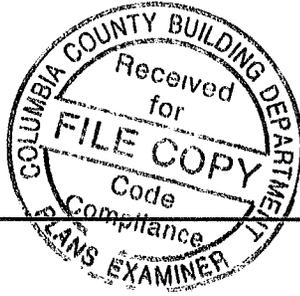


JULIUS LEE PE.



RE: 543737 - Jennings Additions

**1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435**

Site Information:

Project Customer. JENNINGS - O/B Project Name. 543737 Model. ADDITIONS
Lot/Block: Subdivision:
Address: 203 NW SPRINGHILL
City: COLUMBIA CTY State FL

Name Address and License # of Structural Engineer of Record, if there is one, for the building.

Name Unknown at time of seal License #. Unknown at time of seal
Address: Unknown at time of seal
City: Unknown at time of seal State. Unknown at time of seal

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2010/TPI2007 Design Program: MiTek 20/20 7.3
Wind Code: ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 53 individual, dated Truss Design Drawings and 0 Additional Drawings
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet
conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

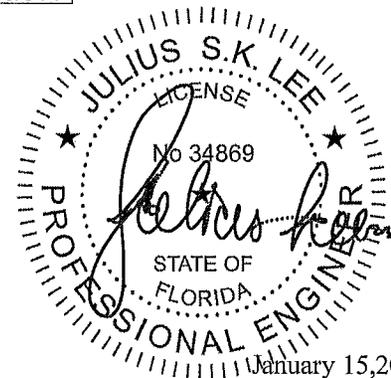
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	17716413	CJ01	1/15/014	18	17716430	PB02	1/15/014
2	17716414	CJ02	1/15/014	19	17716431	PB03	1/15/014
3	17716415	CJ05	1/15/014	20	17716432	PB04	1/15/014
4	17716416	CJ06	1/15/014	21	17716433	PB05	1/15/014
5	17716417	CJ07	1/15/014	22	17716434	PB06	1/15/014
6	17716418	EJ01	1/15/014	23	17716435	PB07	1/15/014
7	17716419	EJ02	1/15/014	24	17716436	PB07G	1/15/014
8	17716420	EJ03	1/15/014	25	17716437	T01	1/15/014
9	17716421	EJ04	1/15/014	26	17716438	T02	1/15/014
10	17716422	EJ05	1/15/014	27	17716439	T03	1/15/014
11	17716423	EJ06	1/15/014	28	17716440	T04	1/15/014
12	17716424	FG1	1/15/014	29	17716441	T05	1/15/014
13	17716425	HJ01	1/15/014	30	17716442	T06	1/15/014
14	17716426	HJ02	1/15/014	31	17716443	T07	1/15/014
15	17716427	HJ03	1/15/014	32	17716444	T08	1/15/014
16	17716428	HJ04	1/15/014	33	17716445	T09	1/15/014
17	17716429	PB01	1/15/014	34	17716446	T09G	1/15/014

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

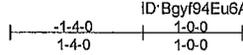
My license renewal date for the state of Florida is February 28, 2015.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

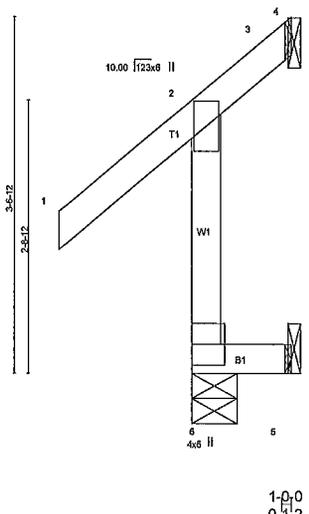


Job	Truss	Truss Type	Qty	Ply	Jennings Additions	17716413
543737	CJ01	Jack-Open Truss	2	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055 7,350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:23:56 2014 Page 1



Scale = 1:22.1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.59	In (oc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.24	Vert(LL) 0.00 6 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) 0.00 6 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) -0.06 3 n/a n/a		
	Code FBC2010/TPI2007			Weight: 10 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=148/0-5-8 (min. 0-1 8) 5=-3/Mechanical, 3=-37/Mechanical
Max Horz 6=89(LC 12)
Max Uplift 5=-124(LC 12) 3=-98(LC 12)
Max Grav 6=212(LC 23) 5=39(LC 10) 3=22(LC 10)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.

- NOTES** (7-9)
- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 5 and 98 lb uplift at joint 3.
 - 6) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss
 - 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - 8) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - 9) Truss Design Engineer: Julius Lee, PE, Florida P. E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



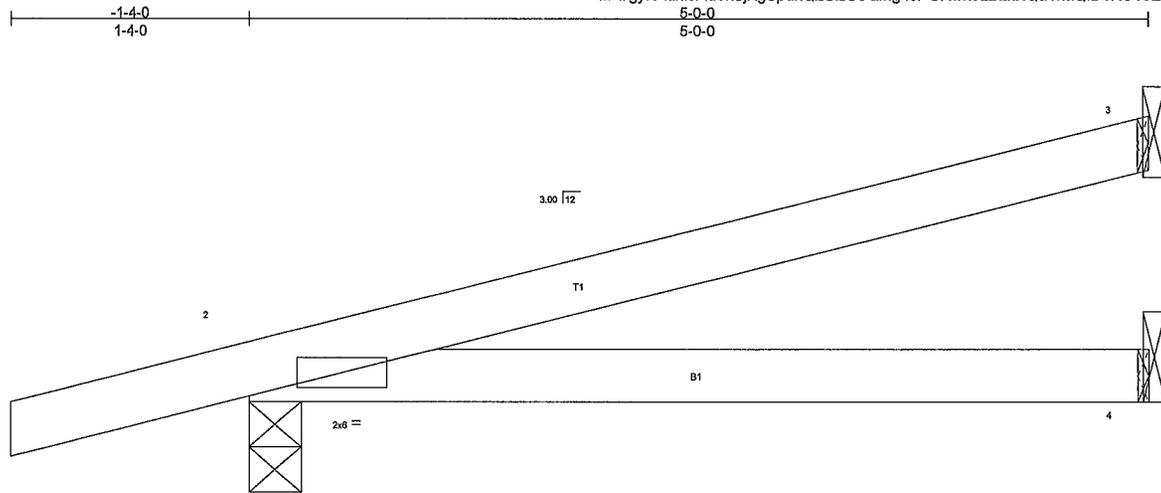
January 15, 2014

<p>WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719</p>	<p>Julius Lee PE, 1109 Coastal Bay Boynton Beach FL 33435</p>
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Job 543737	Truss CJ07	Truss Type Jack-Open Truss	Qty 4	Ply 1	Jennings Additions Job Reference (optional)	17716417
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:23:59 2014 Page 1

ID:Bgyl94Eu6AuekSjXgSpdhQzGZGs-zmg4JPGRwk8LBzkNqa1naQID3KOs3EHqFL1U7jzv5ck



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) 0.08	4-7	>756	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.25	Vert(TL) 0.07	4-7	>880	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2010/TFI2007	(Matrix-M)					Weight: 17 lb	FT = 20%

LUMBER	TOP CHORD 2x4 SP No.2	BRACING	TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
	BOT CHORD 2x4 SP No.2		BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=80/Mechanical, 2=217/0-3-8 (min. 0-1-8) 4=29/Mechanical
 Max Horz 2=84(LC 8)
 Max Uplift 3=82(LC 8) 2=-243(LC 8) 4=-43(LC 8)
 Max Grav 3=97(LC 2) 2=259(LC 2) 4=57(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown
 TOP CHORD 2-3=-394/779
 BOT CHORD 2-4=-869/430

- NOTES (7-9)
- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TODL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C, End GCpl=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed, C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.80 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 3, 243 lb uplift at joint 2 and 43 lb uplift at joint 4.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - 9) Truss Design Engineer Julius Lee PE, Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 15, 2014

<p>WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.</p> <p>Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719</p>	<p>Julius Lee PE 1109 Coastal Bay Boynton Beach, FL 33435</p>
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Job 543737	Truss EJ02	Truss Type MONO TRUSS	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17716419
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Builders FirstSource, Lake City FL 32055 7 350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:01 2014 Page 1

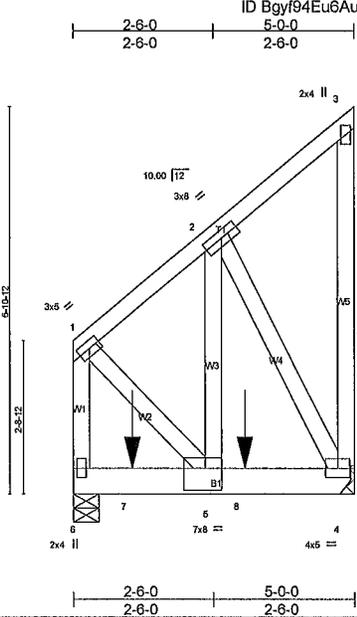


Plate Offsets (X,Y) [5-0-3-8,0-4-12]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plates Increase 1.25	TC 0.20	Vert(LL) -0.01	5-6	>999	MT20	244/190
TCDL 7 0	Lumber Increase 1.25	BC 0.57	Vert(TL) -0.03	5-6	>999		
BCLL 0 0 *	Rep Stress Incr NO	WB 0.51	Horz(TL) 0.00	4	n/a		
BCDL 5 0	Code FBC2010/TP12007	(Matrix-M)				Weight 51 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=1501/0-5-8 (min. 0-1 15) 4=1071/Mechanical
 Max Horz 6=148(LC 5)
 Max Uplift 6=-314(LC 6) 4=-394(LC 8)
 Max Grav 6=1622(LC 2) 4=1181(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-680/128, 1-6=-990/187
 BOT CHORD 5-8=-170/503, 4-8=-170/503
 WEBS 1-5=-194/724, 2-5=-324/1207 2-4=-1066/359

NOTES (9-11)

- 1) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C, Encl GCpi=0.18, MWFRS (envelope) end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 6 and 394 lb uplift at joint 4.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1241 lb down and 280 lb up at 1-0-12, and 1241 lb down and 280 lb up at 3-0-12 on bottom chord The design/selection of such connection device(s) is the responsibility of others
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
- 11) Truss Design Engineer Julius Lee, PE; Florida P.E. License No 34869. Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 4-6=-10 1-3=-44
 Concentrated Loads (lb)
 Vert. 7=-1159(B) 8=-1159(B)



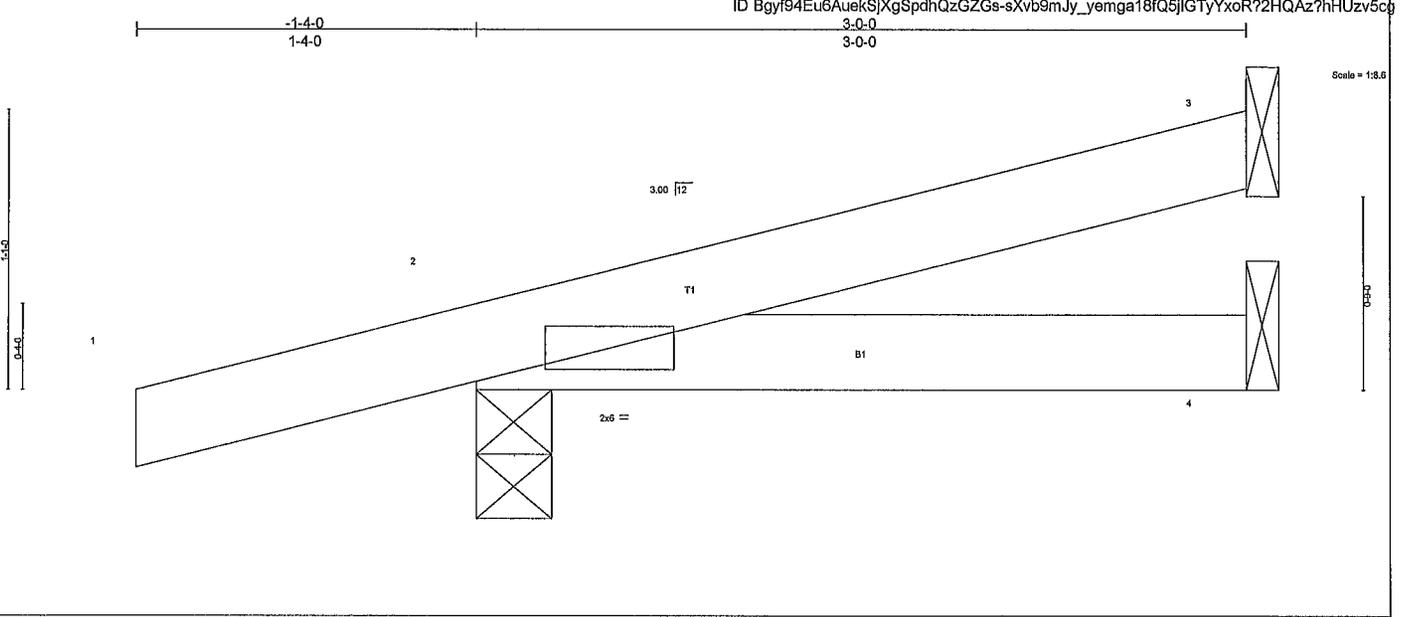
January 15, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-74-73 BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE.
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 543737	Truss EJ04	Truss Type Jack-Partial Truss	Qty 2	Ply 1	Jennings Additions Job Reference (optional)	17716421
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Builders FirstSource Lake City FL 32055 7,350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:03 2014 Page 1
ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-sXvb9mJy_yemga18fQ5jIGTyYxoR?2HQAz?hHUzv5c9



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.11	Vert(LL) 0.01 4-7 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.08	Vert(TL) 0.01 4-7 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 2 n/a n/a	
BCDL 5.0	Code FBC2010/TP12007	(Matrix-M)		Weight: 11 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection. In accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=44/Mechanical 2=154/0-3-8 (min. 0-1-8), 4=19/Mechanical
Max Horz 2=59(LC 8)
Max Uplift 3=46(LC 8), 2=-179(LC 8) 4=-28(LC 8)
Max Grav 3=53(LC 2) 2=186(LC 2) 4=34(LC 3)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

- NOTES** (7-9)
- 1) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=20ft, Cat. II Exp C, Encl GCpl=0 18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 3 179 lb uplift at joint 2 and 26 lb uplift at joint 4.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - 8) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - 9) Truss Design Engineer Julius Lee PE, Florida P E License No. 34869 Address 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 15, 2014

<p>WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719</p>	<p>Julius Lee PE, 1109 Coastal Bay Boynton Beach, FL 33435</p>
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Job	Truss	Truss Type	Qty	Ply	Jennings Additions
543737	EJ06	Jack-Closed Truss	4	1	17716423

Builders FirstSource Lake City FL 32055 7,350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:04 2014 Page 1
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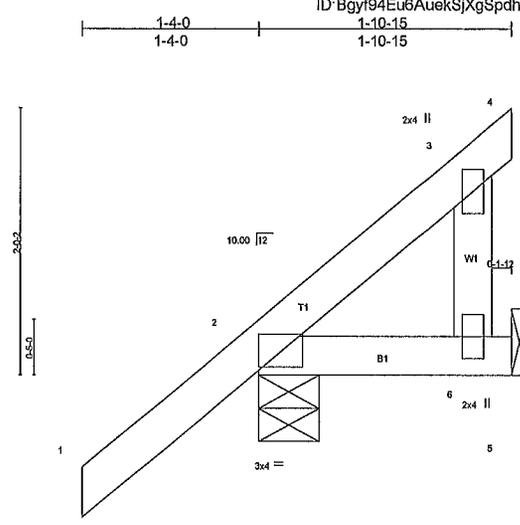


Plate Offsets (X,Y). [2:0-4,0-0-4] 1-10-15 0-0-12

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0 18	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.04	Vert(LL) -0.00 9 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.00 9 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.00 2 n/a n/a		
	Code FBC2010/TP12007			Weight: 11 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1 10-15 oc purlins except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=32/Mechanical, 2=121/0-5-8 (min. 0-1-8)
 Max Horz 2=125(LC 12)
 Max Uplift 6=-54(LC 12) 2=-57(LC 12)
 Max Grav 6=50(LC 21) 2=147(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (7-9)
- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl. GCpi=0 18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SP No 2 crushing capacity of 585 psi.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 6 and 57 lb uplift at joint 2.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
 - 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - 8) Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - 9) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 15, 2014

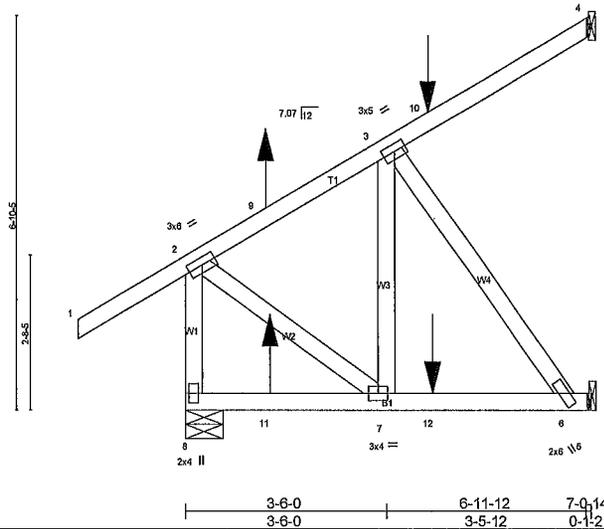
<p>WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719</p>	<p>Julius Lee PE. 1109 Coastal Bay Boynton Beach, FL 33435</p>
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Job	Truss	Truss Type	Qty	Ply	Jennings Additions
543737	HJ01	Diagonal Hip Glrder	1	1	17716425

Builders FirstSource Lake City FL 32055 7,350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:08 2014 Page 1



Scale = 1:38.6



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.28	Vert(LL) 0.03 6-7 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.17	Vert(TL) -0.02 6-7 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix-M)	Horz(TL) -0.01 4 n/a n/a		
	Code FBC2010/TPI2007			Weight: 49 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

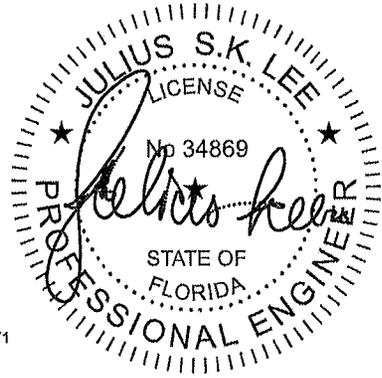
REACTIONS (lb/size) 8=239/0-7 12 (min 0-1-8) 4=68/Mechanical 5=97/Mechanical
Max Horz 8=210(LC 8)
Max Uplift 8=540(LC 6) 4=-108(LC 8) 5=-420(LC 8)
Max Grav 8=297(LC 2) 4=82(LC 2) 5=119(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
TOP CHORD 2-8=-298/410, 2-9=-126/344
BOT CHORD 7 12=-342/79, 8-12=-342/79
WEBS 2-7=-310/122, 3-6=-135/584

- NOTES** (9-11)
- Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TC DL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C, Encl. GCpl=0.18, MWFRS (envelope) gable end zone; end vertical left exposed Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 540 lb uplift at joint 8, 108 lb uplift at joint 4 and 420 lb uplift at joint 5.
 - Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 157 lb up at 1-5-12, 157 lb up at 1-5-12, and 2 lb down and 176 lb up at 4-3-11, and 2 lb down and 176 lb up at 4-3-11 on top chord and 134 lb up at 1-5-12, 134 lb up at 1-5-12 and 10 lb down and 71 lb up at 4-3-11 and 10 lb down and 71 lb up at 4-3-11 on bottom chord The design/selection of such connection device(s) is the responsibility of others
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-44 2-4=-44 5-8=-10
Concentrated Loads (lb)
Vert: 9=49(F=24, B=24) 10=-3(F=-1 B=-1) 11=16(F=8, B=8) 12=-6(F=-3, B=-3)



January 15, 2014

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Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

Job 543737	Truss HJ03	Truss Type Diagonal Hip Girder	Qty 2	Ply 1	Jennings Additions Job Reference (optional)	17716427
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries Inc. Wed Jan 15 13:24:08 2014 Page 1
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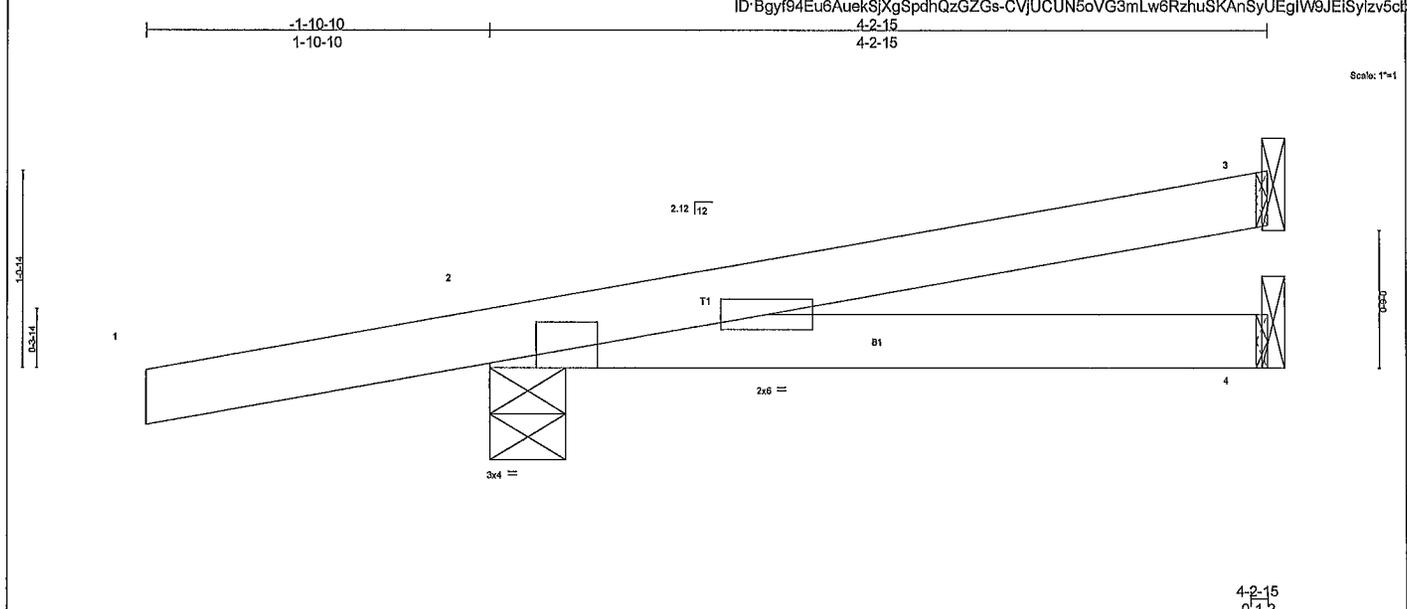


Plate Offsets (X,Y). [2:0-3-0,Edge] 4-2-15 0-1-2

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.23	Vert(LL) 0.04 4-7 >999 240	MT20	244/180
TCDL 7.0	Lumber Increase 1.25	BC 0.17	Vert(TL) 0.03 4-7 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 2 n/a n/a		
BCDL 5.0	Code FBC2010/TP12007	(Matrix-M)		Weight: 16 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

REACTIONS (lb/size) 3=62/Mechanical 2=220/0-4-15 (min. 0-1-8), 4=27/Mechanical
 Max Horz 2=59(LC 8)
 Max Uplift 3=62(LC 8), 2=258(LC 8) 4=36(LC 8)
 Max Grav 3=75(LC 2) 2=264(LC 2) 4=47(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=195/363
 BOT CHORD 2-4=388/205

- NOTES (7-9)**
- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 3 258 lb uplift at joint 2 and 36 lb uplift at joint 4.
 - 6) *Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
 - 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - 8) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - 9) Truss Design Engineer: Julius Lee, PE, Florida P. E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



January 15, 2014

Job 543737	Truss PB01	Truss Type GABLE	Qty 12	Ply 1	Jennings Additions Job Reference (optional)	17716429
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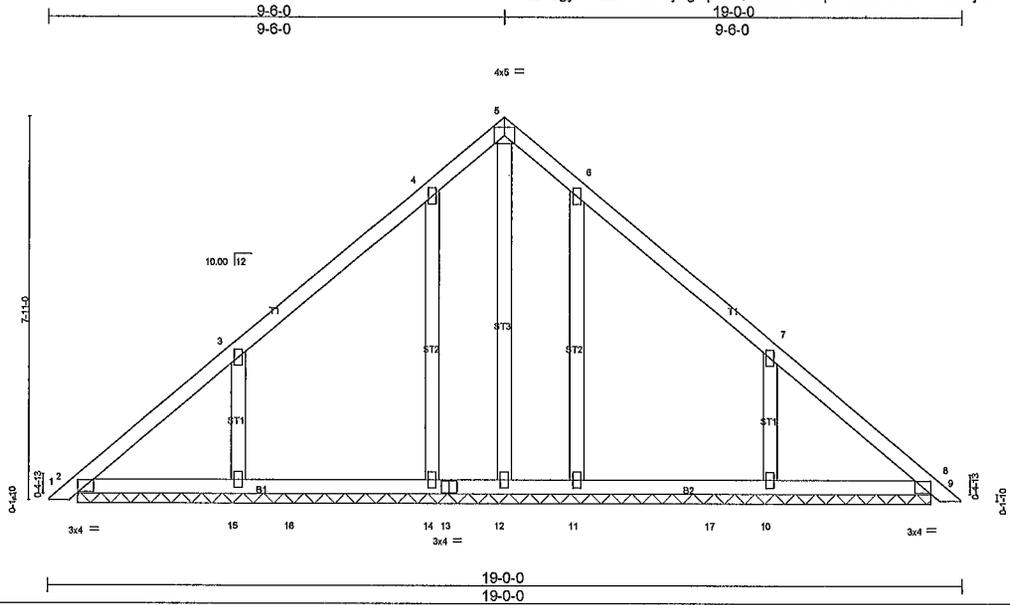


Plate Offsets (X,Y) [2:0-2-1.0-1-8], [8:0-2-1.0-1-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.22	Vert(LL) 0.00 9 n/r 120	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.11	Vert(TL) 0.00 9 n/r 120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.27	Horz(TL) 0.01 8 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix)		Weight: 99 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 17-8-9.
(lb) Max Horz 2=197(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2 12 8 except 10=192(LC 13) 11=134(LC 13)
15=192(LC 12), 14=136(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 2 12, 8 except 10=343(LC 22), 11=334(LC 22)
22) 15=343(LC 21) 14=336(LC 21)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown
WEBS 5-12=258/209, 7-10=386/296, 6-11=303/223, 3-15=386/296, 4-14=303/224

- NOTES** (13-15)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C Encl GCpl=0.18 MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.80 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1
 - All plates are 2x4 MT20 unless otherwise indicated
 - Gable requires continuous bottom chord bearing
 - Gable studs spaced at 4-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12, 8 except (j=lb) 10=192 11=134, 15=192, 14=136.
 - *Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss
 - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable or consult qualified building designer
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 15, 2014

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Job	Truss	Truss Type	Qty	Ply	Jennings Additions	17716431
543737	PB03	GABLE	1	1		

Builders FirstSource, Lake City FL 32055 7,350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 16 13:24:13 2014 Page 1
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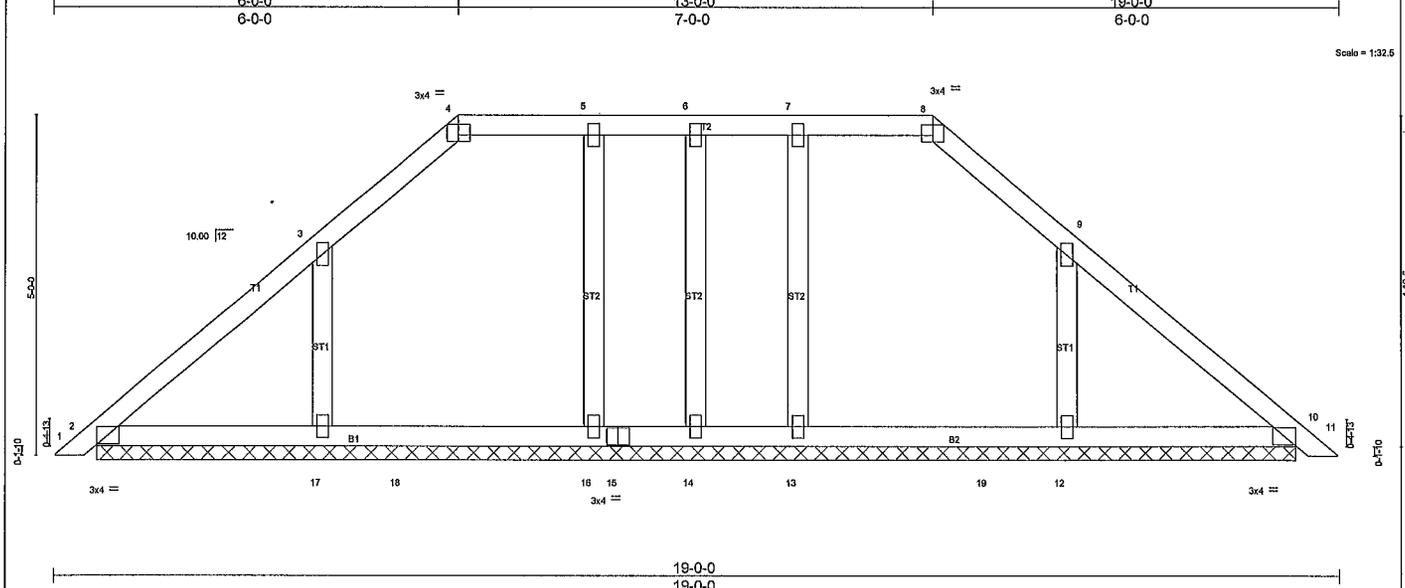


Plate Offsets (X,Y) [2:0-2-1,0-1-8], [4:0-2-0,0-1-13], [8:0-2-0,0-1-13], [10:0-2-1,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.13	Vert(LL)	0.00 11	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.11	Vert(TL)	0.00 11	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.07	Horz(TL)	0.00 10	n/a	n/a		
BCDL 5.0	Code	FBC2010/TPI2007	(Matrix)					Weight: 87 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 OTHERS 2x4 SP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 8-0-0 oc purlins
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 17-8-9.
 (lb) - Max Horz 2=-123(LC 10)
 Max Uplift All uplift 100 lb or less at joint(s) 2 14 13, 16, 10 except 12=-149(LC 13)
 17=-151(LC 12)
 Max Grav All reactions 250 lb or less at joint(s) 2 14, 13, 16, 10 except 12=298(LC 22)
 17=301(LC 21)

FORCES (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 9-12=-307/228, 3-17=-307/228

- NOTES** (14-16)
- Unbalanced roof live loads have been considered for this design.
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C End GCpl=0.18, MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.80 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
 - Provide adequate drainage to prevent water ponding
 - All plates are 2x4 MT20 unless otherwise indicated
 - Gable requires continuous bottom chord bearing
 - Gable studs spaced at 4-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 14, 13, 16, 10 except (it=lb) 12=149, 17=151
 - "Semi-rigid pitchbreaks Including heels" Member end fixity model was used in the analysis and design of this truss
 - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer Julius Lee, PE: Florida P E License No. 34869; Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



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Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 543737	Truss PB05	Truss Type GABLE	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17718433
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:16 2014 Page 1
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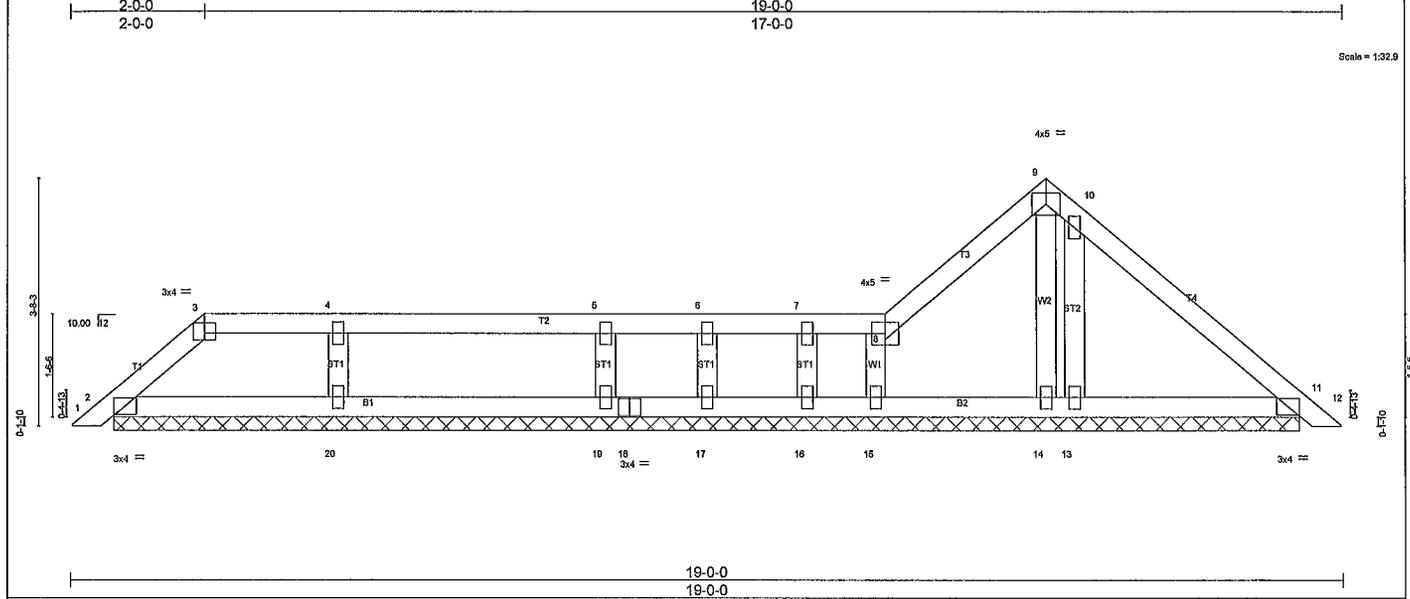


Plate Offsets (X, Y), [2:0-2-1,0-1-8], [3:0-2-0,0-1-13], [11:0-2-1,0-1-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.13	Vert(LL) 0.00 12 n/r 120	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.09	Vert(TL) 0.00 12 n/r 120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.06	Horz(TL) 0.00 11 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix)		Weight: 74 lb FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 17-8-9.
 (lb) - Max Horiz 2=-89(LC 10)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 17, 16, 20, 19, 11, 15 except 13=-173(LC 13), 14=-118(LC 22)
 Max Grav All reactions 250 lb or less at joint(s) 2, 17, 16, 19, 11, 15, 14 except 13=303(LC 22), 20=250(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown
 WEBS 10-13=-266/200

- NOTES** (14-16)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft, Cat. II Exp C; Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1
 - Provide adequate drainage to prevent water ponding
 - All plates are 2x4 MT20 unless otherwise indicated
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 4-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 17, 16, 20, 19, 11, 15 except (l=lb) 13=173, 14=118.
 - Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss
 - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869' Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



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Job 543737	Truss PB07	Truss Type Piggyback Truss	Qty 9	Ply 1	Jennings Additions Job Reference (optional)	17716435
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24 19 2014 Page 1
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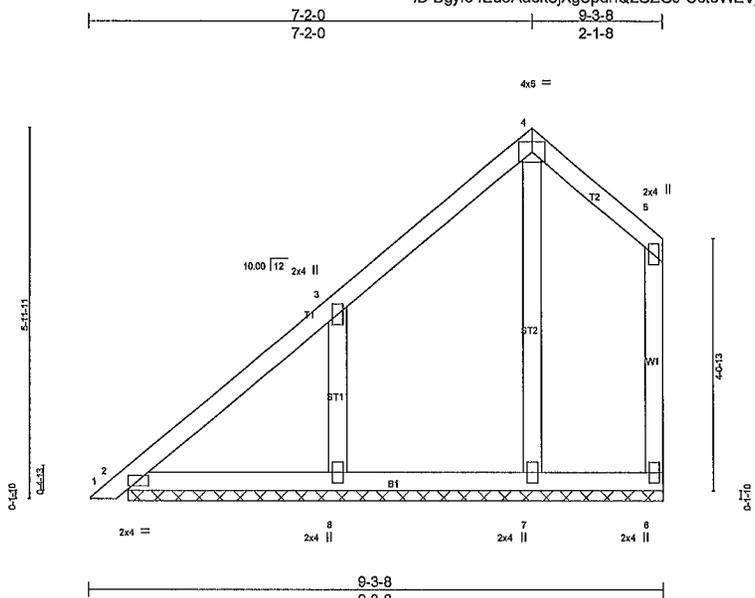


Plate Offsets (X,Y) [2:0-2:1,0-1-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.19	Vert(LL) 0.00 1 n/r 120	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.07	Vert(TL) 0.00 1 n/r 120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.09	Horz(TL) 0.00 6 n/a n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix)		Weight: 48 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
 MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 8-7-13.
 (lb) - Max Horz 2=162(LC 12)
 Max Uplift All uplift 100 lb or less at joint(s) 6, 2, 7 except 8=178(LC 12)
 Max Grav All reactions 250 lb or less at joint(s) 6, 2, 7 except 8=291(LC 21)

FORCES

(lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown
 WEBS 3-8=365/298

NOTES

- Unbalanced roof live loads have been considered for this design
- Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl GCpl=0.18 MWFRS (envelope) and C-C Exterior(2) zone-C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 585 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7 except (if=lb) 8=178.
- "Semi-rigid pitchbreaks including haels" Member end fixity model was used in the analysis and design of this truss
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- Note Visually graded lumber designation SP, represents new lumber design values as per SPIB.
- Truss Design Engineer: Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

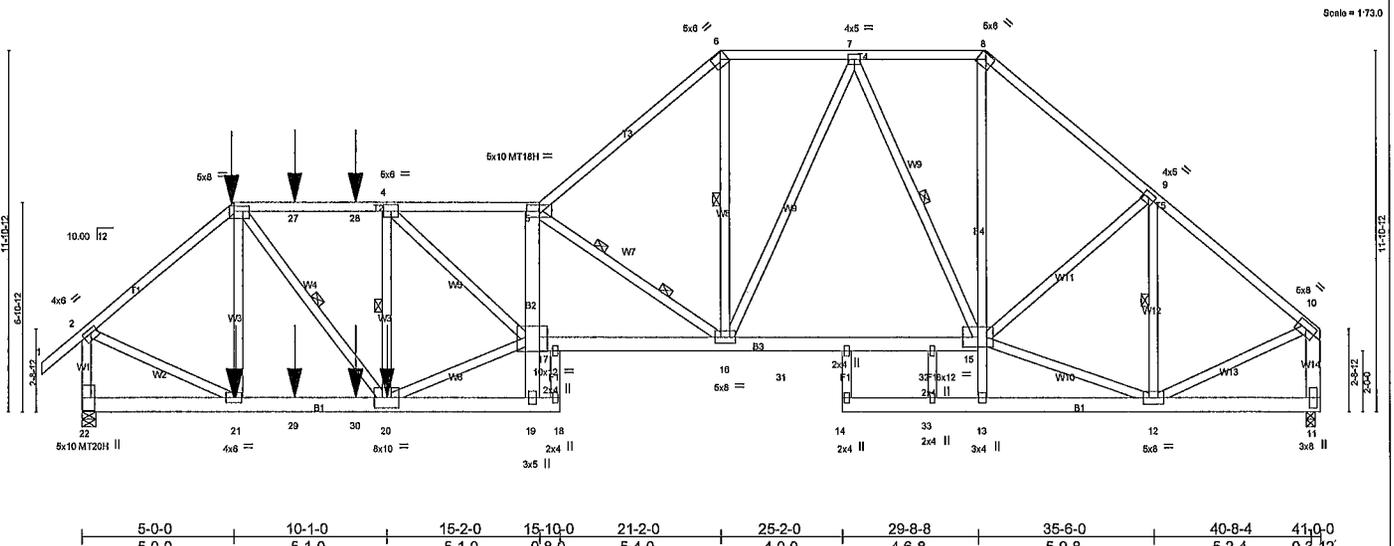
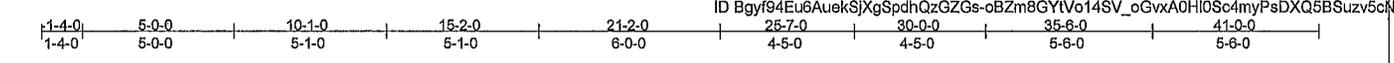


January 15, 2014

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Job 543737	Truss T01	Truss Type Piggyback Base Truss	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17716437
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Builders FirstSource, Lake City FL 32055 7,360 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:22 2014 Page 1
ID Bgyf94Eu6AuekSJXgSpdhQzGZGs-oBZm8GYIvo14SV_oGvxAOHI0Sc4myPsDXQ5BSuzv5cm



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
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TCLL 20.0	Plates Increase 1.25	TC 0 84	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0 92	Vert(LL) -0.44 14 >999 240	MT20H	187/143
BCLL 0.0 *	Rep Stress Incr NO	WB 0 97	Vert(TL) -0.80 14 >612 180	MT18H	244/190
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.27 11 n/a n/a	Weight: 374 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2 *Except* T3. 2x4 SP M 31	TOP CHORD Structural wood sheathing directly applied or 2-0-5 oc purlins, except end verticals
BOT CHORD 2x6 SP No.2 *Except* B3: 2x6 SP SS, B4 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 5-3-3 oc bracing. Except: 10-0-0 oc bracing 17 19 13-15
WEBS 2x4 SP No.3 *Except* W6 W5: 2x4 SP No.2, W14. 2x6 SP No.2	WEBS 1 Row at midpt 3-20 4-20, 6-16 7 15, 9-12 2 Rows at 1/3 pts 5-16
	JOINTS 1 Brace at Jt(s) 15

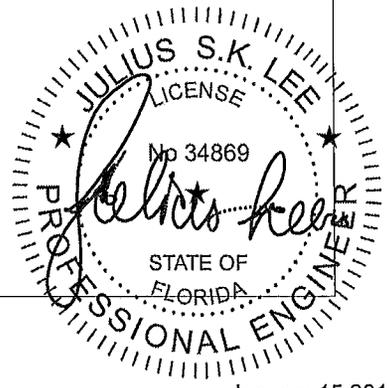
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 22=2460/0-5-8 (min 0-3-4) 11=1848/0-3-8 (min. 0-2-5)
 Max Horz 22=305(LC 5)
 Max Uplift 22=-1712(LC 9) 11=-620(LC 9)
 Max Grav 22=2735(LC 2), 11=1986(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown
 TOP CHORD 2-3=-2562/1678, 3-27=-3387/1867 27-28=-3387/1867 4-28=-3387/1867 4-5=-5366/2506,
 5-6=-3371/1360, 6-7=-2575/1145, 7-8=-1913/749, 8-9=-2565/916, 9-10=-1914/627
 2-22=-2714/1726, 10-11=-1945/630
 BOT CHORD 21-22=-292/286, 21-29=-1358/1877 29-30=-1358/1877 20-30=-1358/1877 19-20=-158/336,
 5-17=-890/0, 16-17=-2534/5438 16-31=-815/2292 31-32=-815/2292 15-32=-815/2292,
 13-15=-0/270 8-15=-508/1314
 WEBS 3-21=-685/29 3-20=-851/2457 4-20=-2152/971 17 20=-1844/3341 4-17=-880/2658,
 5-16=-3543/1958, 6-16=-714/1747 7-16=-470/749 7 15=-999/564, 9-15=-299/718,
 12-15=-443/1544, 9-12=-1142/374, 2-21=-1315/2088, 10-12=-451/1505

- NOTES** (12-14)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) end vertical left exposed; Lumber DOL=1 60 plate grip DOL=1 60
 - Provide adequate drainage to prevent water ponding
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 22=1712 11=620.
 - 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 92 lb down and 208 lb up at 5-0-0, and 92 lb down and 208 lb up at 7-0-12, and 92 lb down and 208 lb up at 9-0-12 on top chord, and 126 lb down and 514 lb up at 5-0-0, 37 lb down and 84 lb up at 7-0-12 and 37 lb down and 84 lb up at 9-0-12 and 1151 lb down and 404 lb up at 10-1-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

Continued on page 2



January 15, 2014

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 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE.
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 543737	Truss T02	Truss Type Piggyback Base Truss	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17716438
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Builders FirstSource Lake City, FL 32065 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:24 2014 Page 1
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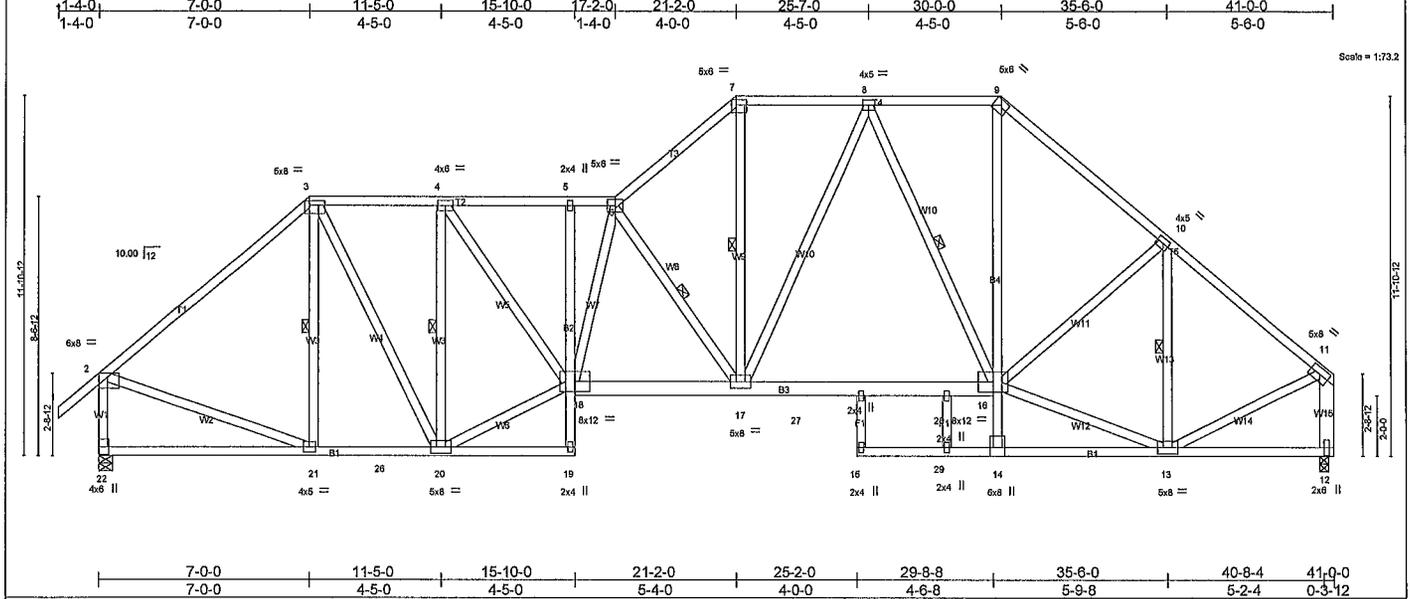


Plate Offsets (X,Y). [2:0-3-8,Edge], [3:0-6-0,0-1 12], [7:0-4-4,0-2-0], [9:0-3-0,0-2-1]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.79	Vert(LL)	-0.74	15	>855	240	244/180
TCDL 7.0	Lumber Increase	1.25	BC 0.99	Vert(TL)	1.18	15	>414	180	
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.91	Horz(TL)	0.17	12	n/a	n/a	
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)						
								Weight. 362 lb	FT = 20%

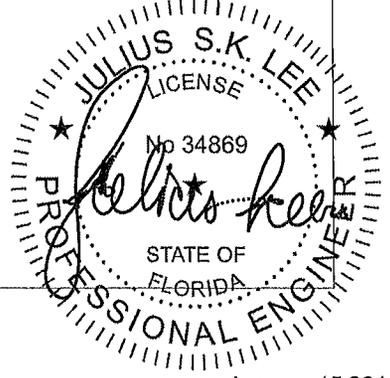
LUMBER	BRACING
TOP CHORD 2x4 SP No.2 *Except* T1 2x4 SP M 31	TOP CHORD Structural wood sheathing directly applied except end verticals. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 8-2-0 oc bracing 16-17 10-0-0 oc bracing: 14-16
BOT CHORD 2x4 SP No.1 *Except* B2 2x4 SP No.3, B3 2x6 SP No.2 B4 2x4 SP No.2	WEBS 1 Row at midpt 3-21 4-20, 6-17 7-17 8-16, 10-13 JOINTS 1 Brace at J(s) 16
WEBS 2x4 SP No.3 *Except* W1 2x4 SP No.2 W15 2x6 SP No.2	

REACTIONS (lb/size) 22=1476/0-5-8 (min 0-2-6) 12=1556/0-3-8 (min. 0-2-9)
 Max Horz 22=307(LC 9)
 Max Uplift 22=410(LC 12) 12=336(LC 13)
 Max Grav 22=1588(LC 2) 12=1651(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1948/875 3-4=-2076/1037 4-5=-3265/1517 5-6=-3264/1516, 6-7=-3287/1492 7-8=-2565/1231 8-9=-2164/1017 9-10=-2795/1203, 10-11=-2027/850 2-22=-1969/912 11-12=-2103/877
BOT CHORD 21-22=-312/315 21-26=-512/1318, 20-26=-512/1318, 17 18=-1315/3276, 17-27=-741/2150, 27-28=-741/2150, 16-28=-741/2150, 9-16=-570/1420
WEBS 3-21=-329/178 3-20=-554/1261 4-20=-1942/867, 18-20=-816/2017 4-18=-846/2093, 6-18=-767/289 6-17=-1799/933 7 17=-773/1786, 8-17=-162/370 8-16=-630/360 13-16=-594/1769 10-16=-162/623, 10-13=-1291/494, 11-13=-582/1554, 2-21=-398/1297

- NOTES** (10-12)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf; h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone; end vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (It=lb) 22=410, 12=336.
 - "Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer Julius Lee, PE: Florida P E License No 34869; Address. 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 Continued on page 2



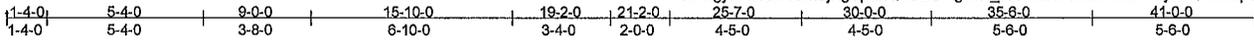
January 15, 2014

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 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 543737	Truss T03	Truss Type Piggyback Base Truss	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17716439
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5x10 MT18H ==

Scale = 1/24.8

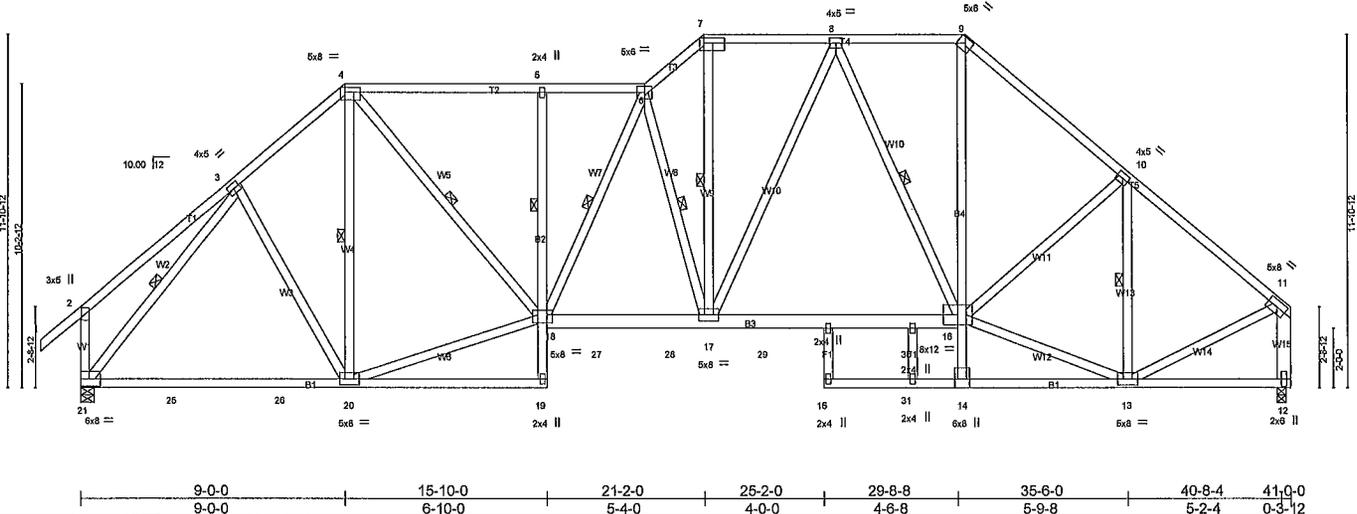


Plate Offsets (X,Y)	[4,0-6-4,0-2-0], [7,0-8-4,0-2-0], [9,0-3-0,0-2-1], [18,0-2-4,0-3-4]
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.80	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 1.00	Vert(LL) -0.75 15 >649 240	MT18H	244/190
BCLL 0.0 *	Rep Stress Incr NO	WB 0.75	Vert(TL) 1.16 15 >420 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.16 12 n/a n/a		
				Weight: 366 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD 2x4 SP No.1 *Except*	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except.
B2: 2x4 SP No.3, B3, 2x6 SP No.2, B4, 2x4 SP No.2	1 Row at midpt 5-18
WEBS 2x4 SP No.3 *Except*	8-2-0 oc bracing 16-17
W15: 2x6 SP No.2	10-0-0 oc bracing 14-16
	WEBS 1 Row at midpt 4-20 4-18, 6-18 6-17 7-17 8-16, 10-13, 3-21
	JOINTS 1 Brace at Jt(s). 16
	MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 12=1590/0-3-8 (min 0-2-9) 21=1561/0-5-8 (min. 0-2-8)
 Max Horz 21=307(LC 9)
 Max Uplift 12=336(LC 13) 21=410(LC 12)
 Max Grav 12=1651(LC 2), 21=1588(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-344/297 3-4=-2082/994 4-5=-2705/1286, 5-6=-2701/1282 6-7=-3323/1523 7-8=-2619/1234,
 8-9=-2193/1020, 9-10=-2833/1206, 10-11=-2052/851 2-21=-465/334 11 12=-2134/879
 BOT CHORD 21-25=-528/1303, 25-26=-528/1303, 20-26=-528/1303 5-18=-363/274, 18-27=-1023/2756
 27-28=-1023/2756, 17-28=-1023/2756, 17-29=-745/2192 29-30=-745/2192 16-30=-745/2192,
 9-16=-572/1444
 WEBS 3-20=-117/266, 4-20=-406/149 18-20=-470/1360, 4-18=-722/1755, 6-18=-871/217
 6-17=-1629/855, 7-17=-822/1877 8-17=-161/400, 8-16=-643/362 13-16=-595/1787
 10-16=-161/639, 10-13=-1307/495 11 13=-583/1575, 3-21=-2044/728

- NOTES (11-13)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCCL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 12=336, 21=410.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



January 15, 2014

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Julius Lee PE
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 543737	Truss T04	Truss Type Hip Truss	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17716440
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Builders FirstSource, Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries Inc. Wed Jan 15 13:24:29 2014 Page 1
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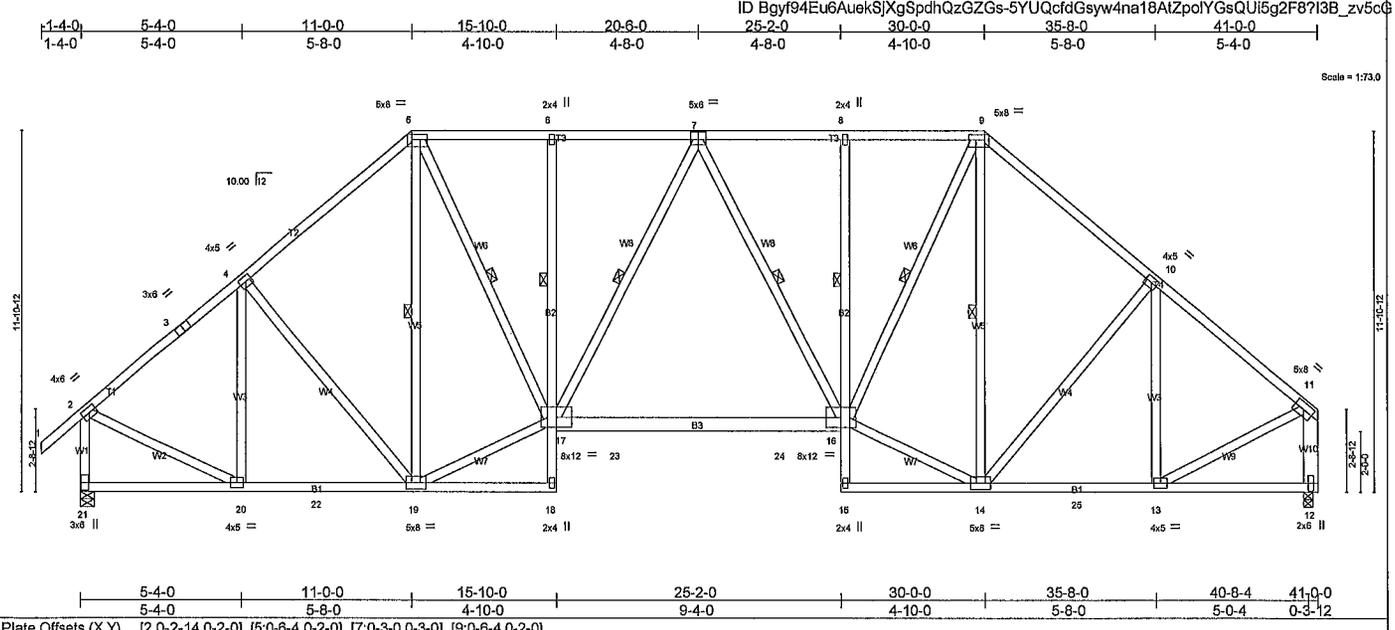


Plate Offsets (X,Y), [2,0-2-14,0-2-0], [5,0-8-4,0-2-0], [7,0-3-0,0-3-0], [9,0-6-4,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.70	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1 25	BC 0.84	Vert(LL) 0.31 16-17 >999 240		
BCLL 0.0 *	Lumber Increase 1 25	WB 0.56	Vert(TL) -0.55 16-17 >882 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix-M)	Horz(TL) 0.14 12 n/a n/a		
	Code FBC2010/TPI2007			Weight: 355 lb	FT = 20%

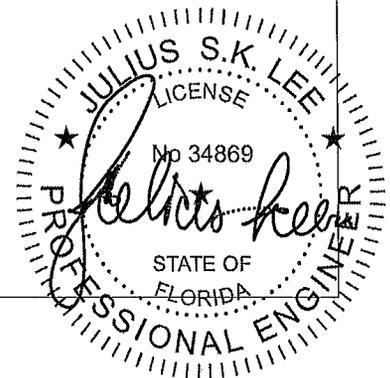
LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-1 13 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
WEBS 2x4 SP No.3 *Except*	1 Row at midpt 6-17 8-16
W10: 2x6 SP No.2	1 Row at midpt 5-19 5-17 7-17 7-16, 9-16, 9-14
	MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 21=1564/0-5-8 (min 0-2-9) 12=1498/0-3-8 (min 0-2-7)
 Max Horz 21=308(LC 9)
 Max Uplift 21=371(LC 12) 12=345(LC 13)
 Max Grav 21=1669(LC 2) 12=1588(LC 2)

FORCES (lb) Max. Comp./Max. Ten All forces 250 (lb) or less except when shown
 TOP CHORD 2-3=1974/870 3-4=1843/886, 4-5=2162/1084 5-6=2423/1241 6-7=2436/1244, 7-8=2430/1238, 8-9=2417/1234, 9-10=2151/1072 10-11=1933/845, 2-21=2132/966, 11-12=2036/879
 BOT CHORD 20-21=288/276, 20-22=581/1411 19-22=581/1411 6-17=271/195, 17-23=871/2238, 23-24=871/2238, 16-24=871/2238, 8-16=271/196, 14-25=553/1386, 13-25=553/1386
 WEBS 4-20=558/280 5-19=603/178, 17-19=578/1667 5-17=734/1769, 14-16=566/1656 9-16=743/1778, 9-14=608/270 10-13=565/307 11-13=591/1507 2-20=547/1539

- NOTES** (10-12)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C End GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 21=371 12=345.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note: Visually graded lumber designation SP, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee, PE: Florida P. E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard
 Continued on page 2



January 15, 2014

<p>WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719</p>	<p>Julius Lee PE. 1109 Coastal Bay Boynton Beach, FL 33435</p>
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Job 643737	Truss T05	Truss Type Piggyback Base Truss	Qty 16	Ply 1	Jonnings Additons Job Reference (optional)	17716441
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Builders FirstSource, Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:32 2014 Page 1
 ID:BgYf94Eu6AuekSjXgSpdhQzGZGs-V6AYEh89lfe1Jr06WQOAKaeVJl1ohqzVjjoJzv5cd

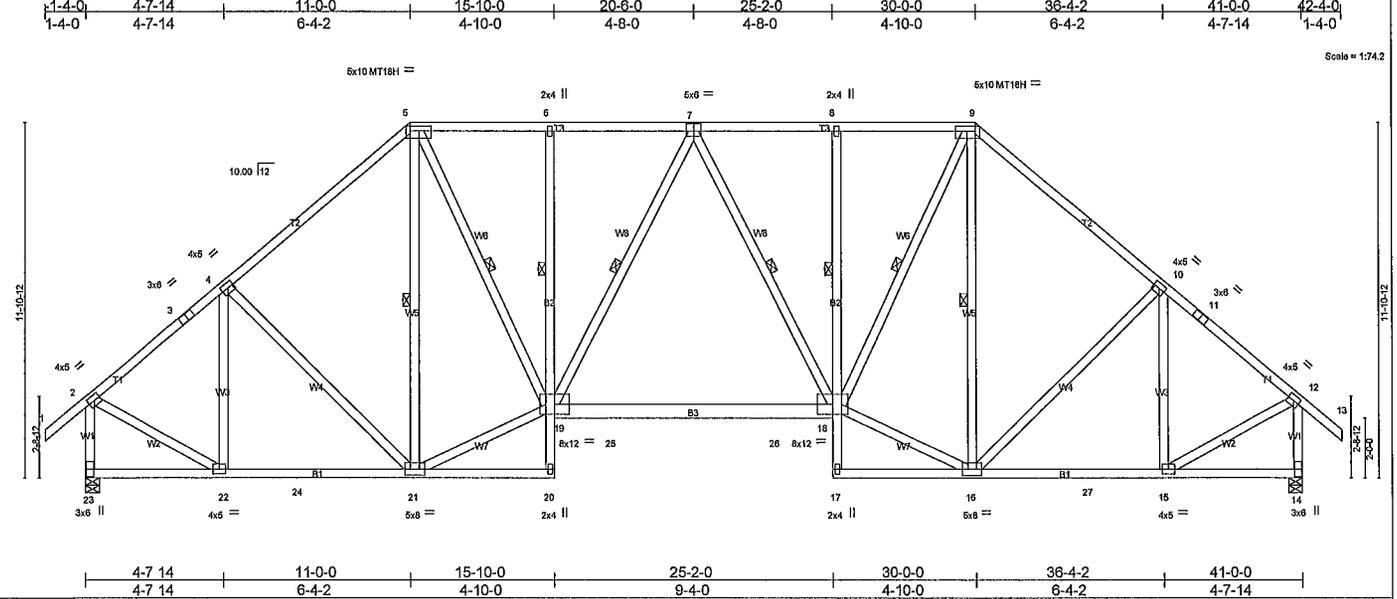


Plate Offsets (X, Y), [5;0-8-4,0-2-0], [7;0-3-0,0-3-0], [9;0-8-4,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.80	Vert(LL)	0.31 18-19	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.85	Vert(TL)	-0.55 18-19	>889	180	MT18H	244/190
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.55	Horz(TL)	0.14 14	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)					Weight: 353 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2 *Except* T2 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied, except end verticals
BOT CHORD 2x4 SP No.2 *Except* B2, 2x4 SP No 3, B3, 2x6 SP SS	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 1 Row at midpt 8-19, 8-18 1 Row at midpt 5-21 5-19, 7 19 7 18 9-18, 9-16
WEBS 2x4 SP No.3	WEBS
	MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 23=1566/0-5-8 (min. 0-2-9) 14=1566/0-5-8 (min 0-2-9)
 Max Horz 23=-327(LC 8)
 Max Uplift 23=-373(LC 12) 14=-373(LC 13)
 Max Grav 23=1671(LC 2), 14=1671(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1896/848, 3-4=-1782/855, 4-5=-2158/1073, 5-6=-2434/1236, 6-7=-2448/1240
 7-8=-2443/1236, 8-9=-2429/1232 9-10=-2158/1073, 10-11=-1782/855, 11-12=-1896/848,
 2-23=-2141/970, 12-14=-2141/970
 BOT CHORD 22-23=-309/313, 22-24=-541/1451 21-24=-541/1451 6-19=-269/188, 19-25=-829/2325,
 25-26=-829/2325, 18-26=-829/2325, 8-18=-269/188, 16-27=-521/1428, 15-27=-521/1428
 WEBS 4-22=-648/324 4-21=-153/251 5-21=-645/188 19-21=-539/1746 5-19=-713/1797
 16-18=-533/1737 9-18=-718/1804, 9-16=-656/276 10-16=-154/251 10-15=-648/324,
 2-22=-581/1577 12-15=-581/1578

- NOTES** (11-13)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II Exp C, Encl GCpl=0.18 MWFRS (envelope) and C-C Exterior(2) zone end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 23=373, 14=373.
 - Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 Continued on page 2



January 15, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
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 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown
 is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of
 the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB 89 and BCS11 Building Component
 Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 543737	Truss T06	Truss Type Piggyback Base Truss	Qty 2	Ply 1	Jennings Additions Job Reference (optional)	17716442
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Builders FirstSource Lake City FL 32055 7 350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:33 2014 Page 1
 ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-zJjxS1gmwAQWGBKvPjelzbl_I1_J1UMr3dGHKlv5cC

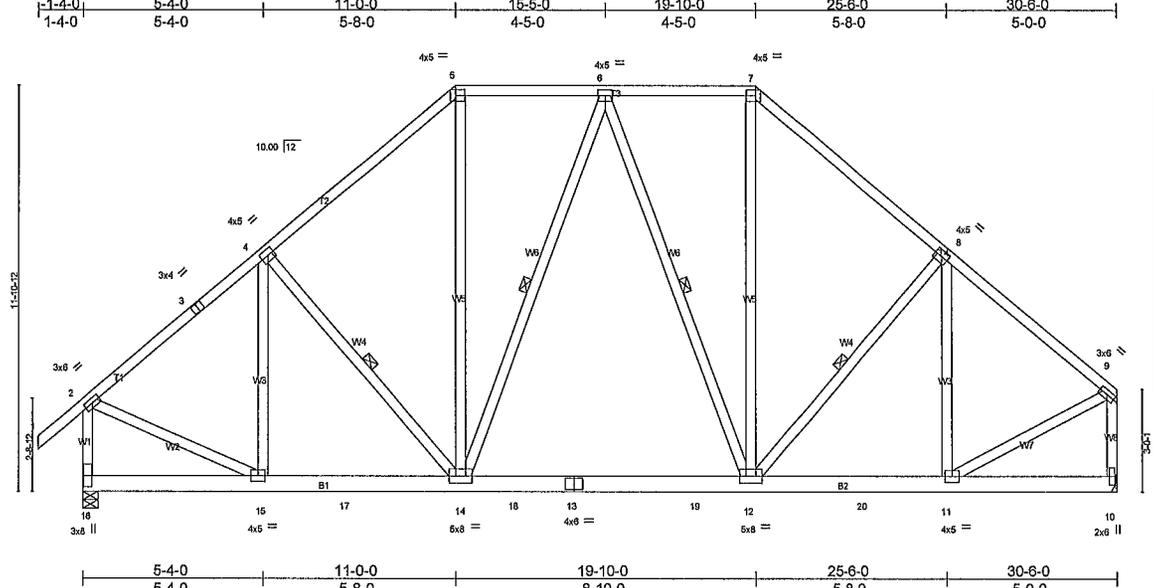


Plate Offsets (X,Y): [5:0-3-4,0-2-0], [7:0-3-4,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.45	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.29	Vert(LL) -0.07 12-14 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.53	Vert(TL) -0.12 12-14 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.02 10 n/a n/a		
	Code FBC2010/TPI2007			Weight: 265 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 4-5-10 oc purlins except end verticals.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 4-14 6-14, 6-12 8-12

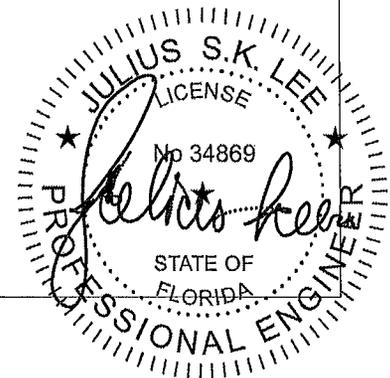
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 16=1035/0-5-8 (min 0-1-11) 10=970/Mechanical
 Max Horz 16=311(LC 10)
 Max Uplift 16=227(LC 12) 10=201(LC 13)
 Max Grav 16=1048(LC 2) 10=970(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1265/521 3-4=-1133/537 4-5=-1296/645, 5-6=-991/585, 6-7=-978/576, 7-8=-1279/634, 8-9=-1184/486, 2-16=-1399/809, 9-10=-1306/524
 BOT CHORD 15-16=-287/289 15-17=-323/872, 14-17=-323/872 14-18=-200/769, 13-18=-200/769
 13-19=-200/769 12-19=-200/769, 12-20=-286/821 11-20=-286/821
 WEBS 4-15=-317/144, 4-14=-262/236, 5-14=-211/461 7 12=-204/462, 8-11=-365/187 2-15=-247/920, 9-11=-305/904

- NOTES** (9-12)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCCL=3.0psf h=20ft; Cat. II Exp C, End GCpl=0.18 MWFRS (envelope) and C-C Exterior(2) zone; end vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCCL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Truss mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=t=b) 16=227 10=201
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee PE: Florida P.E. License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435
 - Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



January 15, 2014

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Julius Lee PE
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	Jennings Additions	17716443
543737	T07	Plggyback Base Truss	3	1	Job Reference (optional)	

Builders FirstSource, Lake City FL 32055

7 350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:35 2014 Page 2
 ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-whrhtll1RogEVVUIX8gD20oH2rYEVJb8XxINPdzv5cA

LOAD CASE(S) Standard

1) Regular Lumber Increase=1.25 Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-44 3-5=-44, 5-7=-44, 15-16=-10 13-14=-10, 13-20=-61(F=-51), 20-21=-91(F=-51) 12-21=-61(F=-51) 11-22=-40 10-22=-10, 8-10=-10



WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 543737	Truss T09	Truss Type Common Truss	Qty 2	Ply 1	Jennings Additions Job Reference (optional)	17716445
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Builders FirstSource, Lake City FL 32055
 7 350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:38 2014 Page 1
 ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-KGXqVkkvkj2oNyDtCGDwgePoV2dzijVaDvz20yzy5c7

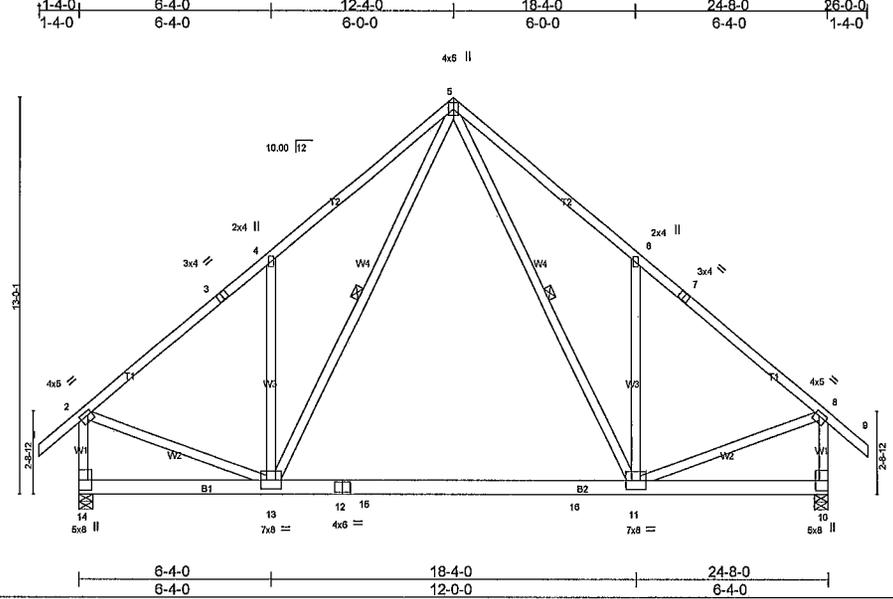


Plate Offsets (X,Y) [2,0-1-12,0-1-8], [8,0-1 12,0-1-8], [10,Edge,0-3-8]

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2010/TPI2007	CSI TC 0.70 BC 0.57 WB 0.67 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.32 11 13 >906 240 Vert(TL) -0.60 11-13 >489 180 Horz(TL) 0.01 10 n/a n/a	PLATES MT20 GRIP 244/190 Weight: 198 lb FT = 20%
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LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP SS
 WEBS 2x4 SP No.3 *Except*
 W4, 2x4 SP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-3-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 5-11 5-13

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 14=1147/0-5-8 (min 0-2-0), 10=1147/0-5-8 (min 0-2-0)
 Max Horz 14=447(LC 8)
 Max Uplift 14=552(LC 12) 10=552(LC 13)
 Max Grav 14=1291(LC 21) 10=1295(LC 21)

FORCES (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1780/650, 3-4=-1623/668, 4-5=-1999/1001 5-6=-1984/1001 6-7=-1608/668, 7-8=-1765/650,
 2-14=-1848/730 8-10=-1834/730
 BOT CHORD 13-14=-440/440, 13-15=-210/831, 12-15=-210/831 12-16=-210/831 11-16=-210/831
 WEBS 5-11=-664/1132 6-11=-625/611 5-13=-664/1142 4-13=-625/511 2-13=-330/1291
 8-11=-330/1305

- NOTES (9-11)**
- Unbalanced roof live loads have been considered for this design.
 - Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl. GCPI=0 18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (It=lb) 14=552, 10=552.
 - Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
 - In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-44, 2-5=-44 5-8=-44, 8-9=-44, 13-14=-10, 13-15=-61(F=-51), 15-16=-91(F=-51) 11-16=-61(F=-51) 10-11=-10



January 15, 2014

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Job	Truss	Truss Type	Qty	Ply	Jennings Additions
543737	T09G	Common Truss	1	1	17716446
<p>Builders FirstSource Lake City FL 32065 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24.40 2014 Page 2 ID Bgyf94Eu6Auek\$JXgSpdhQzGZGs-GfeawQI9GKIWcGNFJhGOI3V4VsMYAgjtgDS84rzy5c5</p> <p>13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.</p> <p>14) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.</p> <p>15) Truss Design Engineer: Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435</p> <p>LOAD CASE(S) Standard</p>					

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

Job 543737	Truss T11	Truss Type Scissor Truss	Qty 3	Ply 1	Jennings Additions Job Reference (optional)	17716448
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Builders FirstSource Lake City FL 32055 ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-9Qu5mnpqKYpy5tg1YXKKvfvVTig6OBSbrQMDczv5c7 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:44 2014 Page 1

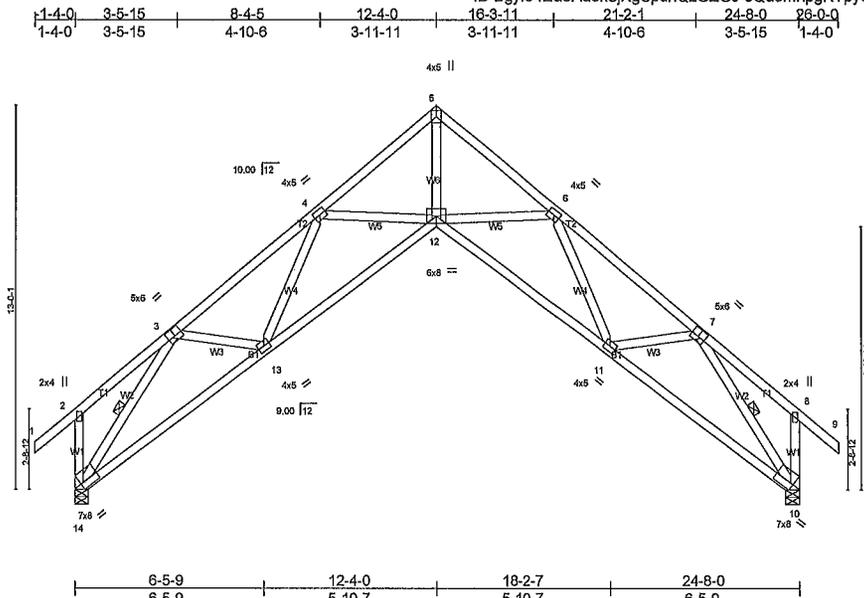


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [10:0-1-3,Edge], [14:0-1-3,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.35	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.43	Vert(LL) -0.17 12 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.78	Vert(TL) -0.32 11 12 >925 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.56 10 n/a n/a		
				Weight: 165 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-8-1 oc purlins, except end verticals
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-5-12 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 3-14, 7 10

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 14=723/0-5-8 (min. 0-1-8), 10=723/0-5-8 (min. 0-1-8)
 Max Horz 14=-489(LC 10)
 Max Uplift 14=-351(LC 12) 10=-351(LC 13)
 Max Grav 14=860(LC 2) 10=860(LC 2)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
 TOP CHORD 3-4=-2123/761 4-5=-2310/522 5-6=-2310/552 6-7=-2076/723, 2-14=-311/233 8-10=-306/234
 BOT CHORD 13-14=-789/1438 12-13=-833/2152, 11-12=-404/2120, 10-11=-416/1339
 WEBS 5-12=-652/2632, 6-12=-383/489 6-11=-581/220 7-11=-109/479, 4-12=-349/449 4-13=-502/775, 3-13=-7422, 3-14=-1859/639, 7 10=-1966/608

- NOTES** (9-11)
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II, Exp C Encl., GCPI=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Bearing at joint(s) 14, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 14 and 351 lb uplift at joint 10
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 15, 2014

<p>WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE. Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719</p>	<p>Julius Lee PE 1109 Coastal Bay Boynton Beach FL 33435</p>
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Job 543737	Truss T12	Truss Type Special Truss	Qty 16	Ply 1	Jennings Additions Job Reference (optional)	17716450
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Builders FirstSource, Lake City FL 32055 ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-1B7bb9sANnJOZV_onNPG3lqaD44e2Dn2WTOZLNzv5bz 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:48 2014 Page 1

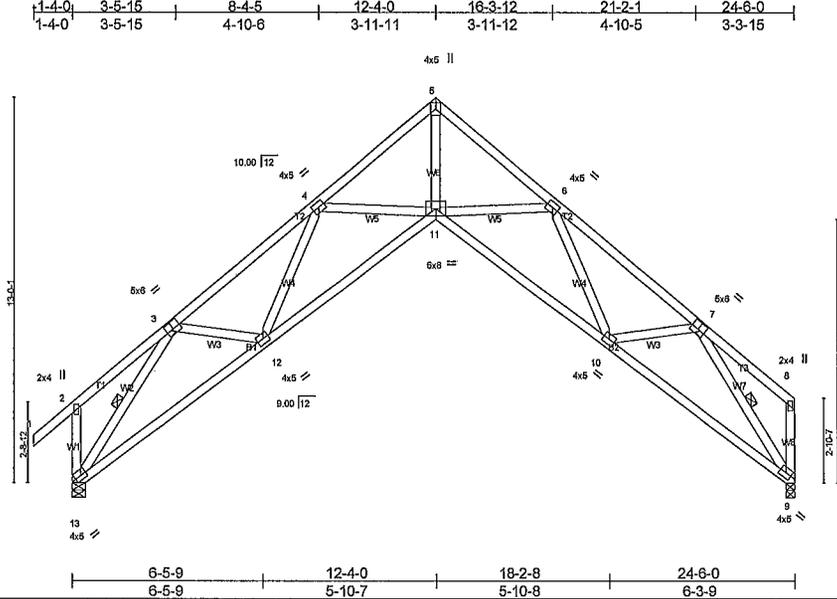


Plate Offsets (X,Y): [3-0-3-0-0-3-0], [7-0-3-0-0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.37	In (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.43	Vert(LL) 0.17 11 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.75	Vert(TL) -0.29 11-12 >990 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.52 9 n/a n/a		
	Code FBC2010/TPI2007			Weight: 161 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-8-13 oc purlins, except end verticals
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-10-12 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 3-13, 7-9

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 13=721/0-5-8 (min 0-1-8) 9=652/0-3-8 (min 0-1-8)
 Max Horz 13=380(LC 9)
 Max Uplift 13=-195(LC 12) 9=-170(LC 13)
 Max Grav 13=857(LC 2), 9=772(LC 2)

FORCES (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.
 TOP CHORD 3-4=-2065/919 4-5=-2213/788, 5-6=-2212/787 6-7=-1997/842 2-13=-310/236
 BOT CHORD 12-13=-679/1350, 11 12=-734/2052 10-11=-685/2007 9-10=-555/1245
 WEBS 3-12=-12/402 4-12=-473/83, 4-11=-326/418, 5-11=-871/2506, 6-11=-344/423, 6-10=-523/176,
 7 10=-79/437, 3-13=-1812/707 7-9=-1873/808

- NOTES** (9-11)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 6) Bearing at joint(s) 13, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 13 and 170 lb uplift at joint 9.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - 10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - 11) Truss Design Engineer: Julius Lee, PE. Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 15, 2014

<p>WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719</p>	<p>Julius Lee PE 1109 Coastal Bay Boynton Beach FL 33435</p>
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Job 543737	Truss T13G	Truss Type Common Truss	Qty 1	Ply 1	Jennings Additions	17716452
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Builders FirstSource, Lake City FL 32055
 7,350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24.51 2014 Page 1
 ID: Bgyf94Eu6AuekSjXgSpdhQzGZGs-SmpkEBu3gihyQyiNTVyzhOS4ol9FFeNUCRdEiyz5bvw

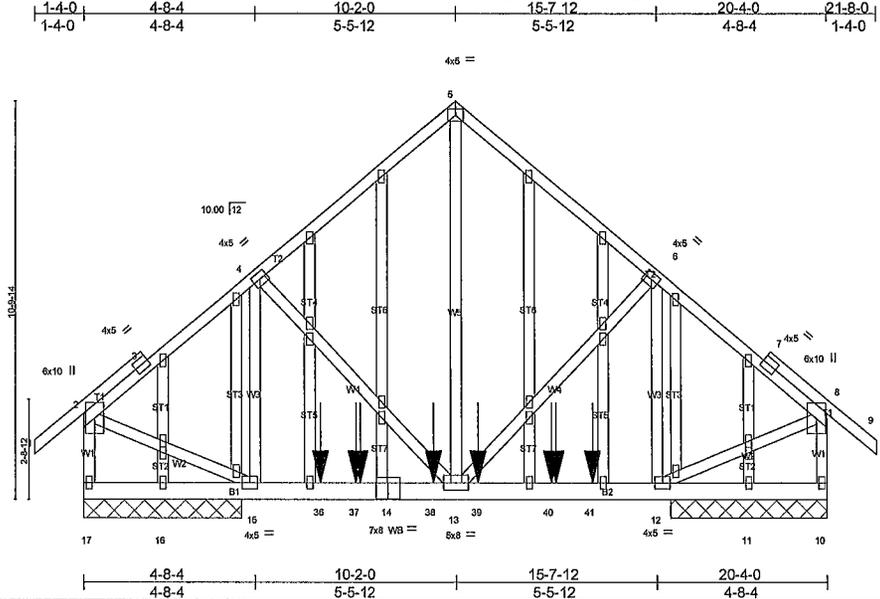


Plate Offsets (X,Y) [14:0-4-0,0-0-4], [25:0-1-13,0-1-0], [35:0-1-13,0-1-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.41	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.18	Vert(LL) 0.02 12-13 >999 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.50	Vert(TL) -0.02 12-13 >999 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.00 10 n/a n/a		
				Weight: 241 lb	FT = 20%

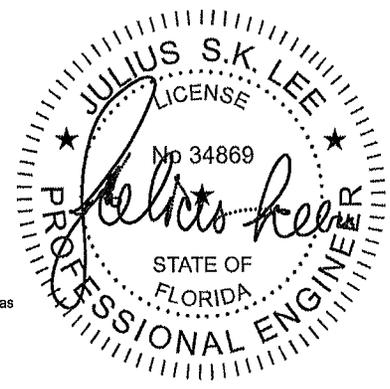
LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD
BOT CHORD 2x6 SP No.2	BOT CHORD
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 Rigid ceiling directly applied or 10-0-0 oc bracing
 MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 4-3-12.
 (lb) - Max Horz 17=373(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) 17 10, 16, 11 except 12=688(LC 9) 15=708(LC 8)
 Max Grav All reactions 250 lb or less at joint(s) 16, 11 except 17=284(LC 2), 12=688(LC 2) 15=672(LC 2) 10=284(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.
 TOP CHORD 4-5=-338/352 5-6=-338/345, 2-17=-259/75 8-10=-259/49
 BOT CHORD 16-17=-352/348, 15-16=-352/348, 15-36=-238/275 36-37=-238/275, 14-37=-238/275
 14-38=-238/275, 13-38=-238/275
 WEBS 6-12=-531/457 4-15=-531/465

- NOTES** (13-15)
- Unbalanced roof live loads have been considered for this design
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf h=20ft; Cat. II Exp C Encl GCpi=0.18; MWFRS (envelope) gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1
 - All plates are 2x4 MT20 unless otherwise indicated
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17 10 16 11 except (j=lb) 12=688 15=706.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 26 lb down and 64 lb up at 6-5-9 58 lb down and 91 lb up at 7-5-4 53 lb down and 93 lb up at 7-6-12, 53 lb down and 93 lb up at 9-6-12, 53 lb down and 93 lb up at 10-9-4, 53 lb down and 93 lb up at 12-9-4 and 58 lb down and 91 lb up at 12-10-12, and 28 lb down and 64 lb up at 13-10-8 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - Truss Design Engineer Julius Lee PE: Florida P E License No 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



Continued on page 2
 LOAD CASE(S) Standard

January 15, 2014

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach FL 33435

Job 543737	Truss T14	Truss Type Special Truss	Qty 3	Ply 1	Jennings Additions Job Reference (optional)	17716453
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24.53 2014 Page 1
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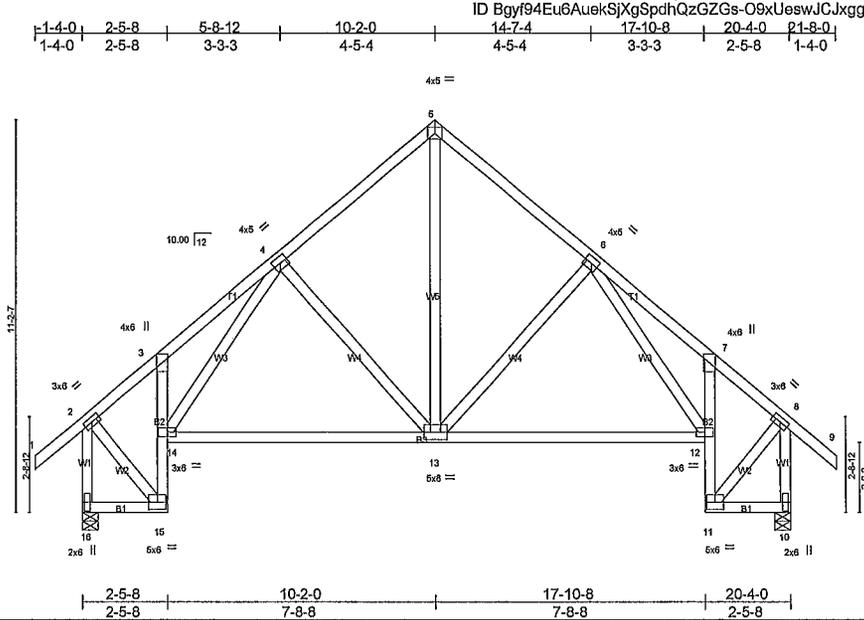


Plate Offsets (X, Y). [3:0-4-6,Edge], [7:0-4-6,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.45	Vert(LL) -0.15 13-14 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.84	Vert(TL) -0.28 13-14 >858 180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.57	Horz(TL) 0.62 10 n/a n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)			
				Weight: 153 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 5-5-13 oc purlins, except end verticals
BOT CHORD 2x4 SP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 2x4 SP No.3	MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 16=606/0-5-8 (min 0-1-8) 10=606/0-5-8 (min. 0-1-8)
 Max Horz 16=-337(LC 10) 10=-167(LC 13)
 Max Uplift 16=-167(LC 12) 10=-167(LC 13)
 Max Grav 16=721(LC 2), 10=721(LC 2)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-660/325, 3-4=-1077/543 4-5=-832/431 5-6=-832/430, 6-7=-995/473, 7-8=-626/301
 2-16=-1030/428, 8-10=-983/461
 BOT CHORD 15-16=-326/310, 14-15=-372/130, 3-14=-303/181 13-14=-229/727 12-13=-159/708
 11 12=-312/87 7 12=-338/162
 WEBS 5-13=-379/681 6-13=-356/268, 4-13=-385/291 4-14=-269/299 2-15=-153/575, 8-11=-141/558

- NOTES** (8-10)
- Unbalanced roof live loads have been considered for this design.
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl GCpl=0.18, MWFRS (envelope) and C-C Exterior(2) zone end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 16=167 10=167
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

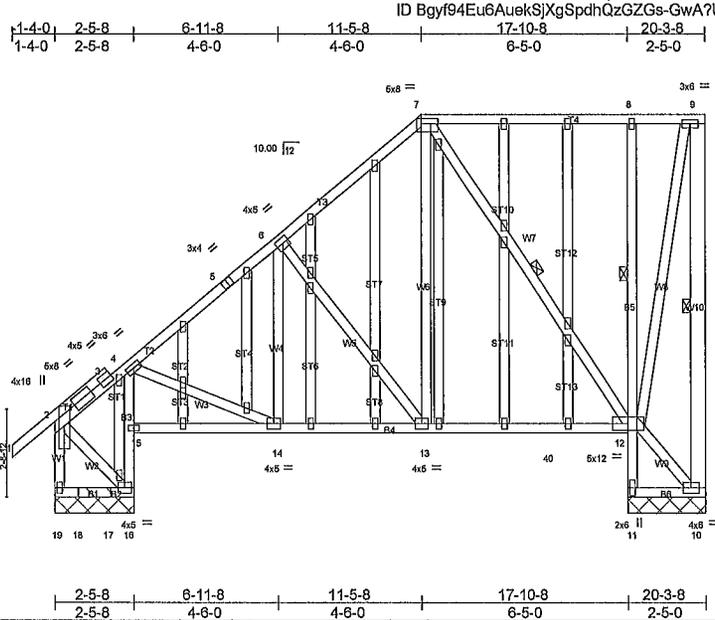


January 15, 2014

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Job 543737	Truss T15G	Truss Type GABLE	Qty 1	Ply 1	Jennings Additions	17716455
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Builders FirstSource Lake City FL 32065 7 350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:57 2014 Page 1



Job 543737	Truss T17	Truss Type Monopitch Truss	Qty 11	Ply 1	Jennings Additions Job Reference (optional)	17716457
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Builders FirstSource Lake City FL 32055 7,350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:24:59 2014 Page 1
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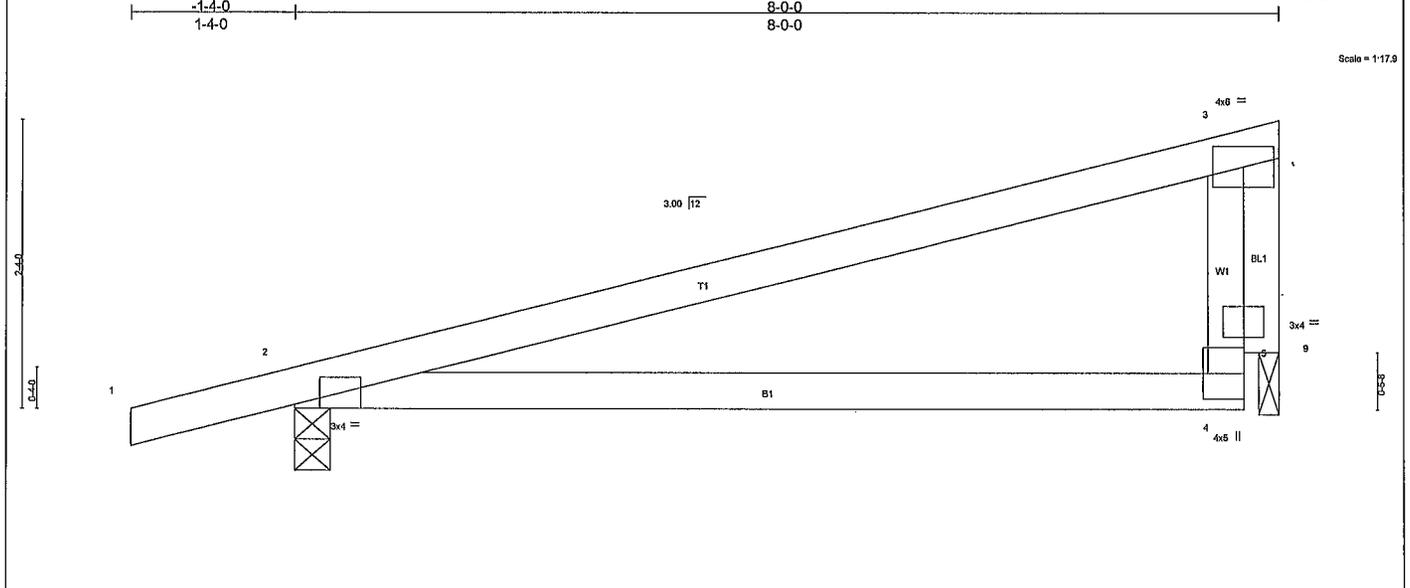


Plate Offsets (X,Y) [2,0-2-7,Edge], [4,Edge,0-3-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.56	Vert(LL) 0.27 4-8 >351 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.46	Vert(TL) 0.23 4-8 >411 180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.01 2 n/a n/a		
BCDL 5.0	Code FBC2010/TP12007	(Matrix-M)		Weight: 31 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 9-6-11 oc bracing
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

REACTIONS (lb/size) 2=303/0-3-8 (min. 0-1-8) 9=164/0-2-0 (min. 0-1-8)
 Max Horz 2=83(LC 8)
 Max Uplift 2=-249(LC 8) 9=-140(LC 8)
 Max Grav 2=362(LC 2) 9=194(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-866/1677
 BOT CHORD 2-4=-1853/949

- NOTES** (9-11)
- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCCL=3.0psf; h=20ft; Cat. II Exp C, Encl, GCPI=0.18, MWFRS (envelope) and C-C Exterior(2) zone porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - 5) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TP1 1 angle to grain formula Building designer should verify capacity of bearing surface
 - 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 9
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 2=249 9=140.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code.
 - 10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - 11) Truss Design Engineer: Julius Lee PE: Florida P. E. License No. 34869; Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435



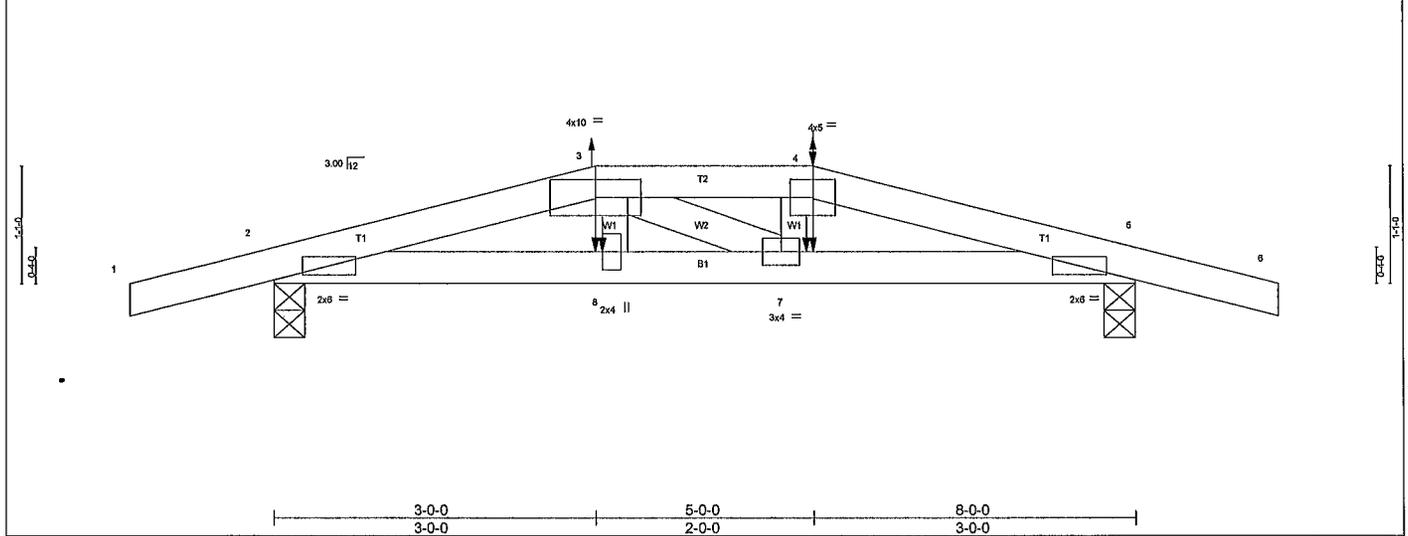
LOAD CASE(S) Standard

January 15, 2014

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Job 543737	Truss T19	Truss Type Hip Truss	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17716459
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Builders FirstSource Lake City FL 32055 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:25:01 2014 Page 1
 ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-9lQWkboKJnyYdUTl2c8J5VsshKaObGCyV_2mJ7zv5brn
 Scale = 1/20.6



LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2010/TP12007	CSI TC 0.13 BC 0.19 WB 0.03 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.03 7-8 >999 240 Vert(TL) -0.03 7-8 >999 180 Horz(TL) 0.01 5 n/a n/a	PLATES GRIP MT20 244/190 Weight: 32 lb FT = 20%
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LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

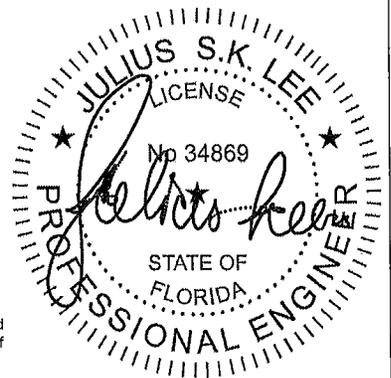
BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
 BOT CHORD Rigid ceiling directly applied or 8-3-6 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=307/0-3-8 (min. 0-1-8) 5=312/0-3-8 (min 0-1-8)
 Max Horz 2=19(LC 4)
 Max Uplift 2=289(LC 4) 5=299(LC 5)
 Max Grav 2=366(LC 2), 5=371(LC 2)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown
 TOP CHORD 2-3=626/524, 3-4=618/549, 4-5=644/554
 BOT CHORD 2-8=489/593, 7-8=499/600, 5-7=510/610

- NOTES (11-13)**
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C, Encl. GCpi=0.18 MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=l=b) 2=289 5=299.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 8 lb down and 52 lb up at 3-0-0, and 25 lb down and 121 lb up at 5-0-0 on top chord and 31 lb down and 82 lb up at 3-0-0, and 31 lb down and 82 lb up at 4-11-4 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee, PE, Florida P.E. License No 34869 Address 1109 Coastal Bay Blvd Boynton Beach FL 33435



LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-44, 3-4=-44 4-6=-44 9-12=-10
 Concentrated Loads (lb)
 Vert: 3=0(B) 4=-18(B) 8=-26(B) 7=-26(B)

January 15, 2014

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Job 543737	Truss V02	Truss Type GABLE	Qty 2	Ply 1	Jennings Additions Job Reference (optional)	17716461
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Builders FirstSource, Lake City FL 32055 7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jan 15 13:25:03 2014 Page 1
ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-54XGHI1brOCFsdgA0AnAwy9k7Fb36mFzIXsN0zv5b6

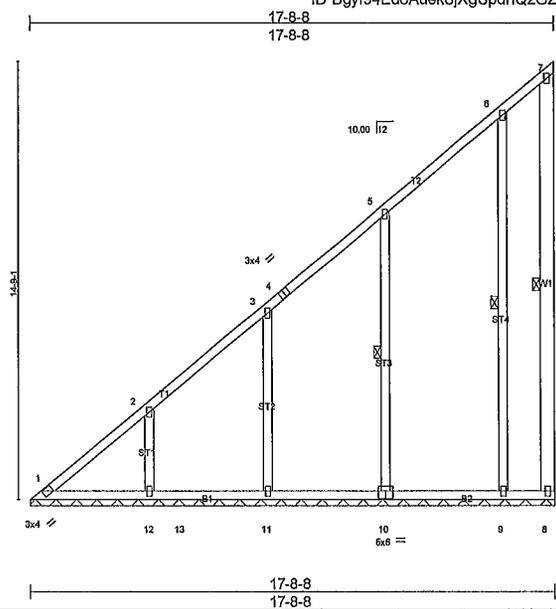


Plate Offsets (X,Y): [10:0-3,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL) n/a n/a 999	MT20	244/180
TCDL 7.0	Lumber Increase 1.25	BC 0.14	Vert(TL) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.28	Horz(TL) -0.00 8 n/a n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix)			
				Weight: 139 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x6 SP No.2	WEBS 1 Row at midpt 7-8, 5-10 6-9
OTHERS 2x4 SP No.3 *Except* ST4, 2x4 SP No 2	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS	All bearings 17-8-8.
(lb) - Max Horz	1=493(LC 12)
Max Uplift	All uplift 100 lb or less at joint(s) 8 except 1=-114(LC 10) 12=-189(LC 12) 11=-173(LC 12) 10=-185(LC 12) 9=-138(LC 12)
Max Grav	All reactions 250 lb or less at joint(s) 8 except 1=349(LC 12) 12=345(LC 21) 11=411(LC 21), 10=431(LC 21) 9=315(LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-801/678, 2-3=-594/502, 3-4=-393/303, 4-5=-373/336
WEBS 2-12=-386/293, 3-11=-368/280, 5-10=-390/296, 6-9=-300/227

- NOTES (9-11)**
- 1) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft, Cat. II Exp C, Encl GCpl=0 18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - 2) All plates are 2x4 MT20 unless otherwise indicated.
 - 3) Gable requires continuous bottom chord bearing.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
 - 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (j=lb) 1=114, 12=189, 11=173, 10=185, 9=138.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
 - 10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard



January 15, 2014

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Job 543737	Truss V04	Truss Type Valley Truss	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17716463
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Bulliders FirstSource, Lake City FL 32055
 7.350 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:25 08 2014 Page 1
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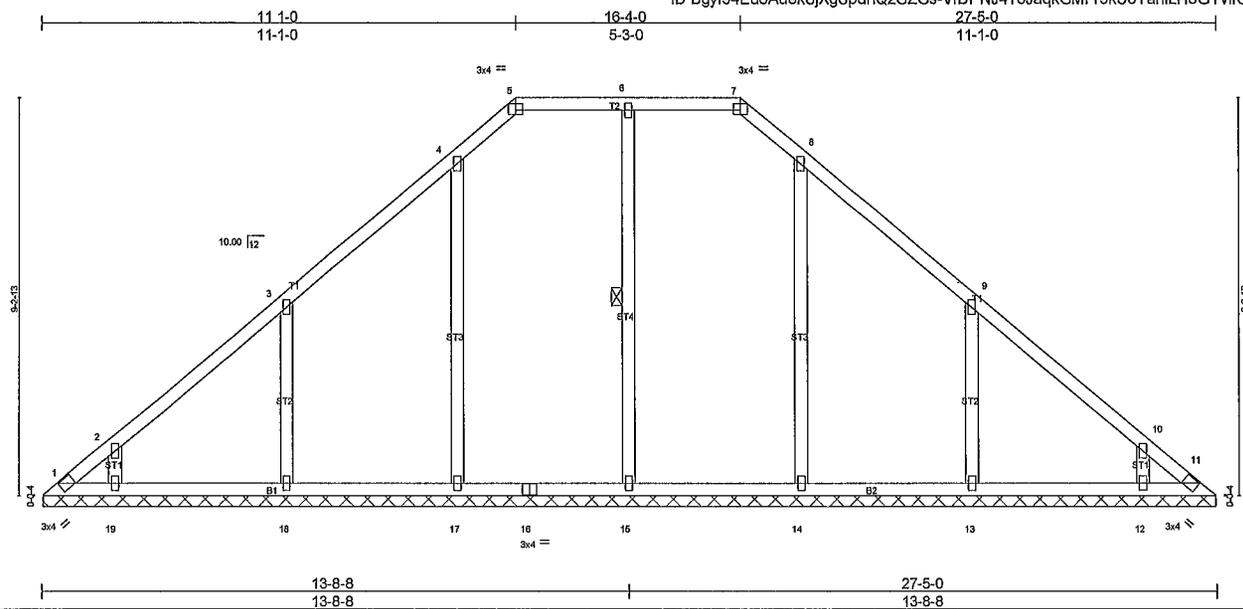


Plate Offsets (X,Y): [5:0-2-0,0-1 13], [7:0-2-0,0-1 13]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.24	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.15	Vert(TL)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.32	Horz(TL)	0.01	11	n/a		
BCDL 5.0	Code	FBC2010/TPI2007	(Matrix)					Weight: 140 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.3	WEBS 1 Row at midpt 6-15

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 27-4-6.
 (lb) - Max Horz 1=-228(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) 15 except 1=-132(LC 10) 11=-112(LC 11)
 17=-115(LC 12) 18=-195(LC 12) 19=-143(LC 12) 14=-108(LC 13) 13=-196(LC 13)
 12=-142(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 1 11 19 12 except 15=302(LC 1) 17=380(LC 21) 18=373(LC 21) 14=370(LC 22) 13=375(LC 22)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-384/341 10-11=-384/341
 BOT CHORD 1 19=-247/290, 18-19=-247/290, 17-18=-247/290, 16-17=-247/290, 15-16=-247/290,
 14-15=-247/290, 13-14=-247/290, 12-13=-247/290 11-12=-247/290
 WEBS 4-17=-279/187, 3-18=-404/312, 2-19=-312/234 8-14=-279/187 9-13=-404/312, 10-12=-312/234

- NOTES** (11-13)
- 1) Unbalanced roof live loads have been considered for this design
 - 2) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TC DL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All plates are 2x4 MT20 unless otherwise indicated
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf
 - 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15 except (if=lb) 1=132 11=112, 17=115, 18=195, 19=143, 14=108, 13=196, 12=142.
 - 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
 - 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 12) Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - 13) Truss Design Engineer: Julius Leo, PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach FL 33435

LOAD CASE(S) Standard



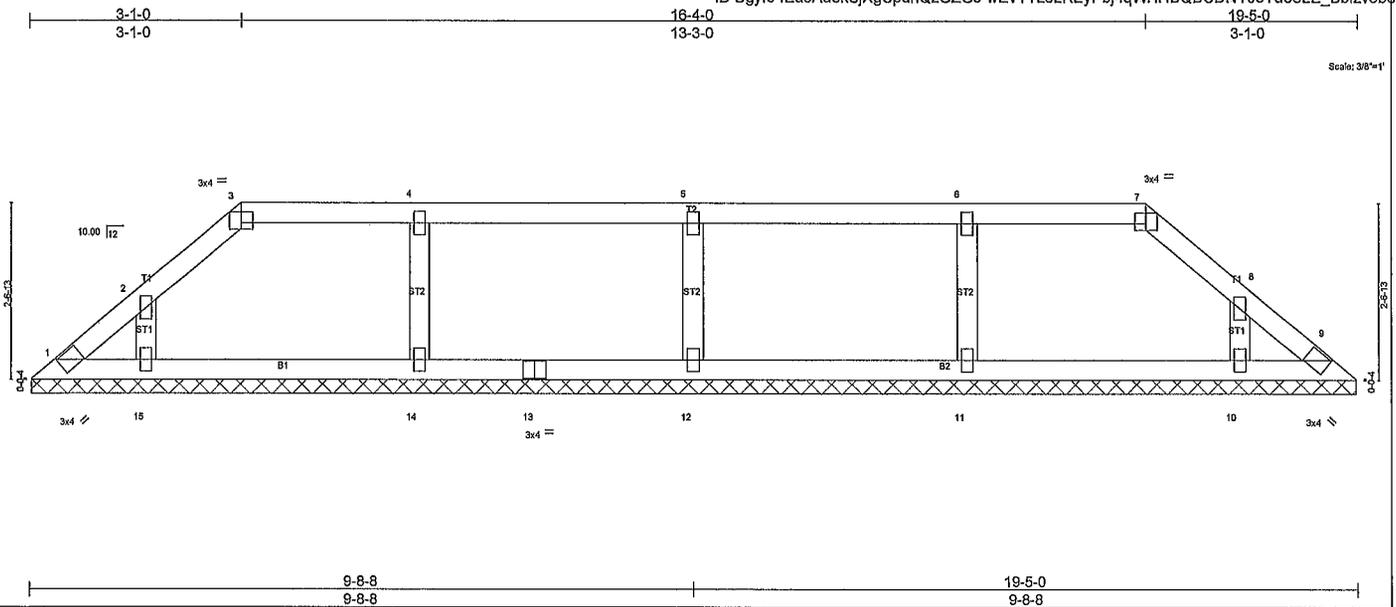
January 15, 2014

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Job 543737	Truss V06	Truss Type Valley Truss	Qty 1	Ply 1	Jennings Additions Job Reference (optional)	17716465
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Builders FirstSource, Lake City FL 32056

7 360 s Sep 27 2012 MITek Industries, Inc. Wed Jan 15 13:25:09 2014 Page 1
ID Bgyf94Eu6AuekSjXgSpdhQzGZGs-wEvY?L6LREyPbj4qW-IHBQBcdNYJoTuo8LE_Bbfzv5bo



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.15	Vert(LL) n/a	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.09	Vert(TL) n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(TL) 0.00	9	n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix)				Weight: 70 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

REACTIONS All bearings 19-4-6.
(lb) - Max Horz 1=58(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 12, 14, 15, 11, 10
Max Grav All reactions 250 lb or less at joint(s) 1, 9, 14, 15, 11, 10 except 12=263(LC 27)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

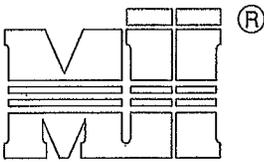
- NOTES** (11 13)
- Unbalanced roof live loads have been considered for this design.
 - Wind, ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCCL=3.0psf; h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding
 - All plates are 2x4 MT20 unless otherwise indicated
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 12, 14, 15, 11, 10.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Note Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 - Truss Design Engineer: Julius Lee, PE; Florida P E License No 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard



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MITek Industries, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

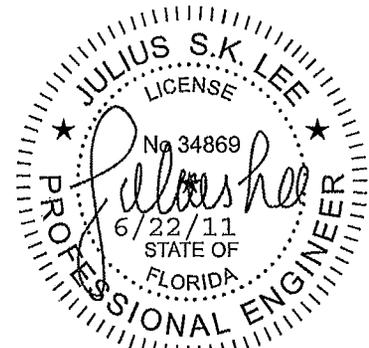
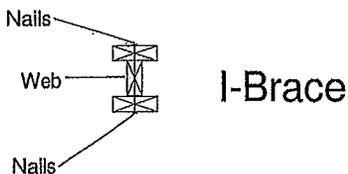
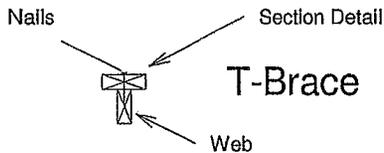
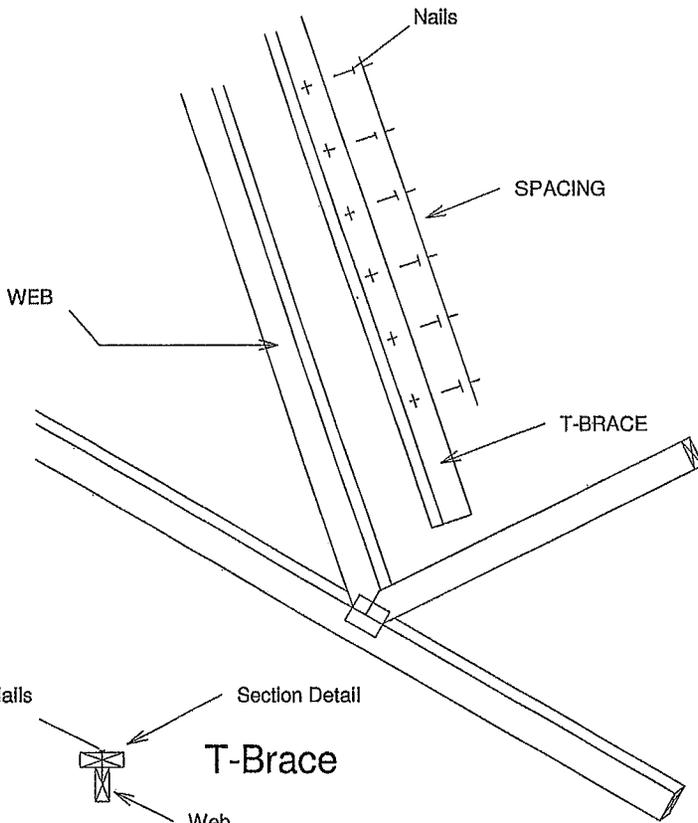
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.

Note: Nail along entire length of T-Brace / I-Brace
(On Two-Ply's Nail to Both Plies)

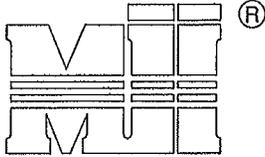
Web Size	Brace Size for One-Ply Truss	
	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Web Size	Brace Size for Two-Ply Truss	
	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.



1109 COASTAL BAY
BOYNTON BC, FL 33435



MITek Industries, Inc.

NOTES:

- TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
- THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

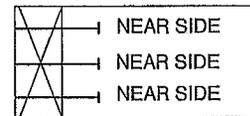
	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

SIDE VIEW

3 NAILS



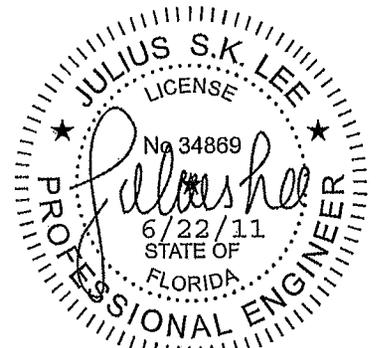
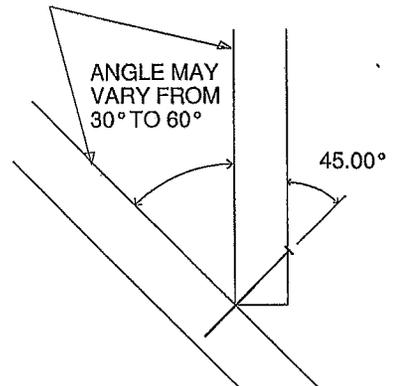
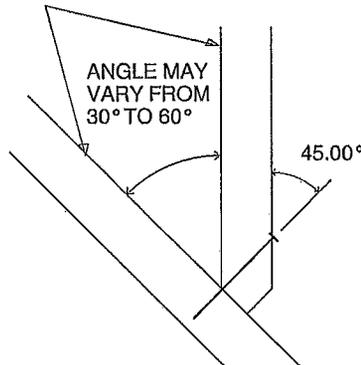
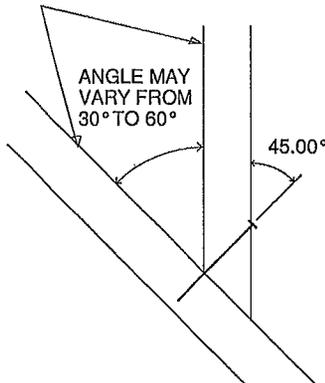
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

$3 \text{ (nails)} \times 84.5 \text{ (lb/nail)} \times 1.15 \text{ (DOL)} = 291.5 \text{ lb Maximum Capacity}$



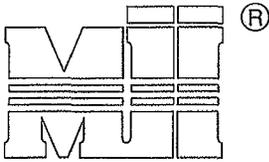
1109 COASTAL BAY
BOYNTON BC, FL 33435

FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

MiTek Industries, Chesterfield, MO

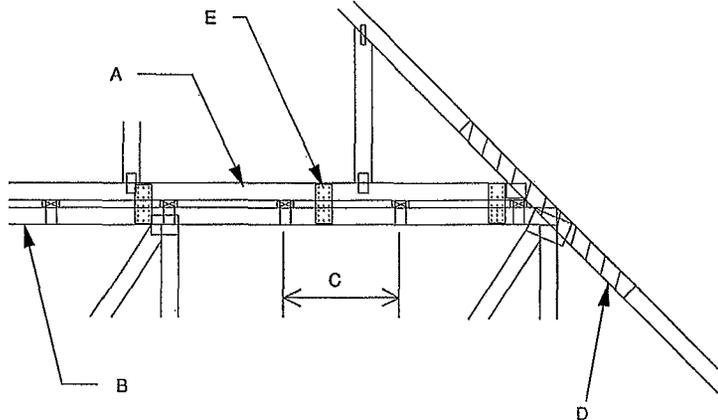


MiTek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
 MAX MEAN ROOF HEIGHT = 30 FEET
 MAX TRUSS SPACING = 24" O.C.
 CATEGORY II BUILDING
 EXPOSURE B or C
 ASCE 7-10
 DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERRING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

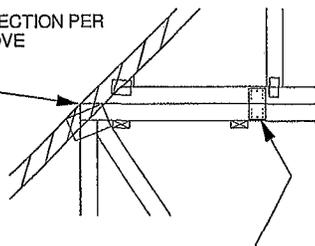
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH
- D - 2 X $\frac{1}{4}$ " X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND
 - 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 - 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ REGARDLESS OF SPAN)



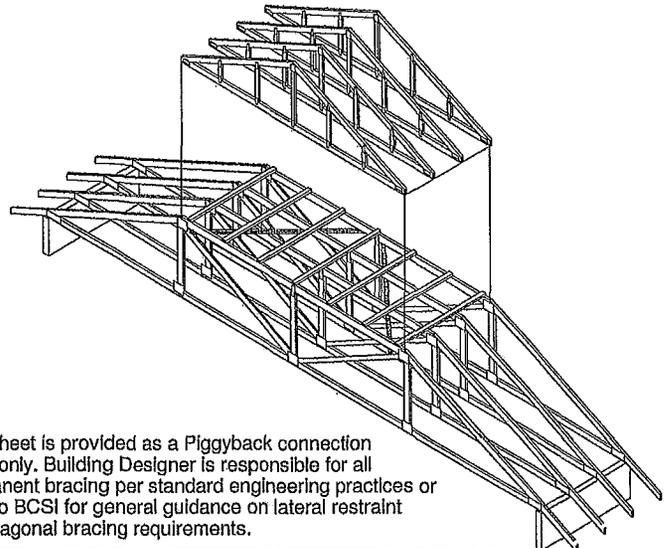
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS.

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER NOTE D ABOVE

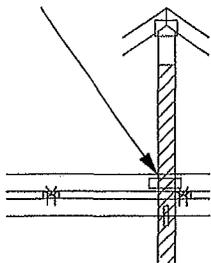


FOR ALL WIND SPEEDS, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



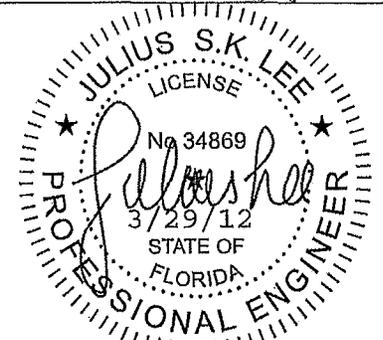
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

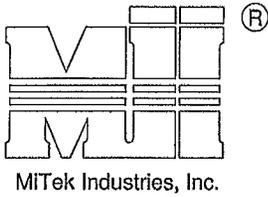


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

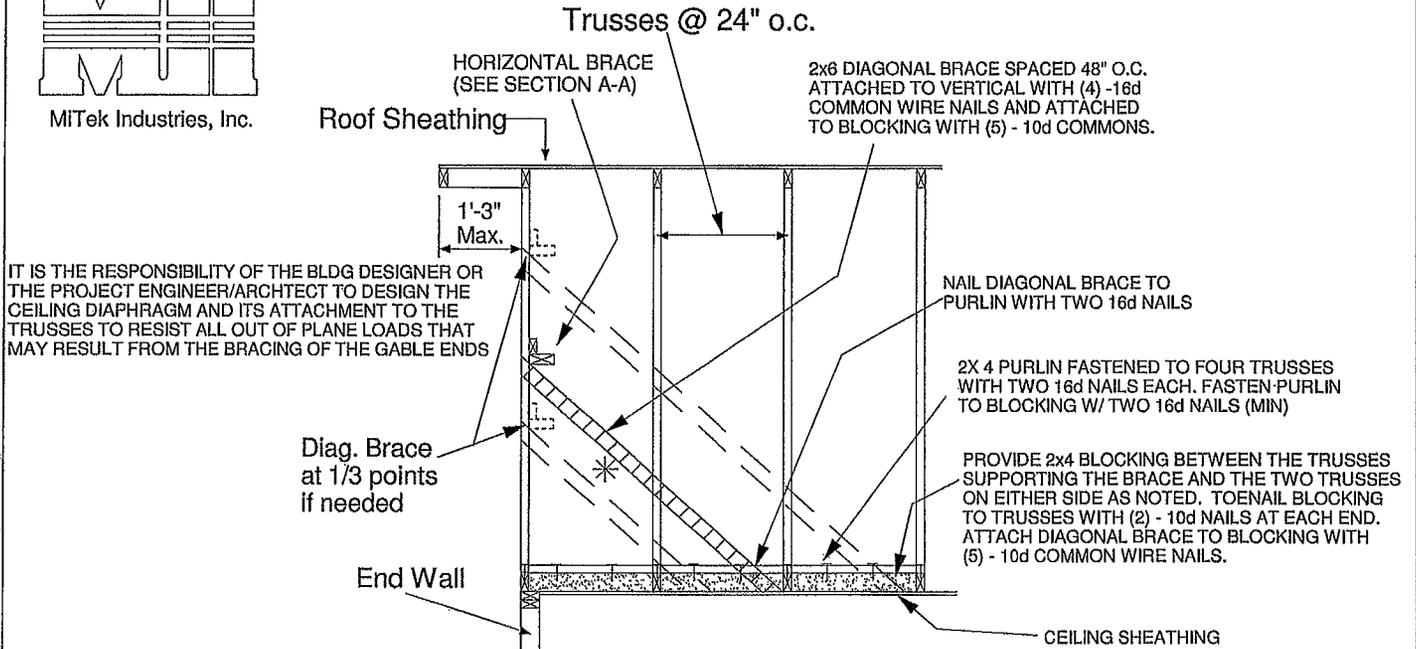
- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x $\frac{1}{4}$ " x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



IT IS THE RESPONSIBILITY OF THE BLDG DESIGNER OR THE PROJECT ENGINEER/ARCHTCT TO DESIGN THE CEILING DIAPHRAGM AND ITS ATTACHMENT TO THE TRUSSES TO RESIST ALL OUT OF PLANE LOADS THAT MAY RESULT FROM THE BRACING OF THE GABLE ENDS

BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

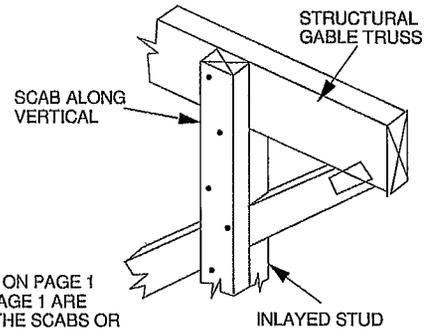
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1 - ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2 - ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O C (2X 4 STUDS MINIMUM)

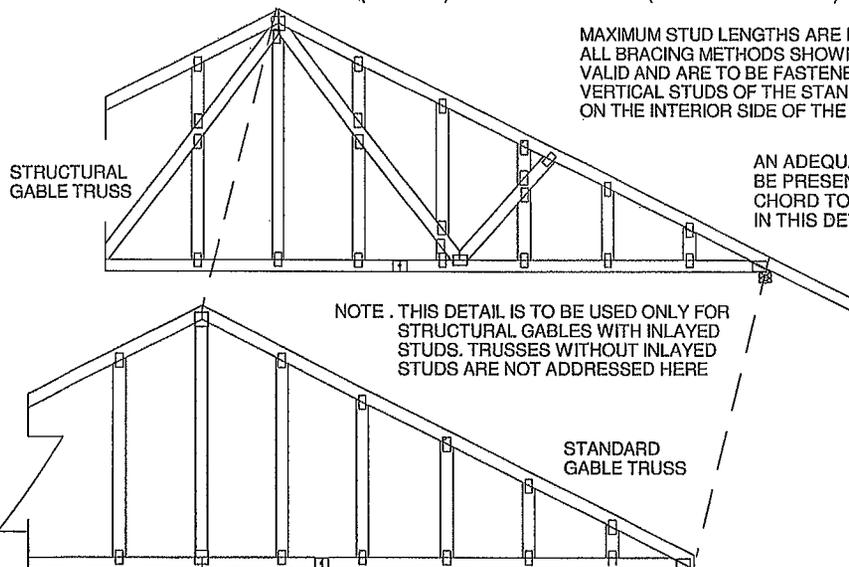


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1 ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.

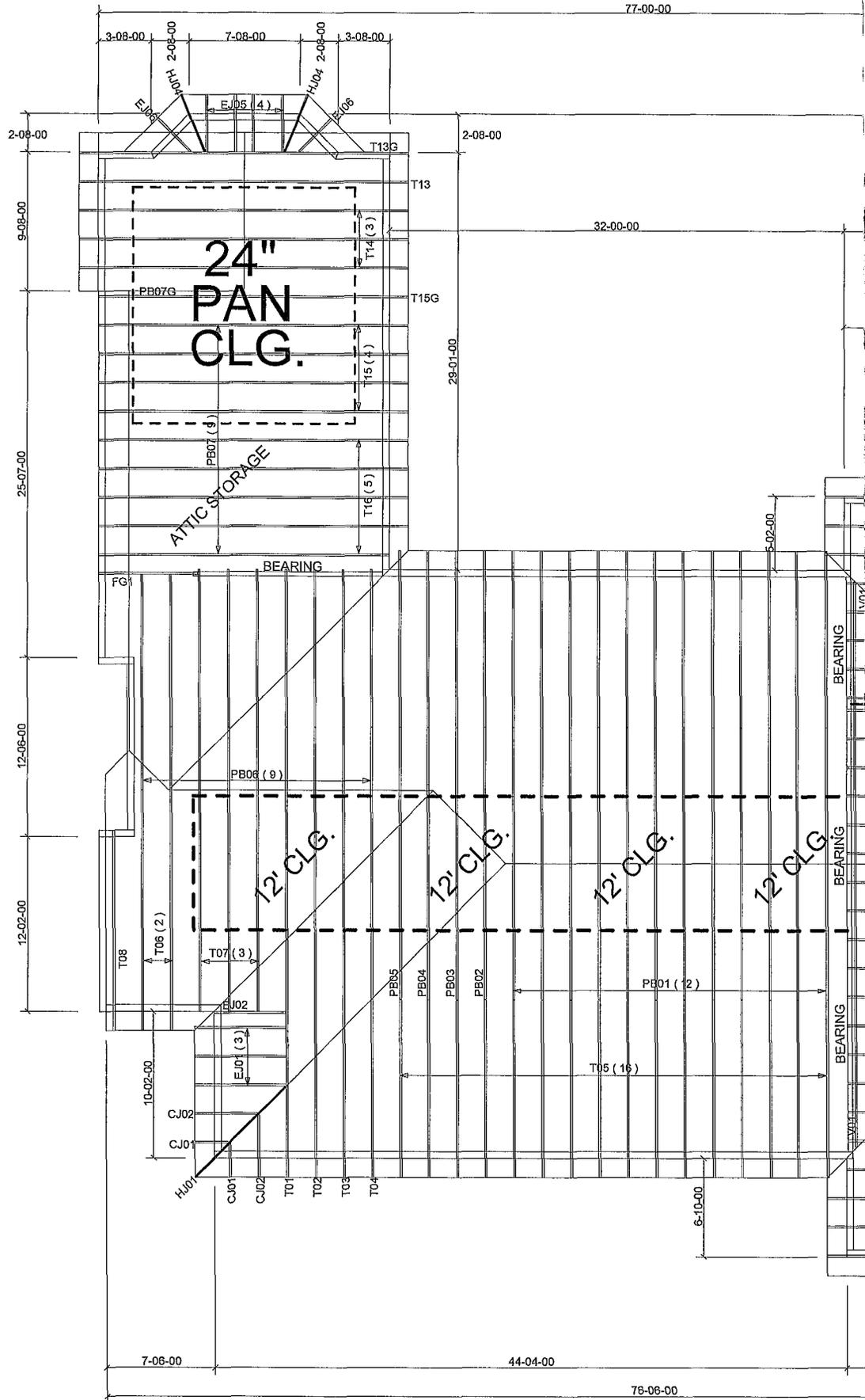
AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.



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NOTE . THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYED STUDS. TRUSSES WITHOUT INLAYED STUDS ARE NOT ADDRESSED HERE



MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERH