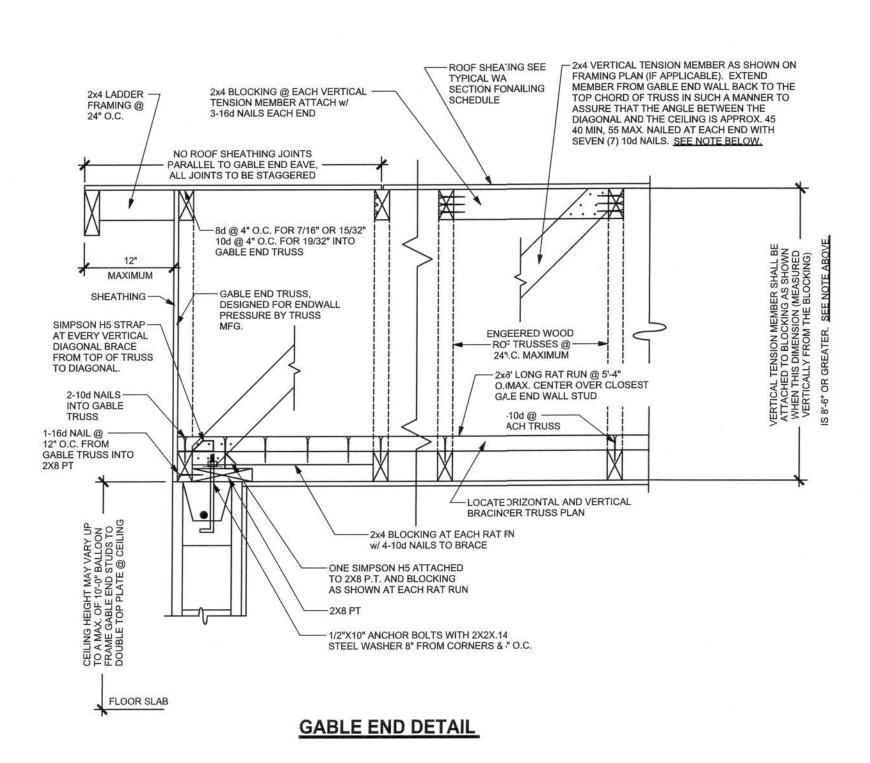


TYPICAL 1 STORY BLOCK WALL SE;TION

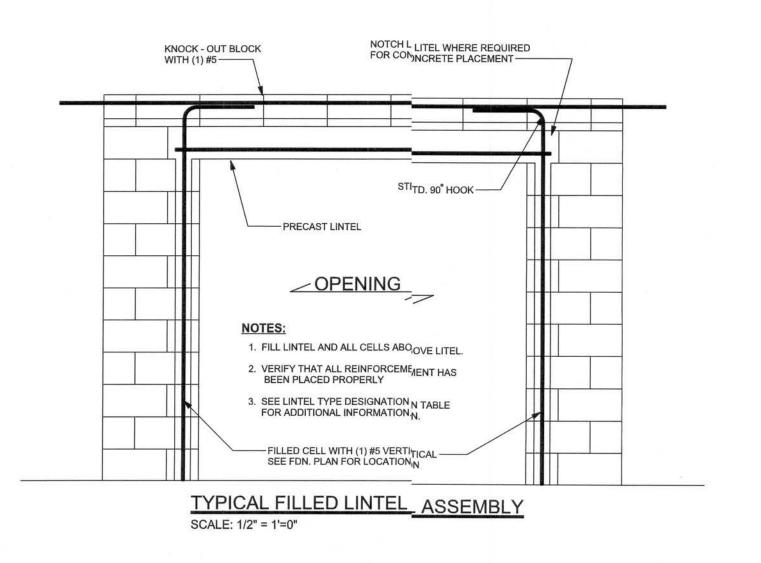
SCALE: 1" = 1'-0'

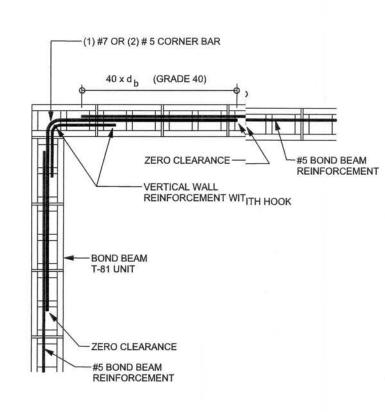


MASONRY TRUSS ANCHOR TABLE OBTAIN UUPLIFT REQUIREMENTS FROM TRUSS

OBTAIN UUPLIFT REQUIREMENTS FROM TE MANUFAGCTURER'S ENGINEERING

IPLIFT I LBS.	TRUSS CONNECTOR MASONRY *			
< 120 05	TA22	10-10d x 1 1/2"		
< 160{ ₀₅	TA22	11-10d		
< 860 ₅₀	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS		
< 117! ₇₅	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS		
< 104(40	META20	7-10d, 1 1/2"		
< 149 90	META20	10-10d, 1 1/2"		
< 178180	HETA20	7-16d		
< 178180	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOC 14 - 16d SINKER IN GIRDER		
< 213(30	HHETA20	17-10d, 1 1/2"		
< 231(10	HHETA24	21-10d, 1 1/2"		
< 396 ⁶ 5	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT		
< 1098380	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT		
< 1053530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT		





TYPICAL BOND BEAM CORNIER DETAIL

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR*		TO PLATES	TO RAFTER/TRUSS	S TO STUDS		
< 420	< 245	< 245 H5A		3-8d			
< 455	< 265	H5	4-8d	4-8d			
< 360	< 235	H4	4-8d	4-8d			
< 455	< 320	НЗ	4-8d	4-8d			
< 415	< 365	H2.5	5-8d	5-8d			
< 600	< 535	H2.5A	5-8d	5-8d			
< 950	< 820	H6	8-8d	8-8d			
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"			
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"			
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"			
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"			
< 760	< 655	H10-2	6-10d	6-10d			
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"			
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"			
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"			
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"			
< 2900	< 2490	2 - HTS24					
< 2050	< 1785	LGT2	14 -16d	14 -16d			
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION		
		HEAVY GIRDER HEDOWNS			TO FOUNDATION		
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT		
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT		
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT		
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT		
		STUD STRAP CONNECTOR*			TO STUDS		
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d		
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d		
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d		
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d		
< 885	< 760	SP4			6-10d, 1 1/2"		
< 1240	< 1065	SPH4			10-10d, 1 1/2"		
< 885	< 760	SP6			6-10d, 1 1/2"		
< 1240	< 1065	SPH6			10-10d, 1 1/2"		
< 1235	< 1165	LSTA18	14-10d				
< 1235	< 1235	LSTA21	16-10d				
< 1030	< 1030	CS20	18-8d				
< 1705	< 1705	CS16	28-8d				
		STUD ANCHORS*	TO STUDS		TO FOUNDATION		
< 1350	< 1305	LTT19	8-16d		1/2" AB		
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB		
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB		
< 4175	< 3695	HTT16	18 - 16d		5/8" AB		
< 1400	< 1400	PAHD42	16-16d				
< 3335	< 3335	HPAHD22	16-16d				
10.00000000	< 2200	ABU44	12-16d		1/2" AB		
< 2200			L TANKEN U		20000000		
< 2200 < 2300	< 2300	ABU66	12-16d		1/2" AB		

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' WALL HEIGHT

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REG LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

WIND EXPOSURE = B
 WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

5 | 21.8 | -29.1 | 18.5 | -22.6 |

Doors & Windows | 21.8 | -29.1 |
Worst Case (Zone 5, 10 ft2) |
8x7 Garage Door | 19.5 | -22.9 |
16x7 Garage Door | 18.5 | -21.0 |

Zone Effective Wind Area (ft2)

19.9 -21.8 | 18.1 | -18.1

2 19.9 -25.5 18.1 -21.8

3 O'hg -68.3 -42.4

4 21.8 -23.6 18.5 -20.4

2 O'hg -40.6

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

WINDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

REVISIONS

SOFTPIAN

32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

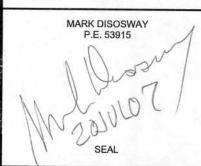
Do not proceed without clarification.

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its common law copyrights and property right in
these instruments of service. This document is
not to be reproduced, altered or copied in any
form or manner without first the express written

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my

permission and consent of Mark Disosway.

LIMITATION: This design is valid for one building, at specified location.



John Thomas

Bob Hubbel Addition

ADDRESS: 350 Drew Feagle Rd. Lake City, Florida 32055

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419

PRINTED DATE:
July 20, 2007

DRAWN BY: CHECKED BY:
David Disosway

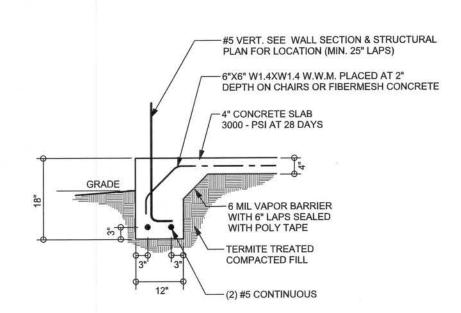
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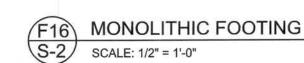
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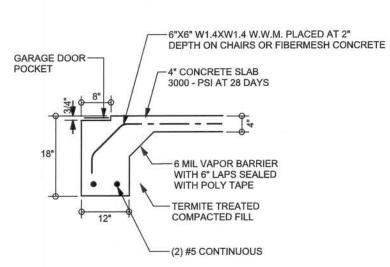
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DRAWING NUMBER

S-1

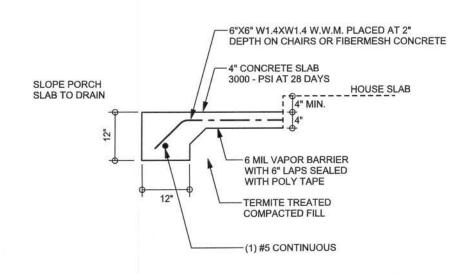
OF 2 SHEETS







GARAGE DOOR FOOTING SCALE: 1/2" = 1'-0"

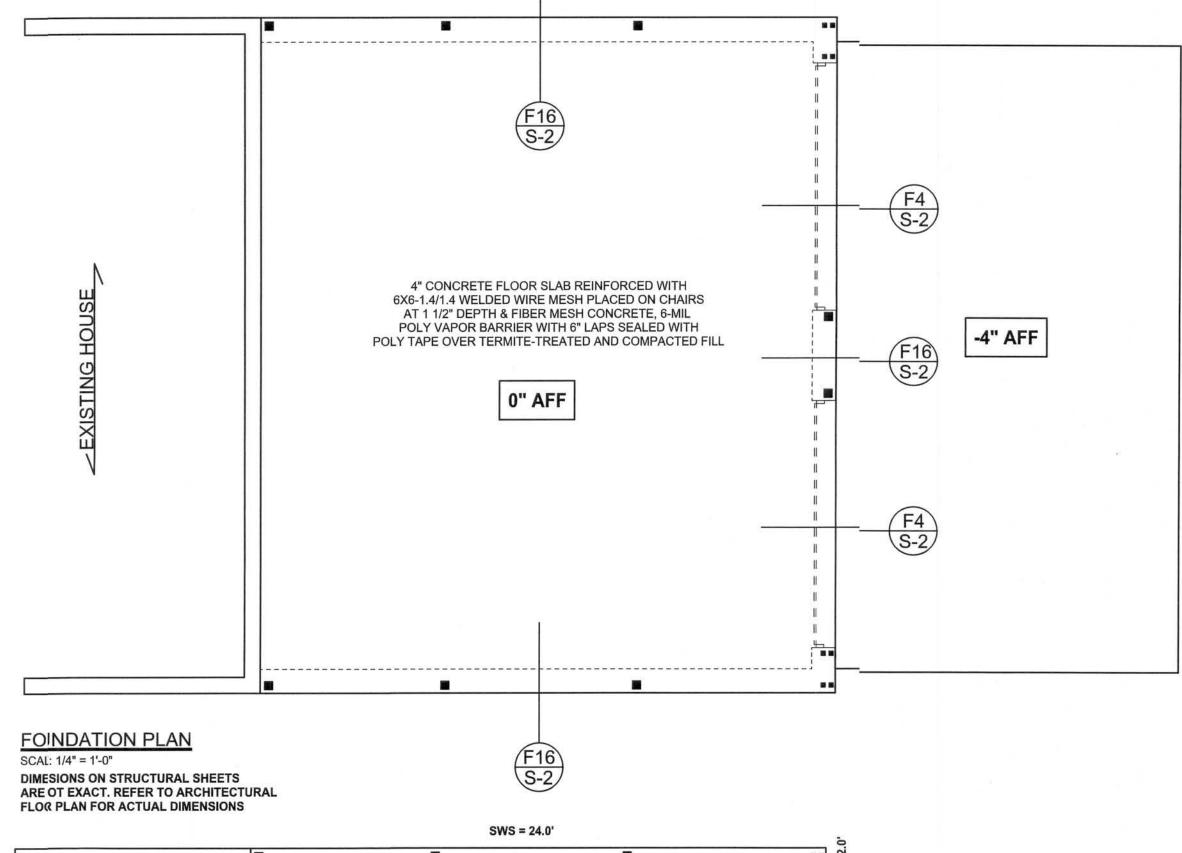


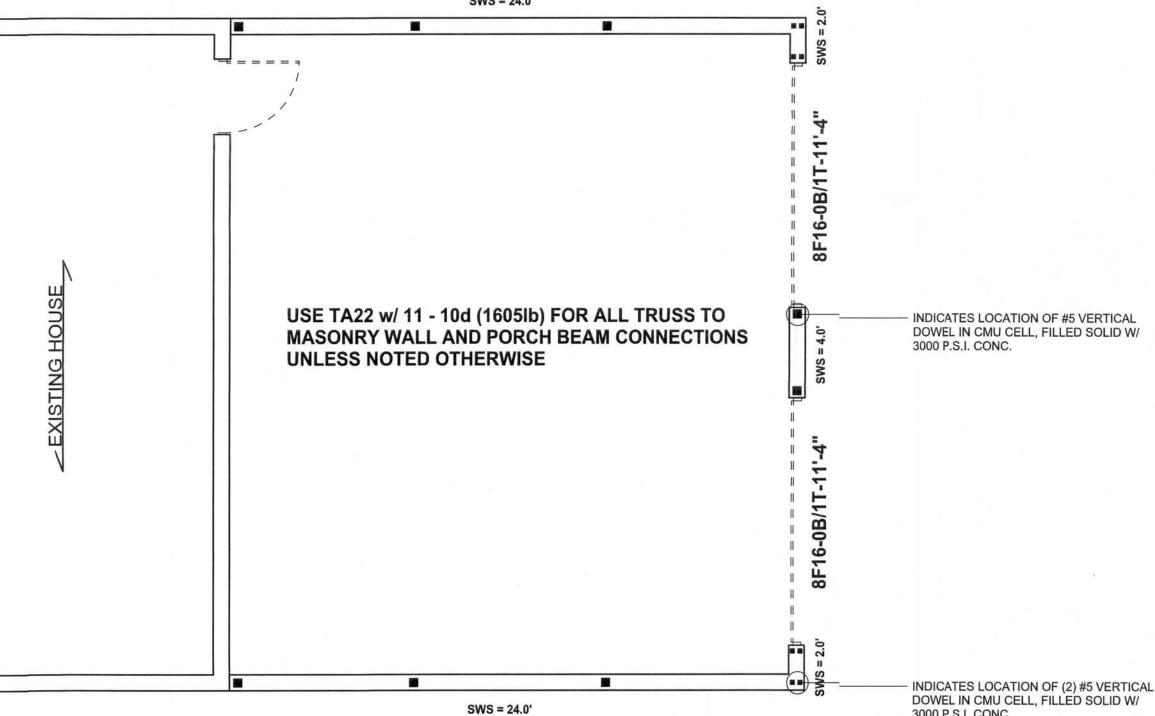


MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.





STRICTURAL PLAN SCALE1/4" = 1'-0"

STRICTURAL PLAN NOTES

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

ALL LOAD BEARING FRAME WALL HEADERS SN-: SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT 3" MIN. SPACING

WINDOWS & DOORS UP TO 6'X8' 3/16" TAPCONS @ 2' O.C.

1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12' 3/16" TAPCONS @ 16" O.C. 1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'HX20'W 3/16" TAPCONS @ 12" O.C. 1/4" TAPCONS @ 18" O.C.

(2) 1/4" TAPCONS & 12" O.C.

GARAGE DOOR UP TO 10'W (2) 3/16" TAPCONS & 16" O.C. (2) 1/4" TAPCONS & 24" O.C. GARAGE DOOR UP TO 18'W (2) 3/16" TAPCONS & 8" O.C.

INDICATES LOCATION OF #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/

3000 P.S.I. CONC.

3000 P.S.I. CONC.

INDICATES LOCATION OF (2) #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	8.0'	8.0'
LONGITUDINAL	10.0'	48.0'

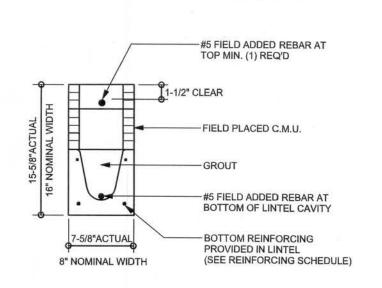
MATERIALS

ACI 318 and ACI 530

TYPE DESIGNATION

F = FILLED WITH GROUT / U = UNFILLED / S = SOLID OUANTITY OF #5 FIELD ADDED REBAR AT BOTTOM OF LINTEL CAVITY

NOMINAL WIDTH--QUANTITY OF #5 FIELD ADDED REBAR AT TOP



SAFE LOAD TABLE NOTES 1. All values based on minimum 4 inch nominal bearing. Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches. 2. N.R. = Not Rated Safe loads are superimposed allowable loads. 4. Safe loads based on grade 40 or grade 60 field rebar. 5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only

7. For composite lintel heights not shown, use safe load from next lower height shown. 8. For lintels lengths not shown, use safe load from next longest length shown 9. All safe loads in units of pounds per linear 10. All safe loads based on simply supported 11. The number in the the parenthesis

6. The designer may evaluate concentrated

calculating the maximum resisting moment

and shear at d-away from face of support.

loads from the safe load tables by

reduction 6472 ⇒ (.85) = 5501 plf

indicates the percent reduction for grade 40

gravity load = 6472\H0.0469;(15)\H0.0781; w/ 15%

Example 7'-6" lintel type 8F32-1B safe

field added rebar.

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS

1. fc 8" precast lintel = 3500 psi 2. fc prestressed lintel = 6000 psi 3. Grout per ASTM C476 fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump 4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi 5. Rebar per ASTM A615 grade 60 6. Prestressing strand per ASTM A416 grade 270 low relaxation 7. Mortar per ASTM C270 type M or S

in lieu of concrete masonry units.

GENERAL NOTES 1. Provide full mortar bed and head joints. Shore filled lintels as required. 3. Installation of lintel must comply with the architectural and/or structural documents. 4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting. 5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection. 6. Bottom field added rebar to be located at the bottom of the lintel cavity. 7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage. 8. Cast-in-place concrete may be provided in composite lintel

9. Safe load rating based on rational design analysis per

Product Approvals: Miami-Dade County, Florida No. 03-0606.05 11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating. 12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

Applied vertical load
Safe vertical load

+ Applied horizontal load
Safe horizontal load 13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

N.R.

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

SAFE LOAD - POUNDS PER LINEAR FOOT

8F8-0B 8F12-0B 8F16-0B 8F20-0B 8F24-0B 8F28-0B 8F32-0B
 8F8-1B
 8F12-1B
 8F16-1B
 8F20-1B
 8F24-1B
 8F28-1B
 8F32-1B

 3069
 4605
 6113
 7547
 8974
 10394
 11809

3069 4605 6113 7547 8974 10394 11809

3069 3719 5163 6607 8054 9502 10951

3069 4605 6113 7547 8974 10394 11809

 2561
 2751
 3820
 4890
 5961
 7034
 8107

 2693
 4605
 6113
 7547
 8974
 10394
 11809

 1969
 2110
 2931
 3753
 4576
 5400
 6224

 2189
 4375
 6113
 7547 (7)
 8672
 10294
 11809

 1349
 1438
 1999
 2560
 3123
 3686
 4249

 1663
 2000
 5365
 7547
 7042
 2020
 2020

 1663
 3090
 5365
 7547(36)
 7342(19)
 8733(19)
 10127(10)

 1105
 1173
 1631
 2090
 2549
 3009
 3470

1238 2177 3480 5381 8360 10394₍₃₇₎ 8825₍₁

 752
 1245
 1843
 2564
 3486
 4705(37)
 6390(47)

 535
 890
 1247
 2093
 2777
 2163
 2536

582 945 1366 1846 2423 3127 4006

 540
 873
 1254
 1684
 2193
 2805
 3552

 471
 755
 1075
 1428
 1838
 2316
 2883

471 755 1075 1428 1838 2316 2883

424 706 1002 1326 1697 2127 2630

 442
 706
 1002
 1326
 1697
 2127
 2630

 NR
 NR
 NR
 NR
 NR
 NR
 NR

 458
 783
 1370
 1902
 2245
 2517
 2712

 NR
 NR
 NR
 NR
 NR
 NR

 412
 710
 1250
 1733
 2058
 2320
 2513

 NR
 NR
 NR
 NR
 NR
 NR
 NR

 300
 536
 950
 1326
 1609
 1849
 2047

 NR
 NR
 NR
 NR
 NR
 NR

235 418 750 1037 1282 1515 1716

873 1254 1684 2193 2805 3552

2622 4360 7168 (45) 6036 (19) 7181 (19) 8328 (20 2177 3480 3031 3707 4383 5061

1729 2661 3898 5681 8467(44) 6472 (15

2'-10" (34") PRECAST

3'-6" (42") PRECAST

4'-0" (48") PRECAST

4'-6" (54") PRECAST

5'-4" (64") PRECAST

5'-10" (70") PRECAST

6'-6" (78") PRECAST

7'-6" (90") PRECAST

9'-4" (112") PRECAST

10'-6" (126") PRECAST

11'-4" (136") PRECAST

12'-0" (144") PRECAST

13'-4" (160") PRECAST

14'-0" (168") PRECAST

14'-8" (176") PRESTRESSED

15'-4" (184") PRESTRESSED

17'-4" (208") PRESTRESSED

19'-4" (232") PRESTRESSED

21'-4" (256") PRESTRESSED

22'-0" (264") PRESTRESSED

24'-0" (288") PRESTRESSED

<	(Caeti		SAFE LOAD - POUNDS PER LINEAR FOOT							
	=	TYPE	8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-1T	
LENG	TH		8F8-2T	8F12-2T	8F16-2T	8F20-2T	8F24-2T	8F28-2T	8F32-2T	
21 401	(2.411)	DDECAGE	1972	3173	4460	5747	7034	8321	9608	
2'-10"	(34")	PRECAST	1972	3173	4460	5747	7034	8321	9608	
3'-6"	(4011)	DDECAGE	1569	2524	3547	4569	5591	6613	7636	
3-0	(42")	PRECAST	1569	2524	3547	4569	5591	6613	7636	
4'-0"	(48")	PRECAST	1363	2192	3079	3966	4853	5740	6627	
	(40)	TRECAST	1363	2192	3079	3966	4853	5740	6627	
4'-6" (54")	PRECAST	1207	1940	2724	3508	4292	5077	5861		
	(0.)	THEORET	1207	1940	2724	3508	4292	5077	5861	
5'-4"	(64")	PRECAST	1016	1632	2290	2949	3607	4265	4924	
J-4	(04)	FRECASI	1016	1632	2290	2949	3607	4265	4924	
5'-10"	(70")	PRECAST	909	1492	2093	2694	3295	3897	4498	
J 10	(, 0)	TILOAGI	929	1492	2093	2694	3295	3897	4498	
6'-6"	(78")	PRECAST	835 (12)	1340	1880	2419	2959	3498	4038	
	(,	11120/101	835	1340	1880	2419	2959	3498	4038	
7'-6"	(90")	PRECAST	727 (23)	1021	1634 (12)	2102 (11)	2571(10)	3039 (10)	3508	
	(00)	TREOAGT	727	1166	1634	2102	2571	3039	3508	
9'-4"	4" (112") PRE	PRECAST	591	680	1133 (15)	1471 (15)	1811 (15)	2152 (16)	2494 (
			591	851	1326	1705	2084	2463	2842	
10'-6"	(126")	PRECAST	530	552	914 (15)	1185 (15)	1458 (15)	1732 (15)	2007 (
and the same of th	()		530	686	1183	1526	1865	2204	2544	
11'-4"	(136")	PRECAST	474	485	798 (15)	1034 (15)	1272 (15)	1510 (15)	1749 (1	
one see	(,	11120/101	494	599	1028	1422	1738	2053	2369	
12'-0"	(144")	PRECAST	470 (9)	441	723 (14)	936 (14)	1151(15)	1366 (15)	1582 (1	
	(,	11120/101	470	543	928	1349	1649	1948	2247	
13'-4"	(160")	PRECAST	418 (15)	373	606 (14)	783 (14)	962 (14)	1141 (14)	1321 (1	
	1.00/	11120/101	428	455	770	1145	1444	1718	1993	
14'-0"	(168")	PRECAST	384 (15)	346	559 (14)	723 (14)	887 (14)	1052 (14)	1218 (1	
101000	()	111201101	410	420	709	1050	1434 (8)	1694 (8)	1954	
14'-8"	(176")	PRESTRESSE	239	323	519 (13)	671 (13)	823 (13)	976 (14)	1129 (1	
	()	2 (246	390	655	968	1324 (8)	1625 (11)	1874 (1	
15'-4"	(184")	PRESTRESSE	224	302	485 (13)	626 (13)	767 (13)	909 (13)	1052 (1	
	(101)		230	364	609	897	1224 (8)	1562 (14)	1801 (1	
17'-4"	(208")	PRESTRESSE	187	255	404 (12)	520 (12)	637 (12)	754 (12)	872 (1	
12072 751			192	303	500	732	993 (8)	1268 (14)	1470 (1	
19'-4"	(232")	PRESTRESSE	162	222	347 (11)	446 (11)	546 (12)	646 (12)	746 (1	
	Message Control		166	261	424	616	831 (8)	1057 (14)	1225 (1	
21'-4"	(256")	PRESTRESSE	142	198	306 (11)	393 (11)	480 (11)	567 (11)	654 (1	
		os cens especialista de la Califa I	142	230	369	531	713 (7)	903 (13)	1046 (1	
22'-0"	(264")	PRESTRESSE		192	295 (10)	378 (11)	461 (10)	545 (11)	629 (1	
			137	221	354	508	681 (7)	861 (13)	997 (1	
24'-0"	(288")	PRESTRESSE	124	175	267 (10)	341 (10)	416 (10)	491 (10)	566 (1	
A WAY COME	Maria Company		124	200	316	450	600 (7)	756 (12)	875 (1:	

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

(্ৰৈ কেন্দ্ৰেৰ	SAFE LOAD - POUNDS PER LINEAR FOOT								
		TYPE	ODLIC	8RF6-0B	8RF10-0B	8RF14-0B	8RF18-0B	8RF22-0B	8RF26-0B	8RF30-0B	
LENG	TH		8RU6	8RF6-1B	8RF10-1B	8RF14-1B	8RF18-1B	8RF22-1B	8RF26-1B	8RF30-1B	
4'-4" (52")	PRECAST	1635	1749	3355	3280	4349	5421	6493	7567		
	FRECASI	1033	1891	3699	5206	6639	8060	9479	10893		
4'-6" (54")	PRECAST	1494	1596	3063	2992	3968	4946	5924	6904		
	-0 (04) FRECAST	1434	1756	3699	5206	6639	8060	9479	10893		
5'-8"	(68")	PRECAST	866	920	1770	1716	2277	2839	3402	3966	
0 0 (00)	(00)	TILONOT	000	1167	2481	4567	6389	8060 (34)	7917 (19)	9311 (19	
5'-10" (70")	(70")	PRECAST	810	859	1653	1600	2124	2649	3174	3700	
2.02			010	1113	2342	4242	6639 (10)	8060 (39)	7402 (19)	8706 (19	
6'-8"	(80")) PRECAST	797	901	1825	3120	5048	7747	9448	7360	
	(00)		191	901	1825	3120	5048	7915	9479	10893 (32	
7'-6"	(90"))") PRECAST	669	755	1490	2459	3776	5743	7239	5623	
1 0 (00)	(00)		009	755	1490	2459	3776	5743	8998 (19)	10893 (48	
9'-8" (116")	16") PRECAST	411	466	999	1568	2253	3129	4091	3146		
	12350/		711	526	999	1568	2253	3129	4150	5891 (47	

SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

CASTI-CASTIG			SAFE LOAD - POUNDS PER LINEAR FOOT							
		TYPE	8RF6-1T	8RF10-1T	8RF14-1T	8RF18-1T	8RF22-1T	8RF26-1T	8RF30-1T	
LENGTH			8RF6-2T	8RF10-2T	8RF14-2T	8RF18-2T	8RF22-2T	8RF26-2T	8RF30-2T	
4'-4"	(52")	DDECAST	905	1748	2635	3522	4409	5296	6183	
4-4 (52)	PRECAST	905	1748	2635	3522	4409	5296	6183		
4'-6" (54")	(5A")	PRECAST	867	1675	2525	3374	4224	5074	5924	
	(54)		867	1675	2525	3374	4224	5074	5924	
5'-8" (68")	(68")	PRECAST	675	1301	1960	2618	3277	3935	4594	
	(00)		675	1301	1960	2618	3277	3935	4594	
5'-10" (70")	(70")	PRECAST	655	1262	1900	2538	3176	3815	4453	
	(10)		655	1262	1900	2538	3176	3815	4453	
6'-8"	(80")	PRECAST	570	1012	1651	2204	2758	3312	3865	
0-0	(00)		570	1097	1651	2204	2758	3312	3865	
7'-6"	(90")	") PRECAST	506	797	1462 (8)	1952 (7)	2442 (6)	2931 (6)	3257	
1-0	(30)	FRECASI	506	967	1462	1952	2442	2931	3421	
9'-8" (116	(116")	PRECAST	395	491	931 (12	1301 (15)	1640 (15)	1980 (15)	2322 (16	
0 0	(110)	FREUAS I	395	589	1135	1514	1893	2272	2652	

REVISIONS

SOFTPLAN

PE No.53915, OB 868, Lake City, FL Stated dimensins supercede scaled dimensions. Rifer all questions to Mark Disosway P.E. for resolution. Do not procee without clarification. COPYRIGHTSAND PROPERTY RIGHTS: Mark Disosway P.E. hereby expressly reserve its common lay copyrights and property right in these instrumets of service. This document is not to be reproluced, altered or copied in any form or manne without first the express written permission ancconsent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this lan, and that the applicable portions of the lan, relating to wind engineer comply with setion R301.2.1, florida building code residentia 2004, to the best of my LIMITATION: his design is valid for one building, at speified location. MARK DISOSWAY P.E. 53915

WINDLOAD EIGINEER: Mark Disosway,

John Thomas

Fob Hubbel Addition

ADDRESS: 35(Drew Feagle Rd. LakeCity, Florida 32055

MarkDisosway P.E. PO. Box 868 Lake Cty, Florida 32056 Phone:(386) 754 - 5419 Fax: (386) 269 - 4871

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FINALS DATE: 20 / Jul / (7

JOB NUMBER: 707201 DRAWING NUMBER

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OF 2 SHEETS