c/c# 11113

Columbia County Building Permit Application

For Office Use Only Application # 0710-40 Date Received 10/19 By JW Permit # 26469

Application Approved by - Zoning Official cfs Date 10/24/07 Plans Examiner OK JTH Date 10-23-07

Flood Zone X Development Permit — Zoning A-3 Land Use Plan Map Category A-3

Comments

NOC EH Deed or PA Site Plan N/A State Road Info Parent Parcel # Development Pe

Fax 386-462-4503

Name Authorized Person Signing Permit Ronald Clark Phone 352-538-6929

Address 15816 NW CR 1491, ALACHUA, FL 32615

Owners Name Clay and Isa Burdick Phone

911 Address 1625 SW Shilo Street, Ft. White, FL 32038

Contractors Name Ronald Clark Construction, Inc. Phone 352-538-6929

Address 15816 NW CR 1491, Alachua, FL 32615

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Builder/Contractor above

Mortgage Lenders Name & Address

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive En

Property ID Number 14-75-16 R04218-211 Estimated Cost of Construction $124,000.00

Subdivision Name Lee Perry Unrec. Lot 11 Block Unit Phase

Driving Directions 47 to Ft. White, turn left on 27, turn right on Shilo Street, 2 miles job is on right.

Type of Construction Single family dwelling Number of Existing Dwellings on Property 0

Total Acreage 5.02 Lot Size Do you need a - Culvert Permit or Culvert Waiver or Have an Existing

Actual Distance of Structure from Property Lines - Front 100' Side 180' (Rt) Side 100' (Lt) Rear 500'

Total Building Height 16' Number of Stories 1 Heated Floor Area 1440 TOTAL 2484 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

C462-139-58-094-0

Owner Builder or Authorized Person by Notarized Letter

Contractor Signature

Contractors License Number CRC1326560

Competency Card Number

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 19th day of October 2007.

Lisa Huchingson
MY COMMISSION # DD607758
EXPIRES: October 22, 2010
(407)398-0153 FloridaNotaryService.com

JW called Ronald 10.29.07 - LM. 10-23-07 Lisa Huchingson


04219-211
ZONE X
D
E

FORM 600A-2004R EnergyGauge® 4.5

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	burdick	Builder:	ron clark const.
Address:		Permitting Office:	COLUMBIA
City, State:	,	Permit Number:	26469
Owner:		Jurisdiction Number:	221000
Climate Zone:	North		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	1440 ft²	
7.	*Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)*		
	a. U-factor: (or Single or Double DEFAULT)	7a. Description (Dble Default) Area 181.0 ft²	
	b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 181.0 ft²	
8.	Floor types		
	a. Slab-On-Grade Edge Insulation	R=0.0, 200.0(p) ft	
	b. N/A		
	c. N/A		
9.	Wall types		
	a. Frame, Wood, Exterior	R=13.0, 1360.0 ft²	
	b. N/A		
	c. N/A		
	d. N/A		
	e. N/A		
10.	Ceiling types		
	a. Under Attic	R=30.0, 1440.0 ft²	
	b. Under Attic	R=19.0, 244.0 ft²	
	c. N/A		
11.	Ducts		
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 266.0 ft	
	b. N/A		

12.	Cooling systems		
	a. Central Unit	Cap: 29.2 kBtu/hr SEER: 13.00	
	b. N/A		
	c. N/A		
13.	Heating systems		
	a. Electric Heat Pump	Cap: 31.8 kBtu/hr HSPF: 8.30	
	b. N/A		
	c. N/A		
14.	Hot water systems		
	a. Electric Resistance	Cap: 40.0 gallons EF: 0.92	
	b. N/A		
	c. Conservation credits (*HR-Heat recovery, Solar DHP-Dedicated heat pump*)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, *MZ-H-Multizone heating*)		

Glass/Floor Area: 0.13 Total as-built points: 20362 Total base points: 21733 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: SUNCOAST INSULATORS 825 NW 253rd Terrace Newberry FL 32669 (352) 472-2615 Fax (352) 472-2633

DATE: 10/2/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Ronald Clark

DATE: 10-2-07

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2004R EnergyGauge® 4.5

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	40.0	0.92	3		1.00	2635.00	1.00	7905.0
				As-Built Total:							7905.0

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling *Points*	+ Heating *Points*	+ Hot Water *Points*	= Total *Points*	Cooling *Points*	+ Heating *Points*	+ Hot Water *Points*	= Total *Points*
5612	8216	7905	21733	5076	7381	7905	20362

PASS

FORM 600A-2004R EnergyGauge® 4.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Point
.18	1440.0	20.17	5229.0	1.Double, Clear	W	2.0	6.0	70.0	20.73	1.04	1512.0
				2.Double, Clear	N	2.0	6.0	39.0	24.58	1.00	963.0
				3.Double, Clear	E	2.0	6.0	51.0	18.79	1.06	1016.0
				4.Double, Clear	S	2.0	6.0	21.0	13.30	1.26	351.0
				As-Built Total:				**181.0**			**3842.0**
WALL TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM	=	Points
Adjacent	0.0	0.00	0.0	1. Frame, Wood, Exterior			13.0	1360.0	3.40		4624.0
Exterior	1360.0	3.70	5032.0								
Base Total:	**1360.0**		**5032.0**	**As-Built Total:**				**1360.0**			**4624.0**
DOOR TYPES	Area X	BWPM	= Points	Type				Area X	WPM	=	Points
Adjacent	0.0	0.00	0.0	1.Exterior Insulated				56.0	8.40		470.4
Exterior	56.0	12.30	688.8								
Base Total:	**56.0**		**688.8**	**As-Built Total:**				**56.0**			**470.4**
CEILING TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1440.0	2.05	2952.0	1. Under Attic			30.0	1440.0	2.05 X	1.00	2952.0
				2. Under Attic			19.0	244.0	2.70 X	1.00	658.8
Base Total:	**1440.0**		**2952.0**	**As-Built Total:**				**1684.0**			**3610.8**
FLOOR TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM	=	Points
Slab	200.0(p)	8.9	1780.0	1. Slab-On-Grade Edge Insulation			0.0	200.0(p	18.80		3760.0
Raised	0.0	0.00	0.0								
Base Total:			**1780.0**	**As-Built Total:**				**200.0**			**3760.0**
INFILTRATION	Area X	BWPM	= Points					Area X	WPM	=	Points
	1440.0	-0.59	-849.6					1440.0	-0.59		-849.6

FORM 600A-2004R

EnergyGauge® 4.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT						
Winter Base Points:			**14831.2**	**Winter As-Built Points:**						**15457.6**
Total Winter Points	X	System Multiplier	= Heating Points	Total Component (System - Points)	X	Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
				(sys 1: Electric Heat Pump 31800 btuh ,EFF(8.3) Ducts:Unc(S),Unc(R),Int(AH),R6.0						
				15457.6		1.000	(1.069 x 1.189 x 0.93) 0.411		1.000	7380.6
14831.2		***0.5540***	***8216.5***	***15457.6***		***1.00***	***1.162***	***0.411***	***1.000***	***7380.6***

FORM 600A-2004R

EnergyGauge® 4.5

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1440.0	18.69	4818.0	1.Double, Clear	W	2.0	6.0	70.0	38.52	0.85	2290.0	
				2.Double, Clear	N	2.0	6.0	39.0	19.20	0.90	673.0	
				3.Double, Clear	E	2.0	6.0	51.0	42.06	0.85	1819.0	
				4.Double, Clear	S	2.0	6.0	21.0	35.87	0.78	584.0	
				As-Built Total:				181.0			5366.0	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	1. Frame, Wood, Exterior			13.0	1360.0	1.50		2040.0	
Exterior	1360.0	1.70	2312.0									
Base Total:	1360.0		2312.0	As-Built Total:				1360.0			2040.0	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	1.Exterior Insulated				56.0	4.10		229.6	
Exterior	56.0	6.10	341.6									
Base Total:	56.0		341.6	As-Built Total:				56.0			229.6	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1440.0	1.73	2491.2	1. Under Attic			30.0	1440.0	1.73 X 1.00		2491.2	
				2. Under Attic			19.0	244.0	2.34 X 1.00		571.0	
Base Total:	1440.0		2491.2	As-Built Total:				1684.0			3062.2	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	200.0(p)	-37.0	-7400.0	1. Slab-On-Grade Edge Insulation			0.0	200.0(p	-41.20		-8240.0	
Raised	0.0	0.00	0.0									
Base Total:			-7400.0	As-Built Total:				200.0			-8240.0	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1440.0	10.21	14702.4					1440.0	10.21		14702.4	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 17266.2			**Summer As-Built Points:**					**17160.2**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 28200btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)					
17266.2	0.3250	5611.5	17160 **17160.2**	1.00 **1.00**	(1.09 x 1.147 x 0.91) **1.138**	0.260 **0.260**	1.000 **1.000**	5076.1 **5076.1**

IN... ...ld Clark

Clayton Burdick
cel# 305-522-3902

WARRANTY DEED

(STATUTORY FORM - SECTION 689.02, F.S.)
This document prepared by and to be returned to:
Kyle E. Petteway
Grunder & Petteway, P. A.
23349 NW CR 236, Suite 10
High Springs, Florida, 32643

Tax Parcel Number: R04218-211

Inst:2006030235 Date:12/27/2006 Time:14:54
Doc Stamp-Deed : 490.00
DC,P.DeWitt Cason,Columbia County B:1105 P:25

THIS INDENTURE made December 20, 2006,

BETWEEN Judy Ross, an unremarried widow, whose post office address is 1625 SW Shiloh Street, Ft. White, Florida, , herein called Grantor, and

Clayton Burdick, Jr. and Isa Burdick, husband and wife, whose post office address is 144 Harbor Lane, Tavernier, Florida, 33070, herein called Grantee,

Witnesseth that said grantor, for and in consideration of the sum of TEN AND NO/100 ($10.00) Dollars, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in the county(ies) of Columbia state of Florida, to wit:

Lot 11, Lee Perry, an unrecorded subdivision:

Commence at the NE corner of SW 1/4 of NW 1/4, Section 14, Township 7 South, Range 16 East, Columbia County, Florida, thence South 89 deg. 00 min. 27 sec. West 517.37 feet, thence South 00 deg. 50 min. 39 sec. East 807.37 feet, to the point of beginning, thence continue South 00 deg. 50 min. 39 sec East 458.16 feet to the North Line of Shiloh Road, thence North 89 deg. 43 min. 31 sec. East along the North line of Shiloh Road 477.29 feet, thence North 00 deg. 50 min. 22 sec. West 458.16 feet, thence South 89 deg. 43 min. 31 sec. West 477.32 feet to the point of beginning. SRW

AND SAID GRANTOR does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

Grantor and grantee are used for singular or plural, as context requires.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Witness: Print Name Kyle E. Petteway — Judy Ross

Witness: Print Name [illegible]

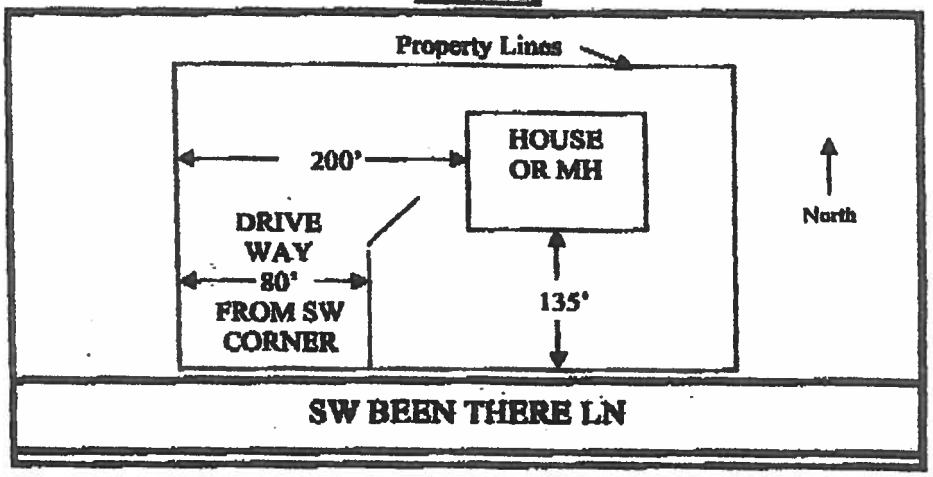
State of Florida
County of Alachua

The foregoing instrument was acknowledged before me this 20th day of December, 2006 by Judy Ross who
( ) is personally known to me
(✓) who has produced a valid Florida driver's license as identification
( ) who produced ____________ as identification

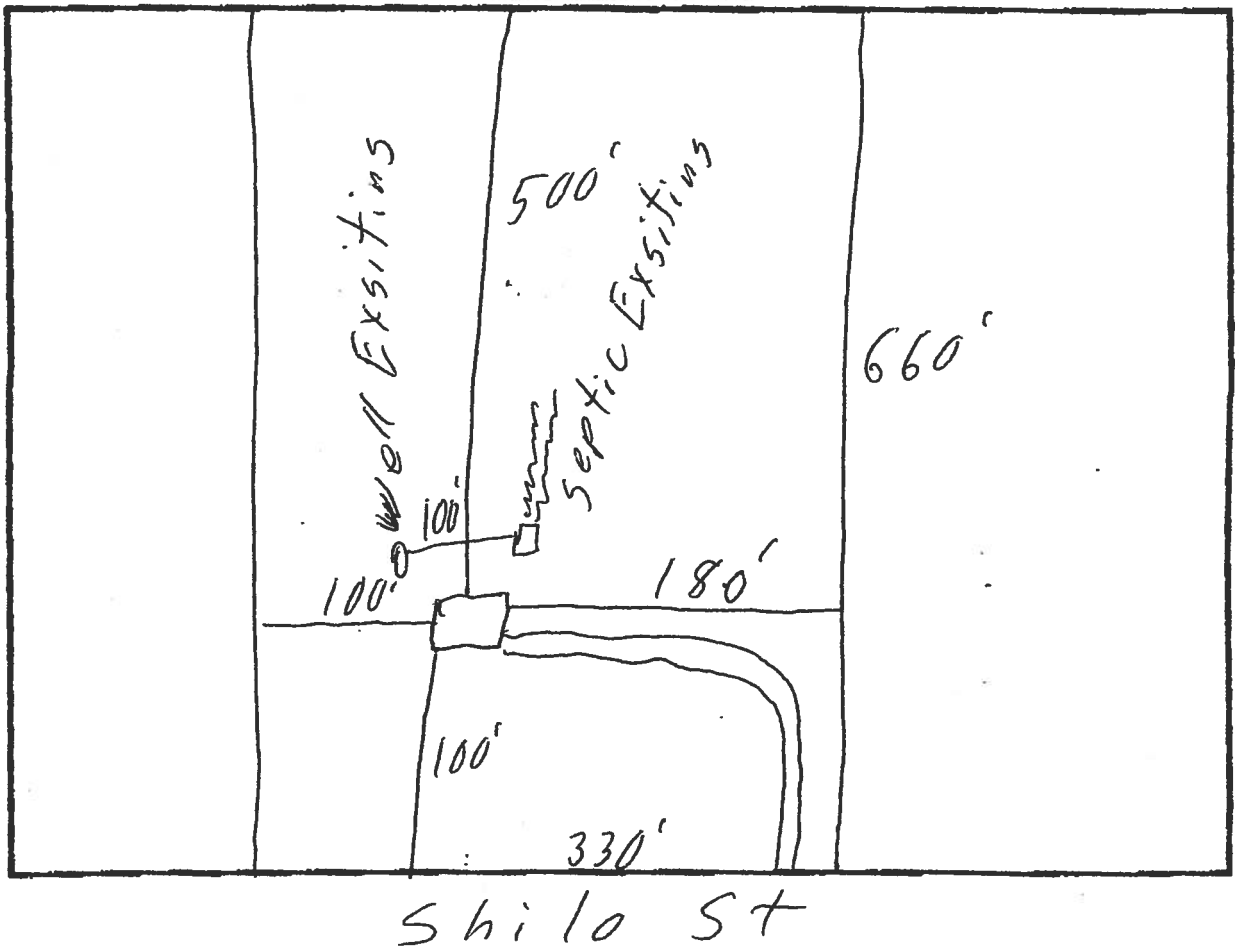
Notary Public at Large, State of Florida
(SEAL)
8537

1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



SITE PLAN BOX:



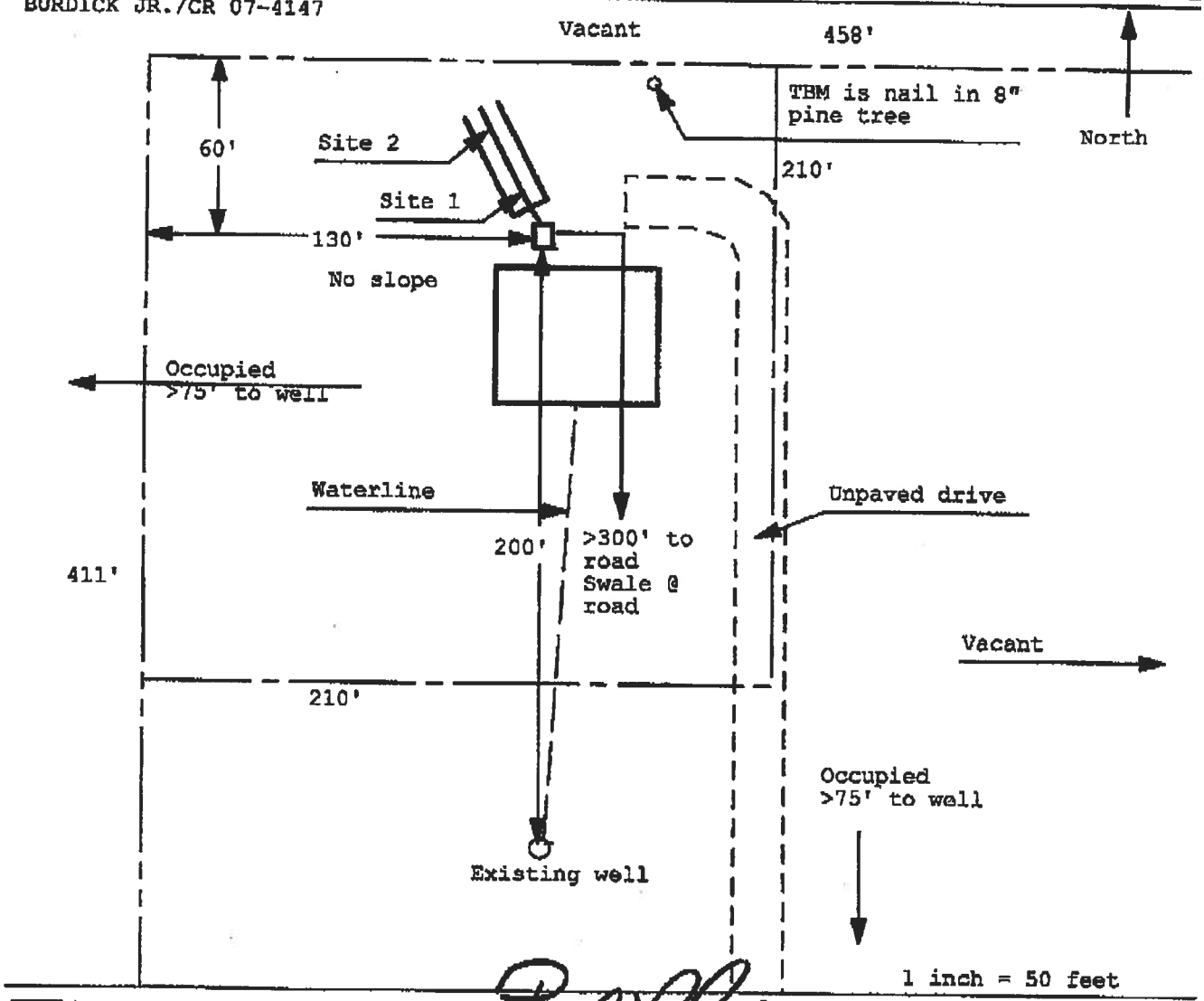
~~[illegible]~~ Clark 0710-40

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 07852

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

BURDICK JR./CR 07-4147



Site Plan Submitted By Paul Lloyd Date 10/31/07

Plan Approved ✓ Not Approved ____ Date 11-5-07

By [illegible] Columbia CPHU

Notes: _____

1125 SW Shiloh St.

Clayton Burdick

Job: Burdick
Date: 10-1-07
By: Michael Chartier

Load Short Form
Entire House
Bounds Heating & Air

Project Information

For: Ronald Clark Construction

Design Information

	Htg	Clg
Outside db (°F)	*31*	*93*
Inside db (°F)	68	75
Design TD (°F)	37	18
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	50

Infiltration

Method	Simplified
Construction quality	Tight
Fireplaces	0

HEATING EQUIPMENT

Make	Carrier
Trade	Base 13 Puron HP
Model	25HBA330A30

Efficiency	8.3 HSPF	
Heating input	31800	Btuh @ 47°F
Heating output	30	°F
Temperature rise	973	cfm
Actual air flow	0.041	cfm/Btuh
Air flow factor	1.00	in H2O
Static pressure		
Space thermostat		

COOLING EQUIPMENT

Make	Carrier
Trade	Base 13 Puron HP
Cond	25HBA330A30
Coil	FY4ANF030

Efficiency	13 SEER	
Sensible cooling	20440	Btuh
Latent cooling	8760	Btuh
Total cooling	29200	Btuh
Actual air flow	973	cfm
Air flow factor	0.047	cfm/Btuh
Static pressure	1.00	in H2O
Load sensible heat ratio	0.85	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
laundry	88	2121	1881	88	88
M.Bath	66	654	217	27	10
Kitchen	132	1564	2715	65	127
Dining	144	3673	2449	152	114
Bed room 2	154	3346	1972	138	92
Bath	72	1083	459	45	21
Bed room 3	154	3346	2181	138	102
Hall	40	48	70	2	3
Great room	342	3196	4633	132	216
Master suite	248	4546	4278	188	200

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Entire House	1440	23578	20856	973	973
Other equip loads		2623	1276		
Equip. @ 0.98 RSM			21689		
Latent cooling			3776		
TOTALS	1440	26201	25466	973	973

Job: Burdick
Date: 10-1-07
By: Michael Chartier

Duct System Summary
Entire House
Bounds Heating & Air

Project Information

For: Ronald Clark Construction

	Heating	Cooling
External static pressure	1.00 in H2O	***1.00*** in H2O
Pressure losses	0.30 in H2O	0.30 in H2O
Available static pressure	0.70 in H2O	0.70 in H2O
Supply / return available pressure	0.50 / 0.20 in H2O	0.50 / 0.20 in H2O
Lowest friction rate	***0.100*** in/100ft	***0.100*** in/100ft
Actual air flow	973 cfm	973 cfm
Total effective length (TEL)	335 ft	

Supply Branch Detail Table

Name	Design (Btuh)		Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
laundry	c	1881	88	88	0.100	6	16x2	VIFx	5.0	145.0	st2
M.Bath	h	654	27	10	0.100	4	16x1	VIFx	8.0	145.0	st2
Kitchen	c	2715	65	127	0.100	7	16x3	VIFx	11.0	170.0	st1
Dining	h	3673	152	114	0.100	8	16x4	VIFx	23.0	155.0	st1
Bed room 2	h	3346	138	92	0.100	8	16x4	VIFx	34.0	195.0	st1
Bath	h	1083	45	21	0.100	5	16x1	VIFx	40.0	195.0	st1
Bed room 3	h	3346	138	102	0.100	8	16x4	VIFx	46.0	195.0	st1
Hall	c	70	2	3	0.100	4	16x1	VIFx	34.0	205.0	st1
Great room-A	c	2317	66	108	0.100	7	16x3	VIFx	19.0	160.0	st1
Great room	c	2317	66	108	0.100	7	16x3	VIFx	27.0	155.0	st1
Master suite-A	c	2139	94	100	0.100	7	16x3	VIFx	12.0	180.0	st1
Master suite	c	2139	94	100	0.100	7	16x3	VIFx	15.0	165.0	st2

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	765	775	0.100	775	13	16 x 9	RectFbg	
st2	Peak AVF	208	198	0.100	469	8	16 x 4	RectFbg	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	973	973	94.0	0.050	398	20	16x 22		VlFx	rt1

Return Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
rt1	Peak AVF	973	973	0.050	458	19	18 x 17	RectFbg	

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Entergy	Wood-edge steel door in wood frame	00-0720.05
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER	Entergy	Double-wood edge steel door in wood fr.	01-0314.29
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	Capitol	650 FIN Alum. Single hung	42963.03-122-47
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	EIK	Prestige 30 HD	01-0919.11
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS	Simpson	H16, H2.5, H10	FL1423+FL4
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

Ronald Clark
APPLICANT SIGNATURE

10-12-07
DATE

R-1305 01-04

Roof Ventilation Requirements

Attic area = 1440 sq.ft.

Net free air flow area required is

$= \frac{1440}{300} =$ 4.8 sq.ft.

Half of 4.8 sq.ft. should be provided by off ridge or on ridge vents.

$\frac{2.4}{0.72} =$ 3.3 vents required

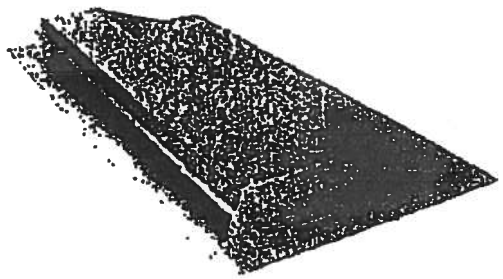
4'x 1' off ridge vent gives 104 sq.in. per off ridge vent

Balance of 1.5 sq.ft. to be provided by soffit vents

OFF RIDGE ROOF VENT

FL approval # FL 4270.1

Must be provided with a balanced air intake from eave only or two way venting will occur if not properly balanced.



- Screened external wind baffle provides superior air flow with maximum weather protection.
- Utilized with gable and / or undereave soffit vents, this system provides an effective natural air flow.
- Self flashing flanges allow for easy installation.
- Install Off Ridge on roof slope for unobstructed peak line.
- Available in other colors and guages.

MODEL #	DESCRIPTION	SIZE	TOTAL SQ. IN.	NO. PER CARTON	WT. PER CARTON
VEPGA4R01	26GA GALV. - 4'	4'	104	2	27.6 lbs.
VEPGA6R01	26GA GALV. - 6'	6'	156	2	34 lbs.
VEPGA8R01	26GA GALV. - 8'	8'	207	2	43.5 lbs.
VEPGA10R01	26GA GALV. - 10'	10'	259	2	54.5 lbs.
VEPWG4R01	26GA WHITE GALV.	4'	104	2	27.6 lbs.
VEPBG4R01	26GA BROWN GALV.	4'	104	2	27.5 lbs.
VEPBLK4R01	26GA BLACK GALV.	4'	104	2	27.5 lbs.
VEPWG6R01	26GA WHITE GALV.	6'	156	2	34 lbs.
VEPBG6R01	26GA BROWN GALV.	6'	156	2	34 lbs.
VEPBLK6R01	26GA BLACK GALV.	6'	156	2	34 lbs.
VEPWG8R01	26GA WHITE GALV.	8'	207	2	43.5 lbs.
VEPBG8R01	26GA BROWN GALV.	8'	207	2	43.5 lbs.
VEPBLK8R01	26GA BLACK GALV.	8'	207	2	43.5 lbs.
VEPWA4R01	.019 NOM WHITE ALUM.	4'	104	2	9 lbs.
VEPBA4R01	.019 NOM BROWN ALUM.	4'	104	2	9 lbs.
VEPALA4R01	.019 NOM MILL FINISH ALUM.	4'	104	2	9 lbs.
VEPGATV4	*26GA GALV. TILE	4'	144	2	29 lbs.
VEPGATV6	*26GA GALV. TILE	6'	196	2	40.5 lbs.
VEPGATV8	*26GA GALV. TILE	8'	248	2	52 lbs.
VEPGATV10	*26GA GALV. TILE	10'	300	2	63 lbs.

*Non Stocking - Special Order Only

Schafer Engineering LLC

14952 Main St. Alachua FL 32615

Prepared for:

RONALD CLARK CONSTRUCTION
THE BURDICK RESIDENCE

By:

Schafer Engineering, LLC

386-462-1340 / 352-375-6329

SCHAFER ENGINEERING, LLC

September 26, 2007
SUMMARY: Wind Load Analysis for Ronald Clark Construction \ The Burdick Residence
Wind Speed: 110 M.P.H. \ No Copies Permitted \ Designed For One Use Only
Florida Building Code \ Latest Edition

Foundation:
20" wide x 10" deep stemwall footing with (2) #5 rebar continuous minimum. CMU walls must have #5 dowels at 72" o.c. maximum with a standard 90 degree ACI hook in footing and a 4" slab on grade. Monolithic slab to be 12" wide x 20" deep minimum with (2) #5 rebar continuous with 12" minimum coverage on face of foundation. It is assumed that ideal soil conditions and pad preparation are provided.

Walls:
8" CMU block with vertical #5 reinforcing bar in grout filled cell at 72" o.c. maximum spacing. Wall heights are 8' maximum. Provide an 8" x 8" bond beam with 1-#5 rebar horizontal continuous at the top course. Install pre-cast, pre-engineered lintels or pre-engineered steel lintels spanning over all openings. One #5 rebar each corner. One #5 rebar each side of door and window openings. Two #5 rebar in openings wider than 12'-0". One #5 rebar where girders or girder trusses bear on masonry wall.

Shearwalls:
Transverse: 39'-0" Allowable pounds per foot unit shear on shearwalls: 314 plf
Longitudinal: 45'-0" Unit shear transferred from diaphragm: Trs 72 plf Long: 84 plf

Trusses:
Pre-engineered Pre-fabricate trusses with the bracing system designed by the manufacturer. Trusses must be installed and anchored according to the truss engineering requirements. Trusses must bear on all exterior walls and porch headers.

Roof Sheathing:
7/16" osb minimum attached to the top chords of the trusses with 8d/113 gauge ring shank nails spaced at 3" o.c. edges and 6" interior. Install ceiling diaphragm on open porches using the same grade material, nail size, and nail spacing as the roof sheathing.

Columns:
Install 4" x 4" x 8' syp #2 pt columns @ 120" o.c. maximum spacing. Simpson CB44 \ CC 44 or equal.

9-27-07

Bruce Schafer P. E. #48984
7104 N. W. 42nd Lane \ Gainesville, Florida 32606

ASCE 7-02

9/26/07

Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	20.67	ft
Mean Roof Height (Ht)	14.75	ft
Width Perp. to Wind (B)	46.00	ft
Width Parallel to Wind (L)	54.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.32	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.9033	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8679	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	0.96	
Nb	4.6*NatFreq*B/Vzm	2.98	
Nd	15.4*NatFreq*Depth/Vzm	11.73	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.5795	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.2790	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0816	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.7121	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.08	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.87	Gust Factor (G)	0.87

ASCE 7-02

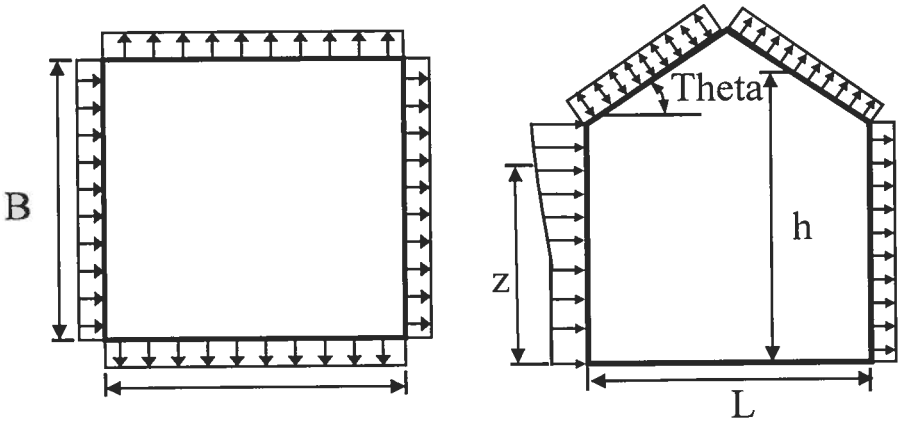
Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2)	
					Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
20.67	0.70	1.00	1.00	21.70	11.86	18.27
20	0.70	1.00	1.00	21.70	11.86	18.27
15	0.70	1.00	1.00	21.70	11.86	18.27

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 46 ft wall)	-0.47	-10.39	-3.98
Leeward Walls (Wind Dir Parallel to 54 ft wall)	-0.50	-10.93	-4.52
Side Walls	-0.70	-14.02	-7.61
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.21	-6.39	0.02
Windward - Max Positive	0.29	1.29	7.70
Leeward Normal to Ridge	-0.60	-12.48	-6.07
Overhang Top	-0.21	-3.19	-3.19
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 7.375 ft	-0.90	-17.11	-10.70
Dist from Windward Edge: 7.375 ft to 14.75 ft	-0.90	-17.11	-10.70

ASCE 7-02

Wind Load Design per ASCE 7-02

Dist from Windward Edge: 14.75 ft to 29.5 ft	-0.50	-10.93	-4.52
Dist from Windward Edge: > 29.5 ft	-0.30	-7.84	-1.43

* Horizontal distance from windward edge

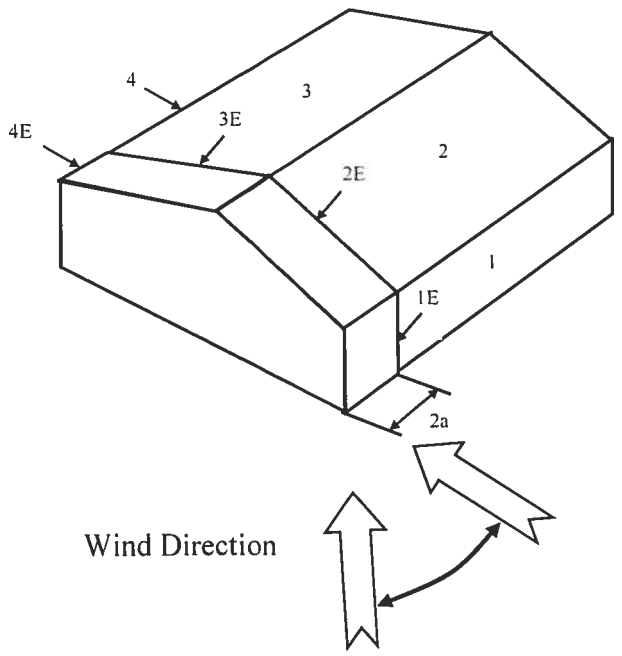
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-02

Wind Load Design per ASCE 7-02

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

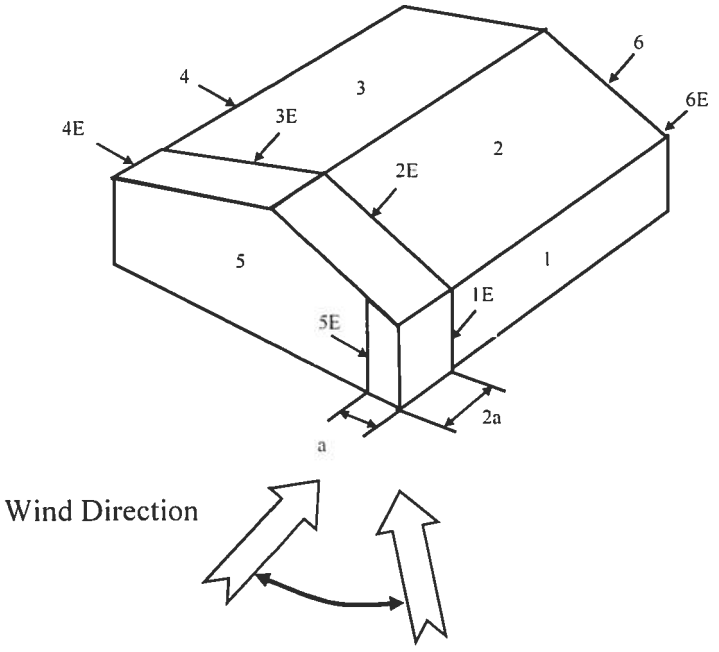
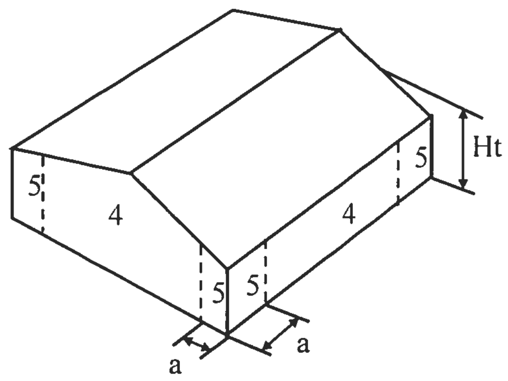


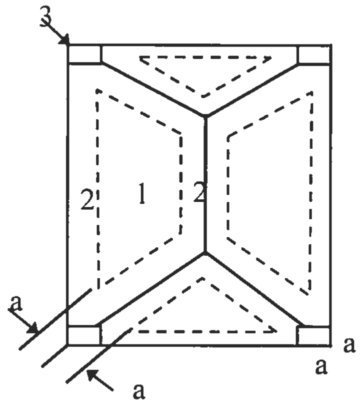
Figure 6-5 - External Pressure Coefficients, GCp

ASCE 7-02

Wind Load Design per ASCE 7-02

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Hipped Roof
10 < Theta <= 30

a = 4.6 ==> 4.60 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp Max	GCp Min	Wind Press (lb/ft^2) Max	Wind Press (lb/ft^2) Min
	16	7	112.00	5	0.81	-1.03	17.71	-21.53
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi

ASCE 7-02

Wind Load Design per ASCE 7-02

	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

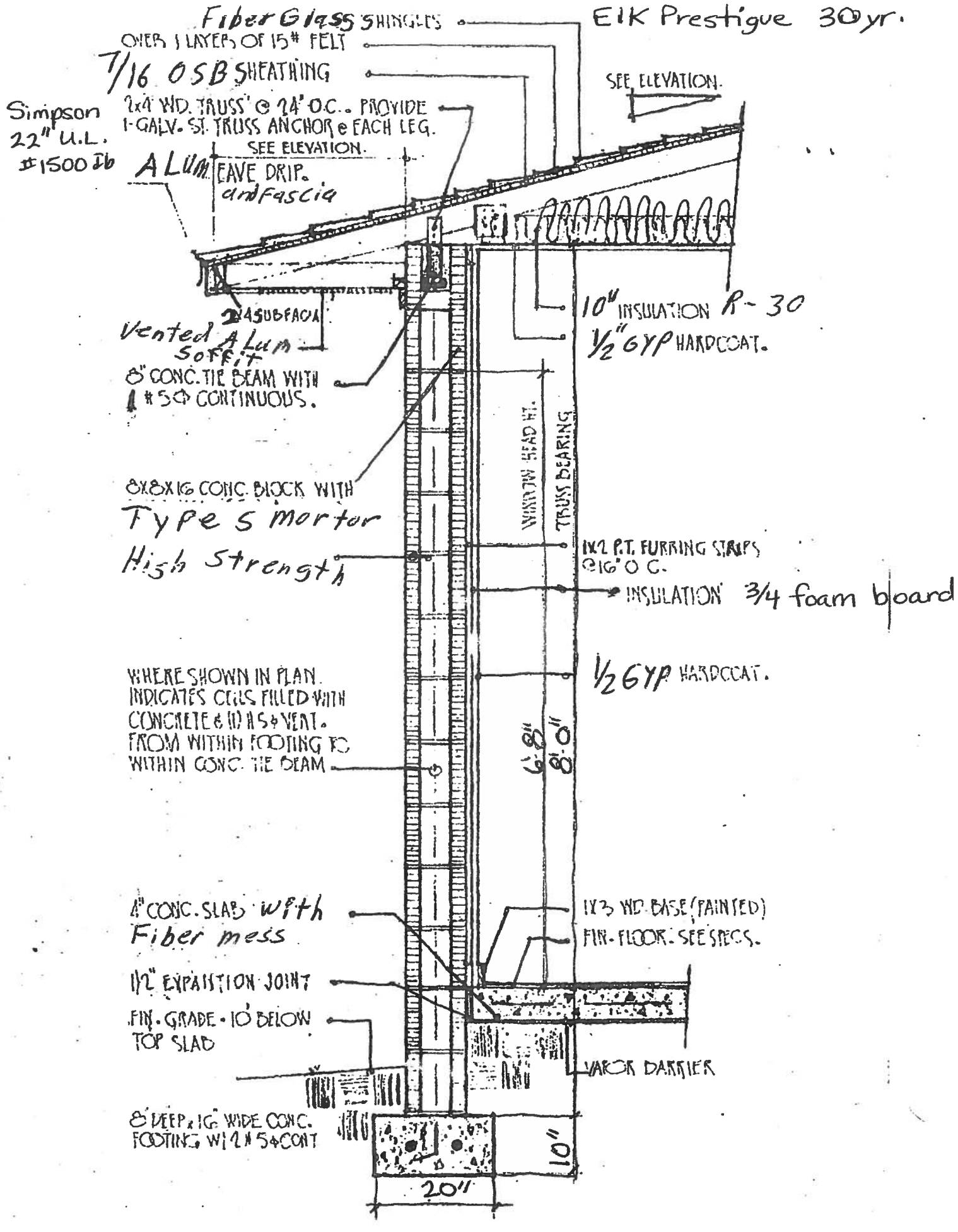
Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

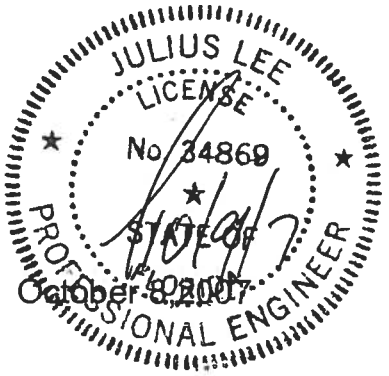
Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.27	-18.66	-10.93
	P (-GCpi) -psf	5.14	-12.25	-4.52
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.29	-18.66	-10.93
	P (-GCpi) -psf	3.29	-18.66	-10.93

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	54.00	ft
B	Roof dimension parallel to wind direction	46.00	ft
L/B	Ratio of L to B	1.174	
Theta	Slope of Roof	26.6	Deg
Cf	Force Coefficient	1.17	
X	Distance to center of pressure from windward edge	0.41	ft



SECTION: 8" Block scale 3/4" = 1'



Project Information for: L256400

Address:	1625 Southwest Shilo Street
...	Gainesville, FL
County:	Alachua
Truss Count:	15
Design Program:	MiTek 20/20 6.3
Building Code:	FBC2004/TPI2002

Truss Design Load Information:

Gravity:	**Wind:**	
Roof (psf): 42.0	Wind Standard: ASCE 7-02	Wind Exposure: B
Floor (psf): N/A	Wind Speed (mph): 110	

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
Ronald W. Clark Florida License No. CRC1326560
Address: 15816 Northwest County Road 1491 Alachua, Florida 32615

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1898785	CJ1	10/8/07
2	J1898786	CJ3	10/8/07
3	J1898787	CJ5	10/8/07
4	J1898788	EJ7	10/8/07
5	J1898789	HJ9	10/8/07
6	J1898790	PB01	10/8/07
7	J1898791	T01	10/8/07
8	J1898792	T02	10/8/07
9	J1898793	T03	10/8/07
10	J1898794	T04	10/8/07
11	J1898795	T05	10/8/07
12	J1898796	T06	10/8/07
13	J1898797	T07	10/8/07
14	J1898798	T08	10/8/07
15	J0	EJ7_ALT	10/8/07

Builders FirstSource

Project Information for: L256400

Address: 1625 Southwest Shilo Street
Gainesville, FL
County: Alachua
Truss Count: 15
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

October 8,2007

Truss Design Load Information:

Gravity:	**Wind:**	
Roof (psf): 42.0	Wind Standard: ASCE 7-02	Wind Exposure: B
Floor (psf): N/A	Wind Speed (mph): 110	

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
Ronald W. Clark Florida License No. CRC1326560
Address: 15816 Northwest County Road 1491 Alachua, Florida 32615

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1898785	CJ1	10/8/07
2	J1898786	CJ3	10/8/07
3	J1898787	CJ5	10/8/07
4	J1898788	EJ7	10/8/07
5	J1898789	HJ9	10/8/07
6	J1898790	PB01	10/8/07
7	J1898791	T01	10/8/07
8	J1898792	T02	10/8/07
9	J1898793	T03	10/8/07
10	J1898794	T04	10/8/07
11	J1898795	T05	10/8/07
12	J1898796	T06	10/8/07
13	J1898797	T07	10/8/07
14	J1898798	T08	10/8/07
15	J0	EJ7_ALT	10/8/07

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898785
L256400	CJ1	JACK	8	1	Job Reference (optional)

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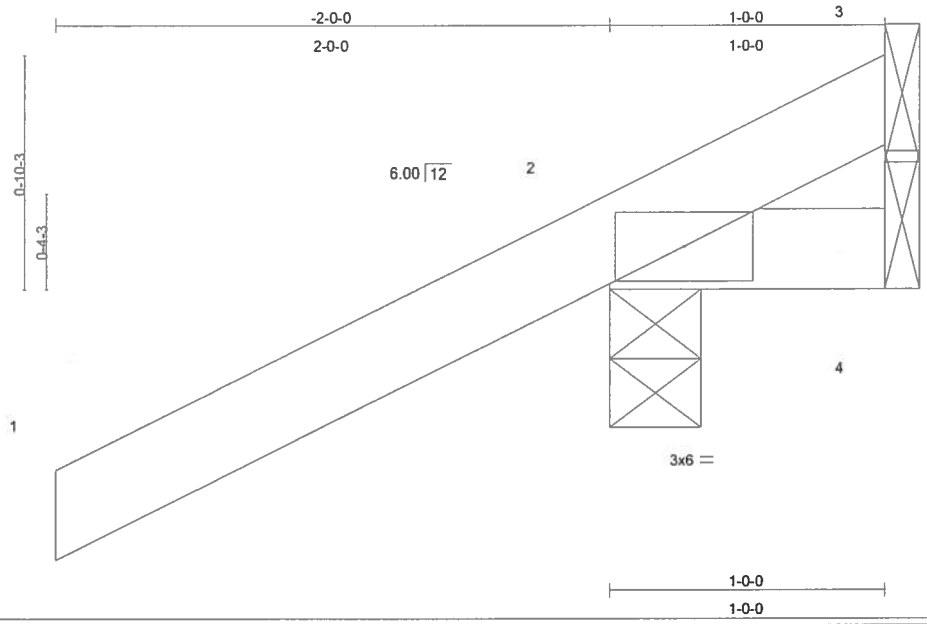

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=257/0-4-0, 4=5/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=-287(load case 6), 4=-9(load case 4), 3=-91(load case 1)
Max Grav 2=257(load case 1), 4=14(load case 2), 3=128(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/76
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3.

Continued on page 2

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October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898785
L256400	CJ1	JACK	8	1	Job Reference (optional)

LOAD CASE(S) Standard

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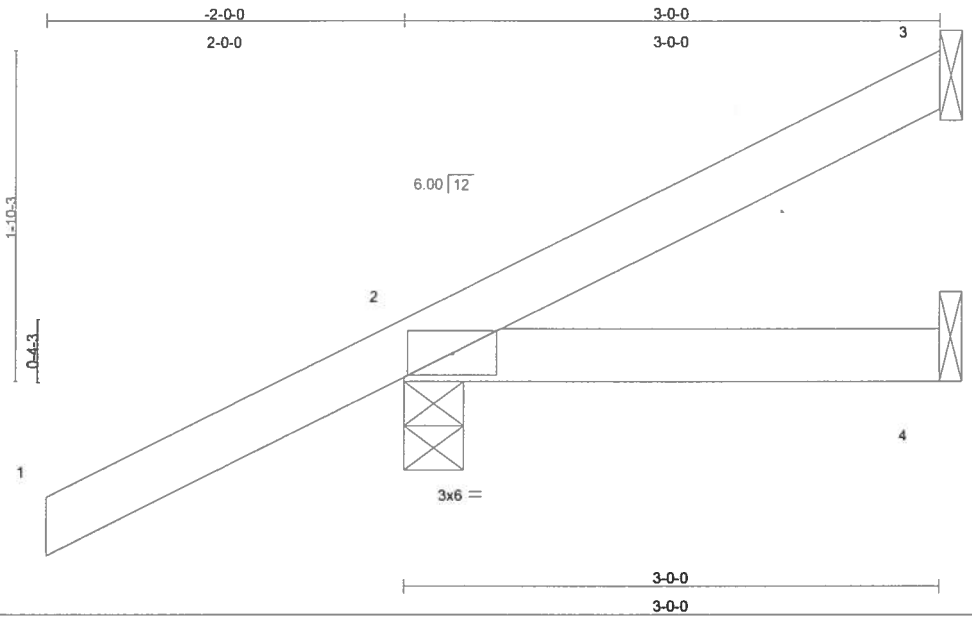
October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898786
L256400	CJ3	JACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Oct 08 13:08:48 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=251/0-4-0, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift 3=-27(load case 7), 2=-240(load case 6), 4=-26(load case 4)
Max Grav 3=29(load case 1), 2=251(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4.

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898786
L256400	CJ3	JACK	8	1	Job Reference (optional)

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898787
L256400	CJ5	JACK	8	1	Job Reference (optional)

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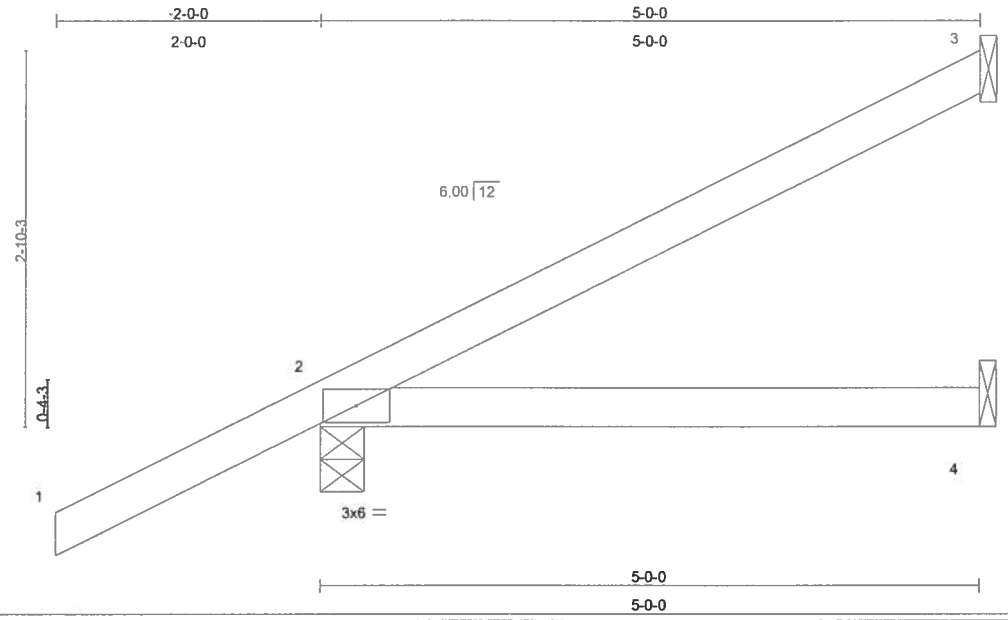

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.09	2-4	>671	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=296/0-4-0, 4=24/Mechanical
Max Horz 2=178(load case 6)
Max Uplift 3=-86(load case 6), 2=-261(load case 6), 4=-46(load case 4)
Max Grav 3=102(load case 1), 2=296(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4.

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898787
L256400	CJ5	JACK	8	1	Job Reference (optional)

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK
L256400	EJ7	MONO TRUSS	34	1	J1898788
					Job Reference (optional)

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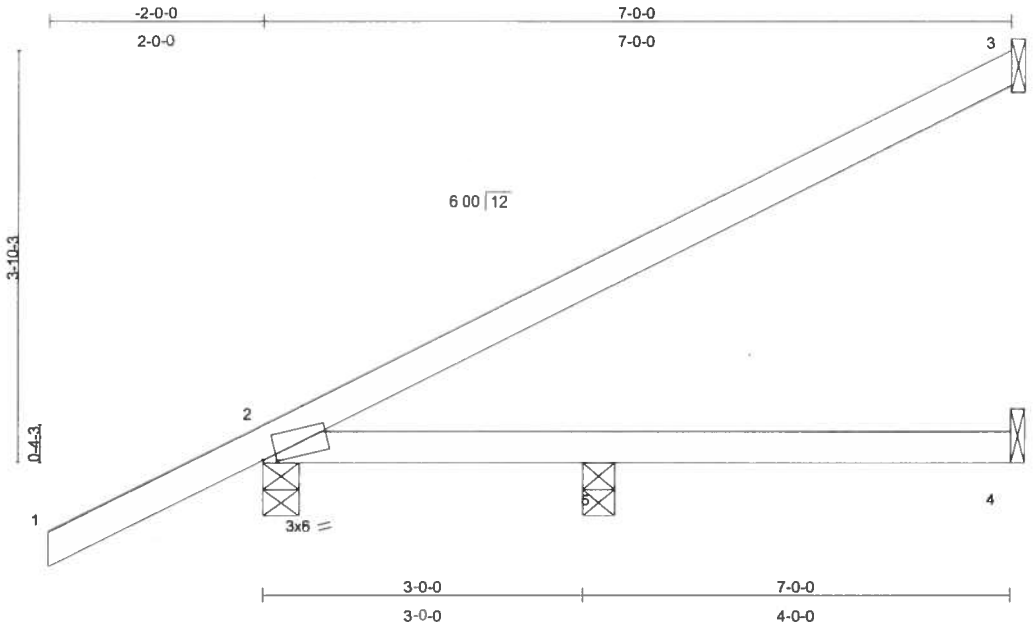

Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.02	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	-0.01	2-5	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=144/Mechanical, 2=293/0-4-0, 4=7/Mechanical, 5=106/0-3-8
Max Horz 2=161(load case 6)
Max Uplift 3=-85(load case 6), 2=-177(load case 6), 5=-49(load case 5)
Max Grav 3=144(load case 1), 2=293(load case 1), 4=43(load case 2), 5=143(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-118/50
BOT CHORD 2-5=-0/0, 4-5=-0/0

JOINT STRESS INDEX
2 = 0.90

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 3, 177 lb uplift at joint 2 and 49 lb uplift at joint 5.

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LOAD CASE(S) Standard

Builders FirstSource

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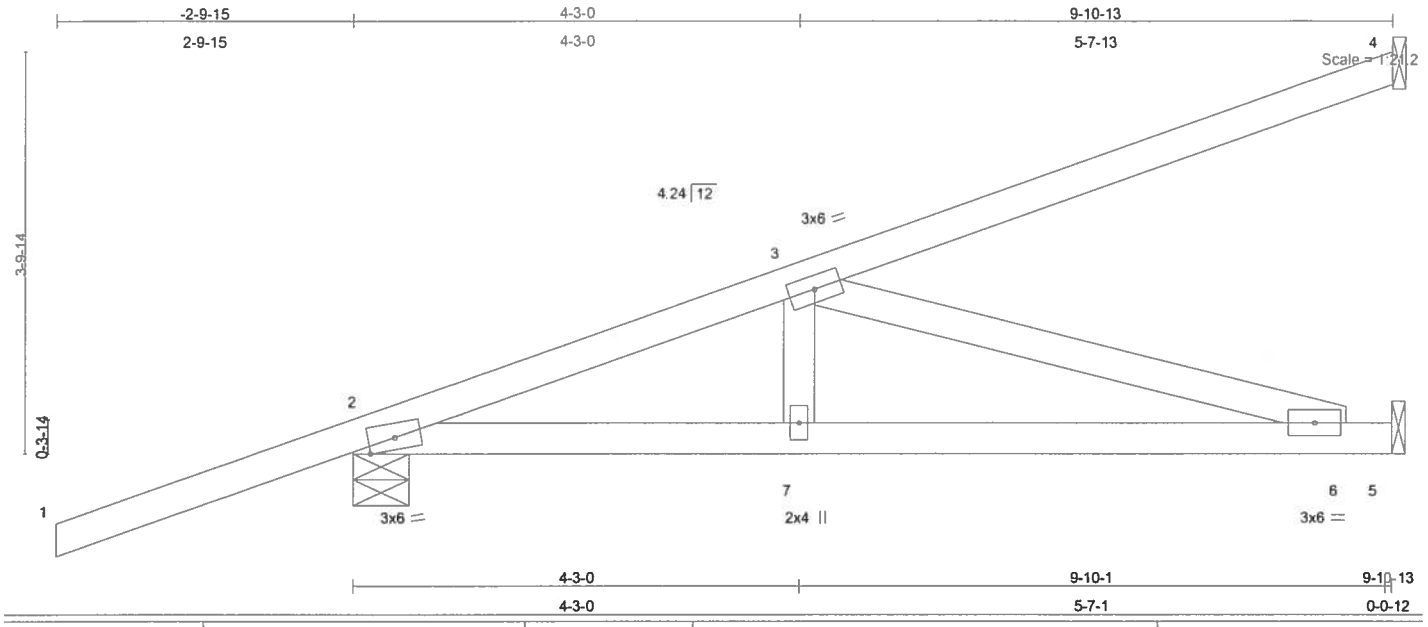
October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898789
L256400	HJ9	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Oct 08 13:08:50 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.10	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.12	6-7	>986	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-13 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=458/0-6-6, 5=217/Mechanical
Max Horz 2=270(load case 3)
Max Uplift 4=-233(load case 3), 2=-404(load case 3), 5=-180(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-642/358, 3-4=-105/65
BOT CHORD 2-7=-530/593, 6-7=-530/593, 5-6=0/0
WEBS 3-7=-94/189, 3-6=-618/553

JOINT STRESS INDEX
2 = 0.78, 3 = 0.22, 6 = 0.17 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 404 lb uplift at joint 2 and 180 lb uplift at joint 5.

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October 8,2007

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Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898789
L256400	HJ9	MONO TRUSS	4	1	Job Reference (optional)

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

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Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK
L256400	PB01	VALLEY 3x6 =	5	1	J1898790 Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Oct 08 13:08:51 2007 Page 1

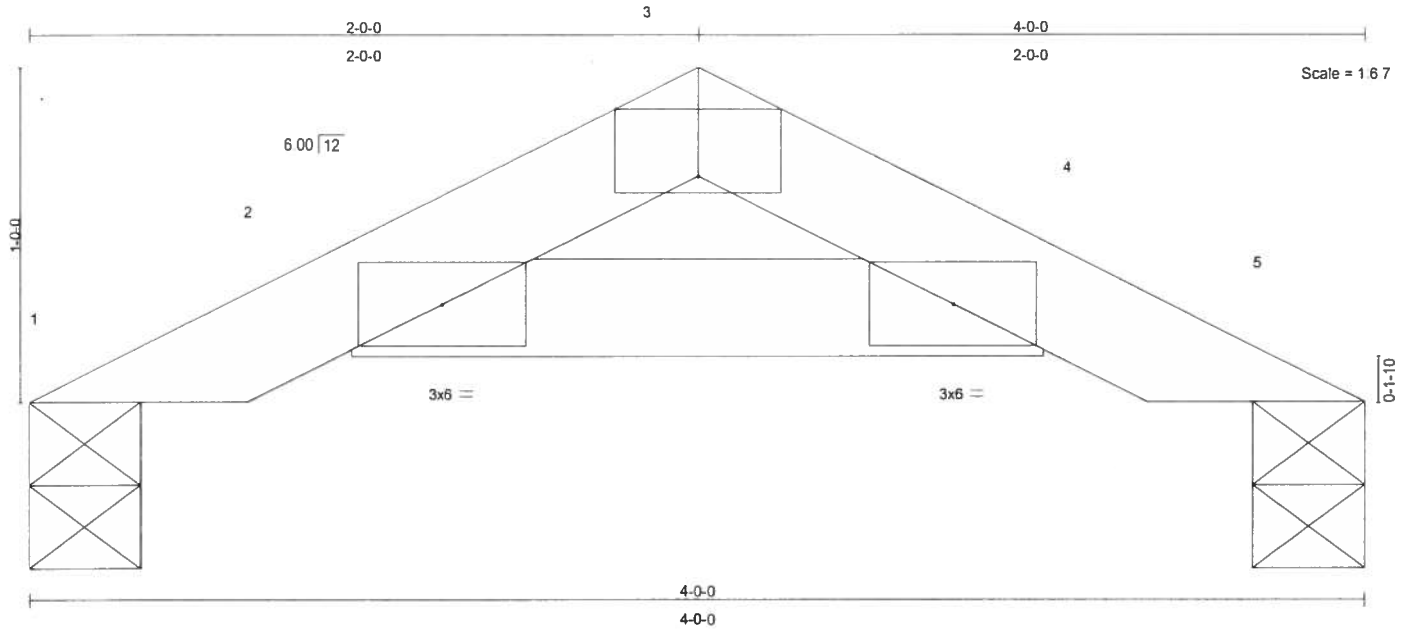

Plate Offsets (X,Y): [3:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 10 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=118/0-4-0, 5=118/0-4-0
Max Horz 1=-12(load case 4)
Max Uplift 1=-25(load case 6), 5=-25(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-47/46, 2-3=-192/161, 3-4=-192/161, 4-5=-47/46
BOT CHORD 2-4=-119/187

JOINT STRESS INDEX
2 = 0.16, 3 = 0.09 and 4 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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October 8,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898790
L256400	PB01	VALLEY	5	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1 and 25 lb uplift at joint 5.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898791
L256400	T01	HIP	2	1	Job Reference (optional)

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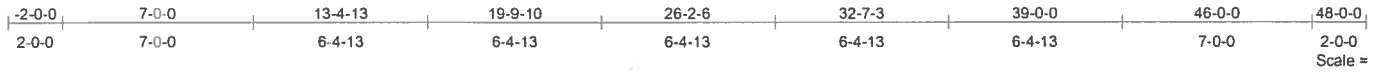

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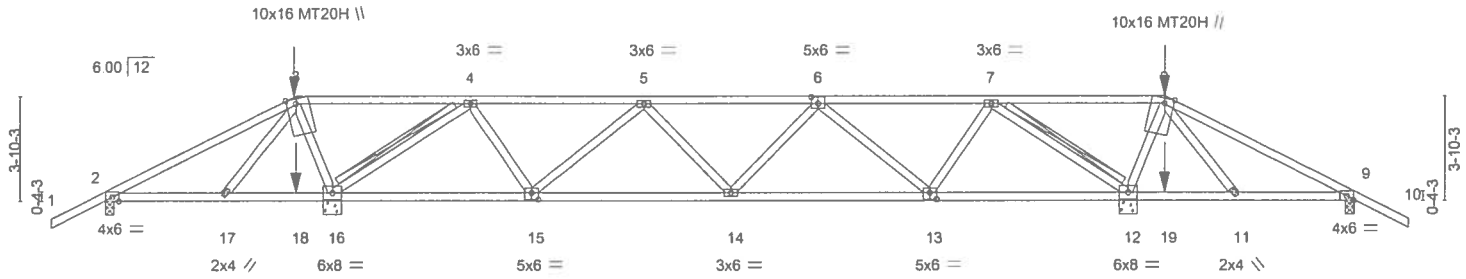

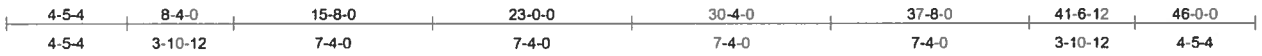

Plate Offsets (X,Y): [3:0-2-4,Edge], [6:0-3-0,0-3-0], [8:0-2-4,Edge], [13:0-3-0,0-3-0], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.14	14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.30	13-14	>999	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.82	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 228 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 3-6 2 X 4 SYP No.1D, 6-8 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-16, 7-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=-360/0-4-0, 16=3618/0-8-0, 12=3618/0-8-0, 9=-360/0-4-0
Max Horz 2=-77(load case 6)
Max Uplift 2=-604(load case 10), 16=-1463(load case 4), 12=-1456(load case 3), 9=-604(load case 9)
Max Grav 2=92(load case 4), 16=3627(load case 9), 12=3627(load case 10), 9=94(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-368/1479, 3-4=-674/1919, 4-5=-1219/406, 5-6=-2237/757, 6-7=-1219/396, 7-8=-666/1919, 8-9=-383/1479, 9-10=0/47
BOT CHORD 2-17=-1277/390, 17-18=-1352/494, 16-18=-1352/494, 15-16=-166/606, 14-15=-713/2167, 13-14=-716/2167, 12-13=-181/606, 12-19=-1352/475, 11-19=-1352/475, 9-11=-1277/370
WEBS 3-17=-207/200, 3-16=-1702/804, 4-16=-3058/1113, 4-15=-274/1158, 5-15=-1274/527, 5-14=0/205, 6-14=0/205, 6-13=-1274/527, 7-13=-274/1158, 7-12=-3058/1113, 8-12=-1702/796, 8-11=-207/200

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JOINT STRESS INDEX

2 = 0.45, 3 = 0.98, 4 = 0.91, 5 = 0.38, 6 = 0.85, 7 = 0.91, 8 = 0.98, 9 = 0.45, 11 = 0.34, 12 = 0.48, 13 = 0.80, 14 = 0.37, 15 = 0.80, 16 = 0.48 and 17 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 3 and 8.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 604 lb uplift at joint 2, 1463 lb uplift at joint 16 , 1456 lb uplift at joint 12 and 604 lb uplift at joint 9.
9) Girder carries hip end with 7-0-0 end setback.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-8=-117(F=-63), 8-10=-54, 2-18=-10, 18-19=-22(F=-12), 9-19=-10

Concentrated Loads (lb)

Vert: 3=-268(F) 8=-268(F) 18=-217(F) 19=-217(F)

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October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898792
L256400	T02	HIP	2	1	Job Reference (optional)

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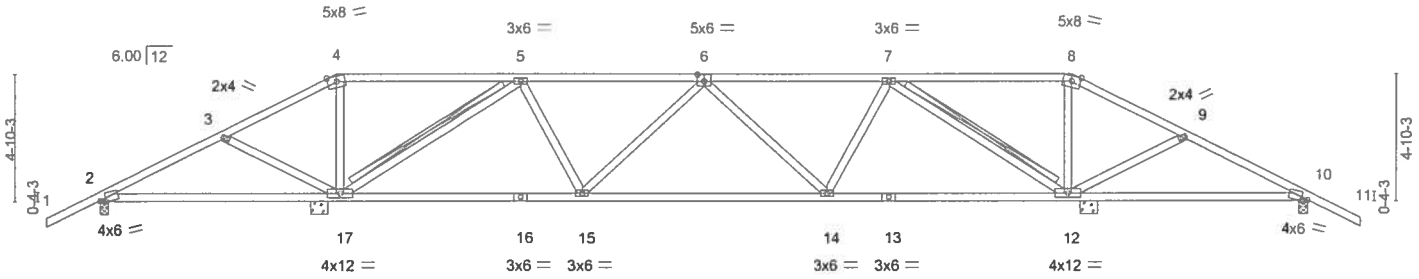

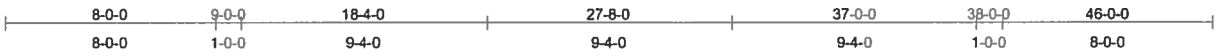

Plate Offsets (X,Y): [2:0-2-13,Edge], [6:0-3-0,0-3-0], [10:0-2-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.28	10-12	>391	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	14-15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.02	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 231 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-17, 7-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=128/0-4-0, 17=1450/0-8-0, 12=1450/0-8-0, 10=128/0-4-0
Max Horz 2=-89(load case 7)
Max Uplift 2=-242(load case 6), 17=-495(load case 5), 12=-485(load case 4), 10=-263(load case 7)
Max Grav 2=128(load case 1), 17=1466(load case 10), 12=1466(load case 11), 10=128/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-95/479, 3-4=-211/638, 4-5=-133/533, 5-6=-661/407, 6-7=-661/407, 7-8=-133/533, 8-9=-211/638, 9-10=-118/479, 10-11=0/47
BOT CHORD 2-17=-409/163, 16-17=-100/525, 15-16=-100/525, 14-15=-234/838, 13-14=-113/525, 12-13=-113/525, 10-12=-409/163
WEBS 3-17=-247/304, 4-17=-579/317, 5-17=-1226/612, 5-15=-66/364, 6-15=-282/192, 6-14=-282/192, 7-14=-66/364, 7-12=-1226/612, 8-12=-579/317, 9-12=-247/304

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October 8,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898792
L256400	T02	HIP	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.86, 3 = 0.33, 4 = 0.57, 5 = 0.44, 6 = 0.47, 7 = 0.44, 8 = 0.57, 9 = 0.33, 10 = 0.86, 12 = 0.25, 13 = 0.19, 14 = 0.44, 15 = 0.44, 16 = 0.19 and 17 = 0.25

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 242 lb uplift at joint 2, 495 lb uplift at joint 17, 485 lb uplift at joint 12 and 263 lb uplift at joint 10.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898793
L256400	T03	HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Oct 08 13:08:55 2007 Page 1

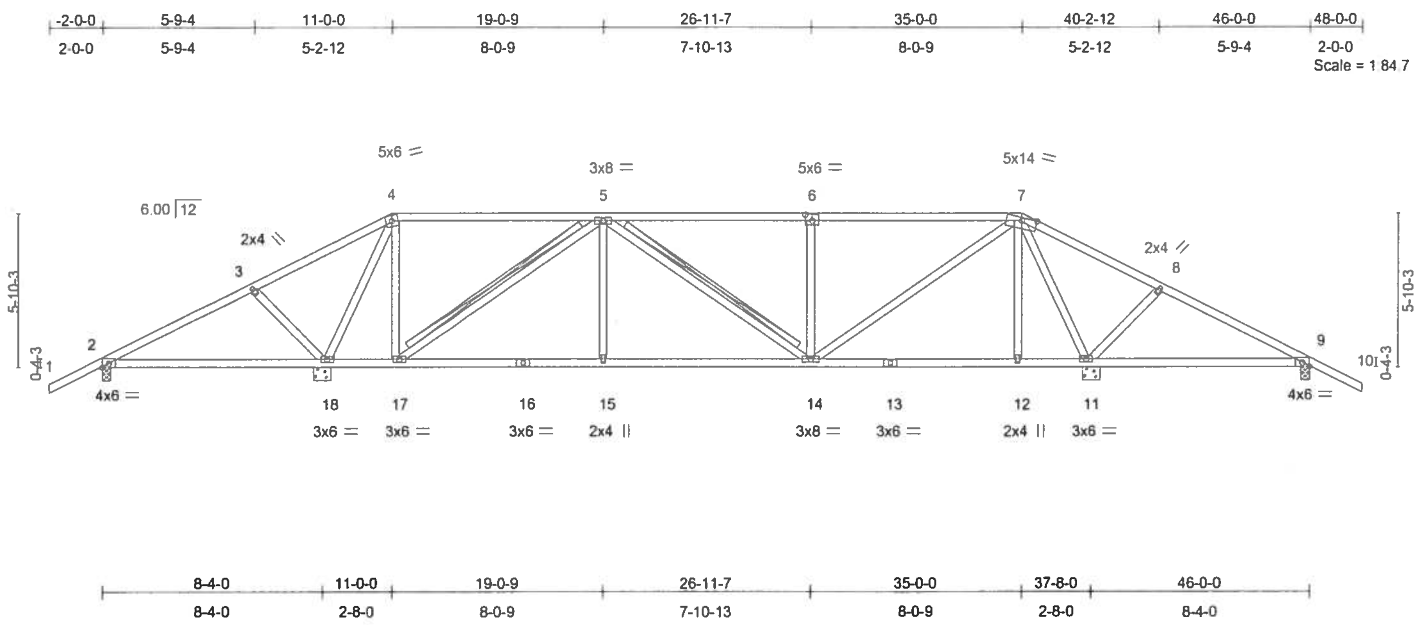

Plate Offsets (X,Y): [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.18	9-11	>565	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.17	15-17	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 249 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-18,9-11.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-17, 5-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=187/0-4-0, 18=1389/0-8-0, 11=1405/0-8-0, 9=176/0-4-0
Max Horz 2=-101(load case 7)
Max Uplift 2=-226(load case 6), 18=-455(load case 5), 11=-437(load case 4), 9=-256(load case 7)
Max Grav 2=188(load case 10), 18=1390(load case 10), 11=1406(load case 11), 9=178(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/355, 3-4=-125/518, 4-5=-136/185, 5-6=-872/572, 6-7=-876/574, 7-8=-111/541, 8-9=-88/378, 9-10=0/47
BOT CHORD 2-18=-299/187, 17-18=0/179, 16-17=-211/879, 15-16=-211/879, 14-15=-211/879, 13-14=0/173, 12-13=0/173, 11-12=0/170, 9-11=-319/175
WEBS 3-18=-264/295, 4-18=-1236/517, 4-17=-151/557, 5-17=-908/473, 5-15=0/245, 5-14=-16/1, 6-14=-446/313, 7-14=-470/931, 7-12=0/128, 7-11=-1256/509,

Continued on page 2

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October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898793
L256400	T03	HIP	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.43, 3 = 0.33, 4 = 0.72, 5 = 0.56, 6 = 0.64, 7 = 0.50, 8 = 0.33, 9 = 0.44, 11 = 0.45, 12 = 0.33, 13 = 0.30, 14 = 0.87, 15 = 0.33, 16 = 0.37, 17 = 0.35 and 18 = 0.45

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 2, 455 lb uplift at joint 18, 437 lb uplift at joint 11 and 256 lb uplift at joint 9.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898794
L256400	T04	HIP	2	1	Job Reference (optional)

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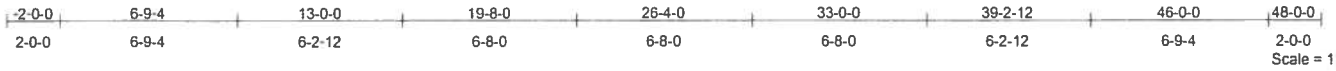

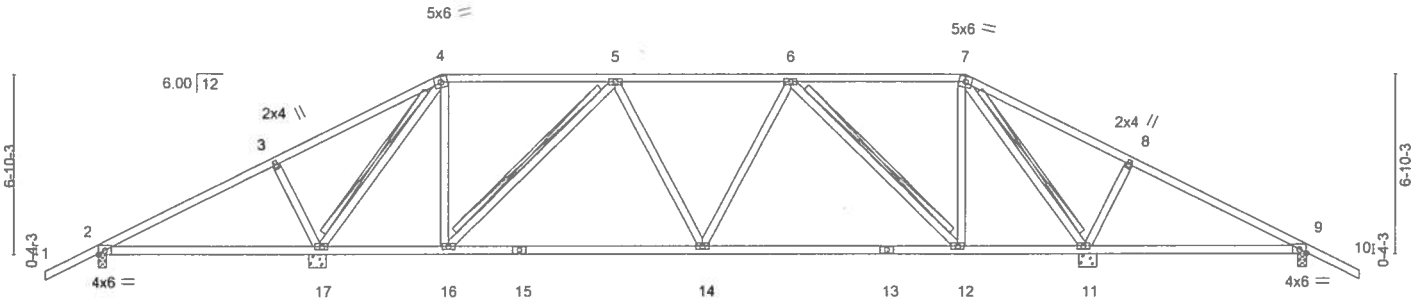

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.21	9-11	>468	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.30	14-16	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 248 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-17, 5-16, 6-12, 7-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=263/0-4-0, 17=1315/0-8-0, 11=1315/0-8-0, 9=263/0-4-0
Max Horz 2=-113(load case 7)
Max Uplift 2=-228(load case 6), 17=-411(load case 5), 11=-395(load case 4), 9=-254(load case 7)
Max Grav 2=267(load case 10), 17=1315(load case 1), 11=1315(load case 1), 9=267(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-70/202, 3-4=-46/348, 4-5=-430/365, 5-6=-872/580, 6-7=-430/365, 7-8=-26/348, 8-9=-49/202, 9-10=0/47
BOT CHORD 2-17=-158/161, 16-17=-39/419, 15-16=-182/844, 14-15=-182/844, 13-14=-182/844, 12-13=-182/844, 11-12=-20/419, 9-11=-158/161
WEBS 3-17=-326/350, 4-17=-1155/467, 4-16=-136/502, 5-16=-617/324, 5-14=0/181, 6-14=0/181, 6-12=-617/324, 7-12=-136/502, 7-11=-1155/467, 8-11=-326/350

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October 8,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898794
L256400	T04	HIP	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.38, 3 = 0.33, 4 = 0.59, 5 = 0.44, 6 = 0.44, 7 = 0.59, 8 = 0.33, 9 = 0.38, 11 = 0.39, 12 = 0.36, 13 = 0.42, 14 = 0.44, 15 = 0.42, 16 = 0.36 and 17 = 0.39

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 2, 411 lb uplift at joint 17, 395 lb uplift at joint 11 and 254 lb uplift at joint 9.

LOAD CASE(S) Standard

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October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898795
L256400	T05	HIP	2	1	Job Reference (optional)

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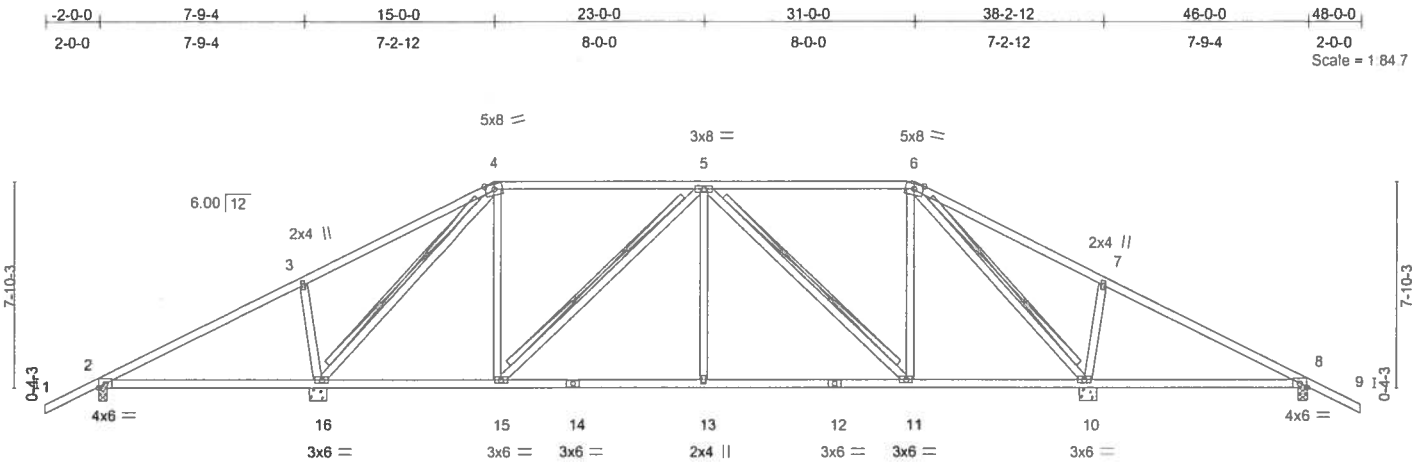

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.25	2-16	>388	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.18	8-10	>559	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-16, 5-15, 5-11, 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=327/0-4-0, 16=1251/0-8-0, 10=1251/0-8-0, 8=327/0-4-0
Max Horz 2=-125(load case 7)
Max Uplift 2=-225(load case 6), 16=-374(load case 5), 10=-357(load case 4), 8=-250(load case 7)
Max Grav 2=329(load case 10), 16=1251(load case 1), 10=1251(load case 1), 8=329(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-79/114, 3-4=-14/171, 4-5=-607/481, 5-6=-607/481, 6-7=-14/171, 7-8=-79/114, 8-9=0/47
BOT CHORD 2-16=-41/170, 15-16=-73/601, 14-15=-162/883, 13-14=-162/883, 12-13=-162/883, 11-12=-162/883, 10-11=-44/601, 8-10=-41/170
WEBS 3-16=-393/421, 4-16=-1019/381, 4-15=-56/387, 5-15=-434/199, 5-13=0/249, 5-11=-434/199, 6-11=-56/387, 6-10=-1019/381, 7-10=-393/421

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October 8,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898795
L256400	T05	HIP	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.28, 3 = 0.33, 4 = 0.59, 5 = 0.56, 6 = 0.59, 7 = 0.33, 8 = 0.28, 10 = 0.36, 11 = 0.35, 12 = 0.37, 13 = 0.33, 14 = 0.37, 15 = 0.35 and 16 = 0.36

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 2, 374 lb uplift at joint 16, 357 lb uplift at joint 10 and 250 lb uplift at joint 8.

LOAD CASE(S) Standard

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October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898796
L256400	T06	HIP	2	1	Job Reference (optional)

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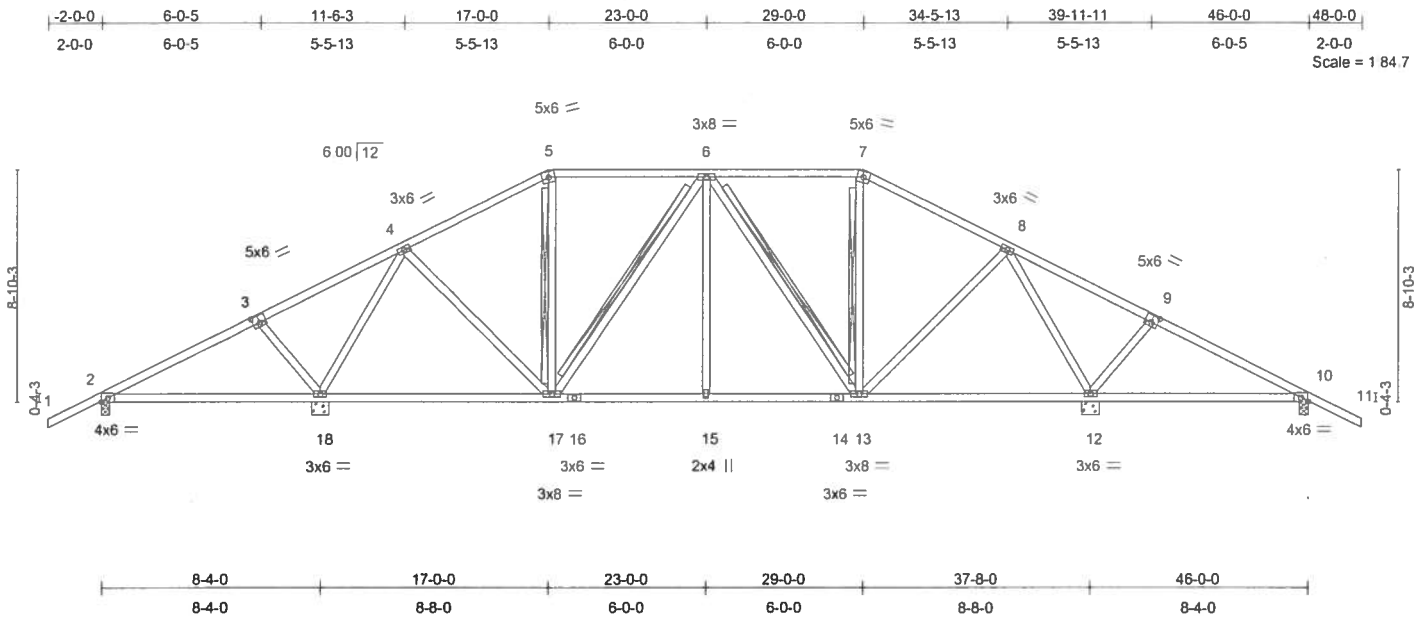

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [9:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.21	2-18	>464	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.15	17-18	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.02	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 269 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-17, 6-17, 6-13, 7-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=284/0-4-0, 18=1294/0-8-0, 12=1294/0-8-0, 10=284/0-4-0
Max Horz 2=-136(load case 7)
Max Uplift 2=-225(load case 6), 18=-383(load case 6), 12=-350(load case 7), 10=-258(load case 7)
Max Grav 2=293(load case 10), 18=1294(load case 1), 12=1294(load case 1), 10=293(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/117, 3-4=-53/297, 4-5=-727/515, 5-6=-592/522, 6-7=-592/522, 7-8=-727/515, 8-9=-9/297, 9-10=-58/117, 10-11=0/47
BOT CHORD 2-18=-84/109, 17-18=-38/322, 16-17=-88/721, 15-16=-88/721, 14-15=-88/721, 13-14=-88/721, 12-13=0/322, 10-12=-84/109
WEBS 3-18=-294/317, 4-18=-1103/565, 4-17=-59/388, 5-17=-30/118, 6-17=-295/132, 6-15=0/142, 6-13=-295/132, 7-13=-30/118, 8-13=-59/388, 8-12=-1103/565, 9-12=-294/317

Continued on page 2

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October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898796
L256400	T06	HIP	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.46, 3 = 0.58, 4 = 0.39, 5 = 0.44, 6 = 0.57, 7 = 0.44, 8 = 0.39, 9 = 0.58, 10 = 0.46, 12 = 0.43, 13 = 0.57, 14 = 0.31, 15 = 0.33, 16 = 0.31, 17 = 0.57 and 18 = 0.43

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 2, 383 lb uplift at joint 18, 350 lb uplift at joint 12 and 258 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898797
L256400	T07	HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Oct 08 13:09:00 2007 Page 1

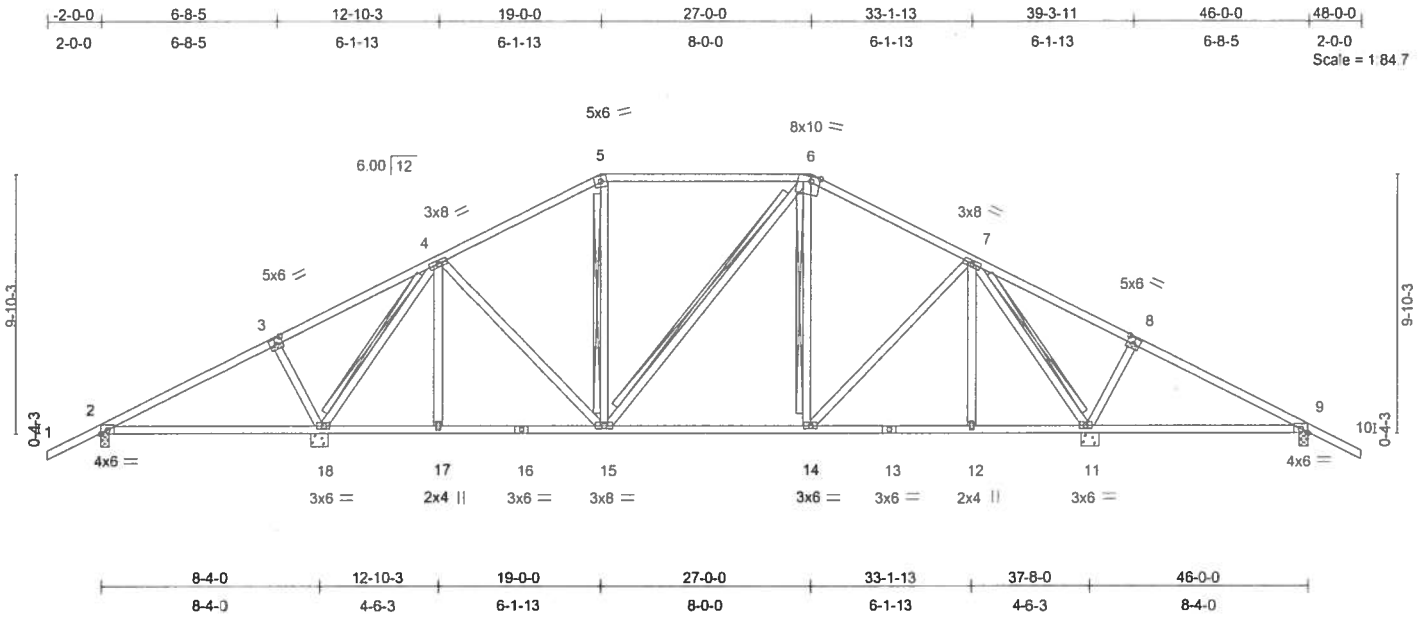

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [6:0-4-3,Edge], [8:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.21	9-11	>475	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.17	2-18	>570	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.03	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 273 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-18,9-11.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-18, 5-15, 6-15, 6-14 , 7-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=316/0-4-0, 18=1263/0-8-0, 11=1258/0-8-0, 9=320/0-4-0
Max Horz 2=-148(load case 7)
Max Uplift 2=-220(load case 6), 18=-405(load case 6), 11=-366(load case 7), 9=-256(load case 7)
Max Grav 2=327(load case 10), 18=1263(load case 1), 11=1258(load case 1), 9=328(load case 11)

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October 8,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898797
L256400	T07	HIP	2	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-108/86, 3-4=-30/232, 4-5=-765/555, 5-6=-623/566, 6-7=-766/557, 7-8=0/225, 8-9=-110/69, 9-10=0/47

BOT CHORD 2-18=-40/123, 17-18=-29/465, 16-17=-29/465, 15-16=-29/465, 14-15=-11/624, 13-14=0/468, 12-13=0/468, 11-12=0/468, 9-11=-33/106

WEBS 3-18=-328/343, 4-18=-1077/482, 4-17=0/118, 4-15=-52/270, 5-15=-133/107, 6-15=-106/103, 6-14=-114/105, 7-14=-62/265, 7-12=0/118, 7-11=-1070/466, 8-11=-328/342

JOINT STRESS INDEX

2 = 0.37, 3 = 0.76, 4 = 0.73, 5 = 0.68, 6 = 0.72, 7 = 0.73, 8 = 0.76, 9 = 0.38, 11 = 0.40, 12 = 0.33, 13 = 0.23, 14 = 0.36, 15 = 0.56, 16 = 0.23, 17 = 0.33 and 18 = 0.40

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2, 405 lb uplift at joint 18, 366 lb uplift at joint 11 and 256 lb uplift at joint 9.

LOAD CASE(S) Standard

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October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898798
L256400	T08	HIP	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Oct 08 13:09:01 2007 Page 1

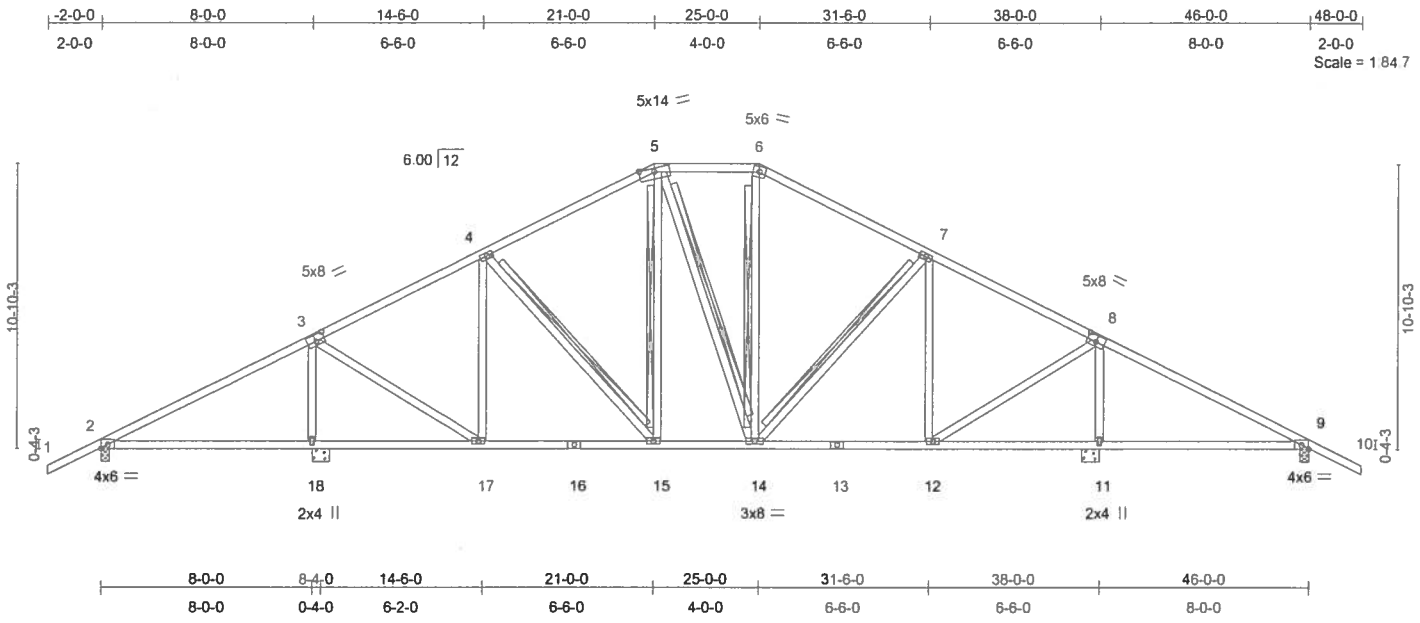

Plate Offsets (X,Y): [3:0-4-0,0-3-0], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.23	9-11	>403	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.16	9-11	>603	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 280 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-15, 5-15, 5-14, 6-14 , 7-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=309/0-4-0, 18=1269/0-8-0, 11=1267/0-8-0, 9=311/0-4-0
Max Horz 2=160(load case 6)
Max Uplift 2=-218(load case 6), 18=-405(load case 6), 11=-362(load case 7), 9=-261(load case 7)
Max Grav 2=321(load case 10), 18=1269(load case 1), 11=1267(load case 1), 9=326(load case 11)

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October 8,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK J1898798
L256400	T08	HIP	7	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-79/171, 3-4=-775/524, 4-5=-787/620, 5-6=-632/624, 6-7=-788/620, 7-8=-776/525, 8-9=-61/167, 9-10=0/47

BOT CHORD 2-18=-70/160, 17-18=-57/147, 16-17=-84/618, 15-16=-84/618, 14-15=-10/632, 13-14=-85/619, 12-13=-85/619, 11-12=-51/144, 9-11=-63/158

WEBS 3-18=-1168/698, 3-17=-273/778, 4-17=-362/227, 4-15=-39/116, 5-15=-62/136, 5-14=-145/146, 6-14=-62/138, 7-14=-42/117, 7-12=-361/226, 8-12=-272/775, 8-11=-1165/696

JOINT STRESS INDEX

2 = 0.36, 3 = 0.63, 4 = 0.40, 5 = 0.76, 6 = 0.45, 7 = 0.40, 8 = 0.63, 9 = 0.36, 11 = 0.41, 12 = 0.44, 13 = 0.30, 14 = 0.64, 15 = 0.37, 16 = 0.30, 17 = 0.44 and 18 = 0.41

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x6 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 2, 405 lb uplift at joint 18, 362 lb uplift at joint 11 and 261 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
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1400 Coastal Ray Blvd
Boynton Beach, FL 33435

October 8,2007

Job	Truss	Truss Type	Qty	Ply	RONALD CLARK - BURDICK
L256400	EJ7_ALT	MONO TRUSS	34	1	J1898788A Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Oct 08 16:09:40 2007 Page 1

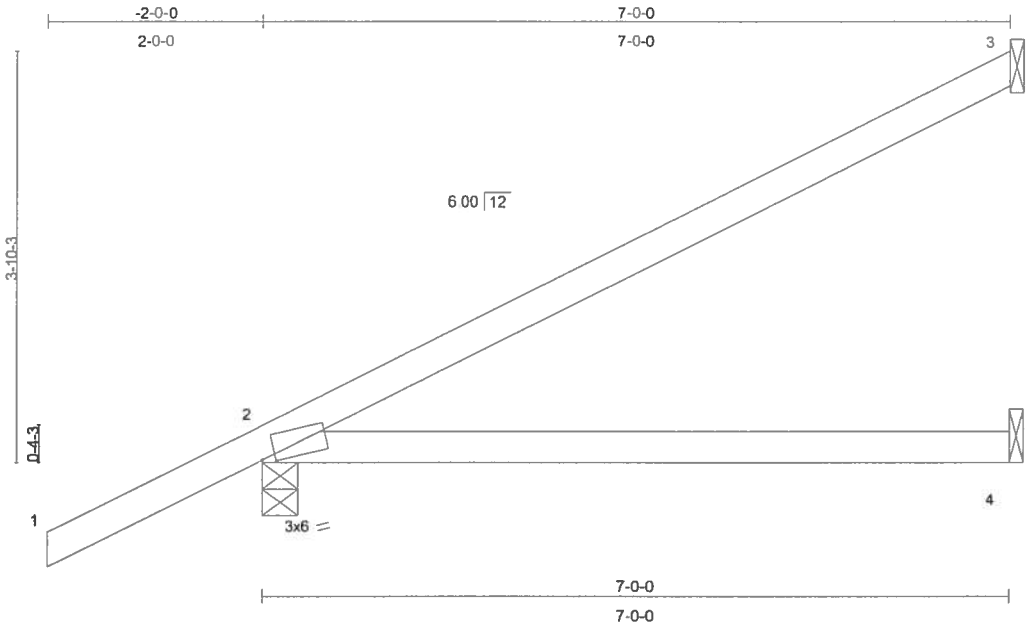

Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.32	2-4	>253	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.16	2-4	>506	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical
Max Horz 2=161(load case 6)
Max Uplift 3=-94(load case 6), 2=-225(load case 6), 4=-64(load case 5)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-130/54
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.51

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NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3, 225 lb uplift at joint 2 and 64 lb uplift at joint 4.

LOAD CASE(S) Standard

October 8,2007

Julius Lee
Truss Design Engineer
Florida PE No. 31880
1100 Coastal Ray Blvd
Boynton Beach, FL 33435

October 8,2007

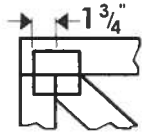
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

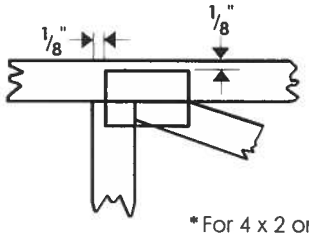
Builders FirstSource

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

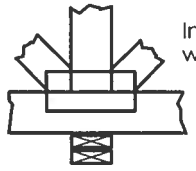
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

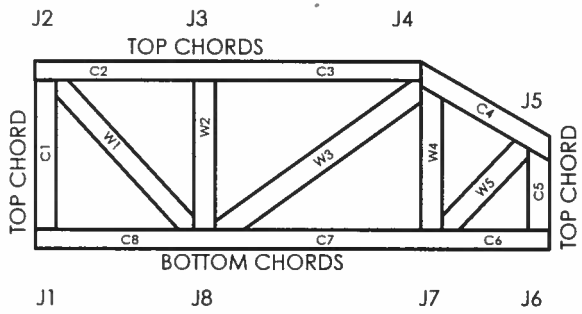
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	6' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	8' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	8' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 5"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	15' 6"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 6"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

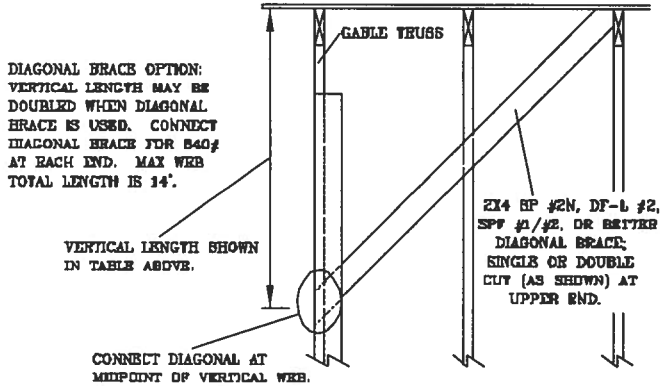
SOUTHERN PINE
#3
STUD
STANDARD

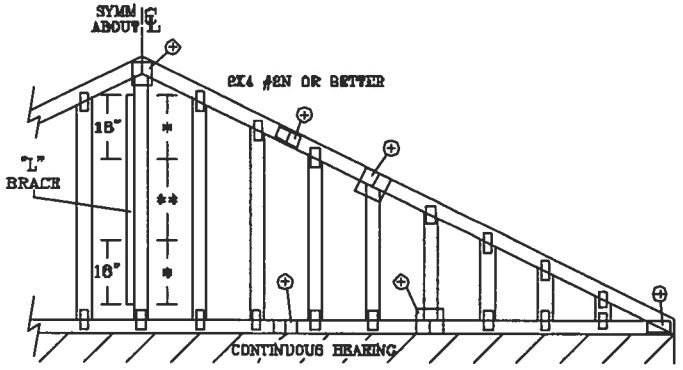
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#2
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13015
DATE	11/26/03
DRWG	MITEK STD GABLE 16 E HT
	-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 8"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 8"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 5"
		STUD	3' 3"	4' 6"	4' 8"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 5"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 8"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	6' 6"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	8' 4"	8' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	8' 4"	8' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 8"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 8"	5' 8"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

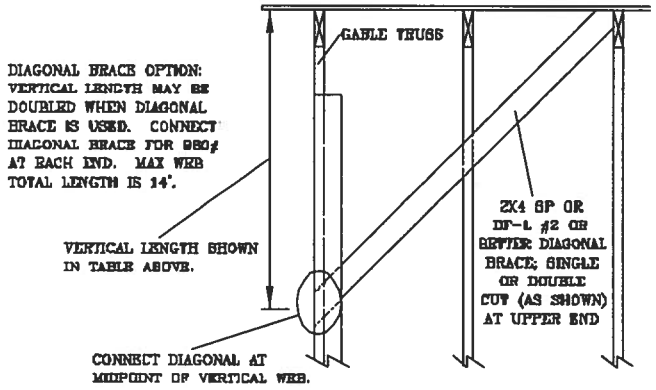
SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

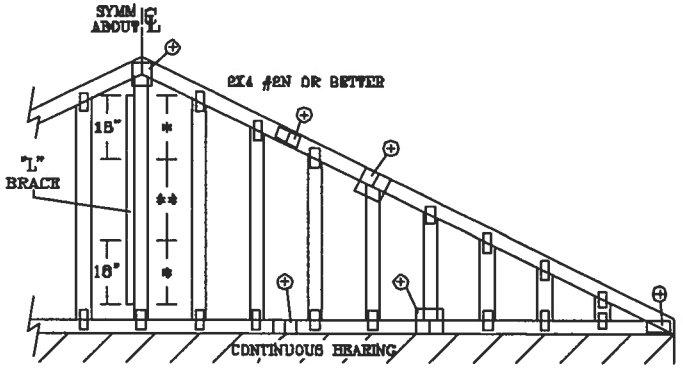
HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#2
#2

DOUGLAS FIR-LARCH
#1
#2



DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 980# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR PX3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

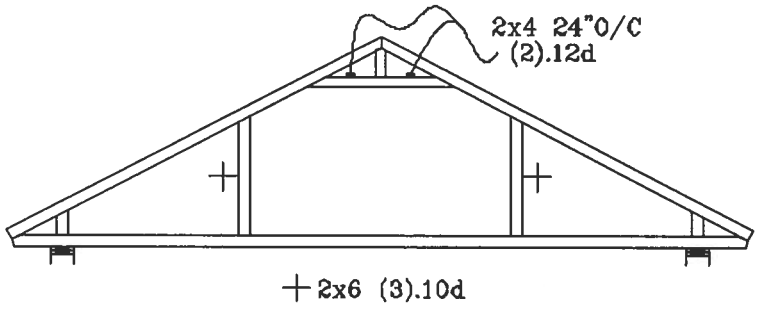
No: 34669
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

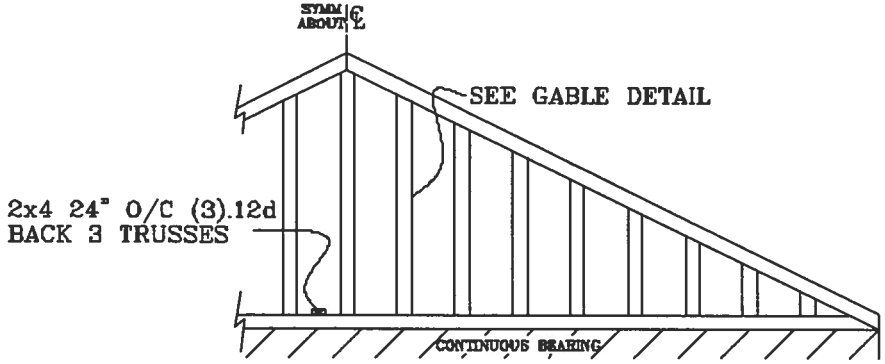
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MTEK STD GABLE 30' E W7
-ENG

TYPICAL ATTIC TRUSS BRACING

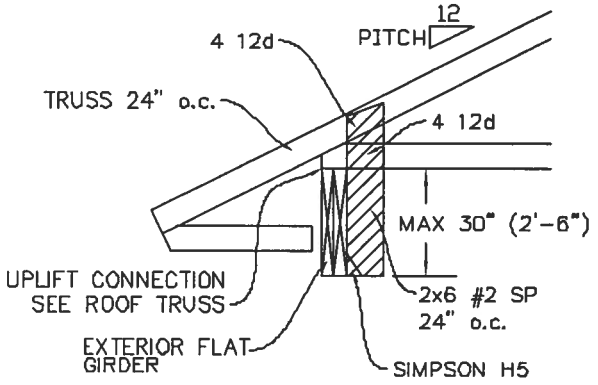


GABLE END TRUSS DETAIL

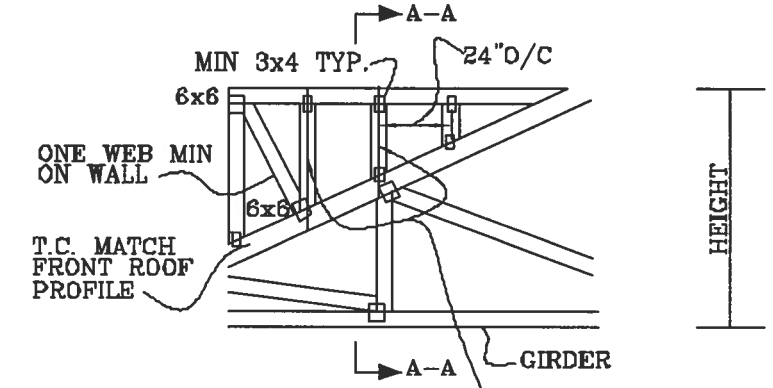


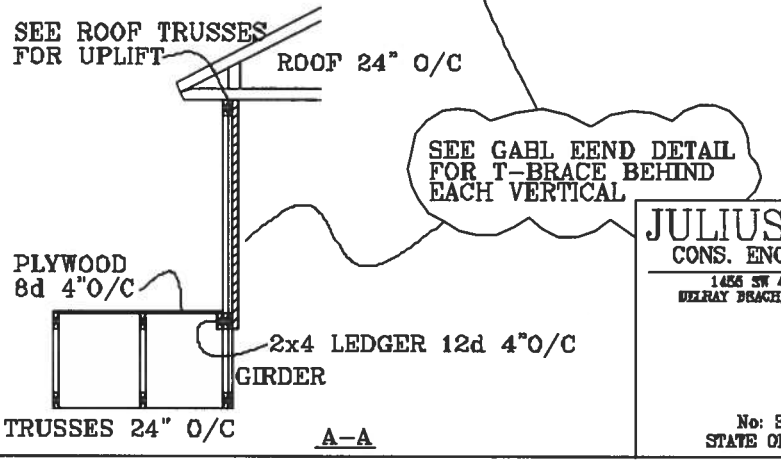
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

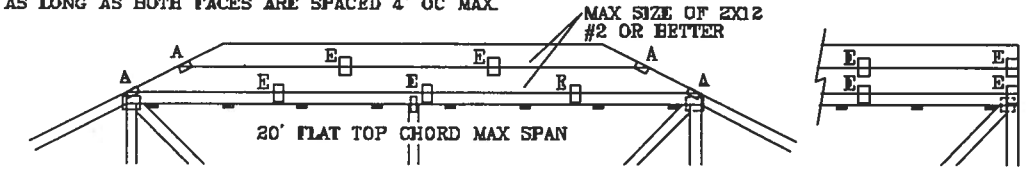
110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

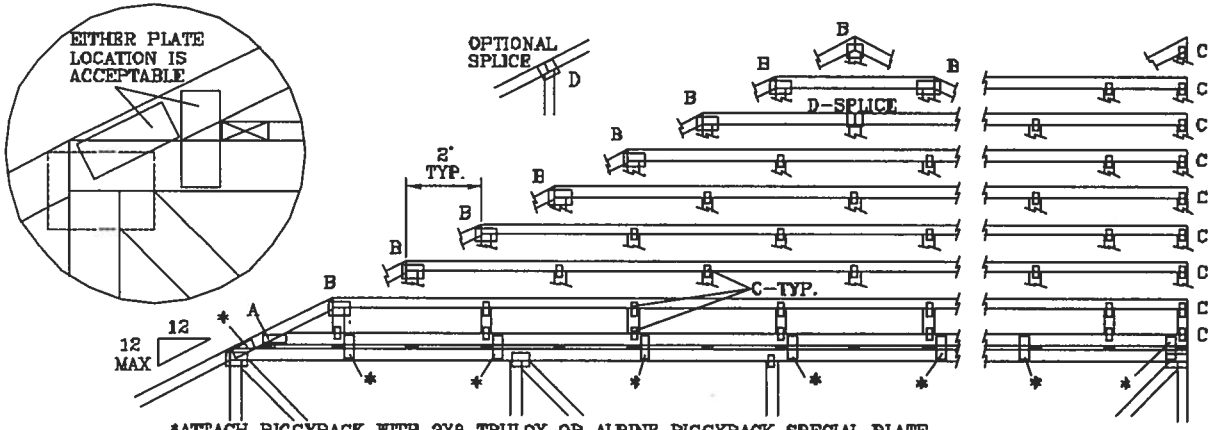
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.



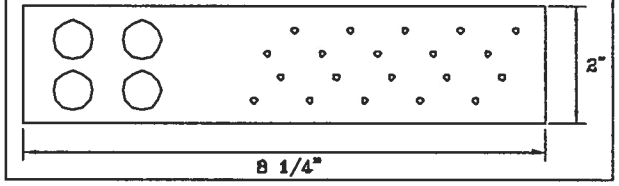


*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

*** PIGGYBACK SPECIAL PLATE**

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34868
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

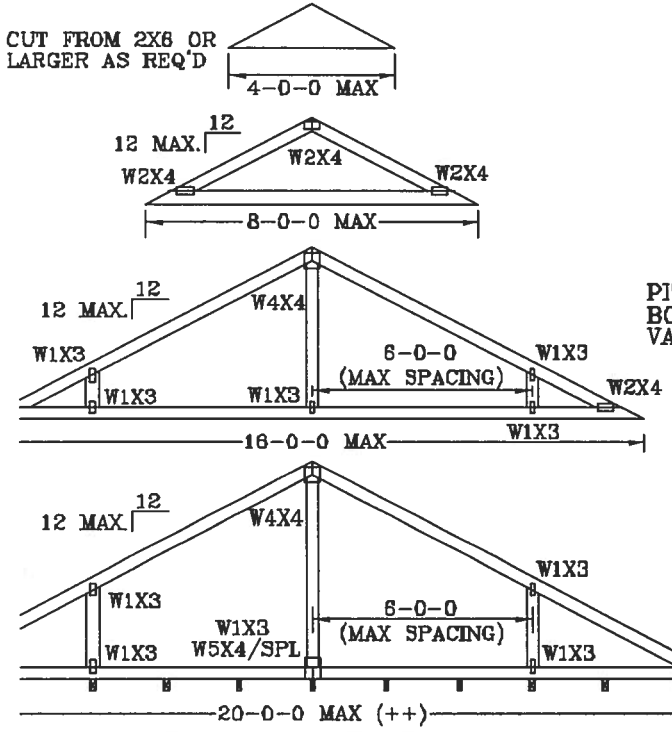
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

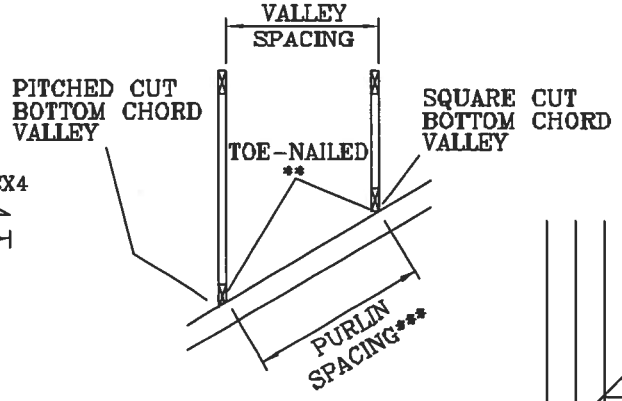
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

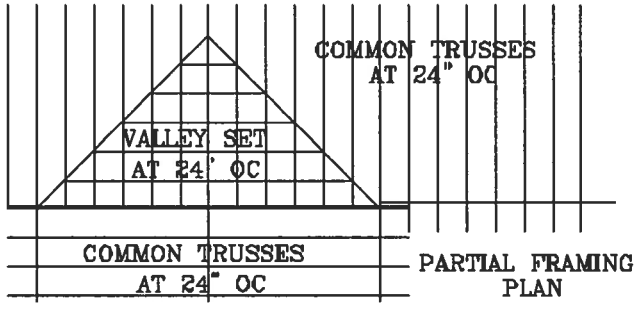
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.





THIS DRAWING REPLACES DRAWING A105

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR.; SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

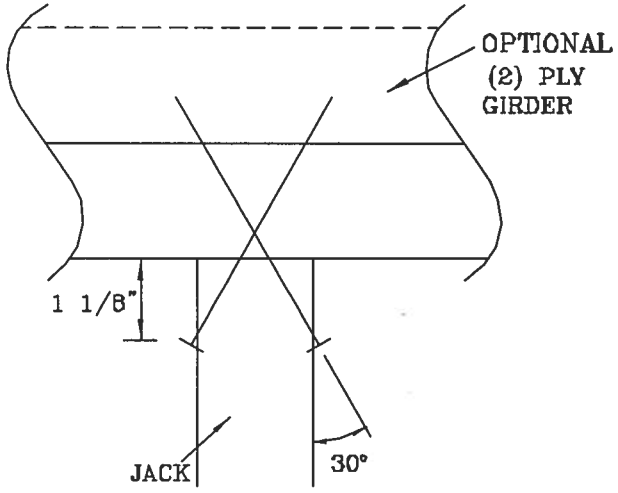
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

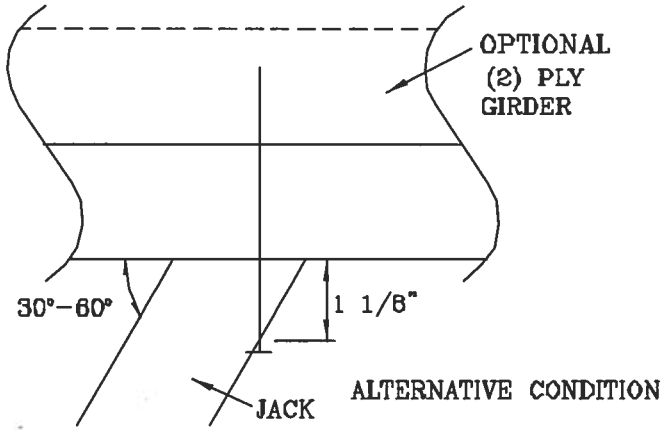
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1450 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34669
STATE OF FLORIDA

TC LL	PSF	REF TOE-NAIL
TC DL	PSF	DATE 09/12/07
BC DL	PSF	DRWG CNTONAIL1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.	1.00	
SPACING		

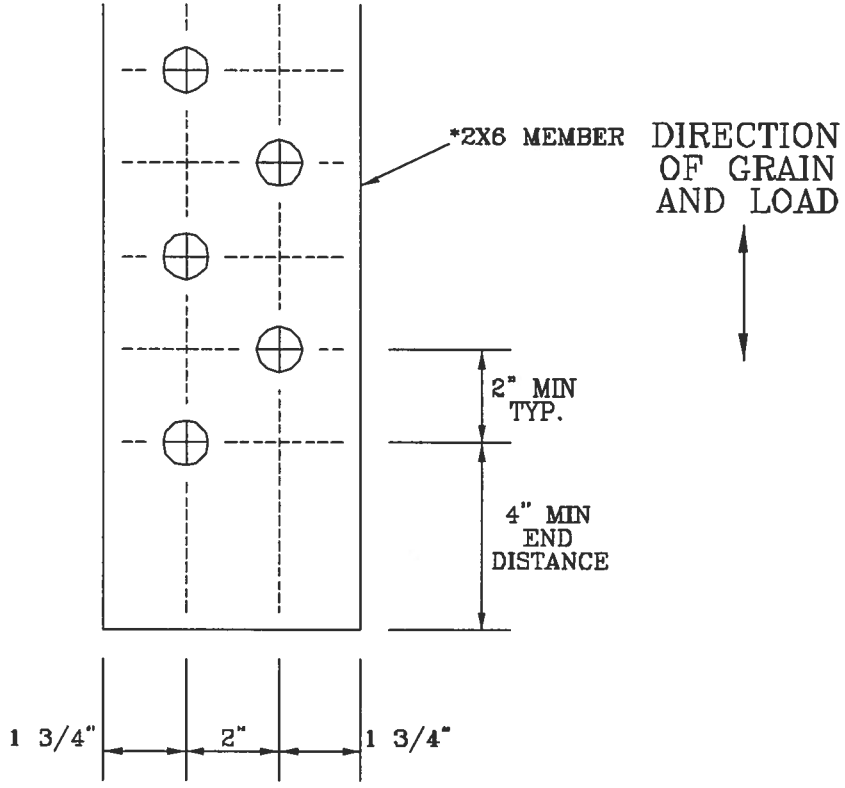
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

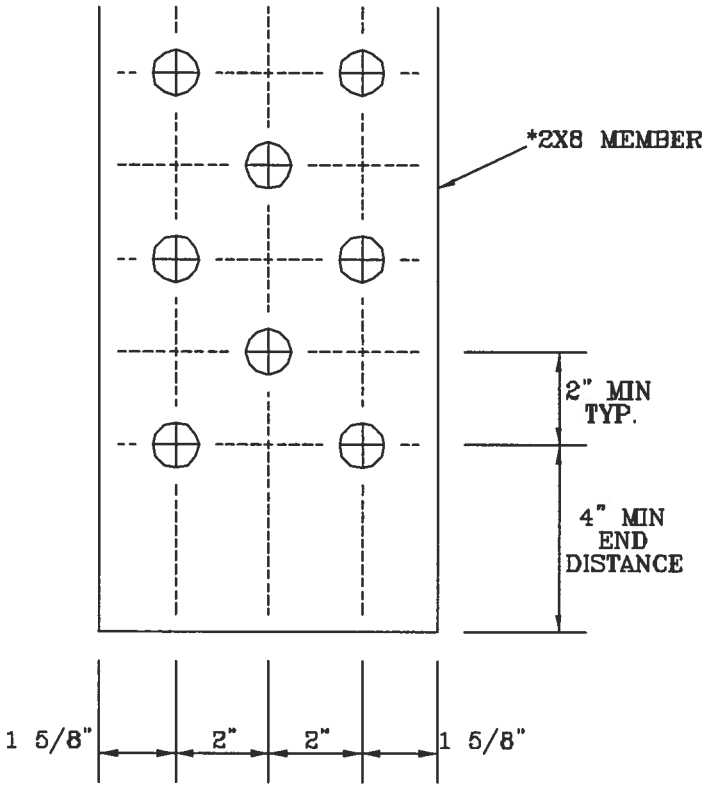
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628,016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

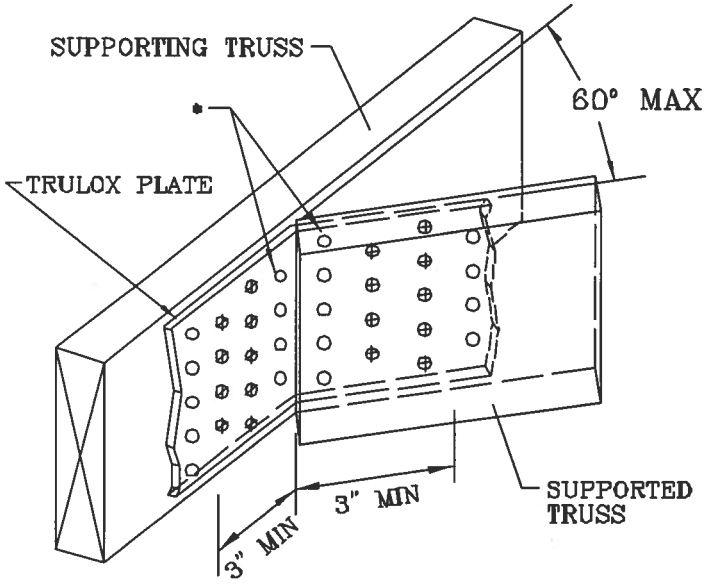
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

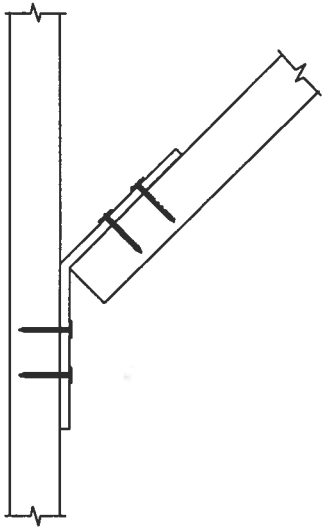
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

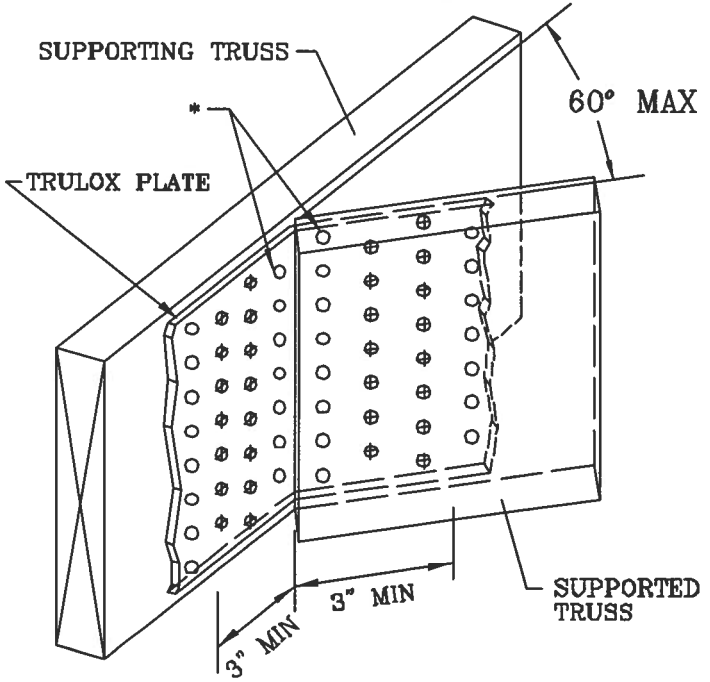
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

==WARNING== TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI) (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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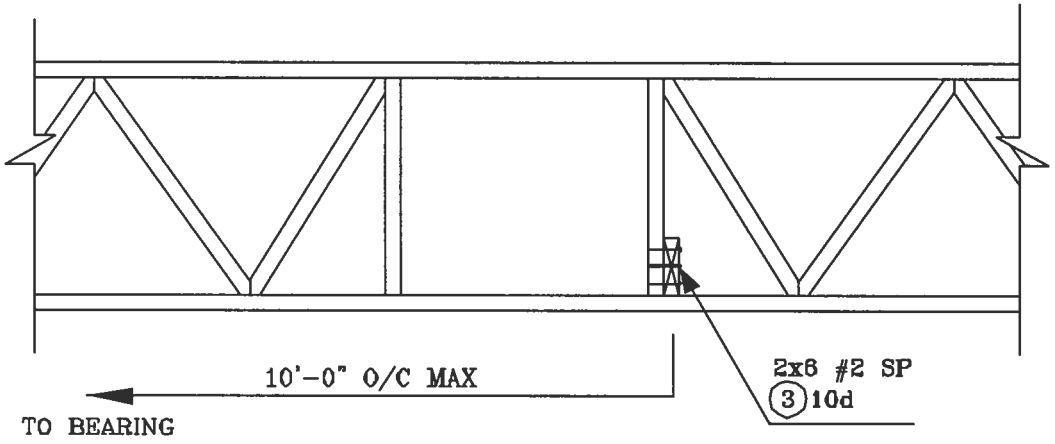
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

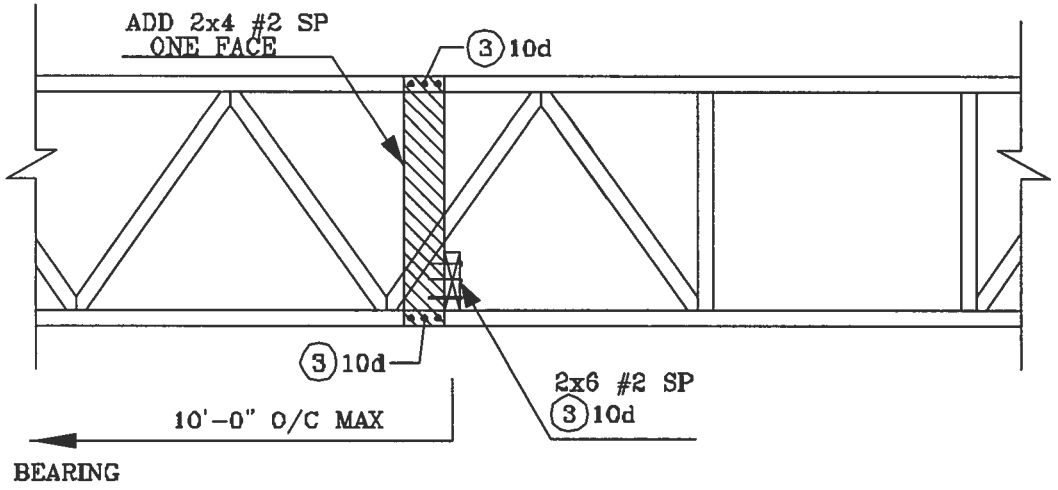
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

Permit # 26469

Tax Folio No. R04285-211

NOTICE OF COMMENCEMENT

STATE OF FLORIDA

SS:

COUNTY OF COLUMBIA

Inst:200712026131 Date:11/27/2007 Time:2:39 PM
DC,P.DeWitt Cason,Columbia County Page 1 of 2

THE UNDERSIGNED, hereby gives notice that improvements will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description of Property (legal description and street address if available): (see attached)
2. General description of improvements: A SINGLE FAMILY DWELLING
3. Owner information:
   a. Name and Address:
      ISA AND CLAYTON BURDICK
      144 HARBOR LANE
      TAVERNIER, FL 33070
   b. Interest in Property: FEE SIMPLE
   c. Name and address of fee simple title holder (if other than owner):

COLUMBIA COUNTY, FLORIDA CLERK CIRCUIT COURT

State of Florida, County of Columbia
I HEREBY CERTIFY that the Foregoing is a correct copy from the Official Records of this office.
Hal Harden D.C.
P. DeWitt Cason, Clerk of Courts

4. Contractor (Name and Address):
   RONALD CLARK CONSTRUCTION
   15816 NW CR 1491
   ALACHUA, FL 32615
5. Surety:
   a. Name and Address:

   b. Amount of Bond: $
6. Lender (Name and Address): Wachovia Mortgage Corporation
   Residential Construction Lending
   3563 Phillips Hwy, Suite 400C, 2nd Floor
   Jacksonville, FL 32207
7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7, Florida Statutes (Name and Address):
   Name:
   Address:
8. In addition to himself, Owner designates of to receive a copy of Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.
9. Expiration date of Notice of Commencement (the expiration date is one (1) year from the date of recording unless a different date is specified):

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

Verification pursuant to Section 92.525, Florida Statues: Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Isa Burdick Date: 11507
Owner

Date: 11507
Owner

The following instrument was acknowledged before me this 15 day of November, 2007 by Isa Burdick and Clayton Burdick
who is personally known to me or who has produced as Identification
and who did take an oath.

Josephine A Kaczka
Notary Public

My Commission Expires: 8/10/2010

JOSEPHINE A. KACZKA
Comm# DD0353710
Expires 8/10/2010
Florida Notary Assn., Inc

JOSEPHINE A. KACZKA
Comm# DD0353710
8/10/2010
Florida Notary Assn., Inc

NOTICE OF COMMENCEMENT - FLORIDA (source: #244254)

DEC-07-07 FRI 09:50 AM CLERK/CONSTRUCTION 462 4257 P.01

SCHEDULE A

LEGAL DESCRIPTION

Lot 11, Lee Perry, an unrecorded subdivision:

Commence at the NE corner of SW 1/4 of NW 1/4, Section 14, Township 7 South, Range 16 East, Columbia County, Florida, thence South 89 deg. 00 min. 27 sec. West 517.37 feet, thence South 00 deg. 50 min. 39 sec. East 807.37 feet, to the point of beginning, thence continue South 00 deg. 50 min. 39 sec. East 458.16 feet to the North Line of Shiloh Road, thence North 89 deg. 43 min. 31 sec. East along the North line of Shiloh Road 477.29 feet, thence North 00 deg. 50 min. 22 sec. West 458.16 feet, thence South 89 deg. 43 min. 31 sec. West 477.32 feet to the point of beginning.

Lake City • (386) 755-363
Fax • (386) 752-545
Jacksonville • (904) 381-890
Fax • (904) 381-890
Quincy • (850) 442-349
Fax • (850) 442-400

CAL-TECH TESTING, INC.

ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625
4784 Rosselle St. • Jacksonville, FL 32254
2230 Greensboro Hwy., Quincy, FL 32351

JOB NO.: 07-634
DATE TESTED: 12/7/07

REPORT OF IN-PLACE DENSITY TEST

ASTM METHOD: ✓ (D-2922) Nuclear _____ (D-2937) Drive Cylinder _____ Other

PROJECT: Barn Keepers House 26469

CLIENT: Edgley Const.

GENERAL CONTRACTOR: SAC EARTHWORK CONTRACTOR: SAC

SOIL USE (SEE NOTE): 1 - Pad SPECIFICATION REQUIREMENTS: 95%

TECHNICIAN: C. Day

MODIFIED (ASTM D-1557): X X X STANDARD (ASTM D-698): _____

TEST NO.	TEST LOCATION	TEST: ___DEPTH ___ELEV. ___LIFT	PROCTOR NO.	WET DENS. LBS.CU.FT.	DRY DENS. LBS.CU.FT.	MOIST PERCENT	% MAX. DENS.
1	N.W. Corner 8' E x 8' S.	12"	07-157-10	108.5	103.6	4.7	99
2	East Side, Center 8' W.	12"		111.8	105.6	5.9	101
3	S.W. Corner 10' E. x 10' N.	12"		112.3	101.9	10.2	97

REMARKS: _____

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT. MOIST.
07-157-10	Tan fine Sand w/ Silt & Trace of Clay	104.6	12.7

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test location and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 14-7S-16-04218-211 **Building permit No.** 000026469

Use Classification SFD, UTILITY **Fire:** 0.00

Permit Holder RONALD CLARK **Waste:**

Owner of Building CLAY & ISA BURDICK **Total:** 0.00

Location: 1625 SW SHILOAH ST, FT. WHITE, FL

Date: 07/25/2008

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)