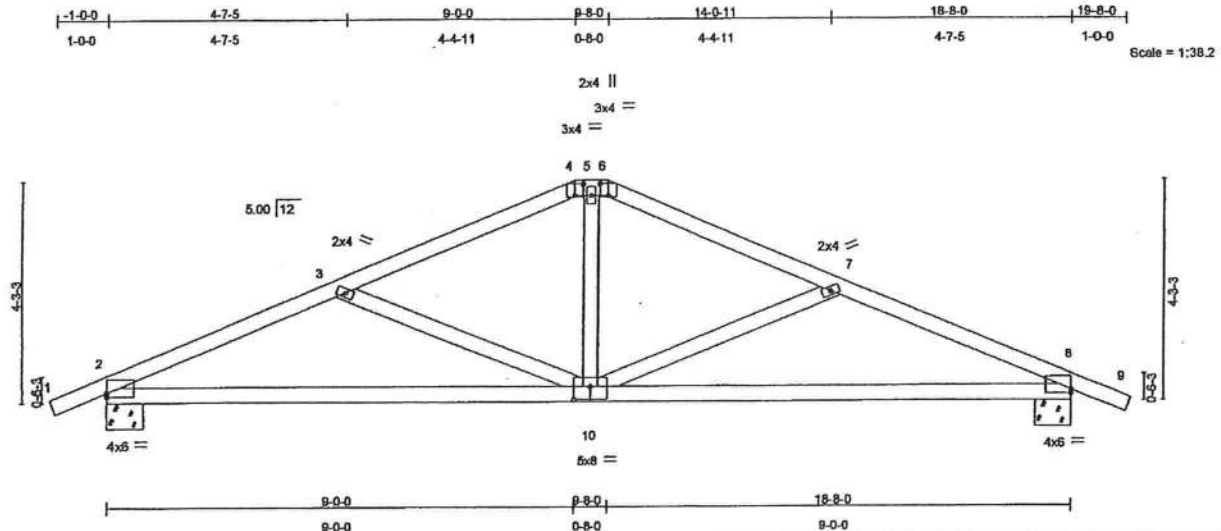


Plate Offsets (X,Y)		[2-0-0-1-0-0-15]		[4-0-2-0-0-2-11]		[6-0-2-0-0-2-11]		[8-0-0-1-0-0-15]		[10-0-4-0-0-3-0]	
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP		
TCLL 18.0	Plates Increase	1.25	TC 0.27	Vert(LL)	-0.13	8-10	>999	240	MT20	244/190	
TCDL 7.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.30	8-10	>720	180			
BCLL 10.0	Rep Stress Incr	YES	WB 0.18	Horz(TL)	0.03	8	n/a	n/a			
BCDL 10.0	Code FBC2004/TP12002		(Matrix)								
											Weight: 82 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

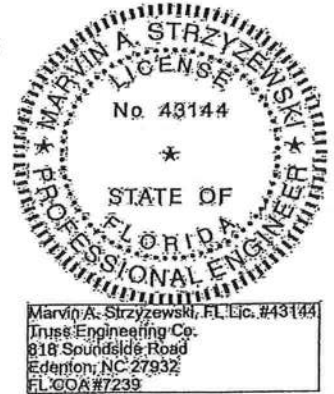
REACTIONS (lb/size) 2=838/0-8-8, 8=838/0-8-8
 Max Horz 2=71(LC 6)
 Max Uplift 2=227(LC 6), 8=227(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/10, 2-3=-1278/432, 3-4=-1059/317, 4-5=-937/332, 5-6=-937/332, 6-7=-1059/317, 7-8=-1278/432, 8-9=0/10
 BOT CHORD 2-10=-299/1133, 8-10=-299/1133
 WEBS 3-10=-244/191, 7-10=-244/191, 5-10=-28/558

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 2 and 227 lb uplift at joint 8.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M07473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/IFPI Quality Criteria, D58-89 and CSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A MITEK AFFILIATE

818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513812
DAVENPORT	HGRD1	HIP	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:54 2007 Page 1

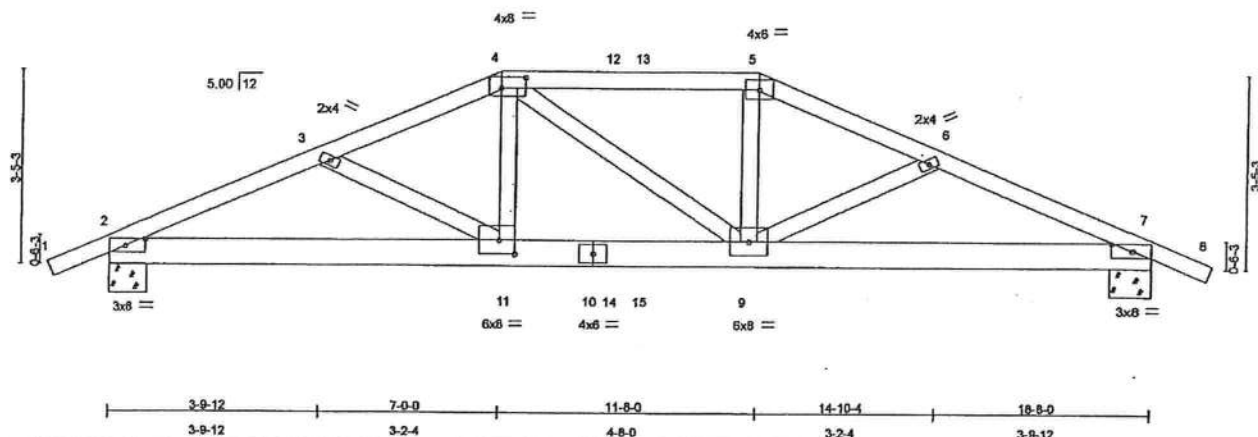
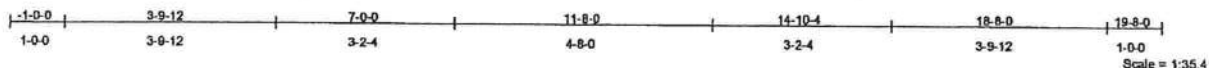


Plate Offsets (X,Y): [2:0-4-4-0-1-8], [4:0-5-4-0-2-4], [7:0-4-4-0-1-8], [11:0-3-8-0-3-0]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 16.0	Plates Increase	1.25	TC 0.44	Vert(LL)	0.13 9-11	>999	240
TCDL 7.0	Lumber Increase	1.25	BC 0.59	Vert(TL)	-0.22 9-11	>889	180
BCLL 10.0	Rep Stress Incr	NO	WB 0.17	Horz(TL)	0.06 7	n/a	n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)				
							Weight: 102 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-8 oc bracing.

REACTIONS (lb/size) 2=1807/0-8-6, 7=1807/0-8-6
Max Horz 2=81 (LC 7)
Max Uplift 2=855 (LC 7), 7=855 (LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/14, 2-3=3481/1661, 3-4=3522/1704, 4-12=3288/1623, 12-13=3288/1623, 5-13=3288/1623, 5-6=3519/1702, 6-7=3480/1680, 7-8=0/14
BOT CHORD 2-11=1478/3082, 10-11=1541/3291, 10-14=1541/3291, 14-15=1541/3291, 9-15=1541/3291, 7-9=1417/3081
WEBS 3-11=115/298, 4-11=388/931, 4-9=73/67, 5-9=388/928, 6-9=114/284

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 855 lb uplift at joint 2 and 855 lb uplift at joint 7.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 98 lb down and 83 lb up at 7-0-0, 98 lb down and 83 lb up at 9-0-12, and 98 lb down and 83 lb up at 9-7-4, and 98 lb down and 83 lb up at 11-8-0 on top chord, and 663 lb down and 404 lb up at 7-0-0, 110 lb down and 72 lb up at 9-0-12, and 110 lb down and 72 lb up at 9-7-4, and 663 lb down and 404 lb up at 11-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

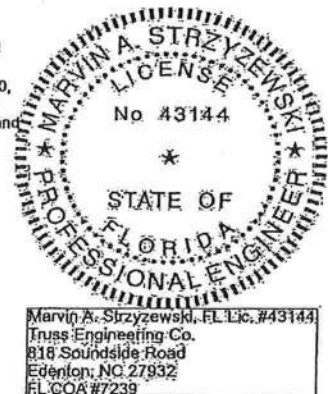
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=48, 4-5=48, 5-8=48, 2-7=40

Concentrated Loads (lb)

Vert: 4=98(B) 5=98(B) 11=663(B) 9=663(B) 12=98(B) 13=98(B) 14=110(B) 15=110(B)



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCES PAGE M1-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513815
DAVENPORT	H51	SPECIAL	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 & Oct 16 2007 METek Industries, Inc. Tue Nov 13 08:17:57 2007 Page 1

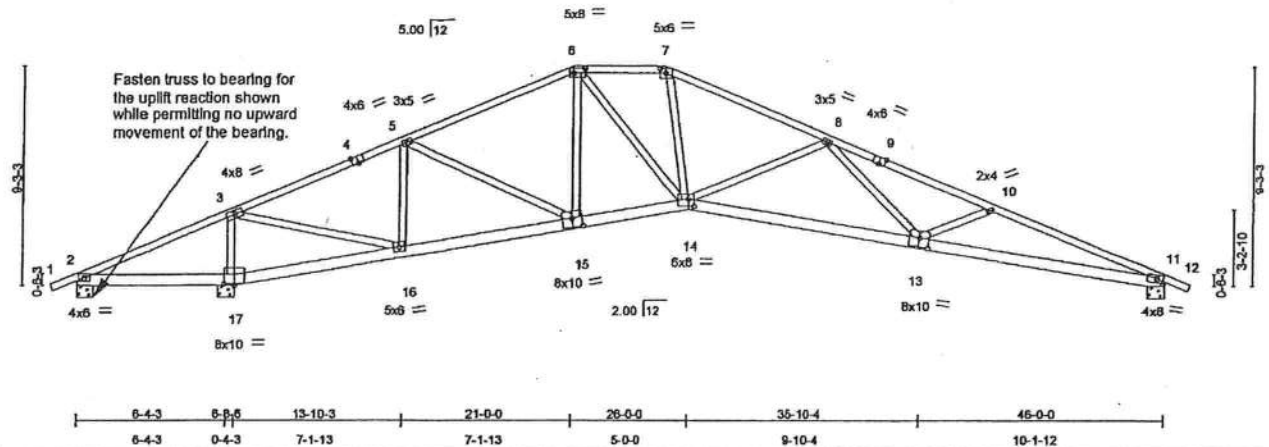
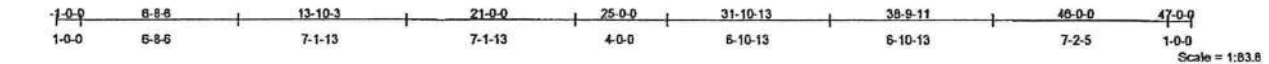


Plate Offsets (X,Y): [4:0-3:0-Edge], [6:0-5:12:0-2-8], [7:0-3:0:0-2-4], [9:0-3:0-Edge], [11:0-3:5:0-2-0], [13:0-5:0:0-4-8], [14:0-2:12:0-3-12], [15:0-5:0:0-4-8], [17:0-5:0:0-13]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.76	Vert(LL)	-0.36 13-14	>909	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.92	Vert(TL)	-0.73 13-14	>645	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.70	Horz(TL)	-0.27 17	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
								Weight: 269 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-0 oc bracing.

REACTIONS (lb/size) 2=672/0-8-0, 17=3124/0-8-6, 11=1575/0-8-6
Max Horz 11=150(LC 6)
Max Uplift=714(LC 11), 17=554(LC 6), 11=379(LC 7)
Max Grav=48(LC 7), 17=3124(LC 1), 11=1575(LC 1)

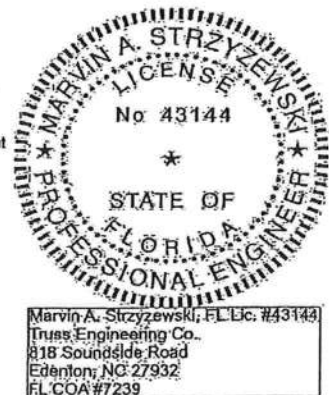
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/14, 2-3=616/2280, 3-4=1171/387, 4-5=1055/399, 5-6=1961/881, 6-7=2375/825, 7-8=2796/861, 8-9=4155/1221, 9-10=4268/1210, 10-11=4501/1408, 11-12=0/11
BOT CHORD 2-17=2010/843, 16-17=2191/709, 15-16=132/1050, 14-15=239/1785, 13-14=849/3418, 11-13=1187/4120
WEBS 3-17=2404/857, 3-16=828/3234, 5-16=992/452, 5-15=127/848, 6-15=287/127, 6-14=227/1020, 7-14=126/831, 8-14=813/473, 8-13=73/834, 10-13=173/316

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 714 lb uplift at joint 2, 554 lb uplift at joint 17 and 379 lb uplift at joint 11.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED METEK REFERENCE PAGE MU-7473 BEFORE USE.

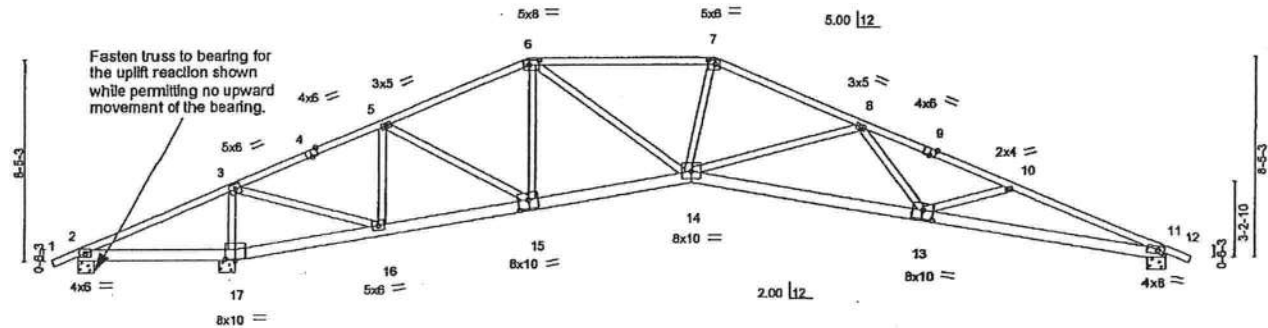
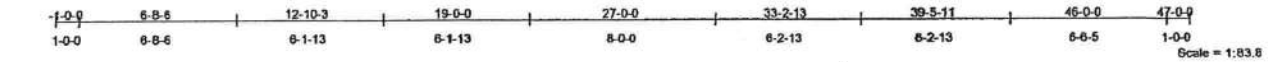
Design valid for use only with Miltek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, B58-89 and B5311 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERED BY
TRENCO
A METEK AFFILIATE

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513816
DAVENPORT	HS2	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:58 2007 Page 1



6-4-3	6-8-6	12-10-3	19-0-0	26-0-0	35-10-4	46-0-0
6-4-3	0-4-3	6-1-13	6-1-13	7-0-0	9-10-4	10-1-12

Plate Offsets (X,Y): [4:0-3-0 Edge], [6:0-5-4-0-2-4], [7:0-3-8-0-2-4], [9:0-3-0 Edge], [11:0-3-5-0-2-0], [13:0-5-0-0-4-8], [15:0-5-0-0-4-8], [17:0-5-0-0-13]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	L/def
TCLL 18.0	Plates Increase	1.25	YC 0.74	Vert(LL)	-0.36 13-14	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.94	Vert(TL)	-0.72 13-14	>652
BCLL 10.0	Rep Stress Incr	YES	WB 0.90	Horz(TL)	-0.28 17	n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)			
				PLATES	GRIP	
				MT20	244/190	
				Weight: 265 lb		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-11 oc bracing.

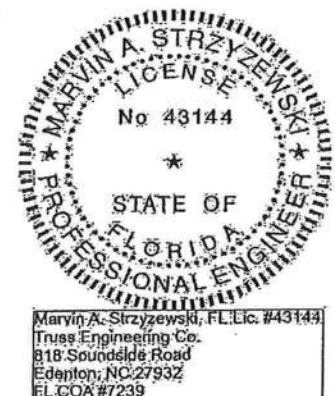
REACTIONS (lb/size) 17=3204/0-8-6, 2=741/0-8-0, 11=1563/0-8-6
Max Horz 11=137(LC 6)
Max Uplift 17=566(LC 5), 2=799(LC 11), 11=368(LC 7)
Max Grav 17=3204(LC 1), 2=45(LC 7), 11=1563(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 7-8=2819/907, 8-9=4181/1218, 9-10=4224/1208, 10-11=4432/1437, 11-12=0/11, 6-7=2705/902, 1-2=0/14,
2-3=678/2453, 3-4=848/279, 4-5=785/297, 5-6=1817/661
BOT CHORD 13-14=911/3584, 11-13=1217/4053, 16-17=2355/768, 15-16=43/758, 14-15=261/1669, 2-17=2169/701
WEBS 10-13=135/314, 8-13=7/660, 8-14=977/423, 7-14=48/744, 6-14=293/1348, 6-15=423/217, 5-15=253/1105,
5-16=1173/481, 3-16=813/3094, 3-17=2463/873

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 17, 799 lb uplift at joint 2 and 368 lb uplift at joint 11.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-1473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D31-89 and ICS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53717.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513617
DAVENPORT	H53	SPECIAL	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:17:59 2007 Page 1

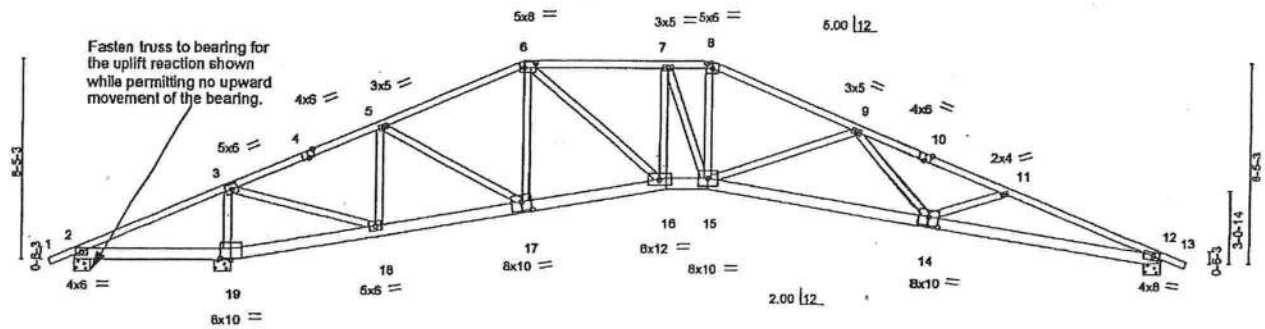
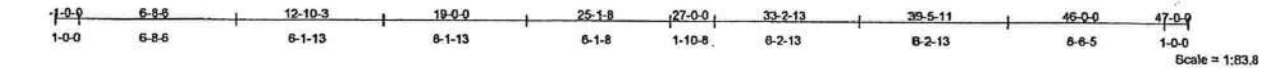


Plate Offsets (X,Y):	[4-0-3-0-Edge]	[6-0-5-12-0-2-8]	[8-0-3-0-0-2-4]	[10-0-3-0-Edge]	[12-0-3-5-0-2-0]	[14-0-5-0-0-4-8]	[17-0-5-0-0-4-8]	[19-0-5-0-0-0-13]
----------------------	----------------	------------------	-----------------	-----------------	------------------	------------------	------------------	-------------------

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.76	Vert(LL)	-0.35 14-15	>666	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.94	Vert(TL)	-0.70 14-15	>666	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.65	Horz(TL)	-0.27 19	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
								Weight: 277 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-13 oc bracing.

REACTIONS (lb/size) 19=3189/0-8-6, 2=736/0-8-0, 12=1564/0-8-6
Max Horz 12=137(LC 8)
Max Uplift 19=566(LC 5), 2=794(LC 11), 12=368(LC 7)
Max Grav 18=3199(LC 1), 2=43(LC 7), 12=1564(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 8-9=2927/926, 9-10=4197/1239, 10-11=4280/1228, 11-12=4448/1424, 12-13=0/11, 6-7=2553/893, 7-8=2666/906,
1-2=0/14, 2-3=677/2443, 3-4=850/282, 4-5=790/301, 5-6=1819/656
BOT CHORD 14-15=923/3559, 12-14=1205/4064, 15-18=479/2539, 18-19=2346/768, 17-18=477/63, 16-17=252/1655,
2-19=2160/701
WEBS 11-14=110/286, 9-14=19/666, 9-15=899/434, 8-15=181/895, 7-15=130/386, 7-16=523/188, 6-16=311/1240,
6-17=383/209, 5-17=239/1092, 5-18=1178/483, 3-18=818/3092, 3-19=2457/872

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Bearing at joint(s) 12 considers parallel to grain value using ANSV/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 19, 794 lb uplift at joint 2 and 368 lb uplift at joint 12.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, F.L.E.C. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL.COR.#7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSV/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513818
DAVENPORT	HS4	SPECIAL	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Tue Nov 13 08:17:59 2007 Page 1

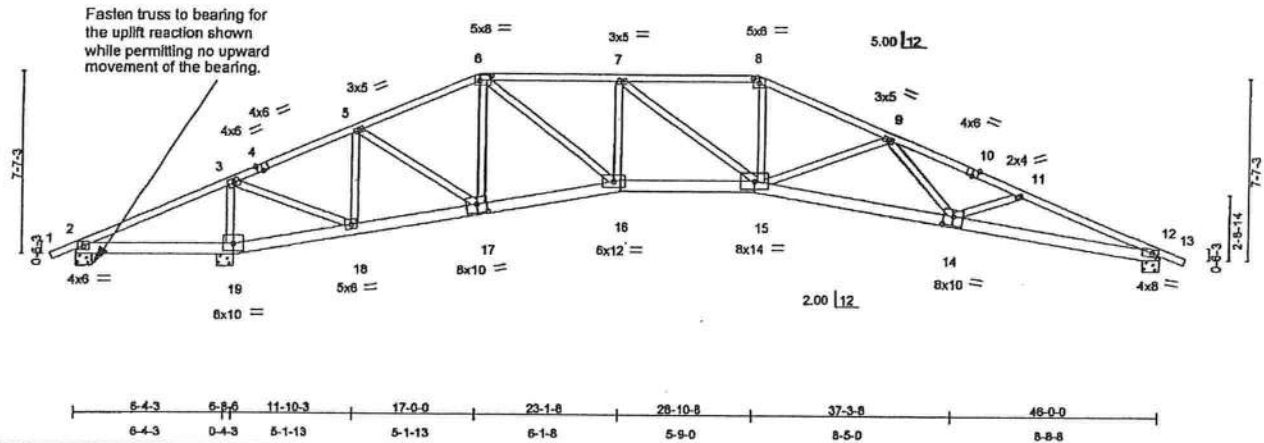


Plate Offsets (X,Y): [4:0-3:0-Edge] [6:0-5:12:0-2-8] [8:0-3:0:0-2-4] [10:0-3:0-Edge] [12:0-3:5:0-2-0] [14:0-5:0:0-4-8] [17:0-5:0:0-4-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Udefl	L/d
TCLL 18.0	Plates Increase	1.25	TC 0.77	Vert(LL)	-0.33 14-15	>999	240
TCDL 7.0	Lumber Increase	1.25	BC 0.95	Vert(TL)	-0.67 14-15	>703	180
BCLL 10.0	Rep Stress Incr	YES	WB 0.54	Horz(TL)	-0.27 19	n/a	n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)				
Weight: 271 lb							

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-7-2 oc bracing.

REACTIONS (lb/size) 19=3302/0-8-8, 2=825/0-8-0, 12=1550/0-8-6
Max Horz 12=124(LC 6)
Max Uplift 19=827(LC 5), 2=898(LC 11), 12=355(LC 7)
Max Grav 19=3302(LC 1), 2=37(LC 7), 12=1550(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 8-9=3213/1045, 9-10=4213/1271, 10-11=4262/1260, 11-12=4381/1419, 12-13=0/11, 6-7=2630/947, 7-8=2890/1005
1-2=0/14, 2-3=754/2661, 3-4=445/154, 4-5=391/170, 5-6=1537/588
14-15=988/3658, 12-14=1202/3991, 15-16=567/2615, 18-19=2551/842, 17-18=0/387, 18-17=216/1399,
2-19=2360/773
WEBS 11-14=28/233, 9-14=5/514, 9-15=719/351, 8-15=164/950, 7-15=163/391, 7-16=513/309, 8-16=457/1589,
6-17=819/279, 5-17=343/1320, 5-18=1379/527, 3-18=794/2939, 3-19=2528/890

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 12 considers parallel to grain value using ANSI/TP1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 627 lb uplift at joint 19, 898 lb uplift at joint 2 and 355 lb uplift at joint 12.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with MTEK connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, D58-87 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MTEK AFFILIATE
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	City	Ply	DAVENPORT_125	E4513819
DAVENPORT	HSS	SPECIAL	1	1	Job Reference (optional)	
Meranda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:18:00 2007 Page 1

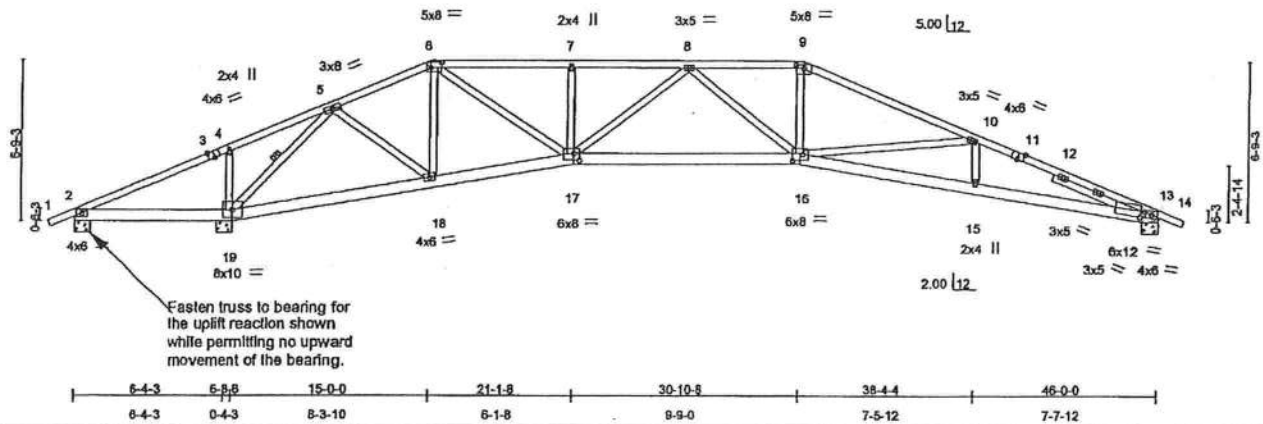
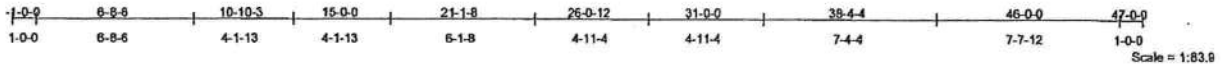


Plate Offsets (X,Y): [3-0-3-0-Edge], [8-0-5-12-0-2-8], [9-0-4-0-0-1-13], [11-0-3-0-Edge], [13-0-1-11-0-3-0], [16-0-5-4-0-3-4], [17-0-2-12-0-3-8]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl
TCLL 18.0	Plates Increase	1.25	TC 0.81	Vert(LL)	-0.36 16-17	>999
TCOL 7.0	Lumber Increase	1.25	BC 0.77	Vert(TL)	-0.73 16-17	>845
BCLL 10.0	Rep Stress Incr	YES	WB 0.62	Horz(TL)	-0.30 19	n/a
BCOL 10.0	Code FBC2004/TPI2002		(Matrix)			
						Weight: 289 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD 2 X 6 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-9-3 oc bracing.
WEBS 2 X 4 SYP No.2	WEBS 1 Row at midpt 5-19
SLIDER Right 2 X 4 SYP No.2 4-0-13	

REACTIONS (lb/size) 13=1561/0-8-6, 19=3294/0-8-6, 2=829/0-8-0
Max Horz 13=112(LC 6)
Max Uplift 13=345(LC 7), 19=665(LC 5), 2=927(LC 11)
Max Grav 13=1561(LC 1), 19=3294(LC 1), 2=48(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 9-10=3803/1159, 10-11=4334/1396, 11-12=4361/1387, 12-13=4429/1378, 13-14=0/11, 6-7=2748/990,
7-8=2748/990, 8-9=3312/1136, 1-2=0/14, 2-3=783/2559, 3-4=759/2613, 4-5=836/2525, 5-6=1281/507
BOT CHORD 15-16=1168/4071, 13-15=1185/4049, 18-17=844/3150, 18-19=199/109, 17-18=176/1177, 2-19=2321/799
WEBS 10-15=0/222, 10-16=713/350, 8-18=82/244, 8-17=542/235, 7-17=268/250, 6-17=590/1939, 6-18=856/361,
5-18=344/1502, 5-19=3388/1036, 4-19=405/314, 9-18=165/1031

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 13, 665 lb uplift at joint 19 and 927 lb uplift at joint 2.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.,
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITek REFERENCE PAGE M1-1473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Gty	Ply	DAVENPORT_125	E4513820
DAVENPORT	H88S	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 MTEK Industries, Inc. Tue Nov 13 08:18:01 2007 Page 1

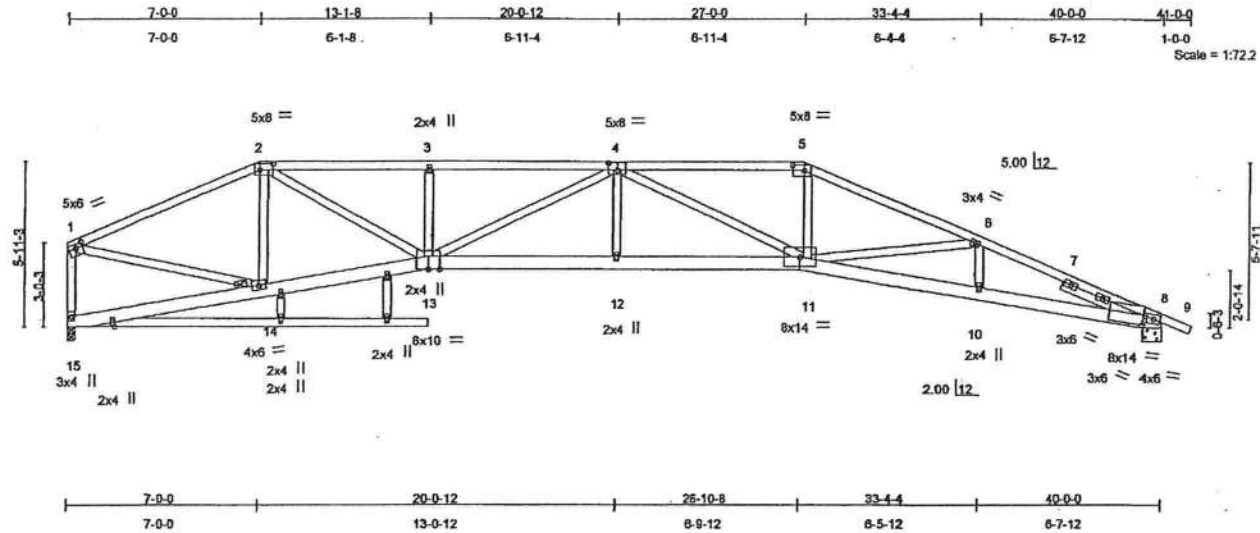


Plate Offsets (X,Y): [1:0-3-0-0-1-12], [2:0-5-12-0-2-8], [4:0-4-0-0-3-0], [5:0-5-0-0-2-4], [8:0-0-11-0-4-4]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 18.0	Plates Increase	1.25	TC 0.71	Vert(LL)	-0.46 11-12	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.82	Vert(TL)	-0.91 11-12	>522
BCLL 10.0	Rep Stress Incr	YES	WB 0.84	Horz(TL)	-0.42 15	n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)			n/a
						Weight: 262 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2 *Except* 5-9 2 X 4 SYP No.1D	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD 2 X 6 SYP No.2 *Except* 17-18 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-1-12 oc bracing.
WEBS 2 X 4 SYP No.2	JOINTS 1 Brace at Jt(s): 14
SLIDER Right 2 X 4 SYP No.2 3-2-8	
REACTIONS (lb/size) 8=1764/0-8-6, 15=1698/0-3-8 Max Horz 8=174(LC 7) Max Uplift 8=352(LC 5), 15=328(LC 5)	
FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD 5-6=4671/1625, 6-7=5037/1692, 7-8=5090/1681, 8-9=0/11, 2-3=4289/1579, 3-4=4289/1580, 4-5=4225/1544, 1-2=2478/875, 1-15=1578/814	
BOT CHORD 10-11=1809/4675, 8-10=1601/4647, 12-13=1652/4888, 11-12=1652/4889, 14-15=70/138, 13-14=735/2253	
WEBS 6-10=0/155, 6-11=310/243, 5-11=335/1431, 4-11=908/270, 4-12=0/317, 4-13=706/231, 3-13=305/269, 2-13=835/2405, 2-14=557/353, 1-14=683/2170	

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 8, 15 considers parallel to grain value using ANS/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 352 lb uplift at joint 8 and 328 lb uplift at joint 15.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513821
DAVENPORT	H87S	SPECIAL	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 06:18:02 2007 Page 1

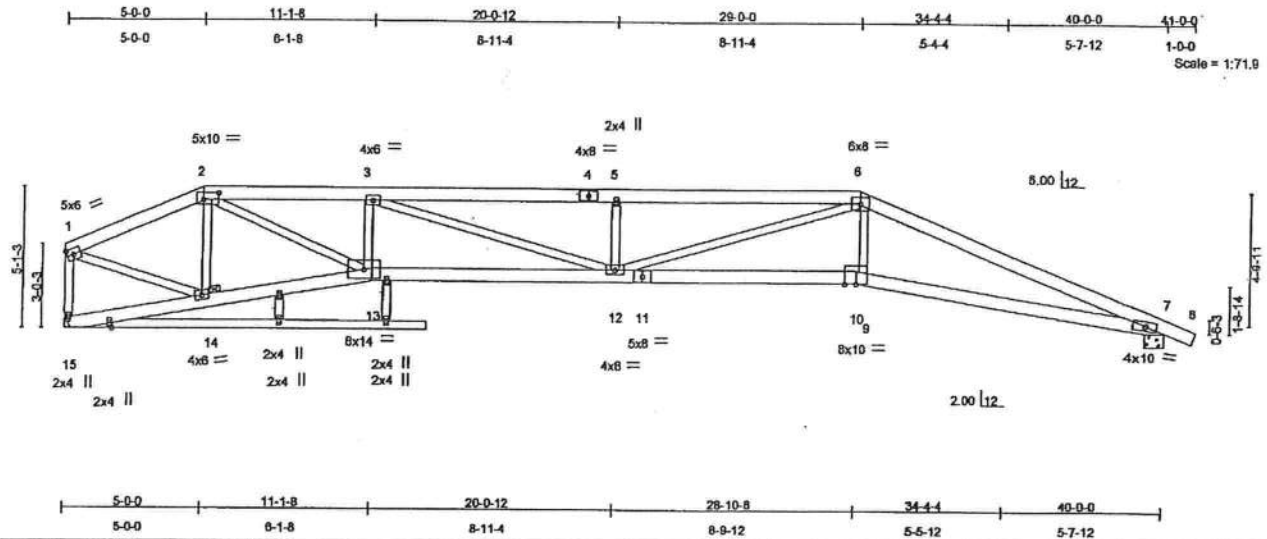


Plate Offsets (X, Y): [2-0-6-12 0-2-12]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.56	Vert(LL)	-0.51 10-12	>922	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.74	Vert(TL)	-1.02 10-12	>463	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.69	Horz(TL)	-0.41 15	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 279 lb

LUMBER
TOP CHORD 2 X 6 SYP No.2
BOT CHORD 2 X 6 SYP No.2 *Except*
7-10 2 X 6 SYP No.1D, 17-18 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-9 oc bracing.
JOINTS 1 Brace at J(s): 14

REACTIONS (lb/size) 7=1768/D-8-6, 15=1698/Mechanical
Max Horz 7=164(LC 7)
Max Uplift 7=373(LC 5), 15=353(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 6-7=5354/1809, 7-8=0/16, 2-3=4755/1754, 3-4=5899/2145, 4-5=5899/2145, 5-6=5899/2145, 1-2=2139/768, 1-15=1612/812
BOT CHORD 9-10=1634/4675, 7-9=1688/4929, 12-13=1635/4718, 11-12=1645/4758, 10-11=1645/4758, 14-15=24/74, 13-14=656/1952
WEBS 6-9=176/1245, 8-12=426/1444, 5-12=420/373, 3-12=436/1267, 3-13=747/471, 2-13=1123/3154, 2-14=786/402, 1-14=679/2005

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to glider(s) for truss to truss connections.
- 6) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 373 lb uplift at joint 7 and 353 lb uplift at joint 15.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, F.L.E., #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D38-89 and IBC11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotof Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Alliance
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513822
DAVENPORT	H586	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 a Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:16:02 2007 Page 1

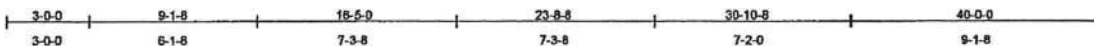
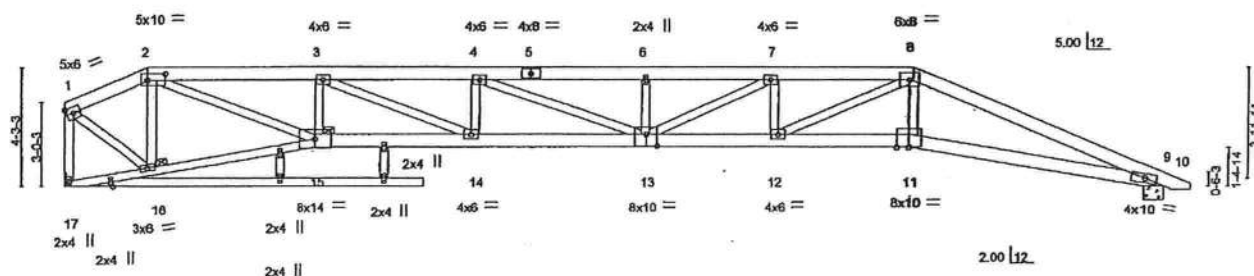
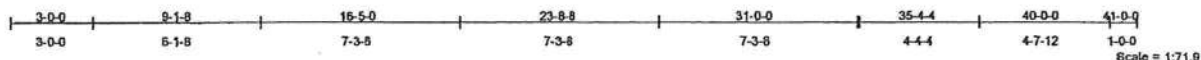


Plate Offsets (X,Y): [2:0-7-4-0-2-12], [13:0-5-0-0-5-0]

LOADING (psf)	SPACING	CS1	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.38	Vert(LL)	-0.80 13-14	>790	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.80	Vert(TL)	-1.19 13-14	>399	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.84	Horz(TL)	-0.45 17	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)						
							Weight: 276 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.2 *Except*
2-5 2 X 8 SYP No.1D, 5-8 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
15-17 2 X 4 SYP No.2, 19-20 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-1 oc bracing.
JOINTS 1 Brace at Jt(s): 15, 16

REACTIONS (lb/size) 9=1745/0-8-6, 17=1699/Mechanical
Max Horz 9=140(LC 7)
Max Uplift 9=372(LC 5), 17=378(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 8-9=5377/1878, 9-10=0/8, 2-3=4787/1754, 3-4=6580/2396, 4-5=6954/2537, 5-8=6954/2537, 6-7=6952/2535,
7-8=8264/2285, 1-2=1542/560, 1-17=1672/809
BOT CHORD 9-11=1768/4856, 14-15=1726/4836, 13-14=2340/6560, 12-13=2229/6264, 11-12=1745/4917, 16-17=6/28,
15-16=489/1408
WEBS 3-15=972/509, 2-15=1315/3862, 2-16=1073/510, 1-16=620/1750, 4-14=433/312, 3-14=667/1873, 6-13=267/252,
4-13=151/432, 7-12=570/277, 7-13=279/815, 8-12=537/1662, 8-11=212/1137

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Bearing at Joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 9 and 378 lb uplift at joint 17.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, F.L.E., #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and ECSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

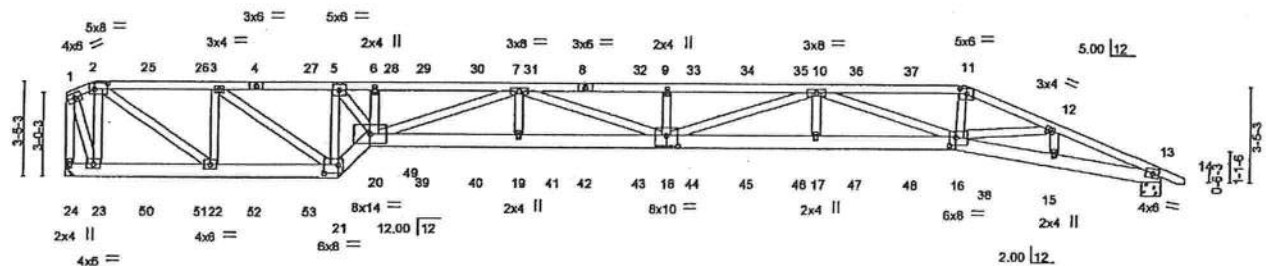
ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513023
DAVENPORT	HSGRD18	SPECIAL	1	3	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 18 2007 MITek Industries, Inc. Tue Nov 13 08:18:04 2007 Page 1



Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513823
DAVENPORT	HSGRD1S	SPECIAL	1	3	Job Reference (optional)	

Meranda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:18:04 2007 Page 2

NOTES

- 8) Bearing at Joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1416 lb uplift at Joint 13 and 1181 lb uplift at Joint 24.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 119 lb down and 114 lb up at 33-0-0, 108 lb down and 122 lb up at 30-11-4, 108 lb down and 122 lb up at 28-11-4, 108 lb down and 122 lb up at 26-11-4, 108 lb down and 122 lb up at 24-11-4, 108 lb down and 122 lb up at 22-11-4, 108 lb down and 122 lb up at 20-11-4, 108 lb down and 122 lb up at 18-11-4, 108 lb down and 122 lb up at 16-11-4, 108 lb down and 122 lb up at 14-11-4, 108 lb down and 122 lb up at 12-11-4, 108 lb down and 122 lb up at 10-11-4, 86 lb down and 1 lb up at 8-11-4, 86 lb down and 1 lb up at 6-11-4, 86 lb down and 1 lb up at 4-11-4, and 86 lb down and 1 lb up at 2-11-4, and 118 lb down and 1 lb up at 1-0-0 on top chord, and 690 lb down and 518 lb up at 33-0-0, 98 lb down at 30-11-4, 98 lb down at 28-11-4, 98 lb down at 26-11-4, 98 lb down at 24-11-4, 98 lb down at 22-11-4, 98 lb down at 20-11-4, 98 lb down at 18-11-4, 98 lb down at 16-11-4, 98 lb down at 14-11-4, 98 lb down at 12-11-4, 98 lb down at 10-11-4, 105 lb down and 84 lb up at 8-11-4, 105 lb down and 84 lb up at 6-11-4, 105 lb down and 84 lb up at 4-11-4, and 105 lb down and 84 lb up at 2-11-4, and 105 lb down and 84 lb up at 0-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 11-14=23, 2-11=23, 1-2=23, 13-16=20, 16-20=20, 20-21=20, 21-24=20

Concentrated Loads (lb)

Vert: 11=119(B) 8=108(B) 2=86(B) 4=86(B) 23=105(B) 25=86(B) 26=86(B) 27=86(B) 28=108(B) 29=108(B) 30=108(B) 31=108(B) 32=108(B) 33=108(B) 34=108(B) 35=108(B) 36=108(B) 37=108(B) 38=690(B) 39=98(B) 40=98(B) 41=98(B) 42=98(B) 43=98(B) 44=98(B) 45=98(B) 46=98(B) 47=98(B) 48=98(B) 49=98(B) 50=105(B) 51=105(B) 52=105(B) 53=105(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-T4T3 BEFORE USE.

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-87 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513824
DAVENPORT	J	MONO TRUSS	20	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Tue Nov 13 08:18:04 2007 Page 1

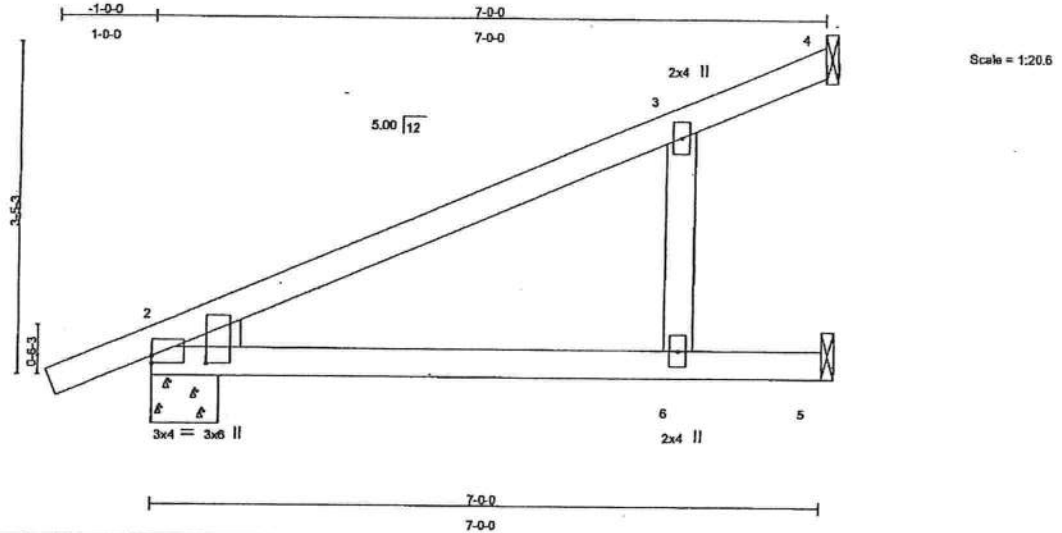


Plate Offsets (X,Y): [2:0-0-0-0-0-15], [2:0-0-14-0-6-12]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d
TCLL 16.0	Plates Increase	1.25	TC 0.39	Vert(LL)	-0.12	2-6	>639
TCDL 7.0	Lumber Increase	1.25	BC 0.44	Vert(TL)	-0.28	2-6	>309
BCLL 10.0	Rep Stress Incr	YES	WB 0.02	Horz(TL)	0.01	4	n/a
BCDL 10.0	Code FBC2004/TP12002		(Matrix)				
							Weight: 28 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=132/Mechanical, 2=355/0-8-6, 5=145/Mechanical
Max Horz 2=143(LC 6)
Max Uplift 4=19(LC 6), 2=127(LC 6), 5=72(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/10, 2-3=106/26, 3-4=17/48
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=17/165

NOTES (5)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=f.60 plate grip DOL=f.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to glider(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 4, 127 lb uplift at joint 2 and 72 lb uplift at joint 5.
- 5) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 4 and 5.

LOAD CASE(S) Standard



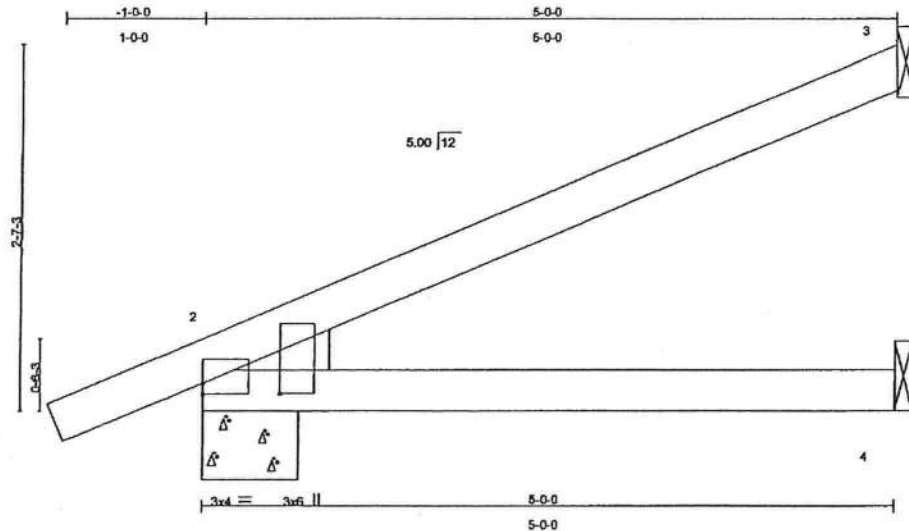
Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES MD-1473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513825
DAVENPORT	J1	JACK	4	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						
7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:18:05 2007 Page 1						



Scale = 1:14.2

Plate Offsets (X,Y): [2-0-0-0-0-0-15] [2-0-0-14-0-6-12]												
LOADING (psf)		SPACING 2-0-0		CSI		DEFL		In (loc)	I/defl	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase 1.25		TC	0.18	Vert(LL)	-0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase 1.25		BC	0.20	Vert(TL)	-0.06	2-4	>937	180		
BCLL	10.0	Rep Stress Incr YES		WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2004/TP12002		(Matrix)								
Weight: 18 lb												

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-6, 4=92/Mechanical
Max Horz 2=109(LC 6)
Max Uplift 3=87(LC 6), 2=117(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/10, 2-3=-64/33
BOT CHORD 2-4=0/0

NOTES (5)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 117 lb uplift at joint 2.
- 5) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

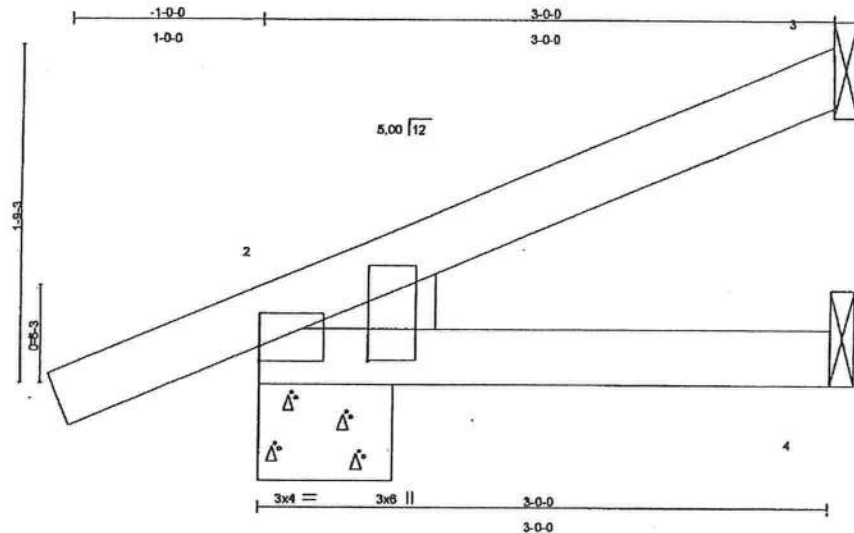
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513826
DAVENPORT	J2	JACK	4	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 & Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:18:05 2007 Page 1



Scale = 1:10.4

Plate Offsets (X,Y): [2-0-0-0-0-15] [2-0-0-14-0-6-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.00	2-4	>999	240	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	-0.01	2-4	>999	180	
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 12 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-0-0 oc purfins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=43/Mechanical, 2=193/0-8-6, 4=52/Mechanical

Max Horz 2=74(LC 6)

Max Uplift 3=-38(LC 6), 2=-112(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/10, 2-3=-37/15

BOT CHORD 2-4=0/0

NOTES (5)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3 and 112 lb uplift at joint 2.
- 5) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
316 Soundside Road
Edenton, NC 27932
FL COR #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITEK AFFILIATE

815 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513832
DAVENPORT	JGRD1	MONO TRUSS	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MITEK Industries, Inc. Tue Nov 13 08:16:07 2007 Page 1

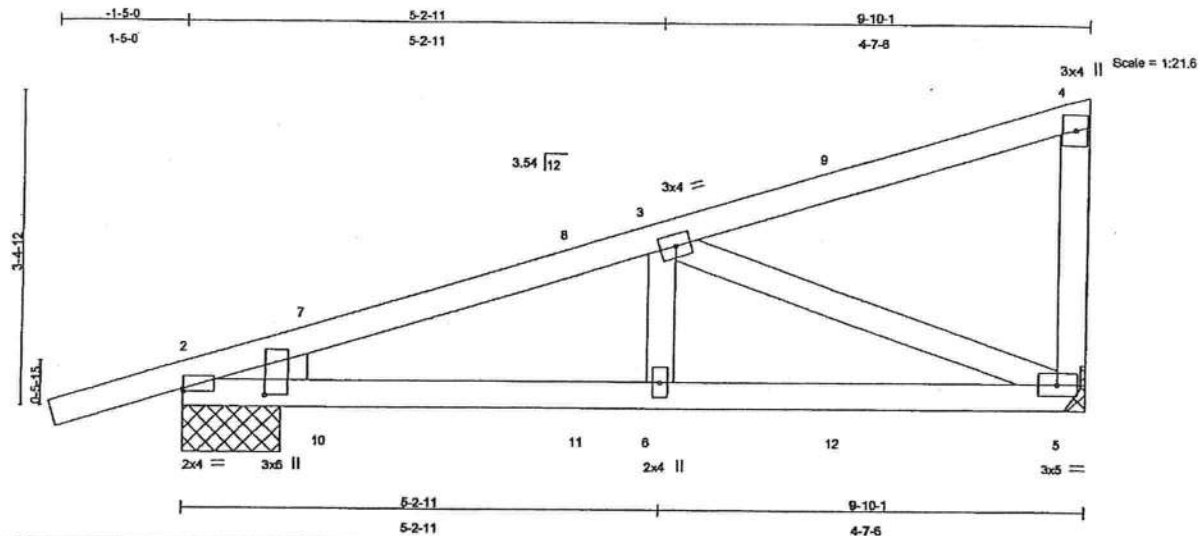


Plate Offsets (X,Y): [2:0-0.1:0-0.6] [2:0-0.13:0-10-8]

LOADING (psf)	SPACING	2:0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.29	Vert(LL)	-0.03	5-6	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.32	Vert(TL)	-0.06	5-6	>999	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.27	Horz(TL)	0.01	5	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 47 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-7-1 oc bracing.

REACTIONS (lb/size) 5=547/Mechanical, 2=519/1-0-10
Max Horz 2=142(LC 3)
Max Uplift 5=297(LC 7), 2=256(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/11, 2-7=877/337, 7-8=843/360, 3-8=812/338, 3-9=119/63, 4-9=84/3, 4-5=128/160
BOT CHORD 2-10=424/795, 10-11=424/795, 6-11=424/795, 6-12=424/795, 5-12=424/795
WEBS 3-6=0/277, 3-5=776/402

NOTES

- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 5 and 256 lb uplift at joint 2.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 44 lb up at 4-4-12, 44 lb up at 4-4-12, 50 lb down and 117 lb up at 7-2-11, 50 lb down and 117 lb up at 7-2-11, and 44 lb down at 1-6-12, and 44 lb down at 1-6-12 on top chord, and 22 lb up at 1-6-12, 22 lb up at 1-6-12, 12 lb down at 4-4-12, 12 lb down at 4-4-12, and 52 lb down at 7-2-11, and 52 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)

Vert 1-4=48, 2-5=40

Concentrated Loads (lb)

Vert 8=6(F=3, B=3) 9=101(F=50, B=50) 10=43(F=22, B=22) 11=24(F=12, B=12) 12=104(F=52, B=52)



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSII/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 563 O'Conor Drive, Madison, WI 53719.

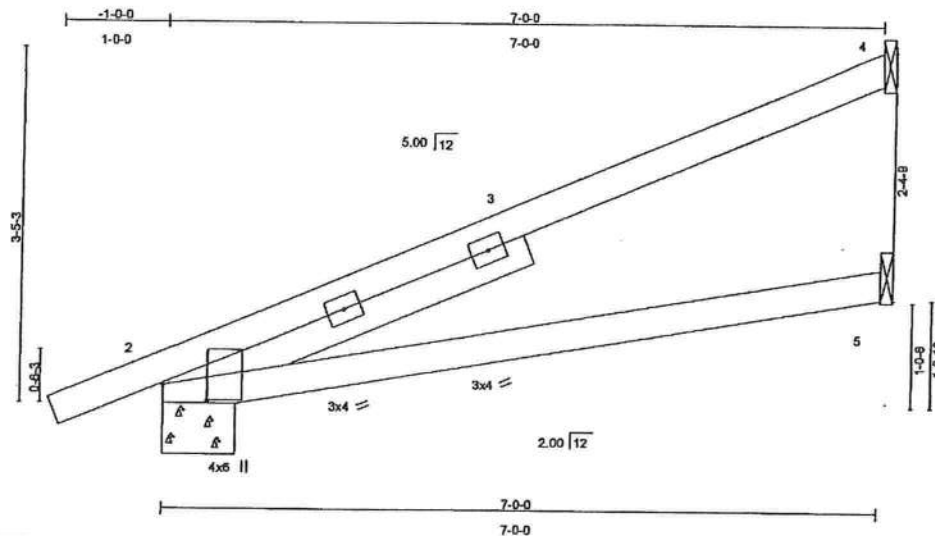
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513834
DAVENPORT	JS	SPECIAL	11	1	Job Reference (optional)	

Maronde Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Tue Nov 13 08:18:06 2007 Page 1



Scale = 1:19.3

Plate Offsets (X,Y): [2-0-1-13.0-5-2]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL		PLATES	
TCLL	16.0	Plates Increase 1.25		TC	0.47	In	(loc)	L/defl	L/d
TCDL	7.0	Lumber Increase 1.25		BC	0.45	Vert(LL)	-0.12	2-5	>890
BCLL	10.0	Rep Stress Incr YES		WB	0.00	Vert(TL)	-0.30	2-5	>276
BCDL	10.0	Code FBC2004/TP12002		(Matrix)		Horz(TL)	-0.02	4	n/a
								MT20	GRIP
								244/190	
								Weight: 29 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
SLIDER Left 2 X 4 SYP No.2 3-9-9

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 4=154/Mechanical, 2=351/0-8-8, 5=138/Mechanical
Max Horz 2=141(LC 6)
Max Uplift 4=140(LC 6), 2=109(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/6, 2-3=88/0, 3-4=78/47
BOT CHORD 2-5=-18/18

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 4 and 109 lb uplift at joint 2.
- 6) Attach with (2) 18d Common Toe-Nails (0.162"x3.5") at joints 4 and 5.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenon, NC 27932
FL GOA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7478 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-97 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenon, NC 27932

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MITex Industries, Inc. Tue Nov 13 08:18:08 2007 Page 1



LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
SLIDER Left 2 X 4 SYP No.2 2-8-9

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=107/Mechanical, 2=267/0-8-6, 5=98/Mechanical
Max Horz 2=106(LC 6)
Max Uplift 4=98(LC 6), 2=98(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/8, 2-3=-84/0, 3-4=-55/32
BOT CHORD 2-5=-13/13

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDF=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 4 and 98 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 4 and 5.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, F.L.E.C. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

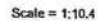
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not a truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Miltek Affiliate
815 Soundside Road
Edenton, NC 27932

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:18:09 2007 Page 1

LOAD CASE(S) Standard

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Mitek designer. Bracing shown is for lateral support of individual web member only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/APTI Quality Criteria, D58-89 and SCSII Building Component Safety Information available from Trus Plate Institute, 583 O'Connor Drive, Madison, WI 53719.

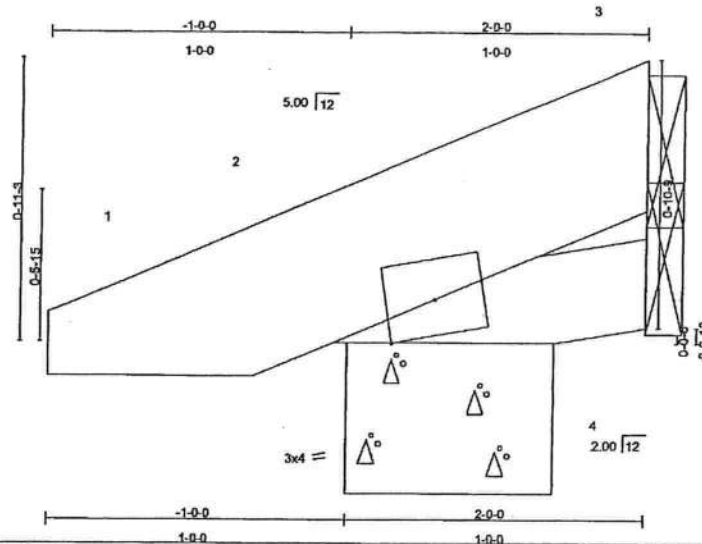
ENGINEERING BY
TRENCO
A MITEC COMPANY

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513837
DAVENPORT	JG3	SPECIAL	2	1	Job Reference (optional)	

Mwonds Homes Inc., Sanford, FL

7.010 & Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:18:09 2007 Page 1



Scale = 1/8" = 1'-0"

LOADING (psf)	SPACING	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.06	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002	(Matrix)							
								Weight: 7 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 4=18/Mechanical, 2=84/0-8-6, 3=11/Mechanical
Max Horz 2=29(LC 6)
Max Uplift 2=54(LC 6), 3=14(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/2, 2-3=18/4
BOT CHORD 2-4=3/3

NOTES (7)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSVTP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 2 and 14 lb uplift at joint 3.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

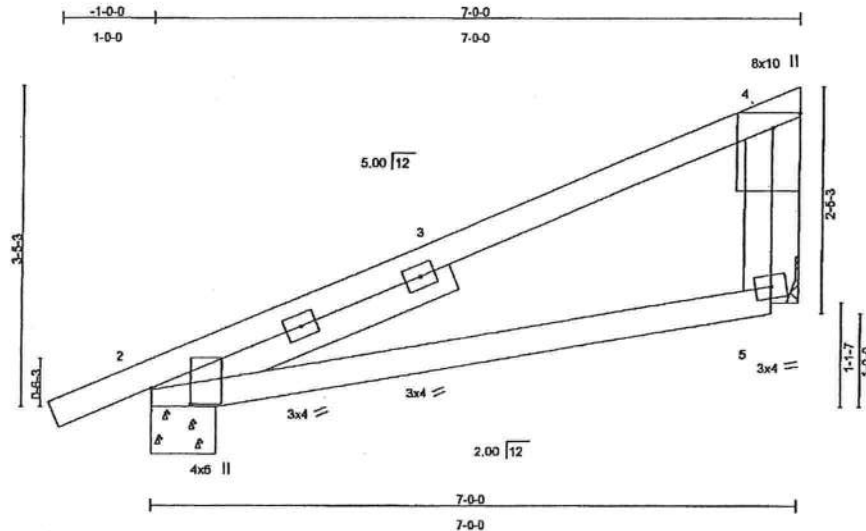
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
Design valid for use only with Mitek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSVTP1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotorio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513038
DAVENPORT	USA	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:18:10 2007 Page 1



Scale = 1:21.3

Plate Offsets (X,Y): [2:0-2-1.0-5-2], [4:Edge:0-3-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.10	2-5	>816	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.24	2-5	>326	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.01	Horz(TL)	0.00	5	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 34 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
SLIDER Left 2 X 4 SYP No.2 3-5-13

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=335/0-8-6, 5=276/Mechanical
Max Horz 2=134(LC 8)
Max Uplift 2=107(LC 8), 5=93(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/6, 2-3=-85/0, 3-4=-74/44
BOT CHORD 2-5=-17/17
WEBS 4-5=-146/166

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=8.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(I) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 2 and 93 lb uplift at joint 5.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-747S BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrications, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and ECSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513839
DAVENPORT	JSGRD1	SPECIAL	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						
7,010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:18:10 2007 Page 1						

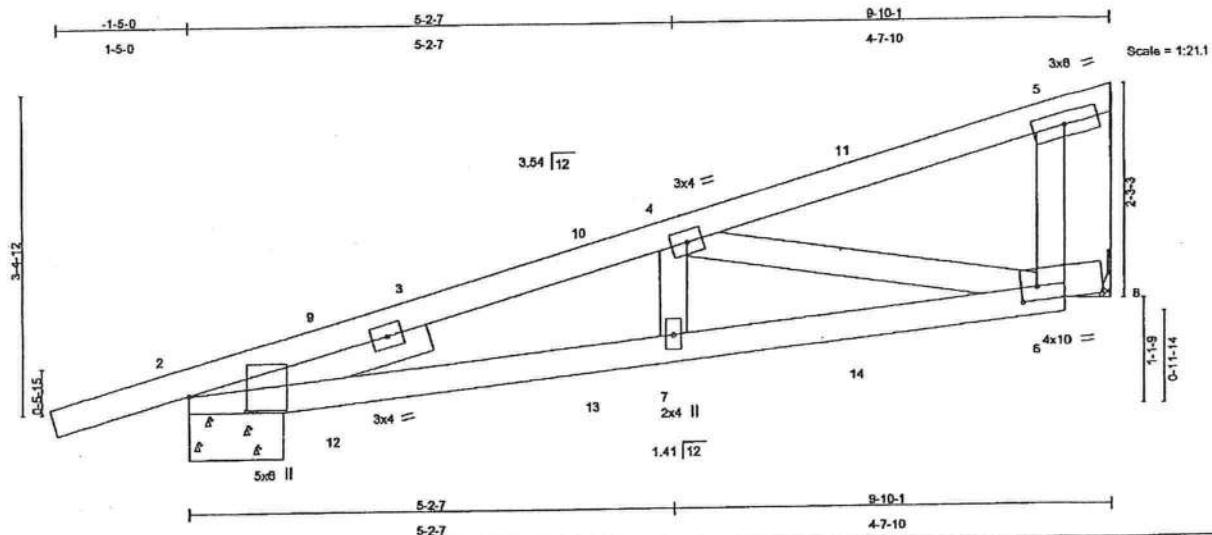


Plate Offsets (X,Y): [2:0-2-0-0-7-6], [6:0-2-0-0-1-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.32	Vert(LL)	-0.04	6-7	>999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.39	Vert(TL)	-0.08	6-7	>999		
BCLL 10.0	Rep Stress Incr	NO	WB 0.24	Horz(TL)	0.02	8	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 50 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2 *Except*
 5-6 2 X 6 SYP No.2
 SLIDER Left 2 X 4 SYP No.2 2-7-11

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-6-7 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 7-5-5 oc bracing.

REACTIONS (lb/size) 2=550/0-11-14, 8=562/Mechanical
 Max Horz 2=133(LC 3)
 Max Uplift 2=273(LC 7), 8=334(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/6, 2-9=1307/640, 3-9=1272/657, 3-10=1272/658, 4-10=1241/632, 4-11=348/215, 5-11=288/149,
 5-6=169/462
 BOT CHORD 2-12=703/1201, 12-13=697/1208, 7-13=697/1222, 7-14=706/1203, 6-14=701/1227
 WEBS 4-7=0/254, 4-6=913/501, 5-6=596/368, 6-8=172/203

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise).
Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 2 and 334 lb uplift at joint 8.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 11 lb down and 85 lb up at 4-4-12, 11 lb down and 65 lb up at 4-4-12, 61 lb down and 132 lb up at 7-2-11, 61 lb down and 132 lb up at 7-2-11, and 43 lb down at 1-6-13, and 43 lb down at 1-6-13 on top chord, and 21 lb up at 1-6-13, 21 lb up at 1-6-13, 18 lb down at 4-4-12, 18 lb down at 4-4-12, and 58 lb down at 7-2-11, and 58 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-5=46, 2-6=40

Continued on page 2



Marvin A. Strzyzewski, F.L.E.C. #43144
 Truss Engineering Co.
 818 Soundside Road
 Eden, NC 27932
 FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and ICS1 Building Component Safety Information available from Truss Plate Institute, 583 O'Connell Drive, Madison, WI 53719.

TRENCO
 A Mitek Affiliate

818 Soundside Road
 Eden, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513839
DAVENPORT	JSGRD1	SPECIAL	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 18 2007 MITek Industries, Inc. Tue Nov 13 08:18:10 2007 Page 2

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 10=-22(F=-11, B=-11) 11=-121(F=-61, B=-61) 12=41(F=21, B=21) 13=-36(F=-18, B=-18) 14=-116(F=-58, B=-58)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE ME-1473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and IBCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate

818 Soundside Road
 Eden, NC 27632

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513843
DAVENPORT	51	SPECIAL	8	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 18 2007 Mitek Industries, Inc. Tue Nov 13 08:14 2007 Page 1

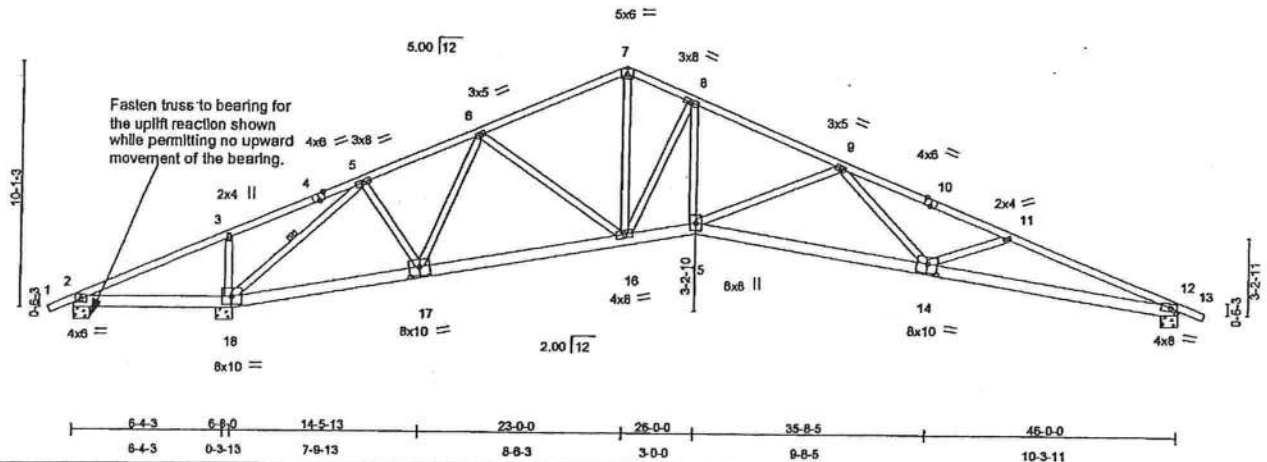
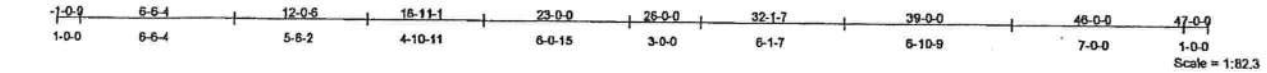


Plate Offsets (X,Y): [4-0-3-0,Edge], [10-0-3-0,Edge], [12-0-3-5-0-2-0], [14-0-5-0-0-4-8], [17-0-5-0-0-4-8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.77	Ver(LL)	-0.38 14-15	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.95	Ver(TL)	-0.76 14-15	>816	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.87	Horz(TL)	-0.32 18	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TP12002							
							Weight: 278 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-6-8 oc purfins.
BOT CHORD 2 X 6 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-4-2 oc bracing.
WEBS 2 X 4 SYP No.2 *Excep1*	WEBS 1 Row at midpt 5-18
8-16 2 X 4 SYP No.1D	

REACTIONS (lb/size) 2=625/0-8-0, 18=3055/0-8-6, 12=1596/0-8-8
 Max Horz 12=162(LC 6)
 Max Uplift 2=653(LC 11), 18=583(LC 6), 12=390(LC 7)
 Max Grav 2=64(LC 7), 18=3055(LC 1), 12=1596(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/14, 2-3=618/2149, 3-4=466/2002, 4-5=455/2090, 5-6=1388/469, 6-7=2057/686, 7-8=2021/710,
 8-9=2888/854, 9-10=4276/1198, 10-11=4337/1186, 11-12=4570/1410, 12-13=0/11
 BOT CHORD 2-18=1893/647, 17-18=41/727, 16-17=240/1800, 15-16=401/2605, 14-15=845/3528, 12-14=1191/4183
 WEBS 3-18=391/323, 5-18=3513/938, 5-17=131/1055, 6-17=799/305, 6-16=61/470, 7-16=356/1318, 8-16=1816/451,
 8-15=306/1593, 9-15=930/458, 9-14=54/797, 11-14=178/331

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 653 lb uplift at joint 2, 683 lb uplift at joint 18 and 390 lb uplift at joint 12.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL GOR #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
 Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	City	Ply	DAVENPORT_125	E4513844
DAVENPORT	T1	COMMON	8	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:15:15 2007 Page 1

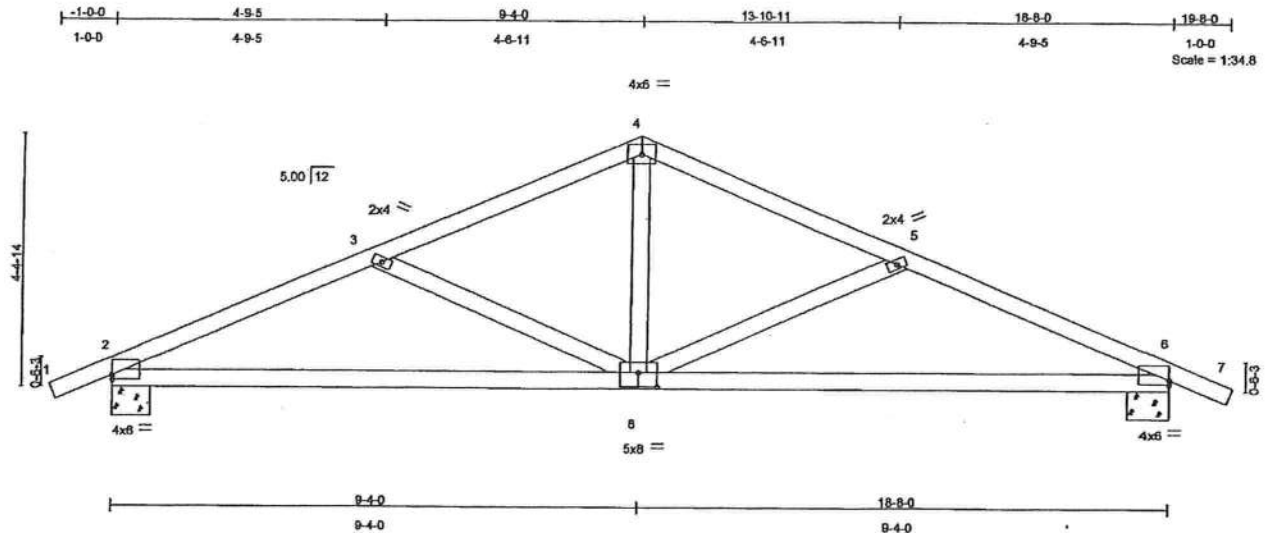


Plate Offsets (X,Y): [2-0-0-1-0-0-15], [8-0-0-1-0-0-15], [8-0-4-0-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.27	Vert(LL)	-0.13	6-8	>999	240	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.30	6-8	>713	180	
BCLL 10.0	Rep Stress Incr	YES	WB 0.11	Horz(TL)	0.03	6	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 82 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-1-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=838/0-8-6, 6=838/0-8-6
 Max Horz 2=73(LC 6)
 Max Uplift 2=230(LC 6), 6=230(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/10, 2-3=1276/426, 3-4=1056/313, 4-5=1056/313, 5-6=1276/426, 6-7=0/10
 BOT CHORD 2-8=293/1131, 6-8=283/1131
 WEBS 3-8=246/193, 4-8=37/573, 5-8=246/193

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.80 plate grip DOL=1.80. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2 and 230 lb uplift at joint 6.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17473 BEFORE USE.
 Design valid for use only with Mittek connection. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/APA Quality Criteria, D58-BF and ICC-ES Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A MITEK Alliance
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513845
DAVENPORT	T2	COMMON	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL						

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:18:15 2007 Page 1

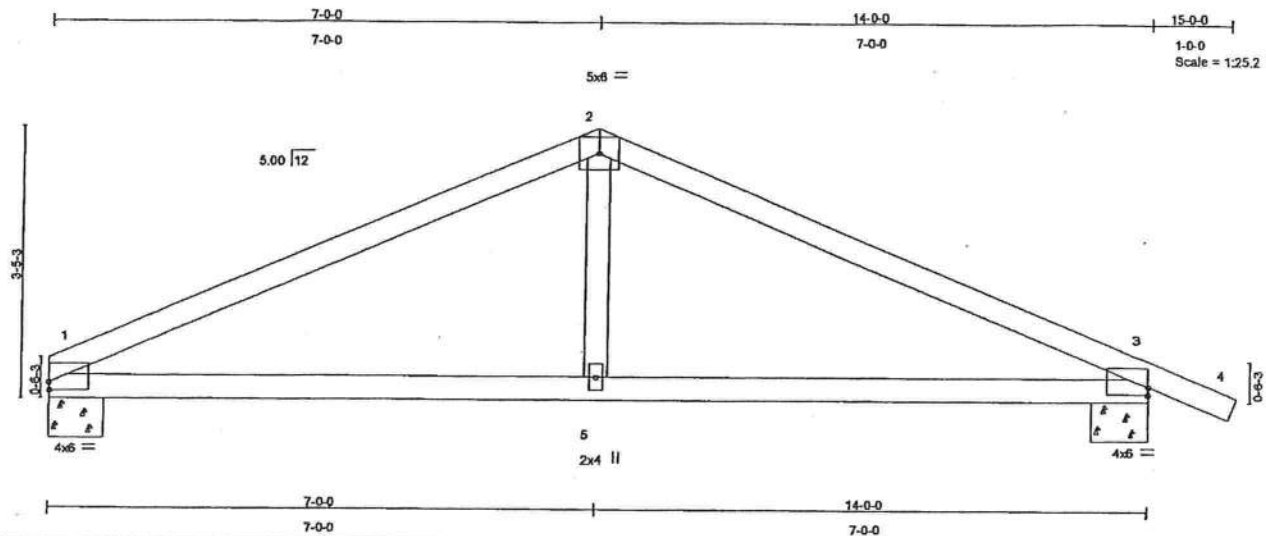


Plate Offsets (X,Y): [1:0-0-1.0-1-3] [3:0-0-1.0-1-3]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl
TCLL 16.0	Plates Increase	1.25	TC 0.31	Vert(LL)	-0.08	1-5 >999
TCCL 7.0	Lumber Increase	1.25	BC 0.53	Vert(TL)	-0.15	1-5 >999
BCLL 10.0	Rep Stress Incr	YES	WB 0.06	Horz(TL)	0.02	3 n/a
BCCL 10.0	Code FBC2004/TP12002		(Matrix)			
			PLATES GRIP			
			MT20 244/190			
			Weight: 50 lb			

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 1=570/0-8-6, 3=641/0-8-6
 Max Horz 1=-63(LC 7)
 Max Uplift 1=-109(LC 6), 3=-193(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-862/266, 2-3=-877/269, 3-4=0/10
 BOT CHORD 1-5=-137/746, 3-5=-137/746
 WEBS 2-5=0/332

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.00. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 1 and 193 lb uplift at joint 3.

LOAD CASE(S) Standard



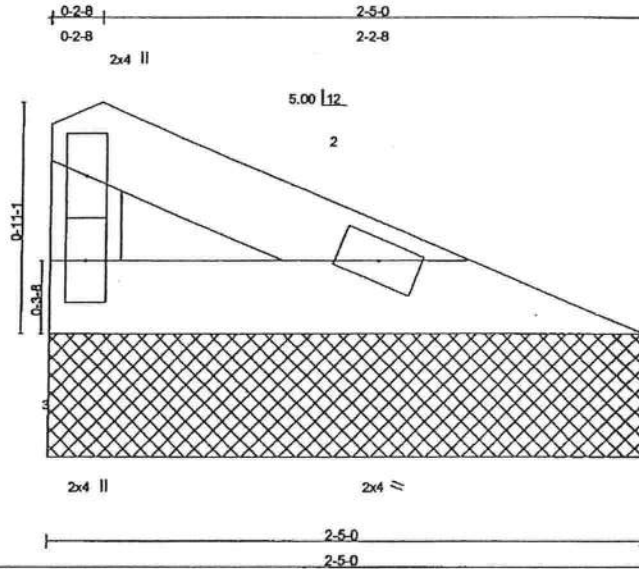
Marvin A. Strzyzewski, FL Lic. #43144
 Truss Engineering Co.
 818 Soundside Road
 Edenton, NC 27932
 FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/1P11 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53717.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513856
DAVENPORT	VIS	VALLEY	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL			7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 11:44:43 2007 Page 1			



Scale: 1.5"=1'

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.01	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=50/2-5-0, 2=50/2-5-0
Max Horz 3=27(LC 7)
Max Uplift 3=20(LC 7), 2=5(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-3=27/34, 1-2=21/10
BOT CHORD 2-3=-0/33

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=8.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 3 and 5 lb uplift at joint 2.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, F.L.E. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE NO-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, D51-89 and BCS11 Building Component Safety Information, available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53717.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MTEK Industries, Inc. Tue Nov 13 11:44:58 2007 Page 1



Weight: 14 lb

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.2

TOP CHORD

Structural wood sheathing directly applied or 4-5-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

(lb/size) 3=152/4-5-0, 2=152/4-5-0
Max Horz 3=81(LC 7)
Max Uplift 3=50(LC 7), 2=25(LC 7)

b) - Maximum Compression/Maximum Tension

TOP CHORD 1-3=-81/92, 1-2=-46/25
BOT CHORD 2-3=0/78

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

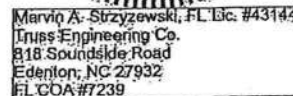
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Gable requires continuous bottom chord bearing.

4) **WARNING:** Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 3 and 25 lb uplift at joint 2.

LOAD CASE(S) Standard



November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MU-7473 BEFORE USE.

Design valid for use only with Mittek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper installation of component is responsibility of building designer - not Mittek designer. Bracing shown is for general guidance only. Individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS1/IN11 Quality Criteria, D58-89 and ICC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

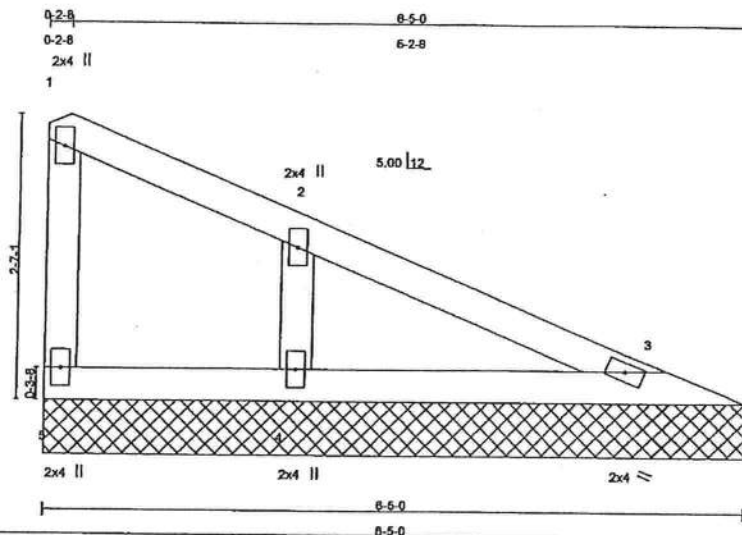
ENGINEERING BY
TRENCO
A MITEC ARM

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513858
DAVENPORT	V3S	GABLE	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 MITek Industries, Inc. Tue Nov 13 08:18:22 2007 Page 1



Scale = 1:10.1

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Vdefl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.10	Vert(LL)	n/a	-	n/a	999	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.08	Vert(TL)	n/a	-	n/a	999	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.03	Horz(TL)	0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						Weight: 23 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=48/6-5-0, 3=118/6-5-0, 4=310/6-5-0
Max Horz 5=96(LC 7)
Max Uplift 5=18(LC 7), 4=102(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-5=-28/29, 1-2=-28/6, 2-3=-99/36
BOT CHORD 4-5=-0/117, 3-4=-0/117
WEBS 2-4=-166/184

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 5 and 102 lb uplift at joint 4.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL E.C. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncillo Drive, Madison, WI 53719.

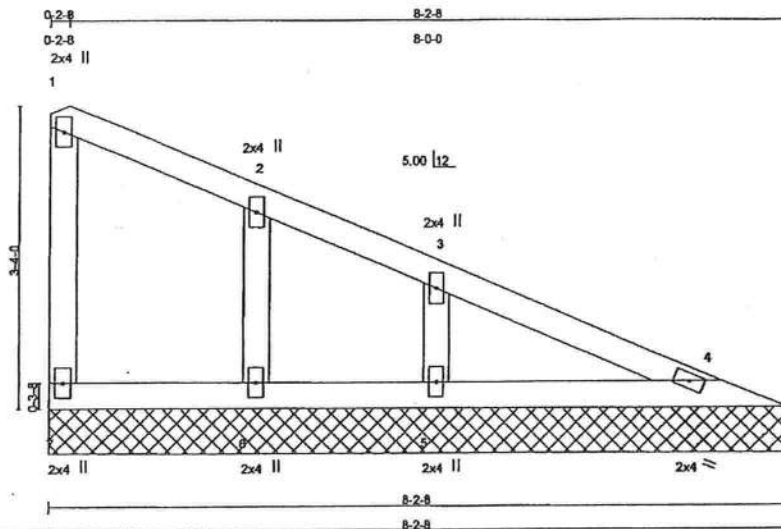
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513859
DAVENPORT	V4S	GABLE	1	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:18:22 2007 Page 1



Scale = 1:22.0

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.07	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.02	Horz(TL)	0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 33 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS

(lb/size) 7=80/8-2-8, 4=113/8-2-8, 6=166/8-2-8, 5=273/8-2-8
Max Horz 7=127(LC 7)
Max Uplift 7=26(LC 7), 6=54(LC 7), 5=90(LC 7)

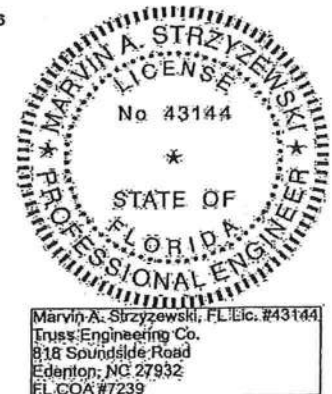
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-7=43/46, 1-2=25/13, 2-3=68/12, 3-4=139/33
BOT CHORD 6-7=0/152, 5-6=0/152, 4-5=0/152
WEBS 2-8=89/96, 3-5=146/159

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 7, 54 lb uplift at joint 6 and 90 lb uplift at joint 5.

LOAD CASE(S) Standard



November 13, 2007

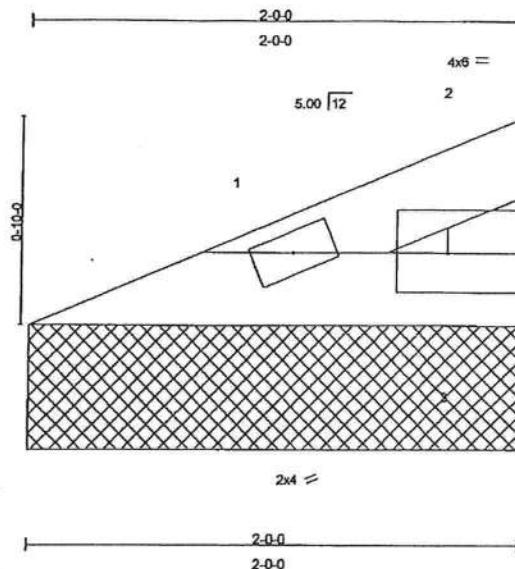


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. A additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-09 and IBC2003 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITEK ALLIANCE

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513860
DAVENPORT	V6	VALLEY	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, FL		7,010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 11:45:24 2007 Page 1				



Scale: 1/8"=1'

Plate Offsets (X,Y): [2:Edge,0-1-14]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Vdefl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.01	Vert(LL)	n/a	-	n/a	999	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	n/a	-	n/a	999	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	-	n/a	n/a	
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						Weight: 5 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=37/2-0-0, 3=37/2-0-0
Max Horz 1=19(LC 6)
Max Uplift 1=4(LC 6), 3=14(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=15/8, 2-3=20/25
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 1 and 14 lb uplift at joint 3.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, P.E., #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

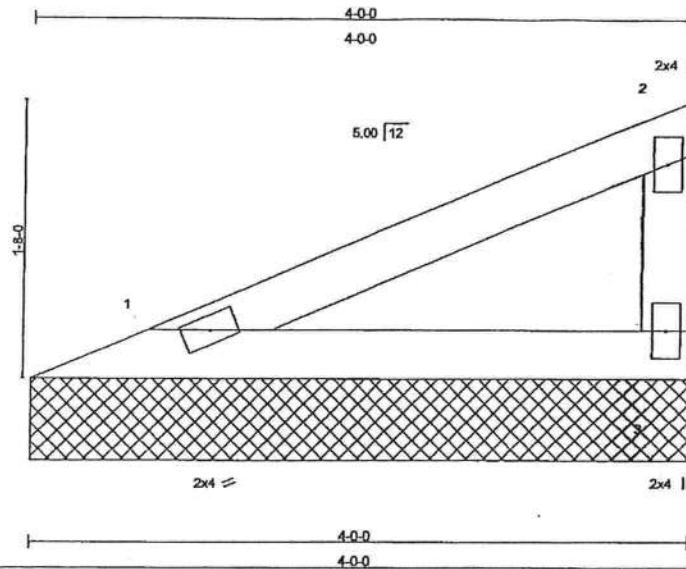
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513861
DAVENPORT	V7	VALLEY	2	1	Job Reference (optional)	

Maronde Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 11:45:33 2007 Page 1



Scale: 1"=1'

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.10	Vert(LL)	n/a	-	n/a	999	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.09	Vert(TL)	n/a	-	n/a	999	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	-	n/a	n/a	
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)						Weight: 13 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=134/4-0-0, 3=134/4-0-0
Max Horz 1=54(LC 6)
Max Uplift 1=22(LC 6), 3=44(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=40/22, 2-3=72/81
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 44 lb uplift at joint 3.

LOAD CASE(S) Standard



November 13, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE NO. 7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSI-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oncofrio Drive, Madison, WI 53719.

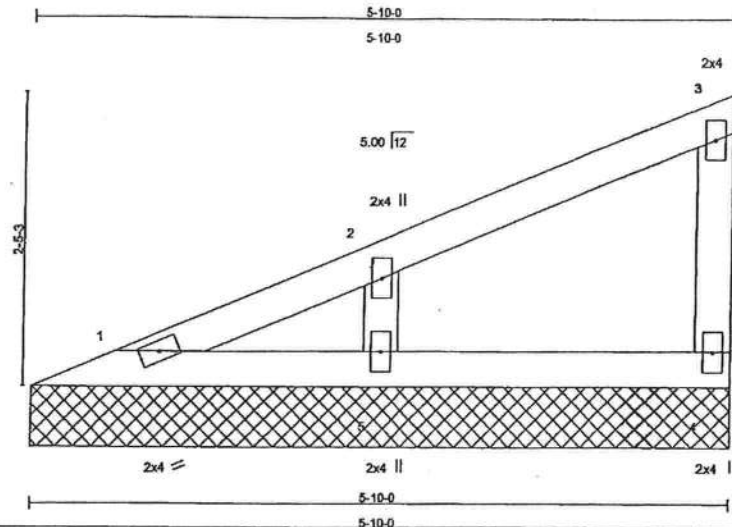
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	DAVENPORT_125	E4513882
DAVENPORT	V8	VALLEY	2	1	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

7.010 s Oct 16 2007 Mitek Industries, Inc. Tue Nov 13 08:18:23 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.07	Vert(TL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.02	Horz(TL)	0.00		n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 21 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-10-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=83/5-10-0, 4=94/5-10-0, 5=269/5-10-0
Max Horz 1=88(LC 6)
Max Uplift 4=31(LC 6), 5=88(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-99/26, 2-3=-31/15, 3-4=-51/57
BOT CHORD 1-5=0/0, 4-5=0/0
WEBS 2-5=-144/161

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Gable requires continuous bottom chord bearing.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 4 and 88 lb uplift at joint 5.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, P.E. Lic. #43144
Truss Engineering Co.
818 Soundside Road
Edenton, NC 27932
FL COA #7239

November 13, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITEK Affiliate

818 Soundside Road
Edenton, NC 27932

COLUMBIA COUNTY OFFICE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-122

Building permit No. 000027223

Use Classification SFD, UTILITY

Fire: 38.52

Permit Holder THEODORE BROCK

Waste: 100.50

Owner of Building MARONDA HOMES

Total: 139.02

Location: 228 SW TIMBER RIDGE DR., LAKE CITY, FL

Date: 04/08/2009

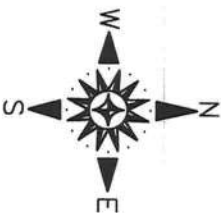


Wayne H. Davis

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

LEGAL DESCRIPTION:
LOT TWENTY-TWO (22) OF "TIMBERLANDS, PHASE 1"
AS PER PLAT THEREOF, AS RECORDED IN PLAT
BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF
COLUMBIA COUNTY, FLORIDA.



PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA

S.W. TIMBER RIDGE DRIVE
60' RIGHT-OF-WAY

CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:
BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT
25', REAR 15', SIDE 10'

SURVEYOR NOTES:

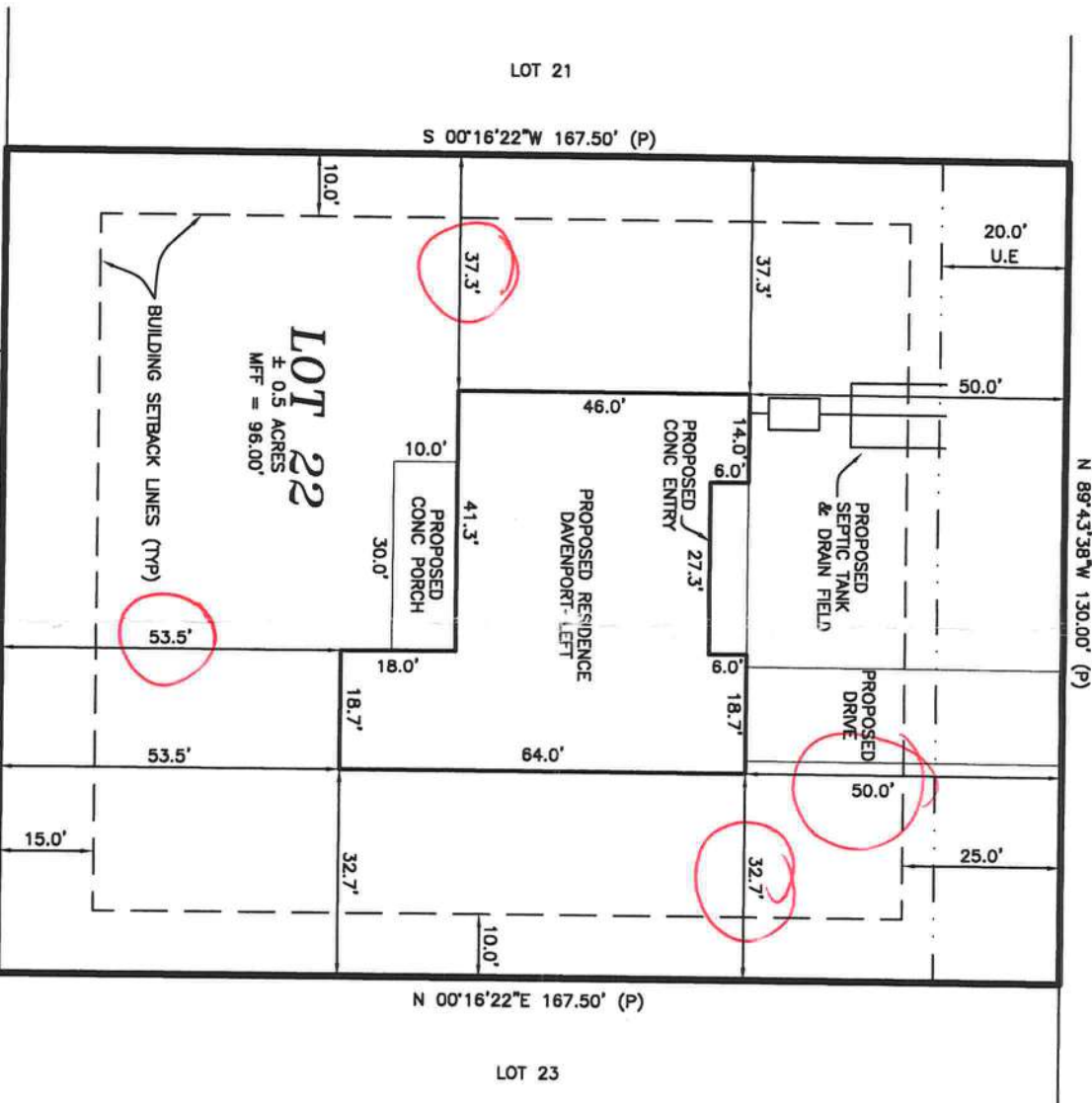
- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.



LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
 - = FOUND 1/2" REBAR & CAP L.B. 6894
 - = SET 1/2" REBAR & CAP L.B. 6894
 - = FOUND 3/4" IRON PIPE
 - = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
 - = SET 4" X 4" CONC. MON. P.S.M. 5582
 - X = SET NAIL & DISK P.S.M. 5582
 - X = FOUND NAIL & DISK
 - ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
 - ⊞ = CATV RISER
 - ⊞ = TELEPHONE PEDESTAL
 - ⊞ = WOOD POWER POLE
- ABBREVIATIONS:**
- A/C = AIR CONDITIONER
 - ASPH = ASPHALT
 - C = CALCULATED FROM MEASURED
 - CATV = CABLE TELEVISION
 - C/B = CONCRETE BLOCK
 - CLF = CHAIN LINK FENCE
 - CM = CONCRETE MONUMENT
 - CONC = CONCRETE
 - ELEC = ELECTRIC
 - ELEV = ELEVATION
 - FND = FOUND
 - FNC = FENCE
 - LB = LICENSED SURVEYOR BUSINESS
 - (M) = FIELD MEASURED
 - MFT = MINIMUM FINISHED FLOOR
 - MH = MANHOLE
 - O.U. = OVERHEAD UTILITIES
 - P = PLAT
 - PB = PLAT BOOK
 - P.U.E. = PUBLIC UTILITIES EASEMENT
 - TRANS = TRANSFORMER
 - TYP = TYPICAL
 - WM = WATER METER
 - WV = WATER VALVE

THIS IS NOT A BOUNDARY SURVEY CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HERON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G-1-6, FLORIDA ADMINISTRATIVE CODE.

BY: *[Signature]*
JAMES E. BRINKMAN, PSM 7 FLA. CERT# 5582
DATE: 6/12/08



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 8th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

DATE: 6/12/08 "THE BENCHMARK IN QUALITY SERVICE"

FIELD WORK COMPLETED ON ****

FIELDBOOK **, PAGE **

DRAWN BY: ZL

CHECKED BY: J.B.

PREPARED FOR: MARONDA

DRAWING NUMBER 127-08

BOUNDARY SURVEY

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA

S.W. TIMBER RIDGE DRIVE

EDGE OF PAVEMENT
60' RIGHT-OF-WAY
4.20' ASPHALT ROAD

CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

BEARING NOTES

BEARINGS SHOWN HEREON ARE REFERRED TO AN ASSUMED VALUE OF S. 00°16'22" W., FOR THE WEST LINE OF LOT 22.

SURVEYOR NOTES:

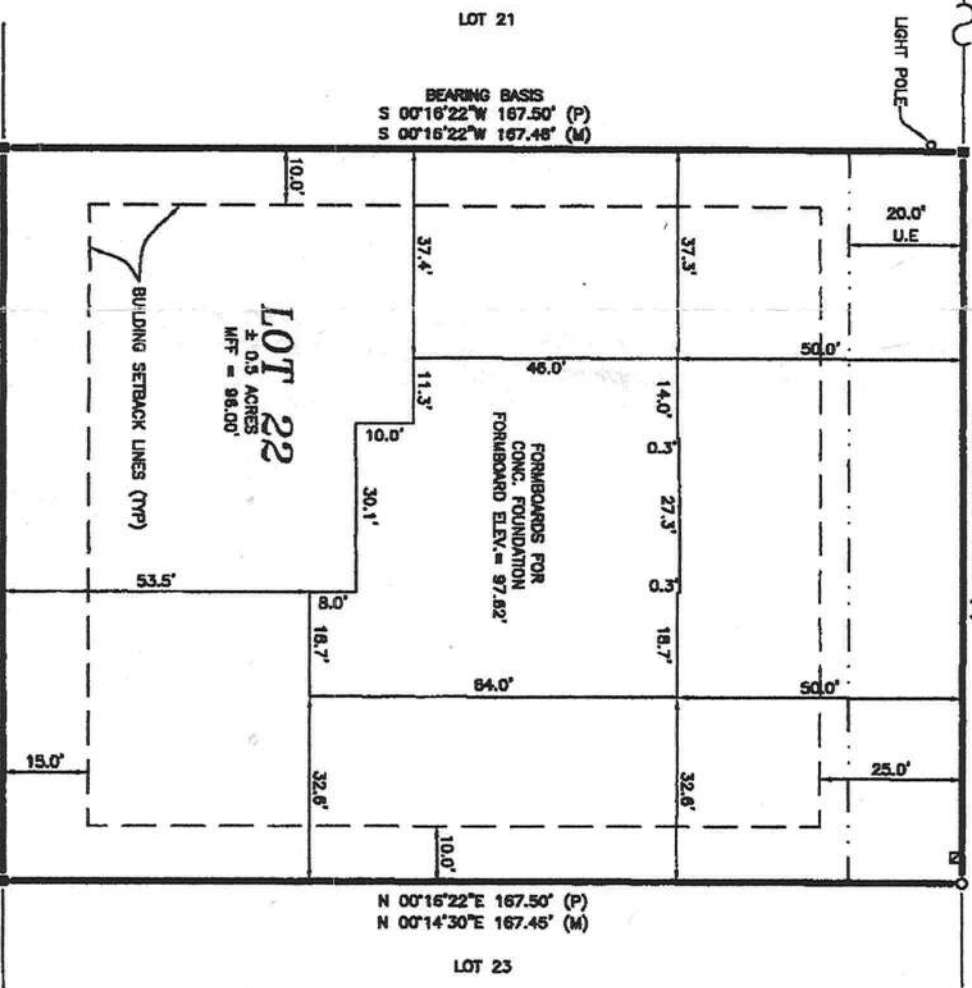
- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE PRIOR TO ANY NEW CONSTRUCTION. THE APPROPRIATE CONCERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN DEPICTED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0-175-B, DATED 1-6-69, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALD FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTES:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THE SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.



LEGEND:

- FOUND 1/2" REBAR NO IDENTIFICATION
- FOUND 1/2" REBAR & CAP L.B. 6894
- SET 1/2" REBAR & CAP L.B. 6894
- FOUND 3/4" IRON PIPE
- FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
- SET 4" X 4" CONC. MON. P.S.M. 5562
- X SET NAIL & DISK P.S.M. 5562
- X FOUND NAIL & DISK
- E CAV RISER
- TELEPHONE PEDESTAL
- WOOD POWER POLE

ABBREVIATIONS:

- | | |
|--------|--------------------------------|
| AC | AIR CONDITIONER |
| ASH | ASBESTOS LIMITED FROM MEASURED |
| CAN | CORROSION RESISTANT |
| CMB | CONCRETE BLOCK |
| CUP | CHAIN LINK FENCE |
| CM | CONCRETE MONUMENT |
| CCNC | CONCRETE |
| ELEC | ELECTRIC |
| ELEV | ELEVATION |
| FND | FOUND |
| FNC | FENCE |
| LB | LEASED SURVEYOR BUSINESS |
| (M) | FIELD MEASURED |
| MFT | MINIMUM FINISHED FLOOR |
| NH | MANHOLE |
| O.U. | OVERHEAD UTILITIES |
| P | PLAT |
| P8 | PUBLIC BOOK |
| P.U.E. | PUBLIC UTILITIES EASEMENT |
| TRANS | TRANSFERMER |
| TYP | TYPICAL |
| WM | WATER METER |
| WW | WATER WAVE |

CERTIFICATE OF SURVEYOR :

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROJECTS, AND I ACCEPT AS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PULSANT TO EXERCISE SUCH STANDARDS, FLORIDA STATUTES AND CHAPTER 61017-8, FLORIDA ADMINISTRATIVE CODE.

DATE: 1/2/09

STATE OF
2019

BRINKMAN SURVEYING & MAPPING, INC.

PHONE: (352) 374-7707

FAX: (352) 374-8757

AC - 1 137000

"THE BENCHMARK IN QUALITY SERVICE"

DATE: 1/15/2009

FIELD WORK COMPLETED ON 1/14/2009 FIELDBOOK 89, PAGE 41

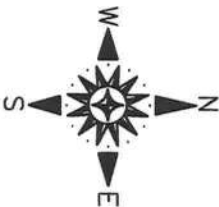
41

PREPARED FOR: MARONDA

127-02

#27223

LEGAL DESCRIPTION:
LOT TWENTY-TWO (22) OF TIMBERLANDS, PHASE 1st
AS PER PLAT THEREOF, AS RECORDED IN PLAT
BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF
COLUMBIA COUNTY, FLORIDA.



BOUNDARY SURVEY
IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA
S.W. TIMBER RIDGE DRIVE
EDGE OF
PAVEMENT
60' RIGHT-OF-WAY
±20' ASPHALT ROAD



Permit # 27223
228 Timber Ridge Dr.

CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:
BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT
25', REAR 15', SIDE 10'

BEARING NOTE:

BEARINGS SHOWN HEREON ARE REFERRED TO AN
ASSUMED VALUE OF S. 00°16'22" W., FOR THE
WEST LINE OF LOT 22.

SURVEYOR NOTES:

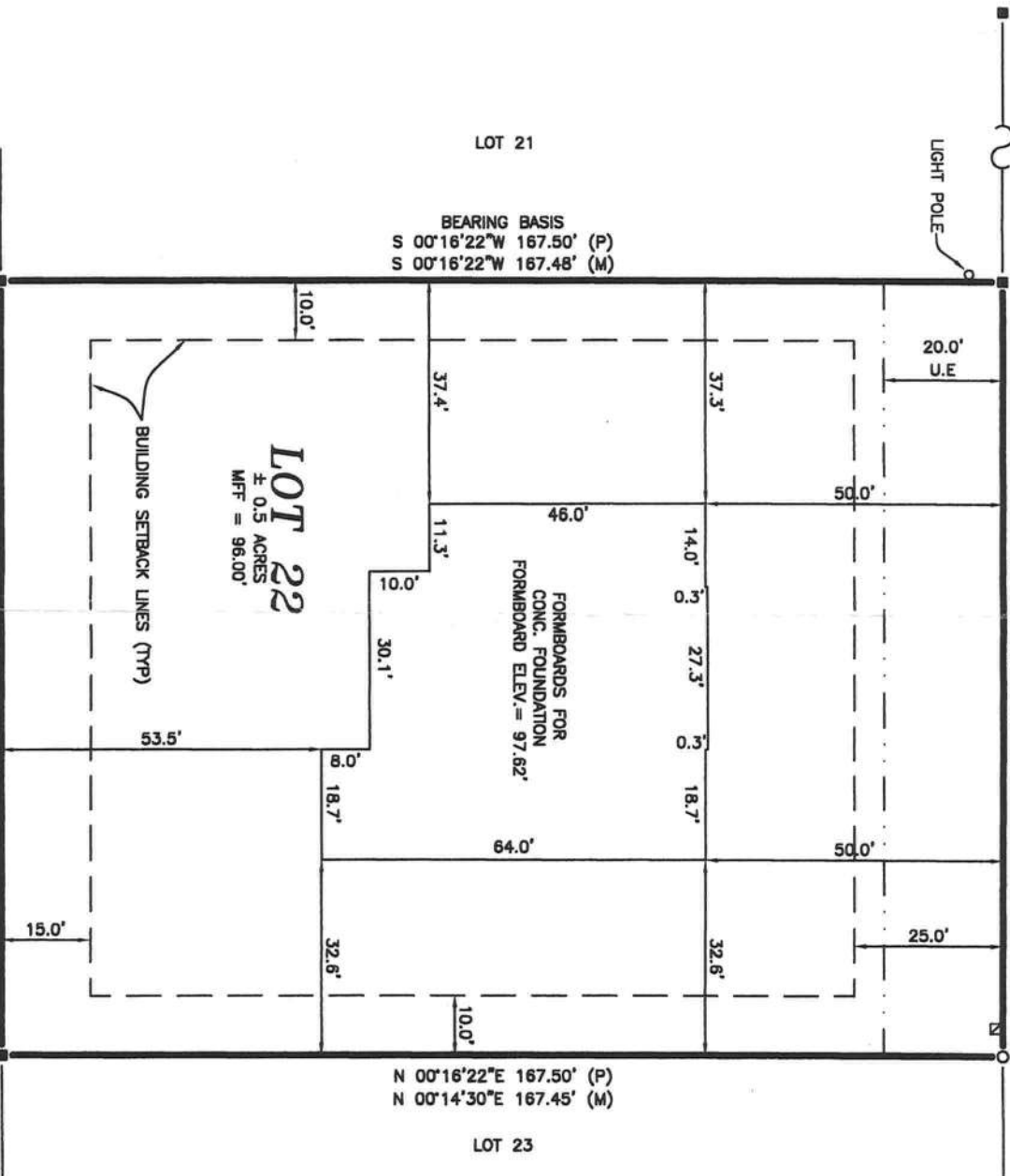
- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-8-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.



LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
 - = FOUND 1/2" REBAR & CAP
 - = SET 1/2" REBAR & CAP
 - = FOUND 3/4" IRON PIPE
 - = FOUND 4" X 4" CONC. MON.
 - = NO IDENTIFICATION
 - = SET 4" X 4" CONC. MON.
 - = SET NAIL & DISK P.S.M. 5582
 - = FOUND NAIL & DISK
 - = CATV RISER
 - = TELEPHONE PEDESTAL
 - = WOOD POWER POLE
- ABBREVIATIONS:**
- A/C = AIR CONDITIONER
 - ASPH = ASPHALT
 - C = CALCULATED FROM MEASURED
 - CATV = CABLE TELEVISION
 - C/B = CONCRETE BLOCK
 - C/F = CHAIN LINK FENCE
 - CM = CONCRETE MONUMENT
 - CONC = CONCRETE
 - ELEC = ELECTRIC
 - ELEV = ELEVATION
 - FND = FOUND
 - FNC = FENCE
 - LB = LICENSED SURVEYOR BUSINESS
 - LB = LICENSED SURVEYOR BUSINESS
 - MFT = MINIMUM FINISHED FLOOR
 - MH = MANHOLE
 - O.U. = OVERHEAD UTILITIES
 - P = PLAT
 - PB = PLAT BOOK
 - P.U.E. = PUBLIC UTILITIES EASEMENT
 - TRANS = TRANSFORMER
 - TYP = TYPICAL
 - WM = WATER METER
 - WV = WATER VALVE

CERTIFICATE OF SURVEYOR :

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.022, FLORIDA STATUTES, AND CHAPTER §1617-6, FLORIDA ADMINISTRATIVE CODE.

BY *James E. Brinkman* - FLA. CERT# 5562
DATE: *1/12/09*



BRINKMAN SURVEYING & MAPPING INC.

4807 NW 8th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: J.D.M.

DATE: 1/15/2009

CHECKED BY: J.B.

FIELD WORK COMPLETED ON 1/14/2009

FIELDBOOK 99, PAGE 41

PREPARED FOR: MARONDA

DRAWING NUMBER
127-08

TREATMENT WORKORDER

☐ Termite Baiting System w/Tubes-under-the slab☐ Treat Only☒ Tubes-under-the slab and Treat☐ Bora-Care

DATE CALLED IN:	1/19	DATE OF SCHEDULE:	1/19
TIME CALLED IN:		TIME SCHEDULE:	

JOB NAME:

Maranda

SUBDIVISION:

Timberland

JOB ADDRESS:

Lot 22

BILLING NAME:

BILLING PHONE:

BILLING ADDRESS:

CALLED IN BY:

PHONE:

PERMIT NUMBER:

321-6892073

27223

LOT & MODEL NUMBER:

22

DATE & TIME COMPLETED:

1/19/09

SQUARE FOOT:

3200

LINEAR FOOT:

BLOCKVOIDS:

SLAB TYPE:

TYPE OF FILL:

APPROX. DEPTH OF FOOTING: Outside:

Inside:

☐ Addition☐ Spot Treat☐ Pool Addition☐ Driveway☐ Final/Completion☐ Other

PESTICIDE USED:

Inaxxpro

TOTAL APPLIED:

150 gal

PERCENT (%) USED:

10%

STICKER POSTED:

215

PRICE PER SQ. FT. =

TOTAL FOR P.T.

ADDITIONAL

TAX:

TOTAL AMOUNT

\$

X

X TECHNICIAN:

I hereby acknowledge the satisfactory completion of the above described work

FEEC.

		CODE	UNIT
ROAD IMPACT FEE	<u>\$1,046.00</u>	<u>218</u>	<u>1</u>
10100003632400			
EMS IMPACT FEE	<u>\$29.88</u>		
10300003632210			
FIRE PROTECTION IMPACT FEE	<u>\$78.63</u>		
10200003632220			
CORRECTIONS IMPACT FEE	<u>\$489.16</u>		
00100003632200			
SCHOOL IMPACT FEE	<u>\$1,500.00</u>		
00100003632900			

TOTAL FEES CHARGED

\$3,063.67

CHECK NUMBER
