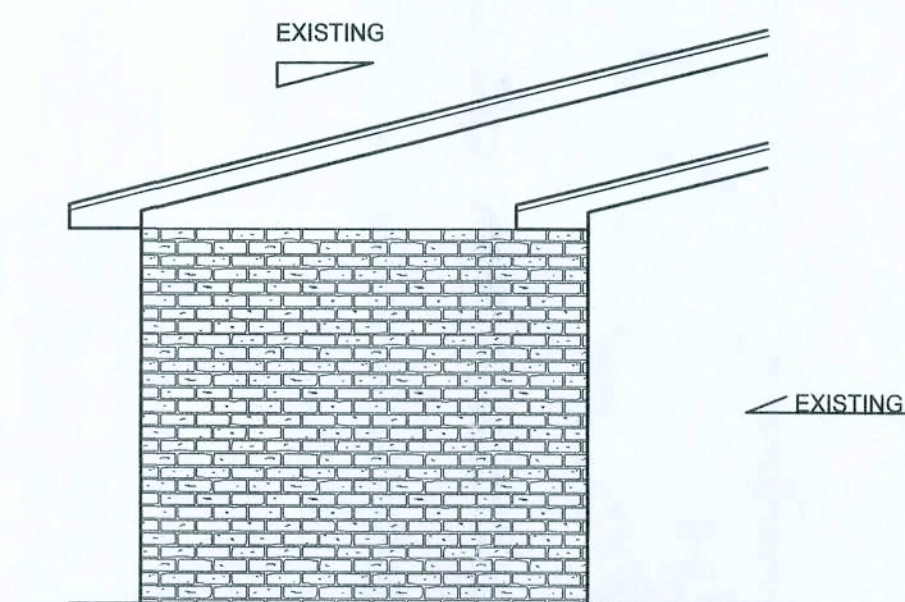
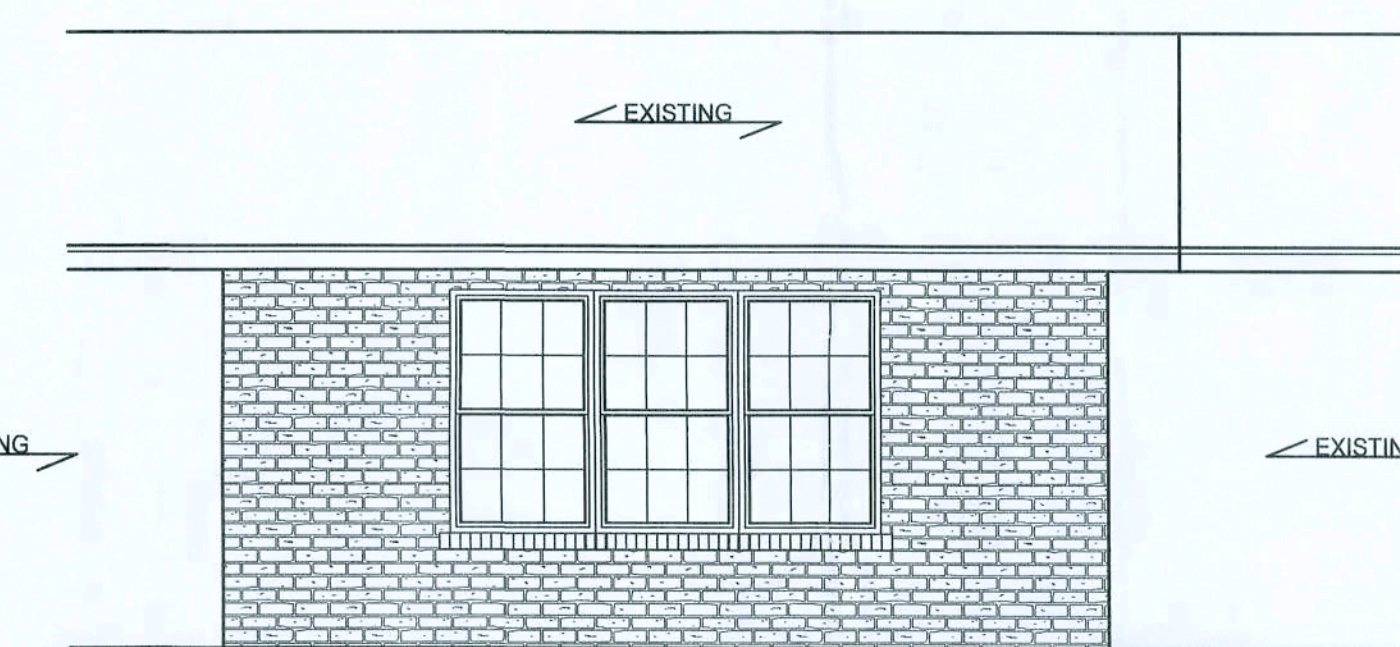


# REVISIONS

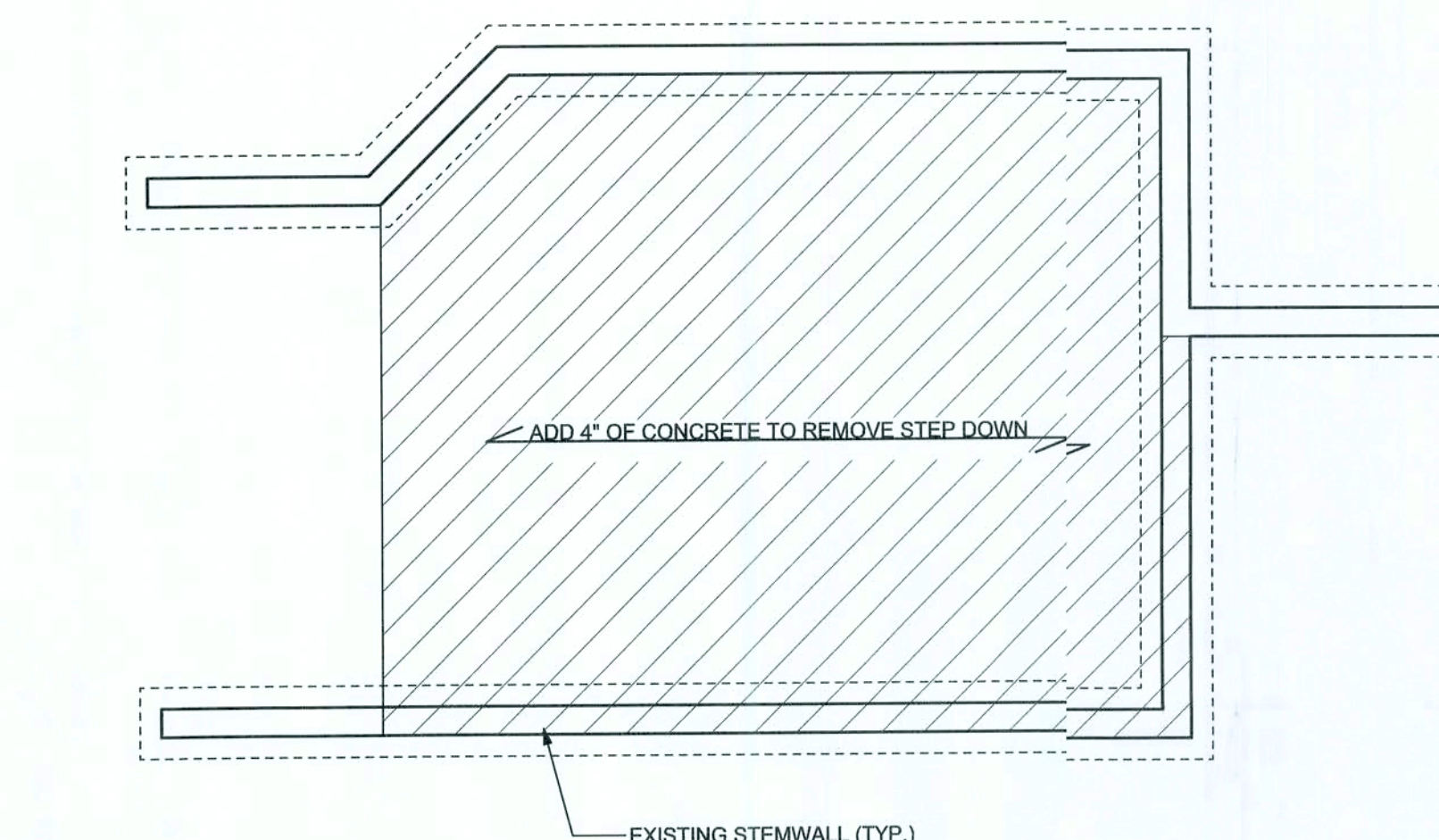

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



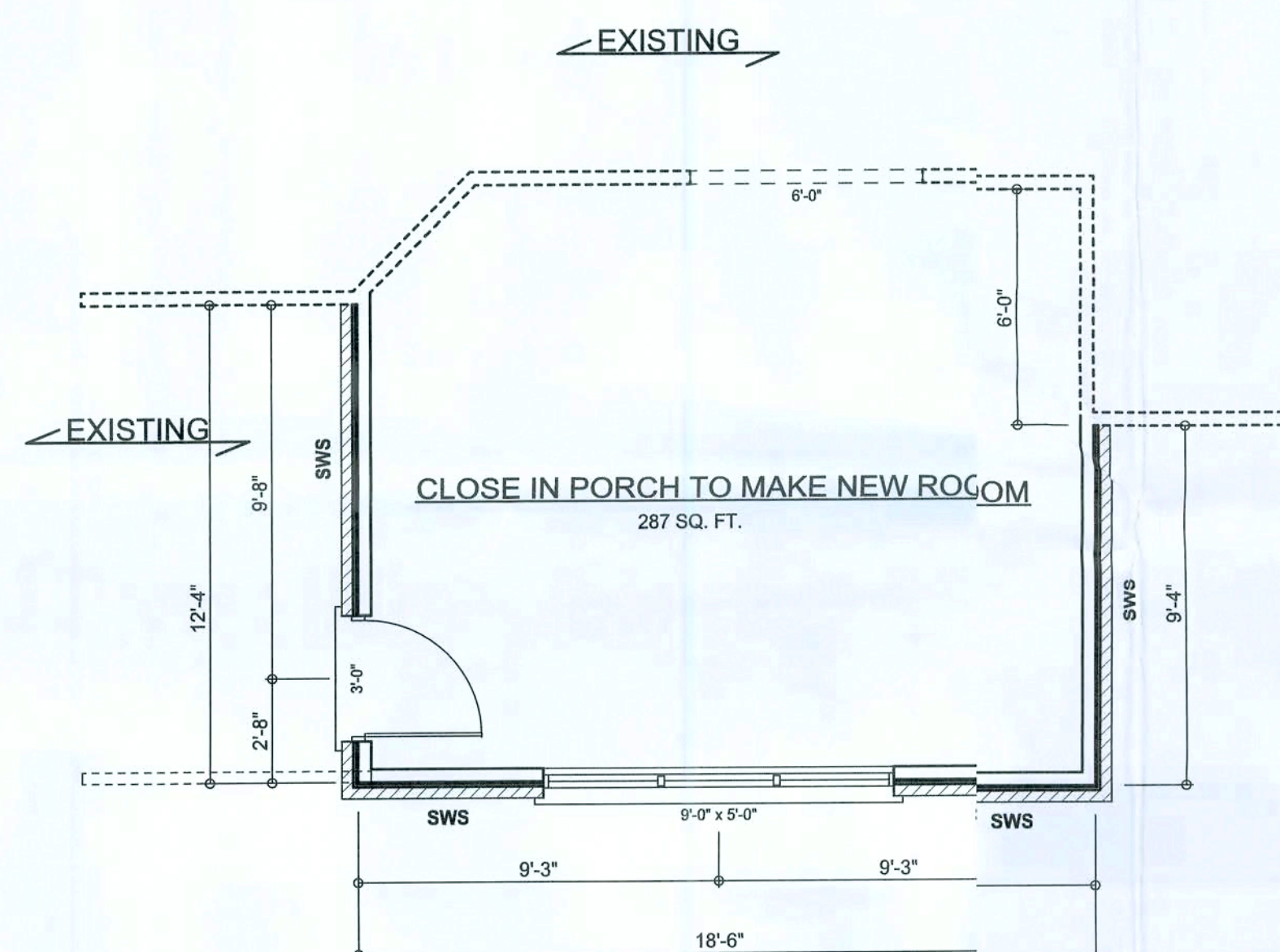
LEFT ELEVATION  
SCALE: 1/4" = 1'-0"



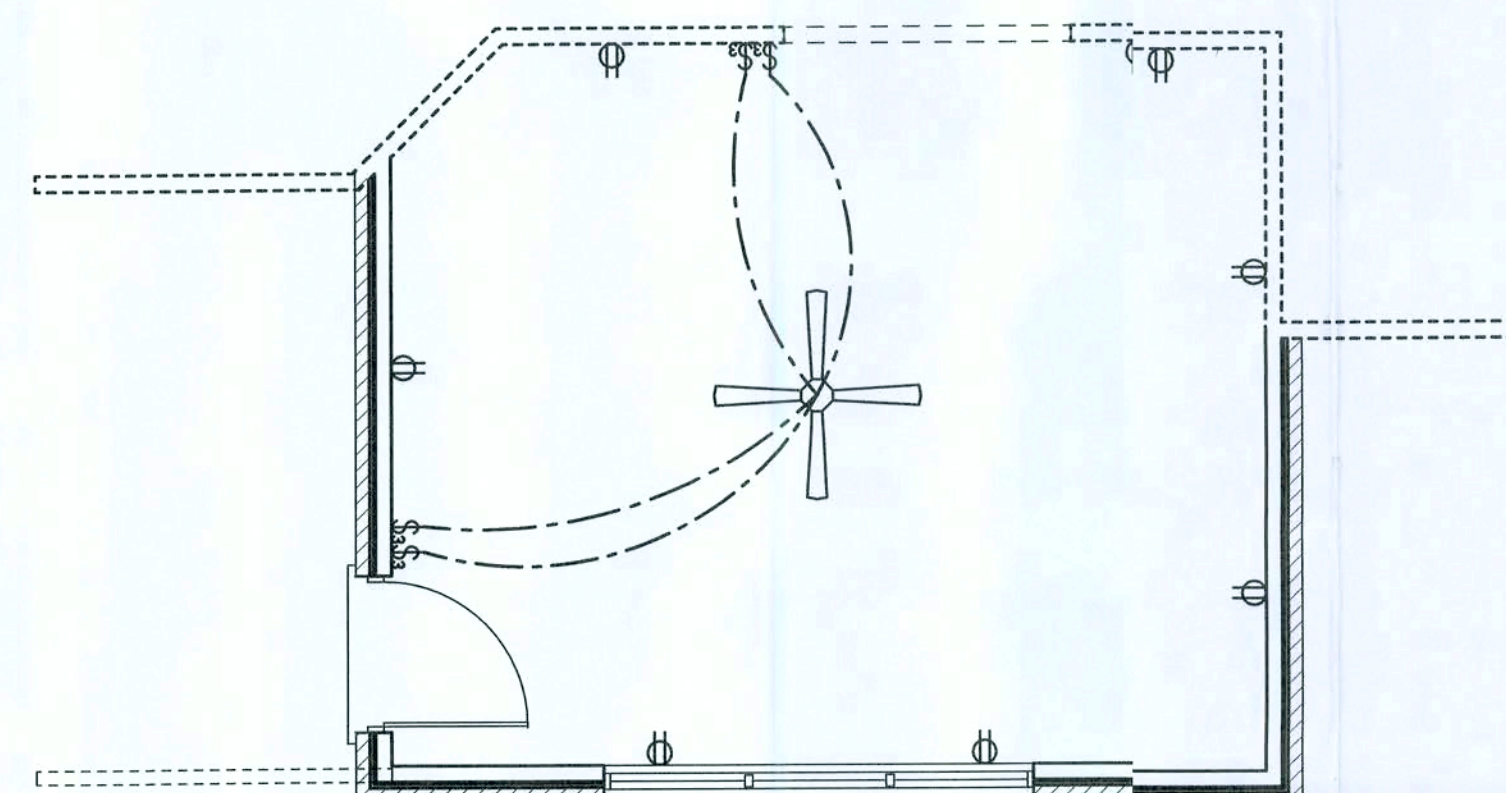
REAR ELEVATION  
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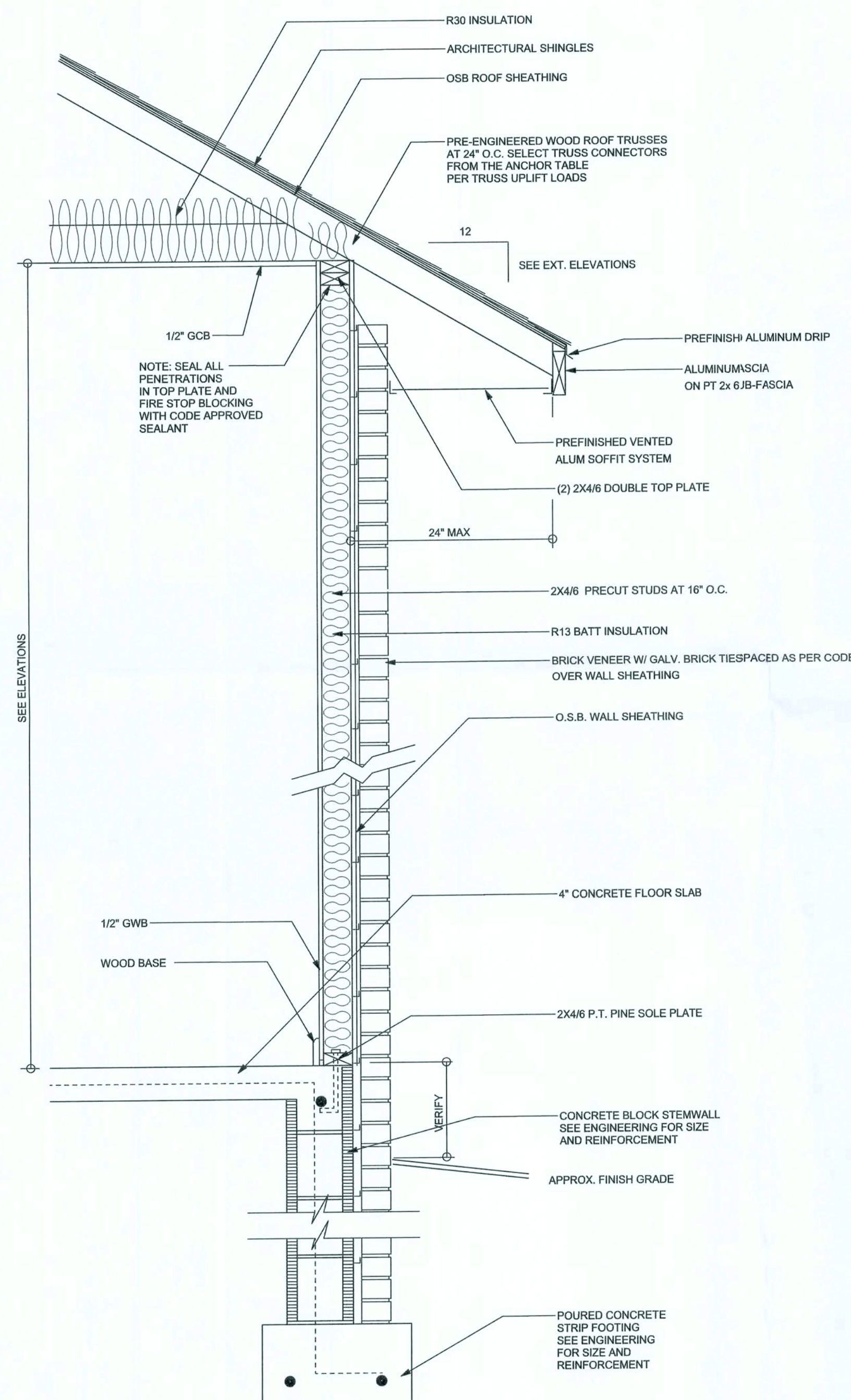
FOUNDATION PLAN (EXISTING)  
SCALE: 1/4" = 1'-0"



FLOOR PLAN  
SCALE: 1/4" = 1'-0"  
BUILDER TO VERIFY ALL DIMENSIONS



ELECTRICAL PLAN  
SCALE: 1/4" = 1'-0"



TYPICAL DESIGN WALL SECTION  
NON - STRUCTURAL DATA  
SCALE: 1" = 1'-0"

## ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD ) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER. AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

## ELECTRICAL LEGEND

	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER

WINDLOAD ENGINEER: Mark Discoway,  
PE No. 33915, P.C. 868, Lake City, FL  
32056, 386-754-819

DIMENSIONS:  
Stated dimension supercede scaled  
dimensions. Refer all questions to  
Mark Discoway, P.E. for resolution.  
Do not proceed without clarification.

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permission and consent of Mark Discoway.

CERTIFICATION: These plans and  
"Windload Engineering", Sheet S-1, attached,  
comply with Florida Building Code Residential  
2004, Section 631.2.1 wind loads, to the best  
of my knowledge.

LIMITATION: This design is valid for one  
building at specified location. In case of conflict,  
structural requirements, scope of work, and  
builder responsibilities on sheet S-1 control.

MARK DISCOWAY  
P.E. 33915

Glenwood King  
Construction

Rod Shields  
Addition

ADDRESS:  
15125 US Hwy 441  
Lake City, Florida 32025

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Lake City, Florida 32056  
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PRINTED DATE:  
January 31, 2008

DRAWN BY: David Discoway  
CHECKED BY:

DESIGNED BY:

FINALS DATE:  
31 / Jan / 08

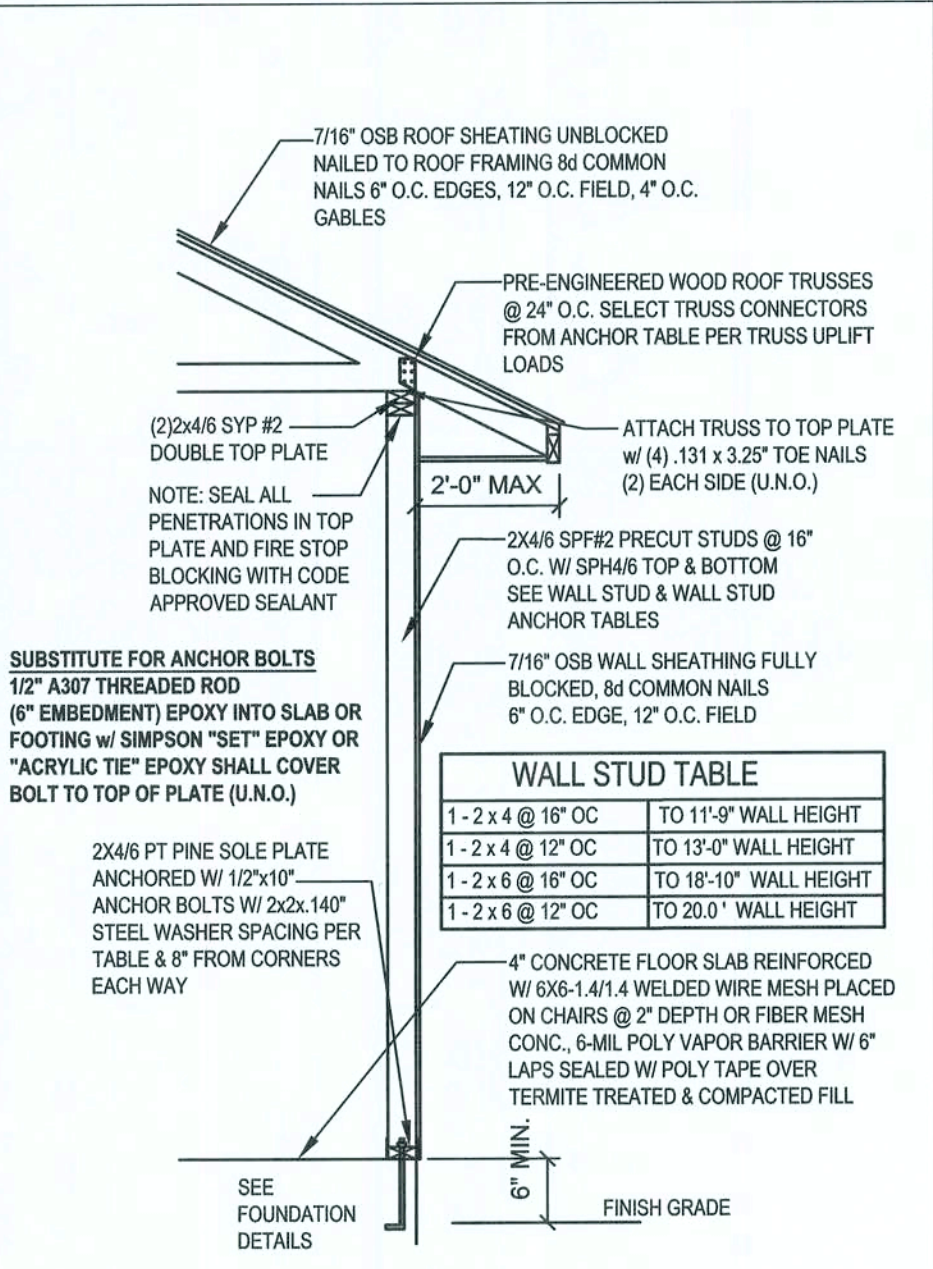
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DRAWING NUMBER

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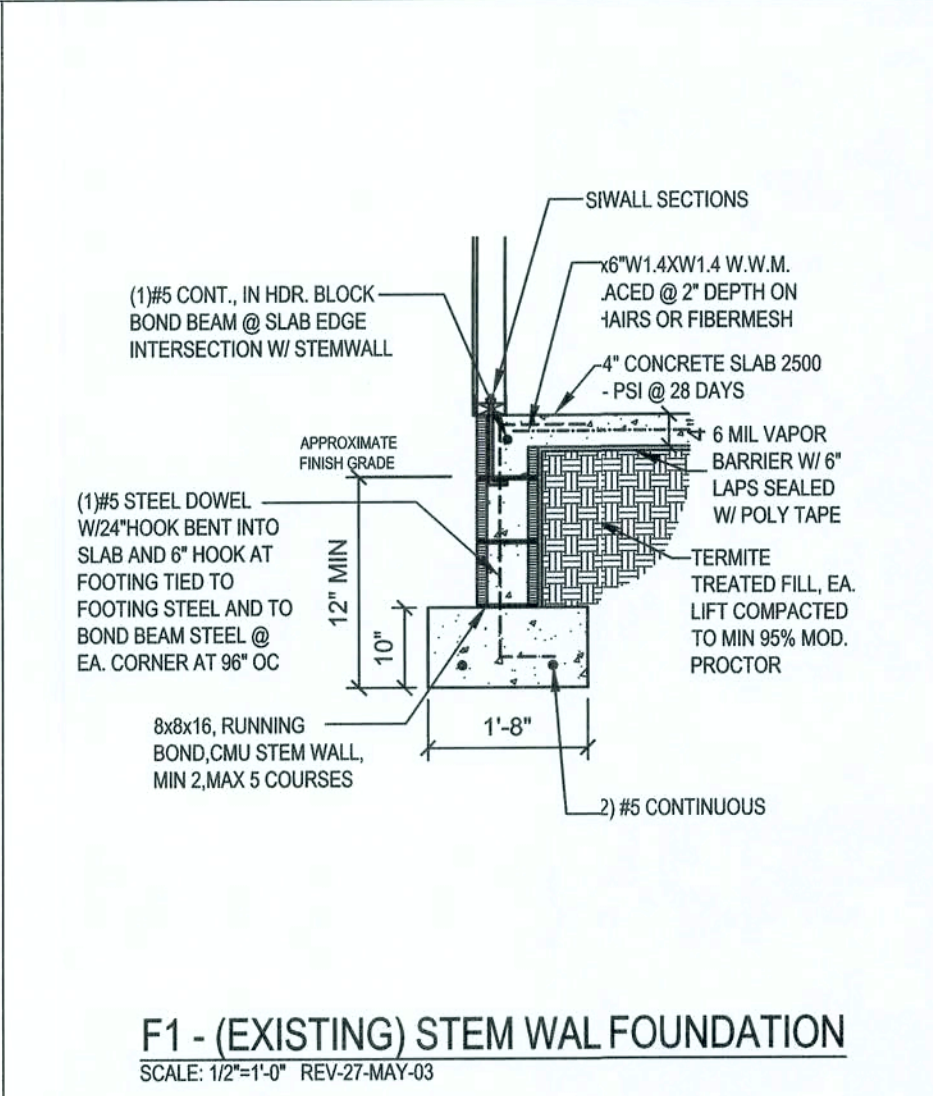
C 2 SHEETS



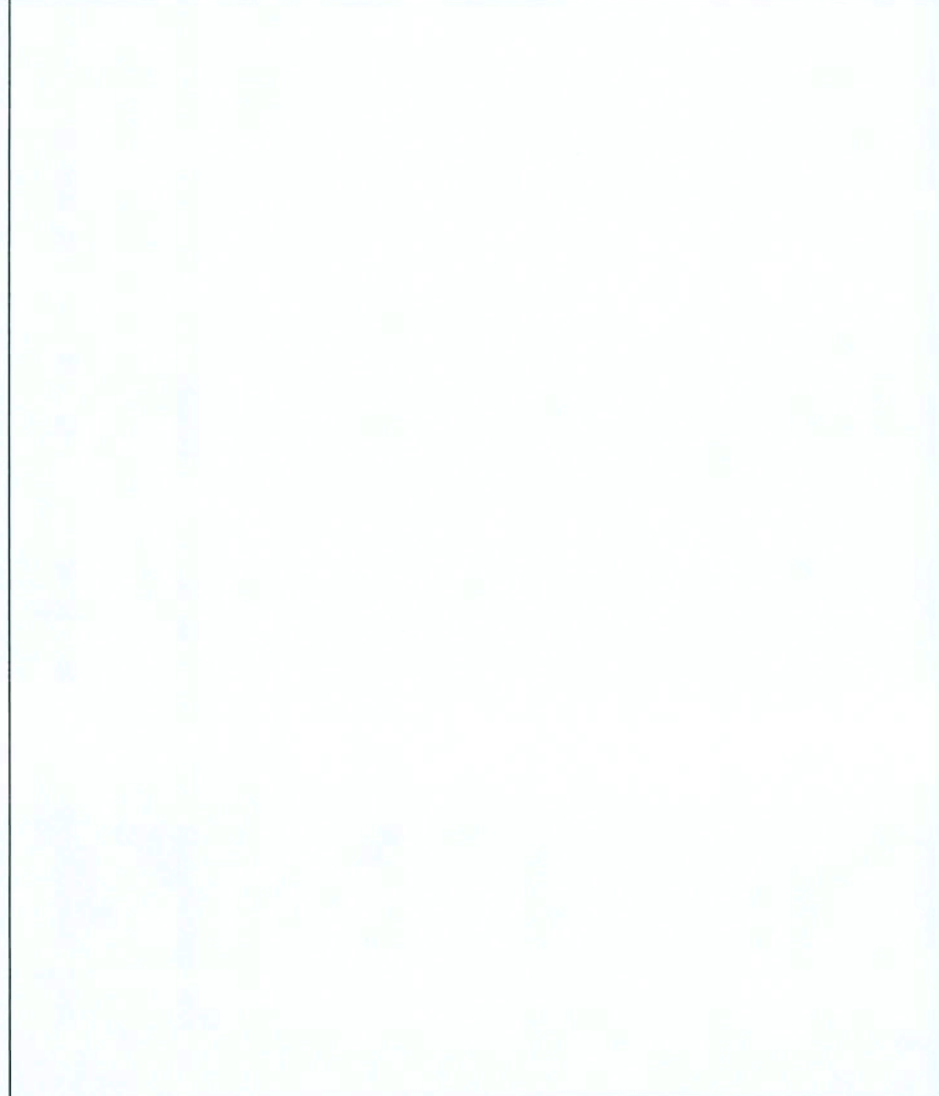


STUD ANCHOR TABLE			
TYPICAL TRUSS UPLIFT & MAX 10'-0" WALL HEIGHT	ANCHOR BOLT SPACING	SPH1 / SPH6 SPACING	N/A
750 LB	48" O.C.	48" O.C.	N/A
950 LB	32" O.C.	32" O.C.	N/A
N/A	N/A	N/A	N/A
1500 LB	24" O.C.	16" O.C.	N/A
2200 LB	LTT01 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 WAILED TO STUD PACK
NOTE: N/A			
NOTE: MINIMUM ANCHOR BOLT SPACING FOR WALLS WITH A HEIGHT GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.C.			

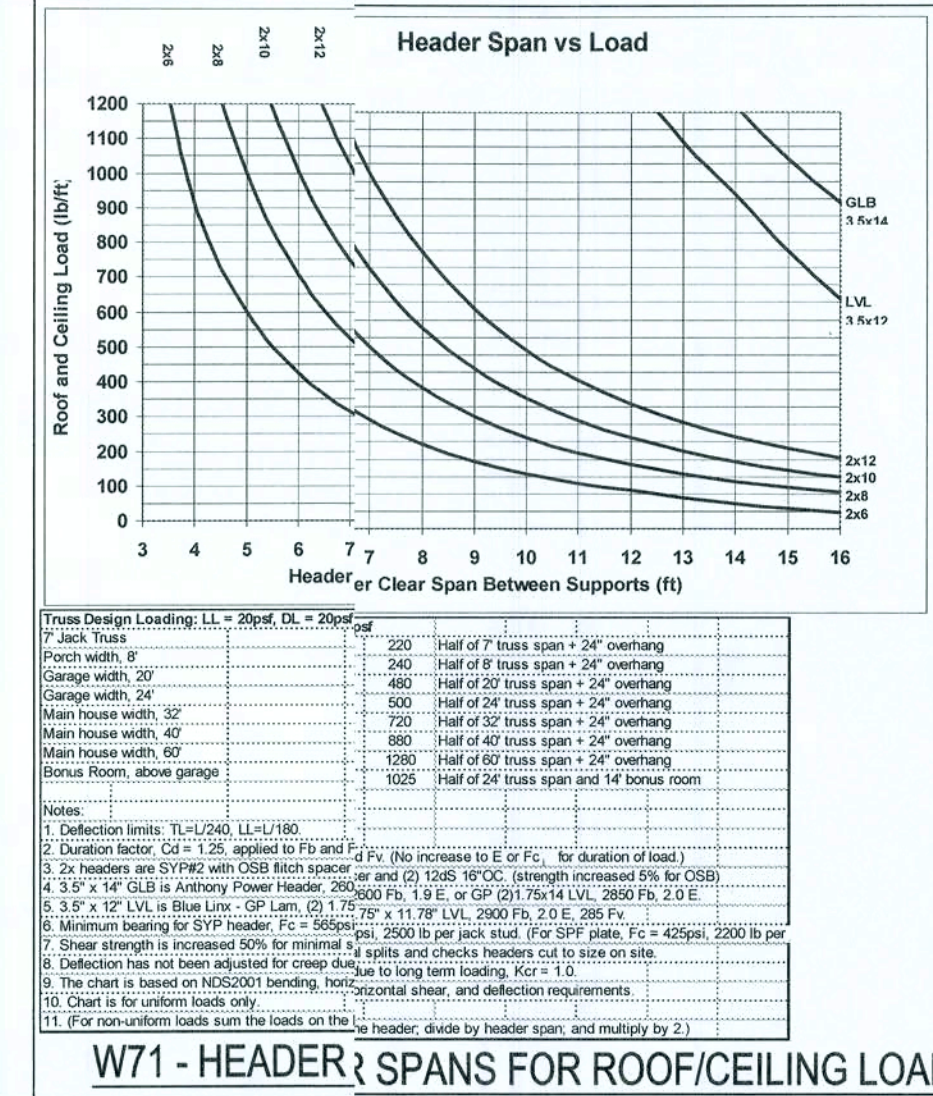
W1 - SINGLE STORY EXT. WALL SECTION  
SCALE: 1/2"=1'-0" REV-01-MAY-06



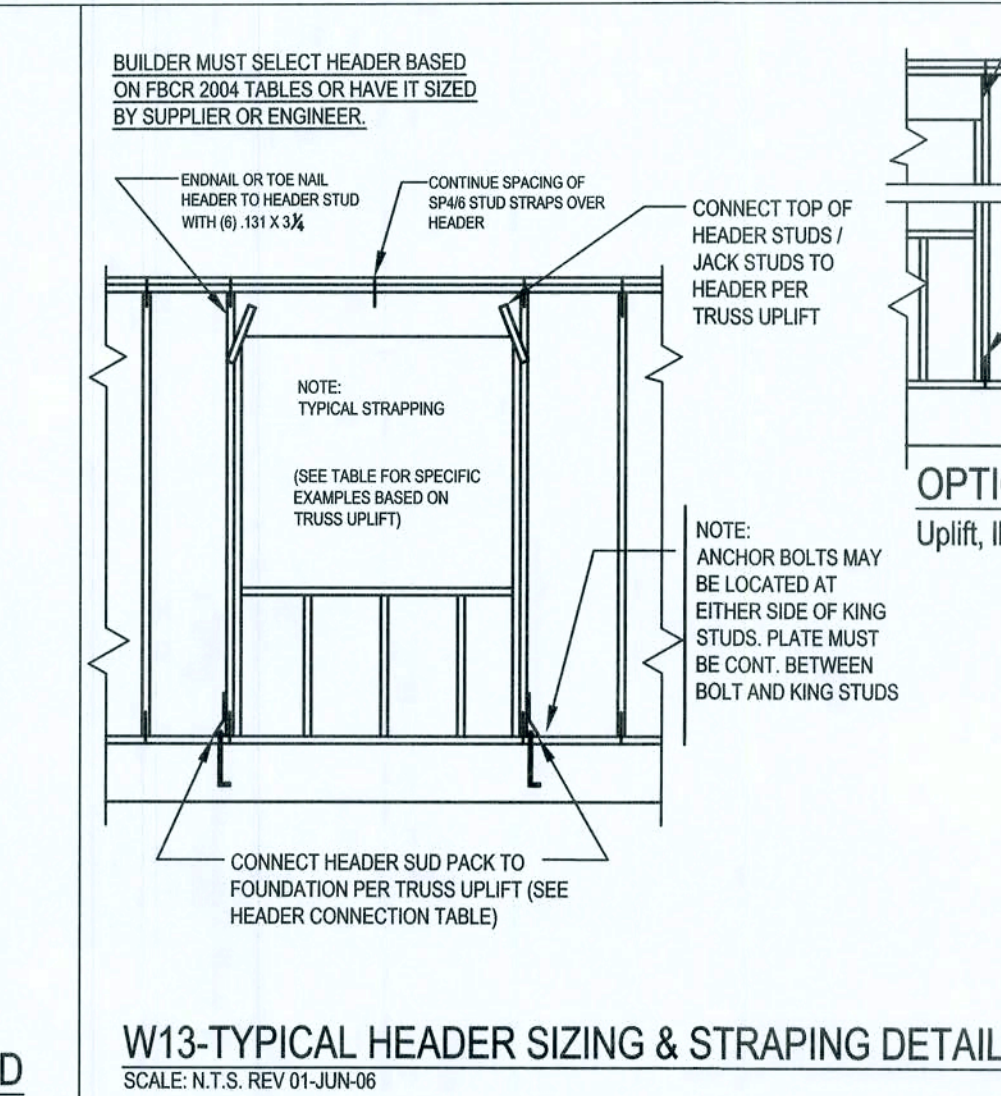
F1 - (EXISTING) STEM WAL FOUNDATION  
SCALE: 1/2"=1'-0" REV-27-MAY-03



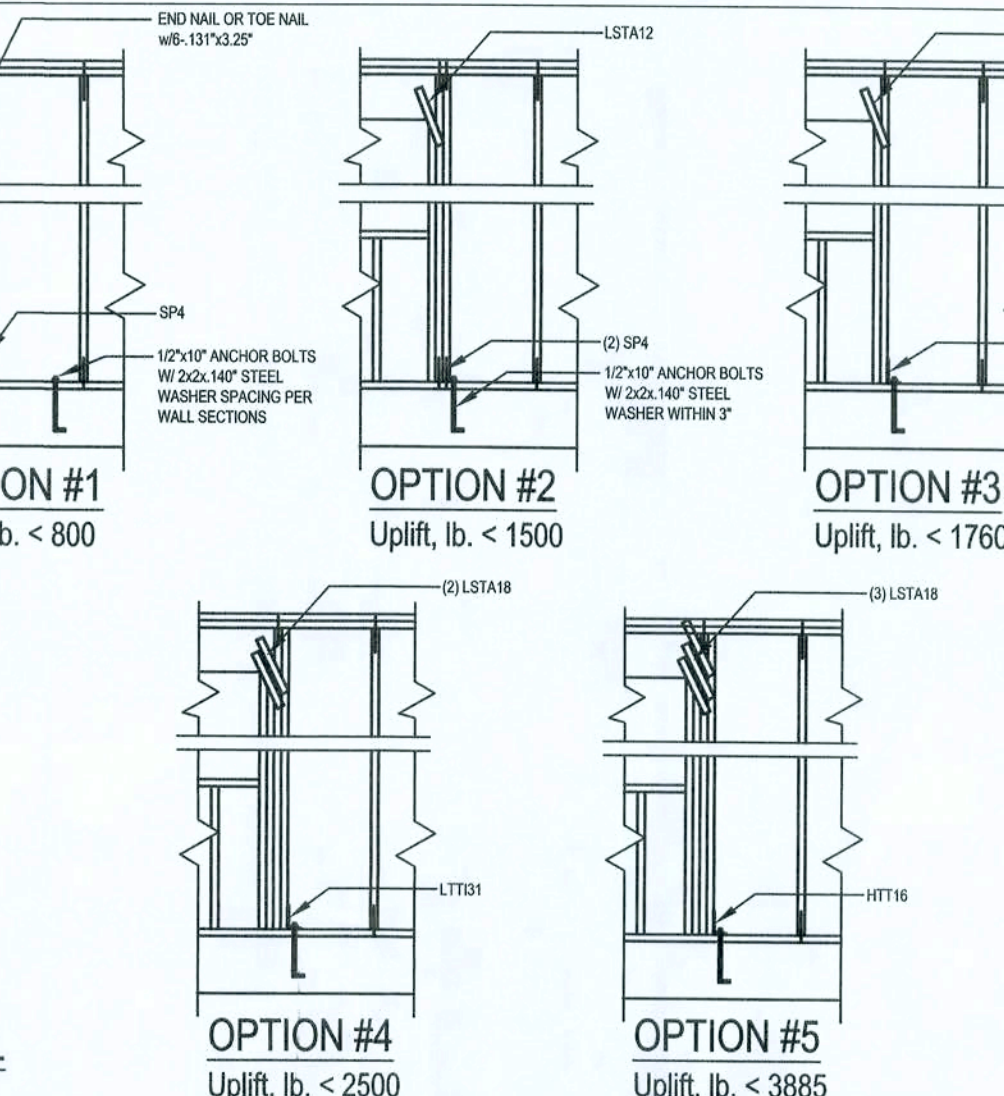
W11 - HEADER SPANS FOR ROOF/CEILING LOAD



W13-TYPICAL HEADER SIZING & STRAPING DETAIL  
SCALE: N.T.S. REV-01-JUN-06



W14-TYPICAL HEADER SIZING & STRAPING DETAIL  
SCALE: N.T.S. REV-01-JUN-06



W15-TYPICAL HEADER SIZING & STRAPING DETAIL  
SCALE: N.T.S. REV-01-JUN-06

Header Span vs Load			
Roof and Ceiling Load (lb/ft)	Header Clear Span Between Supports (ft)	Header Size	Header Type
1200	3	4x12	1x10
1100	4	4x12	1x10
1000	5	4x12	1x10
900	6	4x12	1x10
800	7	4x12	1x10
700	8	4x12	1x10
600	9	4x12	1x10
500	10	4x12	1x10
400	11	4x12	1x10
300	12	4x12	1x10
200	13	4x12	1x10
100	14	4x12	1x10
0	15	4x12	1x10
0	16	4x12	1x10

W15-TYPICAL HEADER SIZING & STRAPING DETAIL  
SCALE: N.T.S. REV-01-JUN-06

## N2-GENERAL NOTES:

**FOUNDATION:** FOR POINT LOADS GRATER THAN 5000 LB OR REPETITIVE TRUSS LOADS GRATER THAN 2000 LB PER TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0" X 1' SQ. FT. FOR EVERY 1000 LB OF BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAY: SHALL BE  $F_c = 3000$  PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICEABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE DESIGNER, OWNER OR HIS REPRESENTATIVE.

**WELDED WIRE REINFORCED SLAB:** 6" x 6" W14 X W14, FB = 56KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THESLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 2'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF S&B PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI. ALL LAPS SLICES 4" DB (25" FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-95 WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 23 INCHES.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICE LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; ID.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

N5 - TRUSS UPLIFT CONNECTOR TABLE			
Uplift	SPF	SYP	Truss Connector
320	455	H3	4-8d
245	350	H5A	4-8d
535	600	H2.5A	5-8d
620	720	H10	6-10dx1 1/2"
850	990	LTS12	8-8dx1 1/2"
1245	1450	HTS20	10-10d or 12-10dx1 1/2"
1265	1470	H16, HTS16-2	10-10dx1 1/2"
1765	2050	LGT2	14-10d Sinker
3655	4200	MGT	3/4" Thd. Rod
SPF	SYP	Strap Co.	connector
760	885	SP4	6-10dx1 1/2"
865	1005	CS20	9-8d or 7-10d
1085	1265	LSTA18-24	7-10d
1170	1360	SPH4	12-10dx1 1/2"
1420	1650	CS16	14-8d or 11-10d
SPF	SYP	Column	in Anchor
1160	1350	LTT19	To Foundation
1985	2310	LTT131	To Column / Truss
2385	2775	HD2A	3/4" x 16" AB
3590	4175	HTT16	3/4" x 16" AB
1975	2300	ABU66	3/4" x 16" AB

Studs Supporting Trusses: The builder is responsible for gravity loads, but you should not use studs that exceed truss bearing location for each 2000 lb of reaction. Check the minimum bearing requirements of the truss and top plate (SPF, FC=425psi=2250psi).

N4-WIND LOAD DESIGN DATA			
Wind Speed	Wind Exposure	Wind Importance Factor	Building Category
110 MPH	B	1.0	II
Internal pressure Coefficient	N/A (Enclosed)		
Building not in the high velocity hurricane zone			
Building not in the wind-borne debris region			
Mean Roof Height	< 30 ft		
Roof Angle	10-45 degrees		
Components And Cladding Wind Pressures (FBCR Table R301.2)			
Zone	Effective Wind Area (ft2)		
4	21.8	23.6	18.5
5	21.8	29.1	18.5
Total Shear Wall Segments			
Actual	17.0	8.9	
Required	10.0	8.0	
All exterior walls are type II shear walls			
Actual SHEAR WALL length is the total of all wall segments with full height sheathing and width to height ratio greater than 1: 3.5 (plus special shear wall segments if noted.) REQUIRED SHEAR WALL length is from WFCM-2001, table 5.7.6 & 5.7.7B with table 5.7.7E adjustment for type II shear wall (or equivalent calculation)			

WINDLOAD ENGINEER: Mark Disoway, P.E. No.53915

N3-WINDLOAD ENGINEER'S SCOPE OF WORK:			
1. Determine header size from FBCR 2004, Tables R502.5(1) or R502.5(2)			
2. Use supplier published data or Southern pine span tables			
3. For engineered lumber beams have suppliers engineer size them			
4. Locate jack studs from FBCR 2004, Tables R502.5(1) or R502.5(2)			
5. Use one jack stud for every 3000 lb vertical load			
6. Total king plus jack studs = studs needed to be there if no opening was there			
7. Calculate the uplift at each end of the header by summing the moments of all truss uplifts and dividing by the length of the header			
8. Select header connections from table below or mfg. catalogo connect header to stud (top connection) and stud to foundation (bottom connection)			
Option #	Uplift, lb.	Top Connector	Bottom Connector
#1	< 800	End nail or toe nail w/5-13"x3.25"	SP4, -10dx1 1/2"
#2	< 1500	LSTA12, 10-10d	755 (2) SR, 6-10dx1 1/2"
#3	< 1750	LSTA18, 14-10d	1055 (2) SR, 6-10dx1 1/2"
#4	< 2500	(2) LSTA18, 14-10d	2110 (2) LTT01, 18-10dx1 1/2"
#5	< 3885	(3) LSTA18, 14-10d	3480 (HTT1, 18-16dx1 1/2")

Uplift greater than 3885 lb requires engineering design

REV-27-JUL-04

WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

Mark Disoway P.E.

POB 868, Lake City, FL 32056 Phone: (386) 754-5419

Fax: (386) 269-4871 Email: windloadengineer@bellsouth.net

LOCATION: 15129 S US Hwy 441 Lake City, Florida 32025

Rod Shields Addition

Builder: Glenwood King Construcion

Designer:

Approved: PLPER53915

Revisions:

1/31/05

31Jan08

Sheet S-1 of 2 Sheets

Windload Engineering

Job # 80-225

REV-01-JUN-06