DATE 06/21/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024648

APPLICANT SUSAN HOLTON PHONE 623-6612

ADDRESS 258 NW BERT AVE LAKE CITY FL 32055

OWNER GATEWAY DEVELOPERS OF LAKE CITY PHONE 961-1086

ADDRESS 117 SW FIELDSTONE COURT LAKE CITY FL 32055

CONTRACTOR JAMES MACK LIPSCOMB PHONE CBC1253543

LOCATION OF PROPERTY 90W, TL ON HEATHRIDGE, CORNER OF HEATHRIDGE & FIELDSTONE
ON THE RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 114400.00

HEATED FLOOR AREA 2288.00 TOTAL AREA 3259.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 18

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 33-3S-16-02438-144 SUBDIVISION EMERALD COVE

LOT 44 BLOCK PHASE UNIT TOTAL ACRES

000001121 Susan L Holden

Culvert Permit No. | Culvert Waiver | Contractor's License Number | Applicant/Owner/Contractor

CULVERT | 06-0423-N | BK | JH

Driveway Connection | Septic Tank Number | LU & Zoning checked by | Approved for Issuance | New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 2743

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by | Foundation date/app. by | Monolithic date/app. by

Under slab rough-in plumbing date/app. by | Slab date/app. by | Sheathing/Nailing date/app. by

Framing date/app. by | Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by | Heat & Air Duct date/app. by | Peri. beam (Lintel) date/app. by

Permanent power date/app. by | C.O. Final date/app. by | Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by | Pool date/app. by

Reconnection date/app. by | Pump pole date/app. by | Utility Pole date/app. by

M/H Pole date/app. by | Travel Trailer date/app. by | Re-roof date/app. by

BUILDING PERMIT FEE $ 575.00 CERTIFICATION FEE $ 16.30 SURCHARGE FEE $ 16.30

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 707.60

INSPECTORS OFFICE Hale Tedder CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Property Appraiser's
Identification Number

TM File No: 05-631

Inst:2005022542 Date:09/14/2005 Time:14:45
Doc Stamp-Deed : 1360.10
DC,P.DeWitt Cason,Columbia County B:1058 P:841

WARRANTY DEED

This Warranty Deed, made this 7th day of September, 2005, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor and GATEWAY DEVELOPERS OF LAKE CITY, LLC, A Florida Limited Liability Company, whose Document number is L04000093284 and whose FEI number is 202222207 and whose post office address is 2806 West US Highway 90, Suite 101, Lake City, FL 32055, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

> Lots 43, 44, 45, 46 and 47, Emerald Cove, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 35-36, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

D D P CORPORATION

(Signature of First Witness)

TERRY MCDAVID

(Typed Name of First Witness)

(Signature of Second Witness)

Karen M. Wright

(Typed Name of Second Witness)

BY: _____ (SEAL)
O. P. Daughtry, III,
President

(Corporate Seal)

Inst:2005022542 Date:09/14/2005 Time:14:45
Doc Stamp-Deed : 1360.10
_____DC,P.DeWitt Cason,Columbia County B:1058 P:842

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 7th day of September, 2005, by O. P. Daughtry, III, President of D D P Corporation, a Florida corporation, on behalf of said corporation, who is personally known to me or who has produced Personal Knowledge as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

TERRY MCDAVID
MY COMMISSION EXPIRES
January 16, 2008
#DD 079305
Bonded thru
Notary Public Underwriters
NOTARY PUBLIC, STATE OF FLORIDA

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

1. Description of property:
   Lot 44 Emerald Cove

2. General description of improvement: Construction of Dwelling

3. Owner information:
   a. Name and address: Gateway Developers of Lake City, FL 32025
      872 SW Jaguar Drive
      Lake City, FL 32025

   b. Interest in property: Fee Simple

   c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: James Mack Lopscomb

5. Surety n/a
   a. Name and address:
   b. Amount of bond:

Inst:2006014867 Date:06/21/2006 Time:09:36
DC,P.DeWitt Cason,Columbia County B:1087 P:1014

6. Lender: Cash

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

8. In addition to himself, Owner designates_____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).
   _____.

_____
Signature of Owner

The foregoing instrument was acknowledged before me this __25_ day of _APRIL_, 2006
by Thomas Eagle, who are personally known to me and who did not take an oath.

_____
Notary Public
My commission expires:_____

Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
WWW.AARONNOTARY.com

11

Tom Eagle

961-1086

BOUNDARY SURVEY OF

\# 24648

LOT 44, EMERALD COVE SUBDIVISION, PHASE 1, AS RECORDED IN PLAT BOOK 8, PAGE 35, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. SITUATED IN SECTIONS 28, 32 AND 33, TOWNSHIP 3 SOUTH, RANGE 16 EAST, OF SAID COUNTY

FOR: GATEWAY DEVELOPERS OF LAKE CITY , LLC

DATE: FEBRUARY 27, 2007

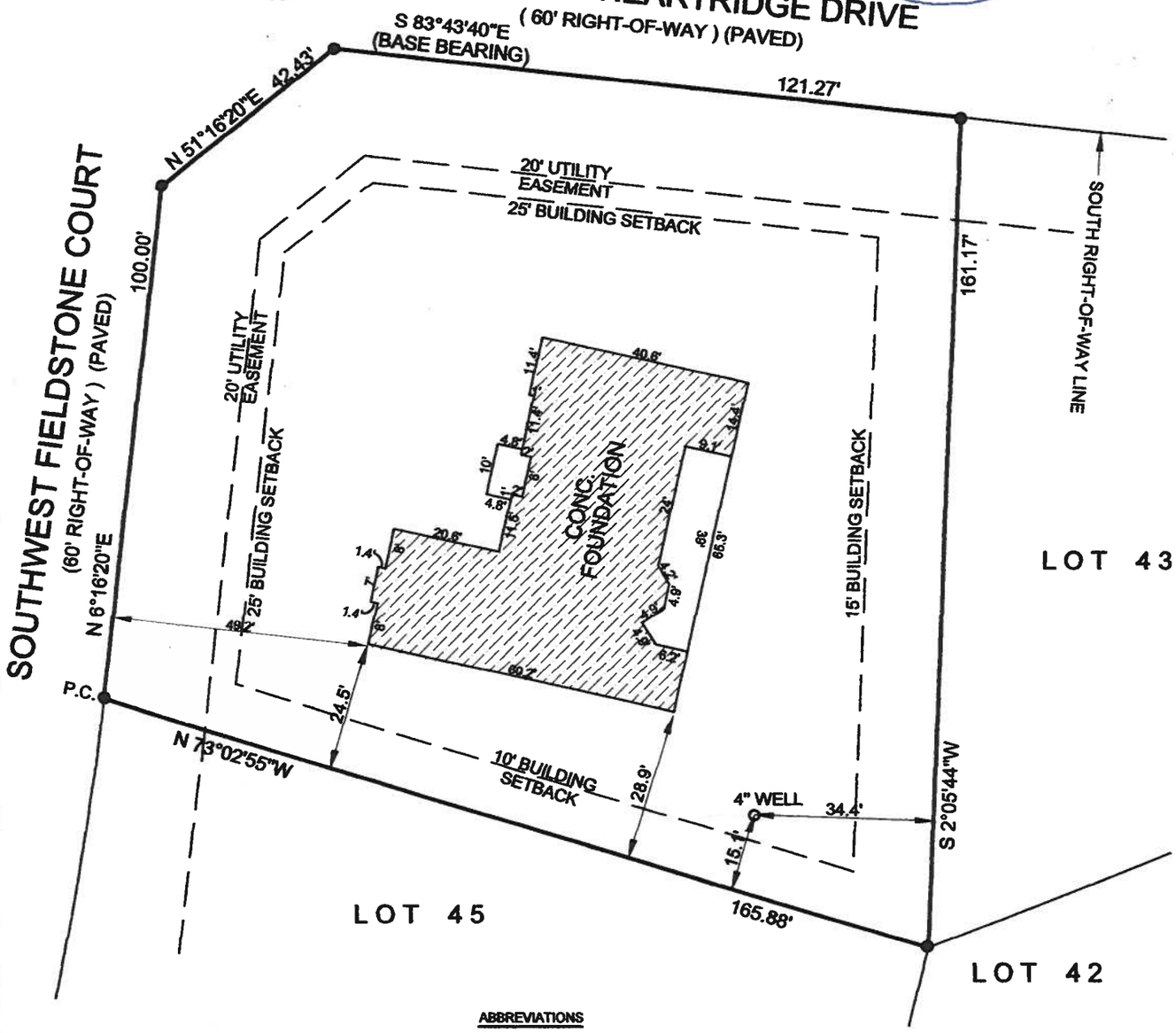
SCALE: 1' = 100'

N

BUILDING SETBACKS
FRONT: 25 FEET
SIDE: 10 FEET
REAR: 15 FEET

SLAB ELEVATION NOTE
CONCRETE SLAB ELEVATION: 114.60'
CENTER LINE INTERSECTION: 100.00'
(ELEVATION BASED ON ASSUMED DATA)

■ ASSUMED ELEVATION: 100.00'



ABBREVIATIONS

R.L.S. - REGISTERED LAND SURVEYOR
P.L.S. - PROFESSIONAL LAND SURVEYOR
L.B. - LICENSED BUISNESS
P.S.M. - PROFESSIONAL SURVEYOR & MAPPER
(A) - ACTUAL
(R) - RECORD
O/S - OFFSET
FDOT - FLORIDA DEPARTMENT OF TRANSPORTATION
± - MORE OR LESS
ORB - OFFICIAL RECORDS BOOK
PG - PAGE (S)
(P) - PLAT
(D) - DEED
(C) - CALCULATED

P.C. - POINT OF CURVATURE
P.T. - POINT OF TANGENCY
P.I. - POINT OF INTERSECTION
P.R.C. - POINT OF REVERSE CURVATURE
P.C.C. - POINT OF COMPOUND CURVATURE
R - RADIUS
R/W - RIGHT-OF-WAY
P.C.P. - PERMANENT CONTROL POINT
N - NORTH
S - SOUTH
E - EAST
W - WEST
CONC. - CONCRETE
STY - STORY
I.P. - IRON PIPE
REB. - REBAR
ST. - STREET
AVE.- AVENUE
NO ID - NO IDENTIFICATION
FD. - FOUND
CM - CONCRETE MONUMENT
P.C.P. - PERMANENT CONTROL POINT
P.R.M. - PERMANENT REFERENCE MONUMENT
E/P - EDGE OF PAVE
E/G - EDGE OF GRADE
C/G - CURB AND GUTTER
ST. M/H - STORM MANHOLE
SS. M/H - SANITARY SEWER MANHOLE
ELEV. - ELEVATION
B.M. - BENCHMARK
℄- CENTERLINE

LEGEND & NOTES

- □ DENOTES CONCRETE MONUMENT SET R.L.S. # 2245
- ■ DENOTES 2" X 2" ALUMINUM PLAT L.B. # 7170
- ○ DENOTES 5/8" REBAR W \ CAP SET L.B. # 7170
- ● DENOTES REBAR / IRON PIPE FOUND, L.B. # 7170
- X—X—X—X DENOTES EXISTING FENCE
- E—E—E—E DENOTES OVERHEAD ELECTRIC

1) FENCE,ROAD AND OVERHEAD ELECTRIC DIMENSIONS MAY NOT BE TO SCALE.
2) CLOSURE EXCEEDS 1 : 10,000
3) PROPERTY SURVEYED AS PER DESCRIPTION PROVIDED BY CLIENT, NO ABSTRACT PROVIDED.
4) NO UNDERGROUND IMPROVEMENTS, IF ANY, LOCATED BY THIS SURVEY.
5) FENCE TIES TAKEN ONLY AT LOCATIONS SHOWN AND DEPICTED HEREON.
6) BEARINGS BASED ON THE SOUTH RIGHT-OF-WAY LINE SW HEATHRIDGE DRIVE (S 83°43'40"E)
7)REVISED: FEBRUARY 27, 2007 TO SHOW CHANGE IN ROAD NAME.

TIMOTHY B. ALCORN
REGISTERED LAND SURVEYOR
FLORIDA CERTIFICATE NO. 6332

DATE: FEBRUARY 27, 2007
" MAP NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER"

J. SHERMAN FRIER & ASSOCIATES, INC.
LAND SURVEYORS
CERTIFICATE OF AUTHORIZATION - LB# 7170
P.O. BOX 580 / 130 WEST HOWARD ST. LIVE OAK,FL. 32064
PHONE: 386-362-4629 FAX: 386-362-5270
P.O. BOX 226 / 135 NE BLOXHAM ST. MAYO, FL. 32066
PHONE: 386-294-1223 FAX: 386-294-1363

DT / TA

JOB# 38-2007

Columbia County Building Permit Application

Left message 6/20/06 — Revised 9-23-04

For Office Use Only Application # 0604-95 Date Received 4-27-06 By LH Permit # 1121/24648

Application Approved by - Zoning Official BLK Date 04.05.06 Plans Examiner OK JTH Date 6-19-06

Flood Zone X Per PLAT Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Den.

Comments

CK# 2743 06-0423-N

FAX 719-9586

Applicants Name Susan Holton Phone 623-6612

Address 258 NW Bert Ave Lake City, FL 32055

Owners Name Gateway Developers of Lake City Phone 961-1086

911 Address 117 SW Fieldstone Court

Contractors Name James Mack Lipscomb Phone 623-9141

Address 872 SW Jaguar Drive Lake City FL 32025

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address GTC Design Group 130 West Howard St Live Oak, FL 32064

Mortgage Lenders Name & Address

Circle the correct power company – (FL Power & Light) – Clay Elec. – Suwannee Valley Elec. – Progressive Energy

Property ID Number 33-3S-16-02438-144 Estimated Cost of Construction $122,000

Subdivision Name Emerald Cove Lot 44 Block ____ Unit 1 Phase 1

Driving Directions 90 W, pass I-75, Left on Heathridge
Lot 44 on the corner of Heathridge + Fieldstone

Type of Construction Frame SFD Number of Existing Dwellings on Property 0

Total Acreage .5 Lot Size ____ Do you need a – (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 32'8" Side 34' Side 44' Rear 63'

Total Building Height 18' Number of Stories 1 Heated Floor Area 2288 Roof Pitch 7/12

Porches 369 Garage 602 Total 3259

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Mack Lipscomb
Contractor Signature
Contractors License Number CBC1253543
Competency Card Number
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

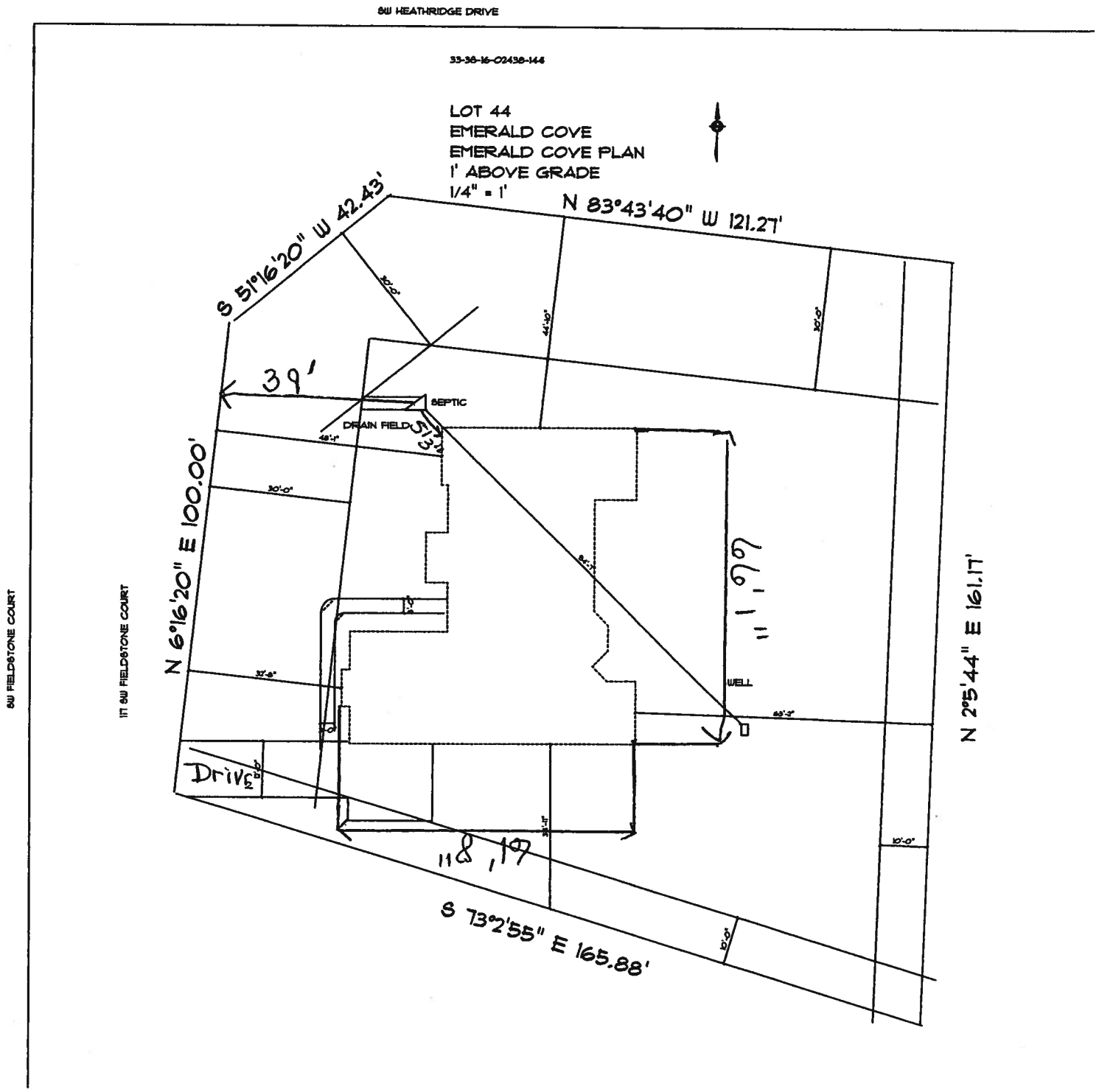
Sworn to (or affirmed) and subscribed before me

this 25 day of April 2006.

Personally known X or Produced Identification ____

Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
WWW.AARONNOTARY.com
NOTARY PUBLIC STATE OF FLORIDA

Notary Signature


SW HEATHRIDGE DRIVE
33-38-16-02438-144
LOT 44
EMERALD COVE
EMERALD COVE PLAN
1' ABOVE GRADE
1/4" = 1'
S 51°16'20" W 42.43'
N 83°43'40" W 121.27'
39'
SEPTIC
DRAIN FIELD
513
N 6°16'20" E 100.00'
111.99
WELL
Drive
118.19
S 73°2'55" E 165.88'
N 2°5'44" E 161.17'
SW FIELDSTONE COURT
117 SW FIELDSTONE COURT

S. Holton

STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0423N

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.

NA

1 in. below

See attached

Notes:

CL

NA

Site Plan submitted by: Susan Hall (Signature) Agent (Title)

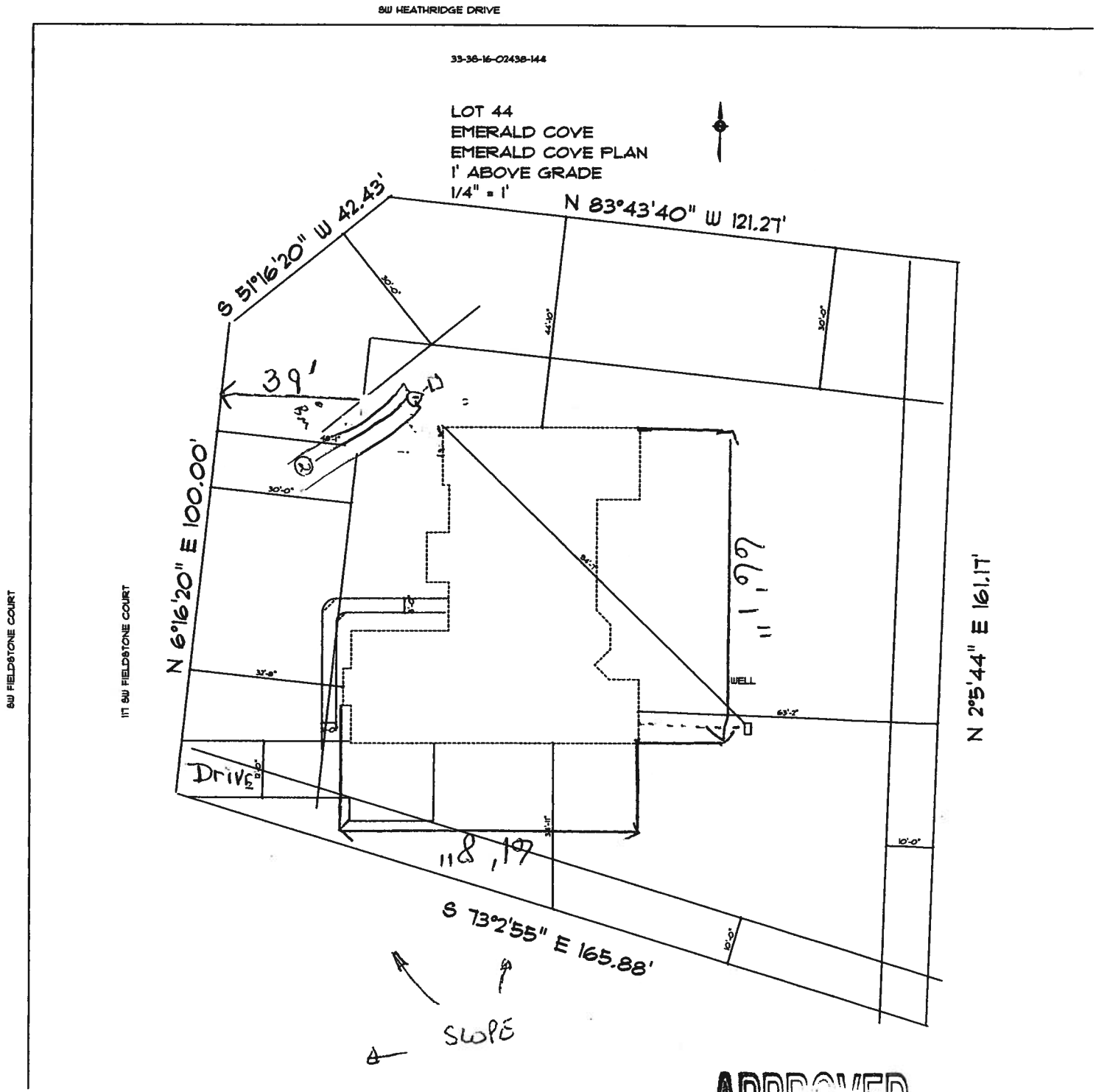
Plan Approved X Not Approved

Date 4/26/06 05/11/06

By County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

06-0423N



APPROVED

S. Holden

Columbia CHD

0604-95

FORM 600A-2004 EnergyGauge® 4.21

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Emerald Cove House**	Builder:	Lipscomb
Address:		Permitting Office:	Columbia
City, State:	,	Permit Number:	24648
Owner:	**Gateway Development**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	4	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	2288 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. (Dble Default) 348.1 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 348.1 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 237.6(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=13.0, 1410.7 ft²	
b.	N/A		
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 3407.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Con. Ret: Con. AH: Garage	Sup. R=6.0, 150.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 52.0 kBtu/hr	
		SEER: 13.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 52.0 kBtu/hr	
		HSPF: 7.00	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons	
		EF: 0.97	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.15

Total as-built points: 33931

Total base points: 33960

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [signature]

DATE: 6/12/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE					AS-BUILT					
FLOOR TYPES	Area X	BSPM	= Points		Type	R-Value	Area X	SPM	=	Points
Slab	237.6(p)	-37.0	-8792.3		Slab-On-Grade Edge Insulation	0.0	237.6(p	-41.20		-9790.4
Raised	0.0	0.00	0.0							
Base Total:			**-8792.3**		**As-Built Total:**		**237.6**			**-9790.4**
INFILTRATION	Area X	BSPM	= Points				Area X	SPM	=	Points
	2288.0	10.21	23360.5				2288.0	10.21		23360.5
Summer Base Points: 29752.1					**Summer As-Built Points:**					**34880.2**
Total Summer Points	X System Multiplier	=	Cooling Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
					(sys 1: Central Unit 52000 btuh ,SEER/EFF(13.0) Ducts:Con(S),Con(R),Gar(AH),R6.0(INS)					
					34880	1.00	(1.00 x 1.147 x 1.00)	0.263	1.000	10503.5
29752.1	**0.4266**		**12692.2**		**34880.2**	**1.00**	**1.147**	**0.263**	**1.000**	**10503.5**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT					
FLOOR TYPES	Area X	BWPM =	Points	Type	R-Value	Area X	WPM =	Points	
Slab	237.6(p)	8.9	2114.9	Slab-On-Grade Edge Insulation	0.0	237.6(p	18.80	4467.4	
Raised	0.0	0.00	0.0						
Base Total:			**2114.9**	**As-Built Total:**		**237.6**		**4467.4**	
INFILTRATION	Area X	BWPM =	Points			Area X	WPM =	Points	
	2288.0	-0.59	-1349.9			2288.0	-0.59	-1349.9	
Winter Base Points:			**17098.2**	**Winter As-Built Points:**					**23584.2**
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
				(sys 1: Electric Heat Pump 52000 btuh ,EFF(7.0) Ducts:Con(S),Con(R),Gar(AH),R6.0					
				23584.2	1.000	(1.000 x 1.169 x 1.00)	0.487	1.000	13430.5
17098.2	**0.6274**	**10727.4**		**23584.2**	**1.00**	**1.169**	**0.487**	**1.000**	**13430.5**

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

Emerald Lakes Sub.
Tom Eagle

#0604-95

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001121

DATE 06/21/2006 PARCEL ID # 33-3S-16-02438-144

APPLICANT SUSAN HOLTON PHONE 623-6612

ADDRESS 258 NW BERT AVE LAKE CITY FL 32055

OWNER GATEWAY DEVELOPERS OF LAKE CITY PHONE 961-1086

ADDRESS 117 SW FIELDSTONE COURT LAKE CITY FL 32055

CONTRACTOR JAMES MACK LIPSCOMB PHONE CBC1253543

LOCATION OF PROPERTY 90W, TL ON HEATHRIDGE, CORNER OF HEATHRIDGE & FIELDSTONE

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 44

SIGNATURE Susan L Holton

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0604-95
James Mack Lipscomb, Owner Gateway Developers of Lake City lot 44 phase 1 of Emerald Cove Subdivision

On the date of May 1, 2006 application 0604-95 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0604-95 when making reference to this application.

1. The Florida Energy Efficiency Code for Building Construction (Form 600A-2001) Line Number six request the conditioned floor area (Sq.ft.). The square footage submitted on the form is 2726 square footage and the total square footage on both the application for building permit and the structural plans indicate the total conditioned floor area (Sq.ft.). as 2288 square footage. Please submit a

corrected Florida Energy Efficiency Code for Building Construction (Form 600A-2001) to this department.

2. Please show compliance with the FRC-2004 sections R309 Garage:

R309.1

A: Opening protection: Openings from a private garage directly into a room used for sleeping purposes shall not be permitted. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors. The attic pulls down stairs access door shall have the same protection rating as the openings between the garage and residence.

B: R309.1.1 Duct penetration. Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage.

C: R309.2 Separation required. The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

3. On the electrical plan show compliance with the National electrical code along with the FRC-2004 requirements. The smoke alarms shown on the electrical plan shall be so installed to meet the requirements of the FRC-2004 section R313.1 Smoke alarms: Smoke alarms shall be installed in the following locations:

1. In each sleeping room.
2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.
3. On each additional story of the dwelling, including basements but not including crawl spaces and uninhabitable attics. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level.

When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit. The alarm shall be clearly audible in all bedrooms over background noise levels with all intervening doors closed. All smoke alarms shall be listed and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

PRODUCT APPROVAL SPECIFICATION SHEET

Location:_____ **Project Name:**_____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

Location

Permit # **(FOR STAFF USE ONLY)**

4. Show on the electrical plans as required by the National Electrical Code article 210.12 that all branch circuits supplying outlets installed within a bedroom shall be protected by Arc-Fault interrupter devices.
5. On the electrical plan show the location of the electrical panel and include the total amperage rating of the electrical service panel also show the overcurrent protection device which shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.
6. As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org (see attached form)

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Phone Number 386-758-1163
Fax Number 386-754-7088

FAX TRANSMITTAL FORM

To:	From:
Name: James Mack Lipscomb	
	Date Sent: 05/01/06
CC: Building permit application 0604-95	
Phone:	Number of Pages: Seven
Fax: 719-9586	

Message: Reference to a building permit application Number: 0604-95
Owners Gateway Developers of Lake City Lot 44 phase 1 of Emerald Cove Subdivision.

PRODUCT APPROVAL SPECIFICATION SHEET

Location: **Project Name:** 0604-95

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	MASONITE	FIBERGLASS SIDE-HINGED DOOR	5507
2. Sliding			
3. Sectional	CRAFT GARAGE DOORS	18'x7' GARAGE DOOR	2792
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	CAPITAL	SINGLE HUNG WINDOWS	6751
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles	TAMKO	3TAB ASPHALT SHINGLES	1956
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

PRODUCT APPROVAL SPECIFICATION SHEET

Location: **Project Name:** 0604-95

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

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5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Project Information for:	L141606		
Builder:	LIPSCOMB EAGLE	Date:	2/3/2006
Lot:	LOT 44 EMERALD COVE	Start Number:	**1376**
Subdivision:	N/A		
County or City:	COLUMBIA		
Truss Page Count:	42		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

LIPSCOMB, JAMES MACK CBC1253543
Address: 255 SE WOODS TERRACE
LAKE CITY, FL 32025 Designer: **74**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	EJ7	0203061376	2/3/2006	41	T26	0203061416	2/3/2006
2	EJ7A	0203061377	2/3/2006	42	T27	0203061417	2/3/2006
3	EJ7B	0203061378	2/3/2006				
4	EJ7G	0203061379	2/3/2006				
5	EJ7T	0203061380	2/3/2006				
6	PB1	0203061381	2/3/2006				
7	PB2	0203061382	2/3/2006				
8	PB3	0203061383	2/3/2006				
9	T01G	0203061384	2/3/2006				
10	T02	0203061385	2/3/2006				
11	T02A	0203061386	2/3/2006				
12	T02G	0203061387	2/3/2006				
13	T03	0203061388	2/3/2006				
14	T03G	0203061389	2/3/2006				
15	T04	0203061390	2/3/2006				
16	T04G	0203061391	2/3/2006				
17	T05	0203061392	2/3/2006				
18	T05A	0203061393	2/3/2006				
19	T05B	0203061394	2/3/2006				
20	T05C	0203061395	2/3/2006				
21	T05G	0203061396	2/3/2006				
22	T06	0203061397	2/3/2006				
23	T07	0203061398	2/3/2006				
24	T08	0203061399	2/3/2006				
25	T09	0203061400	2/3/2006				
26	T10	0203061401	2/3/2006				
27	T11	0203061402	2/3/2006				
28	T12	0203061403	2/3/2006				
29	T14	0203061404	2/3/2006				
30	T15	0203061405	2/3/2006				
31	T16	0203061406	2/3/2006				
32	T17	0203061407	2/3/2006				
33	T18	0203061408	2/3/2006				
34	T19	0203061409	2/3/2006				
35	T20	0203061410	2/3/2006				
36	T21	0203061411	2/3/2006				
37	T22	0203061412	2/3/2006				
38	T23	0203061413	2/3/2006				
39	T24	0203061414	2/3/2006				
40	T25	0203061415	2/3/2006				

FEB 0 3 2006

Dwg.#0203061375

Licensee Details

Licensee Information

Name:	**LIPSCOMB, JAMES MACK (Primary Name)**
	LIPSCOMB & EAGLE DEVELOPMENT INC (DBA Name)
Main Address:	**255 SE WOODS TERRACE** **LAKE CITY Florida 32025**
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**2806 U S HWY WEST SUITE 101** **LAKE CITY FL 32055**
County:	**COLUMBIA**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC1253543**
Status:	**Current,Active**
Licensure Date:	**11/02/2005**
Expires:	**08/31/2006**

Special Qualifications	**Qualification Effective**
Qualified Business License Required	**11/02/2005**

View Related License Information

View License Complaint

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061376

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	EJ7	MONO TRUSS	26	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:28 2006 Page 1

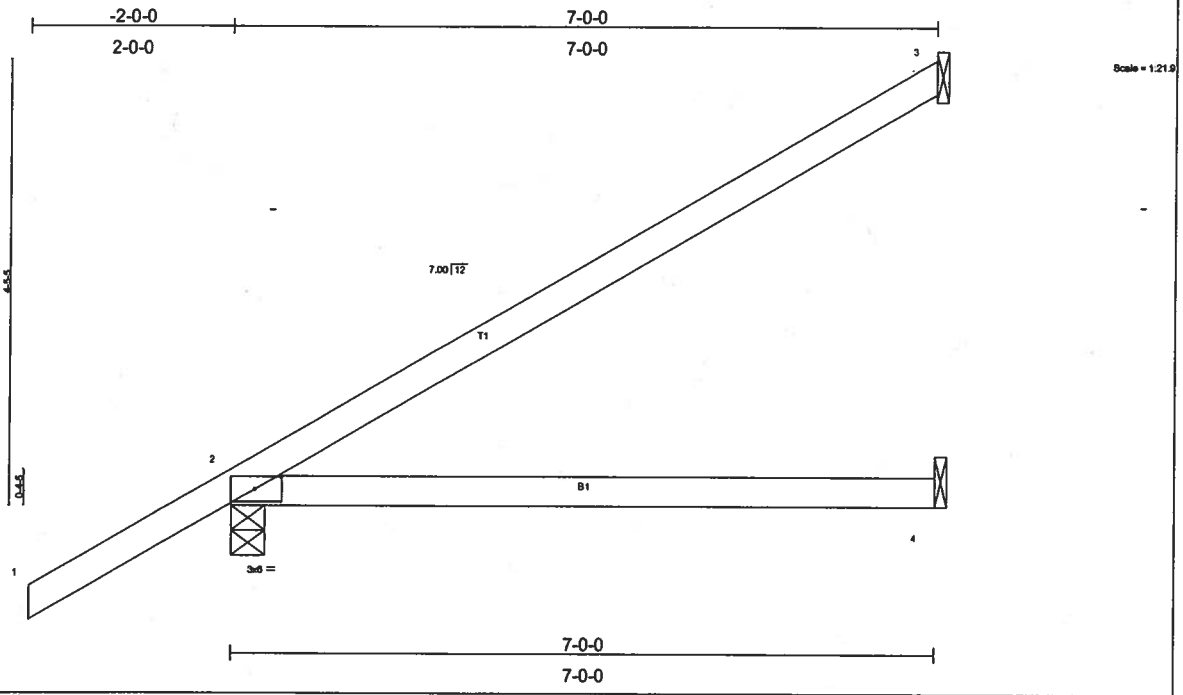

Plate Offsets (X,Y): [2:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.12	2-4	>667	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.20	2-4	>399	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=161/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=261(load case 5)
Max Uplift3=-146(load case 5), 2=-198(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-111/64
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 3 and 198 lb uplift at joint 2.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061377

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	EJ7A	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:29 2006 Page 1

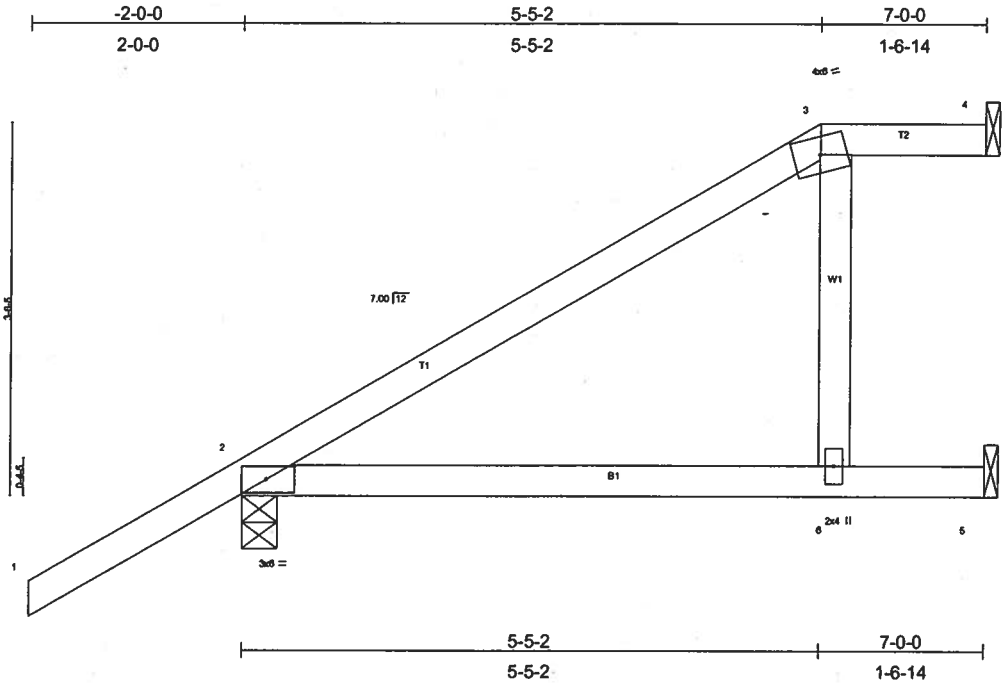

Plate Offsets (X,Y): [2:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.14	2-6	>562	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.24	2-6	>345	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.09	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=104/Mechanical, 2=420/0-4-0, 5=162/Mechanical
Max Horz 2=221(load case 5)
Max Uplift4=-31(load case 3), 2=-217(load case 5), 5=-86(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-97/37, 3-4=-0/1
BOT CHORD 2-6=-7/4, 5-6=0/0
WEBS 3-6=-83/161

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 4, 217 lb uplift at joint 2 and 86 lb uplift at joint 5.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061378

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	EJ7B	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:31 2006 Page 1

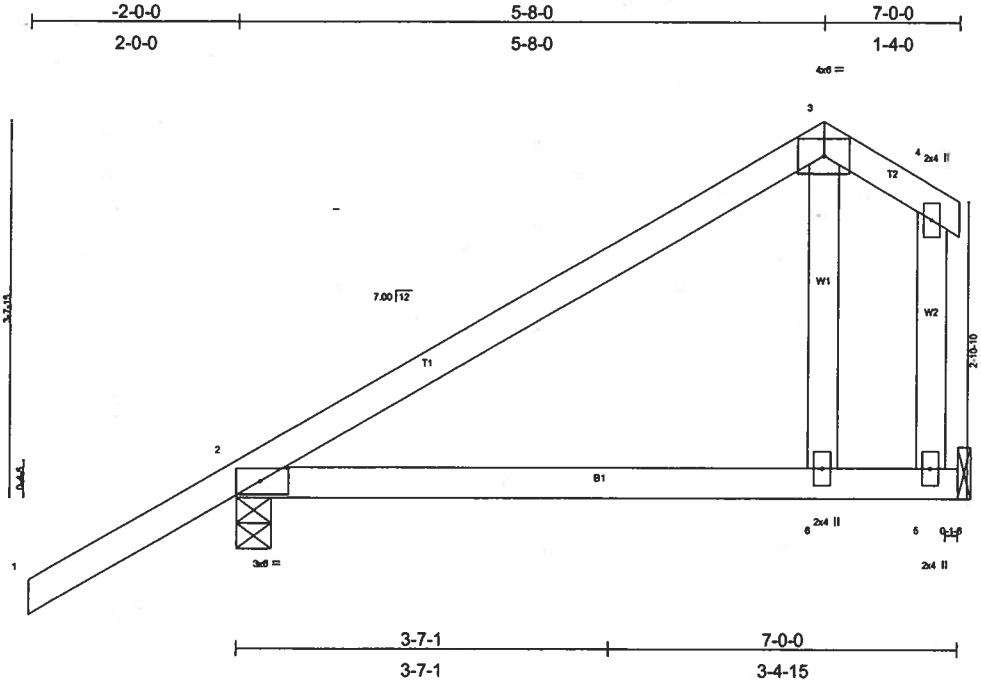

Plate Offsets (X,Y): [2:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.12	2-6	>638	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.20	2-6	>391	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=412/0-4-0, 5=256/Mechanical
Max Horz 2=207(load case 5)
Max Uplift2=-221(load case 5), 5=-94(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-94/46, 3-4=-48/10
BOT CHORD 2-6=-0/0, 5-6=0/0
WEBS 3-6=-77/154, 4-5=-101/11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2 and 94 lb uplift at joint 5.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061379

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	EJ7G	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:33 2006 Page 1

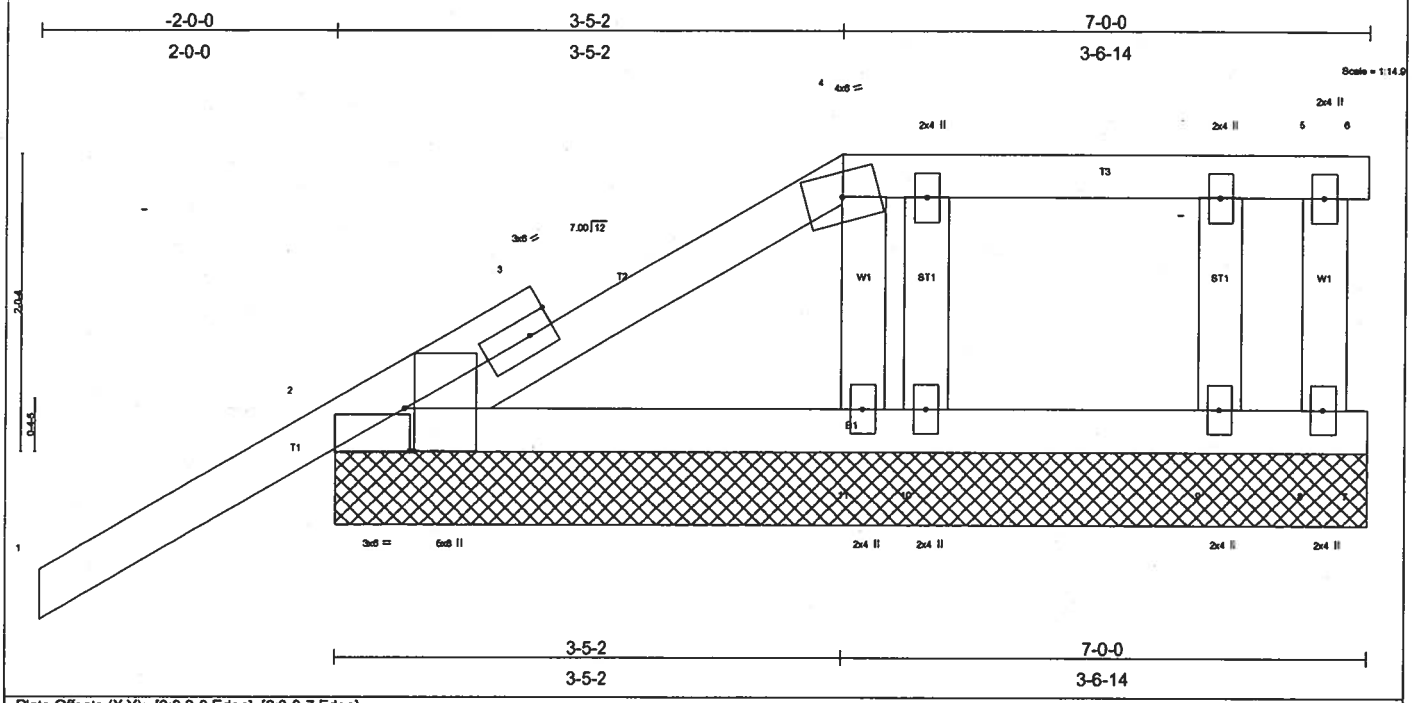

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.04	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.11	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=523/7-0-0, 6=-248/7-0-0, 7=11/7-0-0, 11=444/7-0-0, 8=455/7-0-0, 10=-7/7-0-0, 9=58/7-0-0
Max Horz 2=152(load case 5)
Max Uplift2=-289(load case 5), 6=-265(load case 10), 11=-129(load case 4), 8=-359(load case 3), 10=-36(load case 3)
Max Grav 2=523(load case 1), 6=202(load case 3), 7=11(load case 10), 11=444(load case 1), 8=477(load case 10), 9=58(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-29/112, 2-3=-114/32, 3-4=-33/53, 4-5=-1/1, 5-6=0/0
BOT CHORD 2-11=-22/30, 10-11=0/0, 9-10=0/0, 8-9=0/0, 7-8=0/0
WEBS 4-11=-349/258, 5-8=-484/425

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 2, 265 lb uplift at joint 6, 129 lb uplift at joint 11, 359 lb uplift at joint 8 and 36 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-6=-114(F=-60), 2-7=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061380

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	EJ7T	SPECIAL	7	1	Job Reference (optional)

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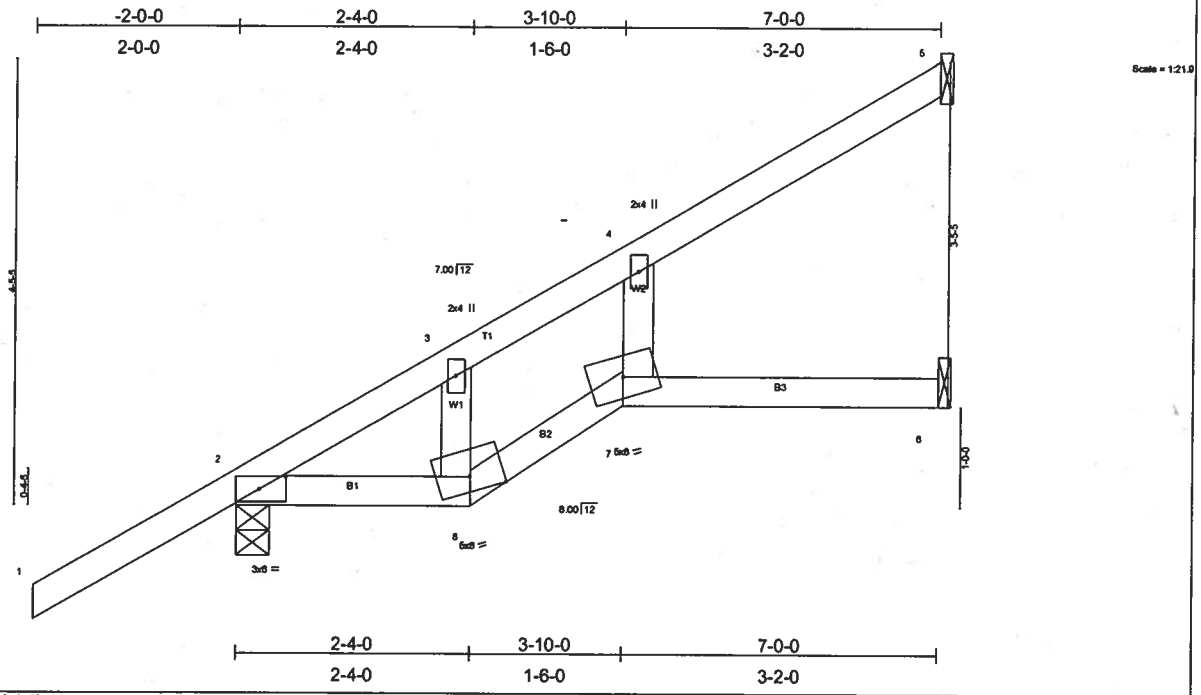

Plate Offsets (X,Y): [2:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.15	7-8	>535	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.24	7-8	>332	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=154/Mechanical, 2=420/0-4-0, 6=111/Mechanical
Max Horz 2=261(load case 5)
Max Uplift5=-109(load case 5), 2=-198(load case 5), 6=-30(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-168/9, 3-4=-130/6, 4-5=-70/70
BOT CHORD 2-8=-0/8, 7-8=-9/26, 6-7=-0/0
WEBS 3-8=-76/70, 4-7=0/86

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 5, 198 lb uplift at joint 2 and 30 lb uplift at joint 6.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061381

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	PB1	HIP PIGGYBACK 4x6 =	2	1	Job Reference (optional)

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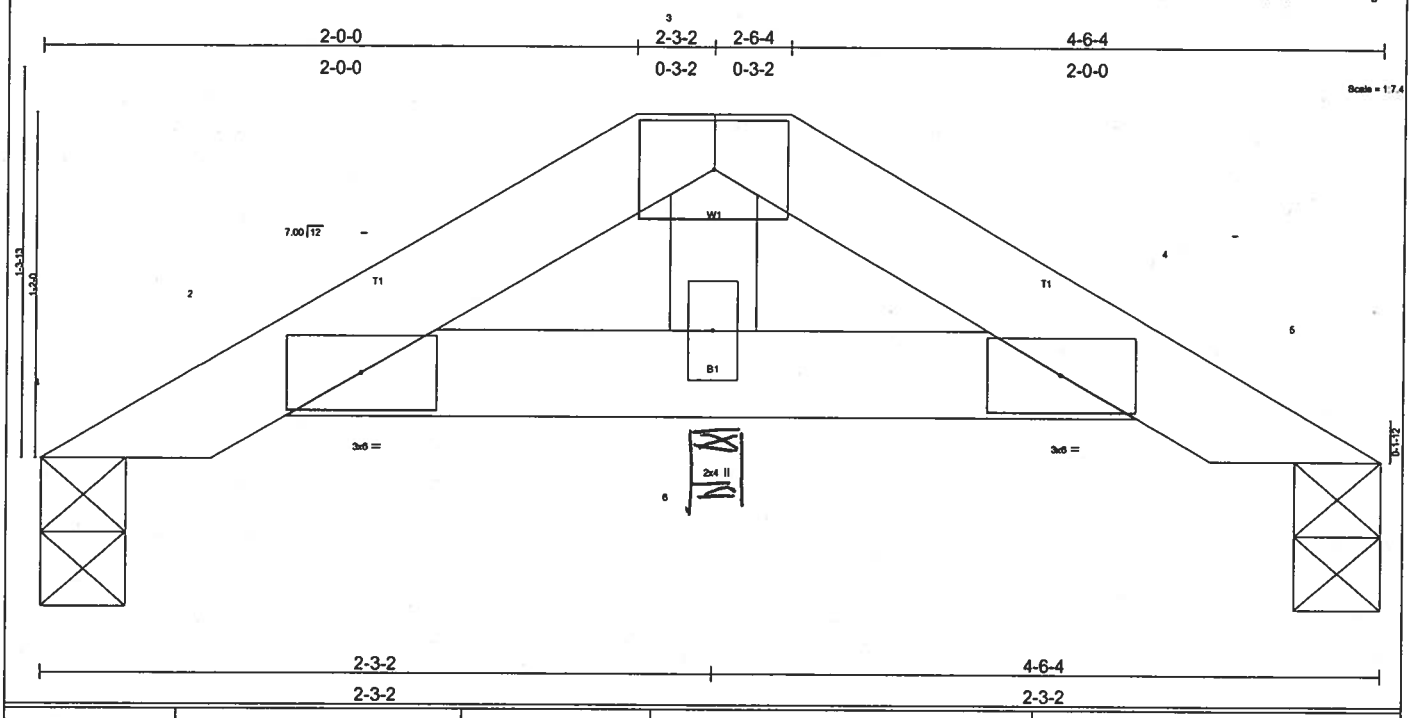

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=165/0-3-8, 5=165/0-3-8
Max Horz 1=43(load case 4)
Max Uplift1=-57(load case 5), 5=-57(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-77/43, 2-3=-300/129, 3-4=-300/129, 4-5=-77/42
BOT CHORD 2-6=-77/262, 4-6=-77/262
WEBS 3-6=-36/137

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1 and 57 lb uplift at joint 5.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061382

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	PB2	PIGGYBACK	9	1	Job Reference (optional)

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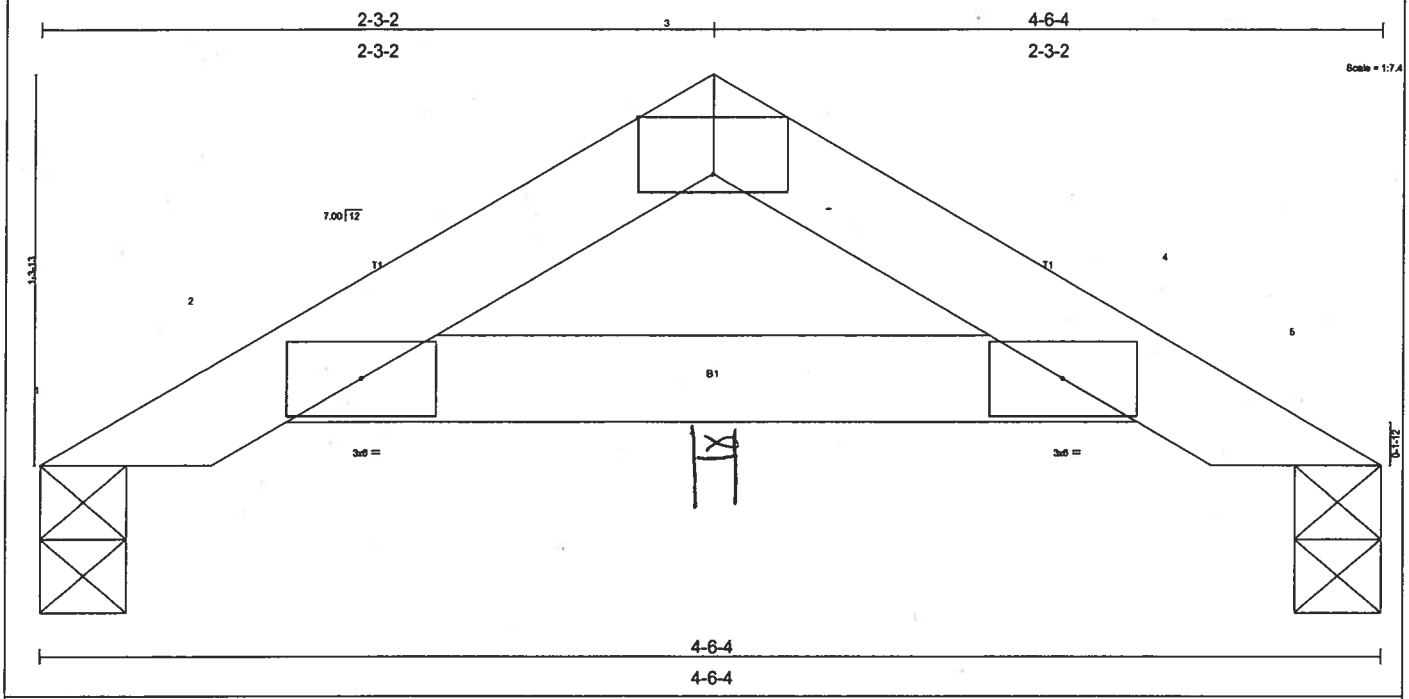

Plate Offsets (X,Y): [3:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=165/0-3-8, 5=165/0-3-8
Max Horz 1=43(load case 4)
Max Uplift1=-57(load case 5), 5=-57(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-77/43, 2-3=-227/110, 3-4=-227/110, 4-5=-77/42
BOT CHORD 2-4=-66/219

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1 and 57 lb uplift at joint 5.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061383

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	PB3	HIP PIGGYBACK	1	1	Job Reference (optional)

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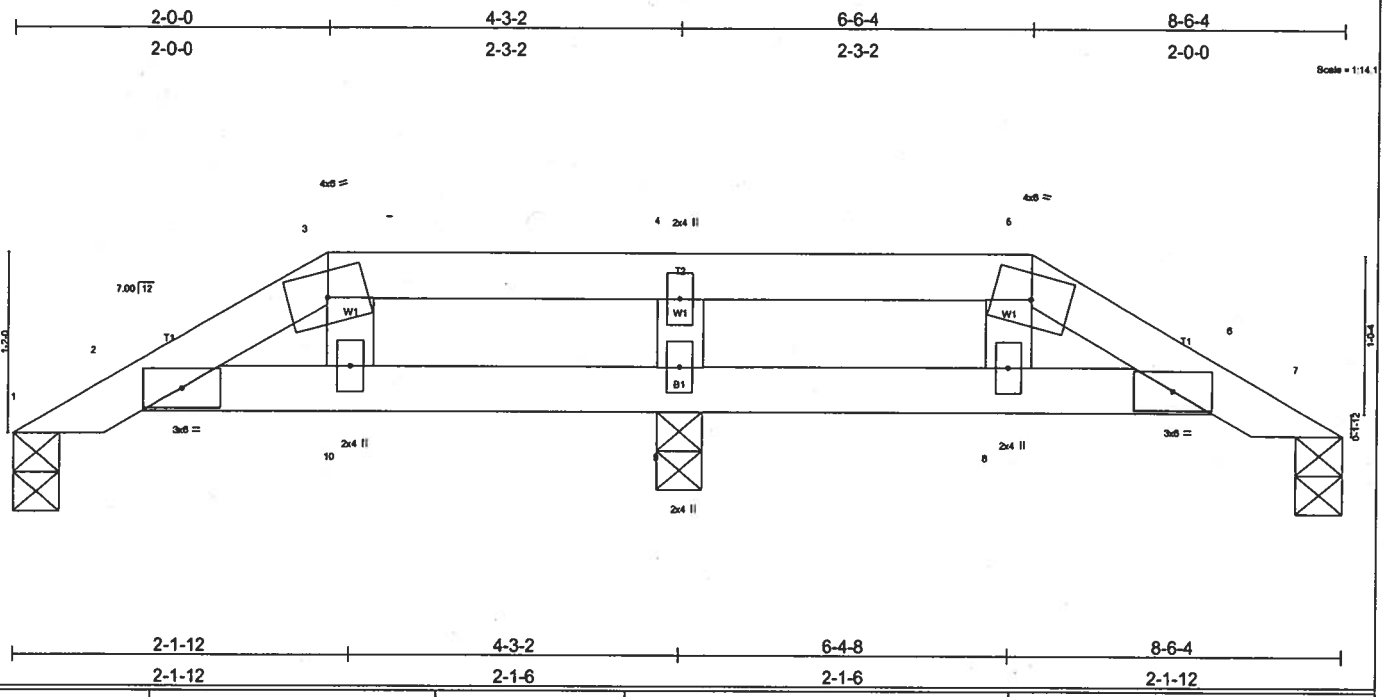

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.02	10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=150/0-3-8, 7=150/0-3-8, 9=365/0-3-8
Max Horz 1=-38(load case 3)
Max Uplift1=-52(load case 5), 7=-55(load case 6), 9=-142(load case 4)
Max Grav 1=152(load case 9), 7=152(load case 10), 9=365(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-71/43, 2-3=-163/95, 3-4=-140/99, 4-5=-140/99, 5-6=-163/95, 6-7=-71/43
BOT CHORD 2-10=-53/138, 9-10=-47/140, 8-9=-47/140, 6-8=-51/138
WEBS 3-10=0/35, 5-8=0/35, 4-9=-203/180

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1, 55 lb uplift at joint 7 and 142 lb uplift at joint 9.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061384

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T01G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:39 2006 Page 1

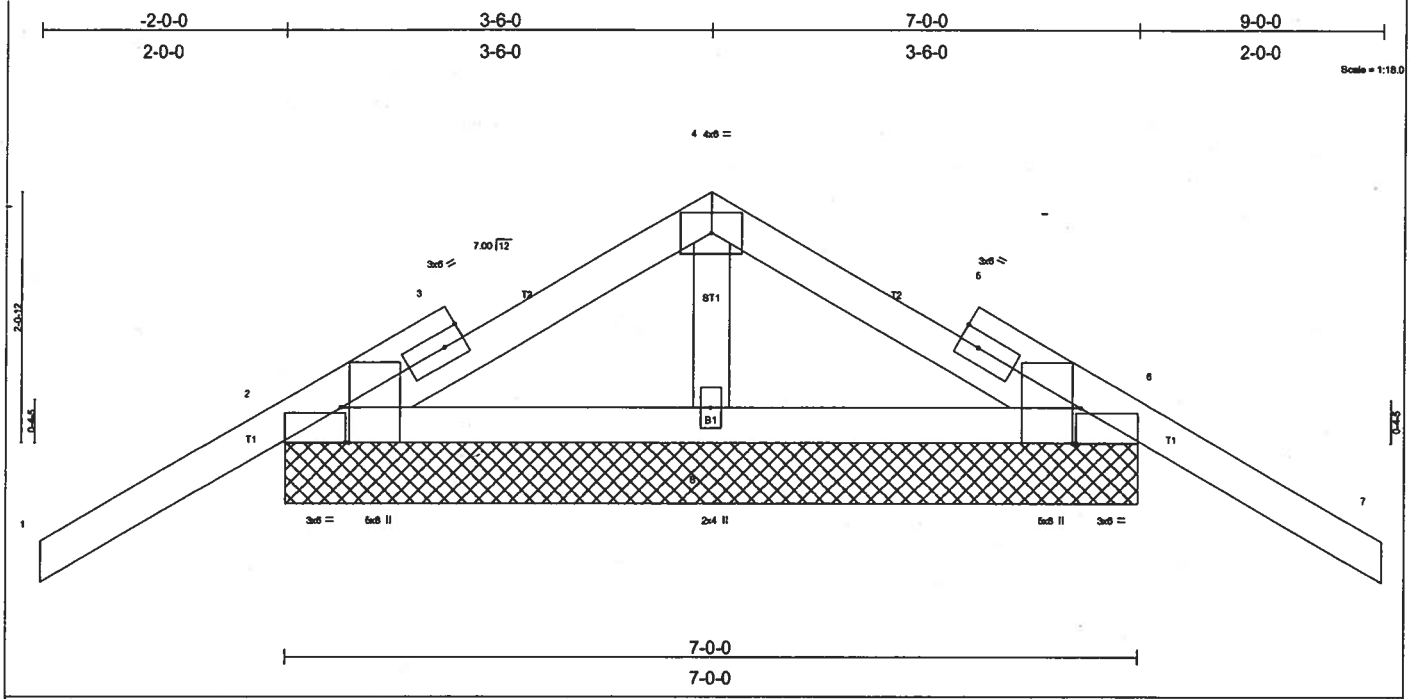

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [6:0-3-8,Edge], [6:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.04	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.06	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=501/7-0-0, 6=501/7-0-0, 8=463/7-0-0
Max Horz 2=68(load case 5)
Max Uplift2=-308(load case 5), 6=-320(load case 6), 8=-79(load case 5)
Max Grav 2=505(load case 9), 6=505(load case 10), 8=463(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-16/112, 2-3=-59/85, 3-4=-5/114, 4-5=0/114, 5-6=-59/85, 6-7=-16/112
BOT CHORD 2-8=-27/77, 6-8=-27/77
WEBS 4-8=-359/110

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2, 320 lb uplift at joint 6 and 79 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061385

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T02	COMMON	10	1	Job Reference (optional)

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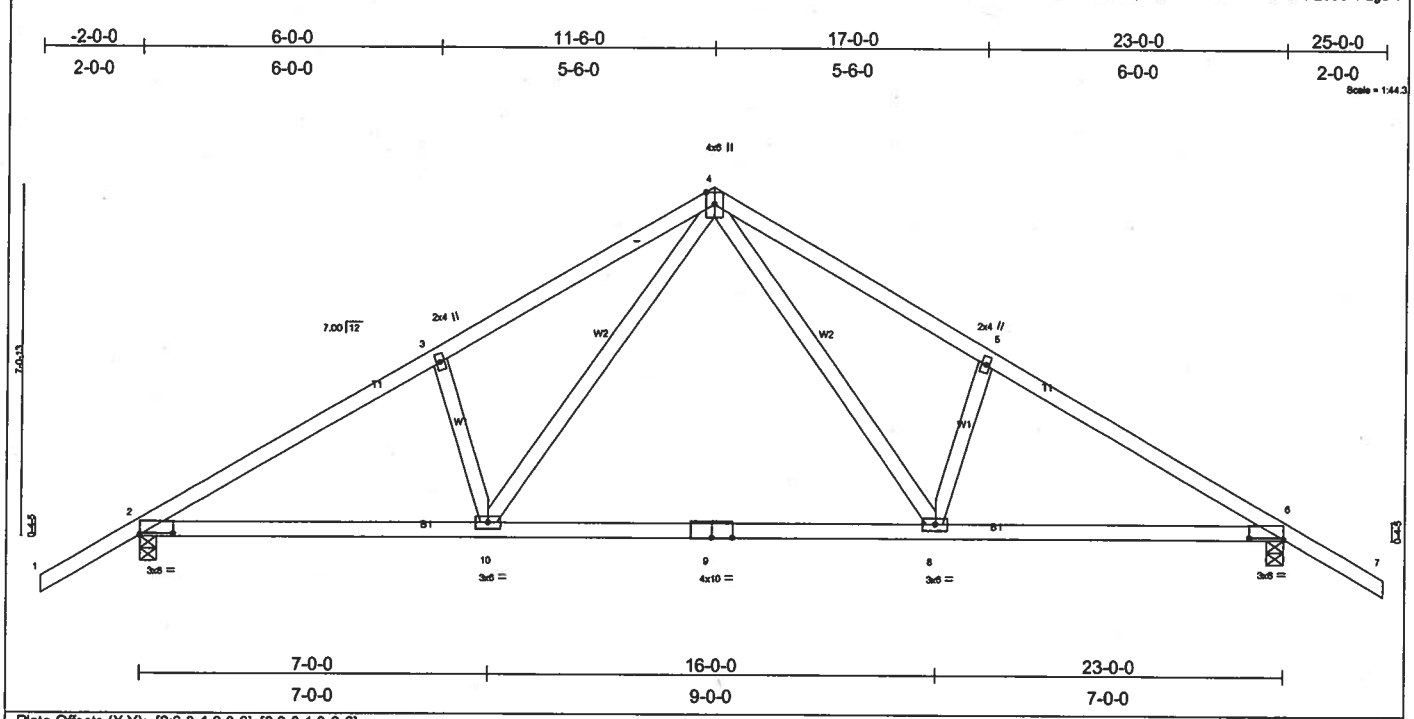

Plate Offsets (X,Y): [2:0-8-1,0-0-6], [6:0-8-1,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.41	8-10	>656	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.68	8-10	>402	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.40	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 115 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-5-3 oc bracing.

REACTIONS (lb/size) 2=1294/0-4-0, 6=1294/0-4-0
Max Horz 2=240(load case 4)
Max Uplift2=-517(load case 5), 6=-517(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-1964/638, 3-4=-1857/717, 4-5=-1857/717, 5-6=-1964/638, 6-7=0/54
BOT CHORD 2-10=-488/1606, 9-10=-225/1046, 8-9=-225/1046, 6-8=-402/1606
WEBS 3-10=-242/252, 4-10=-345/912, 4-8=-345/912, 5-8=-242/252

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 517 lb uplift at joint 2 and 517 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061386

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T02A	COMMON	2	1	Job Reference (optional)

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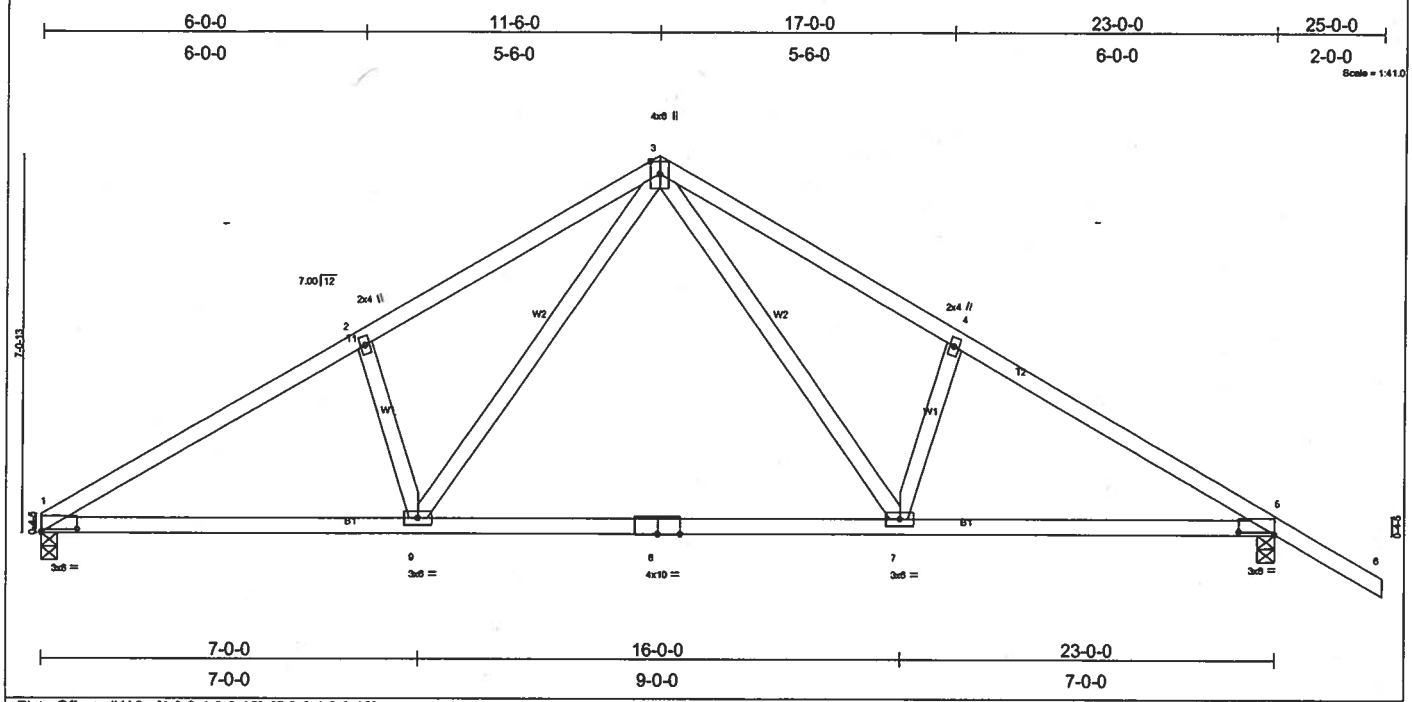

Plate Offsets (X,Y): [1:0-8-1,0-0-10], [5:0-8-1,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.41	7-9	>666	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.67	7-9	>408	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-10-14 oc bracing.

REACTIONS (lb/size) 1=1172/0-3-8, 5=1301/0-4-0
Max Horz 1=-260(load case 3)
Max Uplift1=-388(load case 5), 5=-520(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1999/683, 2-3=-1896/767, 3-4=-1869/731, 4-5=-1976/652, 5-6=0/54
BOT CHORD 1-9=-533/1643, 8-9=-238/1058, 7-8=-238/1058, 5-7=-414/1616
WEBS 2-9=-260/273, 3-9=-392/950, 3-7=-344/911, 4-7=-242/253

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 1 and 520 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-9=-30, 7-9=-80(F=-50), 5-7=-30

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Dwg.#0203061387

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T02G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:42 2006 Page 1

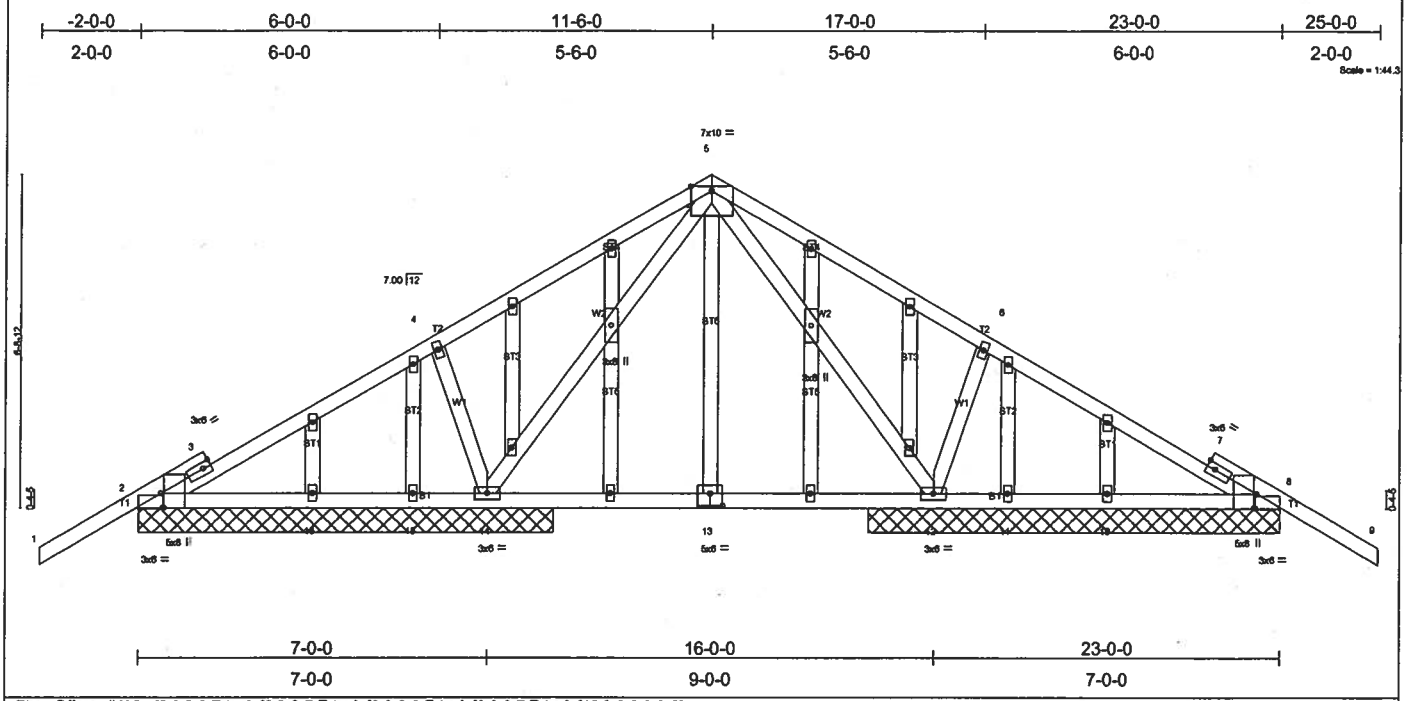

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [8:0-3-8,Edge], [8:0-0-7,Edge], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.09	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.16	12-14	>671	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.64	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 161 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 12-14.

REACTIONS (lb/size) 2=515/8-4-0, 8=515/8-4-0, 14=1333/8-4-0, 12=1333/8-4-0, 15=-172/8-4-0, 16=208/8-4-0, 11=-172/8-4-0, 10=208/8-4-0
Max Horz 2=229(load case 4)
Max Uplift2=-273(load case 5), 8=-292(load case 6), 14=-465(load case 5), 12=-445(load case 6), 15=-172(load case 1), 16=-22(load case 6), 11=-172(load case 1), 10=-19(load case 5)
Max Grav 2=524(load case 9), 8=524(load case 10), 14=1333(load case 1), 12=1333(load case 1), 16=208(load case 1), 10=208(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-14/112, 2-3=-127/110, 3-4=-127/280, 4-5=-88/479, 5-6=-69/479, 6-7=-87/280, 7-8=-63/39, 8-9=-14/112
BOT CHORD 2-16=-99/233, 15-16=-99/233, 14-15=-99/233, 13-14=-44/207, 12-13=-44/207, 11-12=-99/190, 10-11=-99/190, 8-10=-99/190
WEBS 4-14=-587/376, 5-14=-578/202, 5-12=-578/224, 6-12=-587/377

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 2, 292 lb uplift at joint 8, 465 lb uplift at joint 14, 445 lb uplift at joint 12, 172 lb uplift at joint 15, 22 lb uplift at joint 16, 172 lb uplift at joint 11 and 19 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0203061388

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T03	COMMON	2	1	Job Reference (optional)

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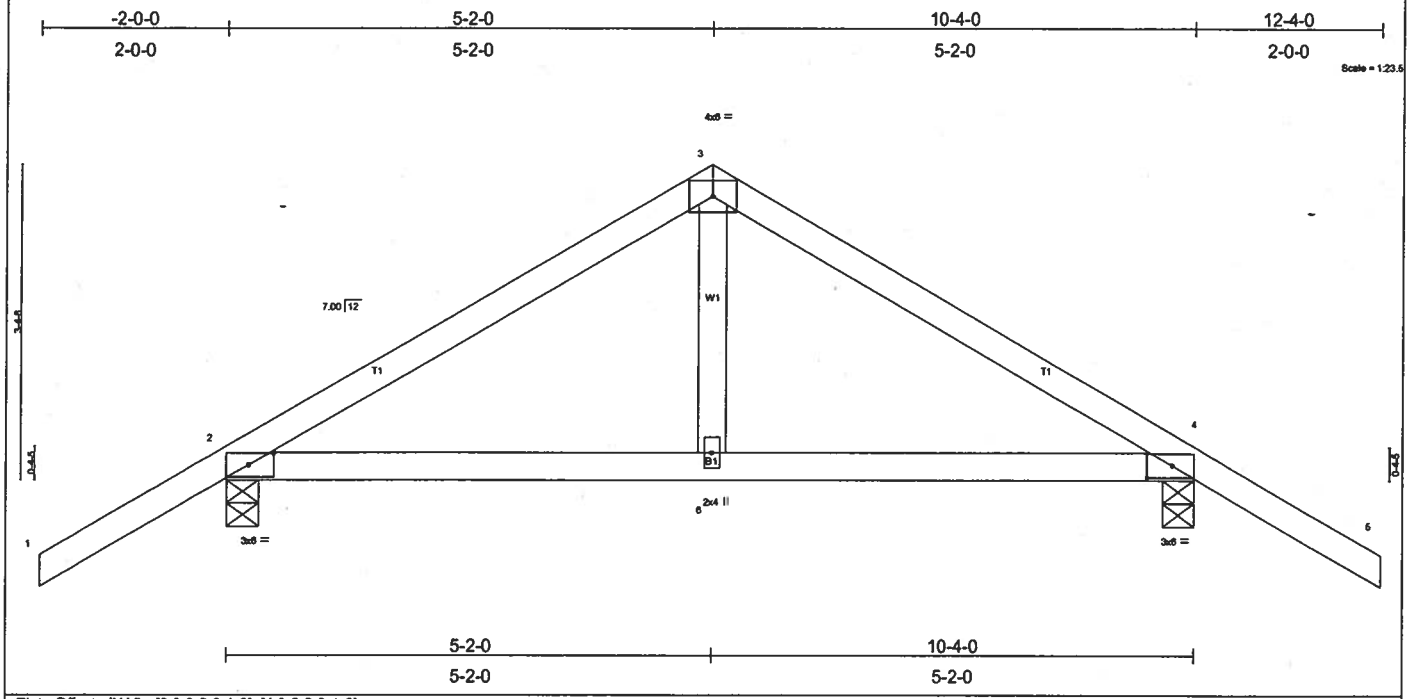

Plate Offsets (X,Y): [2:0-3-3,0-1-8], [4:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.03	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	0.03	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=537/0-4-0, 4=537/0-4-0
Max Horz 2=-112(load case 3)
Max Uplift2=-393(load case 5), 4=-393(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-493/474, 3-4=-493/474, 4-5=0/54
BOT CHORD 2-6=-248/363, 4-6=-248/363
WEBS 3-6=-283/164

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 393 lb uplift at joint 2 and 393 lb uplift at joint 4.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0203061389

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T03G	COMMON	1	1	Job Reference (optional)

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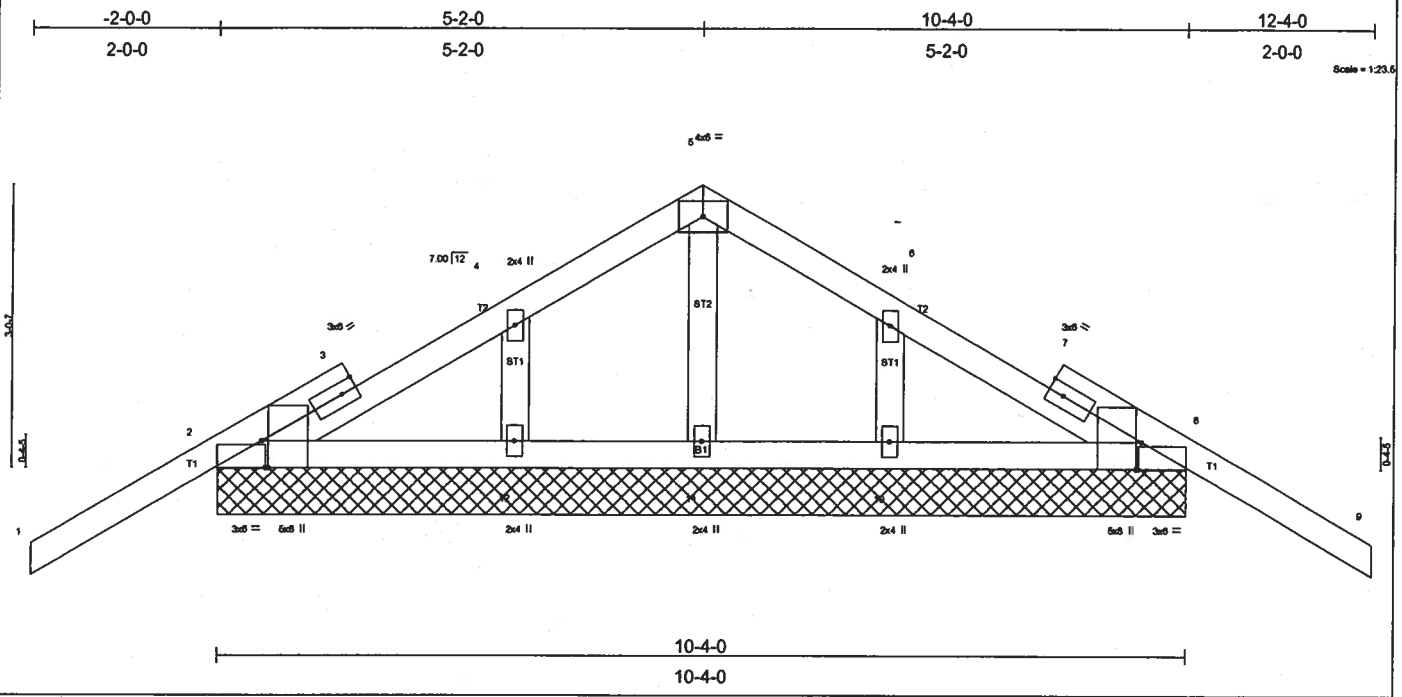

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [8:0-3-8,Edge], [8:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.04	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.06	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=507/10-4-0, 8=507/10-4-0, 11=262/10-4-0, 12=335/10-4-0, 10=335/10-4-0
Max Horz 2=-100(load case 3)
Max Uplift2=-300(load case 5), 8=-313(load case 6), 11=-40(load case 5), 12=-107(load case 5), 10=-110(load case 6)
Max Grav 2=507(load case 1), 8=507(load case 1), 11=262(load case 1), 12=338(load case 9), 10=338(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-14/112, 2-3=-81/80, 3-4=-39/81, 4-5=-61/127, 5-6=-61/127, 6-7=0/72, 7-8=-81/69, 8-9=-14/112
BOT CHORD 2-12=-14/89, 11-12=-14/89, 10-11=-14/89, 8-10=-14/89
WEBS 5-11=-211/41, 4-12=-262/139, 6-10=-262/142

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 300 lb uplift at joint 2, 313 lb uplift at joint 8, 40 lb uplift at joint 11, 107 lb uplift at joint 12 and 110 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0203061390

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T04	COMMON	1	2	Job Reference (optional)

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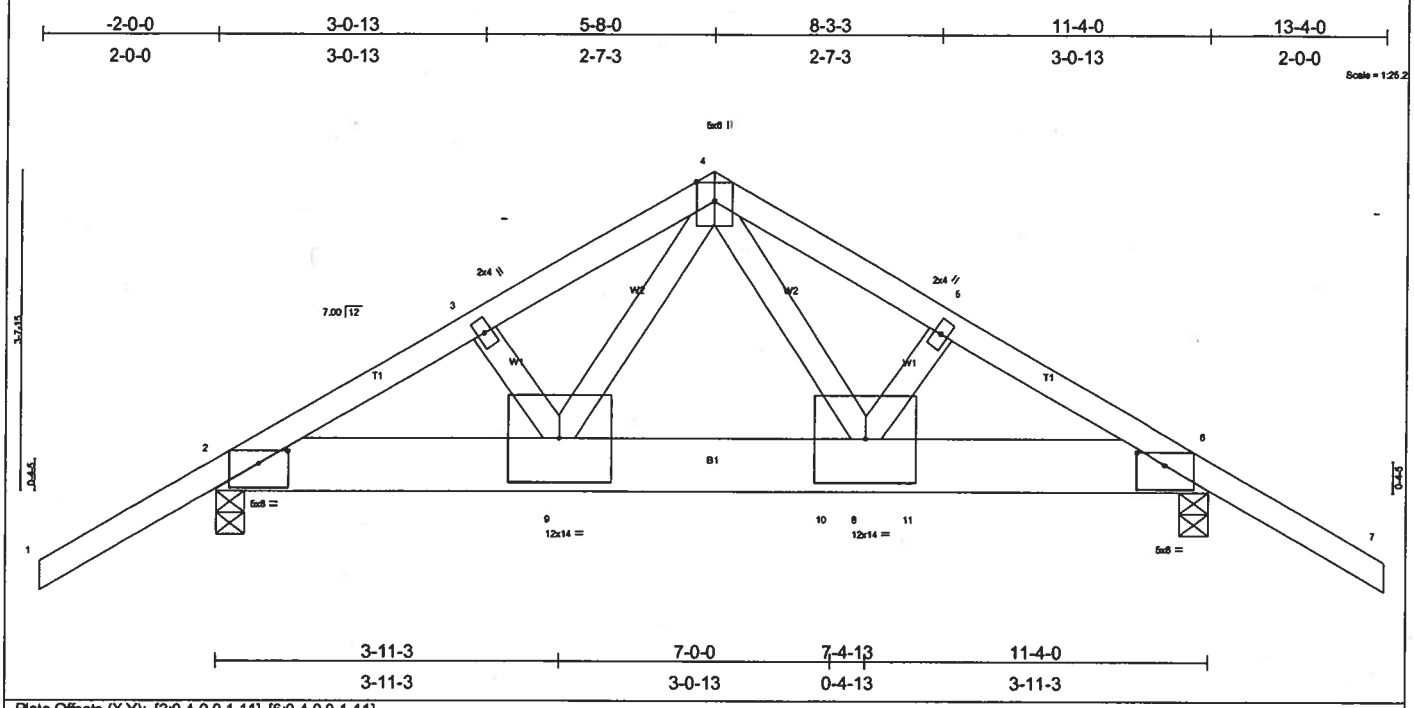

Plate Offsets (X,Y): [2:0-4-0,0-1-11], [6:0-4-0,0-1-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.06	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.09	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.79	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=2281/0-4-0, 6=4910/0-4-0
Max Horz 2=-118(load case 2)
Max Uplift2=-929(load case 4), 6=-1946(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-4004/1436, 3-4=-3923/1449, 4-5=-6285/2350, 5-6=-6354/2337, 6-7=0/59
BOT CHORD 2-9=-1186/3412, 9-10=-1052/3091, 8-10=-1052/3091, 8-11=-1955/5462, 6-11=-1955/5462
WEBS 3-9=-56/93, 4-9=-199/624, 4-8=-1837/4906, 5-8=-68/98

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 929 lb uplift at joint 2 and 1946 lb uplift at joint 6.
6) Girder carries tie-in span(s): 39-8-0 from 8-0-0 to 11-4-0
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3550 lb down and 1340 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-11=-30, 6-11=-814(F=-784)
Concentrated Loads (lb)
Vert: 10=-3550(F)

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Dwg.#0203061391

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T04G	COMMON	1	1	Job Reference (optional)

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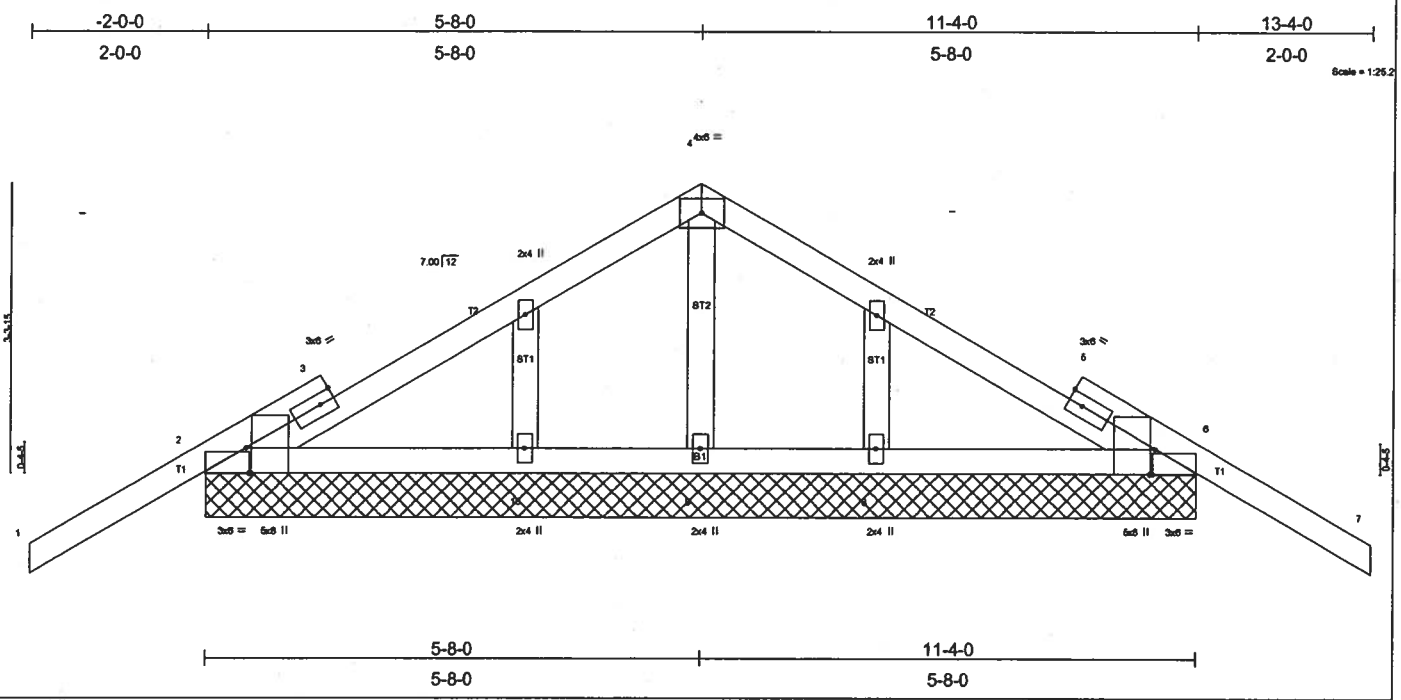

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-7,Edge], [6:0-3-8,Edge], [6:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.03	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	0.04	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=854/11-4-0, 6=854/11-4-0, 9=-76/11-4-0, 10=228/11-4-0, 8=228/11-4-0
Max Horz 2=-110(load case 3)
Max Uplift2=-439(load case 5), 6=-440(load case 6), 9=-76(load case 1), 10=-37(load case 5), 8=-37(load case 5)
Max Grav 2=854(load case 1), 6=854(load case 1), 9=54(load case 5), 10=228(load case 9), 8=228(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-14/112, 2-3=-777/327, 3-4=-673/327, 4-5=-673/327, 5-6=-777/327, 6-7=-14/112
BOT CHORD 2-10=-194/582, 9-10=-194/582, 8-9=-194/582, 6-8=-194/582

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 439 lb uplift at joint 2, 440 lb uplift at joint 6, 76 lb uplift at joint 9, 37 lb uplift at joint 10 and 37 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061392

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T05	COMMON	1	1	Job Reference (optional)

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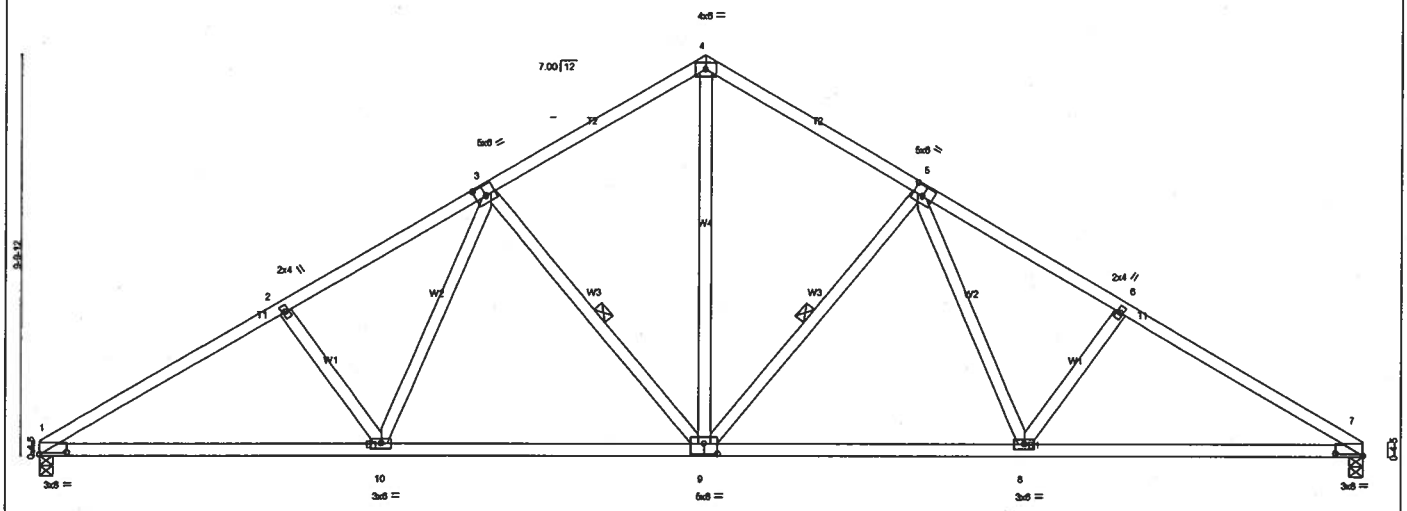
6-0-0	11-0-0	16-2-8	21-5-0	26-5-0	32-5-0
6-0-0	5-0-0	5-2-8	5-2-8	5-0-0	6-0-0

Scale = 1:53.9



8-4-0	16-2-8	24-1-0	32-5-0
8-4-0	7-10-8	7-10-8	8-4-0

Plate Offsets (X,Y): [1:0-8-1,0-0-10], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [7:0-8-1,0-0-10], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.18	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.30	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-3 oc bracing.
WEBS 1 Row at midpt 3-9, 5-9

REACTIONS (lb/size) 1=1348/0-4-0, 7=1348/0-4-0
Max Horz 1=-330(load case 3)
Max Uplift1=-433(load case 5), 7=-433(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2208/792, 2-3=-2046/797, 3-4=-1417/639, 4-5=-1417/639, 5-6=-2046/797, 6-7=-2208/792
BOT CHORD 1-10=-655/1846, 9-10=-441/1505, 8-9=-396/1505, 7-8=-594/1846
WEBS 2-10=-257/253, 3-10=-184/518, 3-9=-553/347, 4-9=-442/1061, 5-9=-553/347, 5-8=-184/518, 6-8=-257/253

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 1 and 433 lb uplift at joint 7.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0203061393

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T05A	SPECIAL	2	1	Job Reference (optional)

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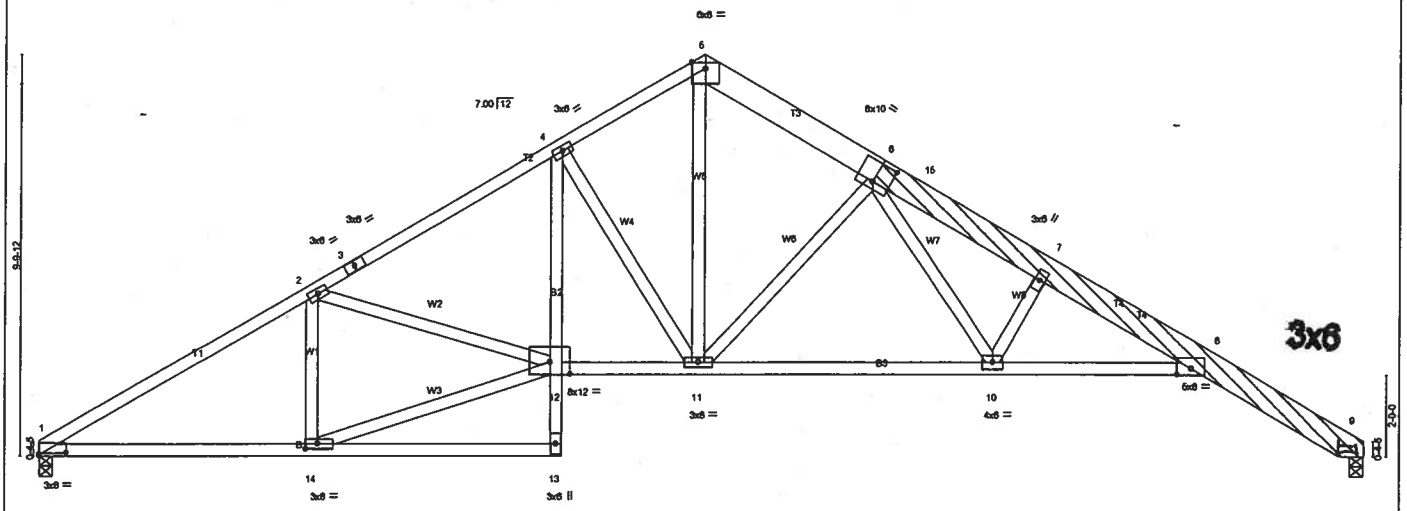
6-7-5	12-9-0	16-2-8	20-4-0	24-5-8	28-7-0	30-5-0	32-5-0
6-7-5	6-1-11	3-5-8	4-1-8	4-1-8	4-1-8	1-10-0	2-0-0

Scale = 1:53.8



6-7-5	12-9-0	18-0-5	23-3-11	28-7-0	30-5-0	32-5-0
6-7-5	6-1-11	5-3-5	5-3-5	5-3-5	1-10-0	2-0-0

Plate Offsets (X,Y): [1:0-8-1,0-0-10], [6:0-5-0,0-6-0], [14:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.42	8-10	>926	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.68	8-10	>569	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.47	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 245 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except* T2 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
LBR SCAB 6-9 2 X 8 SYP 2400F 2.0E one side

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-10 oc bracing.

REACTIONS (lb/size) 1=1343/0-4-0, 9=1280/0-4-0
Max Horz 1=-328(load case 3)
Max Uplift1=-433(load case 5), 9=-430(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2247/783, 2-3=-2300/812, 3-4=-2209/835, 4-5=-1752/722, 5-6=-1733/702, 6-15=-3216/1191, 7-15=-3317/1177, 7-8=-3346/1142, 8-9=-588/245
BOT CHORD 1-14=-637/1856, 13-14=-1/82, 12-13=0/85, 4-12=-231/687, 11-12=-504/1902, 10-11=-514/2026, 8-10=-1002/3328
WEBS 2-14=-385/236, 12-14=-672/1875, 2-12=-56/132, 4-11=-756/397, 5-11=-561/1444, 6-11=-829/411, 6-10=-488/1475, 7-10=-1176/510

NOTES
1) Attached 13-9-13 scab 6 to 9, front face(s) 2 X 8 SYP 2400F 2.0E with 2 row(s) of 0.131"x3" Nails spaced 9" o.c.except : starting at 3-9-4 from end at joint 6, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 8-3-4 from end at joint 6, nail 2 row(s) at 4 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 1 and 430 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0203061394

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T05B	SPECIAL	2	1	Job Reference (optional)

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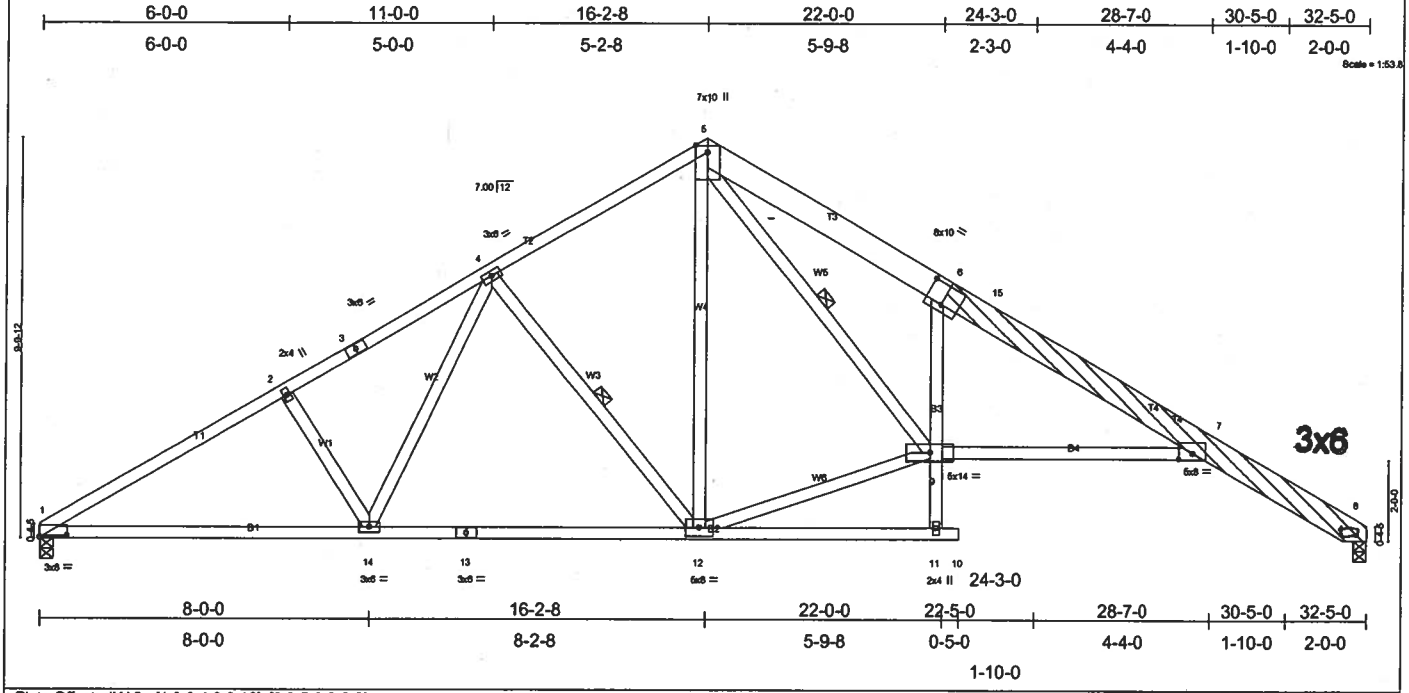

Plate Offsets (X,Y): [1:0-8-1,0-0-10], [6:0-5-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.53	7-9	>729	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.86	7-9	>447	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.51	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 237 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E "Except"
T2 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 "Except"
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
LBR SCAB 6-8 2 X 8 SYP 2400F 2.0E one side

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-9 oc bracing.
WEBS 1 Row at midpt 4-12, 5-9

REACTIONS (lb/size) 1=1349/0-4-0, 8=1292/0-4-0
Max Horz 1=-328(load case 3)
Max Uplift1=-432(load case 5), 8=-428(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2220/788, 2-3=-2075/790, 3-4=-2000/810, 4-5=-1430/640, 5-6=-2963/1175, 6-15=-2584/916, 7-15=-2739/891, 7-8=-593/244
BOT CHORD 1-14=-650/1854, 13-14=-437/1504, 12-13=-437/1504, 11-12=-12/36, 10-11=0/0, 9-11=0/91, 6-9=-1039/558, 7-9=-712/2609
WEBS 2-14=-254/254, 4-14=-199/537, 4-12=-523/335, 5-12=-141/321, 9-12=-230/1214, 5-9=-747/2088

NOTES
1) Attached 11-11-5 scab 6 to 8, front face(s) 2 X 8 SYP 2400F 2.0E with 2 row(s) of 0.131"x3" Nails spaced 9" o.c.except : starting at 0-0-0 from end at joint 6, nail 2 row(s) at 7 o.c. for 2-0-0; starting at 6-4-12 from end at joint 6, nail 2 row(s) at 7 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 432 lb uplift at joint 1 and 428 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061395

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T05C	COMMON	1	2	Job Reference (optional)

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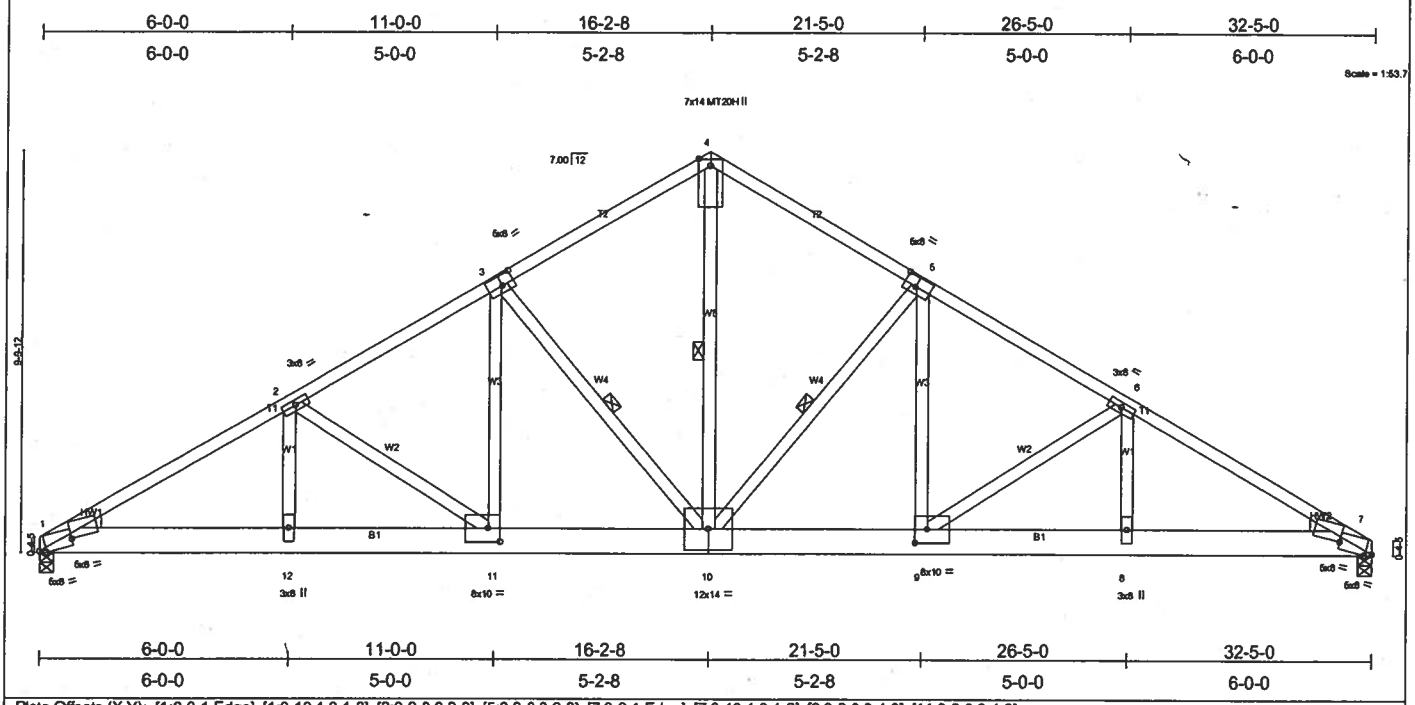


Plate Offsets (X,Y): [1:0-2-1,Edge], [1:0-10-1,0-1-2], [3:0-3-8,0-3-0], [5:0-3-8,0-3-0], [7:0-2-1,Edge], [7:0-10-1,0-1-2], [9:0-3-8,0-4-0], [11:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.37	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.59	9-10	>656	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.92	Horz(TL)	0.14	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 474 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D, T1 2 X 4 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-10, 4-10, 5-10

REACTIONS (lb/size) 1=10128/0-4-4, 7=10128/0-4-4
Max Horz 1=324(load case 3)
Max Uplift1=-3871(load case 4), 7=-3871(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-16758/6388, 2-3=-13533/5205, 3-4=-10340/4055, 4-5=-10340/4055, 5-6=-13533/5205, 6-7=-16758/6390
BOT CHORD 1-12=-5566/14439, 11-12=-5566/14439, 10-11=-4392/11627, 9-10=-4302/11627, 8-9=-5436/14439, 7-8=-5436/14439
WEBS 2-12=-1173/3209, 2-11=-3399/1418, 3-11=-1791/4687, 3-10=-4315/1792, 4-10=-3881/10010, 5-10=-4315/1792, 5-9=-1791/4687, 6-9=-3399/1419, 6-8=-1174/3209

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3871 lb uplift at joint 1 and 3871 lb uplift at joint 7.
7) Girder carries tie-in span(s): 28-5-0 from 0-0-0 to 32-5-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 1-7=-578(F=-548)

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

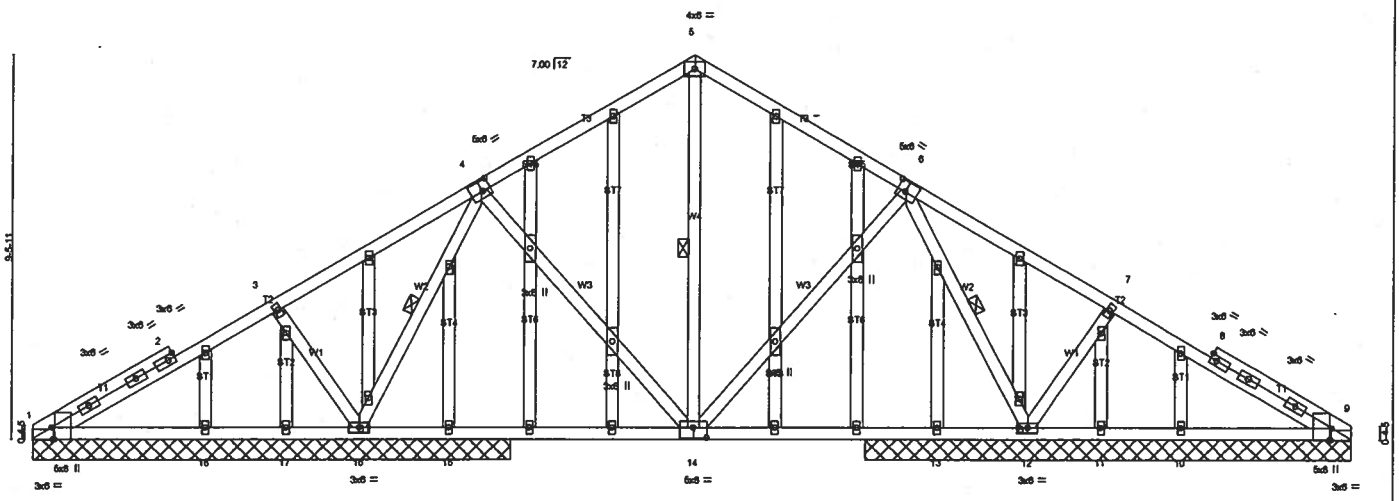
Dwg.#0203061396

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T05G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:53 2006 Page 1

6-0-0	11-0-0	16-2-8	21-5-0	26-5-0	32-5-0
6-0-0	5-0-0	5-2-8	5-2-8	5-0-0	6-0-0

Scale = 1:54.1



8-0-0	16-2-8	24-5-0	32-5-0
8-0-0	8-2-8	8-2-8	8-0-0

Plate Offsets (X,Y): [1:0-3-8,Edge], [1:0-0-7,Edge], [4:0-2-8,0-3-0], [6:0-2-8,0-3-0], [9:0-3-8,Edge], [9:0-0-7,Edge], [14:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.03	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.04	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.32	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-16, 5-14, 6-12

REACTIONS (lb/size) 1=360/11-9-0, 16=1620/11-9-0, 12=1620/12-0-0, 9=360/12-0-0, 15=160/11-9-0, 17=7/11-9-0, 18=186/11-9-0, 13=160/12-0-0, 11=7/12-0-0, 10=186/12-0-0
Max Horz 1=-318(load case 3)
Max Uplift1=-120(load case 6), 16=-782(load case 5), 12=-755(load case 6), 9=-121(load case 6), 18=-25(load case 5), 10=-26(load case 6)
Max Grav 1=364(load case 9), 16=1620(load case 1), 12=1620(load case 1), 9=364(load case 10), 15=160(load case 1), 17=34(load case 5), 18=187(load case 9), 13=160(load case 1), 11=34(load case 6), 10=187(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-226/157, 2-3=-163/151, 3-4=-184/429, 4-5=-632/324, 5-6=-632/320, 6-7=-94/429, 7-8=-41/8, 8-9=-226/15
BOT CHORD 1-18=-137/178, 17-18=-137/178, 16-17=-137/178, 15-16=-137/292, 14-15=-137/292, 13-14=-52/292, 12-13=-52/292, 11-12=-14/79, 10-11=-14/79, 9-10=-14/79
WEBS 3-16=-586/374, 4-16=-1233/525, 4-14=-24/205, 5-14=-117/43, 6-14=-68/205, 6-12=-1233/493, 7-12=-586/376

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 1, 782 lb uplift at joint 16, 755 lb uplift at joint 12, 121 lb uplift at joint 9, 25 lb uplift at joint 18 and 26 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 1-9=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0203061397

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T06	SPECIAL	1	2	Job Reference (optional)

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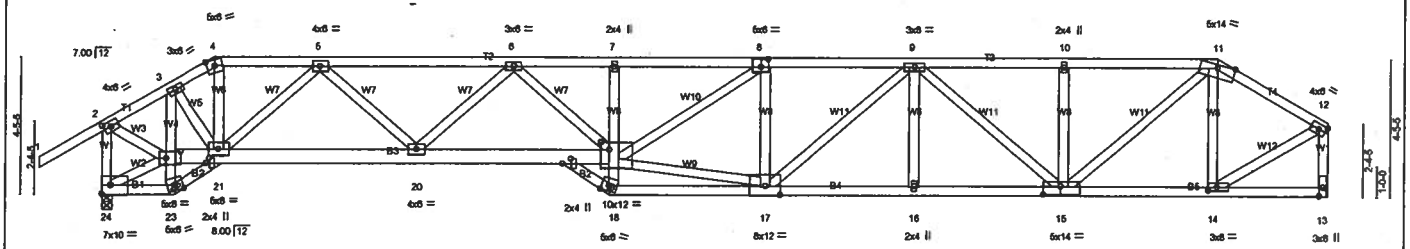
-2-0-0 | 2-4-0 | 3-6-14 | 6-11-12 | 13-2-9 | 16-4-0 | 21-4-3 | 26-2-9 | 31-1-0 | 36-1-2 | 39-8-0

2-0-0 | 2-4-0 | 1-2-14 | 3-4-15 | 6-2-13 | 3-1-7 | 5-0-3 | 4-10-7 | 4-10-7 | 5-0-3 | 3-6-14

Scale = 1:71.2



16-5-12

2-4-0 | 3-10-0 | 10-1-3 | 14-10-0 | 16-4-0 | 21-4-3 | 26-2-9 | 31-1-0 | 36-1-2 | 39-8-0

2-4-0 | 1-6-0 | 6-3-3 | 4-8-13 | 1-6-0 | 4-10-7 | 4-10-7 | 4-10-7 | 5-0-3 | 3-6-14

0-1-12

Plate Offsets (X,Y): [2:0-2-14,0-2-0], [8:0-2-12,0-3-0], [14:0-3-8,0-1-8], [15:0-6-12,0-3-4], [19:0-3-4,0-2-12], [21:0-2-0,0-1-2], [22:0-5-8,0-2-8], [25:0-2-0,0-3-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	0.67	19	>704	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-1.03	17-18	>461	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.98	Horz(TL)	0.30	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 530 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W9 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-5 oc bracing.

REACTIONS (lb/size) 13=4067/Mechanical, 24=4345/0-4-0
Max Horz 24=-239(load case 2)
Max Uplift13=-2294(load case 2), 24=-2267(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-4368/2366, 3-4=-5188/2851, 4-5=-4601/2567, 5-6=-10768/6091, 6-7=-14782/8685, 7-8=-14670/8626, 8-9=-10647/6292, 9-10=-6827/4112, 10-11=-6839/4123, 11-12=-3847/2226, 2-24=-4231/2230, 12-13=-3955/2300
BOT CHORD 23-24=-38/76, 21-22=-1871/3587, 20-21=-4393/8029, 19-20=-7409/13068, 17-18=-207/377, 16-17=-5309/9222, 15-16=-5309/9222, 14-15=-1768/3188, 13-14=-66/87
WEBS 22-23=-11/46, 3-22=-1781/995, 3-21=-949/1600, 4-21=-1273/2316, 5-21=-4713/2771, 5-20=-2186/3848, 6-20=-3232/2114, 6-19=-1594/2410, 18-19=-7/181, 7-19=-301/313, 8-17=-3107/2071, 9-17=-1076/1883, 9-16=0/299, 9-15=-3178/1868, 10-15=-575/600, 11-15=-2781/4720, 11-14=-1601/1113, 2-22=-2114/4022, 12-14=-2044/3676, 8-19=-2673/4626, 17-19=-6131/10619, 22-24=-40/250

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2294 lb uplift at joint 13 and 2267 lb uplift at joint 24.
8) Girder carries tie-in span(s): 4-7-9 from 2-4-0 to 16-4-0; 4-2-2 from 2-4-0 to 16-4-0
9) Girder carries hip end with 0-0-0 right side setback, 16-4-0 left side setback, and 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1258 lb down and 873 lb up at 16-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-117(F=-63), 3-4=-104(F=-50), 4-7=-104(F=-50), 7-11=-117(F=-63), 11-12=-117(F=-63), 23-24=-65(F=-35), 21-22=-71(F=-41), 19-21=-71(F=-41), 13-18=-65(F=-35)
Concentrated Loads (lb)
Vert: 19=-1258(F)

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Dwg.#0203061398

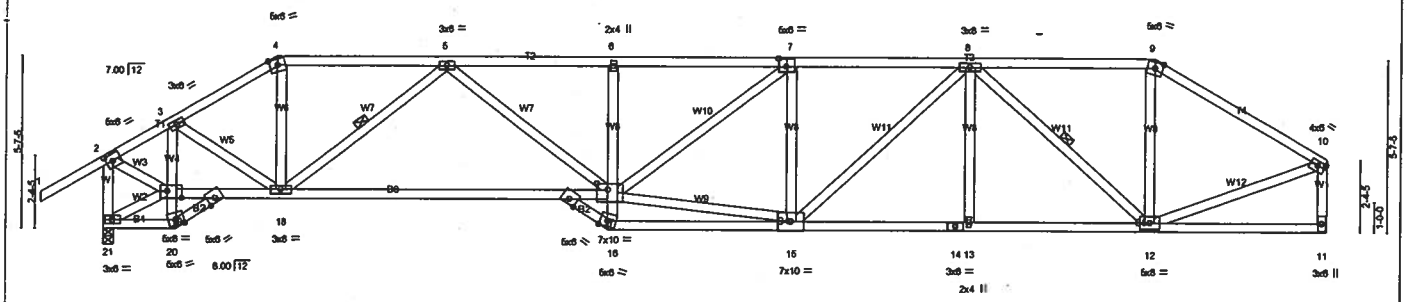
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:56 2006 Page 1

-2-0-0 | 2-4-0 | 5-6-14 | 11-1-3 | 16-4-0 | 22-3-1 | 28-1-4 | 34-1-2 | 39-8-0

2-0-0 | 2-4-0 | 3-2-14 | 5-6-5 | 5-2-13 | 5-11-1 | 5-10-3 | 5-11-15 | 5-6-14

Scale = 1:71.2



5-6-14

2-4-0 | 3-10-0 | 14-10-0 | 16-4-0 | 25-2-9 | 34-1-2 | 39-8-0

2-4-0 | 1-6-0 | 1-8-14 | 9-3-2 | 1-6-0 | 8-10-9 | 8-10-9 | 5-6-14

Plate Offsets (X,Y): [2:0-2-9,0-2-8], [7:0-3-0,0-3-0], [10:Edge,0-1-12], [17:0-4-4,0-2-8], [19:0-5-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.56	17-18	>847	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.94	17-18	>503	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.18	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 256 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-10 oc bracing.
WEBS 1 Row at midpt 5-18, 8-12

REACTIONS (lb/size) 11=1649/Mechanical, 21=1766/0-4-0
Max Horz 21=200(load case 4)
Max Uplift11=-634(load case 3), 21=-665(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1615/751, 3-4=-2192/948, 4-5=-1904/866, 5-6=-3646/1686, 6-7=-3634/1684, 7-8=-3044/1376, 8-9=-1525/700, 9-10=-1818/754, 2-21=-1710/797, 10-11=-1563/645
BOT CHORD 20-21=-15/32, 18-19=-719/1314, 17-18=-1449/2968, 15-16=-43/106, 14-15=-1119/2552, 13-14=-1119/2552, 12-13=-1119/2552, 11-12=-54/91
WEBS 19-20=-9/28, 3-19=-739/253, 3-18=-351/688, 4-18=-291/828, 5-18=-1391/820, 5-17=-398/898, 16-17=0/85, 6-17=-295/252, 7-17=-397/739, 7-15=-750/498, 8-15=-346/683, 8-12=-1427/729, 9-12=-225/616, 2-19=-633/1526, 10-12=-645/1518, 8-13=0/170, 15-17=-1351/2980, 19-21=-163/129

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 634 lb uplift at joint 11 and 665 lb uplift at joint 21.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061399

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T08	SPECIAL	1	1	Job Reference (optional)

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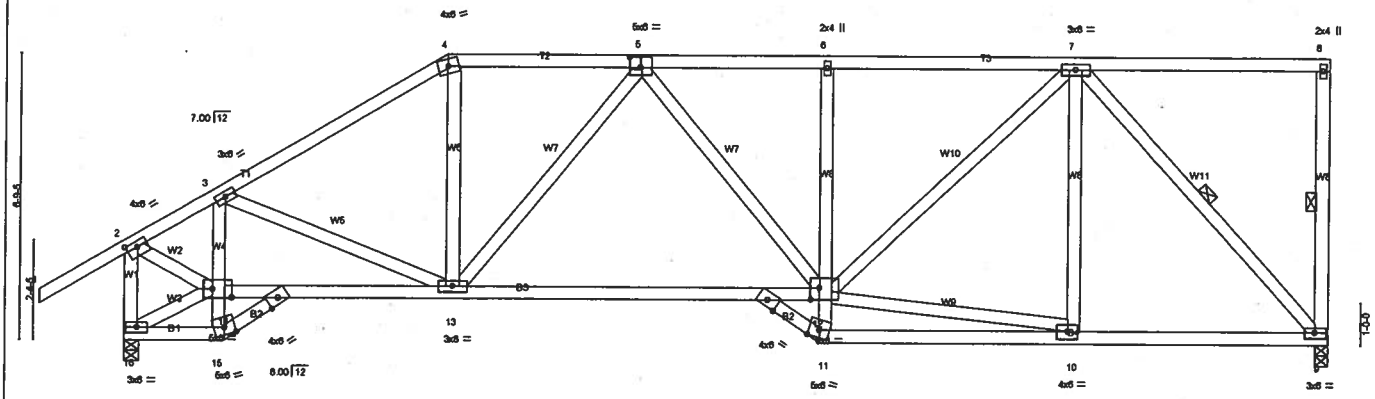
-2-0-0 | 2-4-0 | 7-6-14 | 16-4-0 | 22-4-8 | 28-5-0
2-0-0 | 2-4-0 | 5-2-14 | 8-9-2 | 6-0-8 | 6-0-8

Scale = 1:51.9



2-4-0 | 3-10-0 | 7-6-14 | 14-10-0 | 16-4-0 | 22-4-8 | 28-5-0
2-4-0 | 1-6-0 | 3-8-14 | 7-3-2 | 1-6-0 | 6-0-8 | 6-0-8

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [5:0-3-0,0-3-0], [12:0-2-8,0-3-4], [14:0-5-8,Edge]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.44	Vert(LL)	-0.23	12-13	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.63	Vert(TL)	-0.39	12-13	>863	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.48	Horz(TL)	0.07	9	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						Weight: 205 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-5 oc bracing.
WEBS 1 Row at midpt 8-9, 7-9

REACTIONS (lb/size) 9=1174/0-3-8, 16=1296/0-4-0
Max Horz 16=331(load case 5)
Max Uplift9=-500(load case 3), 16=-429(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1143/577, 3-4=-1484/601, 4-5=-1233/583, 5-6=-1482/654, 6-7=-1484/657, 7-8=-23/11, 8-9=-143/118, 2-16=-1240/663
BOT CHORD 15-16=-17/25, 13-14=-656/946, 12-13=-677/1442, 10-11=-10/37, 9-10=-374/887
WEBS 14-15=-14/33, 3-14=-466/105, 3-13=-221/298, 4-13=-126/420, 11-12=0/75, 6-12=-277/226, 7-12=-382/806, 7-10=0/108, 7-9=-1284/542, 14-16=-257/26, 5-13=-336/342, 5-12=-65/64, 10-12=-368/862, 2-14=-456/1119

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 500 lb uplift at joint 9 and 429 lb uplift at joint 16.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061400

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T09	SPECIAL	1	1	Job Reference (optional)

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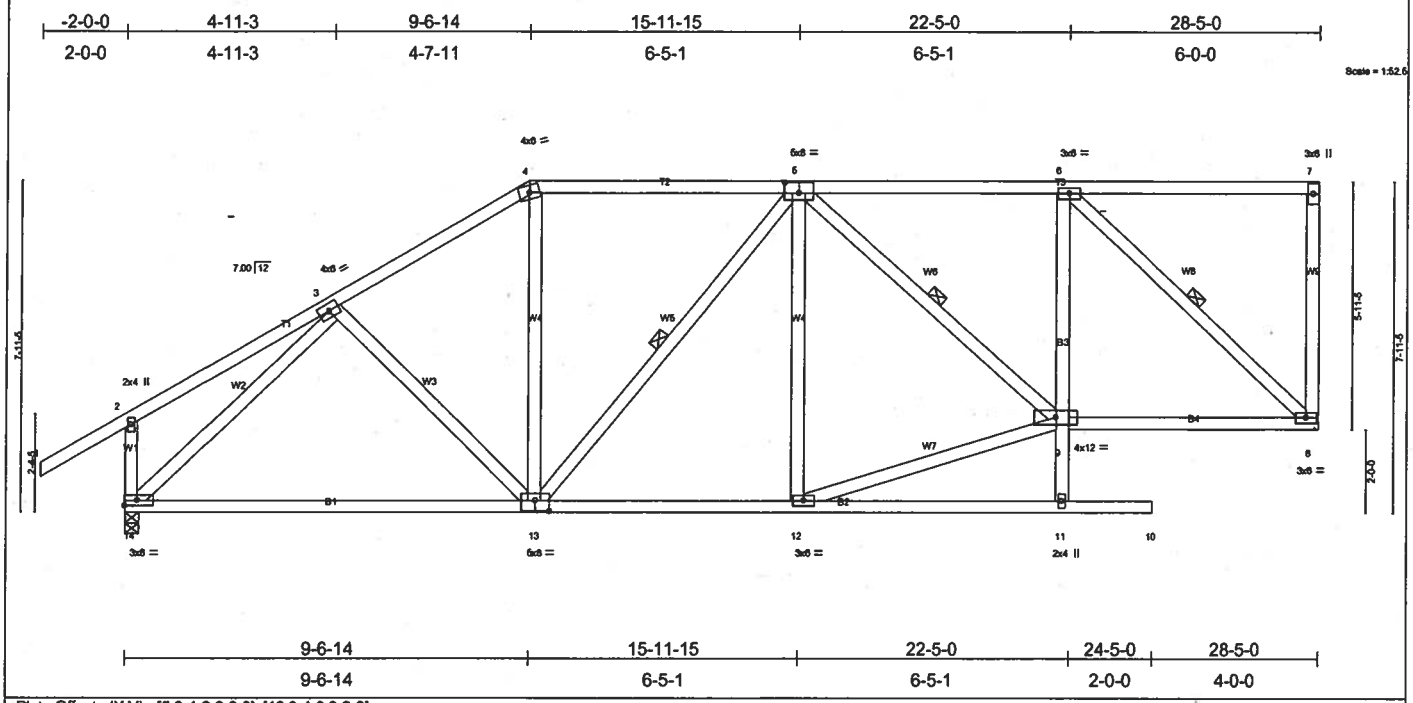

Plate Offsets (X,Y): [5:0-4-0,0-3-0], [13:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.17	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.29	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-4 oc bracing.
WEBS 1 Row at midpt 5-13, 5-9, 6-8

REACTIONS (lb/size) 8=1230/Mechanical, 14=1313/0-4-0
Max Horz 14=385(load case 5)
Max Uplift8=-453(load case 3), 14=-431(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-214/169, 3-4=-1232/491, 4-5=-1022/481, 5-6=-1077/435, 6-7=-34/14, 7-8=-149/122, 2-14=-335/296
BOT CHORD 13-14=-549/920, 12-13=-490/1118, 11-12=-19/18, 10-11=0/0, 9-11=0/165, 6-9=-147/648, 8-9=-436/1086
WEBS 3-13=-162/150, 4-13=-29/276, 5-13=-152/219, 5-12=-194/189, 9-12=-494/1157, 5-9=-55/75, 6-8=-1445/581, 3-14=-1153/340

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 8 and 431 lb uplift at joint 14.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0203061401

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T10	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:37:59 2006 Page 1

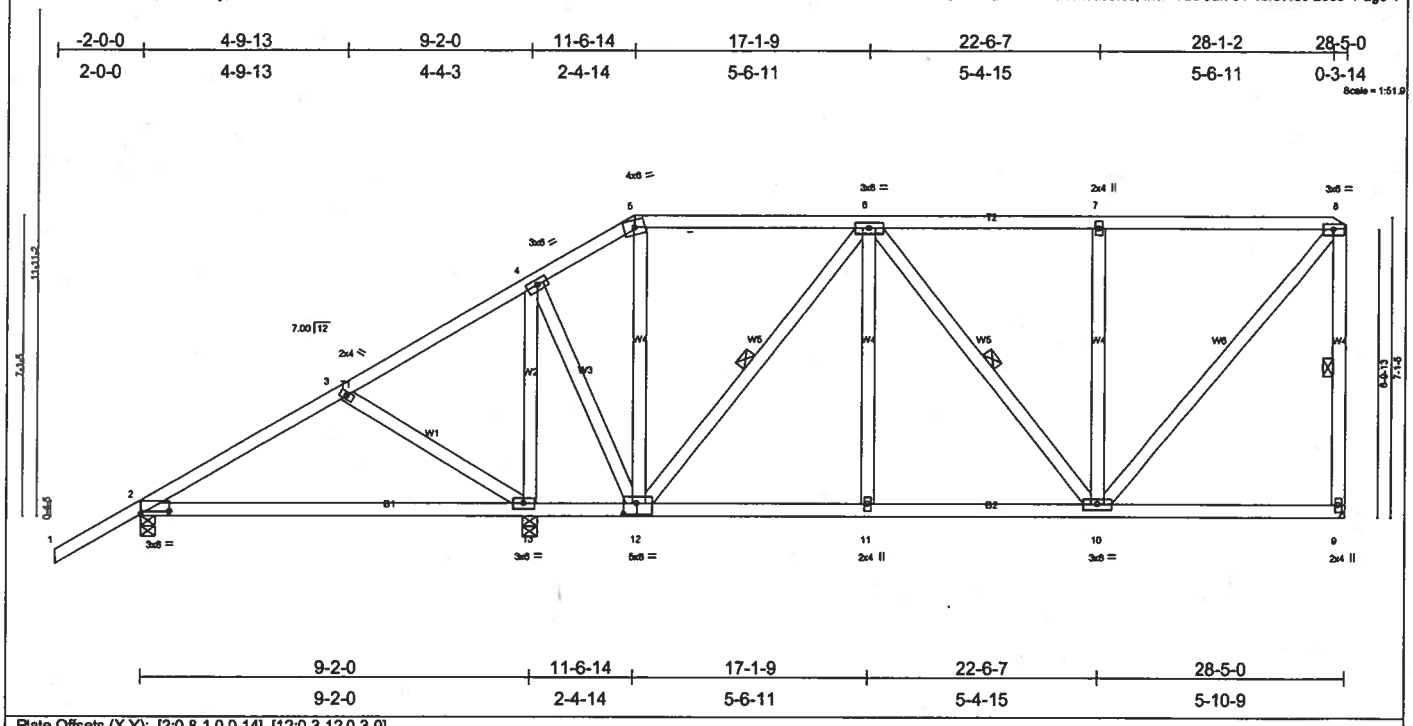

Plate Offsets (X,Y): [2:0-8-1,0-0-14], [12:0-3-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.11	2-13	>968	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.20	2-13	>546	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 191 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 12-13.
WEBS 1 Row at midpt 6-12, 6-10, 8-9

REACTIONS (lb/size) 2=423/0-4-0, 13=1293/0-4-0, 9=762/Mechanical
Max Horz 2=387(load case 5)
Max Uplift2=-176(load case 5), 13=-427(load case 5), 9=-373(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-201/0, 3-4=-139/167, 4-5=-225/116, 5-6=-173/114, 6-7=-471/233, 7-8=-471/233, 8-9=-678/383
BOT CHORD 2-13=-201/143, 12-13=-99/9, 11-12=-262/513, 10-11=-262/513, 9-10=-11/22
WEBS 3-13=-285/219, 4-13=-898/468, 4-12=-287/649, 5-12=-124/96, 6-12=-545/241, 6-11=0/176, 6-10=-67/50, 7-10=-314/270, 8-10=-345/698

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 2, 427 lb uplift at joint 13 and 373 lb uplift at joint 9.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061402

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T11	HIP	1	1	Job Reference (optional)

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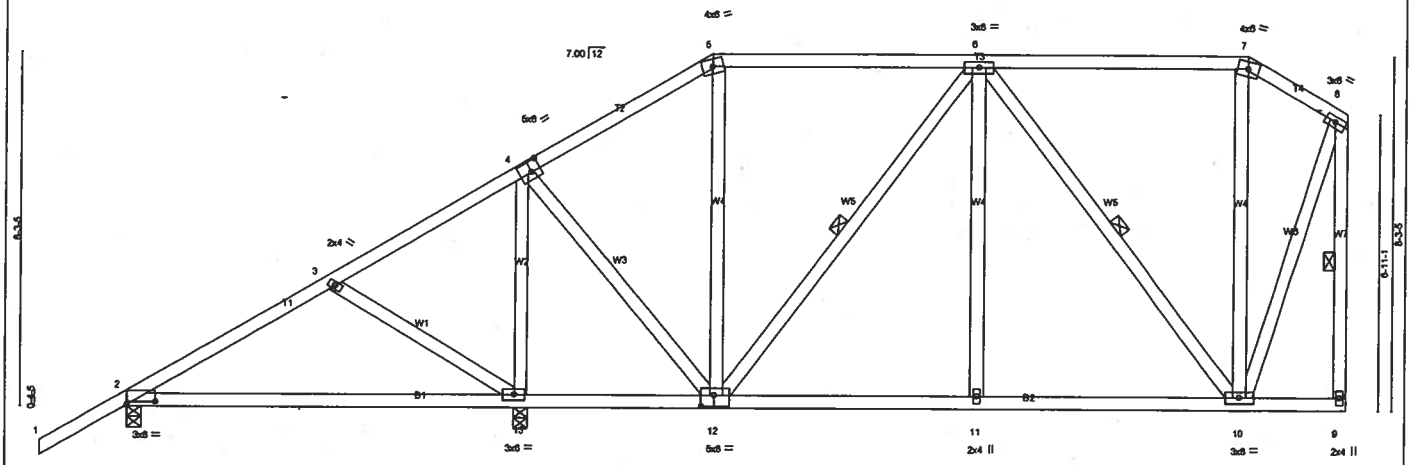
-2-0-0 | 4-9-13 | 9-2-0 | 13-6-14 | 19-10-0 | 26-1-2 | 28-5-0
2-0-0 | 4-9-13 | 4-4-3 | 4-4-14 | 6-3-2 | 6-3-2 | 2-3-14

Scale = 1:51.4



9-2-0 | 13-6-14 | 19-10-0 | 26-1-2 | 28-5-0
9-2-0 | 4-4-14 | 6-3-2 | 6-3-2 | 2-3-14

Plate Offsets (X,Y): [2:0-8-1,0-0-14], [4:0-2-8,0-3-0], [12:0-3-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.13	2-13	>856	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.22	2-13	>495	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 12-13.
WEBS 1 Row at midpt 6-12, 6-10, 8-9

REACTIONS (lb/size) 2=433/0-4-0, 13=1279/0-4-0, 9=766/Mechanical
Max Horz 2=399(load case 5)
Max Uplift2=-173(load case 5), 13=-449(load case 5), 9=-295(load case 3)
Max Grav 2=435(load case 9), 13=1279(load case 1), 9=766(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-219/0, 3-4=-141/150, 4-5=-403/174, 5-6=-301/199, 6-7=-215/138, 7-8=-248/114, 8-9=-752/304
BOT CHORD 2-13=-206/158, 12-13=-82/29, 11-12=-235/474, 10-11=-235/474, 9-10=-1/2
WEBS 3-13=-278/210, 4-13=-899/460, 4-12=-229/599, 5-12=-115/119, 6-12=-305/166, 6-11=0/204, 6-10=-435/238, 7-10=-143/138, 8-10=-286/657

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 173 lb uplift at joint 2, 449 lb uplift at joint 13 and 295 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061403

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:01 2006 Page 1

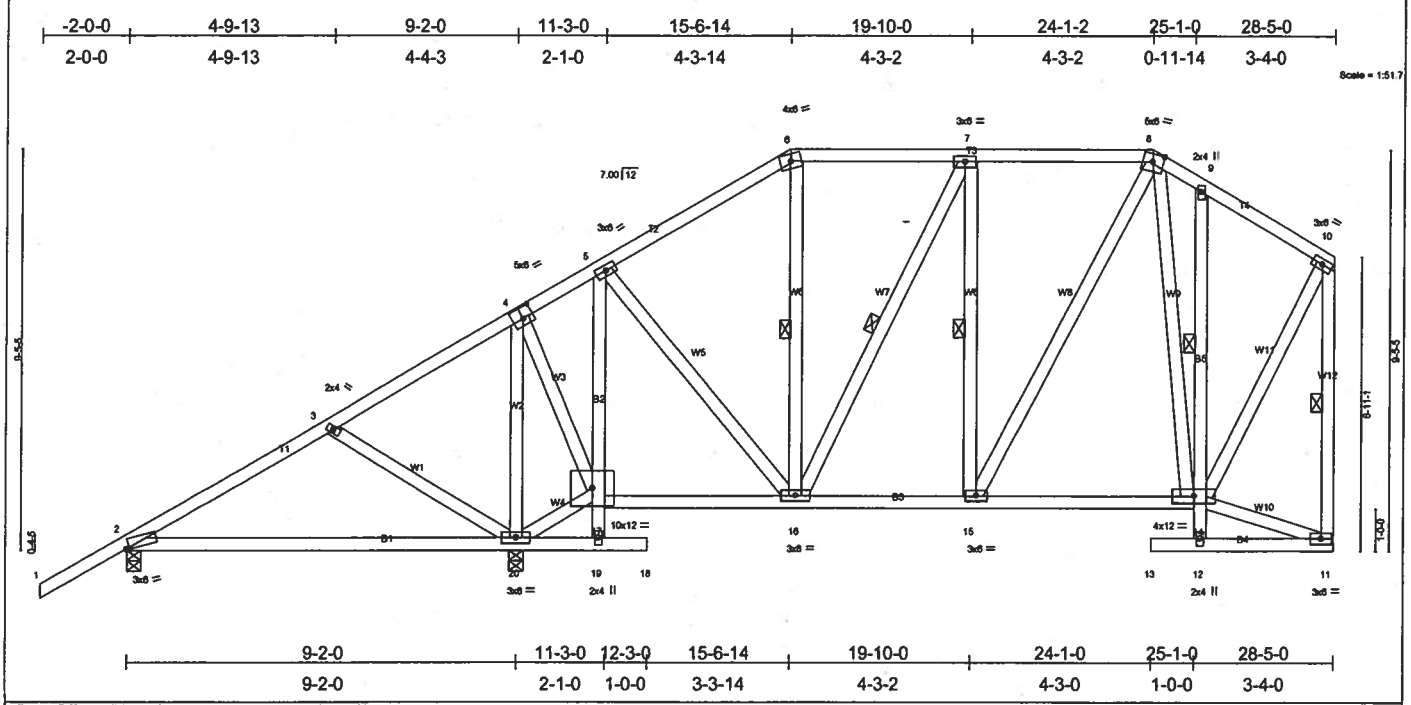


Plate Offsets (X,Y): [2:0-0-14,0-0-3], [4:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.25	Vert(LL) 0.25 2-20 >426 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.40	Vert(TL) 0.21 2-20 >526 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.46	Horz(TL) 0.02 11 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 237 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 9-14
WEBS 1 Row at midpt 6-16, 7-16, 7-15, 10-11

REACTIONS (lb/size) 2=385/0-4-0, 20=1386/0-4-0, 11=776/Mechanical
Max Horz 2=415(load case 5)
Max Uplift2=-236(load case 5), 20=-641(load case 5), 11=-193(load case 3)
Max Grav 2=388(load case 9), 20=1386(load case 1), 11=776(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-136/56, 3-4=-204/239, 4-5=-152/40, 5-6=-476/189, 6-7=-361/212, 7-8=-433/241, 8-9=-328/193, 9-10=-361/143, 10-11=-746/265
BOT CHORD 2-20=-205/87, 19-20=-85/55, 18-19=0/0, 17-19=-69/189, 5-17=-594/309, 16-17=-70/117, 15-16=-169/433, 14-15=-97/311, 12-14=0/81, 9-14=-98/112, 12-13=0/0, 11-12=-46/0
WEBS 3-20=-283/279, 4-20=-889/345, 17-20=-88/21, 4-17=-148/610, 5-16=-147/387, 6-16=-41/92, 7-16=-205/155, 7-15=-107/167, 8-15=-155/286, 8-14=-267/230, 10-14=-166/572, 11-14=0/55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2, 641 lb uplift at joint 20 and 193 lb uplift at joint 11.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Dwg.#0203061404

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T14	SPECIAL	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:02 2006 Page 1

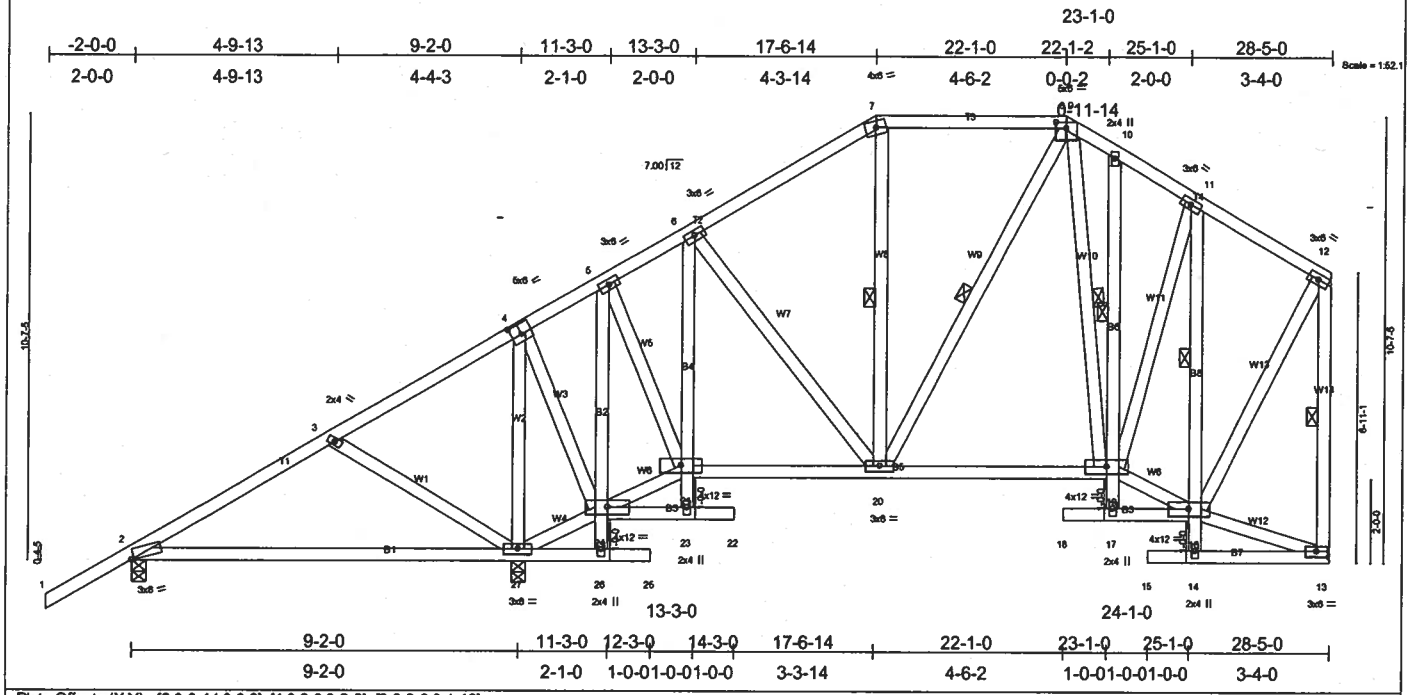


Plate Offsets (X,Y): [2:0-0-14,0-0-3], [4:0-3-0,0-3-0], [9:0-3-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.25	2-27	>427	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	0.20	2-27	>527	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.04	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 262 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3, B6 2 X 4 SYP No.3, B8 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-9.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-27,26-27.
1 Row at midpt 10-19, 11-16
WEBS 1 Row at midpt 7-20, 8-20, 12-13, 9-19
JOINTS 1 Brace at Jt(s): 24, 16

REACTIONS (lb/size) 2=354/0-4-0, 27=1468/0-4-0, 13=799/Mechanical
Max Horz 2=428(load case 5)
Max Uplift2=-225(load case 5), 27=-661(load case 5), 13=-198(load case 6)
Max Grav 2=365(load case 9), 27=1468(load case 1), 13=799(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-136/102, 3-4=-223/295, 4-5=-113/43, 5-6=-361/98, 6-7=-540/237, 7-8=-416/254, 8-9=-397/240, 9-10=-446/255, 10-11=-488/233, 11-12=-365/137, 12-13=-755/252
BOT CHORD 2-27=-199/89, 26-27=-83/95, 25-26=0/0, 24-26=-70/194, 5-24=-731/264, 23-24=-23/6, 22-23=0/0, 21-23=0/60, 6-21=-385/221, 20-21=-122/312, 19-20=-88/392, 17-19=0/67, 10-19=-66/51, 17-18=0/0, 16-17=-45/0, 14-16=0/83, 11-16=-563/193, 14-15=0/0, 13-14=0/3
WEBS 3-27=-279/275, 4-27=-948/371, 24-27=-151/16, 4-24=-146/646, 21-24=-54/99, 5-21=-192/596, 6-20=-121/187, 7-20=-35/121, 8-20=-87/112, 16-19=-61/357, 11-19=-110/352, 12-16=-157/580, 13-16=-5/5, 9-19=-75/196

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 2, 661 lb uplift at joint 27 and 198 lb uplift at joint 13.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061405

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T15	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:04 2006 Page 1

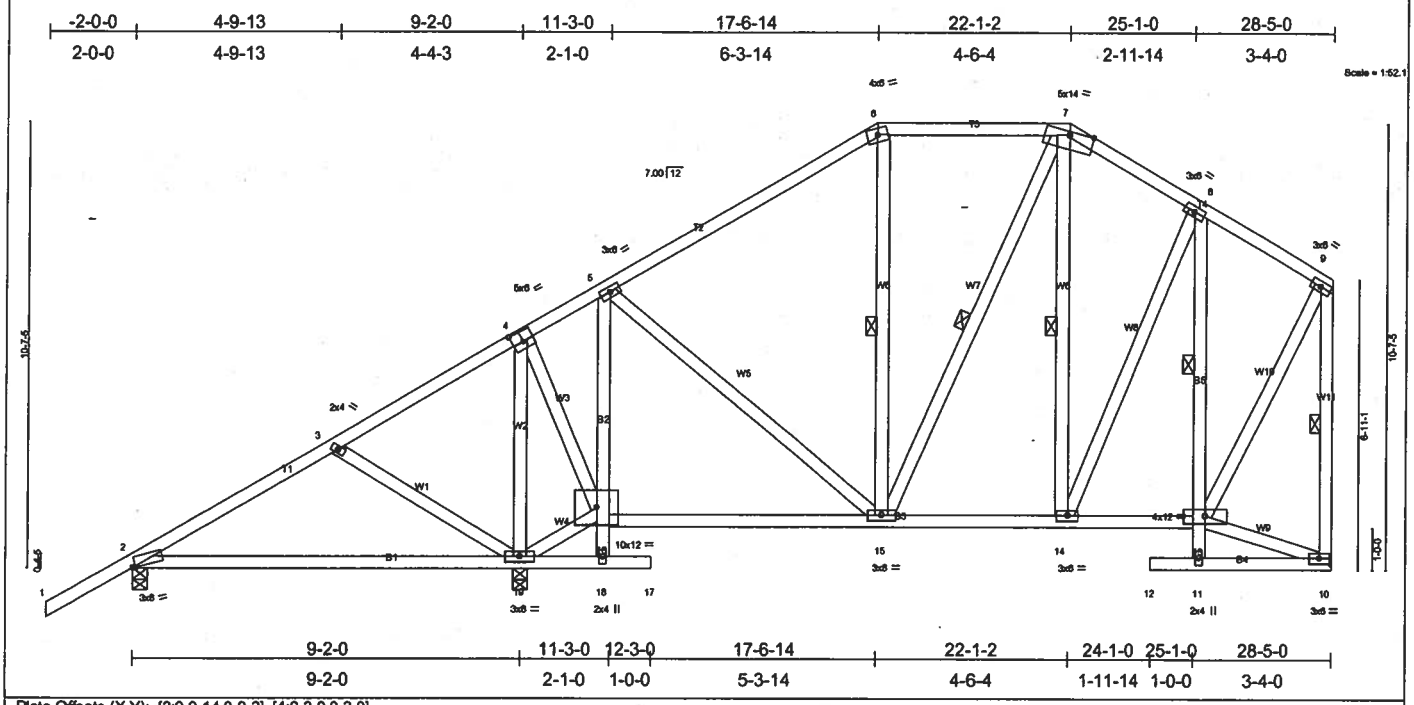


Plate Offsets (X,Y): [2:0-0-14,0-0-3], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.25	2-19	>425	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	0.21	2-19	>526	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 232 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 8-13
WEBS 1 Row at midpt 6-15, 7-15, 7-14, 9-10

REACTIONS (lb/size) 2=385/0-4-0, 19=1386/0-4-0, 10=776/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-230(load case 5), 19=-658(load case 5), 10=-207(load case 6)
Max Grav 2=393(load case 9), 19=1386(load case 1), 10=776(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-146/81, 3-4=-214/237, 4-5=-153/51, 5-6=-526/218, 6-7=-376/256, 7-8=-440/243, 8-9=-356/141, 9-10=-734/261
BOT CHORD 2-19=-213/95, 18-19=-124/36, 17-18=0/0, 16-18=-73/187, 5-16=-598/343, 15-16=-94/146, 14-15=-85/342, 13-14=-83/277, 11-13=0/83, 8-13=-364/177, 11-12=0/0, 10-11=-4/1
WEBS 3-19=-284/280, 4-19=-894/351, 16-19=-69/50, 4-16=-196/660, 5-15=-103/307, 6-15=-124/166, 7-15=-106/132, 7-14=-75/138, 8-14=-130/191, 9-13=-167/564, 10-13=-3/11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2, 658 lb uplift at joint 19 and 207 lb uplift at joint 10.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0203061406

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T16	HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:05 2006 Page 1

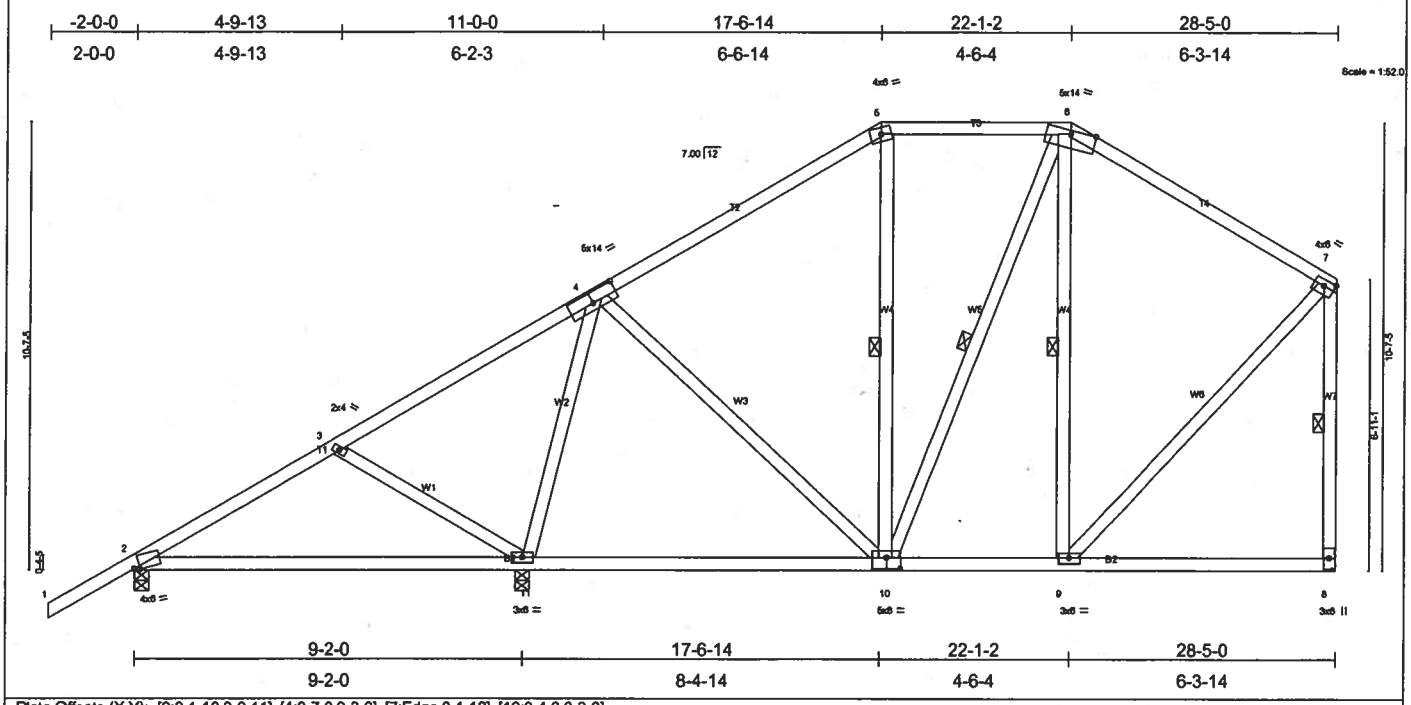


Plate Offsets (X,Y): [2:0-1-10,0-0-11], [4:0-7-0,0-3-0], [7:Edge,0-1-12], [10:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.35	2-11	>316	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	0.30	2-11	>364	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-10, 6-10, 6-9, 7-8

REACTIONS (lb/size) 2=439/0-4-0, 11=1277/0-4-0, 8=762/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-253(load case 5), 11=-639(load case 5), 8=-216(load case 6)
Max Grav 2=445(load case 9), 11=1277(load case 1), 8=762(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-251/57, 3-4=-192/181, 4-5=-505/207, 5-6=-350/248, 6-7=-458/209, 7-8=-664/285
BOT CHORD 2-11=-250/187, 10-11=-106/112, 9-10=-92/322, 8-9=-17/30
WEBS 3-11=-302/308, 4-11=-857/444, 4-10=-63/322, 5-10=-141/187, 6-10=-110/144, 6-9=-168/101, 7-9=-122/429

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2, 639 lb uplift at joint 11 and 216 lb uplift at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

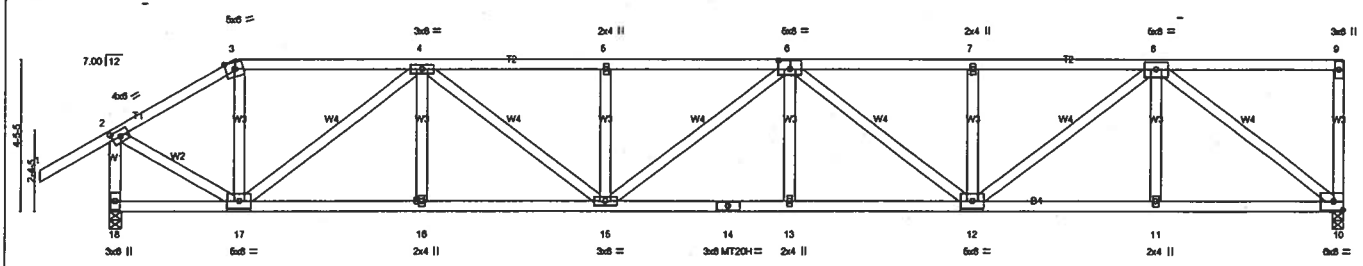
Dwg.#0203061407

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T17	MONO HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:06 2006 Page 1

-2-0-0	3-6-14	9-0-14	14-5-2	19-9-7	25-1-11	30-6-0	36-0-0
2-0-0	3-6-14	5-6-0	5-4-4	5-4-4	5-4-4	5-4-4	5-6-0

Scale: 3/16"=1'



3-6-14	9-0-14	14-5-2	19-9-7	25-1-11	30-6-0	36-0-0
3-6-14	5-6-0	5-4-4	5-4-4	5-4-4	5-4-4	5-6-0

Plate Offsets (X,Y): [2:0-2-14,0-2-0], [6:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.31	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.48	13-15	>897	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.82	Horz(TL)	0.14	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 434 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-13 oc bracing.

REACTIONS (lb/size) 10=3246/0-4-0, 18=3369/0-4-0
Max Horz 18=395(load case 4)
Max Uplift10=-1930(load case 2), 18=-1743(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-3024/1686, 3-4=-2569/1563, 4-5=-6781/4049, 5-6=-6781/4049, 6-7=-5953/3543, 7-8=-5953/3543, 8-9=-88/52, 9-10=-299/297, 2-18=-3269/1752
BOT CHORD 17-18=-359/21, 16-17=-3194/5320, 15-16=-3194/5320, 14-15=-4172/6998, 13-14=-4172/6998, 12-13=-4172/6998, 11-12=-2178/3660, 10-11=-2178/3660
WEBS 3-17=-374/937, 4-17=-3481/2065, 4-16=0/353, 4-15=-1081/1848, 5-15=-616/636, 6-15=-275/156, 6-13=0/346, 6-12=-1322/796, 7-12=-617/635, 8-12=-1727/2901, 8-11=0/343, 8-10=-4518/2690, 2-17=-1604/2927

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1930 lb uplift at joint 10 and 1743 lb uplift at joint 18.
7) Girder carries hip end with 0-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-117(F=-63), 3-9=-117(F=-63), 10-18=-65(F=-35)

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

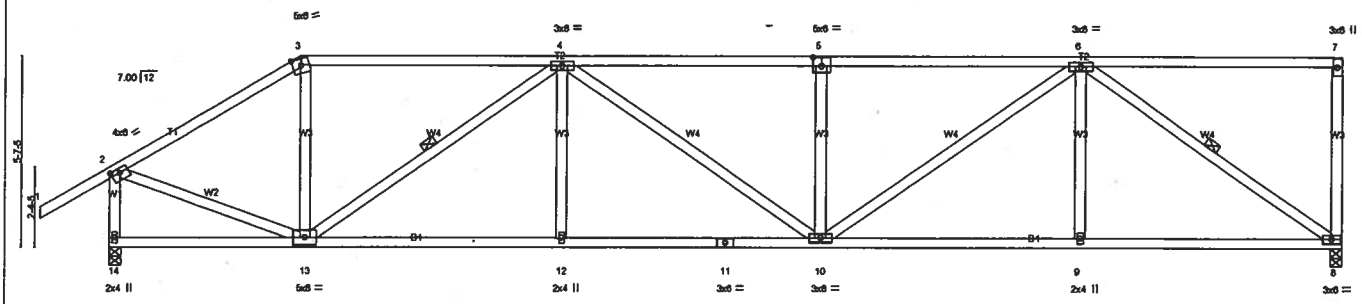
Dwg.#0203061408

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T18	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:07 2006 Page 1

-2-0-0	5-6-14	13-3-0	20-9-7	28-3-13	36-0-0
2-0-0	5-6-14	7-8-3	7-6-7	7-6-7	7-8-3

Scale: 3/16"=1'



5-6-14	13-3-0	20-9-7	28-3-13	36-0-0
5-6-14	7-8-3	7-6-7	7-6-7	7-8-3

Plate Offsets (X,Y): [2:0-3-0,0-1-8], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.22	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.36	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-2-13 oc bracing.
WEBS 1 Row at midpt 4-13, 6-8

REACTIONS (lb/size) 8=1496/0-4-0, 14=1619/0-4-0
Max Horz 14=277(load case 5)
Max Uplift8=-685(load case 3), 14=-490(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1633/651, 3-4=-1365/626, 4-5=-2516/1123, 5-6=-2516/1123, 6-7=-56/26, 7-8=-190/157, 2-14=-1546/692
BOT CHORD 13-14=-231/67, 12-13=-1039/2401, 11-12=-1039/2401, 10-11=-1039/2401, 9-10=-795/1749, 8-9=-795/1749
WEBS 3-13=-133/429, 4-13=-1269/667, 4-12=0/230, 4-10=-102/140, 5-10=-397/332, 6-10=-401/939, 6-9=0/228, 6-8=-2071/941, 2-13=-597/1422

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 685 lb uplift at joint 8 and 490 lb uplift at joint 14.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

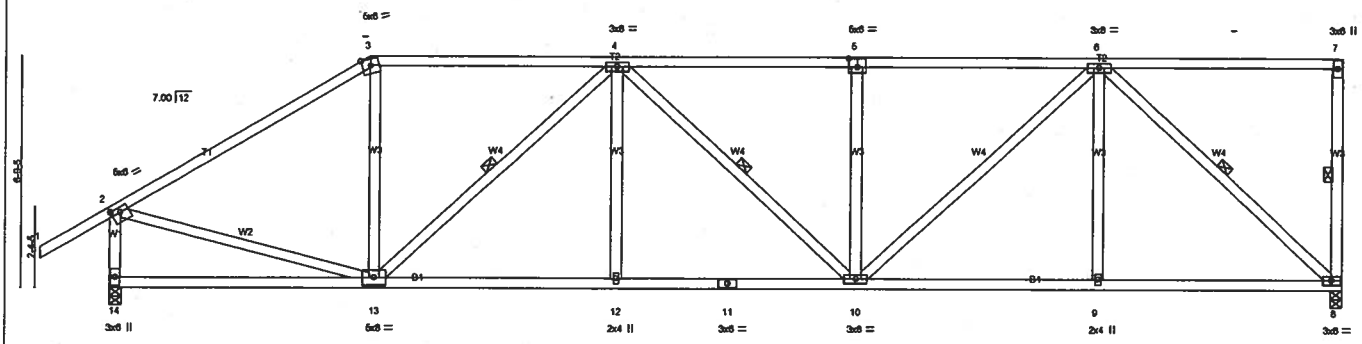
Dwg.#0203061409

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T19	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:08 2006 Page 1

-2-0-0	7-6-14	14-9-0	21-9-7	28-9-13	36-0-0
2-0-0	7-6-14	7-2-3	7-0-7	7-0-7	7-2-3

Scale: 3/16"=1'



7-6-14	14-9-0	21-9-7	28-9-13	36-0-0
7-6-14	7-2-3	7-0-7	7-0-7	7-2-3

Plate Offsets (X,Y): [2:0-3-0,0-1-12], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.16	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.27	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 227 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-10 oc bracing.
WEBS 1 Row at midpt 7-8, 4-13, 4-10, 6-8

REACTIONS (lb/size) 8=1496/0-4-0, 14=1619/0-4-0
Max Horz 14=331(load case 5)
Max Uplift8=-664(load case 3), 14=-504(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1736/670, 3-4=-1421/659, 4-5=-2015/854, 5-6=-2015/854, 6-7=-37/17, 7-8=-175/144, 2-14=-1504/682
BOT CHORD 13-14=-335/124, 12-13=-872/2037, 11-12=-872/2037, 10-11=-872/2037, 9-10=-595/1359, 8-9=-595/1359
WEBS 3-13=-151/427, 4-13=-838/509, 4-12=0/208, 4-10=-42/56, 5-10=-374/306, 6-10=-383/892, 6-9=0/215, 6-8=-1798/787, 2-13=-555/1337

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 664 lb uplift at joint 8 and 504 lb uplift at joint 14.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

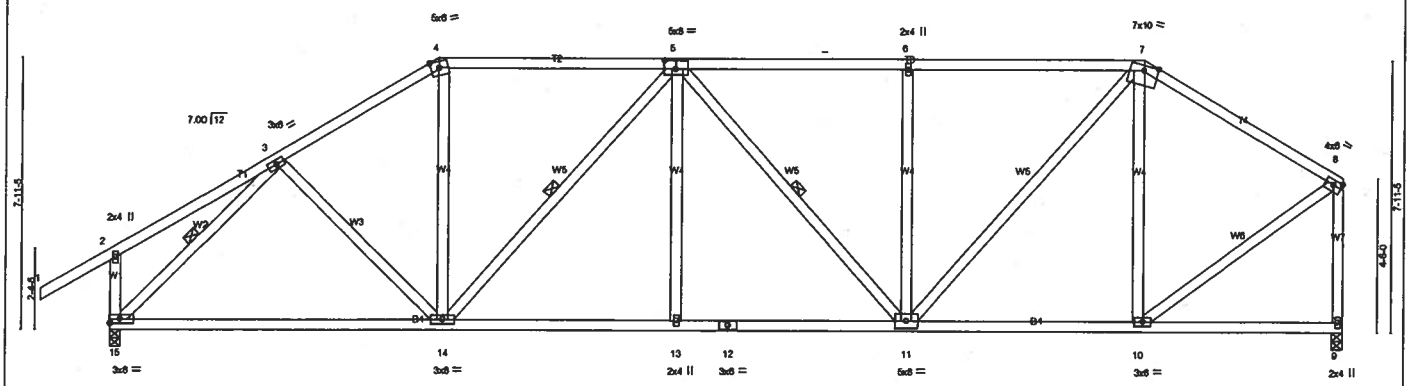
Dwg.#0203061410

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:09 2006 Page 1

-2-0-0	4-11-3	9-6-14	16-5-9	23-2-7	30-1-2	36-0-0
2-0-0	4-11-3	4-7-11	6-10-11	6-8-15	6-10-11	5-10-14

Scale: 3/16"=1'



9-6-14	16-5-9	23-2-7	30-1-2	36-0-0
9-6-14	6-10-11	6-8-15	6-10-11	5-10-14

Plate Offsets (X,Y): [5:0-3-12,0-3-0], [8:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.18	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.30	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 240 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-5 oc bracing.
WEBS 1 Row at midpt 5-14, 5-11, 3-15

REACTIONS (lb/size) 15=1619/0-3-8, 9=1496/0-4-0
Max Horz 15=276(load case 5)
Max Uplift15=-533(load case 5), 9=-494(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-222/173, 3-4=-1672/717, 4-5=-1408/680, 5-6=-1638/760, 6-7=-1638/760, 7-8=-1224/505, 2-15=-342/299, 8-9=-1413/583
BOT CHORD 14-15=-599/1213, 13-14=-786/1766, 12-13=-786/1767, 11-12=-786/1767, 10-11=-353/987, 9-10=-26/39
WEBS 3-14=-221/344, 4-14=-134/483, 5-14=-616/400, 5-13=0/184, 5-11=-216/128, 6-11=-383/334, 7-11=-520/1012, 7-10=-499/308, 3-15=-1569/553, 8-10=-464/1181

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 533 lb uplift at joint 15 and 494 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

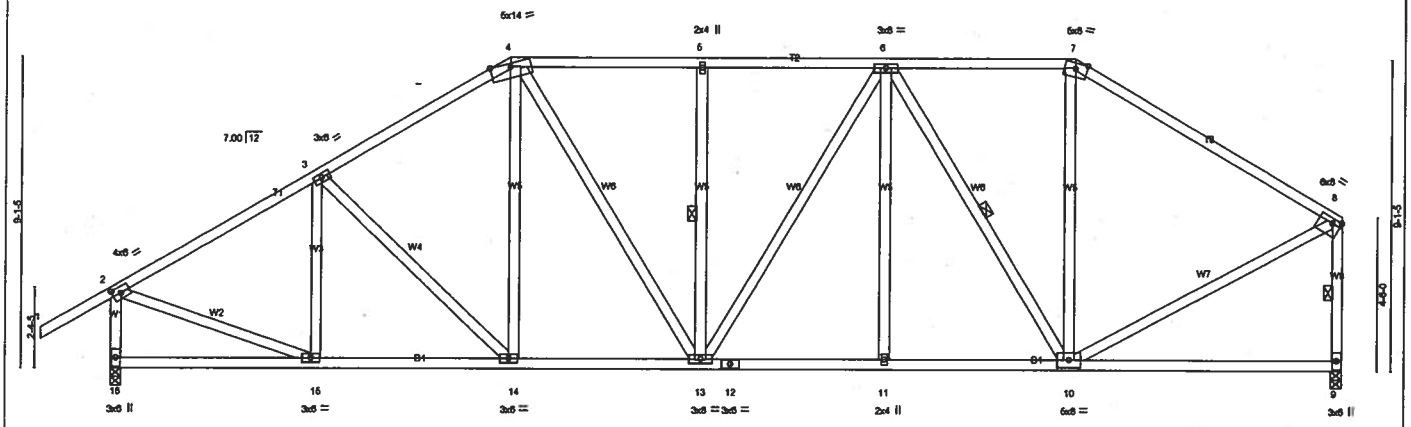
Dwg.#0203061411

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:10 2006 Page 1

-2-0-0	5-11-3	11-6-14	17-1-9	22-6-7	28-1-2	36-0-0
2-0-0	5-11-3	5-7-11	5-6-11	5-4-15	5-6-11	7-10-14

Scale: 3/16"=1'



5-11-3	11-6-14	17-1-9	22-6-7	28-1-2	36-0-0
5-11-3	5-7-11	5-6-11	5-4-15	5-6-11	7-10-14

Plate Offsets (X,Y): [2:0-2-14,0-2-0], [8:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.10	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.17	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-3-7 oc bracing.
WEBS 1 Row at midpt 5-13, 6-10, 8-9

REACTIONS (lb/size) 16=1619/0-3-8, 9=1496/0-4-0
Max Horz 16=313(load case 4)
Max Uplift16=-549(load case 5), 9=-412(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-1633/637, 3-4=-1626/719, 4-5=-1531/749, 5-6=-1531/749, 6-7=-1084/568, 7-8=-1355/557, 2-16=-1531/678, 8-9=-1374/580
BOT CHORD 15-16=-296/194, 14-15=-587/1340, 13-14=-562/1341, 12-13=-560/1433, 11-12=-560/1433, 10-11=-560/1433, 9-10=-51/86
WEBS 3-15=-299/170, 3-14=-206/165, 4-14=-81/179, 4-13=-329/455, 5-13=-312/275, 6-13=-127/217, 6-11=0/147, 6-10=-743/425, 7-10=-128/356, 2-15=-386/1363, 8-10=-408/1129

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 16 and 412 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

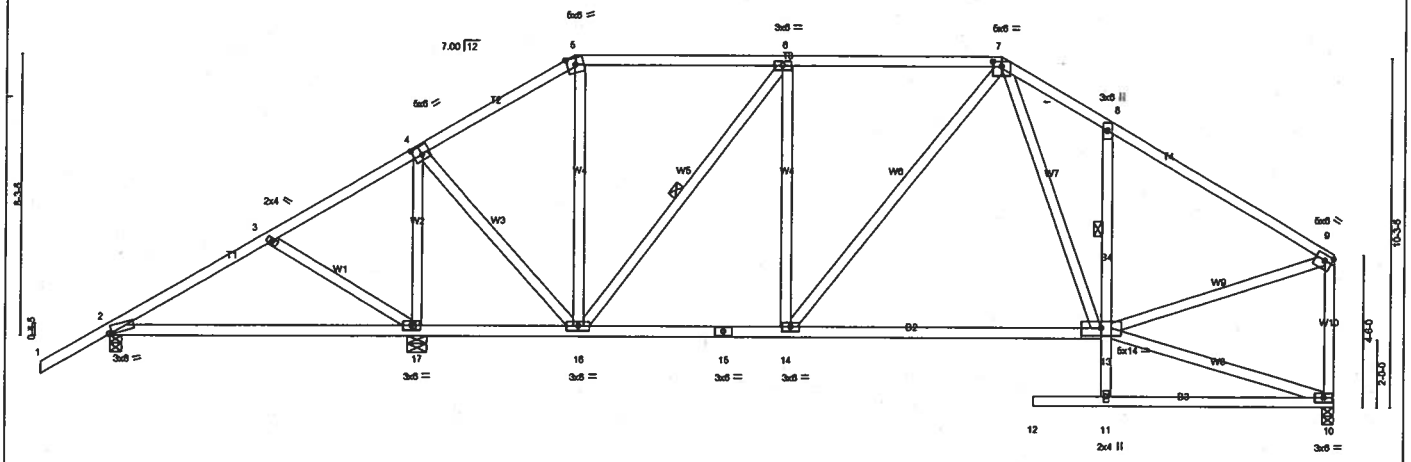
Dwg.#0203061412

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T22	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:11 2006 Page 1

-2-0-0	4-8-12	8-11-15	13-6-14	19-10-0	26-1-2	29-1-0	36-0-0
2-0-0	4-8-12	4-3-3	4-6-14	6-3-2	6-3-2	2-11-14	6-11-0

Scale = 1:64.7



8-11-15	13-6-14	19-10-0	27-1-0	29-1-0	36-0-0
8-11-15	4-6-14	6-3-2	7-3-0	2-0-0	6-11-0

Plate Offsets (X,Y): [2:0-0-14,0-0-3], [4:0-3-0,0-3-0], [7:0-3-0,0-1-12], [9:0-2-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.88	Vert(LL)	0.24	2-17	>441	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	0.19	2-17	>553	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 246 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 8-13
WEBS 1 Row at midpt 6-16

REACTIONS (lb/size) 2=364/0-4-0, 17=1687/0-7-3, 10=1128/0-4-0
Max Horz 2=294(load case 4)
Max Uplift2=-281(load case 5), 17=-653(load case 4), 10=-309(load case 6)
Max Grav 2=379(load case 9), 17=1687(load case 1), 10=1128(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-121/110, 3-4=-164/269, 4-5=-580/262, 5-6=-452/278, 6-7=-896/448, 7-8=-1179/538, 8-9=-1223/412, 9-10=-1064/391
BOT CHORD 2-17=-169/175, 16-17=-174/160, 15-16=-295/896, 14-15=-295/896, 13-14=-203/862, 11-13=0/165, 8-13=-271/284, 11-12=0/0, 10-11=-64/3
WEBS 3-17=-275/259, 4-17=-1303/531, 4-16=-282/948, 5-16=-46/126, 6-16=-748/324, 6-14=-2/225, 7-14=-157/151, 7-13=-183/321, 9-13=-244/968, 10-13=-38/132

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 2, 653 lb uplift at joint 17 and 309 lb uplift at joint 10.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061413

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T23	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:12 2006 Page 1

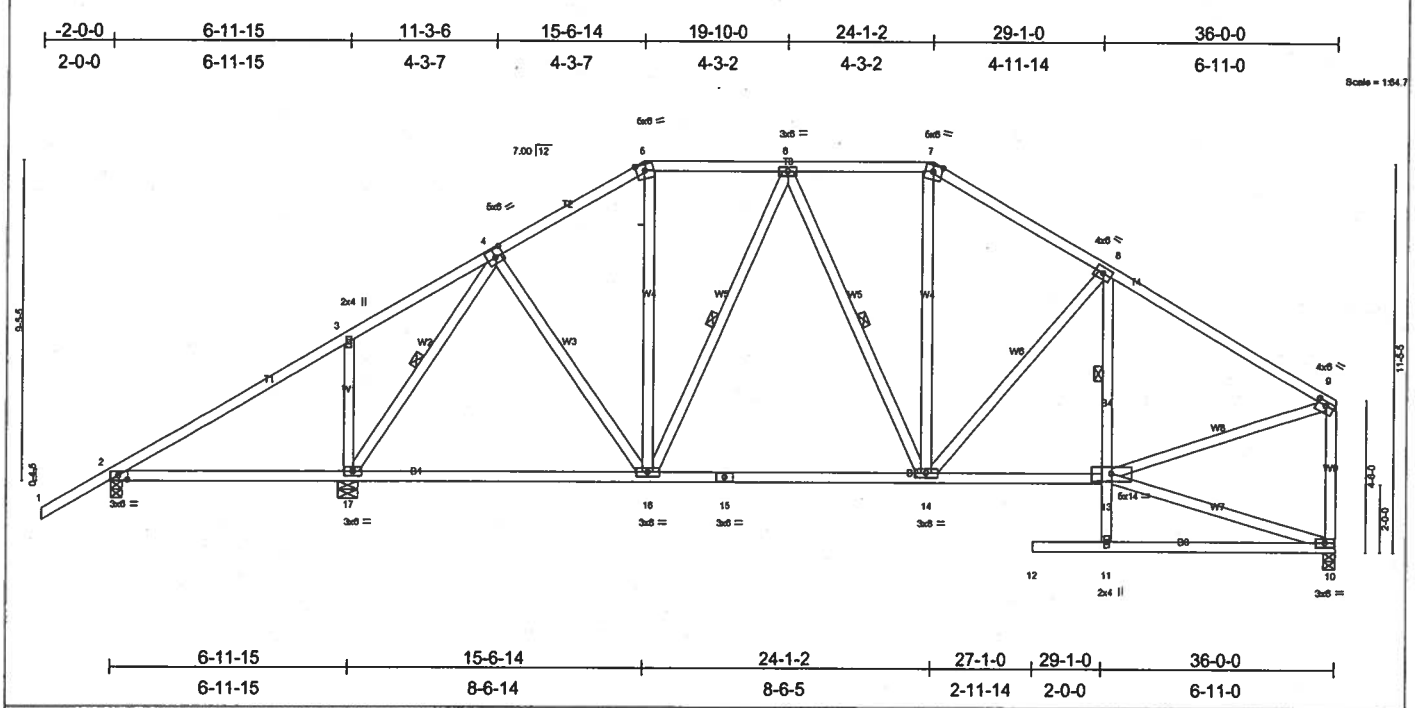


Plate Offsets (X,Y): [2:0-3-3,0-1-8], [4:0-3-0,0-3-0], [9:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.14	2-17	>586	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	0.12	2-17	>672	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.05	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-17.
1 Row at midpt 8-13
WEBS 1 Row at midpt 4-17, 6-16, 6-14

REACTIONS (lb/size) 2=293/0-4-0, 17=1658/0-7-3, 10=1229/0-4-0
Max Horz 2=334(load case 4)
Max Uplift2=-228(load case 5), 17=-639(load case 5), 10=-349(load case 6)
Max Grav 2=312(load case 9), 17=1658(load case 1), 10=1229(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-193/239, 3-4=-47/178, 4-5=-958/445, 5-6=-785/436, 6-7=-943/500, 7-8=-1163/519, 8-9=-1332/465, 9-10=-1142/429
BOT CHORD 2-17=-119/168, 16-17=-231/557, 15-16=-255/911, 14-15=-255/911, 13-14=-302/1071, 11-13=0/169, 8-13=-103/136, 11-12=0/0, 10-11=0/20
WEBS 3-17=-348/335, 4-17=-1245/400, 4-16=-135/436, 5-16=-55/238, 6-16=-371/252, 6-14=-60/197, 7-14=-86/298, 8-14=-220/203, 9-13=-296/1074, 10-13=-45/45

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 2, 639 lb uplift at joint 17 and 349 lb uplift at joint 10.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

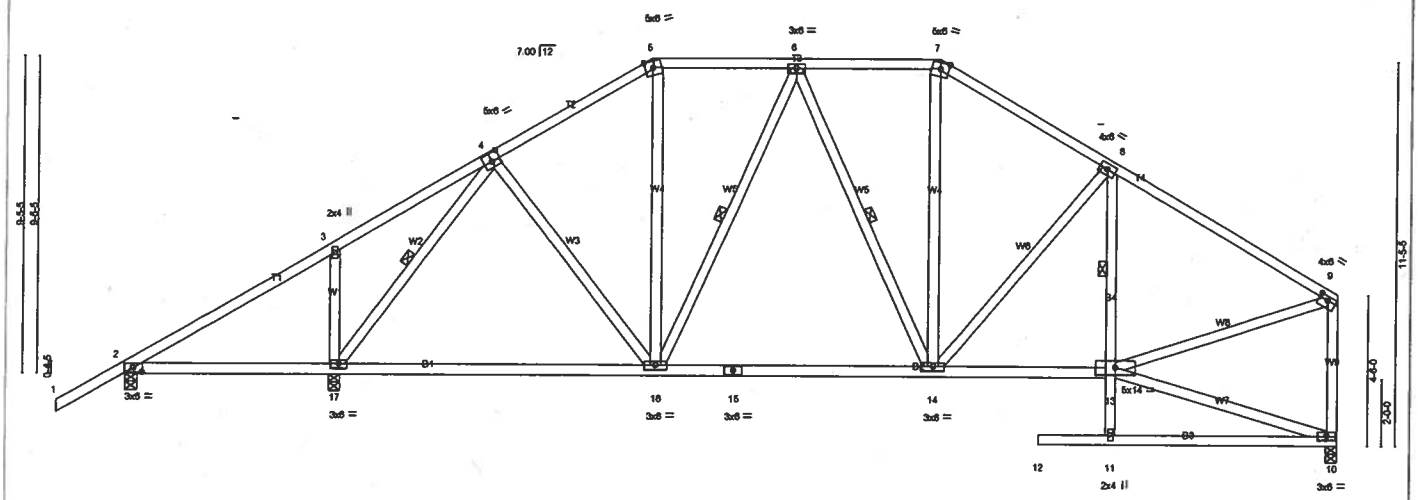
Dwg.#0203061414

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T24	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:36:13 2006 Page 1

-2-0-0	6-1-15	10-10-6	15-6-14	19-10-0	24-1-2	29-1-0	36-0-0
2-0-0	6-1-15	4-8-7	4-8-7	4-3-2	4-3-2	4-11-14	6-11-0

Scale = 1:55.4



6-1-15	15-6-14	24-1-2	27-1-0	29-1-0	36-0-0
6-1-15	9-4-15	8-6-4	2-11-14	2-0-0	6-11-0

Plate Offsets (X,Y): [2:0-3-4,0-1-8], [4:0-3-0,0-3-0], [9:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	0.08	2-17	>943	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.24	16-17	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 252 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-13 oc purlins, except end verticals , and 2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-17.
1 Row at midpt 8-13
WEBS 1 Row at midpt 4-17, 6-16, 6-14

REACTIONS (lb/size) 2=264/0-4-0, 17=1646/0-4-0, 10=1269/0-4-0
Max Horz 2=334(load case 4)
Max Uplift2=-213(load case 5), 17=-618(load case 5), 10=-360(load case 6)
Max Grav 2=282(load case 9), 17=1646(load case 1), 10=1269(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-191/212, 3-4=-59/180, 4-5=-1075/488, 5-6=-879/475, 6-7=-995/522, 7-8=-1223/545, 8-9=-1383/487, 9-10=-1184/447
BOT CHORD 2-17=-110/169, 16-17=-271/677, 15-16=-275/983, 14-15=-275/983, 13-14=-321/1116, 11-13=0/169, 8-13=-115/142, 11-12=0/0, 10-11=0/19
WEBS 3-17=-313/306, 4-17=-1323/453, 4-16=-130/382, 5-16=-65/279, 6-16=-332/250, 6-14=-93/209, 7-14=-98/327, 8-14=-212/199, 9-13=-316/1120, 10-13=-45/46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2, 618 lb uplift at joint 17 and 360 lb uplift at joint 10.
5) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061415

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T25	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:15 2006 Page 1

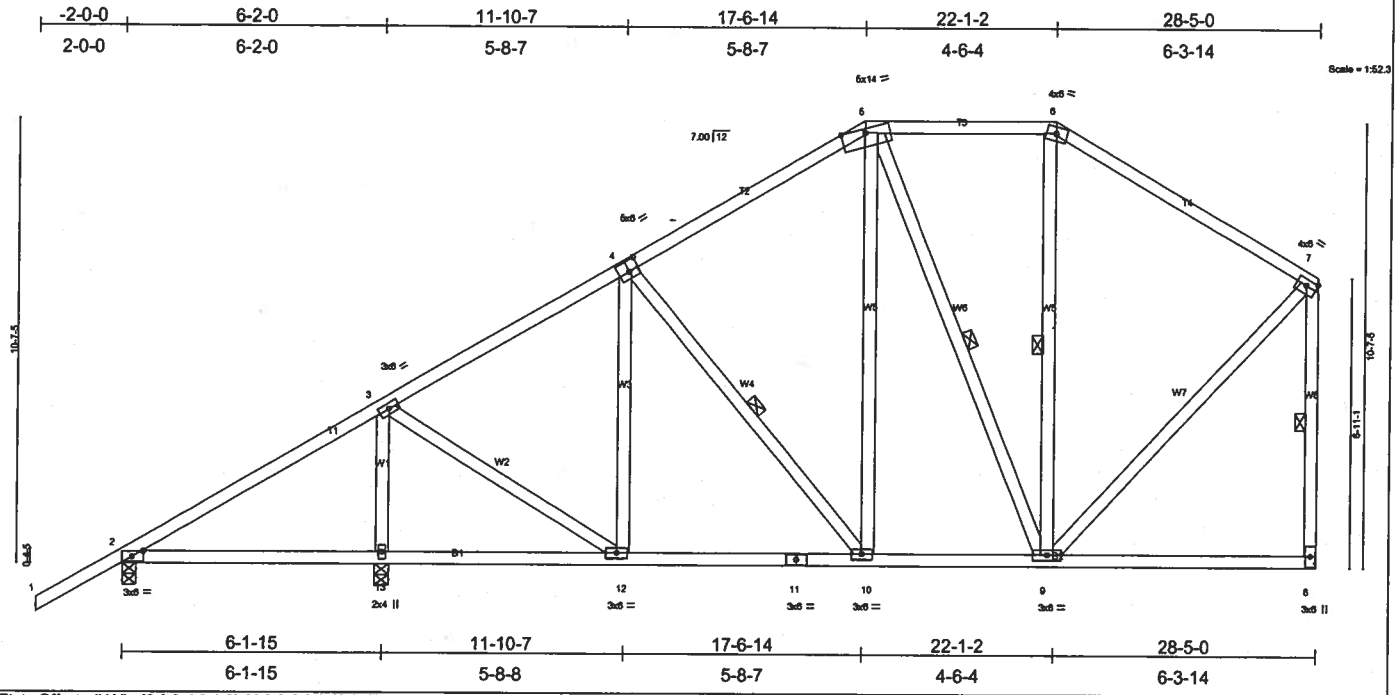


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [4:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.08	2-13	>924	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.07	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 5-9, 6-9, 7-8

REACTIONS (lb/size) 2=332/0-4-0, 13=1234/0-4-0, 8=912/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-187(load case 5), 13=-560(load case 5), 8=-267(load case 5)
Max Grav 2=332(load case 9), 13=1234(load case 1), 8=912(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-211/115, 3-4=-726/225, 4-5=-654/314, 5-6=-418/294, 6-7=-565/260, 7-8=-817/356
BOT CHORD 2-13=-120/57, 12-13=-120/57, 11-12=-259/558, 10-11=-259/558, 9-10=-173/498, 8-9=-17/29
WEBS 3-13=-1037/508, 3-12=-188/684, 4-12=-197/164, 4-10=-111/147, 5-10=-91/226, 5-9=-269/171, 6-9=-120/165, 7-9=-178/567

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 2, 560 lb uplift at joint 13 and 267 lb uplift at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061416

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T26	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:16 2006 Page 1

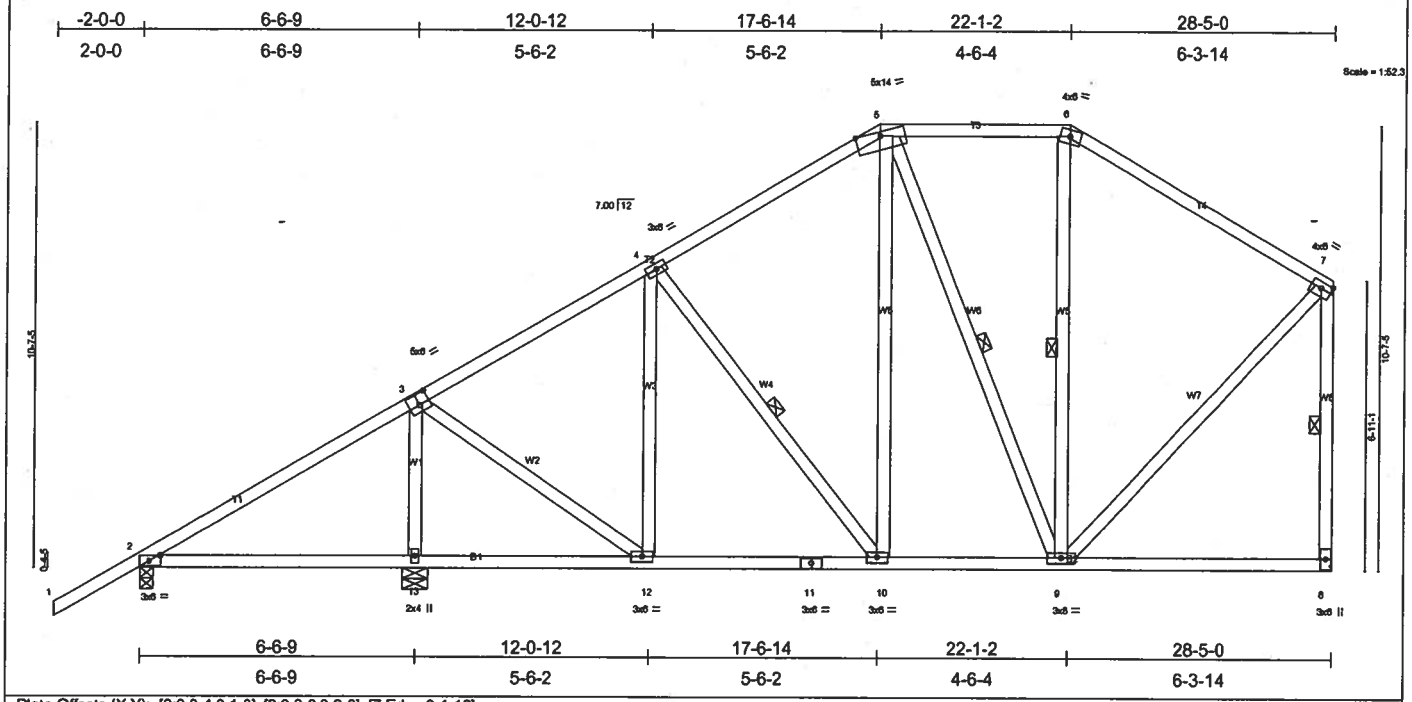


Plate Offsets (X,Y): [2:0-3-4,0-1-8], [3:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.04	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.07	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.30	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 5-9, 6-9, 7-8

REACTIONS (lb/size) 2=351/0-4-0, 13=1230/0-7-3, 8=897/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-128(load case 5), 13=-472(load case 5), 8=-263(load case 6)
Max Grav 2=352(load case 9), 13=1230(load case 1), 8=897(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-203/115, 3-4=-684/231, 4-5=-631/312, 5-6=-408/293, 6-7=-554/259, 7-8=-801/354
BOT CHORD 2-13=-117/61, 12-13=-118/62, 11-12=-250/521, 10-11=-250/521, 9-10=-170/481, 8-9=-17/29
WEBS 3-13=-1025/505, 3-12=-197/646, 4-12=-212/149, 4-10=-89/144, 5-10=-87/203, 5-9=-251/168, 6-9=-124/163, 7-9=-177/553

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 2, 472 lb uplift at joint 13 and 263 lb uplift at joint 8.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0203061417

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE- LOT 44 EMERALD COVE
L141606	T27	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 31 16:38:17 2006 Page 1

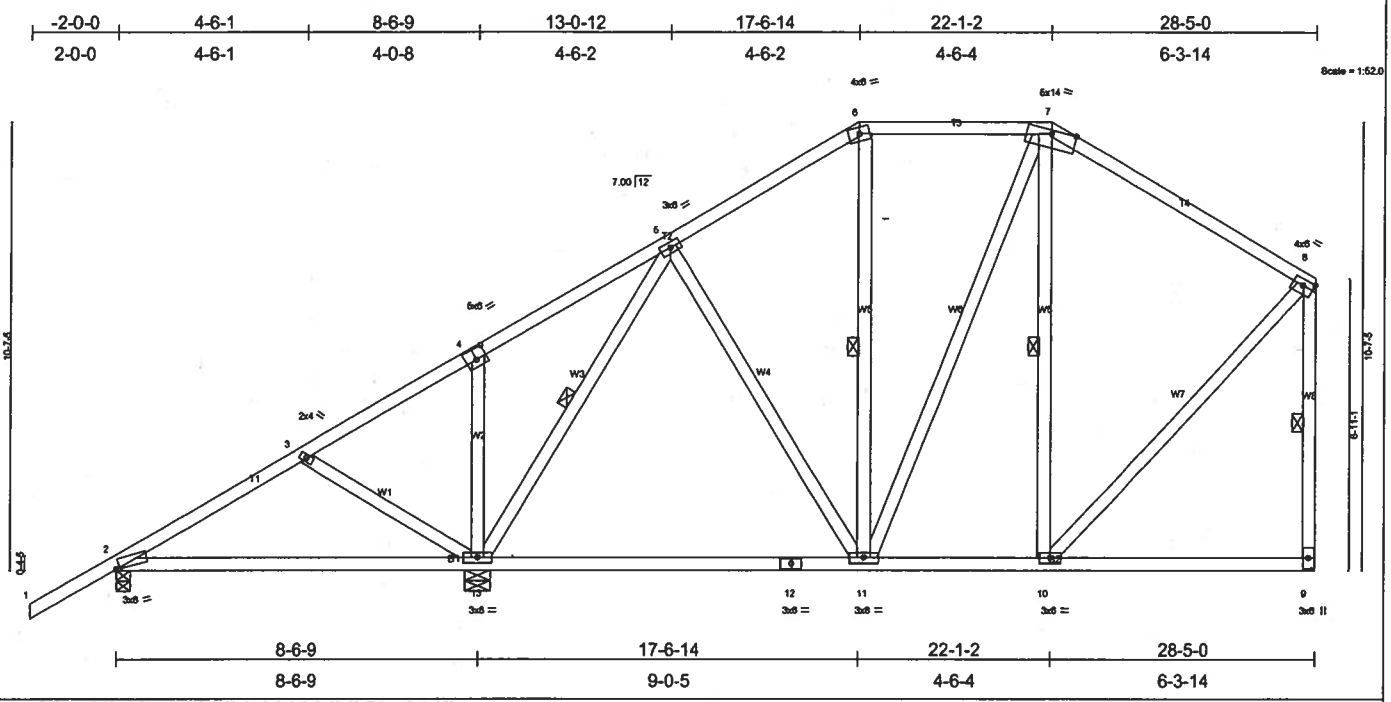


Plate Offsets (X,Y): [2:0-1-2,0-0-3], [4:0-3-0,0-3-0], [8:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.25	2-13	>399	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	0.23	2-13	>445	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-13, 6-11, 7-10, 8-9

REACTIONS (lb/size) 2=414/0-4-0, 13=1265/0-7-3, 9=799/Mechanical
Max Horz 2=432(load case 5)
Max Uplift2=-230(load case 5), 13=-629(load case 5), 9=-226(load case 6)
Max Grav 2=418(load case 9), 13=1265(load case 1), 9=799(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-216/61, 3-4=-201/115, 4-5=-101/130, 5-6=-509/247, 6-7=-384/258, 7-8=-483/220, 8-9=-699/301
BOT CHORD 2-13=-215/151, 12-13=-151/313, 11-12=-151/313, 10-11=-103/345, 9-10=-16/29
WEBS 3-13=-231/252, 4-13=-232/224, 5-13=-729/305, 5-11=-72/178, 6-11=-45/137, 7-11=-110/169, 7-10=-204/114, 8-10=-134/464

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2, 629 lb uplift at joint 13 and 226 lb uplift at joint 9.
6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

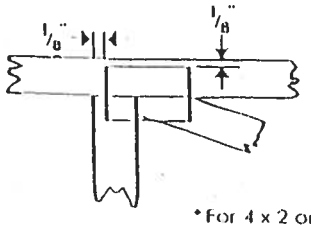
LOAD CASE(S) Standard

FEBRUARY 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

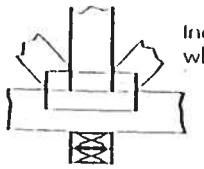
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

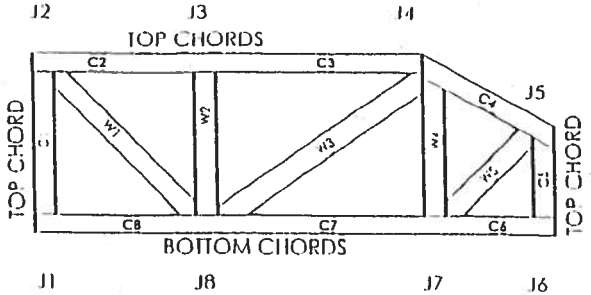
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

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GTC DESIGN GROUP

PROJECT NAME: GATEWAY DEVELOPMENT – EMERALD COVE
PROJECT NUMBER: PF05-034

WIND LOAD AND STRUCTURAL CALCULATIONS FOR

GATEWAY DEVELPMENT
EMERALD COVE HOUSE

12/28/05

GARY GILL, PE
GTC DESIGN GROUP, LLC
P.O. BOX 187
LIVE OAK, FL 32064
386-362-3678
386-362-6133 (FAX)
AUTH. # 9461

Project name: EMERALD COVE
Project: PF05-034
Client GATEWAY I
Calculations: Gary Gill, PE
Date: 11/17/2005

Design Basis

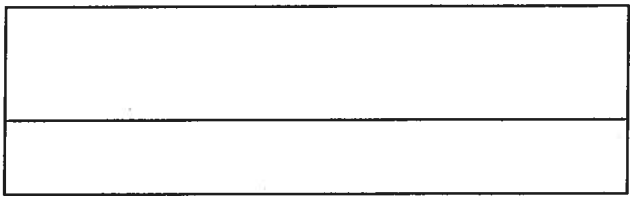
Design Loads

Wind Load	110
Floor Live Load	
Sleep Areas =	30 psf
All Others =	40 psf
Floor Dead Load	10 psf
Wall Dead Load	10 psf
Roof Live Load	20 psf
Roof Dead Load	10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape	Rectangle
Length	66.9 ft
Width	45.708 ft
Type	1 storey sog

References

2001 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

WIND98 v3-02

Wind Load Design per ASCE 7-98

Description: Gateway Development - Emerald Cove House
Analysis by: Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.256	Deg
Type of Roof	Hipped	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	21.33	ft
Mean Roof Height (Ht)	14.65	ft
Width Perp. To Wind Dir (B)	66.90	ft
Width Paral. To Wind Dir (L)	45.71	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.32
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9036	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8681	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

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WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev ft	Kz	Kzt	qz lb/ft^2	Pressure (lb/ft^2) Windward Wall*	
				+GCpi	-GCpi
21.33	0.64	1.00	16.73	8.65	14.10
20	0.62	1.00	16.43	8.45	13.90
15	0.57	1.00	15.13	7.57	13.01

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

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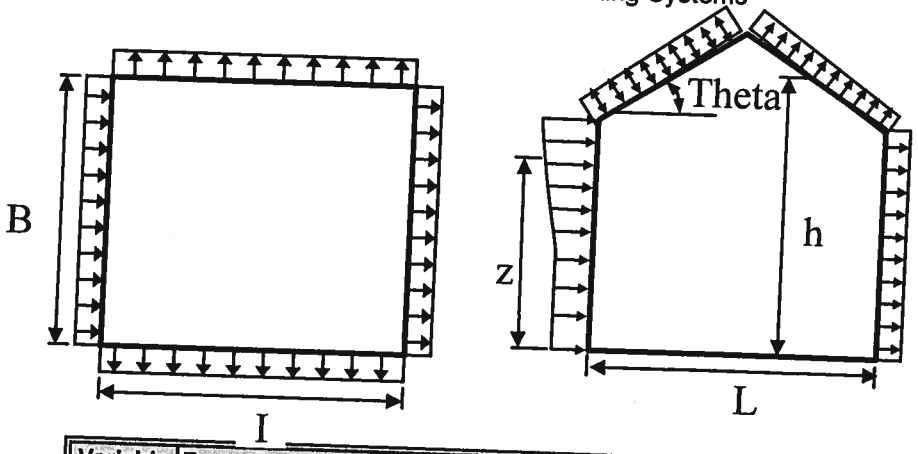
WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*I*Kh*Kht*Kd	15.13	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	.00256*V^2*I*Khcc*Kht*Kd	18.45	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 66.9 ft Face *Additional Runs may be req'd for other wind directions*	Cp	Pressure (psf) +GCpi	 -GCpi
Leeward Walls (Wind Dir Normal to 66.9 ft wall)	-0.50	-9.15	-3.71
Side Walls	-0.70	-11.73	-6.28
Roof - Wind Normal to Ridge (Theta>=10) - for Wind Normal to 66.9 ft face			
Windward - Max Negative	-0.19	-5.20	0.25
Windward - Max Positive	0.28	0.84	6.29
Leeward Normal to Ridge	-0.60	-10.44	-4.99
Overhang Top (Windward)	-0.19	-2.48	-2.48
Overhang Top (Leeward)	-0.60	-7.72	-7.72
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 66.9 ft face			
Dist from Windward Edge: 0 ft to 7.325 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 7.325 ft to 14.65 ft	-0.90	-14.30	-8.85
Dist from Windward Edge: 14.65 ft to 29.3 ft	-0.50	-9.15	-3.71
Dist from Windward Edge: > 29.3 ft	-0.30	-6.58	-1.13

* Horizontal distance from windward edge

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WIND98 v3-02

Wind Load Design per ASCE 7-98

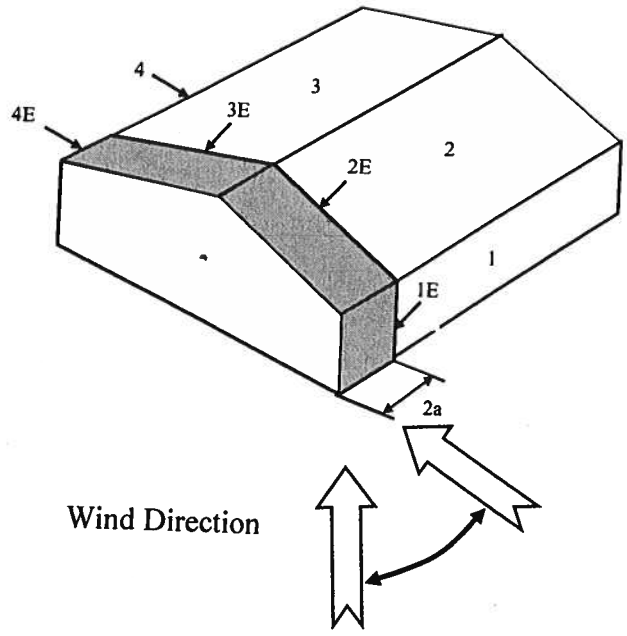
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	15.13	5.75	11.20
2	0.21	0.18	-0.18	15.13	0.45	5.90
3	-0.43	0.18	-0.18	15.13	-9.23	-3.78
4	-0.37	0.18	-0.18	15.13	-8.32	-2.88
5	0.00	0.18	-0.18	15.13	-2.72	2.72
6	0.00	0.18	-0.18	15.13	-2.72	2.72
1E	0.69	0.18	-0.18	15.13	7.72	13.16
2E	0.27	0.18	-0.18	15.13	1.36	6.81
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.00	0.18	-0.18	15.13	-2.72	2.72
6E	0.00	0.18	-0.18	15.13	-2.72	2.72

* p = qh * (GCpf - GCpi)



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WIND98 v3-02

Wind Load Design per ASCE 7-98

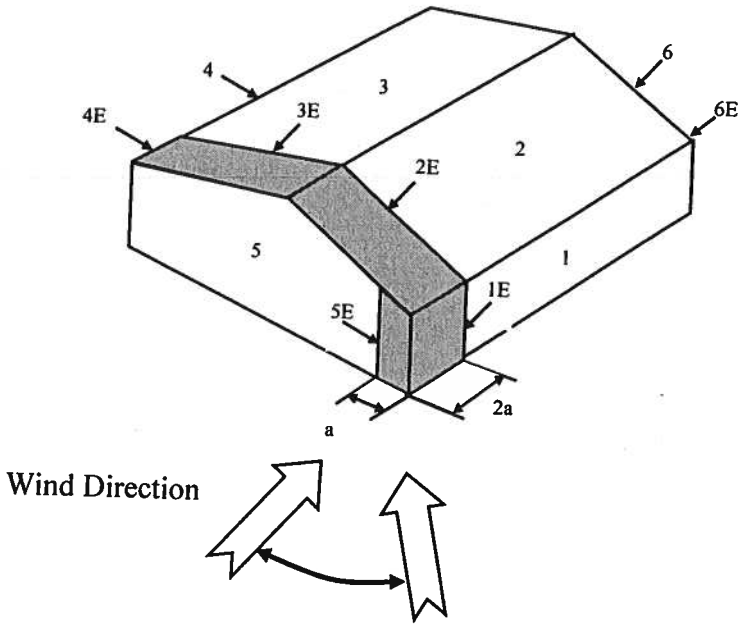
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	15.13

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	15.13	-9.53	-4.09
2	-0.69	0.18	-0.18	15.13	-13.16	-7.72
3	-0.37	0.18	-0.18	15.13	-8.32	-2.88
4	-0.45	0.18	-0.18	15.13	-9.53	-4.09
5	0.40	0.18	-0.18	15.13	3.33	8.78
6	-0.29	0.18	-0.18	15.13	-7.11	-1.66
1E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
2E	-1.07	0.18	-0.18	15.13	-18.92	-13.47
3E	-0.53	0.18	-0.18	15.13	-10.74	-5.30
4E	-0.48	0.18	-0.18	15.13	-9.99	-4.54
5E	0.61	0.18	-0.18	15.13	6.51	11.95
6E	-0.43	0.18	-0.18	15.13	-9.23	-3.78

* p = qh * (GCpf - GCpi)



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WIND98 v3-02

Wind Load Design per ASCE 7-98

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Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		7	Roof Dia	13.89
Vertical Roof height		13.33	Length of Building	66.9
2nd Floor height	0		Width of Building	45.708
1st Floor height	8			

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	5.90	psf	Wall - Leeward	11.2 psf
Leeward Roof - Surface 3	-3.78	psf	Wall - Surface 4	-2.88 psf
			Total Wall	14.08 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz.(psf)	2.97
Leeward Roof Horz.(psf)	-1.90
Total	4.88
Tributary area (roof)	434.85
Roof shear values	**2120.97**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	14.08
Tributary area to each Shearwall (sf)	267.60
Wall shear values to each shearwall	**3767.81**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**5888.78**

2nd Floor shearwalls	Shearwall column #		
	1	2	3
Number of shearwall segments in each column	1	1	1
Shearwall #1 length	0	0	0
Shearwall #2 length	0		
Shearwall #3 length			
Lateral load on shear wall column (lbs)	**0.00**	**0.00**	**0.00**
Percent Full-Height Sheathing	0.00	0.00	0.00
Shear capacity adjustment	0	0	0
Shearwall rating (plf) w/ 1.4	0	0	0
Design Shear Capacity	**0.00**	**0.00**	**0.00**
Stress Ratio	#DIV/0!	#DIV/0!	#DIV/0!
uplift at shear ends	**#DIV/0!**	**#DIV/0!**	**#DIV/0!**
shear and uplift between holddown, v and u	**#DIV/0!**	**#DIV/0!**	**#DIV/0!**

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Full wall length	41.58	60.8	
Shearwall #1 length	28.58	35.8	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.28	0.22	
Rigidities of shearwalls	11.61	14.67	
Lateral load on shearwall column (lbs) based on rigidity	**2600.71**	**3288.07**	
Percent Full-Height Sheathing Shearwall #1	68.73%	58.88%	
Shear capacity adjustment	1	1	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**13804.14**	**17291.40**	
Stress Ratio	0.69	0.63	
uplift at shear ends	**727.98**	**734.76**	
shear and uplift between holddown, v and u	**91.00**	**91.85**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	7	Roof Dia	13.89
Vertical Roof height	13.33	Length of Building	66.9
2nd Floor height	0	Width of Building	45.708
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	8.78	psf
Leeward Wall - Surface 6	-1.66	psf
Total Wall	10.44	

Horizontal loads from parallel to ridge (N/S)

Roof Pressure (interior)	
Windward Roof Horz.(psf)	8.78
Leeward Roof Horz.(psf)	-1.66
Total	10.44
Tributary area (roof) to each shearwall (sf)	304.64
Roof shear values to each shearwall	**3180.44**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	0
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	10.44
Tributary area to each Shearwall (sf)	409.52
Wall shear values to each shearwall	**4275.39**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**7455.83**

2nd Floor shearwalls	Shearwall column #		
	A	B	C
Number of shearwall segments in each column	1	1	
Full wall length	23.58	13.41	
Shearwall #1 length	17.58	5.41	
Shearwall #2 length			
Shearwall #3 length			
Lateral load on shear wall column (lbs)	**0.00**	**0.00**	
Percent Full-Height Sheathing	0.00	0.00	
Shear capacity adjustment	0	0	
Shearwall rating (plf) w/ 1.4	0	0	
Design Shear Capacity	**0.00**	**0.00**	
Stress Ratio	#DIV/0!	#DIV/0!	
uplift at shear ends	**#DIV/0!**	**#DIV/0!**	
shear and uplift between holddown, v and u	**#DIV/0!**	**#DIV/0!**	

1st Floor shearwall (ft)

Number of shearwall segments in each column	A	B	
Full wall length	66.9	66.9	
Shearwall #1 length	36.9	36.9	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	0.22	0.22	
Rigidities of shearwalls	15.14	15.14	
Lateral load on shearwall column (lbs) based on rigidity	**3727.92**	**3727.92**	
Percent Full-Height Sheathing Shearwall #1	55.16%	55.16%	
Shear capacity adjustment	1	1	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**17822.70**	**17822.70**	
Stress Ratio	0.63	0.63	
uplift at shear ends	**808.22**	**808.22**	
shear and uplift between holddown, v and u	**101.03**	**101.03**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
		Elevations including:
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation

☐ ☐ d) Location, size and height above roof of chimneys.
☐ ☐ e) Location and size of skylights
☒ ☐ f) Building height
☐ ☐ e) Number of stories

Floor Plan including:

☒ ☐ a) Rooms labeled and dimensioned.
☐ ☐ b) Shear walls identified.
☐ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 **(see attach forms).**
☐ ☐ d) Show safety glazing of glass, where required by code.
☐ ☐ e) Identify egress windows in bedrooms, and size.
☐ ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, **(Please circle applicable type).**
☐ ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☐ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☒ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☐ ☐ b) All posts and/or column footing including size and reinforcing
☐ ☐ c) Any special support required by soil analysis such as piling
☐ ☐ d) Location of any vertical steel.

Roof System:

☒ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☐ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☒ ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☐ ☐ b) Wood frame wall
1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
☐ ☐ b) Floor joist size and spacing
☐ ☐ c) Girder size and spacing
☐ ☐ d) Attachment of joist to girder
☐ ☐ e) Wind load requirements where applicable
☒ ☐ **Plumbing Fixture layout**

Electrical layout including:

☒ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
☐ ☐ b) Ceiling fans
☐ ☐ c) Smoke detectors
☐ ☐ d) Service panel and sub-panel size and location(s)
☐ ☐ e) Meter location with type of service entrance (overhead or underground)
☐ ☐ f) Appliances and HVAC equipment
☐ ☐ g) Arc Fault Circuits (AFCI) in bedrooms
☐ ☐ h) Exhaust fans in bathroom

HVAC information

☒ ☐ a) Energy Calculations (dimensions shall match plans)
☐ ☐ b) Manual J sizing equipment or equivalent computation
☐ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment
☐ ☐ **Disclosure Statement for Owner Builders**
☒ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done
☐ ☐ **Private Potable Water**

ENGINEERING A BETTER WORLD

24648

21 November 2006

Mr. Mack Lipscomb
Lipscomb and Eagle
872 SW Jaguar Drive
Lake City, Florida 32025

Subject: Lot 44, Emerald Cove Subdivision
ASC Project No. 06G1015
ASC Document No. 060103G

Dear Mr. Lipscomb:

ASC geosciences, inc verifies that Lot # 44 footings were over excavated and confirm that 57 stone was placed prior to concrete placement.

If you have any questions, or concerns, please do not hesitate in calling me.

Sincerely,

ASC geosciences, inc

Tommy Bradshaw
Vice President

Jackie Curry
Senior Lab Analyst

■ address:
ASC geosciences, inc.
366 SW Knox Street, Suite 103
Lake City, Florida 32025

■ contacts:
phone: 386.755.1414
fax: 386.755.8882

www.ascworld.net

C4C648

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JF104378 Company Phone No. 386-755-9911
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Zipstrata Eagle Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 117 SW Fieldstone Ct. Lake City, FL.

Type of Construction (More than one box may be checked) ☑ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside 12 Inside 36 Type of Fill N.A.

Section 4: Treatment Information

Date(s) of Treatment(s) 1-9-07
Brand Name of Product(s) Used Cypress 1.2
EPA Registration No. 53883-92
Approximate Final Mix Solution % 0.25%
Approximate Size of Treatment Area: Sq. ft. 3259 Linear ft. 255 Linear ft. of Masonry Voids 255
Approximate Total Gallons of Solution Applied 540
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☒ Yes ☐ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments Treated Main body Garage + 2 Porches

Name of Applicator(s) Steve Brennan Certification No. (if required by State law) JF104378

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Steve Brennan Date 1-9-07

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Reorder Product #2581 • from CROWNMAX • 1-800-252-4011

#24648

GTC DESIGN GROUP

GTC Design Group, LLC
P.O. Box 187
Live Oak, FL 32064
(Phone) 386.362.3678
(Fax) 386.362.6133
ggill@gtcdesigngroup.com

January 19, 2007

Columbia County Zoning and Building Department
135 NE Hernando Avenue
P. O. Box 1529
Lake City, Florida 32056-1529

SUBJECT: Stem wall – Emerald Cove Lot #44

To Whom It May Concern:

On January 15, 2007, I inspected the stemwall for the abovementioned project. The stem wall is structural sound. The re-bars size, spacing, and placement are adequate.

The stem wall is well secured and constructed properly.

If you have any questions or require additional information, please contact me at your convenience.

Thank you,

Gary Gill, P.E. #51942
Project Manager
1/19/07

GTC DESIGN GROUP

GTC Design Group, LLC
P.O. Box 187
Live Oak, FL 32064
(Phone) 386.362.3678
(Fax) 386.362.6133
ggill@gtcdesigngroup.com

24648
Lot 44

November 30, 2007

Brian Kepner, County Planner
Columbia County Building and Zoning
135 NE Hernando Ave.
Lake City, FL 32055

SUBJECT: Lot 44 & 43 Emerald Cove

Brian,

Per our meeting November 21, 2007, it was noted that the county requires reassurance from the developer/contractor that the homes built on Lot 44 and Lot 43 will not be subject to flooding or severe soil erosion during a storm event.

The developer / contractor have made the following improvements to the lots,

1. Roof gutters and downspouts were added to the structures to re-direct runoff from the roofs. The downspouts are connected to an underground pipe to convey the runoff.
2. On lot 43, a small swale was constructed between the building and the retaining wall to convey runoff water away from the site.
3. Lot 44 and 43 are back to back. The topography of the lots included a small hill along the adjacent property line. Approximately 6 to 7 ft of soil was excavated from the hill top.
4. Each lot has been sodded and landscaped.

If you have any questions or require additional information, please contact me at your convenience.

Thank you,

Gary Gill, P.E. #51942
Project Manager
11/30/07

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 33-3S-16-02438-144 **Building permit No.** 000024648

Use Classification SFD,UTILITY **Fire:** 64.20

Permit Holder JAMES MACK LIPSCOMB **Waste:** 167.50

Owner of Building GATEWAY DEVELOPERS OF LAKE CITY **Total:** 231.70

Location: 117 SW FIELDSTONE COURT, LAKE CITY, FL 32055

Date: 12/21/2007

Wayne A. Russ

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS	NA		
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
. SKYLIGHTS	NA		
1. Skylight			
2. Other			
. STRUCTURAL			474
COMPONENTS			1901
1. Wood connector/anchor	Simpson	ABCS; ACL ~~CCHC~~ CS16; H10; HD2A; SP4; STHD10	503
2. Truss plates	ALPINE HS	METAL CONNECTOR PLATE	1999 538
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

e products listed below did not demonstrate product approval at plan review. I understand that at the e of inspection of these products, the following information must be available to the inspector on the site; 1) copy of the product approval, 2) the performance characteristics which the product was tested d certified to comply with, 3) copy of the applicable manufacturers installation requirements.

nderstand these products may have to be removed if approval cannot be demonstrated during inspection

5/12/05

ractor or Contractor's Authorized Agent Signature | Print Name | Date

tion

Permit # (FOR STAFF USE ONLY)

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. **SHUTTERS**	NA		
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
. **SKYLIGHTS**	NA		
1. Skylight			
2. Other			
. **STRUCTURAL COMPONENTS**			474 190
1. Wood connector/anchor	Simpson	ABU; ACL ~~CC~~ CS16; H10; HD2A, SP4, STHD10	503
2. Truss plates	ALPINE HS	METAL CONNECTOR PLATE	1999 538
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

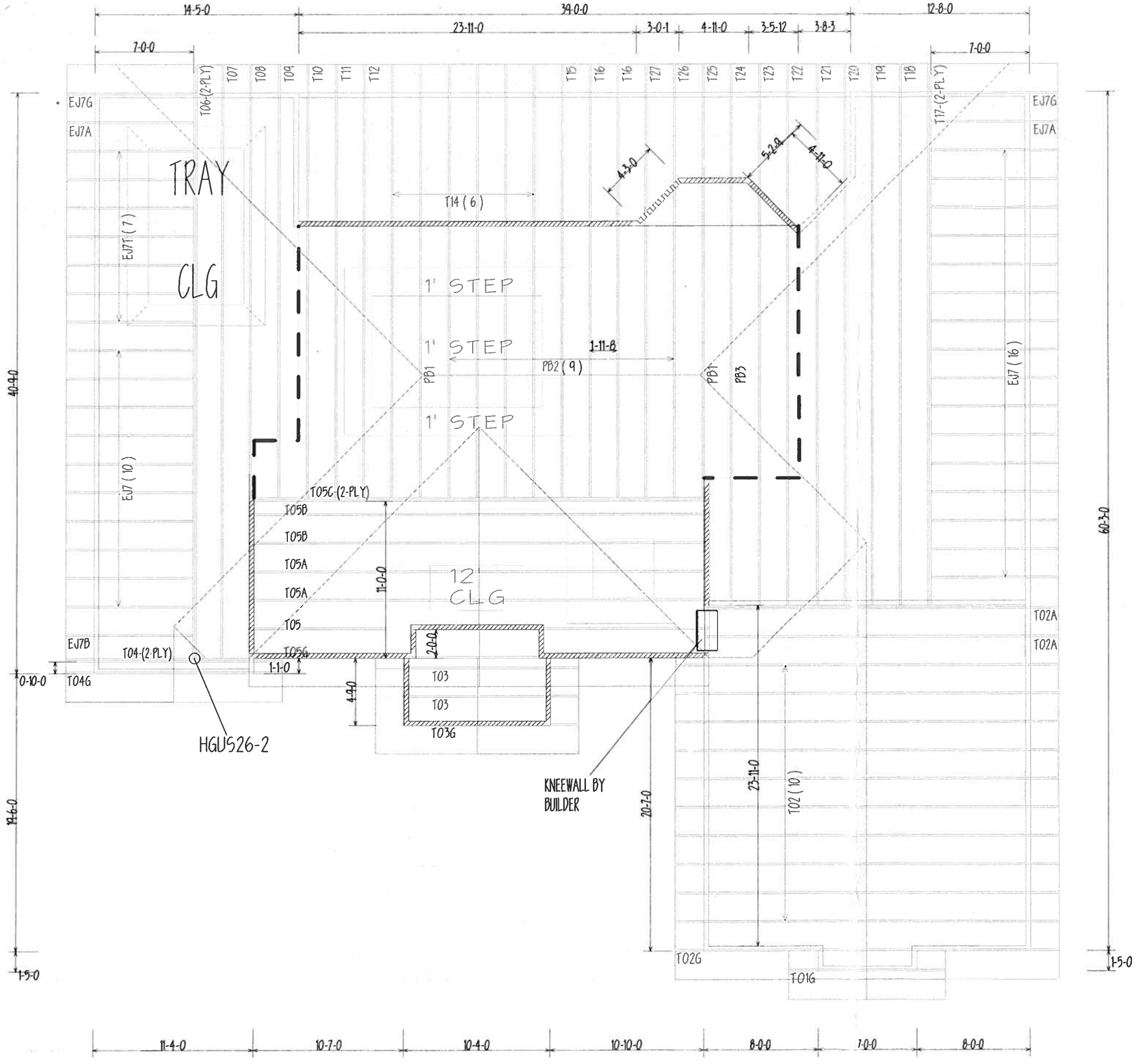
e products listed below did not demonstrate product approval at plan review. I understand that at the e of inspection of these products, the following information must be available to the inspector on the site; 1) copy of the product approval, 2) the performance characteristics which the product was tested d certified to comply with, 3) copy of the applicable manufacturers installation requirements.

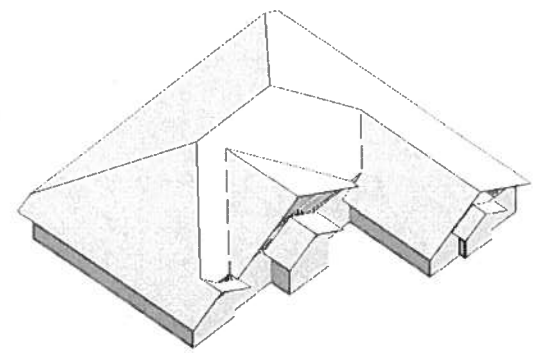
nderstand these products may have to be removed if approval cannot be demonstrated during inspection

8/2/06

ractor or Contractor's Authorized Agent Signature | Print Name | Date

tion | Permit # (FOR STAFF USE ONLY)





7/12 PITCH

BEARING HEIGHT SCHEDULE

8'-0"

10'-0"

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: LIPSCOMB EAGLE

LEGAL ADDRESS: LOT 44 EMERALD COVE

MODEL: EMERALD COVE | REVISION: | SCALE: NTS

DATE: 1-31-06 | DRAWN BY: JRD | JOB #: L141606