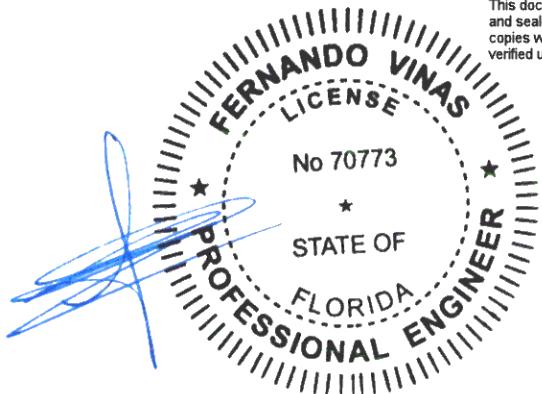


This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature must be verified using the original electronic version.



08/19/2022

COA#0-278
Florida Certificate of Product Approval #FL1999

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-6857 repair
Job Description: Sellers Residence (LIVE DORMER)	
Address: LAKE CITY, FL	

Job Engineering Criteria:	
Design Code: FBC 2017 RES	IntelliVIEW Version: 22.01.01A JRef #: 1XI72150007
Wind Standard: ASCE 7-10 Building Type: Closed	Wind Speed (mph): 130 Design Loading (psf): 40.00

This package contains general notes pages, 5 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	231.22.0925.49340	A08 rep 2
3	231.22.0926.08340	A08 rep1
5	231.22.0932.45980	A01 rep
7	BRCLBSUB0119	
9	160TL	

Item	Drawing Number	Truss
2	231.22.0930.22457	A09 rep
4	231.22.0929.24087	A10 rep
6	A14030ENC101014	
8	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

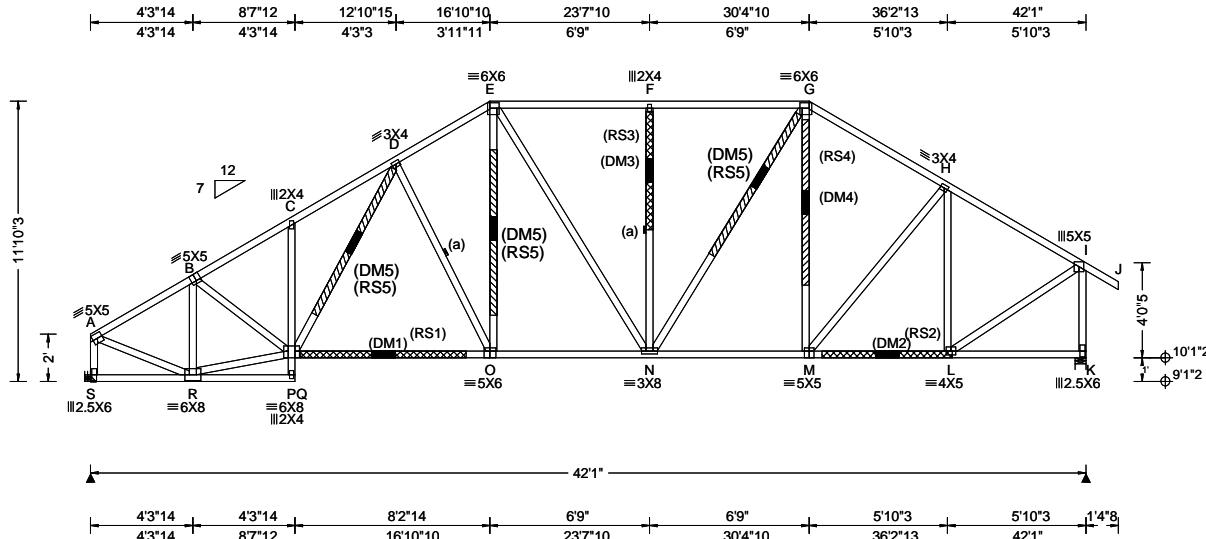
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpininst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 24838 FROM: CDM Page 1 of 2	COMM Ply: 1 Qty: 2	Job Number: 22-6857 repair Sellers Residence (LIVE DORMER) Truss Label: A08 rep 2	Cust: R 215 JRef:1X172150007 T9 DrwNo: 231.22.0925.49340 GA / FV 08/19/2022
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL:	20.00	Wind Std:	ASCE 7-10	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/ R-	/ Rh	Gravity	Non-Gravity	
TCDL:	10.00	Speed:	130 mph	Pf: NA	Ce: NA	VERT(LL): 0.139 O 999 240	VERT(CL): 0.256 O 999 180	S	1921	/ -	/ -	/1043	/287	/368
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA	HORZ(LL): 0.068 K - -	HORZ(TL): 0.125 K - -	K	2121	/ -	/ -	/1050	/315	/ -
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA	Creep Factor: 2.0	Max TC CSI: 0.633	Wind reactions based on MWFRS						
Des Ld:	40.00	EXP: C	Kzt: NA	Building Code:	FBC 2017 RES	Max BC CSI: 0.876	Max Web CSI: 0.761	S Brg Wid = - Min Req = -						
NCBCLL:	10.00	Mean Height:	17.99 ft	TPI Std:	2014	Max TC CSI: 0.633	Max BC CSI: 0.876	K Brg Wid = 5.5 Min Req = 2.5 (Truss)						
Soffit:	2.00	Load Duration:	1.25	Rep Fac:	Yes	Max Web CSI: 0.761	Bearing K is a rigid surface.							
Load Duration:	1.25	Spacing:	24.0 "	FT/RT:20(0)/10(0)	Plate Type(s):	Members not listed have forces less than 375#								
GCpi: 0.18	Wind Duration:	1.60	WAVE	Maximum Top Chord Forces Per Ply (lbs)										
				VIEW Ver: 22.01.01A.0520.12		Chords		Chords		Chords		Tens. Comp.		

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.



COA#0-278

Florida Certificate of Product Approval #FL1999

08/19/2022

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 24838	COMM	Ply: 1	Job Number: 22-6857 repair	Cust: R 215 JRef:1X172150007 T9
FROM: CDM		Qty: 2	Sellers Residence (LIVE DORMER)	DrwNo: 231.22.0925.49340
Page 2 of 2			Truss Label: A08 rep 2	GA / FV 08/19/2022

Additional Notes

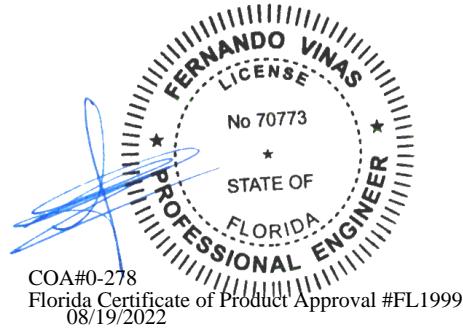
(DM1) Truss member damaged in the indicated area.
 Damage length = 1-0-0 at 12-4-10 from left end. (RS1)
 2x4 x 7-0-0 SP #2 scabs. Attach one scab to each
 face, centered on damage, with 1 row of 0.131"x3",
 min. nails at 3" oc. Offset rows from opposing faces to
 prevent splits.

(DM2) Truss member damaged in the indicated area.
 Damage length = 1-0-0 at 33-8-3 from left end. (RS2)
 2x4 x 5-6-0 SP #2 scabs. Attach one scab to each
 face, centered on damage, with 1 row of 0.131"x3",
 min. nails at 3" oc. Offset rows from opposing faces to
 prevent splits.

(DM3) Truss member damaged in the indicated area.
 Damage length = 1-0-0 at 23-5-14 from left end. (RS3)
 2x4 x 5-0-0 SP #3 scabs. Attach one scab to each
 face, centered on damage, with 1 row of 0.131"x3",
 min. nails at 3" oc. Offset rows from opposing faces to
 prevent splits.

(DM4) Truss member damaged in the indicated area.
 Damage length = 1-0-0 at 30-1-2 from left end. (RS4)
 2x4 x 7-0-0 SP #3 scab. Attach scab to one face,
 centered on damage, with 1 row of 0.131"x3", min.
 nails at 3" oc.

(DM5) Truss member damaged in the indicated area.
 Damage length = 1-0-0. (RS5)
 2x4 x 7-0-0 SP #2 scabs. Attach one scab to each
 face, centered on damage, with 1 row of 0.131"x3",
 min. nails at 3" oc. Offset rows from opposing faces to
 prevent splits.



****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

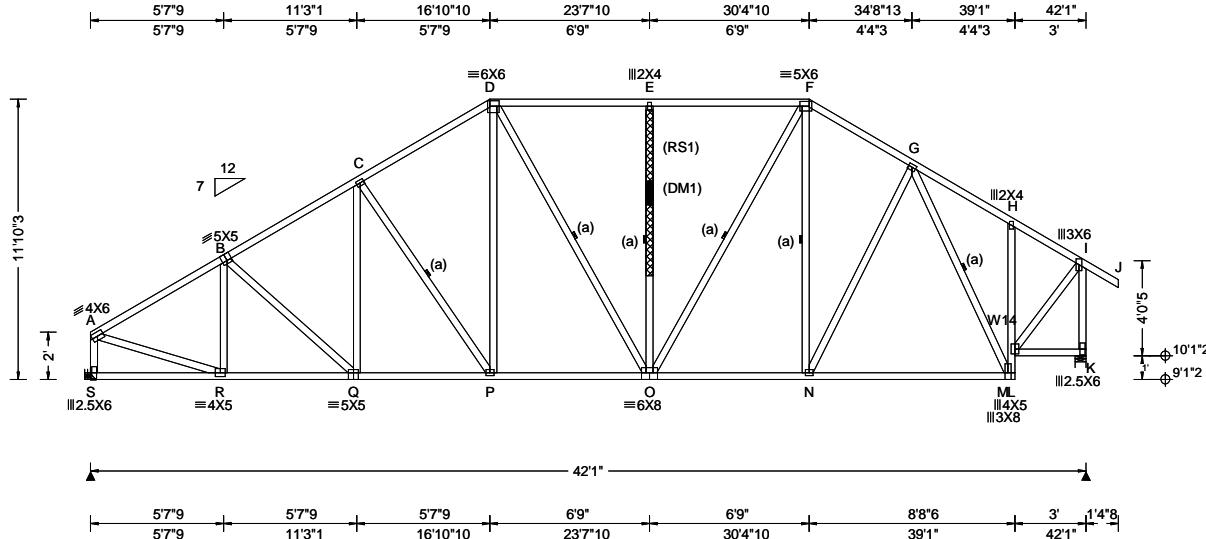
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SEQN: 24836 FROM: CDM	COMM Qty: 1	Job Number: 22-6857 repair Sellers Residence (LIVE DORMER) Truss Label: A09 rep	Cust: R 215 DrwNo: 231.22.0930.22457 GA / FV 08/19/2022	JRef: 1XI72150007 T12
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Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)											
			Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity								
		Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.127 E 999 240	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL					
TCLL: 20.00		Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.262 E 999 180	S	1748	/ -	/ -	/1037	/304	/273					
TCDL: 10.00		Risk Category: II	Snow Duration: NA	HORZ(LL): 0.161 K - -	K	1844	/ -	/ -	/1034	/338	/ -					
BCLL: 0.00		EXP: C Kzt: NA		HORZ(TL): 0.333 K - -	Wind reactions based on MWFRS											
BCDL: 10.00		Mean Height: 16.02 ft	Building Code:	Creep Factor: 2.0	S	Brg Wid	= -	Min Req	= -							
Des Ld: 40.00		TCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.590	K	Brg Wid	= 5.5	Min Req	= 2.2 (Truss)							
NCBLL: 10.00		BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.669	Bearing K is a rigid surface.											
Soffit: 2.00		MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.909	Members not listed have forces less than 375#											
Load Duration: 1.25		C&C Dist a: 4.21 ft	FT/RT:20(0/10)(0)		Maximum Top Chord Forces Per Ply (lbs)											
Spacing: 24.0 "		Loc. from endwall: not in 13.00 ft	Plate Type(s):		Chords	Tens.Comp.		Chords	Tens. Comp.							
		GCpi: 0.18	WAVE	VIEW Ver: 22.01.01A.0520.12	A - B	495	- 2099	E - F	562	- 1595						
		Wind Duration: 1.60			C - D	579	- 2144	G - H	555	- 1569						

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W14 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member

Plating Notes

All plates are 3X4 except as noted.

Hangers / Ties

(I) Hanger Support Required by others

Burlins

In lieu of structural panels use purlins to

TC @ 24

Wind

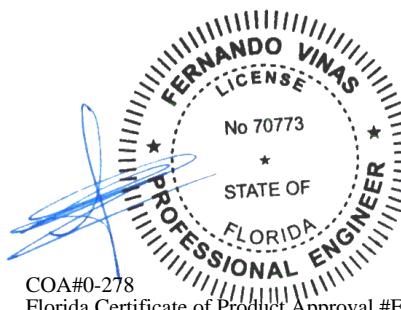
memb

End verticals not exposed to wind pressure.

Additional Notes

(DMT) Hertz model
Damage length =

2x4 x 7-0-0 SP #3 scabs. Attach one scab to each face, centered on damage, with 1 row of 0.131" x 3", min. nails at 3" oc. Offset rows from opposing faces to prevent splits.



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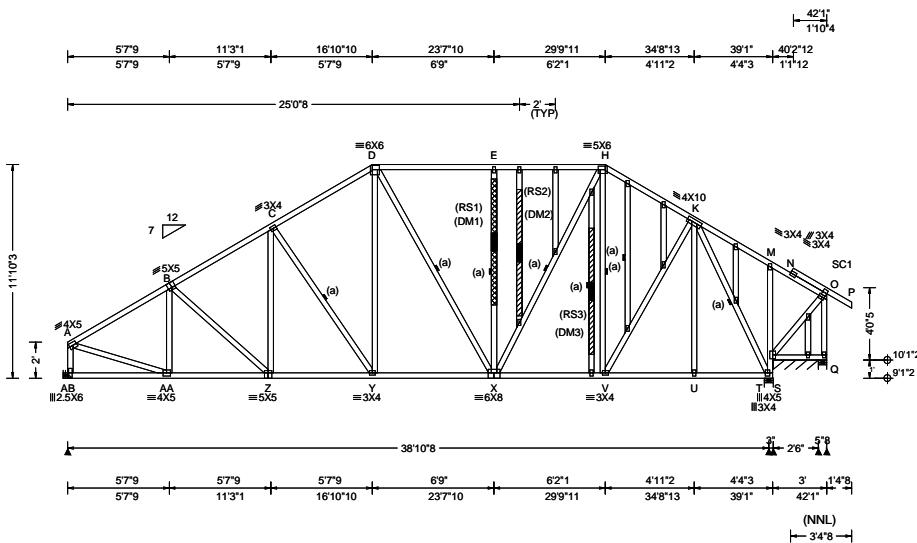
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SEQN: 24832	GABL	Ply: 1	Job Number: 22-6857 repair	Cust: R 215 JRef:1X172150007 T55
FROM: CDM		Qty: 1	Sellers Residence (LIVE DORMER)	DrwNo: 231.22.0929.24087
Page 1 of 2			Truss Label: A10 rep	GA / FV 08/19/2022



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
TCLL:	20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity	Non-Gravity					
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.078 G 999 240	Loc	R+	/R-	/Rh	/Rw	/U	/RL
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.160 G 999 180	AB	1607	/-	/-	/1015	/195	/322
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.031 T - -	T	1796	/-	/-	/1077	/67	/-
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.064 T - -	S*	47	/-	/-	/35	/-	/-
Mean Height: 17.99 ft		Building Code:	Creep Factor: 2.0	Creep Factor: 2.0	Q	172	/-	/-	/131	/57	/-
NCBCLL: 10.00		FBC 2017 RES	Max TC CSI: 0.529	Wind reactions based on MWFRS							
Soffit: 2.00		TPI Std: 2014	Max BC CSI: 0.519	AB Brg Wid = - Min Req = -							
Load Duration: 1.25		Rep Fac: Yes	Max Web CSI: 0.698	T Brg Wid = 6.0 Min Req = 2.1 (Truss)							
Spacing: 24.0 "		FT/RT:20(0)/10(0)		S Brg Wid = 30.0 Min Req = -							
		Plate Type(s):		Q Brg Wid = 5.5 Min Req = 1.5 (Truss)							
		WAVE		Bearings T, S, & Q are a rigid surface.							
				Members not listed have forces less than 375#							
Maximum Top Chord Forces Per Ply (lbs)											
Chords		Tens.Comp.		Chords		Tens. Comp.					
A - B		247	-1915	D - E		317	-1302				
B - C		285	-1918	E - H		317	-1302				
C - D		278	-1643	H - K		163	-1187				

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Hangers / Ties

Hanger support required, by others.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

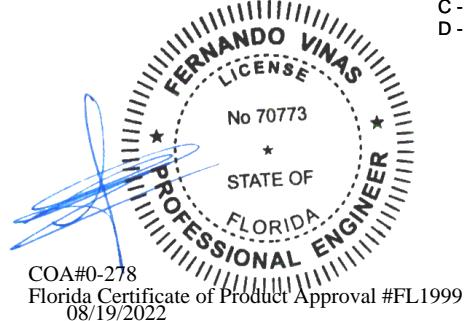
Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.



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SEQN: 24832	GABL	Ply: 1	Job Number: 22-6857 repair	Cust: R 215 JRef:1X172150007 T55
FROM: CDM		Qty: 1	Sellers Residence (LIVE DORMER)	DrwNo: 231.22.0929.24087
Page 2 of 2			Truss Label: A10 rep	GA / FV 08/19/2022

Additional Notes

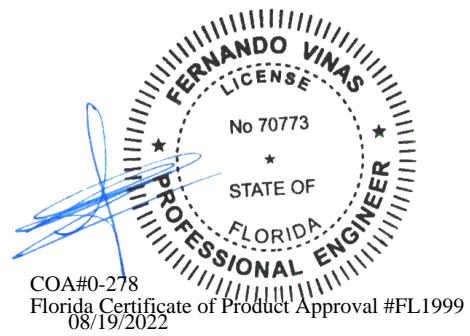
See DWGS A14030ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

(DM1) Truss member damaged in the indicated area. Damage length = 1-0-0 at 23-5-14 from left end. (RS1) 2x4 x 7-0-0 SP #3 scabs. Attach one scab to each face, centered on damage, with 1 row of 0.131"x3", min. nails at 4" oc. Offset rows from opposing faces to prevent splits.

(DM2) Truss member damaged in the indicated area. Damage length = 1-0-0 at 24-10-12 from left end. (RS2) 2x4 x 7-0-0 SP #3 scab. Attach scab to one face, centered on damage, with 1 row of 0.131"x3", min. nails at 4" oc.

(DM3) Truss member damaged in the indicated area. Damage length = 1-0-0 at 28-10-12 from left end. (RS3) 2x4 x 7-0-0 SP #3 scab. Attach scab to one face, centered on damage, with 1 row of 0.131"x3", min. nails at 4" oc.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

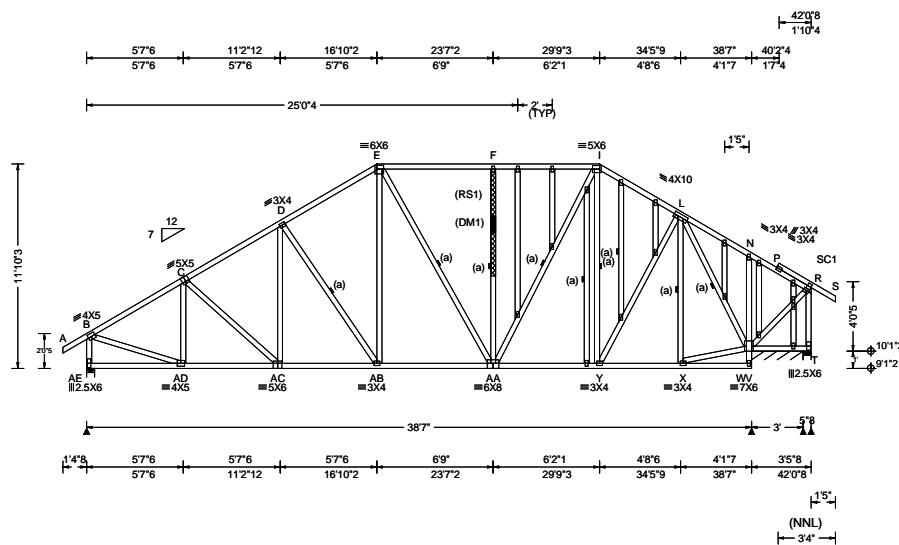
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SEQN: 24834	GABL	Ply: 1	Job Number: 22-6857 repair	Cust: R 215 JRef:1X172150007 T56
FROM: CDM		Qty: 1	Sellers Residence (LIVE DORMER) Truss Label: A01 rep	DrwNo: 231.22.0932.45980 GA / FV 08/19/2022



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF				
TCLL:	20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity	Non-Gravity			
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.074 H 999 240	Loc R+ / R-	/ Rh			
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.152 H 999 180	AE	1669	/ -	/ -	/ 1077 / 211 / 336
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.027 X - -	C*	697	/ -	/ -	/ 420 / 40 / -
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.056 X - -	T	33	/ -164	/ -	/ 67 / 111 / -
Mean Height: 17.60 ft				Creep Factor: 2.0					Wind reactions based on MWFRS
NCBCLL: 10.00		TCDL: 5.0 psf	Building Code:	Max TC CSI: 0.493	AE	Brg Wid = 5.5	Min Req = 2.0 (Truss)		
Soffit: 2.00		BCDL: 5.0 psf	FBC 2017 RES	Max BC CSI: 0.510	V	Brg Wid = 36.0	Min Req = -		
Load Duration: 1.25		Load Duration: h/2 to h	TPI Std: 2014	Max Web CSI: 0.602	T	Brg Wid = 5.5	Min Req = 1.5 (Truss)		
Spacing: 24.0 "		C&C Dist a: 4.20 ft	Rep Fac: Yes						Bearings AE, V, & T are a rigid surface.
Loc. from endwall: not in 13.00 ft		Loc. from endwall: not in 13.00 ft	FT/RT:20(0)/10(0)						Members not listed have forces less than 375#
GCpi: 0.18		Wind Duration: 1.60	Plate Type(s):						Maximum Top Chord Forces Per Ply (lbs)
			WAVE						Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Additional Notes

See DWGS A14030ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

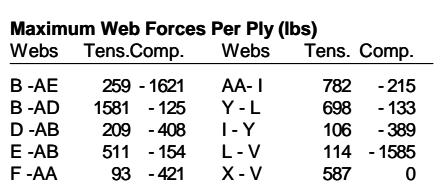
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

(DS1) Truss member damaged in the indicated area. Damage length = 1-0-0 at 23-5-6 from left end. (DS1) 2x4 x 6-0-0 SP #3 scabs. Attach one scab to each face, centered on damage, with 1 row of 0.131" x 3", min. nails at 4" oc. Offset rows from opposing faces to prevent splits.

▲ Maximum Reactions (lbs), or *=PLF					
Loc	R+ / R-	/ Rh	Gravity	Non-Gravity	
AE	1669	/ -	/ -	/ 1077 / 211 / 336	
V*	697	/ -	/ -	/ 420 / 40 / -	
T	33	/ -164	/ -	/ 67 / 111 / -	
					Wind reactions based on MWFRS
					AE Brg Wid = 5.5 Min Req = 2.0 (Truss)
					V Brg Wid = 36.0 Min Req = -
					T Brg Wid = 5.5 Min Req = 1.5 (Truss)
					Bearings AE, V, & T are a rigid surface.
					Members not listed have forces less than 375#
Maximum Top Chord Forces Per Ply (lbs)					
Chords	Tens.Comp.	Chords	Tens. Comp.		
B - C	239 - 1853	E - F	302 - 1235		
C - D	273 - 1854	F - I	302 - 1235		
D - E	265 - 1583	I - L	143 - 1088		

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
AE-AD	240 - 391	AB-AA	1293 - 225
AD-AC	1545 - 441	AA-Y	882 - 61
AC-AB	1519 - 341	Y - X	557 0

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
B - AE	259 - 1621	AA - I	782 - 215
B - AD	1581 - 125	Y - L	698 - 133
D - AB	209 - 408	I - Y	106 - 389
E - AB	511 - 154	L - V	114 - 1585
F - AA	93 - 421	X - V	587 0



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Gable Stud Reinforcement Detail

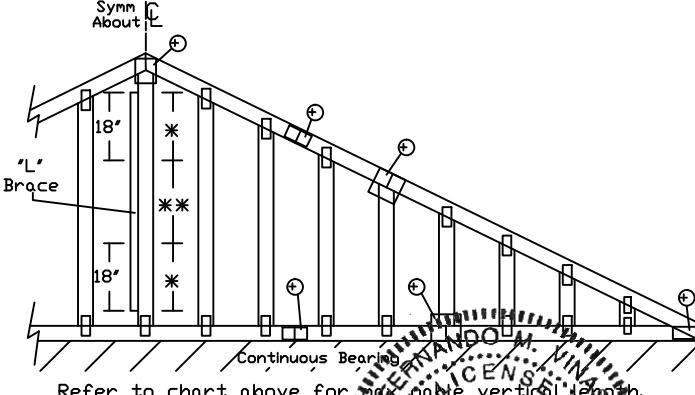
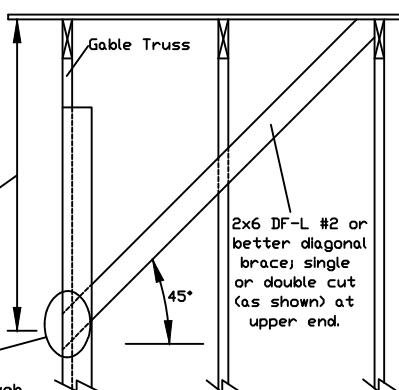
ASCE 7-10: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Length	2x4 Gable Vertical Spacing	Brace Species	Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
					Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF HF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	
		#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	
		Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"	
		Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"	
	SP DFL	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"	
		#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"	
		#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"	
		Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"	
	SPF HF	Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"	
		#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
16" O.C.	SP DFL	Standard	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"	
		#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
	SPF HF	Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"	
		#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP DFL	Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	
		Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	



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Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir		Hem-Fir
#1 / #2	Standard	#2 Stud
#3	Stud	#3 Standard

Group B:

Douglas Fir-Larch		Southern Pine***
#3	Stud	#3 Standard

Group B:

Douglas Fir-Larch		Southern Pine***
#1		#1
#2		#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
Wind Load deflection criterion is L/240.

Provide uplift connections for 100 psf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach "L" braces with 10d (0.128" x 3.0" min) nails.

* For (1) "L" brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.

** For (2) "L" braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

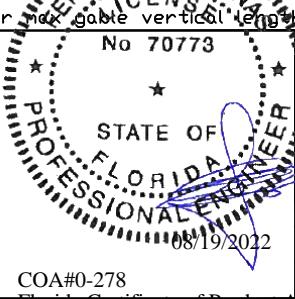
REF ASCE7-10-GAB14030

DATE 10/01/14

DRWG A14030ENC101014

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"



Florida Certificate of Product Approval #FL1999

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

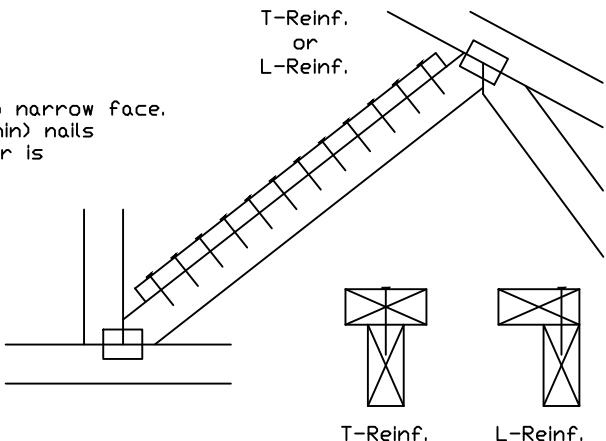
(*) Center scab on wide face of web. Apply (1) scab to each face of web.

T-Reinforcement

OR

L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128" x 3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



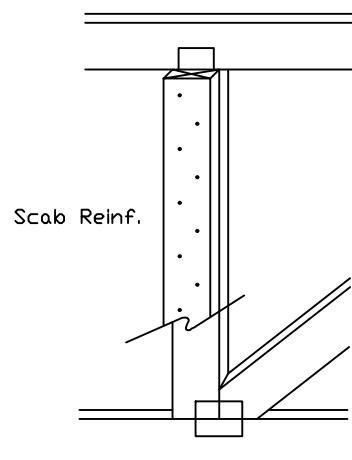
T-Reinf.
or
L-Reinf.

T-Reinf.

L-Reinf.

Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128" x 3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinf.



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No 70773

STATE OF

FLORIDA

COA#0-278

08/19/2022

Florida Certificate of Product Approval #FL1999

PSF

REF CLR Subst.

PSF

DATE 01/02/19

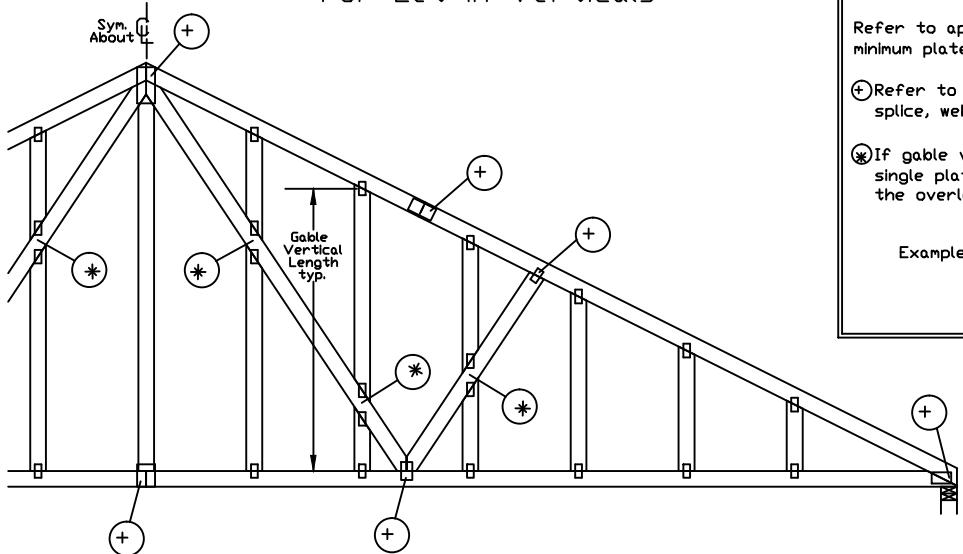
PSF

DRWG BRCLBSUB0119

PSF

Gable Detail

For Let-in Verticals



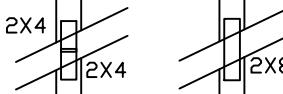
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊕ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



'T' Reinforcement Attachment Detail

'T' Reinforcing Member

Toe-nail

- Or -

'T' Reinforcing Member

End-nail

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148" x 3", min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148" x 3", min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

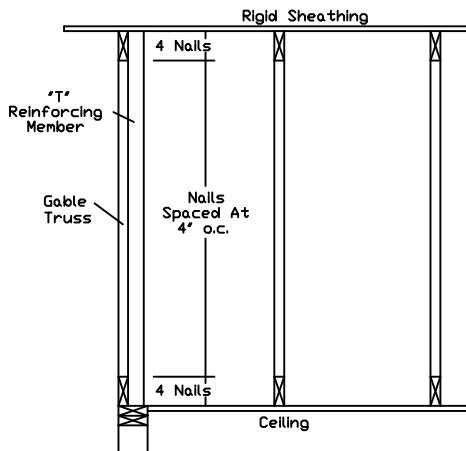
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



WARNING!! READ AND FOLLOW ALL NOTES ON THIS DRAWING!!
IMPORTANT!! FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpinetw.com; TPI: www.tpiinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"

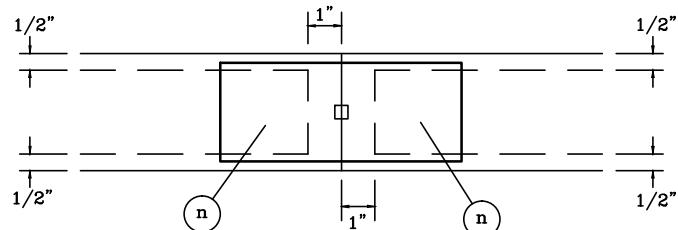
Florida Certificate of Product Approval #111999



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

TRULOX INFORMATION DETAIL

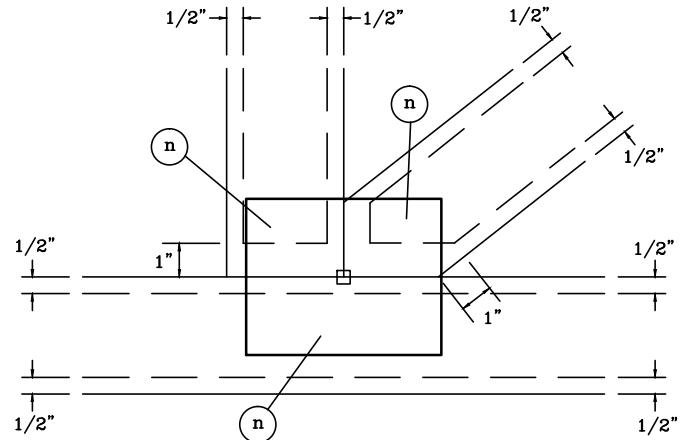
TYPICAL OFF PANEL SPLICE



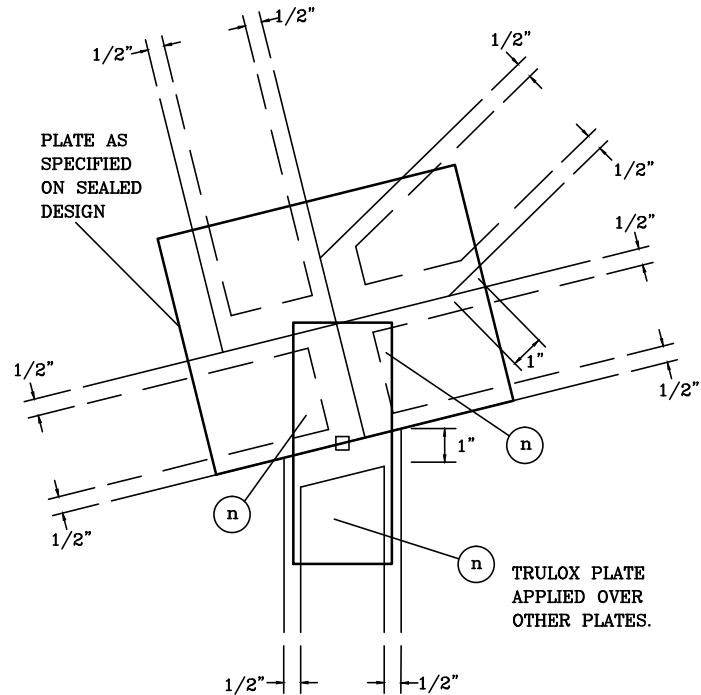
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

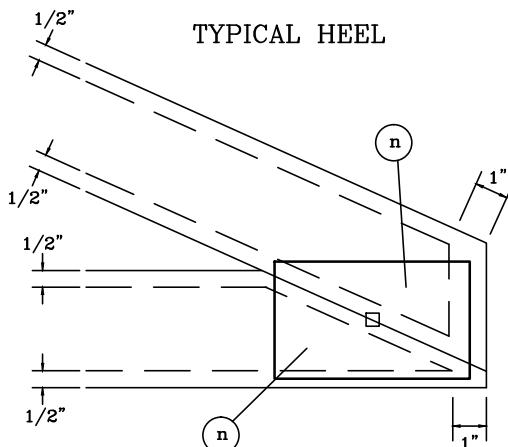
TYPICAL PANEL POINT WITHOUT SPLICE



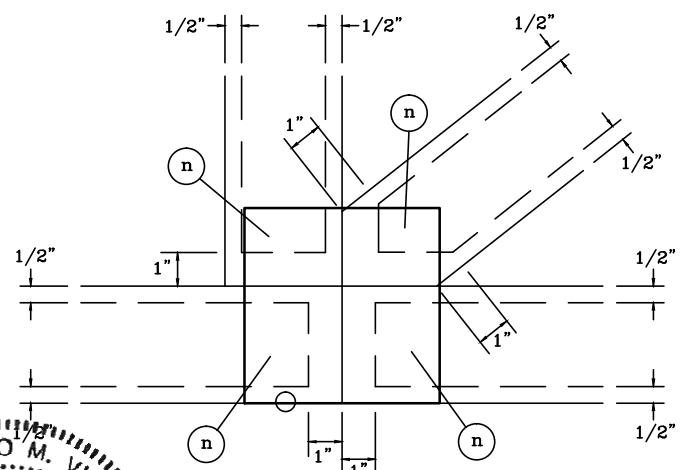
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



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NOTES:

- (n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.
- LOCATES PLATE CORNER OR FLUSH EDGE.
- LOCATES PLATE CENTER.



TRULOX PLATING
160
TL
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DATE 10/01/14