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Alpine, an ITW Company
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)755-6001
 www.alpineitw.com

COA #0 278

Florida Certificate of Product Approval #FL 1999
 04/04/2022



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-7193
Job Description: Poirrier Res	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.00 through 21.02.01 JRef #: 1Xec2150005
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 86 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss
1	094.22.1013.27603	A01
3	094.22.1013.38783	A03
5	094.22.1014.16823	A05
7	094.22.1014.32663	A07
9	094.22.1014.45720	A09
11	094.22.1015.16133	A11
13	094.22.1016.22050	A13
15	094.22.1016.34950	A15
17	094.22.1017.34427	A17
19	094.22.1018.07230	A19
21	094.22.1018.51790	A21
23	094.22.1019.42800	A23
25	094.22.1020.57267	B02
27	094.22.1021.13410	B04
29	094.22.1021.19773	C02
31	094.22.1021.29237	C04
33	094.22.1021.38087	C06
35	094.22.1021.44487	C08
37	094.22.1021.59720	C10
39	094.22.1022.10507	D01
41	094.22.1031.13867	J01
43	094.22.1031.23457	J02
45	094.22.1031.27057	J03
47	094.22.1027.22896	J04
49	094.22.1031.32923	J05

Item	Drawing Number	Truss
2	094.22.1013.34437	A02
4	094.22.1014.10850	A04
6	094.22.1014.28373	A06
8	094.22.1014.37723	A08
10	094.22.1015.11080	A10
12	094.22.1016.05937	A12
14	094.22.1016.27823	A14
16	094.22.1017.01100	A16
18	094.22.1017.58920	A18
20	094.22.1018.11213	A20
22	094.22.1019.16200	A22
24	094.22.1019.54820	B01
26	094.22.1021.05977	B03
28	094.22.1021.15690	C01
30	094.22.1021.22650	C03
32	094.22.1021.33427	C05
34	094.22.1021.42403	C07
36	094.22.1021.53390	C09
38	094.22.1032.02350	C11
40	094.22.1031.11863	D02
42	094.22.1031.21643	J01HJ
44	094.22.1031.25373	J02HJ
46	094.22.1031.29133	J03HJ
48	094.22.1031.31130	J04HJ
50	094.22.1031.35300	J05HJ

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Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 22-7193
Job Description: Poirrier Res	
Address:	

Item	Drawing Number	Truss
51	094.22.1022.26593	J06
53	094.22.1022.32520	J07
55	094.22.1023.48043	J08
57	094.22.1023.51737	J10
59	094.22.1023.55773	J12
61	094.22.1025.10190	J14
63	094.22.1025.14513	J16
65	094.22.1025.20203	J18
67	094.22.1025.23500	J20
69	094.22.1025.46227	J22
71	094.22.1031.00063	J24
73	094.22.1026.09580	J26
75	094.22.1026.13850	J28
77	094.22.1026.18727	PB01
79	094.22.1026.24353	PB03
81	094.22.1026.29043	PB05
83	094.22.1026.33893	PB07
85	094.22.1030.37643	V02
87	BRCLBSUB0119	
89	A14030ENC160118	
91	PB160160118	
93	VALTN160118	

Item	Drawing Number	Truss
52	094.22.1022.29727	J06HJ
54	094.22.1023.46410	J07HJ
56	094.22.1023.49823	J09
58	094.22.1023.53850	J11
60	094.22.1024.00523	J13
62	094.22.1025.12457	J15
64	094.22.1025.16650	J17
66	094.22.1025.22090	J19
68	094.22.1025.25370	J21
70	094.22.1025.59987	J23
72	094.22.1026.07727	J25
74	094.22.1026.11747	J27
76	094.22.1026.15810	J29
78	094.22.1026.21563	PB02
80	094.22.1026.26663	PB04
82	094.22.1026.31273	PB06
84	094.22.1026.39897	V01
86	094.22.1030.43323	V03
88	CNNAILSP1014	
90	GBLLETIN0118	
92	VAL180160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

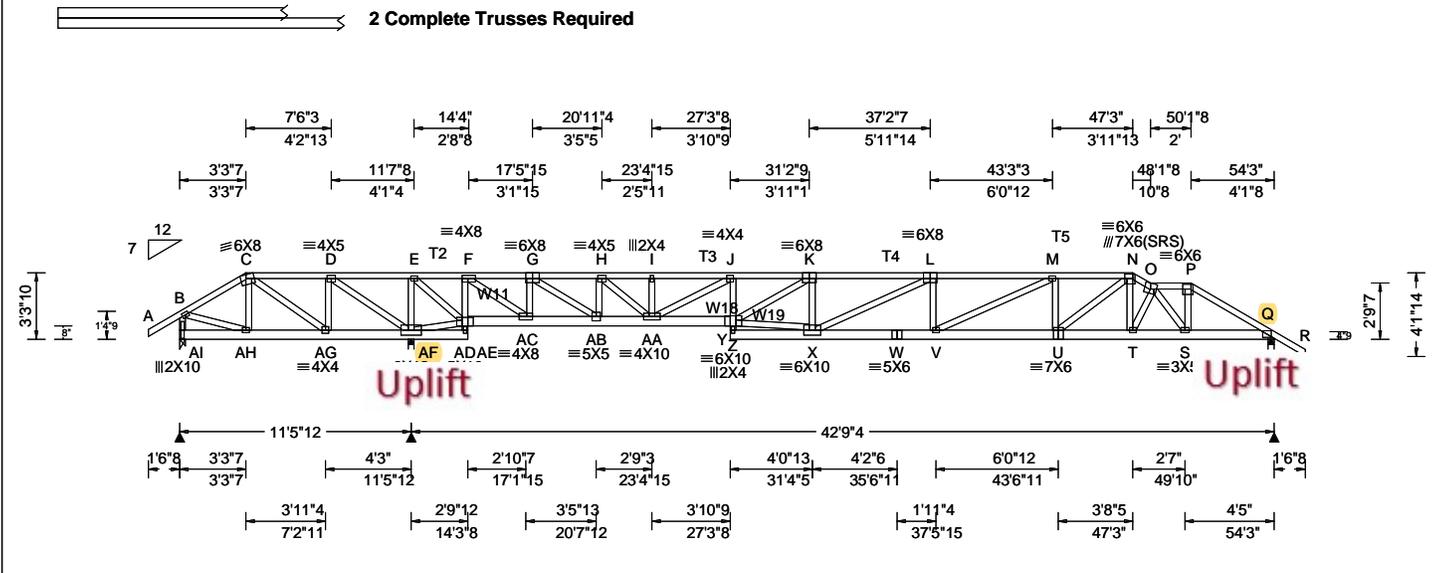
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.



Loading Criteria (psf)

TCLL: 20.00
 TCDL: 10.00
 BCLL: 0.00
 BCDL: 10.00
 Des Ld: 40.00
 NCBCLL: 0.00
 Soffit: 2.00
 Load Duration: 1.25
 Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.00 ft
 TCDL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: 0 to h/2
 C&C Dist a: 5.42 ft
 Loc. from endwall: not in 6.50 ft
 GCp: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: No
 FT/RT:20(0)/10(0)
 Plate Type(s):
 WAVE

Defl/CSI Criteria

PP Deflection in loc L/def L/#
 VERT(LL): 0.315 K 999 240
 VERT(CL): 0.633 K 807 180
 HORZ(LL): 0.029 Q - -
 HORZ(TL): 0.059 Q - -
 Creep Factor: 2.0
 Max TC CSI: 0.465
 Max BC CSI: 0.263
 Max Web CSI: 0.851

VIEW Ver: 21.02.00.1005.17

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
AI	-	-1386	-	/237	-	-
AF	6671	-	-	-	/1420	-
Q	2763	-	-	-	/700	-

Wind reactions based on MWFRS
 AI Brg Wid = 3.5 Min Req = 1.5 (Truss)
 AF Brg Wid = 3.5 Min Req = 2.4 (Truss)
 Q Brg Wid = 3.5 Min Req = 1.5 (Truss)
 Bearings AI, AF, & Q are a rigid surface.
 Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	987 -207	J - K	802 -3680
C - D	2206 -461	K - L	811 -3594
D - E	3980 -860	L - M	922 -3973
E - F	3250 -723	M - N	805 -3377
F - G	1153 -295	N - O	711 -2908
G - H	123 -805	O - P	517 -2071
H - I	429 -2163	P - Q	574 -2285
I - J	429 -2163		

Lumber

Top chord: 2x4 SP #2; T2,T3,T4,T5 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W11,W18,W19 2x4 SP #2;

Nailnote

Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

All plates are 3X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -1386# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



COA #0278

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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
AH-AG	170 -843	X - W	3984 -931
AG-AF	478 -2311	W - V	3984 -931
AD-AC	711 -3195	V - U	3428 -822
AC-AB	254 -966	U - T	2541 -622
AB-AA	964 -160	T - S	2724 -670
AA - Y	3696 -806	S - Q	1947 -486

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - AI	706 -112	H - AA	1625 -364
B - AH	176 -874	AA - J	437 -1772
C - AG	361 -1696	J - Y	725 -145
AG - D	1067 -176	Y - X	3387 -767
D - AF	475 -2141	X - L	134 -437
AF - E	197 -780	V - M	611 -112
AF - AD	835 -3874	M - U	175 -518
E - AD	919 -167	U - N	1064 -233
AD - F	371 -1715	N - T	619 -137
F - AC	2583 -526	T - O	140 -532
AC - G	324 -1458	O - S	264 -1127
AG - AB	2170 -463	S - P	1023 -255

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



Special Loads

AB- H 288 - 1239

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

- TC: From 63 plf at -1.54 to 63 plf at 3.29
- TC: From 32 plf at 3.29 to 32 plf at 52.31
- TC: From 63 plf at 52.31 to 63 plf at 55.79
- BC: From 5 plf at -1.54 to 5 plf at 0.00
- BC: From 20 plf at 0.00 to 20 plf at 3.32
- BC: From 10 plf at 3.32 to 10 plf at 54.25
- BC: From 5 plf at 54.25 to 5 plf at 55.79
- TC: 207 lb Conc. Load at 3.32
- TC: 129 lb Conc. Load at 5.35, 7.35, 9.35, 11.35
- 13.35, 27.35, 29.35, 31.35, 33.35, 35.35, 37.35, 39.35
- 41.35, 43.35, 45.35
- TC: 144 lb Conc. Load at 15.35, 17.35, 19.35, 21.35
- 23.35, 25.35
- TC: 119 lb Conc. Load at 47.35
- TC: 75 lb Conc. Load at 49.35
- TC: 81 lb Conc. Load at 50.68
- BC: 121 lb Conc. Load at 3.32
- BC: 90 lb Conc. Load at 5.35, 7.35, 9.35, 11.35
- 13.35, 27.35, 29.35, 31.35, 33.35, 35.35, 37.35, 39.35
- 41.35, 43.35, 45.35, 47.35
- BC: 54 lb Conc. Load at 15.35, 17.35, 19.35, 21.35
- 23.35, 25.35
- BC: 207 lb Conc. Load at 49.35
- BC: 101 lb Conc. Load at 50.68
- BC: 145 lb Conc. Load at 52.31



COA #0218

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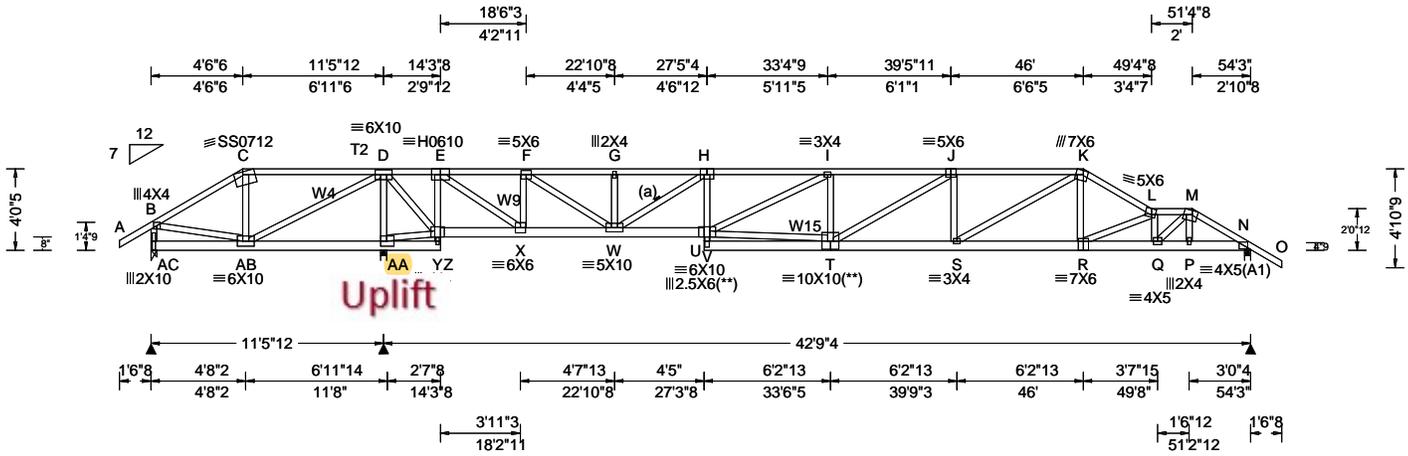
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 5.42 ft
Loc. from endwall: not in 6.50 ft
GCp: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA
Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, 18SS, HS

Defl/CSI Criteria
PP Deflection in loc L/defl L/#
VERT(LL): 0.291 I 999 240
VERT(CL): 0.599 I 850 180
HORZ(LL): -0.030 K - -
HORZ(TL): 0.061 K - -
Creep Factor: 2.0
Max TC CSI: 0.742
Max BC CSI: 0.233
Max Web CSI: 0.893
VIEW Ver: 21.02.01.1216.15

▲ Maximum Reactions (lbs)					
Gravity			Non-Gravity		
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
AC -	- / -905	- / -	/ 109	/ 379	/ 148
AA	3776	- / -	/ 1881	/ 690	- / -
N	1556	- / -	/ 904	/ 283	- / -
Wind reactions based on MWFRS					
AC Brg Wid = 3.5 Min Req = 1.5 (Truss)					
AA Brg Wid = 3.5 Min Req = 2.8 (Truss)					
N Brg Wid = 3.5 Min Req = 1.5 (Truss)					
Bearings AC, AA, & N are a rigid surface.					
Members not listed have forces less than 375#					
Maximum Top Chord Forces Per Ply (lbs)					
Chords	Tens.	Comp.	Chords	Tens.	Comp.

Lumber
 Top chord: 2x4 SP #2; T2 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W4,W9,W15 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 6X8 except as noted.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

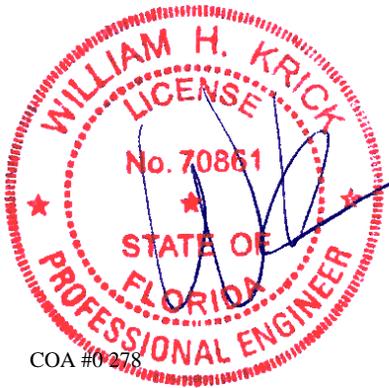
Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 Negative reaction(s) of -905# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1547	-451	H - I	1012	-2951
C - D	1346	-341	I - J	1183	-3303
D - E	3012	-936	J - K	1261	-3323
E - F	637	-185	K - L	1055	-2906
F - G	496	-1466	L - M	1102	-3117
G - H	496	-1466	M - N	834	-2423

Maximum Bot Chord Forces Per Ply (lbs)					
Chords	Tens.	Comp.	Chords	Tens.	Comp.
AB-AA	1261	-3650	S - R	2486	-791
Y - X	1080	-2948	R - Q	3316	-1085
X - W	292	-479	Q - P	2052	-640
W - U	2962	-854	P - N	2049	-642
T - S	3352	-1117			

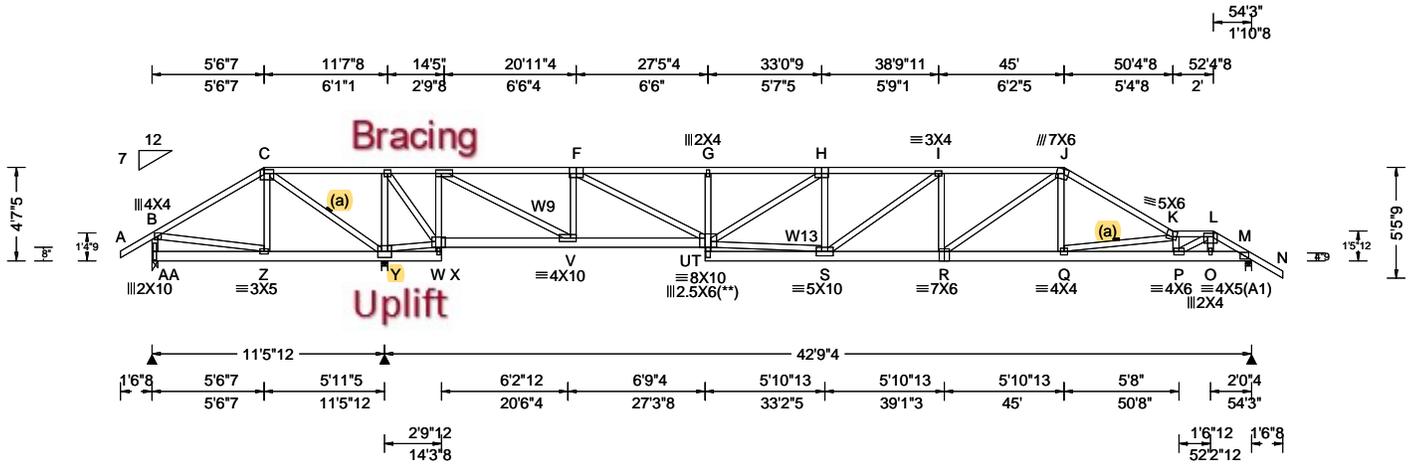
Maximum Web Forces Per Ply (lbs)					
Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - AC	950	-232	W - H	629	-1807
B - AB	462	-1331	H - U	674	-169
C - AB	413	-1017	U - I	196	-406
AB - D	2784	-885	U - T	3035	-939
D - AA	877	-2482	J - S	250	-375
D - Y	1104	-287	S - K	968	-355
AA - Y	1296	-3738	K - R	600	-123
Y - E	632	-1942	R - L	336	-926
E - X	2901	-917	L - Q	441	-1157
X - F	587	-1623	Q - M	1492	-518
F - W	2343	-754			



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.42 ft Loc. from endwall: not in 6.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.272 H 999 240 VERT(CL): 0.558 H 916 180 HORZ(LL): 0.032 E - - HORZ(TL): 0.065 E - - Creep Factor: 2.0 Max TC CSI: 0.886 Max BC CSI: 0.372 Max Web CSI: 0.896 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity AA - /-942 /- /93 /- /52 Y 3820 /- /- /- /639 /- M 1595 /- /- /- /287 /- Wind reactions based on MWFRS AA Brg Wid = 3.5 Min Req = 1.5 (Truss) Y Brg Wid = 3.5 Min Req = 2.8 (Truss) M Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings AA, Y, & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W9,W13 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 55.79
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 52.31
BC: From 10 plf at 52.31 to 10 plf at 54.25
BC: From 5 plf at 54.25 to 5 plf at 55.79
TC: 37 lb Conc. Load at 52.31
TC: -2 lb Conc. Load at 52.37
BC: 33 lb Conc. Load at 52.31
BC: -10 lb Conc. Load at 52.37

Wind
Wind loads and reactions based on MWFRS.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -942# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	1706 -297	H - I	460 -2843
C - D	3265 -548	I - J	476 -2916
D - E	2466 -410	J - K	473 -2803
E - F	70 -524	K - L	584 -3706
F - G	434 -2605	L - M	390 -2455
G - H	432 -2584		

Chords	Tens.Comp.	Chords	Tens. Comp.
Z - Y	225 -1448	R - Q	2360 -391
W - V	379 -2408	Q - P	4042 -661
V - T	645 -118	P - O	2093 -331
S - R	2937 -491	O - M	2077 -331

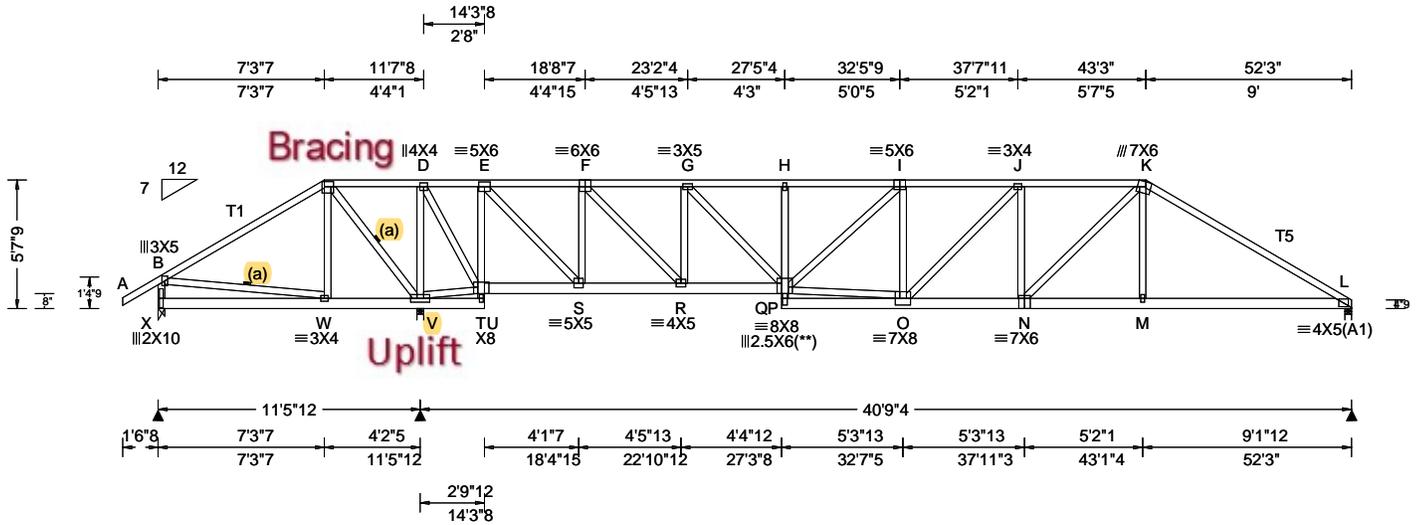
Webs	Tens.Comp.	Webs	Tens. Comp.
B -AA	991 -72	V - F	343 -1476
B - Z	273 -1471	F - T	2218 -380
C - Z	394 0	G - T	143 -377
C - Y	388 -2389	T - S	2679 -444
Y - D	303 -1375	R - J	684 -113
Y - W	525 -3193	J - Q	583 -14
D - W	1330 -218	Q - K	276 -1717
W - E	361 -1944	K - P	272 -1301
E - V	3341 -537	P - L	1914 -309



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 5.22 ft Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.148 I 999 240 VERT(CL): 0.306 I 999 180 HORZ(LL): 0.024 E - - HORZ(TL): 0.050 E - - Creep Factor: 2.0 Max TC CSI: 0.676 Max BC CSI: 0.234 Max Web CSI: 0.825 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity X 79 /-668 /- /99 /276 /176 V 3402 /- /- /1718 /625 /- L 1430 /- /- /841 /249 /- Wind reactions based on MWFRS X Brg Wid = 3.5 Min Req = 1.5 (Truss) V Brg Wid = 3.5 Min Req = 2.4 (Truss) L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings X, V, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1427 -457 H - I 785 -2174 C - D 2057 -578 I - J 859 -2274 D - E 1337 -368 J - K 884 -2244 F - G 479 -1307 K - L 761 -2285 G - H 787 -2184 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. W - V 493 -1197 O - N 2261 -704 T - S 544 -1290 N - M 1864 -533 R - P 1380 -328 M - L 1858 -534 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - X 730 -172 E - S 2114 -670 B - W 525 -1221 S - F 543 -1477 C - V 461 -1538 F - R 1598 -519 V - D 568 -1487 R - G 440 -1117 V - T 760 -2024 G - P 1169 -405 D - T 1416 -412 P - O 2166 -633 T - E 604 -1786 N - K 532 -250
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Lumber
Top chord: 2x4 SP #2; T1,T5 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

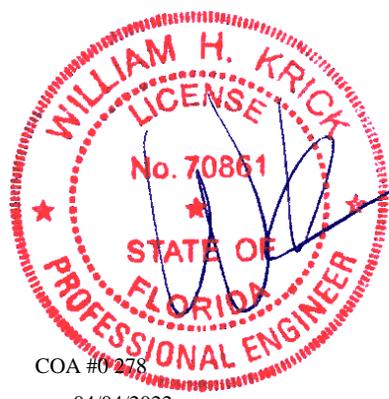
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

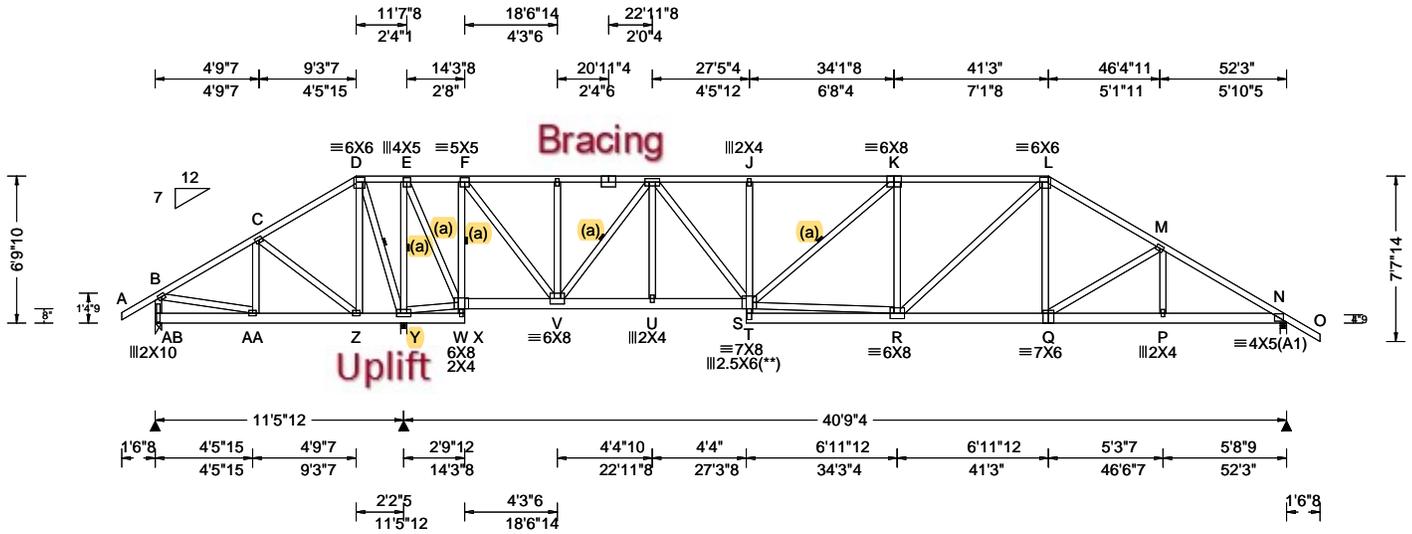
Additional Notes
Negative reaction(s) of -668# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 5.22 ft Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.121 K 999 240 VERT(CL): 0.251 K 999 180 HORZ(LL): 0.024 N - - HORZ(TL): 0.050 N - - Creep Factor: 2.0 Max TC CSI: 0.635 Max BC CSI: 0.133 Max Web CSI: 0.741 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL AB 78 /-641 /- /100 /262 /227 Y 3379 /- /- /1744 /624 /- N 1544 /- /- /961 /273 /- Non-Gravity AB Brg Wid = 3.5 Min Req = 1.5 (Truss) Y Brg Wid = 3.5 Min Req = 2.4 (Truss) N Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings AB, Y, & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

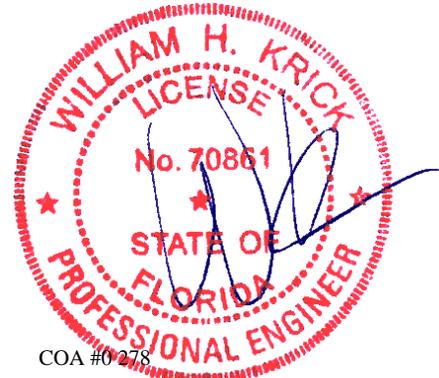
Plating Notes
All plates are 3X4 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
Negative reaction(s) of -641# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

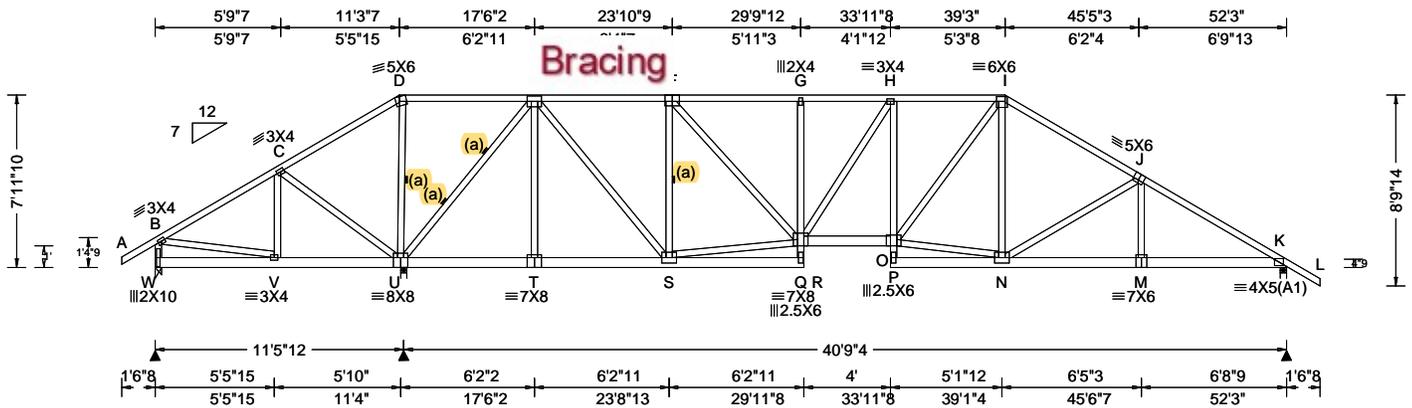
Maximum Bot Chord Forces Per Ply (lbs)	Maximum Web Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.	Webs Tens.Comp. Webs Tens. Comp.
AA- Z 421 -964 Z - Y 593 -1291 W - V 515 -999 V - U 1096 -179	U - S 1098 -179 V - Q 1676 -382 Q - P 1995 -475 P - N 1997 -474 W - F 638 -1768 F - V 1946 -617 V - I 494 -1497 I - S 1083 -375 S - R 1818 -463 L - Q 417 -20 Q - M 136 -379



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 5.22 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.121 G 999 240 VERT(CL): 0.251 G 999 180 HORZ(LL): 0.032 K - - HORZ(TL): 0.067 K - - Creep Factor: 2.0 Max TC CSI: 0.807 Max BC CSI: 0.143 Max Web CSI: 0.864 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity W 98 /-559 /- /104 /235 /260 U 3277 /- /- /1728 /598 /- K 1578 /- /- /995 /277 /- Wind reactions based on MWFRS W Brg Wid = 3.5 Min Req = 1.5 (Truss) U Brg Wid = 3.5 Min Req = 2.7 (Truss) K Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings W, U, & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber Top chord: 2x4 SP #2; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3;	▲ Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1100 -336 G - H 727 -1851 C - D 1557 -392 H - I 776 -1958 D - E 1292 -278 I - J 690 -1936 E - F 477 -1071 J - K 695 -2439 F - G 727 -1847
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Bracing (a) Continuous lateral restraint equally spaced on member.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - W 600 -173 S - F 469 -1205 B - V 377 -929 S - Q 1061 -188 C - U 236 -619 F - Q 1114 -355 D - U 340 -1038 O - N 1566 -307 U - E 741 -2199 O - I 600 -250 E - S 1492 -482 N - J 180 -519
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Plating Notes All plates are 6X8 except as noted.	
Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	
Wind Wind loads based on MWFRS with additional C&C member design. Left end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	

Additional Notes Negative reaction(s) of -559# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions. WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.	
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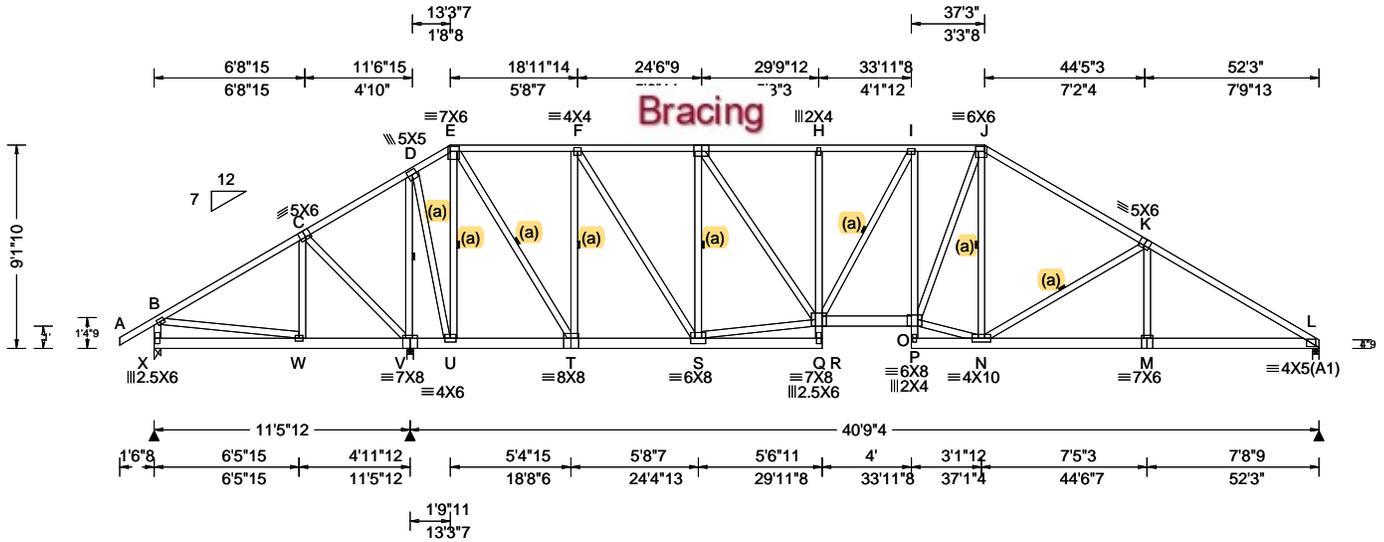


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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 5.22 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.140 H 999 240 VERT(CL): 0.290 H 999 180 HORZ(LL): 0.037 L - - HORZ(TL): 0.076 L - - Creep Factor: 2.0 Max TC CSI: 0.640 Max BC CSI: 0.195 Max Web CSI: 0.726 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity X 354 /-57 /- /192 /32 /276 V 2638 /- /- /1447 /123 /- L 1613 /- /- /993 /95 /- Wind reactions based on MWFRS X Brg Wid = 3.5 Min Req = 1.5 (Truss) V Brg Wid = 3.5 Min Req = 2.2 L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings X, V, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.

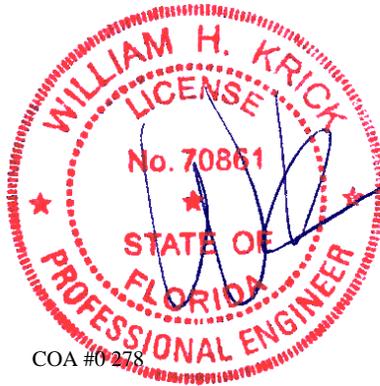
Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
V - U	307 -454	N - M	2246 -555
T - S	874 -171	M - L	2248 -554
Q - O	2002 -467		

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
C - V	173 -447	S - G	432 -1101
V - D	610 -2002	S - Q	1433 -307
D - U	1610 -474	G - Q	930 -303
E - U	558 -1702	O - N	1753 -373
E - T	1696 -550	O - J	799 -305
T - F	519 -1306	N - K	225 -649
F - S	1082 -362		



COA #0278

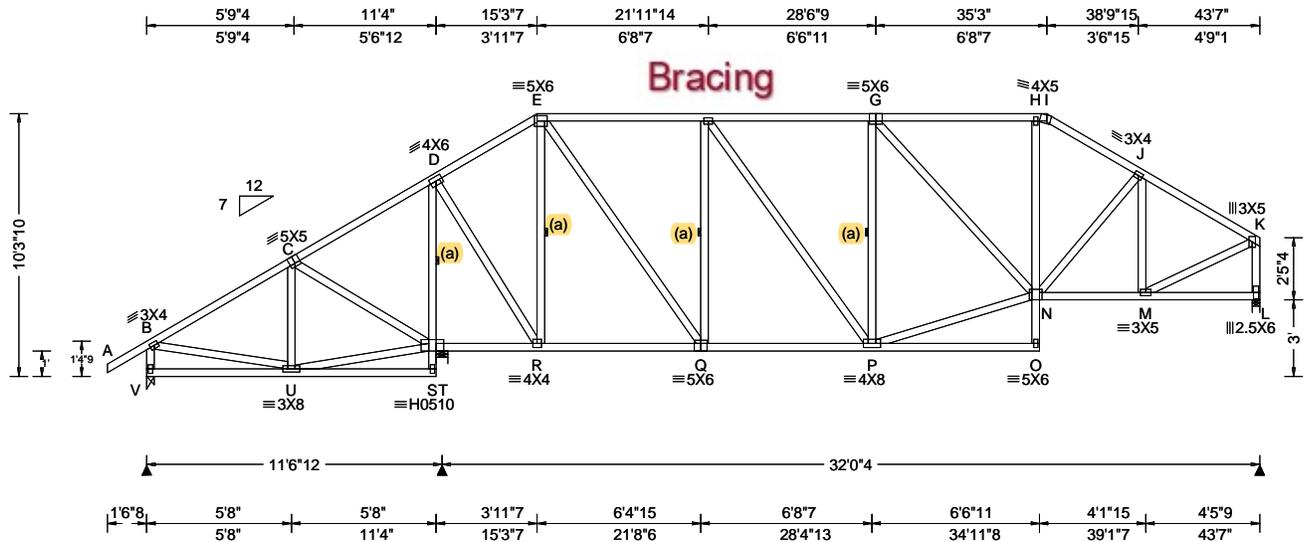
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.36 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.066 G 999 240 VERT(CL): 0.137 G 999 180 HORZ(LL): 0.017 L - - HORZ(TL): 0.035 L - - Creep Factor: 2.0 Max TC CSI: 0.553 Max BC CSI: 0.492 Max Web CSI: 0.815 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity V 477 /- /- /279 /- /255 S 1990 /- /- /1193 /206 /- L 1307 /- /- /746 /54 /- Wind reactions based on MWFRS V Brg Wid = 3.5 Min Req = 1.5 (Truss) S Brg Wid = 5.5 Min Req = 2.3 (Truss) L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings V, S, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 285 -551 H - I 536 -1220 E - F 480 -993 I - J 568 -1435 F - G 537 -1173 J - K 423 -1313 G - H 530 -1204 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. R - Q 434 -107 N - M 1095 -318 Q - P 1014 -314 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - V 99 -431 P - G 307 -473 C - S 176 -451 P - N 1230 -387 S - D 562 -1659 H - N 388 -90 D - R 1179 -344 J - M 209 -439 E - R 350 -886 M - K 1173 -334 E - Q 976 -351 K - L 401 -1270 Q - F 352 -664
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0278

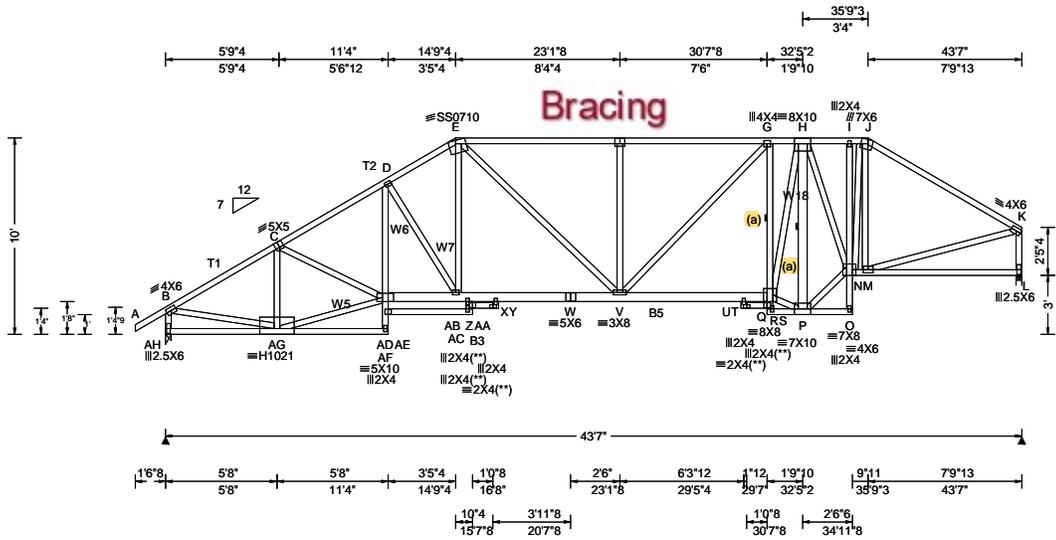
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)

TCLL: 20.00
 TCDL: 10.00
 BCLL: 0.00
 BCDL: 10.00
 Des Ld: 40.00
 NCBCLL: 10.00
 Soffit: 2.00
 Load Duration: 1.25
 Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 16.14 ft
 TCDL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: h to 2h
 C&C Dist a: 4.36 ft
 Loc. from endwall: not in 13.00 ft
 GCp: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: Yes
 FT/RT:20(0)/10(0)
 Plate Type(s):
 WAVE, HS, 18SS

Defl/CSI Criteria

PP Deflection in loc L/def L/#
 VERT(LL): 0.180 Y 999 240
 VERT(CL): 0.342 Y 999 180
 HORZ(LL): 0.073 L - -
 HORZ(TL): 0.140 L - -
 Creep Factor: 2.0
 Max TC CSI: 0.487
 Max BC CSI: 0.769
 Max Web CSI: 0.911
 VIEW Ver: 21.02.01.1216.15

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
AH	1951	-	-	/1169	-	/402
L	1830	-	-	/970	-	-

Wind reactions based on MWFRS
 AH Brg Wid = 3.5 Min Req = 2.3 (Truss)
 L Brg Wid = 3.5 Min Req = 2.2 (Truss)
 Bearings AH & L are a rigid surface.
 Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	147 -2538	G - H	10 -2126
C - D	202 -3130	H - I	66 -1969
D - E	164 -2724	I - J	68 -1978
E - F	0 -2574	J - K	26 -2223
F - G	0 -2574		

Lumber

Top chord: 2x4 SP M-31; T1,T2 2x4 SP #2;
 Bot chord: 2x4 SP #2; B3,B5 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W5 2x4 SP #2; W6,
 W7 2x4 SP M-31; W18 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.
 (**) 5 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

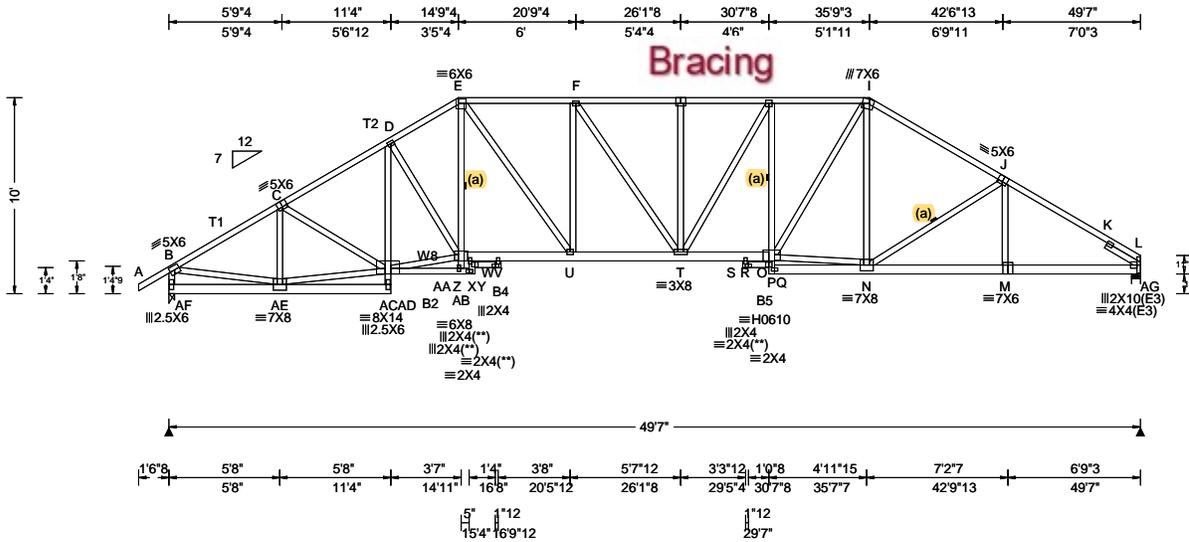
Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



COA #0 278
 01/04/2022
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 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.73 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.96 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.199 G 999 240 VERT(CL): 0.412 G 999 180 HORZ(LL): 0.079 L - - HORZ(TL): 0.164 L - - Creep Factor: 2.0 Max TC CSI: 0.524 Max BC CSI: 0.194 Max Web CSI: 0.921 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AF 2167 /- /- /1296 /- /338 AG 2058 /- /- /1190 /- /- Wind reactions based on MWFRS AF Brg Wid = 3.5 Min Req = 1.8 (Truss) AG Brg Wid = 5.5 Min Req = 1.7 (Truss) Bearings AF & AG are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 5 -2890 G - H 0 -3202 C - D 0 -3295 H - I 0 -3052 D - E 0 -3192 I - J 0 -2900 E - F 0 -3126 J - K 0 -3253 F - G 0 -3202 K - L 165 -3313 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. AA- X 2615 0 T - R 3065 0 X - V 2645 0 R - O 2966 0 V - U 2722 0 N - M 2722 0 U - T 3143 0 M - L 2726 0 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B -AF 111 -2101 E - U 692 0 B -AE 2416 0 U - F 0 -472 AE -C 38 -778 H - O 0 -520 AE-AC 2417 0 O - N 2230 0 C -AC 383 0 O - I 1199 0 AC-AA 2695 0 N - J 246 -377 E -AA 731 -121
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Lumber
Top chord: 2x4 SP M-31; T1,T2 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E; B2,B4,
B5 2x4 SP #2;
Webs: 2x4 SP #3; W8 2x4 SP #2;
Rt Slider: 2x4 SP #3; block length = 2.029'

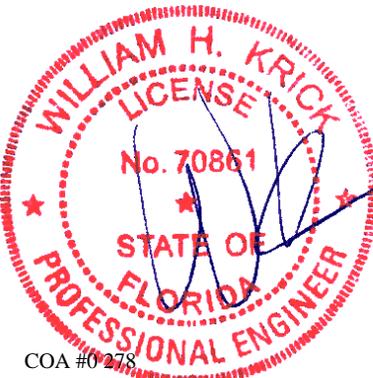
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

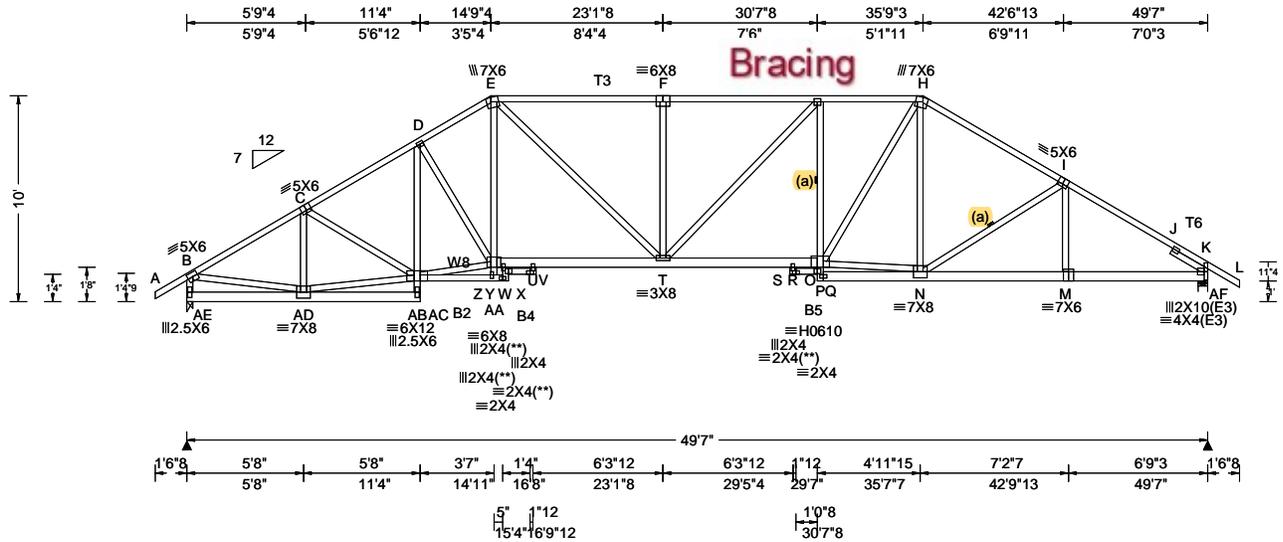


COA #0 278
01/04/2022
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SEQN: 84057 FROM:	COMN Ply: 1 Qty: 2	Job Number: 22-7193 Poirrier Res Truss Label: A12	Cust: R 215 JRef: 1Xec2150005 T64 DrwNo: 094.22.1016.05937 AK / WHK 04/04/2022
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.73 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.96 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.216 F 999 240 VERT(CL): 0.429 F 999 180 HORZ(LL): 0.082 K - - HORZ(TL): 0.162 K - - Creep Factor: 2.0 Max TC CSI: 0.674 Max BC CSI: 0.190 Max Web CSI: 0.920 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AE 2165 - / - / - /1296 - / /355 AF 2164 - / - / - /1280 - / - Wind reactions based on MWFRS AE Brg Wid = 3.5 Min Req = 1.8 (Truss) AF Brg Wid = 5.5 Min Req = 1.8 (Truss) Bearings AE & AF are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
				B - C 5 -2888 G - H 0 -3053 C - D 0 -3290 H - I 0 -2891 D - E 0 -3173 I - J 0 -3237 E - F 0 -3236 J - K 106 -3297 F - G 0 -3236

Lumber
 Top chord: 2x4 SP #2; T3,T4,T6 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E; B2,B4,
 B5 2x4 SP #2;
 Webs: 2x4 SP #3; W8 2x4 SP #2;
 Rt Slider: 2x4 SP #3; block length = 2.029'

Additional Notes
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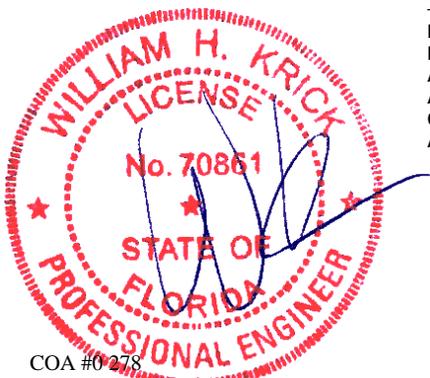
Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.
 (**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

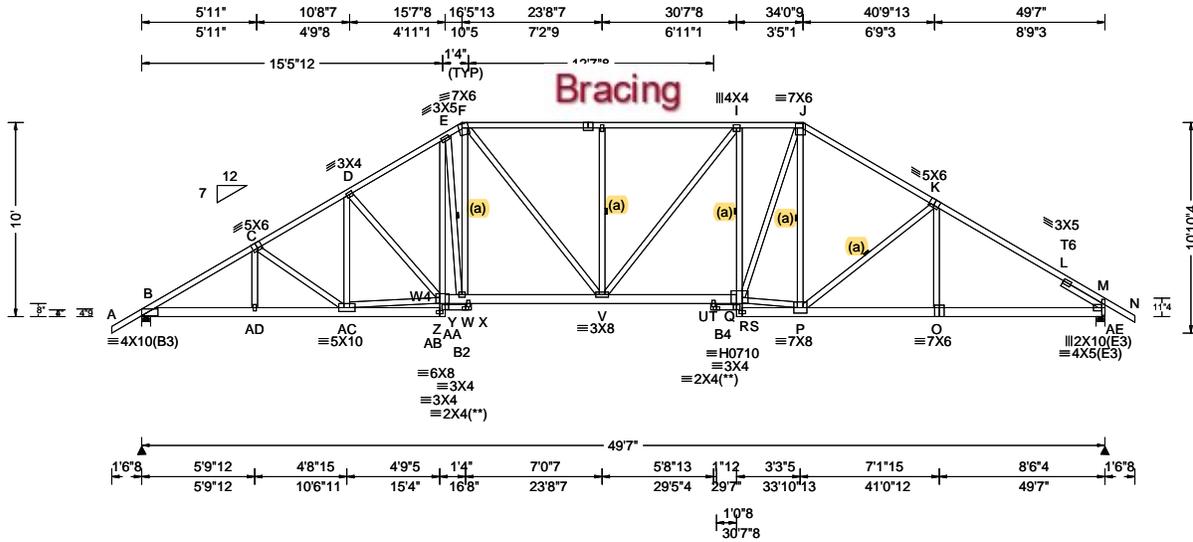
Wind
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 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



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 01/04/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.96 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.96 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.233 H 999 240 VERT(CL): 0.428 H 999 180 HORZ(LL): 0.076 M - - HORZ(TL): 0.139 M - - Creep Factor: 2.0 Max TC CSI: 0.767 Max BC CSI: 0.218 Max Web CSI: 0.893 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 2205 - / - / /1311 - / /332 AE 2319 - / - / /1291 - / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 (Truss) AE Brg Wid = 5.5 Min Req = 1.9 (Truss) Bearings B & AE are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 90 -3651 H - I 0 -2934 C - D 77 -3325 I - J 0 -2810 D - E 35 -3177 J - K 41 -2899 E - F 99 -2965 K - L 59 -3474 F - G 0 -2935 L - M 369 -3547 G - H 0 -2934
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Lumber
Top chord: 2x4 SP #2; T6 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E; B2,B4 2x4 SP #2;
Webs: 2x4 SP #3; W4 2x4 SP #2;
Rt Slider: 2x4 SP #3; block length = 2.537'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
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Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B-AD	3079	0	V-T 2819 0
AD-AC	3078	0	T-Q 2740 0
Z-Y	2537	0	P-O 2902 0
Y-W	2485	0	O-M 2908 0
W-V	2607	0	

Maximum Web Forces Per Ply (lbs)

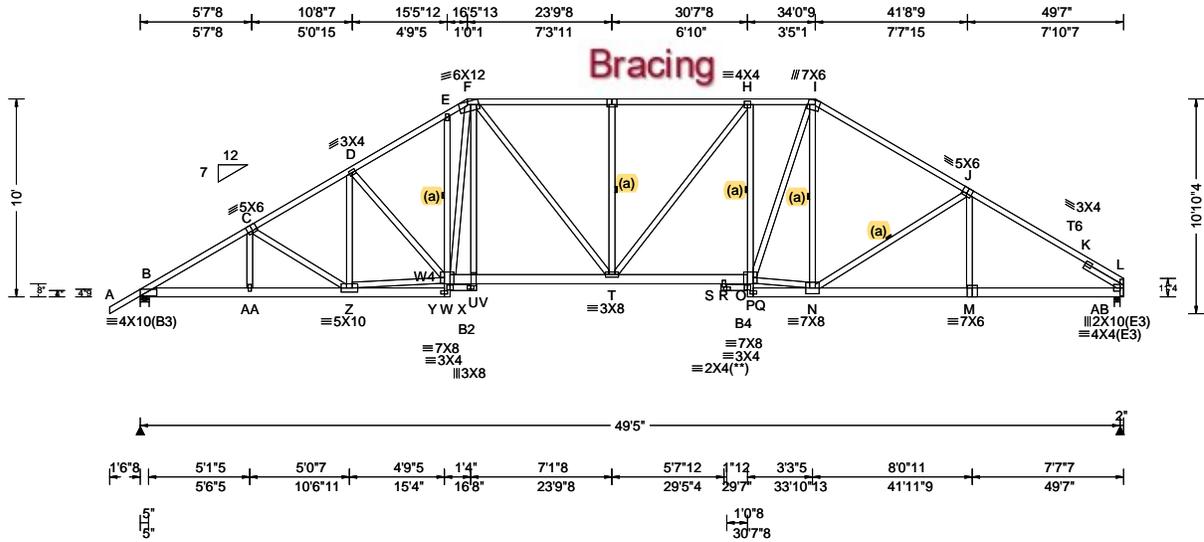
Webs	Tens.Comp.	Webs	Tens. Comp.
AC-Z	2603	0	H-V 0 -473
Z-E	563	-94	I-Q 6 -453
E-Y	272	-566	Q-P 2309 0
F-Y	731	-253	Q-J 1183 0
F-V	521	-11	P-K 258 -649



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.79 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.96 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.204 G 999 240 VERT(CL): 0.421 G 999 180 HORZ(LL): 0.068 L - - HORZ(TL): 0.141 L - - Creep Factor: 2.0 Max TC CSI: 0.916 Max BC CSI: 0.303 Max Web CSI: 0.830 VIEW Ver: 21.02.01.1216.15	Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 2161 -/ - /1295 -/ /307 AB 2067 -/ - /1200 -/ - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 (Truss) AB Brg Wid = 3.5 Min Req = 1.7 (Truss) Bearings B & AB are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 89 -3575 G - H 0 -2820 C - D 75 -3247 H - I 7 -2660 D - E 41 -3113 I - J 36 -2748 E - F 130 -2989 J - K 68 -3095 F - G 0 -2820 K - L 330 -3177
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Lumber
Top chord: 2x4 SP #2; T6 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E; B2,B4 2x4 SP #2;
Webs: 2x4 SP #3; W4 2x4 SP #2;
Rt Slider: 2x4 SP #3; block length = 2.278'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

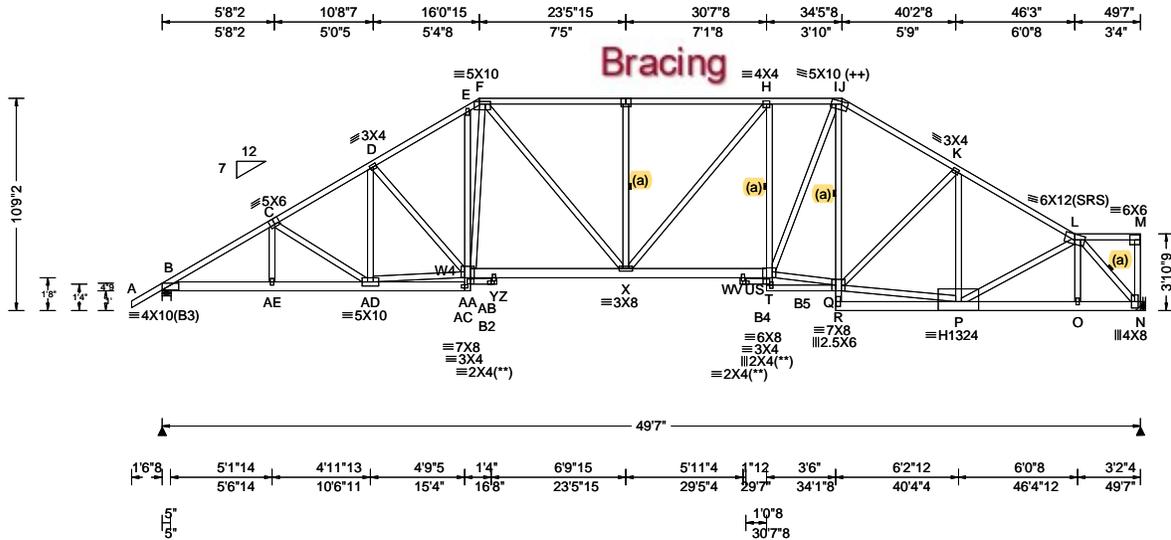
Additional Notes
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.96 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.212 G 999 240 VERT(CL): 0.438 G 999 180 HORZ(LL): 0.077 N - - HORZ(TL): 0.160 N - - Creep Factor: 2.0 Max TC CSI: 0.778 Max BC CSI: 0.199 Max Web CSI: 0.864 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 2174 /- /- /1301 /64 /274 N 2054 /- /- /1133 /47 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 (Truss) N Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 958 -3599 G - H 1058 -2930 C - D 979 -3271 H - I 1026 -2782 D - E 1044 -3137 I - J 883 -2363 E - F 1023 -2967 J - K 955 -2783 F - G 1058 -2930 K - L 807 -2700
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E; B2 2x4 SP M-31; B4, B5 2x4 SP #2;
Webs: 2x4 SP #3; W4 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(++) - This plate works for both joints covered.
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
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Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - AE	3039 -841	X - V	2794 -781
AE-AD	3038 -843	V - S	2726 -750
AA- Y	2459 -640	P - O	1846 -537
Y - X	2592 -698	O - N	1849 -534

Maximum Web Forces Per Ply (lbs)

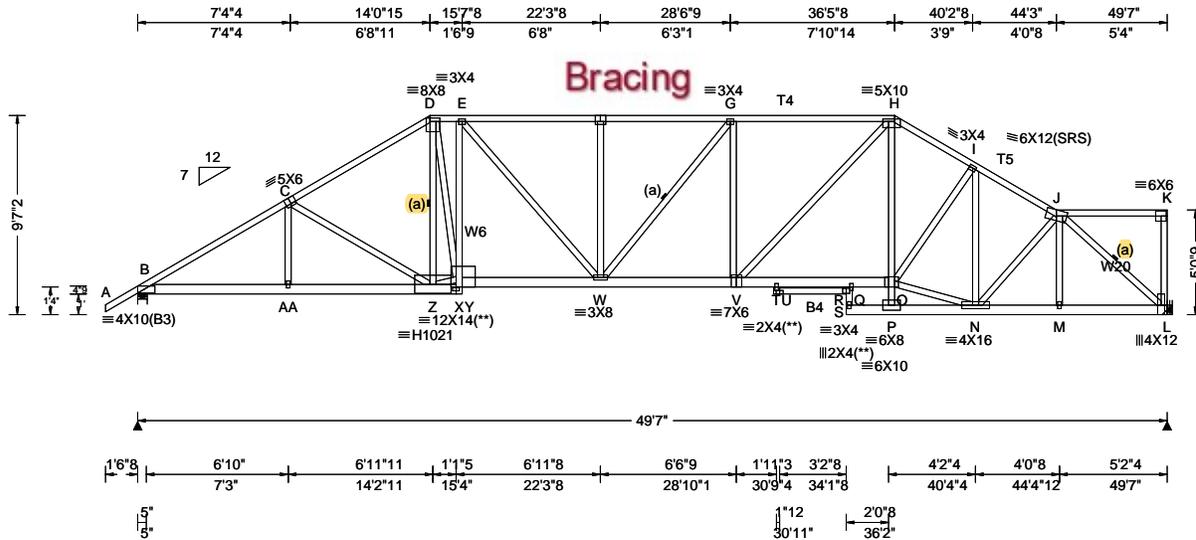
Webs	Tens.Comp.	Webs	Tens. Comp.
AD-AA	2567 -687	S - Q	2267 -597
AA- F	698 -172	Q - P	2246 -621
F - X	520 -219	K - P	227 -487
G - X	348 -474	P - L	477 -107
H - S	342 -595	L - N	772 -2674
S - I	1251 -444		



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.209 F 999 240 VERT(CL): 0.433 F 999 180 HORZ(LL): 0.074 L - - HORZ(TL): 0.154 L - - Creep Factor: 2.0 Max TC CSI: 0.749 Max BC CSI: 0.261 Max Web CSI: 0.982 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 2174 - / - / - /1290 /131 /239 L 2054 - / - / - /1088 /163 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 (Truss) L Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1010 -3562 F - G 1157 -3212 C - D 1022 -3059 G - H 1151 -3165 D - E 1049 -2857 H - I 1040 -2984 E - F 1157 -3212 I - J 871 -2661
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Lumber
Top chord: 2x4 SP #2; T4,T5 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E; B4 2x4 SP #2;
Webs: 2x4 SP #3; W6 2x4 SP M-31; W20 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).

Additional Notes
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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B-AA	2989 -909	T-Q	2476 -768
AA-Z	2988 -911	Q-O	2519 -772
X-W	2881 -895	N-M	2114 -680
W-V	3184 -996	M-L	2116 -678
V-T	2555 -785		

Maximum Web Forces Per Ply (lbs)

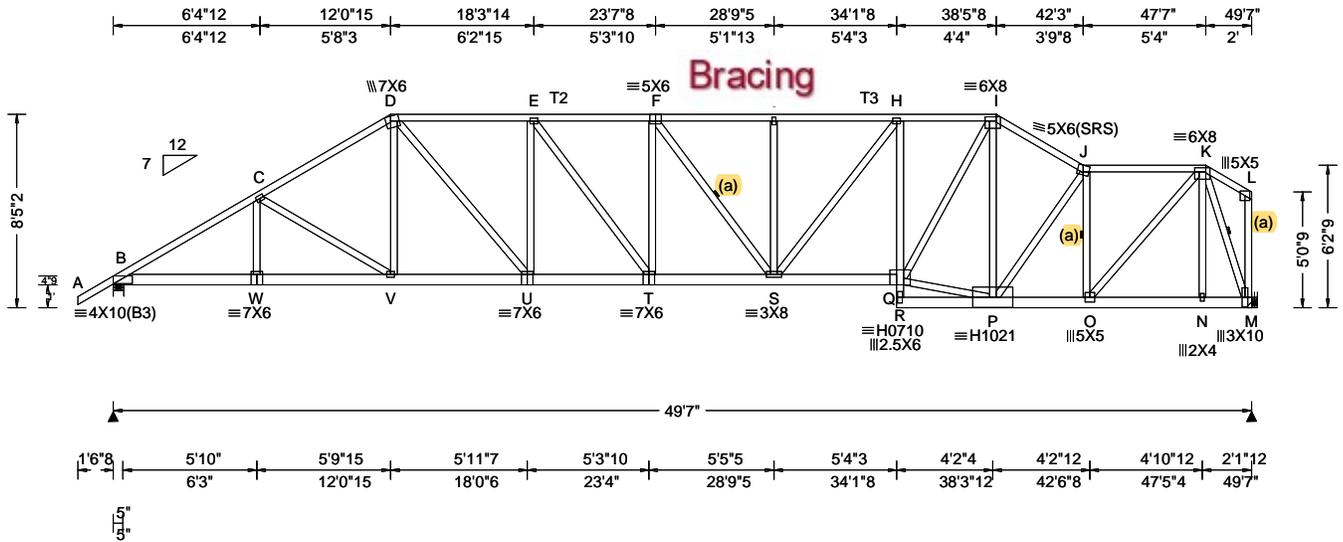
Webs	Tens.Comp.	Webs	Tens. Comp.
C-Z	178 -535	G-V	301 -590
D-Z	242 -532	V-H	1001 -318
D-X	1769 -657	O-H	495 -53
Z-X	2577 -752	O-I	601 -157
X-E	405 -621	O-N	2317 -715
E-W	521 -205	I-N	260 -785
F-W	282 -426	J-L	900 -2810



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.96 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.219 F 999 240 VERT(CL): 0.452 F 999 180 HORZ(LL): 0.060 M - - HORZ(TL): 0.125 M - - Creep Factor: 2.0 Max TC CSI: 0.483 Max BC CSI: 0.193 Max Web CSI: 0.887 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 2174 - / - / /1280 /146 /206 M 2054 - / - / /1076 /189 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 (Truss) M Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1060 -3591 G - H 1276 -3527 C - D 1075 -3181 H - I 1178 -3114 D - E 1219 -3340 I - J 965 -2626 E - F 1271 -3593 J - K 776 -2131 F - G 1276 -3527
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Lumber
Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
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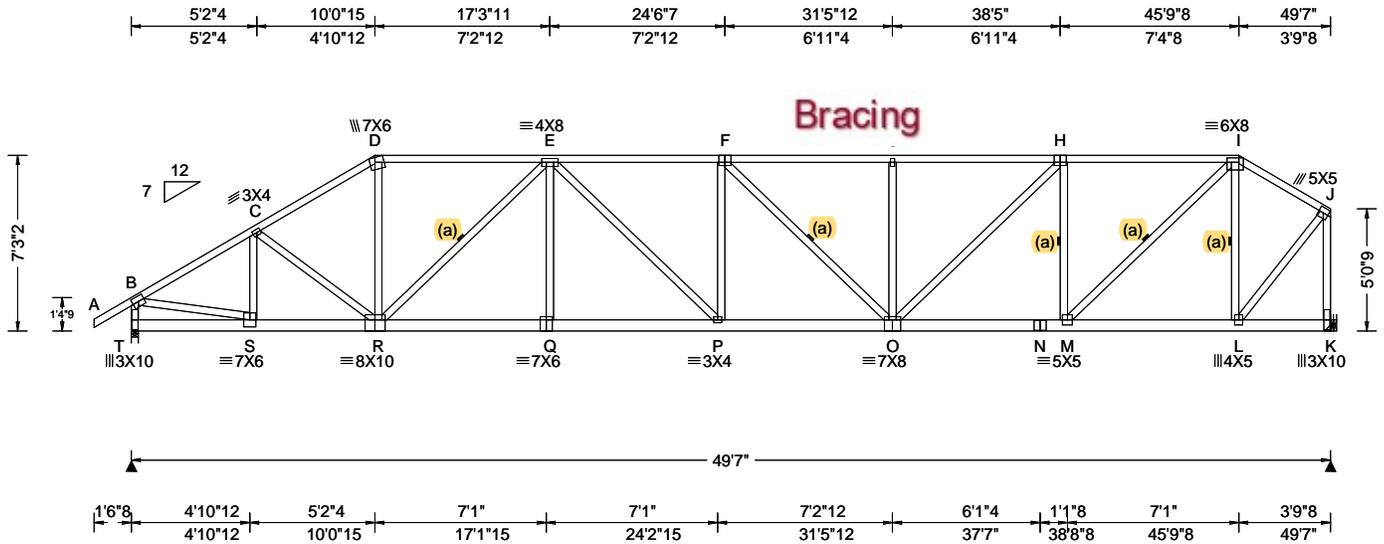


COA #0218

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.96 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.237 F 999 240 VERT(CL): 0.490 F 999 180 HORZ(LL): 0.059 D - - HORZ(TL): 0.122 D - - Creep Factor: 2.0 Max TC CSI: 0.896 Max BC CSI: 0.215 Max Web CSI: 0.914 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL T 2168 - / - / - /1256 /384 /173 K 2060 - / - / - /1060 /379 - Wind reactions based on MWFRS T Brg Wid = 3.5 Min Req = 1.8 (Truss) K Brg Wid = - Min Req = - Bearing T is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 862 -2806 F - G 1178 -3473 C - D 999 -2892 G - H 1178 -3473 D - E 915 -2456 H - I 939 -2579 E - F 1287 -3737 I - J 459 -1296
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

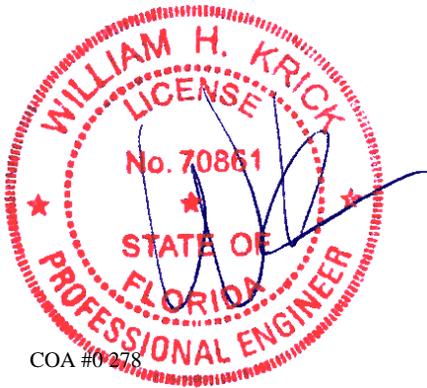
Plating Notes
All plates are 5X6 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
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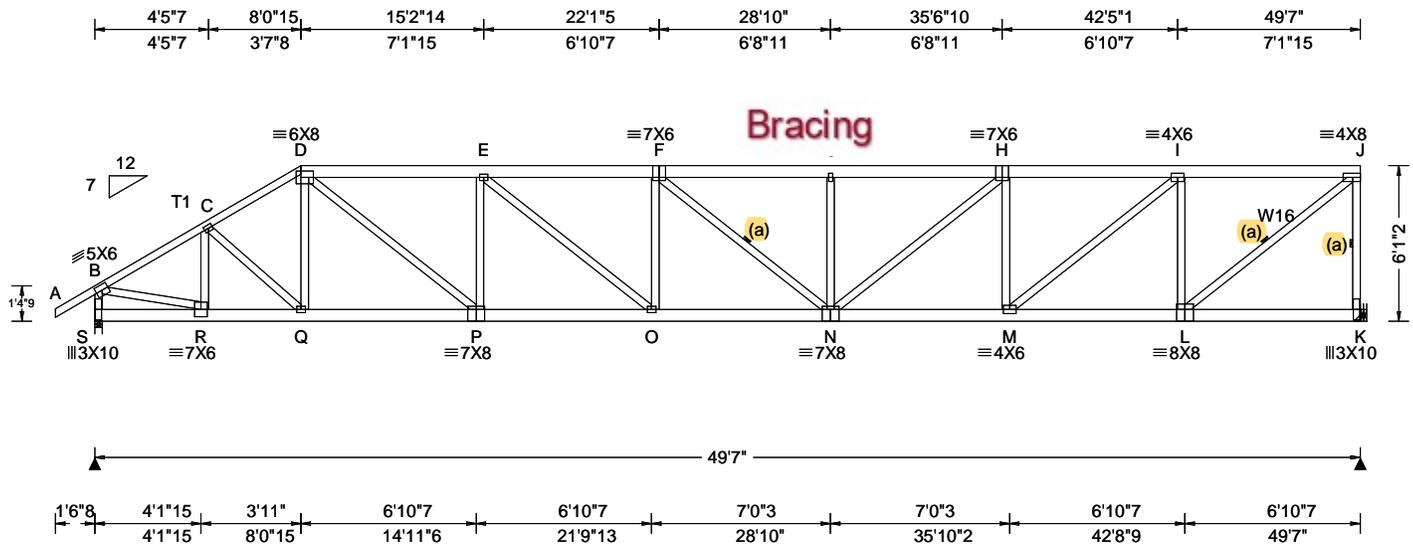


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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.96 ft Loc. from endwall: not in 6.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.227 G 999 240 VERT(CL): 0.469 G 999 180 HORZ(LL): 0.050 D - - HORZ(TL): 0.103 D - - Creep Factor: 2.0 Max TC CSI: 0.275 Max BC CSI: 0.245 Max Web CSI: 0.983 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL S 2168 - / - / - /1233 /382 /191 K 2060 - / - / - /1033 /392 - Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.8 (Truss) K Brg Wid = - Min Req = - Bearing S is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 852 -2696 F - G 1440 -4449 C - D 1006 -2915 G - H 1440 -4449 D - E 1364 -3849 H - I 1162 -3658 E - F 1498 -4491 I - J 747 -2213
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Lumber
Top chord: 2x6 SP 2400f-2.0E; T1 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W16 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

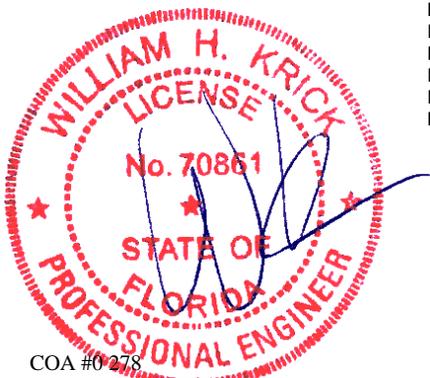
Plating Notes
All plates are 3X4 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

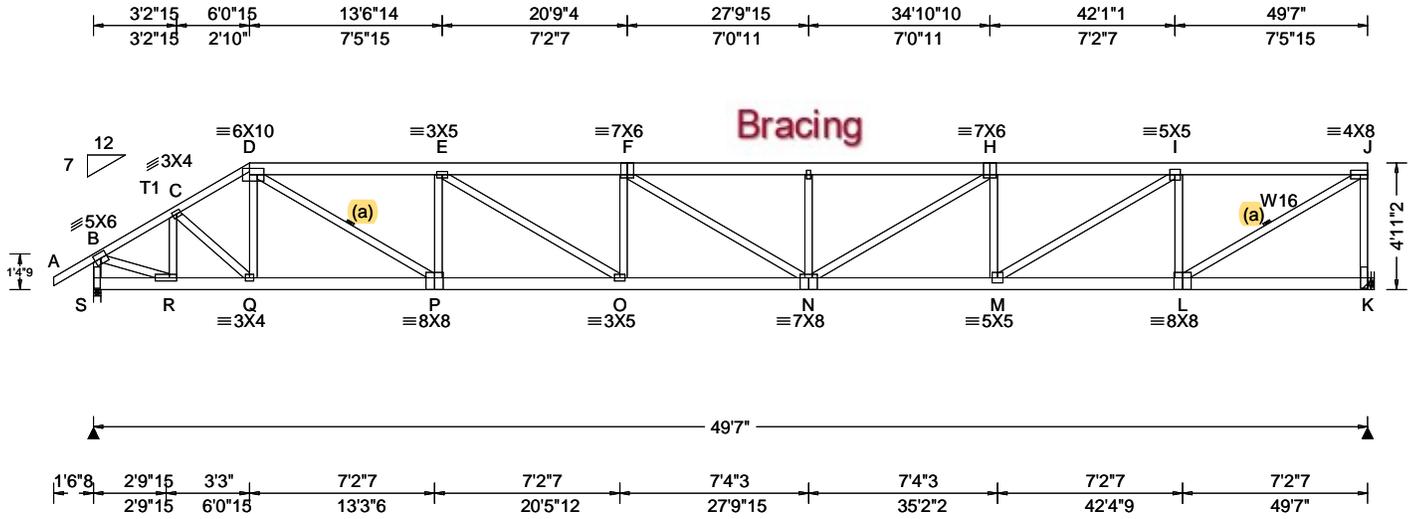
Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



COA #0218
04/04/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.96 ft Loc. from endwall: not in 6.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.326 G 999 240 VERT(CL): 0.675 G 881 180 HORZ(LL): 0.065 D - - HORZ(TL): 0.135 D - - Creep Factor: 2.0 Max TC CSI: 0.288 Max BC CSI: 0.306 Max Web CSI: 0.918 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL S 2168 - / - / - /1204 /389 /149 K 2060 - / - / - /1026 /388 - / - Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.8 (Truss) K Brg Wid = - Min Req = - Bearing S is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 796 -2431 F - G 1809 -5675 C - D 998 -2881 G - H 1809 -5675 D - E 1580 -4520 H - I 1496 -4746 E - F 1827 -5571 I - J 973 -2903

Lumber
Top chord: 2x6 SP 2400f-2.0E; T1 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W16 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

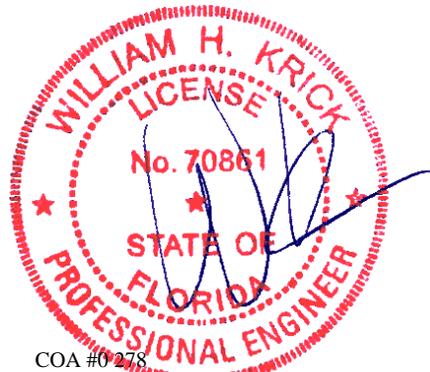
Plating Notes
All plates are 3X10 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
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COA #0278
04/04/2022
Florida Certificate of Product Approval #FL 1999

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
R - Q	2101 -786	O - N	5603 -1840
Q - P	2475 -900	N - M	4810 -1521
P - O	4594 -1612	M - L	3011 -1018

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - S	719 -2100	G - N	246 -449
B - R	2145 -617	N - H	1024 -356
R - C	248 -724	H - M	424 -981
C - Q	523 -167	M - I	2066 -649
D - P	2410 -811	I - L	689 -1650
P - E	511 -1148	L - J	3417 -1146
E - O	1164 -340	J - K	756 -1996
O - F	256 -485		

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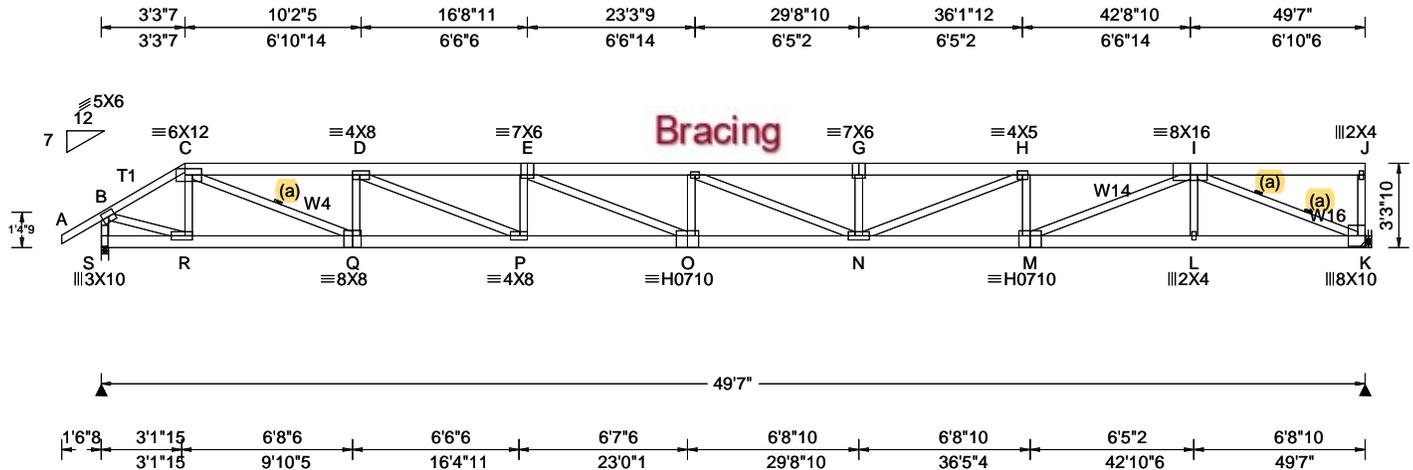
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 84130 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: A22	Cust: R 215 JRRef: 1Xec2150005 T30 DrwNo: 094.22.1019.16200 AK / WHK 04/04/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.96 ft Loc. from endwall: not in 6.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.709 F 839 240 VERT(CL): 1.465 F 406 180 HORZ(LL): 0.103 C - - HORZ(TL): 0.213 C - - Creep Factor: 2.0 Max TC CSI: 0.395 Max BC CSI: 0.489 Max Web CSI: 0.889 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL S 2168 - / - / /1161 /398 /92 K 2060 - / - / /1019 /384 - / - Wind reactions based on MWFRS S Brg Wid = 3.5 Min Req = 1.8 (Truss) K Brg Wid = - Min Req = - Bearing S is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 903 -2637 F - G 2667 -8631 C - D 1976 -5727 G - H 2667 -8631 D - E 2572 -7947 H - I 2207 -7011 E - F 2805 -8922
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Lumber
Top chord: 2x6 SP 2400f-2.0E; T1 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W4,W14,W16 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

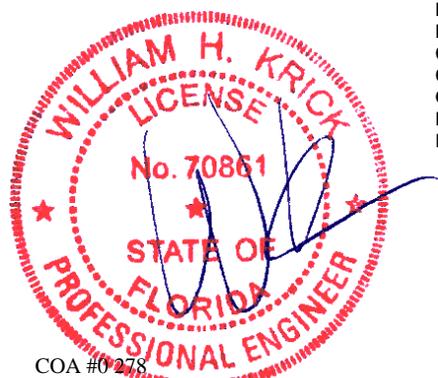
Plating Notes
All plates are 4X10 except as noted.

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
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COA #0278
04/04/2022
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Maximum Bot Chord Forces Per Ply (lbs)

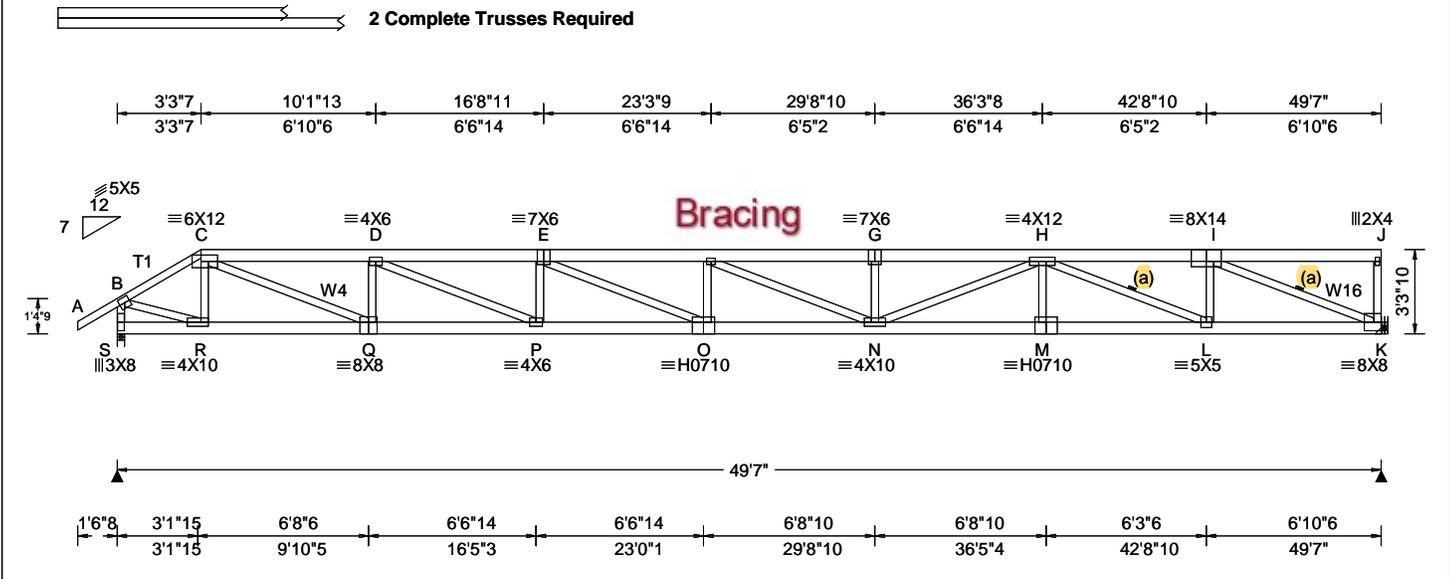
Chords	Tens.Comp.	Chords	Tens. Comp.
R - Q	2243 -808	N - M	7121 -2248
Q - P	5870 -2036	M - L	4274 -1436
P - O	8030 -2603	L - K	4274 -1436
O - N	8950 -2821		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - S	767 -2163	E - O	979 -300
B - R	2334 -718	G - N	216 -397
C - R	248 -522	N - H	1651 -526
C - Q	3800 -1275	H - M	436 -1083
Q - D	592 -1419	M - I	2992 -923
D - P	2280 -672	I - K	1554 -4627
P - E	344 -811		

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf)

TCCL: 20.00
 TCDL: 10.00
 BCLL: 0.00
 BCDL: 10.00
 Des Ld: 40.00
 NCBCLL: 0.00
 Soffit: 2.00
 Load Duration: 1.25
 Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.00 ft
 TCDL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: 0 to h/2
 C&C Dist a: 4.96 ft
 Loc. from endwall: not in 6.50 ft
 GCp: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: No
 FT/RT:20(0)/10(0)
 Plate Type(s):
 WAVE, HS

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
 VERT(LL): 0.664 F 896 240
 VERT(CL): 1.339 F 444 180
 HORZ(LL): 0.097 C - -
 HORZ(TL): 0.196 C - -
 Creep Factor: 2.0
 Max TC CSI: 0.385
 Max BC CSI: 0.496
 Max Web CSI: 0.842
 VIEW Ver: 21.02.01.1216.15

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
S	3835	-	-	-	/904	/38
K	3829	-	-	-	/913	-

Wind reactions based on MWFRS
 S Brg Wid = 3.5 Min Req = 1.6 (Truss)
 K Brg Wid = - Min Req = -
 Bearing S is a rigid surface.
 Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	573 - 2443	F - G	1889 - 7879
C - D	1251 - 5256	G - H	1889 - 7879
D - E	1732 - 7269	H - I	961 - 3973
E - F	1948 - 8148		

Lumber
 Top chord: 2x6 SP 2400f-2.0E; T1 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W4,W16 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Nailnote
 Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Hangers / Ties
 (J) Hanger Support Required, by others

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads and reactions based on MWFRS.
 End verticals exposed to wind pressure. Deflection meets L/360.
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
R - Q	2099 - 447	N - M	6454 - 1516
Q - P	5380 - 1248	M - L	6454 - 1516
P - O	7338 - 1716	L - K	3840 - 897
O - N	8167 - 1919		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - S	458 - 1930	P - E	239 - 677
B - R	2183 - 493	E - O	889 - 209
C - R	175 - 438	N - H	1552 - 361
C - Q	3444 - 832	H - L	653 - 2713
Q - D	380 - 1220	L - I	1307 - 218
D - P	2074 - 486	I - K	997 - 4171



COA #0278
 01/04/2022
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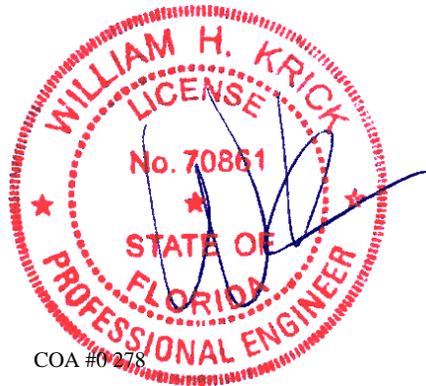
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 84133	COMN	Ply: 2	Job Number: 22-7193	Cust: R215 JRef:1Xec2150005 T112
FROM:		Qty: 1	Poirrier Res	DrwNo: 094.22.1019.42800
Page 2 of 2			Truss Label: A23	AK / WHK 04/04/2022

Additional Notes

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COA #0278

Florida Certificate of Product Approval #FL 1999

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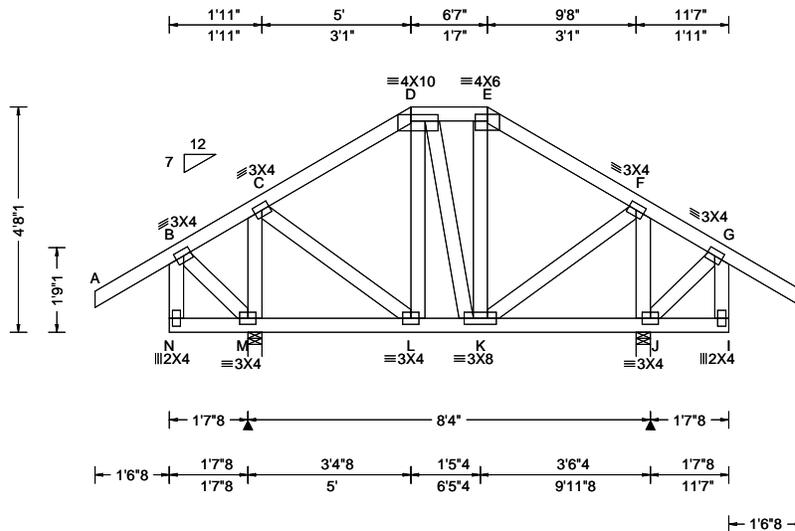
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.76 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.006 E 999 240 VERT(CL): 0.012 E 999 180 HORZ(LL): 0.001 G - - HORZ(TL): 0.002 G - - Creep Factor: 2.0 Max TC CSI: 0.207 Max BC CSI: 0.118 Max Web CSI: 0.225 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1004 /- /- /- /153 /- J 1004 /- /- /- /153 /- Wind reactions based on MWFRS M Brg Wid = 3.5 Min Req = 1.5 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings M & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 62 -536 E - F 64 -544 D - E 34 -418 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. L - K 417 0 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. M - C 127 -825 K - F 588 -65 C - L 590 -65 F - J 127 -819
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

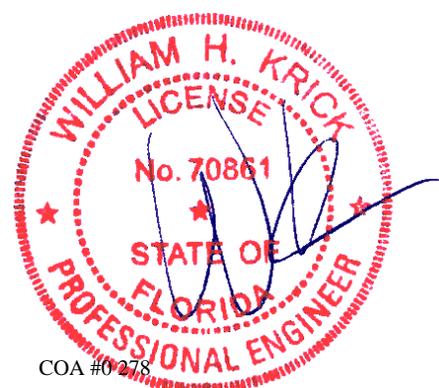
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 5.00
TC: From 32 plf at 5.00 to 32 plf at 6.58
TC: From 63 plf at 6.58 to 63 plf at 13.13
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 5.03
BC: From 10 plf at 5.03 to 10 plf at 6.55
BC: From 20 plf at 6.55 to 20 plf at 11.58
BC: From 5 plf at 11.58 to 5 plf at 13.13
TC: 300 lb Conc. Load at 5.03, 6.55
BC: 126 lb Conc. Load at 5.03, 6.55

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
End verticals exposed to wind pressure. Deflection meets L/360.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

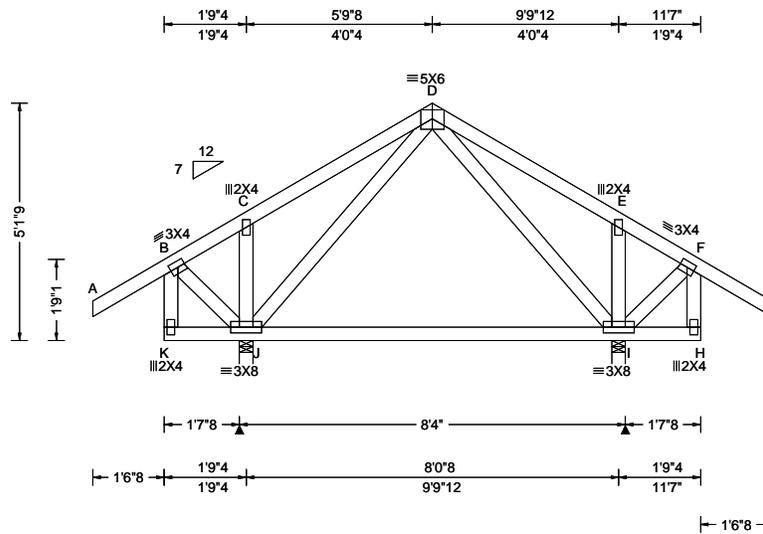


COA #0278

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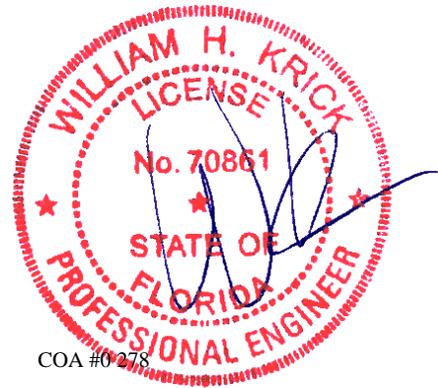
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.99 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.002 D 999 240 VERT(CL): 0.003 F 999 180 HORZ(LL): 0.002 E - - HORZ(TL): 0.004 E - - Creep Factor: 2.0 Max TC CSI: 0.235 Max BC CSI: 0.393 Max Web CSI: 0.184 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 613 /- /- /411 /93 /134 I 613 /- /- /411 /93 /- Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings J & I are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

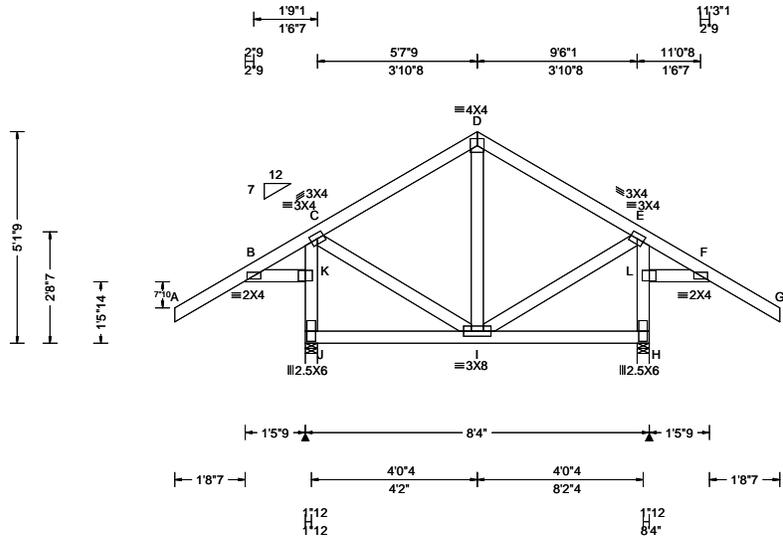
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.99 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.023 B 767 240 VERT(CL): 0.048 F 367 180 HORZ(LL): -0.012 F - - HORZ(TL): 0.025 F - - Creep Factor: 2.0 Max TC CSI: 0.316 Max BC CSI: 0.143 Max Web CSI: 0.722 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J 582 - / - / 369 / 110 / 136 H 554 - / - / 369 / 110 / - Wind reactions based on MWFRS J Brg Wid = 3.5 Min Req = 1.5 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings J & H are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. K - J 313 -549 E - L 304 -511 K - C 304 -538 L - H 313 -522

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

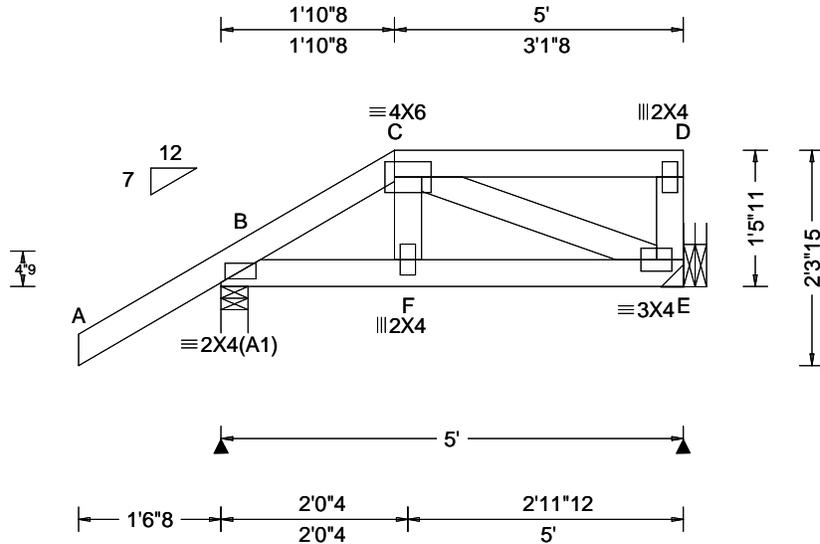


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.002 F 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 F 999 180	B 306 /- /- /- /93 /-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 B - -	E 145 /- /- /- /35 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.001 E - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.198	E Brg Wid = - Min Req = -
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.105	Bearing B is a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.038	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		
	Loc. from endwall: Any	Plate Type(s):		
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1216.15	
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.54 to 63 plf at 1.87
 TC: From 32 plf at 1.87 to 32 plf at 5.00
 BC: From 5 plf at -1.54 to 5 plf at 0.00
 BC: From 10 plf at 0.00 to 10 plf at 5.00
 TC: 11 lb Conc. Load at 1.91
 TC: 13 lb Conc. Load at 3.94
 BC: 16 lb Conc. Load at 1.91
 BC: 26 lb Conc. Load at 3.94

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



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Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location $x=4'9"$ uses the following support conditions: 4'9"

Bearing E (4'9", 9') LUS26

Supporting Member: (2)2x6 SP 2400f-2.0E
 into supporting member,
 into supported member.



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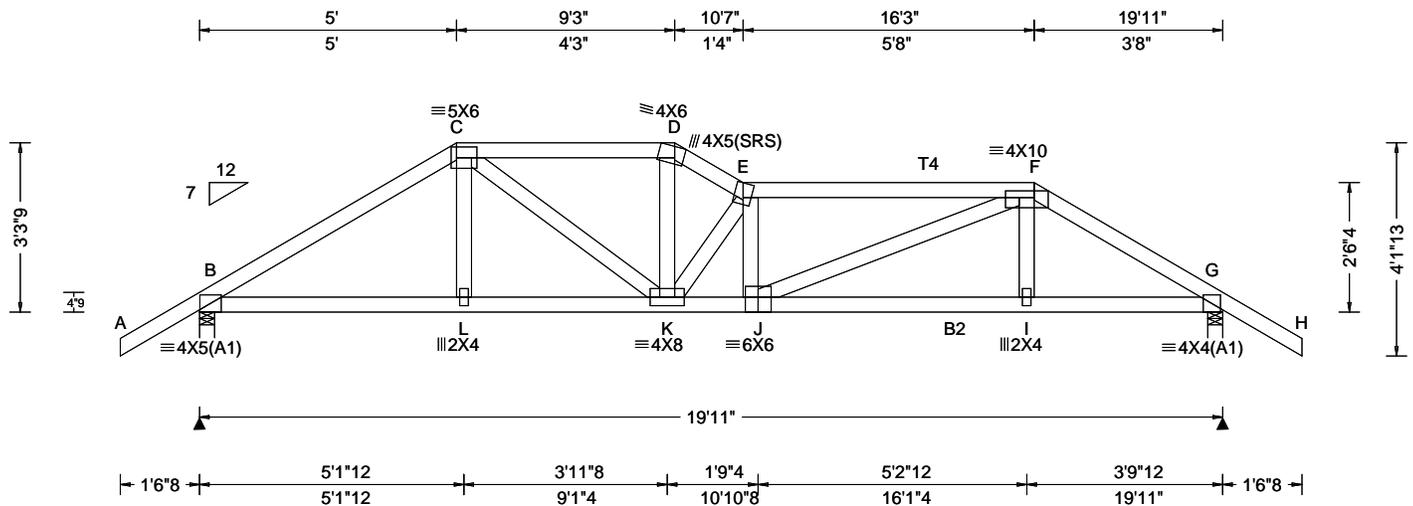
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



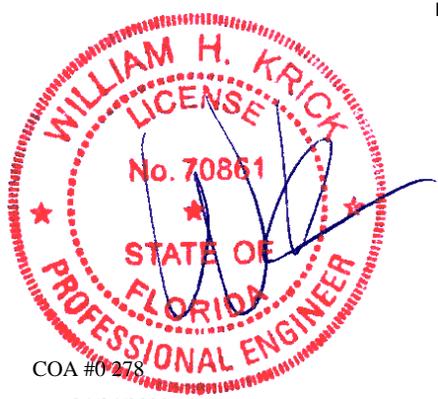
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.123 E 999 240 VERT(CL): 0.246 E 957 180 HORZ(LL): 0.033 C - - HORZ(TL): 0.067 C - - Creep Factor: 2.0 Max TC CSI: 0.815 Max BC CSI: 0.948 Max Web CSI: 0.685 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1728 -/- /- /- /349 -/ G 1418 -/- /- /- /252 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) G Brg Wid = 3.5 Min Req = 1.7 (Truss) Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 542 -2720 E - F 641 -3513 C - D 555 -2901 F - G 358 -2207 D - E 624 -3260
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Lumber
Top chord: 2x4 SP #2; T4 2x4 SP M-31;
Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at -1.54 to 63 plf at 5.00
TC: From 32 plf at 5.00 to 32 plf at 16.25
TC: From 63 plf at 16.25 to 63 plf at 21.46
BC: From 5 plf at -1.54 to 5 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 5.03
BC: From 10 plf at 5.03 to 10 plf at 16.19
BC: From 20 plf at 16.19 to 20 plf at 19.92
BC: From 5 plf at 19.92 to 5 plf at 21.46
TC: 241 lb Conc. Load at 5.03, 9.22
TC: 129 lb Conc. Load at 7.06, 8.06
BC: 281 lb Conc. Load at 5.03, 9.22
BC: 90 lb Conc. Load at 7.06, 8.06
BC: 201 lb Conc. Load at 14.31
BC: 63 lb Conc. Load at 16.19

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

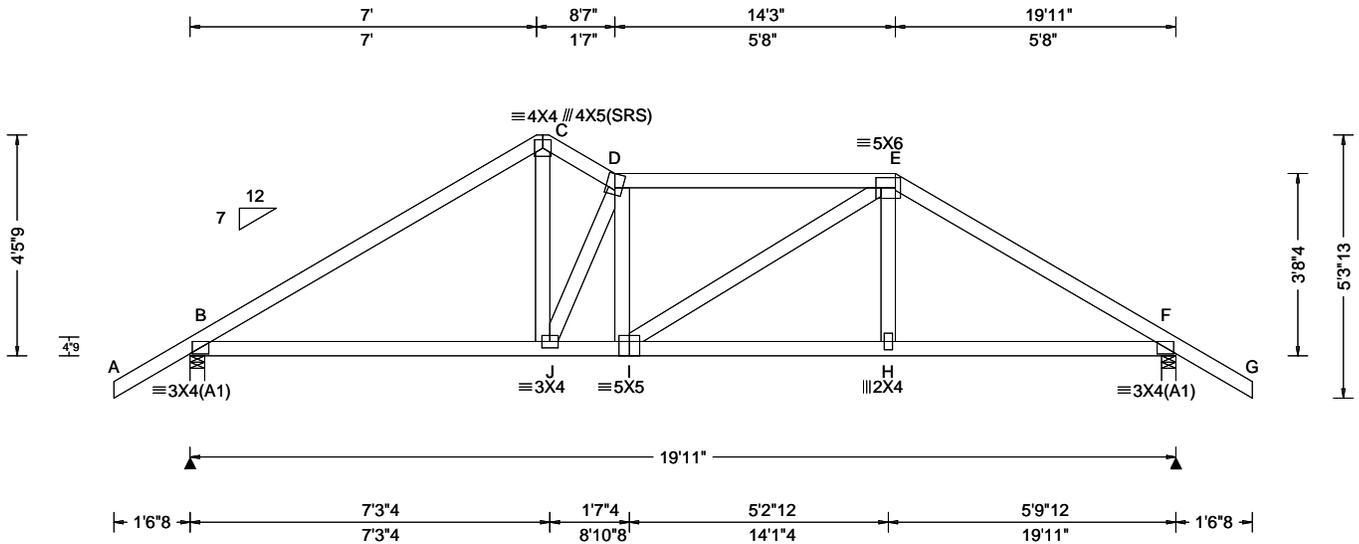
Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.039 D 999 240 VERT(CL): 0.078 D 999 180 HORZ(LL): 0.014 F - - HORZ(TL): 0.028 F - - Creep Factor: 2.0 Max TC CSI: 0.546 Max BC CSI: 0.499 Max Web CSI: 0.280 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 933 /- /- /559 /167 /152 F 933 /- /- /557 /168 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 460 -1157 D - E 587 -1155 C - D 533 -1064 E - F 532 -1222 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - J 907 -267 I - H 986 -360 J - I 1162 -439 H - F 981 -363 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - J 735 -401 J - D 473 -636

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



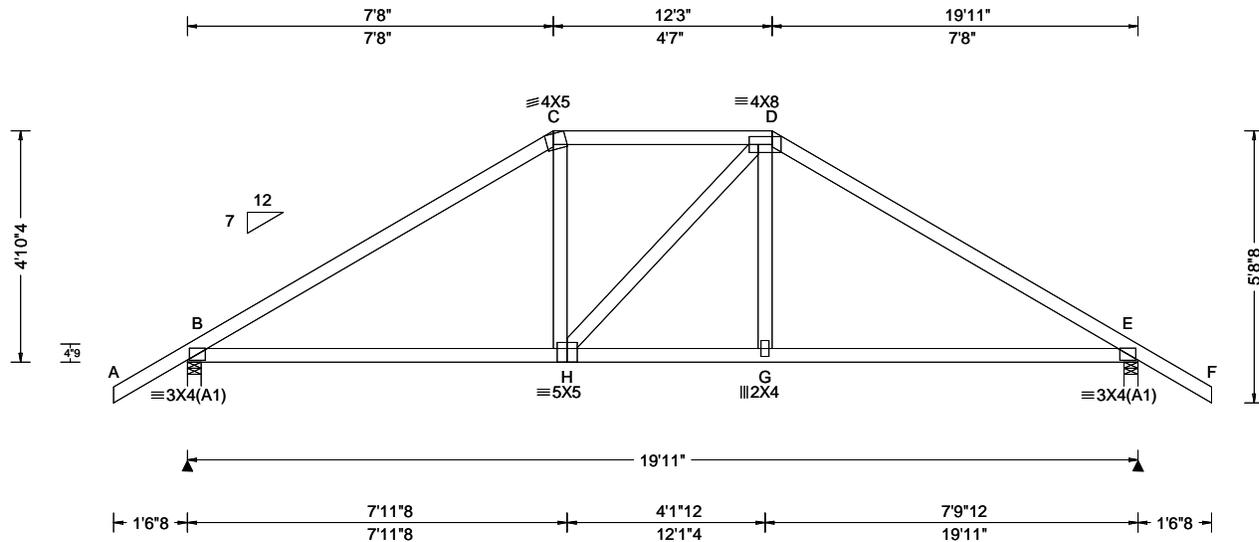
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.020 G 999 240 VERT(CL): 0.041 G 999 180 HORZ(LL): 0.012 E - - HORZ(TL): 0.024 E - - Creep Factor: 2.0 Max TC CSI: 0.712 Max BC CSI: 0.574 Max Web CSI: 0.092 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 933 /- /- /565 /165 /161 E 933 /- /- /565 /165 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens.Comp. B - C 425 -1123 D - E 424 -1126 C - D 424 -878
				Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens.Comp. B - H 869 -226 G - E 871 -233 H - G 876 -231

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

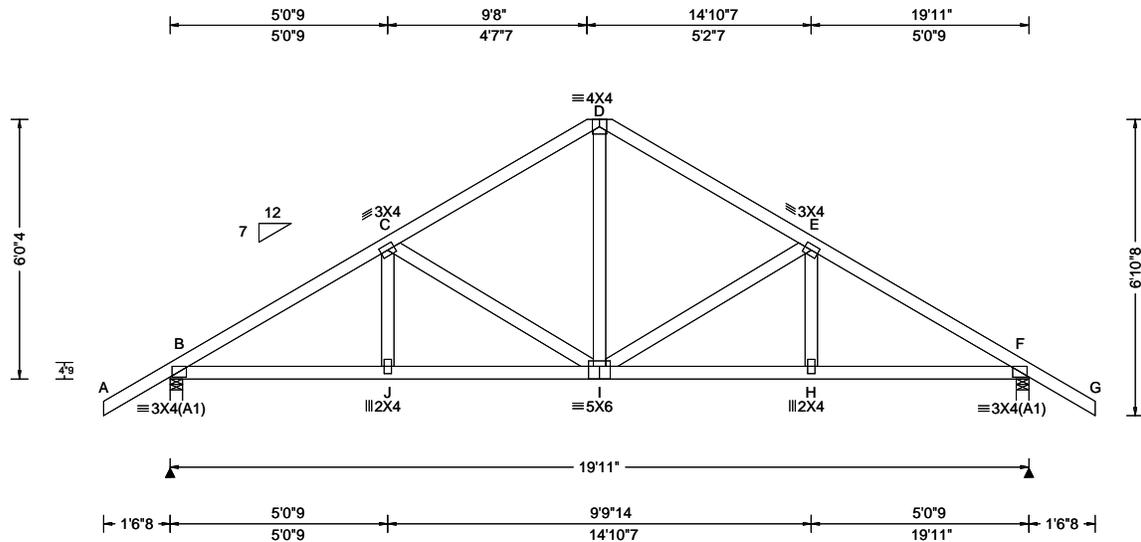


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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.035 I 999 240 VERT(CL): 0.070 I 999 180 HORZ(LL): 0.016 F - - HORZ(TL): 0.033 F - - Creep Factor: 2.0 Max TC CSI: 0.271 Max BC CSI: 0.339 Max Web CSI: 0.231 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 933 /- /- /565 /154 /192 F 933 /- /- /565 /154 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens.Comp. B - C 215 -1241 D - E 205 -881 C - D 205 -881 E - F 214 -1241
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

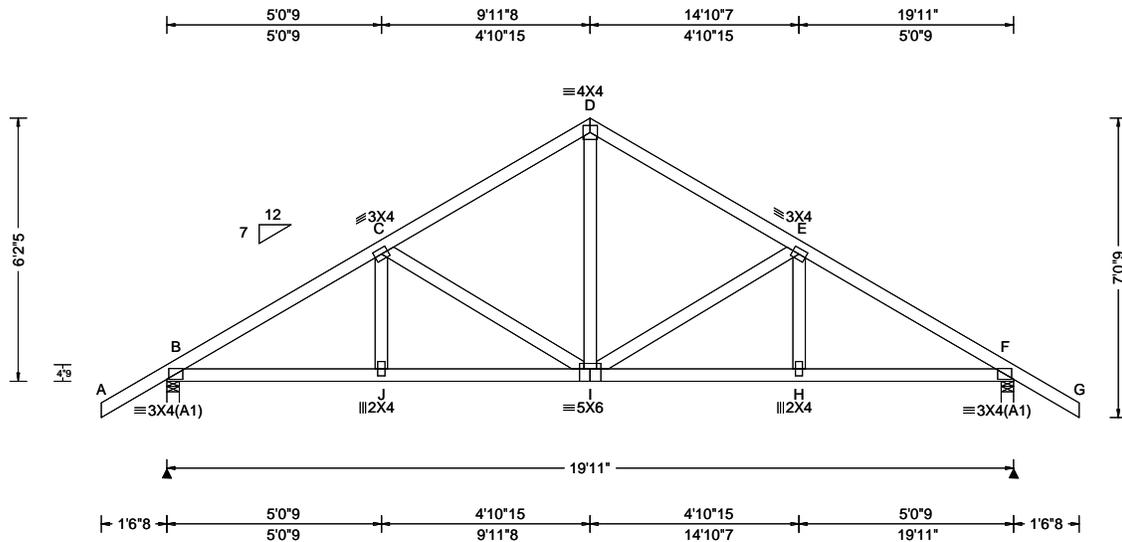
Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens.Comp.
B - J	1006 -88	I - H	1008 -87
J - I	1008 -87	H - F	1006 -88
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.		
D - I	499 -83		



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Loading Criteria (psf)	
TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria	
Wind Std:	ASCE 7-16
Speed:	130 mph
Enclosure:	Closed
Risk Category:	II
EXP:	C Kzt: NA
Mean Height:	0.00 ft
TCDL:	5.0 psf
BCDL:	5.0 psf
MWFRS Parallel Dist:	> 2h
C&C Dist a:	3.00 ft
Loc. from endwall:	not in 9.00 ft
GCp:	0.18
Wind Duration:	1.60

Snow Criteria (Pg,Pf in PSF)	
Pg:	NA Ct: NA CAT: NA
Pf:	NA Ce: NA
Lu:	NA Cs: NA
Snow Duration:	NA
Building Code:	
FBC	7th Ed. 2020 Res.
TPI Std:	2014
Rep Fac:	Yes
FT/RT:	20(0)/10(0)
Plate Type(s):	
WAVE	

Defl/CSI Criteria	
PP Deflection in	loc L/def L/#
VERT(LL):	0.035 I 999 240
VERT(CL):	0.070 I 999 180
HORZ(LL):	0.016 F - -
HORZ(TL):	0.033 F - -
Creep Factor:	2.0
Max TC CSI:	0.271
Max BC CSI:	0.339
Max Web CSI:	0.231
VIEW Ver:	21.02.01.1216.15

▲ Maximum Reactions (lbs)						
	Gravity			Non-Gravity		
Loc	R+	/R-	/Rh	/Rw	/U	/RL
B	933	-/-	-/-	/565	/161	/195
F	933	-/-	-/-	/565	/161	-/-
Wind reactions based on MWFRS						
B	Brg Wid = 3.5		Min Req = 1.5 (Truss)			
F	Brg Wid = 3.5		Min Req = 1.5 (Truss)			
Bearings B & F are a rigid surface.						
Members not listed have forces less than 375#						
Maximum Top Chord Forces Per Ply (lbs)						
Chords	Tens.Comp.		Chords	Tens. Comp.		
B - C	229	-1241	D - E	220	-881	
C - D	220	-881	E - F	228	-1241	

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

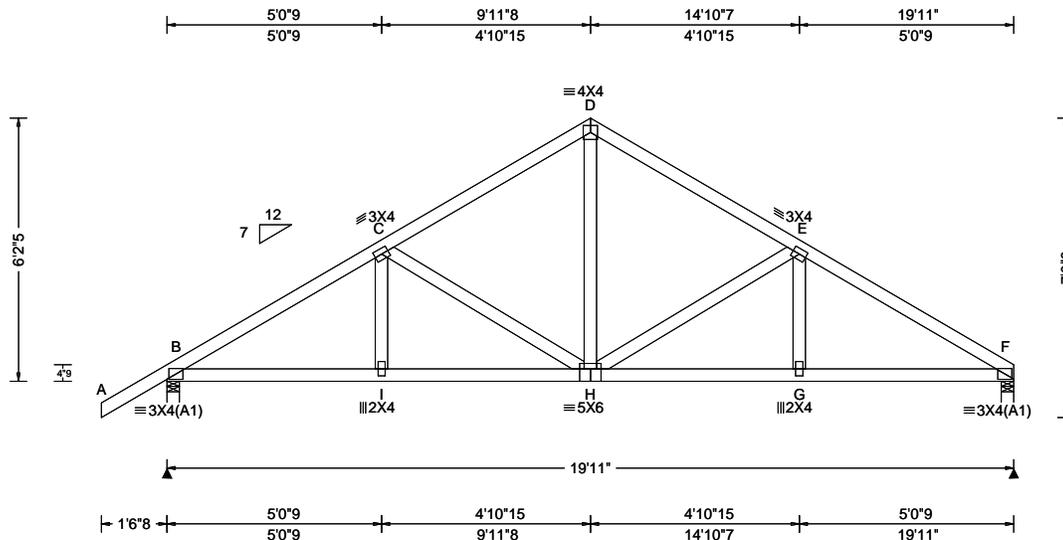
Maximum Bot Chord Forces Per Ply (lbs)					
Chords	Tens.Comp.		Chords	Tens. Comp.	
B - J	1006	-99	I - H	1008	-98
J - I	1008	-98	H - F	1006	-99
Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.				
D - I	499	-83			



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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - I	1014 -138	H - G	1040 -148
I - H	1016 -136	G - F	1038 -149

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - H	504 -93	H - E	143 -394



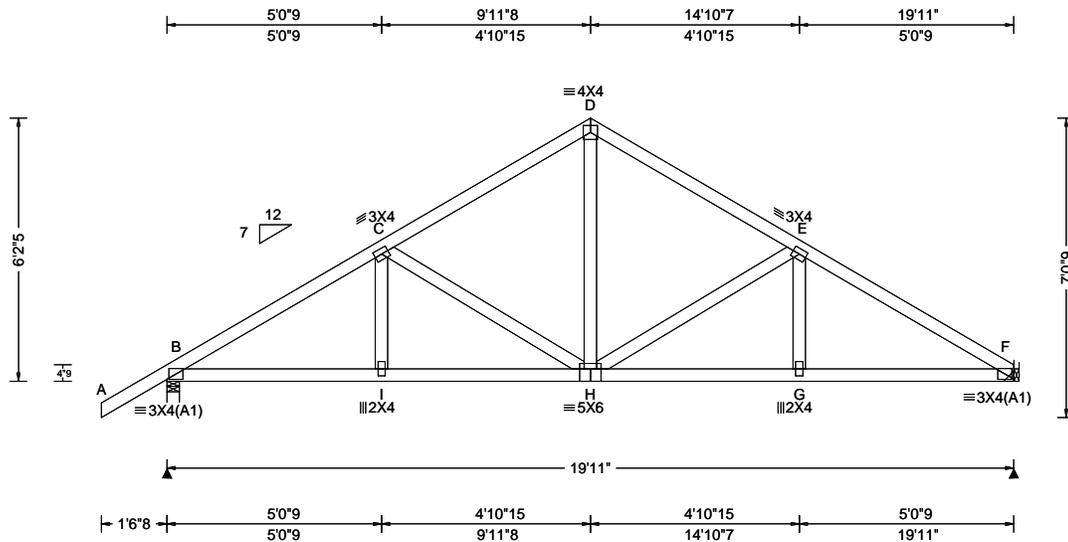
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

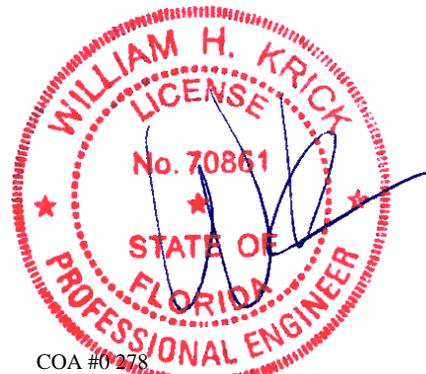
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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - I	1015 -138	H - G	1046 -149
I - H	1017 -137	G - F	1044 -150

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - H	505 -93	H - E	144 -400



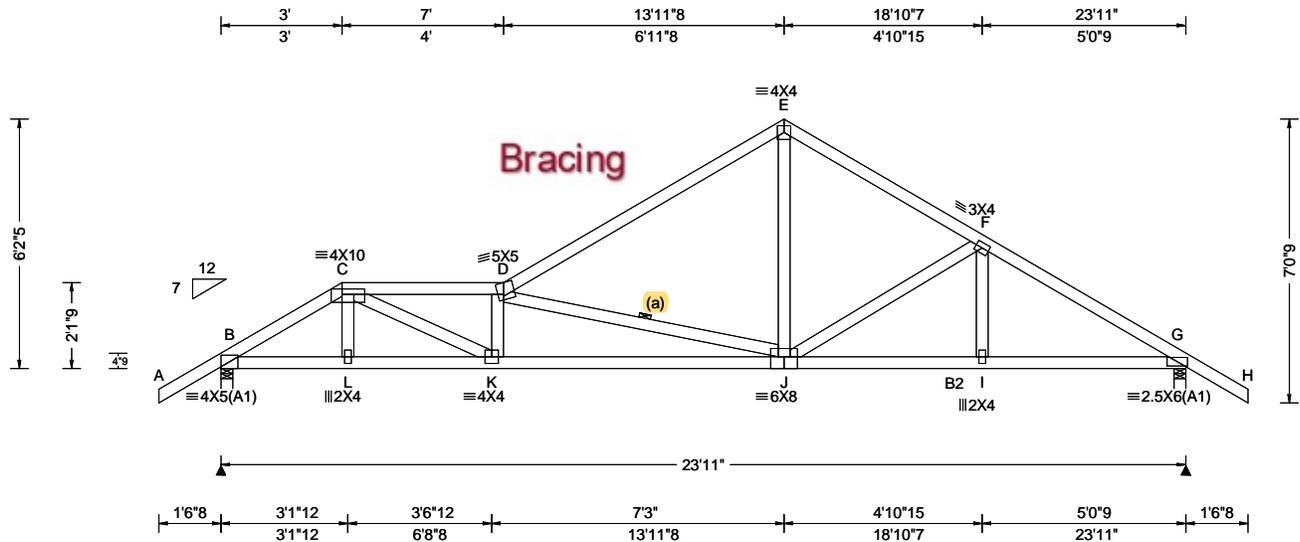
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SEQN: 84173 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: C08	Cust: R215 JRRef: 1Xec2150005 T31 DrwNo: 094.22.1021.44487 AK / WHK 04/04/2022
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.151 D 999 240 VERT(CL): 0.305 D 930 180 HORZ(LL): 0.037 C - - HORZ(TL): 0.075 C - - Creep Factor: 2.0 Max TC CSI: 0.721 Max BC CSI: 0.563 Max Web CSI: 0.729 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1578</td> <td>-</td> <td>-</td> <td>-</td> <td>237</td> <td>-</td> </tr> <tr> <td>G</td> <td>1182</td> <td>-</td> <td>-</td> <td>-</td> <td>207</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1578	-	-	-	237	-	G	1182	-	-	-	207	-
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at -1.54 to 63 plf at 25.46
 BC: From 5 plf at -1.54 to 5 plf at 0.00
 BC: From 20 plf at 0.00 to 20 plf at 3.03
 BC: From 10 plf at 3.03 to 10 plf at 3.94
 BC: From 20 plf at 3.94 to 20 plf at 23.92
 BC: From 5 plf at 23.92 to 5 plf at 25.46
 TC: 123 lb Conc. Load at 3.03
 BC: 66 lb Conc. Load at 3.03
 BC: 382 lb Conc. Load at 3.94

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.



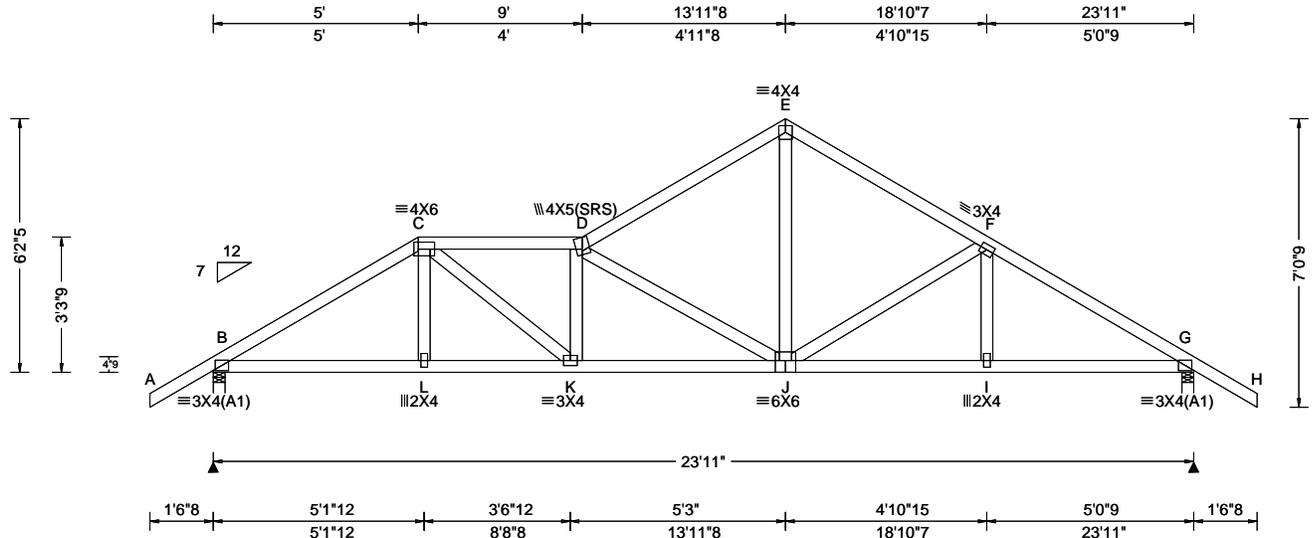
COA #0278

Florida Certificate of Product Approval #FL 1999

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SEQN: 57020 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: C09	Cust: R215 JRRef: 1Xec2150005 T25 DrwNo: 094.22.1021.53390 AK / WHK 04/04/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.075 D 999 240 VERT(CL): 0.153 D 999 180 HORZ(LL): 0.026 G - - HORZ(TL): 0.054 G - - Creep Factor: 2.0 Max TC CSI: 0.321 Max BC CSI: 0.475 Max Web CSI: 0.641 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1099</td> <td>-</td> <td>-</td> <td>/647</td> <td>/55</td> <td>/195</td> </tr> <tr> <td>G</td> <td>1099</td> <td>-</td> <td>-</td> <td>/653</td> <td>/27</td> <td>-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1099	-	-	/647	/55	/195	G	1099	-	-	/653	/27	-	
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



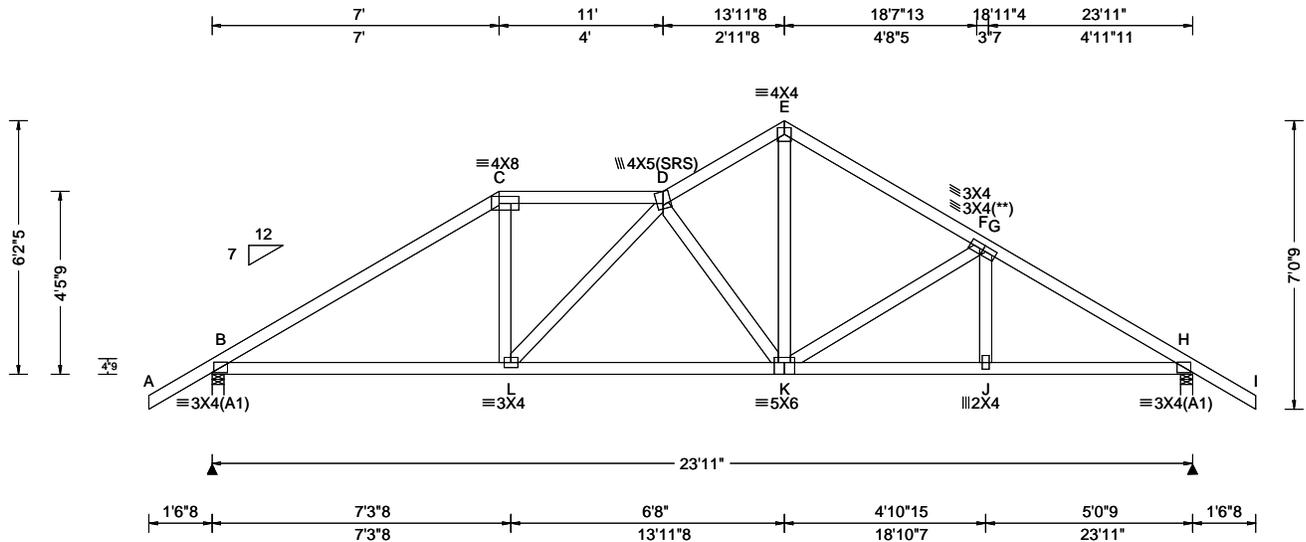
COA #0278

01/04/2022
 Florida Certificate of Product Approval #FL 1999

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SEQN: 84151 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: C10	Cust: R215 JRRef: 1Xec2150005 T26 DrwNo: 094.22.1021.59720 AK / WHK 04/04/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.058 D 999 240 VERT(CL): 0.118 D 999 180 HORZ(LL): 0.023 H - - HORZ(TL): 0.048 H - - Creep Factor: 2.0 Max TC CSI: 0.485 Max BC CSI: 0.556 Max Web CSI: 0.339 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1099</td> <td>-</td> <td>-</td> <td>/653</td> <td>/52</td> <td>/195</td> </tr> <tr> <td>H</td> <td>1099</td> <td>-</td> <td>-</td> <td>/656</td> <td>/35</td> <td>-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1099	-	-	/653	/52	/195	H	1099	-	-	/656	/35	-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

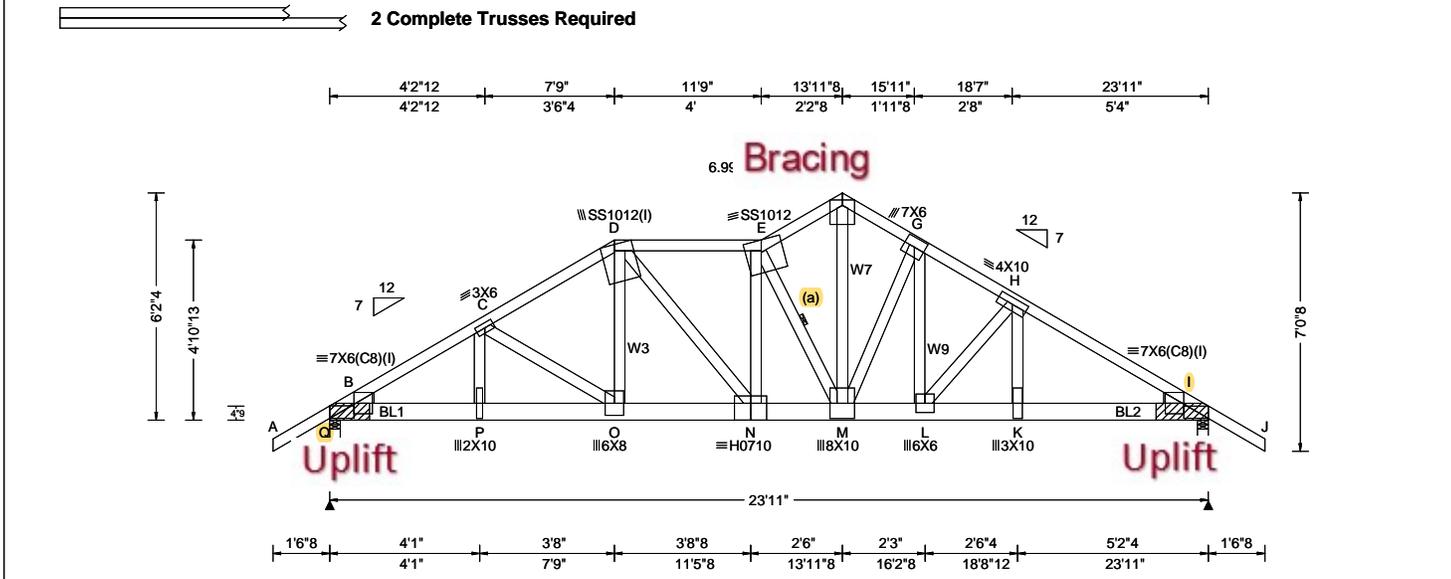


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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.265 E 999 240 VERT(CL): 0.531 E 533 180 HORZ(LL): 0.072 D - - - HORZ(TL): 0.145 D - - - Creep Factor: 2.0 Max TC CSI: 0.850 Max BC CSI: 0.839 Max Web CSI: 0.882 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>Q</td> <td>10491</td> <td>-</td> <td>-</td> <td>-</td> <td>1209</td> <td>-</td> </tr> <tr> <td>I</td> <td>11377</td> <td>-</td> <td>-</td> <td>-</td> <td>1991</td> <td>-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	Q	10491	-	-	-	1209	-	I	11377	-	-	-	1991	-									
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N - M	8522 - 1242																																												

Lumber
 Top chord: 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W3,W9 2x4 SP #2;
 W7 2x4 SP M-31;
 Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Nailnote
 Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 2 Rows @ 3.50" o.c. (Each Row)
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes
 (I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
P - C	1045 0	M - G	517 - 2436
C - O	0 - 969	F - M	7446 - 1155
D - O	2864 - 144	G - L	3019 - 626
D - N	1714 - 471	L - H	425 - 1832
E - M	444 - 4108	H - K	2068 - 465



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SEQN: 84255	SPEC	Ply: 2	Job Number: 22-7193	Cust: R215 JRef:1Xec2150005 T71
FROM:		Qty: 1	Poirrier Res	DrwNo: 094.22.1032.02350
Page 2 of 2			Truss Label: C11	AK / WHK 04/04/2022

Bearing Block(s)

Brg blocks:0.131"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
1	0.000'	1	13"	16	Rigid Surface
2	23.625'	1	17"	23	Rigid Surface

Brg block to be same size and species as chord.
Refer to drawing C>NNAILSP1014 for more information.



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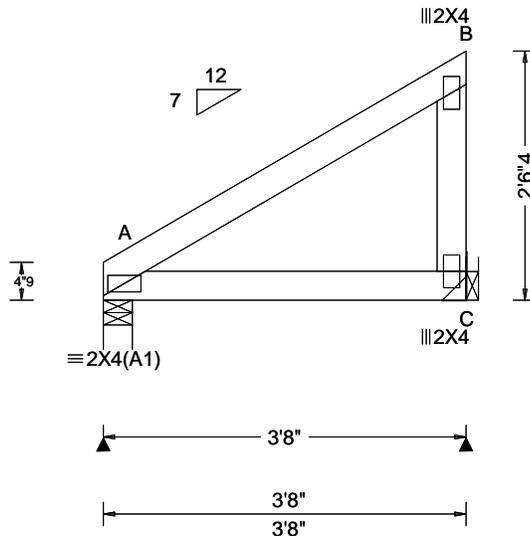
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 84258 FROM:	EJAC Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: D01	Cust: R 215 JRef: 1Xec2150005 T93 DrwNo: 094.22.1022.10507 AK / WHK 04/04/2022
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 0.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.003 A - - HORZ(TL): 0.007 A - - Creep Factor: 2.0 Max TC CSI: 0.239 Max BC CSI: 0.336 Max Web CSI: 0.052 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>227</td> <td>-</td> <td>-</td> <td>-</td> <td>/52</td> <td>/64</td> </tr> <tr> <td>C</td> <td>201</td> <td>-</td> <td>-</td> <td>-</td> <td>/10</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	227	-	-	-	/52	/64	C	201	-	-	-	/10	-
				Loc		Gravity			Non-Gravity																						
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Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) C Brg Wid = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#																															

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Special Loads

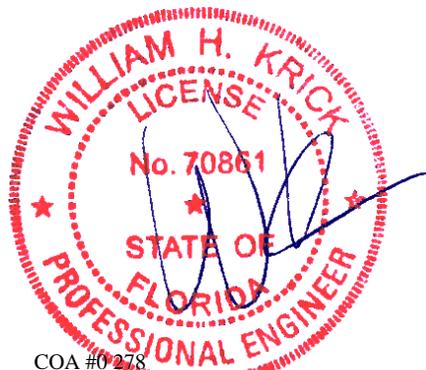
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 63 plf at 0.00 to 63 plf at 3.67
 BC: From 20 plf at 0.00 to 20 plf at 3.67
 BC: 123 lb Conc. Load at 1.71

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
 Right end vertical exposed to wind pressure.
 Deflection meets L/360.
 Wind loading based on both gable and hip roof types.

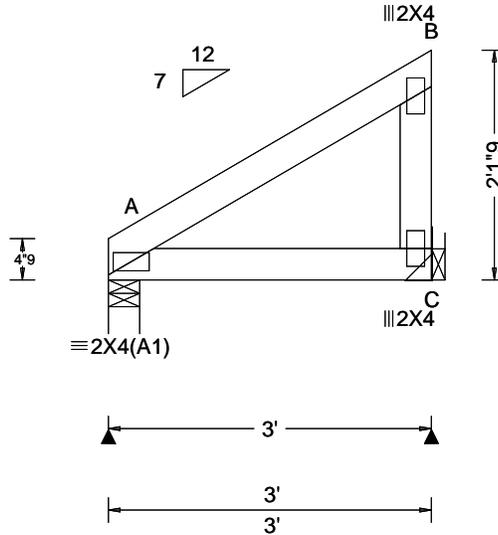


COA #0278

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 A - - HORZ(TL): 0.011 A - - Creep Factor: 2.0 Max TC CSI: 0.376 Max BC CSI: 0.422 Max Web CSI: 0.086 VIEW Ver: 21.02.01.1216.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 689 /- /- /- /44 /53 C 382 /- /- /- /7 /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) C Brg Wid = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Special Loads

----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at 0.00 to 63 plf at 3.00
BC: From 20 plf at 0.00 to 20 plf at 3.00
BC: 822 lb Conc. Load at 1.06

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
Right end vertical exposed to wind pressure.
Deflection meets L/360.
Wind loading based on both gable and hip roof types.

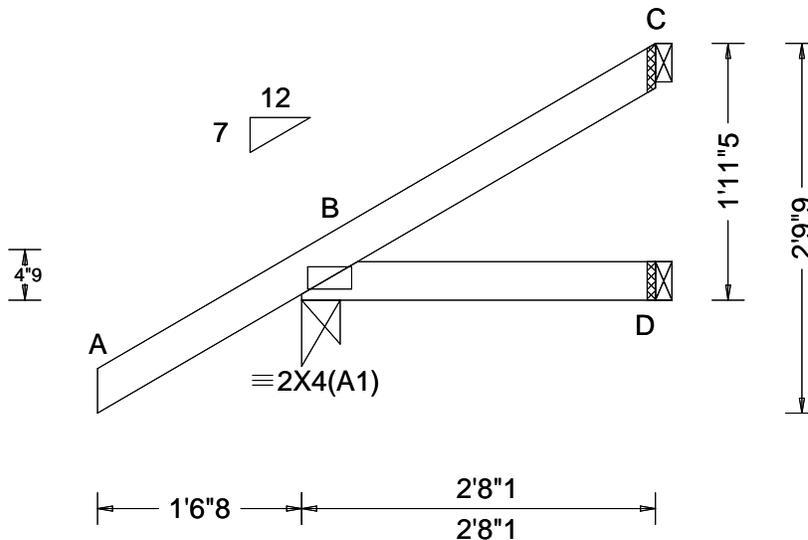


COA #0218

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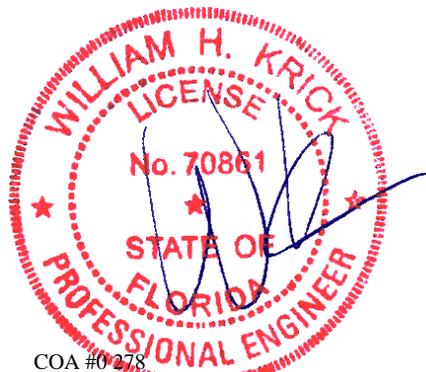
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.048 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>260</td> <td>-</td> <td>-</td> <td>/191</td> <td>/38</td> <td>/79</td> </tr> <tr> <td>D</td> <td>43</td> <td>-</td> <td>-</td> <td>/29</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>49</td> <td>-</td> <td>-</td> <td>/33</td> <td>/33</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	260	-	-	/191	/38	/79	D	43	-	-	/29	-	-	C	49	-	-	/33	/33	-
				Loc		Gravity			Non-Gravity																													
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Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#																																						

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

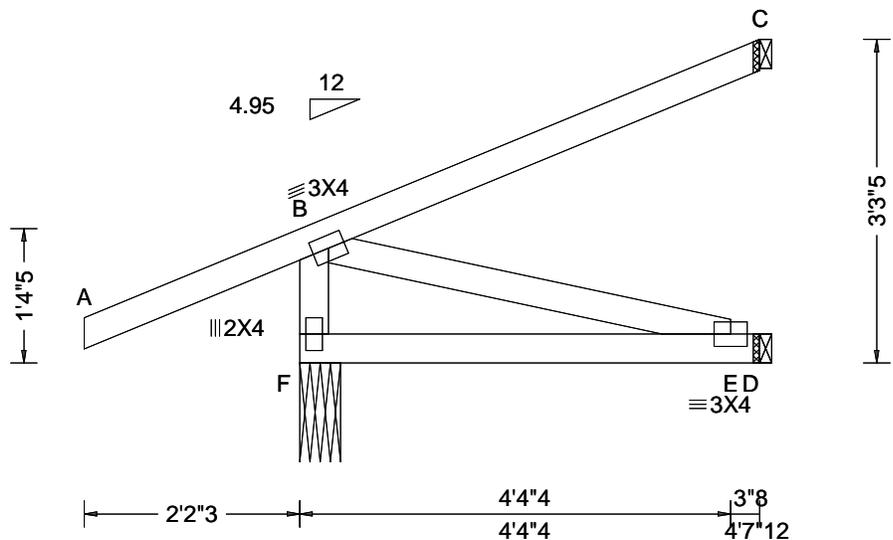


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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.002 E 999 240 VERT(CL): 0.003 E 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.321 Max BC CSI: 0.074 Max Web CSI: 0.025 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity F 192 /- /- /- /46 /- D 31 /- /- /16 /- /- C 78 /- /- /- /32 /- Wind reactions based on MWFRS F Brg Wid = 4.9 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Hipjack supports 3-3-7 setback jacks with no webs.

Wind

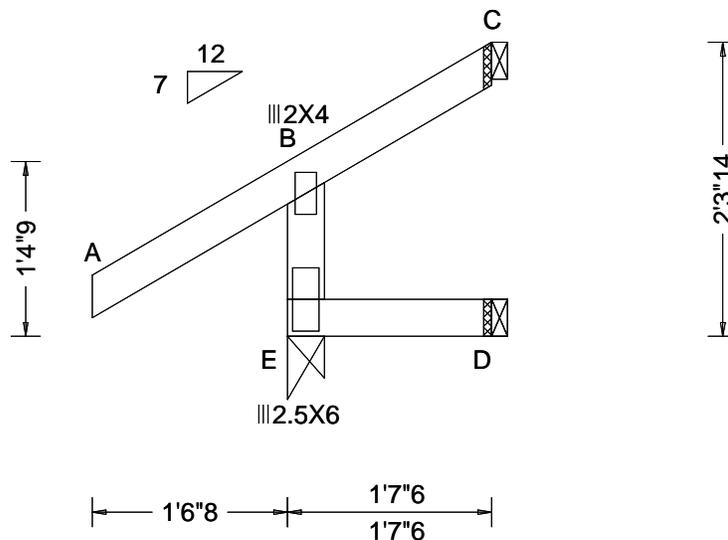
Wind loads and reactions based on MWFRS.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
01/04/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.199 Max BC CSI: 0.025 Max Web CSI: 0.097 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>222</td> <td>/-</td> <td>/-</td> <td>/195</td> <td>/76</td> <td>/-</td> </tr> <tr> <td>D</td> <td>32</td> <td>/-</td> <td>/-</td> <td>/16</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>1</td> <td>/-2</td> <td>/-</td> <td>/47</td> <td>/45</td> <td>/58</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	E	222	/-	/-	/195	/76	/-	D	32	/-	/-	/16	/-	/-	C	1	/-2	/-	/47	/45	/58
				Loc		Gravity			Non-Gravity																													
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

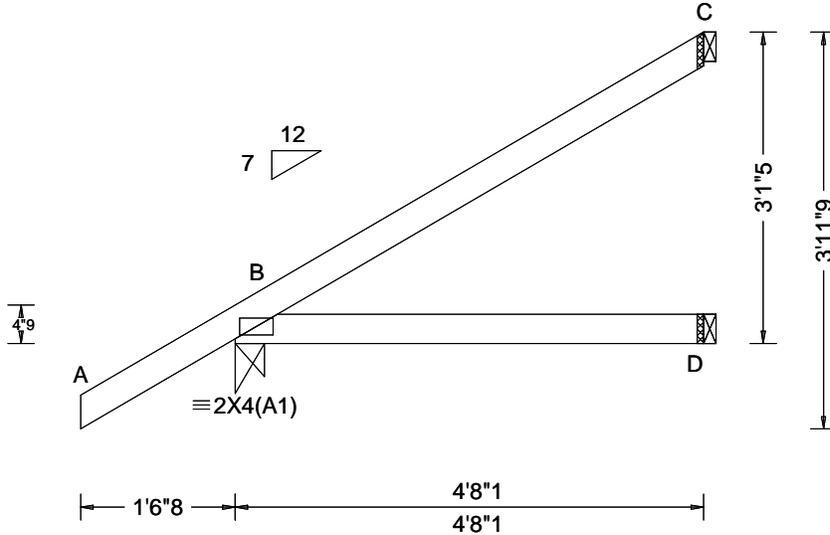
Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



COA #0 278
 01/04/2022
 Florida Certificate of Product Approval #FL 1999

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.003 B - - HORZ(TL): 0.005 B - - Creep Factor: 2.0 Max TC CSI: 0.280 Max BC CSI: 0.203 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>326</td> <td>/-</td> <td>/-</td> <td>/228</td> <td>/34</td> <td>/121</td> </tr> <tr> <td>D</td> <td>84</td> <td>/-</td> <td>/-</td> <td>/49</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>119</td> <td>/-</td> <td>/-</td> <td>/76</td> <td>/67</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	326	/-	/-	/228	/34	/121	D	84	/-	/-	/49	/-	/-	C	119	/-	/-	/76	/67	/-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

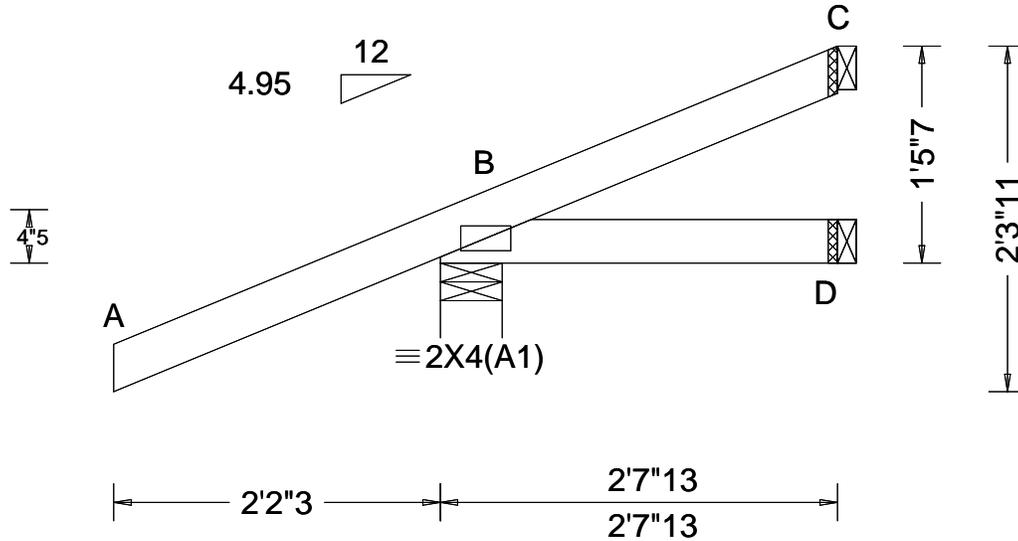


COA #0278

01/04/2022 Florida Certificate of Product Approval #FL 1999

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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.290 Max BC CSI: 0.083 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>167</td> <td>/-</td> <td>/-</td> <td>/-</td> <td>/45</td> <td>/-</td> </tr> <tr> <td>D</td> <td>-</td> <td>/-10</td> <td>/-</td> <td>/9</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-2</td> <td>/-</td> <td>/-</td> <td>/1</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	167	/-	/-	/-	/45	/-	D	-	/-10	/-	/9	/-	/-	C	-	/-2	/-	/-	/1	/-
				Loc		Gravity			Non-Gravity																													
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Lumber

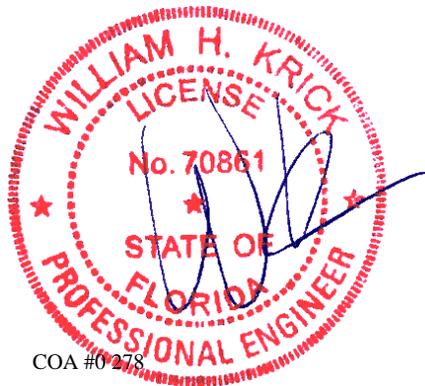
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Loading

Hipjack supports 1-10-8 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

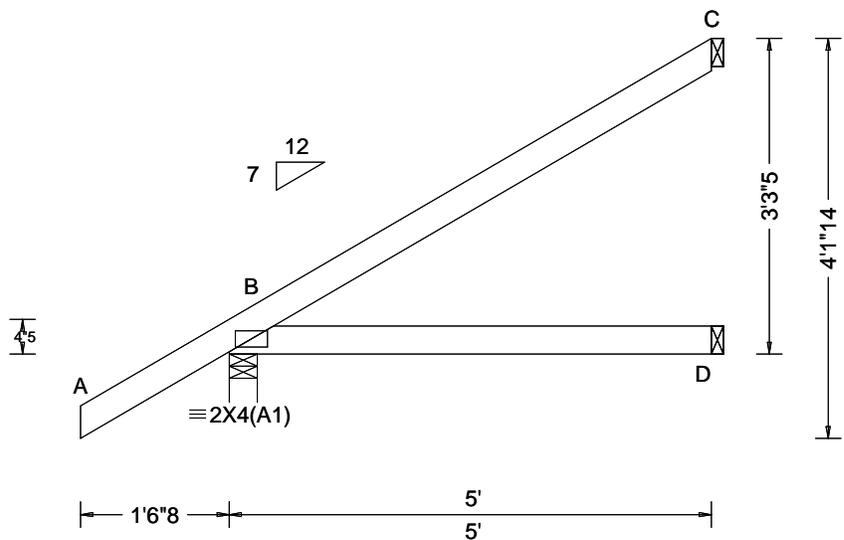


COA #0278

04/04/2022
 Florida Certificate of Product Approval #FL 1999

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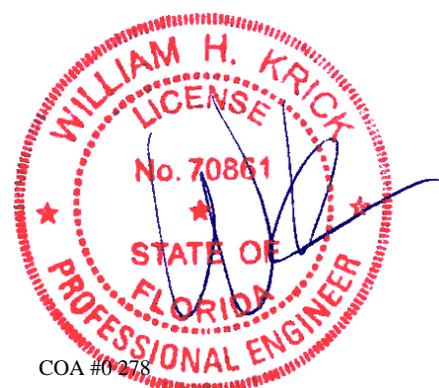




Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.008 B - - Creep Factor: 2.0 Max TC CSI: 0.326 Max BC CSI: 0.237 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>338</td> <td>-</td> <td>-</td> <td>/235</td> <td>/35</td> <td>/128</td> </tr> <tr> <td>D</td> <td>90</td> <td>-</td> <td>-</td> <td>/52</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>129</td> <td>-</td> <td>-</td> <td>/83</td> <td>/71</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	338	-	-	/235	/35	/128	D	90	-	-	/52	-	-	C	129	-	-	/83	/71	-
				Loc		Gravity			Non-Gravity																													
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Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#																																						

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

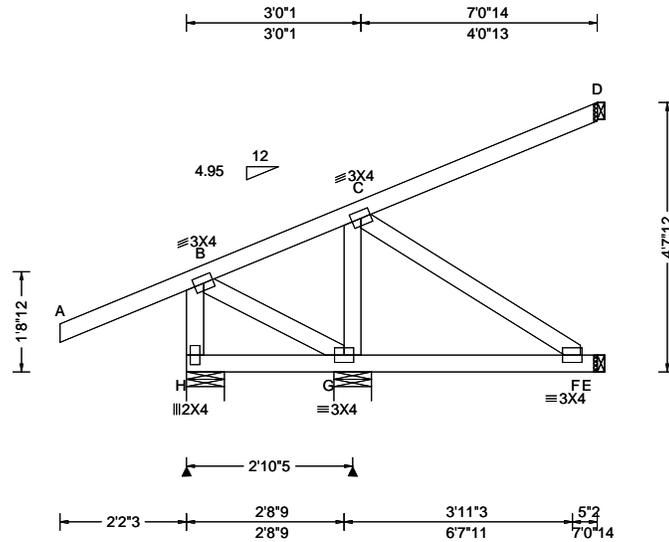
Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



COA #0278
 04/04/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.74 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 F 999 240 VERT(CL): 0.003 F 999 180 HORZ(LL): 0.000 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.381 Max BC CSI: 0.097 Max Web CSI: 0.057 VIEW Ver: 21.02.01.1216.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 73 /- /- /- /70 /- G 331 /- /- /12 /- /- E 25 /- /- /3 /- /- D 156 /- /- /- /61 /- Wind reactions based on MWFRS H Brg Wid = 7.8 Min Req = 1.5 (Truss) G Brg Wid = 7.8 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearings H & G are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Hipjack supports 5-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

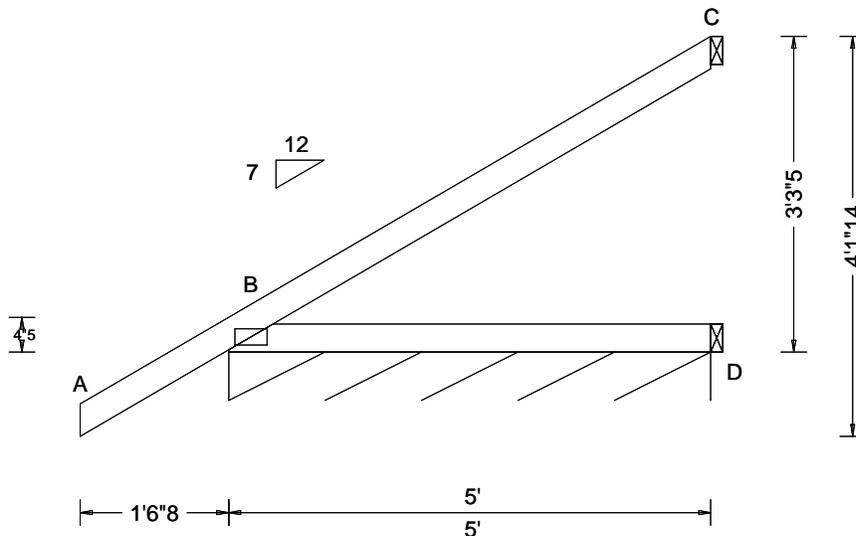


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	B*	68	-	-	/47	/7	/26
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	D	90	-	-	/52	-	-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 B - -	C	129	-	-	/83	/71	-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.008 B - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	B Brg Wid = 60.0 Min Req = -						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.326	D Brg Wid = 1.5 Min Req = -						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.237	C Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	FT/RT:20(0)/10(0)	Max Web CSI: 0.000	Bearing B is a rigid surface.						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.02.00.1005.17	Members not listed have forces less than 375#						
	Loc. from endwall: not in 9.00 ft	WAVE								
	GCp: 0.18									
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0278

Florida Certificate of Product Approval #FL 1999

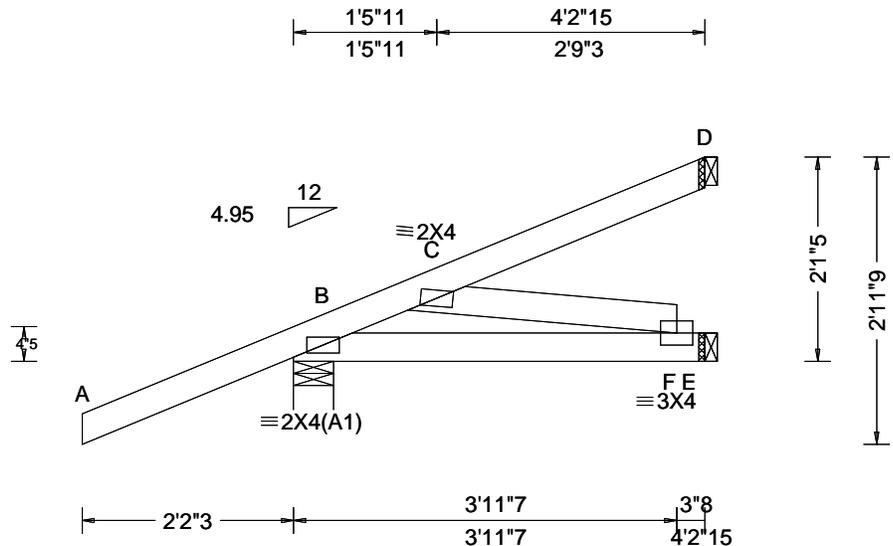
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): -0.001 B 999 240 VERT(CL): -0.002 B 999 180 HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.306 Max BC CSI: 0.066 Max Web CSI: 0.026 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>189</td> <td>-</td> <td>-</td> <td>-</td> <td>/47</td> <td>-</td> </tr> <tr> <td>E</td> <td>16</td> <td>-</td> <td>-</td> <td>/14</td> <td>-</td> <td>-</td> </tr> <tr> <td>D</td> <td>61</td> <td>-</td> <td>-</td> <td>-</td> <td>/23</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	189	-	-	-	/47	-	E	16	-	-	/14	-	-	D	61	-	-	-	/23	-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
B	189	-	-	-	/47	-																																
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

Hipjack supports 3-0-0 setback jacks with no webs.

Wind

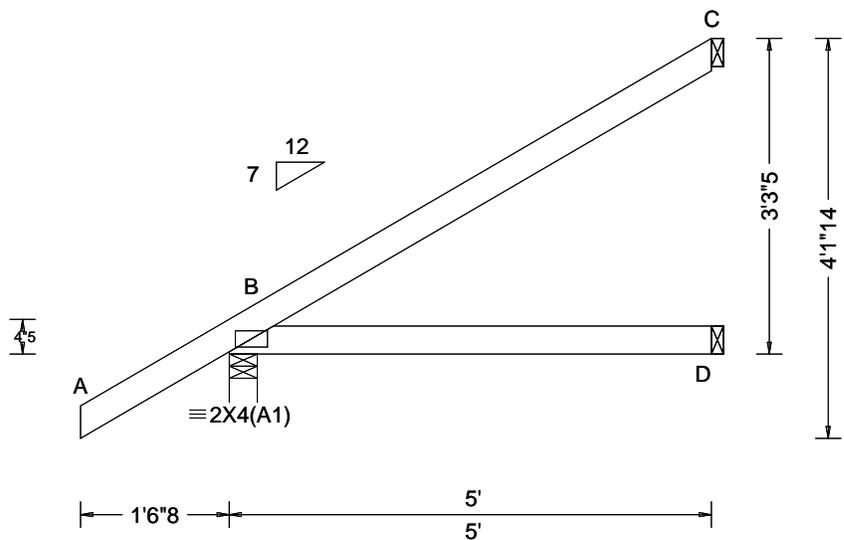
Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.008 B - - Creep Factor: 2.0 Max TC CSI: 0.326 Max BC CSI: 0.237 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 338 /- /- /235 /35 /128 D 90 /- /- /52 /- /- C 129 /- /- /83 /71 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

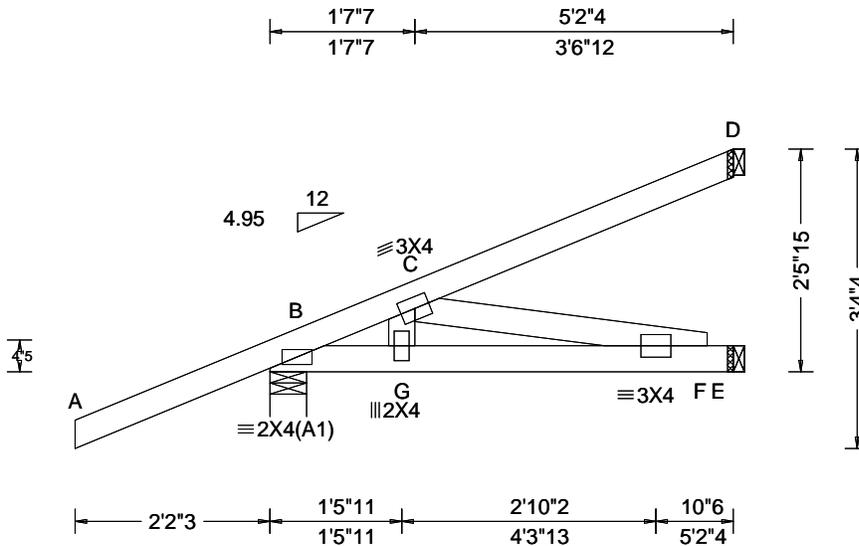
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 C 999 240 VERT(CL): 0.004 C 999 180 HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.306 Max BC CSI: 0.106 Max Web CSI: 0.018 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>212</td> <td>-</td> <td>-</td> <td>-</td> <td>/50</td> <td>-</td> </tr> <tr> <td>E</td> <td>48</td> <td>-</td> <td>-</td> <td>/14</td> <td>-</td> <td>-</td> </tr> <tr> <td>D</td> <td>94</td> <td>-</td> <td>-</td> <td>-</td> <td>/34</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	212	-	-	-	/50	-	E	48	-	-	/14	-	-	D	94	-	-	-	/34	-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
B	212	-	-	-	/50	-																																
E	48	-	-	/14	-	-																																
D	94	-	-	-	/34	-																																
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

Hipjack supports 3-8-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

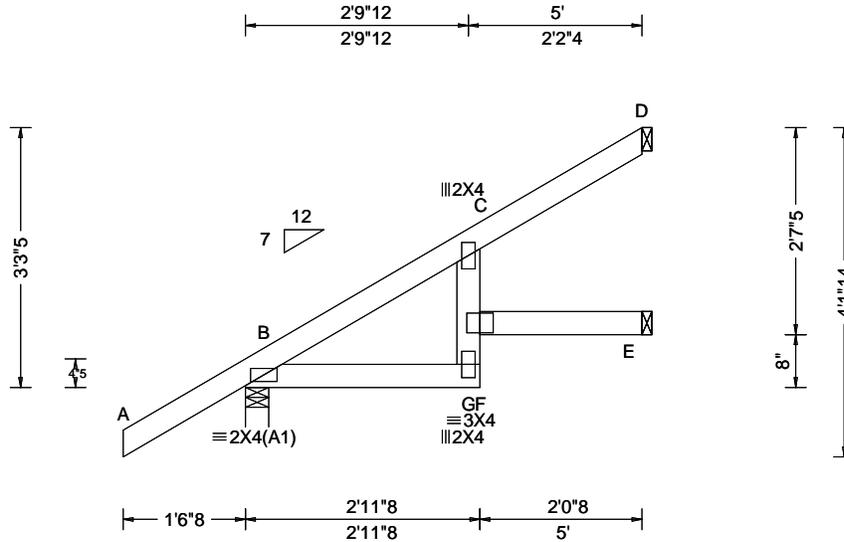


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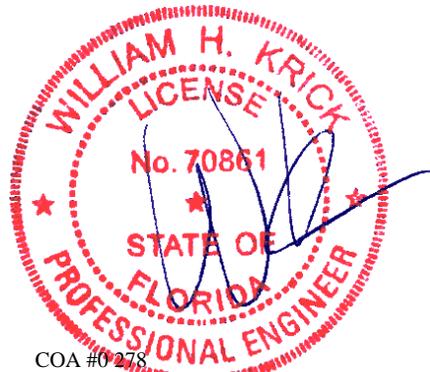
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.046 F 999 240 VERT(CL): 0.092 F 632 180 HORZ(LL): 0.028 C - - HORZ(TL): 0.057 C - - Creep Factor: 2.0 Max TC CSI: 0.312 Max BC CSI: 0.094 Max Web CSI: 0.089 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 338 - / - / - /235 /4 /91 E 54 - / - / - /32 - / - D 144 - / - / - /99 /36 - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

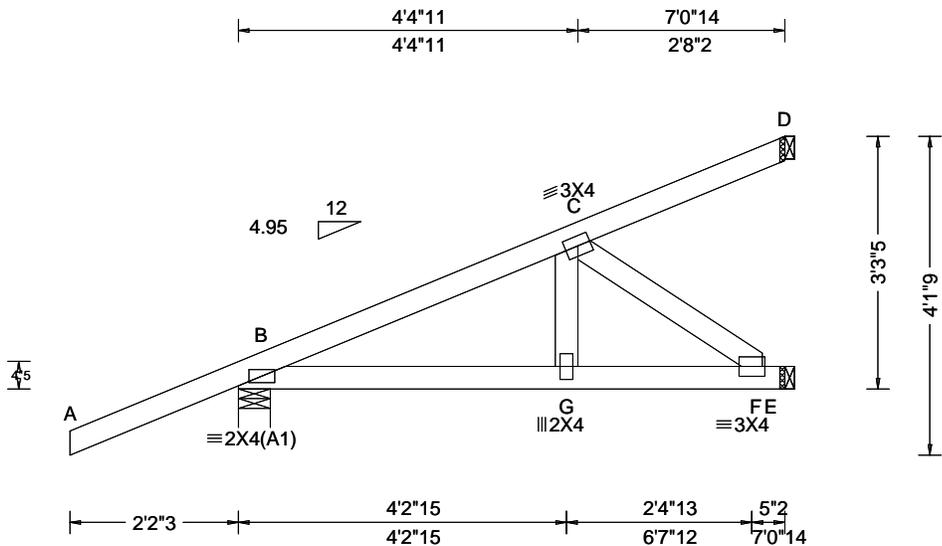
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp1: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.006 G 999 240 VERT(CL): 0.011 G 999 180 HORZ(LL): -0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.306 Max BC CSI: 0.198 Max Web CSI: 0.029 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 283 /- /- /- /61 /- E 191 /- /- /- /7 /- D 112 /- /- /- /41 /- Wind reactions based on MWFRS B Brg Wid = 4.9 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

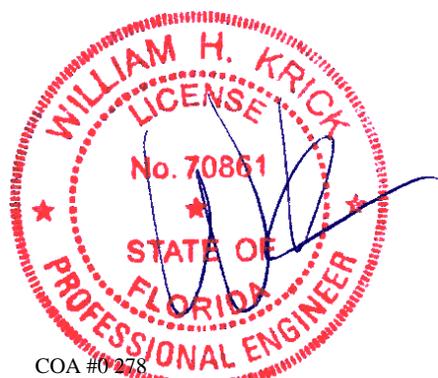
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Hipjack supports 5-0-0 setback jacks with no webs.

Wind

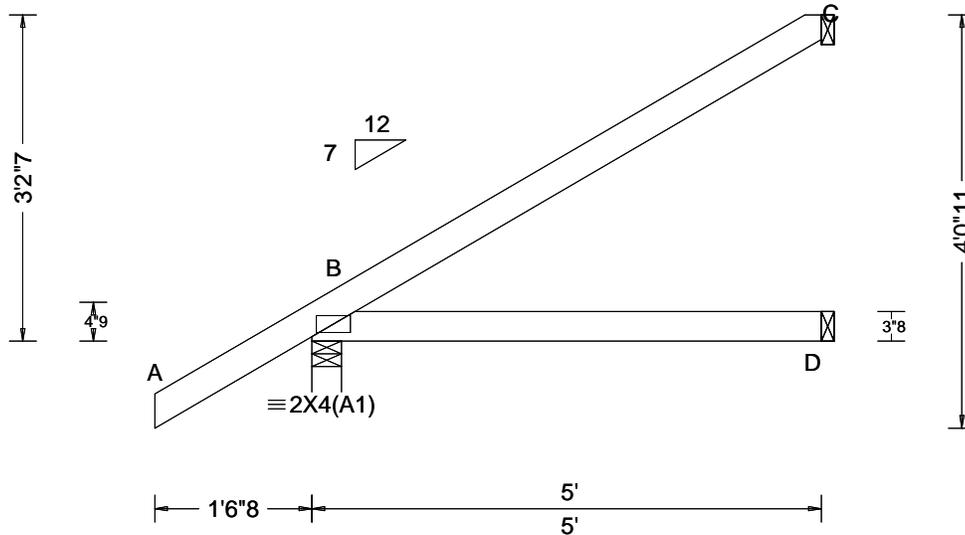
Wind loads and reactions based on MWFRS.
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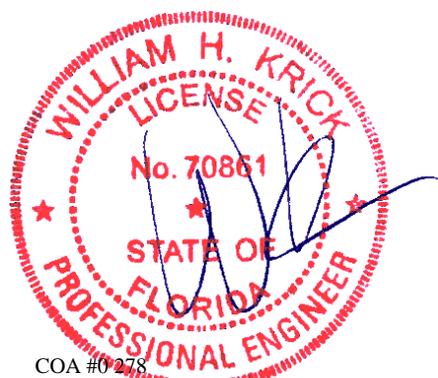
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.007 B - - Creep Factor: 2.0 Max TC CSI: 0.330 Max BC CSI: 0.238 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 338 /- /- /236 /36 /125 D 90 /- /- /52 /- /- C 119 /- /- /78 /69 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

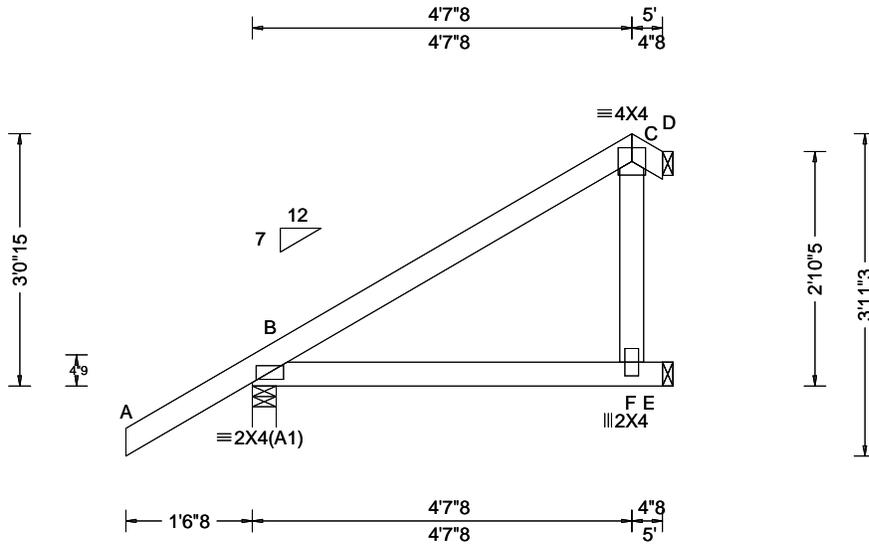


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SEQN: 56686 FROM:	JACK Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: J09	Cust: R215 JRef: 1Xec2150005 T85 DrwNo: 094.22.1023.49823 AK / WHK 04/04/2022
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.009 B 999 180 HORZ(LL): 0.007 D - - HORZ(TL): 0.014 D - - Creep Factor: 2.0 Max TC CSI: 0.278 Max BC CSI: 0.218 Max Web CSI: 0.126 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 338 /- /- /236 /40 /118 E 207 /- /- /123 /121 /- D 75 /-25 /- /74 /11 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



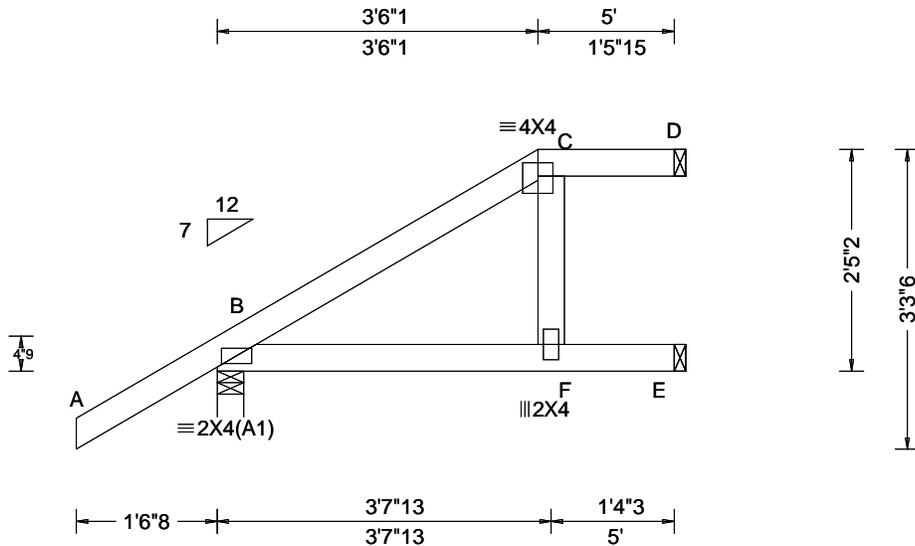
COA #0 278

04/04/2022
 Florida Certificate of Product Approval #FL 1999

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SEQN: 56688 FROM:	JACK Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: J10	Cust: R 215 JRef: 1Xec2150005 T61 DrwNo: 094.22.1023.51737 AK / WHK 04/04/2022
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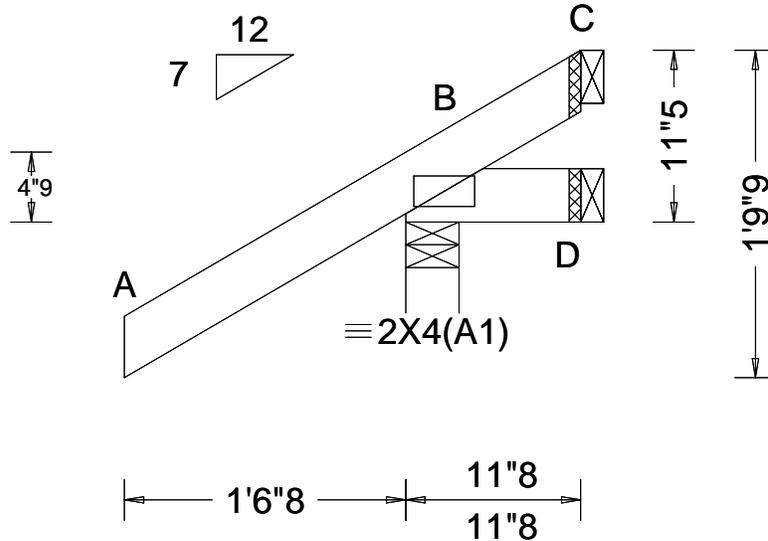
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp1: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.021 C 999 240 VERT(CL): 0.042 C 999 180 HORZ(LL): 0.013 C - - HORZ(TL): 0.026 C - - Creep Factor: 2.0 Max TC CSI: 0.212 Max BC CSI: 0.231 Max Web CSI: 0.090 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity <table border="1"> <thead> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>338</td> <td>-</td> <td>-</td> <td>/238</td> <td>/48</td> <td>/98</td> </tr> <tr> <td>E</td> <td>101</td> <td>-</td> <td>-</td> <td>/64</td> <td>/23</td> <td>-</td> </tr> <tr> <td>D</td> <td>81</td> <td>-</td> <td>/0</td> <td>/42</td> <td>/16</td> <td>/0</td> </tr> </tbody> </table> Non-Gravity B Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#	Loc	R+	/R-	/Rh	/Rw	/U	/RL	B	338	-	-	/238	/48	/98	E	101	-	-	/64	/23	-	D	81	-	/0	/42	/16	/0
				Loc	R+	/R-	/Rh	/Rw	/U	/RL																						
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D	81	-	/0	/42	/16	/0																										
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc. Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.																																



COA #0 278
01/04/2022
Florida Certificate of Product Approval #FL 1999

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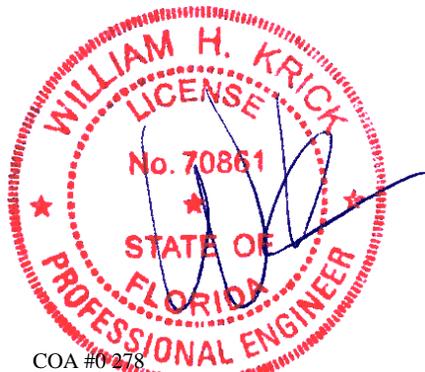
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.040 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>269</td> <td>/-</td> <td>/-</td> <td>/218</td> <td>/67</td> <td>/44</td> </tr> <tr> <td>D</td> <td>4</td> <td>/-19</td> <td>/-</td> <td>/15</td> <td>/16</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-67</td> <td>/-</td> <td>/38</td> <td>/64</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	269	/-	/-	/218	/67	/44	D	4	/-19	/-	/15	/16	/-	C	-	/-67	/-	/38	/64	/-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

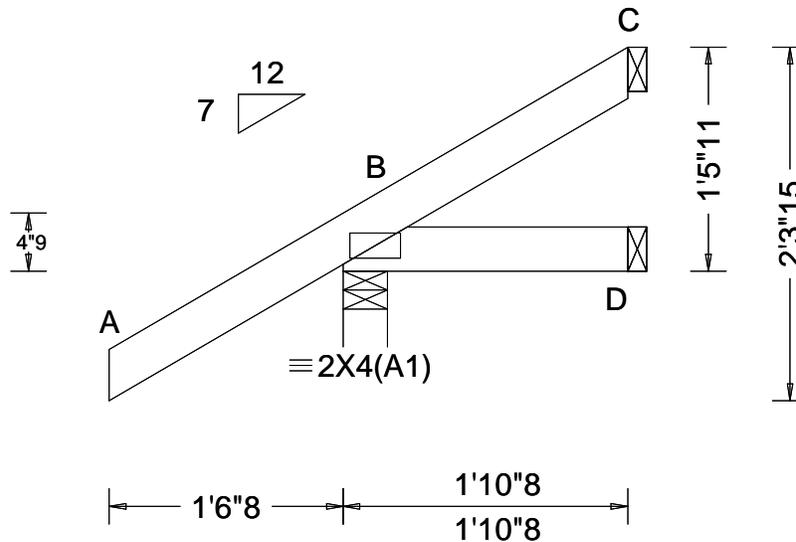


COA #0218

Florida Certificate of Product Approval #FL 1999

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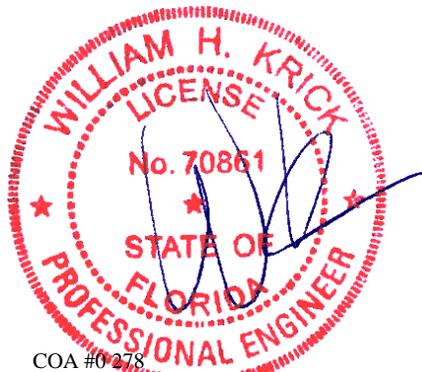
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.057 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>244</td> <td>/-</td> <td>/-</td> <td>/186</td> <td>/43</td> <td>/63</td> </tr> <tr> <td>D</td> <td>26</td> <td>/-</td> <td>/-</td> <td>/21</td> <td>/4</td> <td>/-</td> </tr> <tr> <td>C</td> <td>13</td> <td>/-</td> <td>/-</td> <td>/27</td> <td>/17</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	244	/-	/-	/186	/43	/63	D	26	/-	/-	/21	/4	/-	C	13	/-	/-	/27	/17	/-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

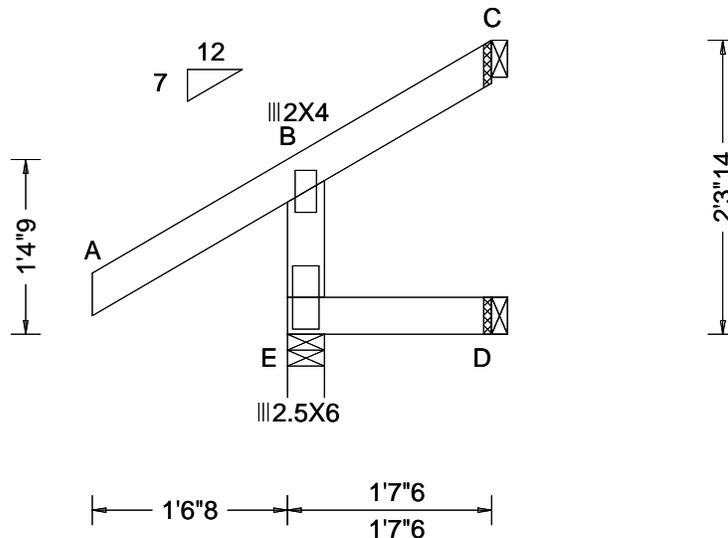


COA #0218

Florida Certificate of Product Approval #FL 1999

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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

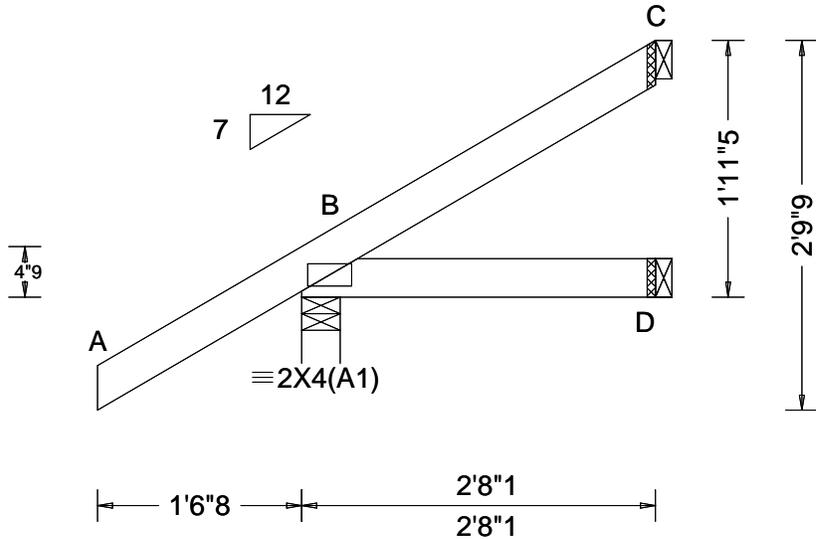


COA #0278

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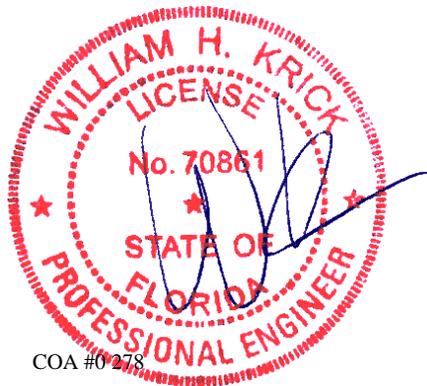
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.048 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>260</td> <td>-</td> <td>-</td> <td>/191</td> <td>/38</td> <td>/79</td> </tr> <tr> <td>D</td> <td>43</td> <td>-</td> <td>-</td> <td>/29</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>49</td> <td>-</td> <td>-</td> <td>/33</td> <td>/33</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	260	-	-	/191	/38	/79	D	43	-	-	/29	-	-	C	49	-	-	/33	/33	-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

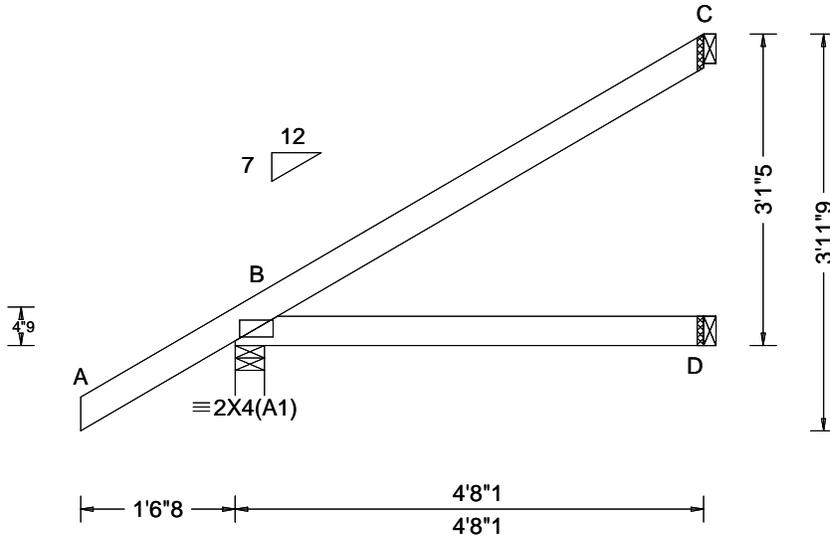


COA #0278

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.003 B - - HORZ(TL): 0.005 B - - Creep Factor: 2.0 Max TC CSI: 0.280 Max BC CSI: 0.203 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity <table border="1"> <thead> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>326</td> <td>-</td> <td>-</td> <td>/228</td> <td>/34</td> <td>/121</td> </tr> <tr> <td>D</td> <td>84</td> <td>-</td> <td>-</td> <td>/49</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>119</td> <td>-</td> <td>-</td> <td>/76</td> <td>/67</td> <td>-</td> </tr> </tbody> </table> Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#	Loc	R+	/R-	/Rh	/Rw	/U	/RL	B	326	-	-	/228	/34	/121	D	84	-	-	/49	-	-	C	119	-	-	/76	/67	-
				Loc	R+	/R-	/Rh	/Rw	/U	/RL																						
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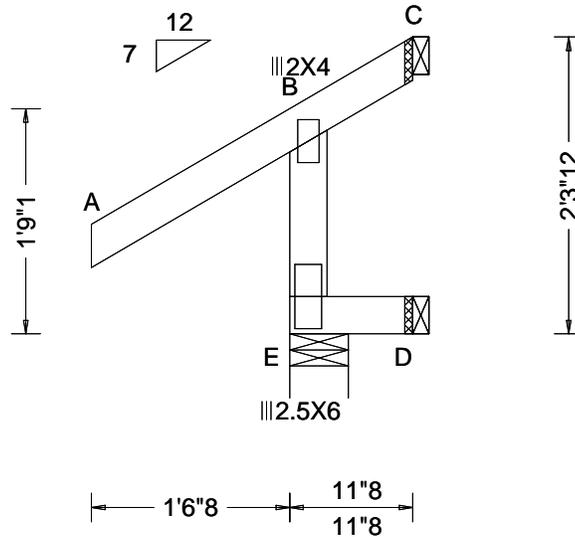


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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCcpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.009 Max Web CSI: 0.133 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>228</td> <td>/-</td> <td>/-</td> <td>/207</td> <td>/79</td> <td>/-</td> </tr> <tr> <td>D</td> <td>19</td> <td>/-</td> <td>/-</td> <td>/10</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>-</td> <td>/-54</td> <td>/-</td> <td>/56</td> <td>/78</td> <td>/44</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	E	228	/-	/-	/207	/79	/-	D	19	/-	/-	/10	/-	/-	C	-	/-54	/-	/56	/78	/44
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

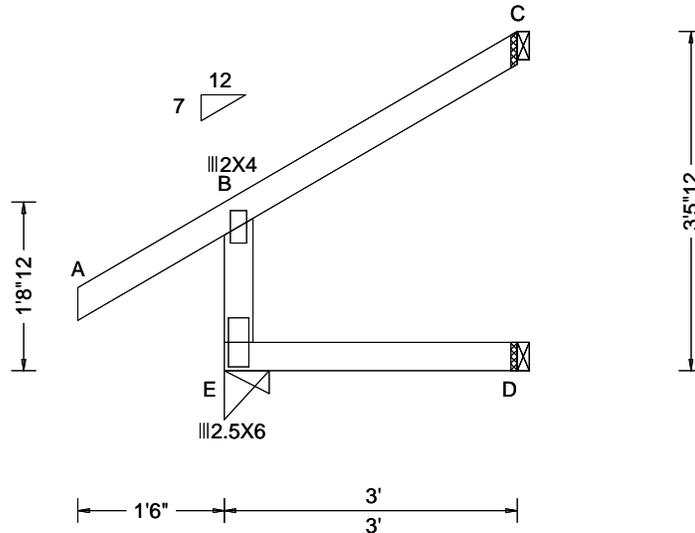


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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.17 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.210 Max BC CSI: 0.098 Max Web CSI: 0.102 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>252</td> <td>/-</td> <td>/-</td> <td>/213</td> <td>/50</td> <td>/-</td> </tr> <tr> <td>D</td> <td>60</td> <td>/-</td> <td>/-</td> <td>/30</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>69</td> <td>/-</td> <td>/-</td> <td>/57</td> <td>/18</td> <td>/61</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	E	252	/-	/-	/213	/50	/-	D	60	/-	/-	/30	/-	/-	C	69	/-	/-	/57	/18	/61
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

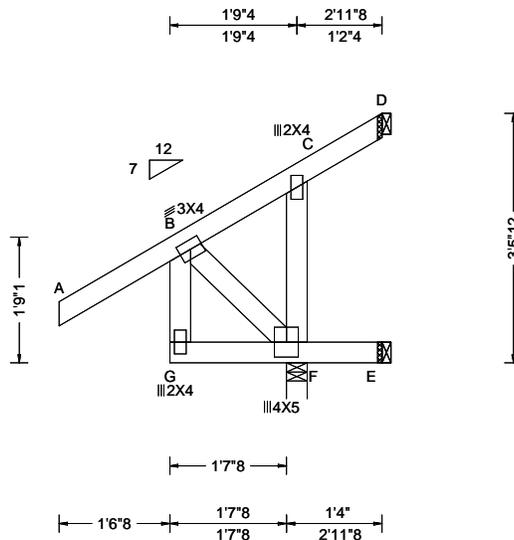


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Lumber

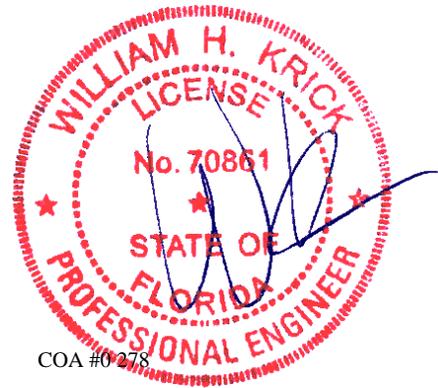
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -214# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

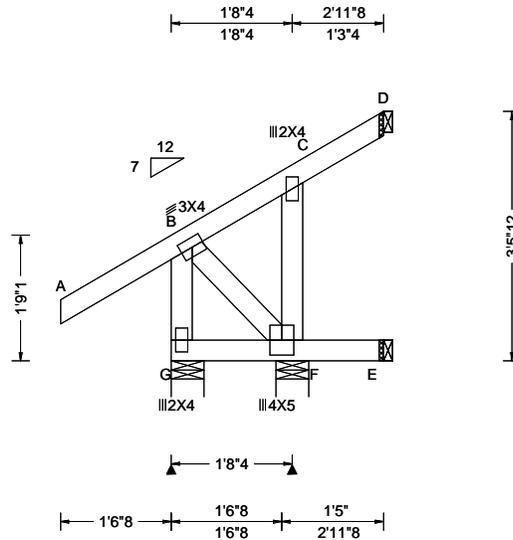


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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.17 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.001 B 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.221 Max BC CSI: 0.023 Max Web CSI: 0.078 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 222 /- /- /126 /13 /86 F 90 /- /- /93 /91 /- E 13 /- /- /7 /- /- D 35 /- /- /26 /18 /- Wind reactions based on MWFRS G Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 5.5 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearings G & F are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

Shim all supports to solid bearing.

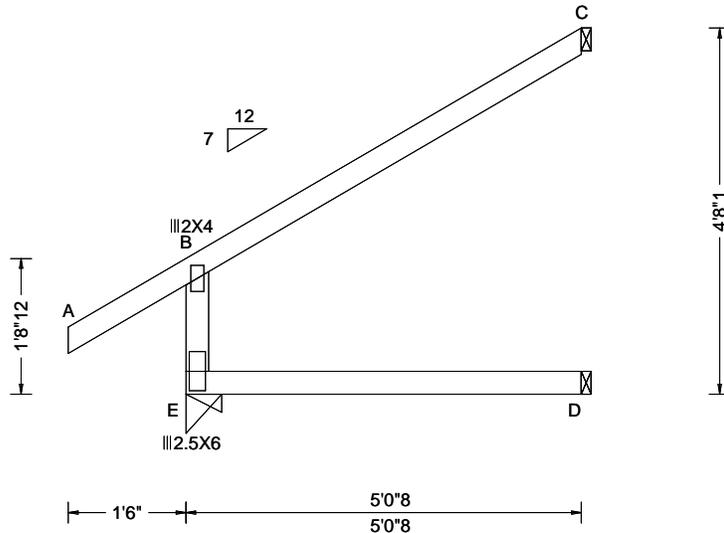


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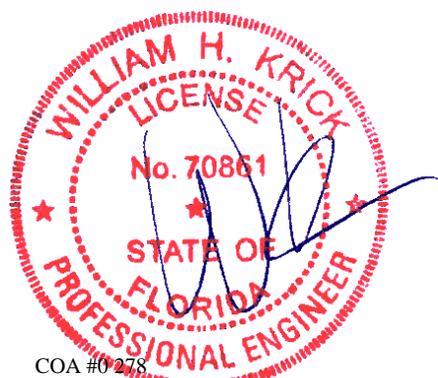
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				B - E 420 -276					

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

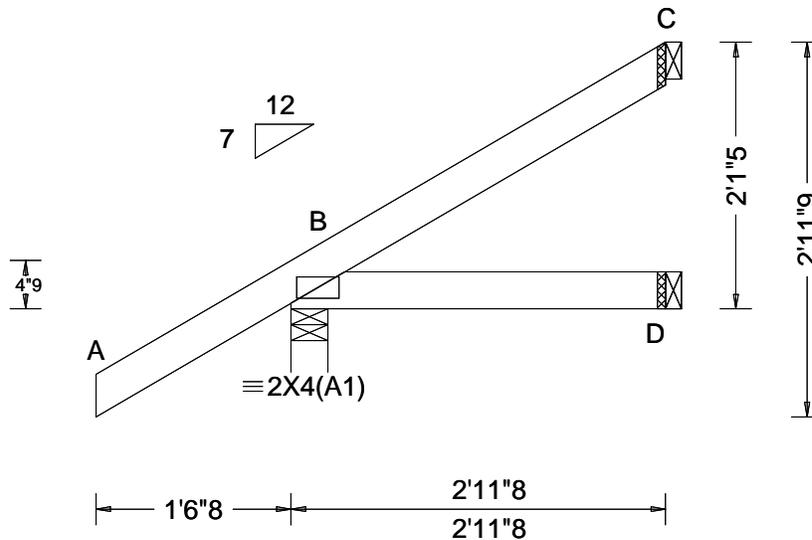
Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



COA #0278
 04/04/2022
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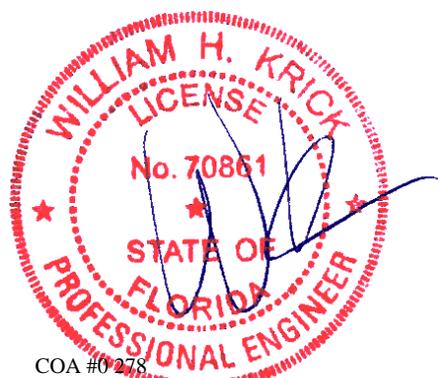
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp1: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.199 Max BC CSI: 0.063 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>268</td> <td>-</td> <td>-</td> <td>/195</td> <td>/37</td> <td>/85</td> </tr> <tr> <td>D</td> <td>49</td> <td>-</td> <td>-</td> <td>/32</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>61</td> <td>-</td> <td>-</td> <td>/36</td> <td>/38</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	268	-	-	/195	/37	/85	D	49	-	-	/32	-	-	C	61	-	-	/36	/38	-
				Loc		Gravity			Non-Gravity																													
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

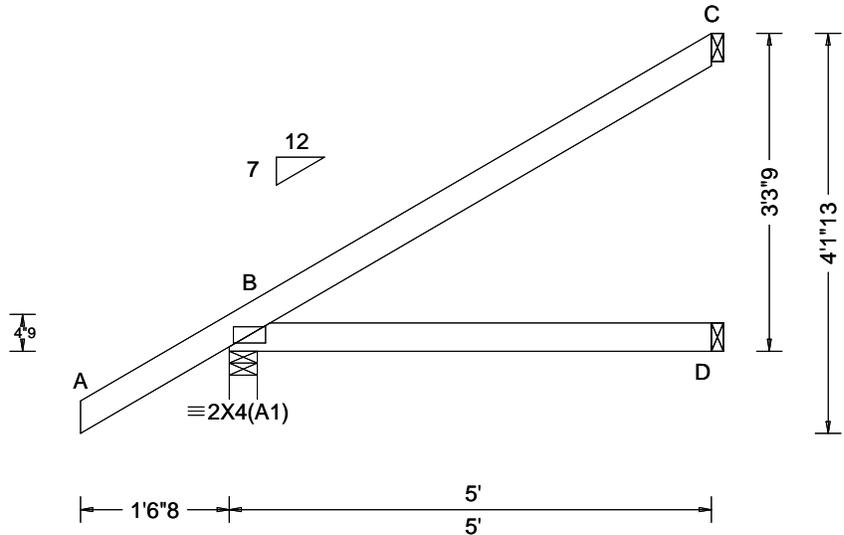
Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.007 B - - Creep Factor: 2.0 Max TC CSI: 0.331 Max BC CSI: 0.238 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) Gravity <table border="1"> <thead> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>338</td> <td>-</td> <td>-</td> <td>/235</td> <td>/34</td> <td>/128</td> </tr> <tr> <td>D</td> <td>90</td> <td>-</td> <td>-</td> <td>/52</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>129</td> <td>-</td> <td>-</td> <td>/83</td> <td>/72</td> <td>-</td> </tr> </tbody> </table> Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#	Loc	R+	/R-	/Rh	/Rw	/U	/RL	B	338	-	-	/235	/34	/128	D	90	-	-	/52	-	-	C	129	-	-	/83	/72	-
				Loc	R+	/R-	/Rh	/Rw	/U	/RL																						
B	338	-	-	/235	/34	/128																										
D	90	-	-	/52	-	-																										
C	129	-	-	/83	/72	-																										
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.																																

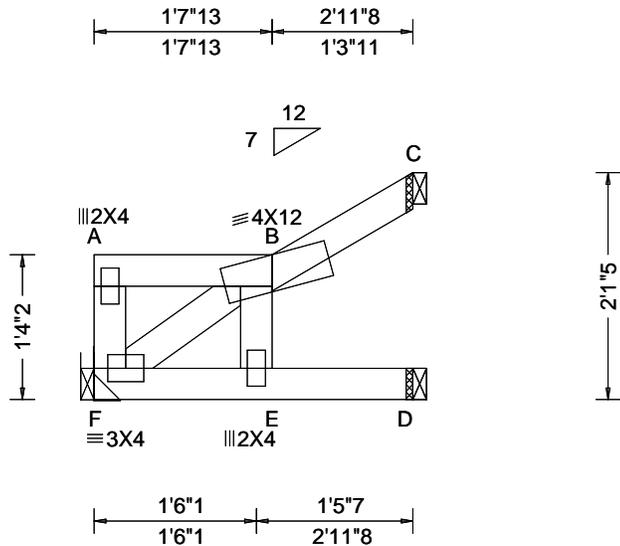


COA #0278

Florida Certificate of Product Approval #FL 1999

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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.009 B 999 240 VERT(CL): 0.018 B 999 180 HORZ(LL): 0.006 A - - HORZ(TL): 0.012 A - - Creep Factor: 2.0 Max TC CSI: 0.059 Max BC CSI: 0.171 Max Web CSI: 0.061 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>F</td> <td>123</td> <td>-</td> <td>-</td> <td>/62</td> <td>/11</td> <td>/25</td> </tr> <tr> <td>D</td> <td>82</td> <td>-</td> <td>-</td> <td>/54</td> <td>/9</td> <td>-</td> </tr> <tr> <td>C</td> <td>41</td> <td>-</td> <td>-</td> <td>/27</td> <td>/23</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	F	123	-	-	/62	/11	/25	D	82	-	-	/54	/9	-	C	41	-	-	/27	/23	-
				Loc		Gravity			Non-Gravity																													
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Wind reactions based on MWFRS F Brg Wid = - Min Req = - D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Members not listed have forces less than 375#																																						

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties

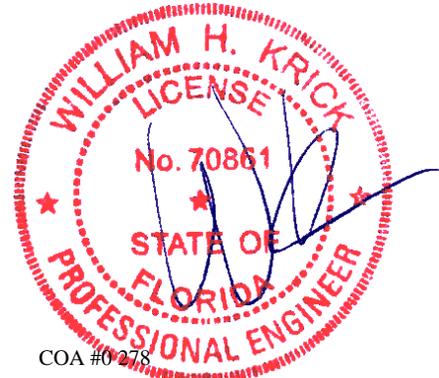
(J) Hanger Support Required, by others

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

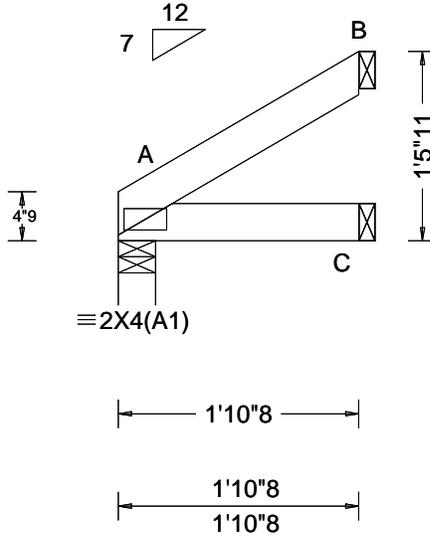
Wind loads based on MWFRS with additional C&C member design.
 Left end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



COA #0218
 04/04/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.000 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.044 Max BC CSI: 0.028 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A 74 /- /- /- /19 /- C 33 /- /- /2 /- /- B 37 /- /- /- /10 /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) C Brg Wid = 1.5 Min Req = - B Brg Wid = 1.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at 0.00 to 63 plf at 1.88
BC: From 10 plf at 0.00 to 10 plf at 1.88
TC: -2 lb Conc. Load at 1.88
BC: -10 lb Conc. Load at 1.88

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

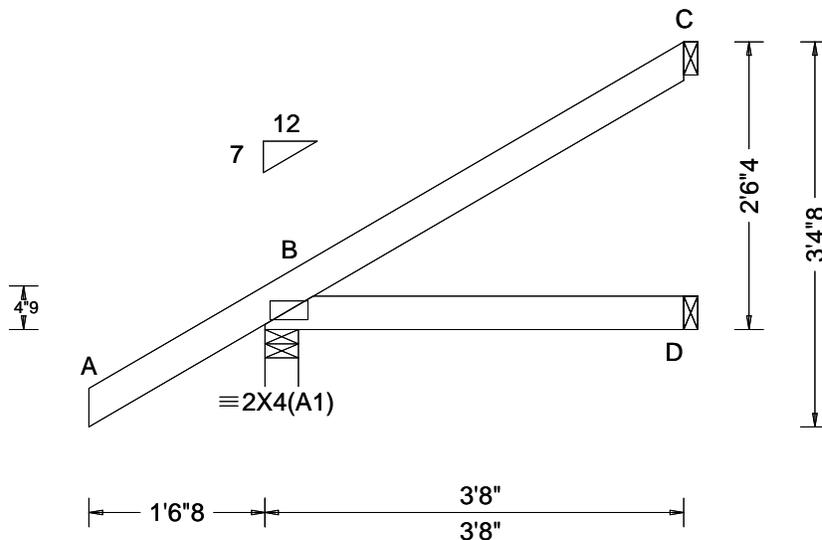


COA #0278

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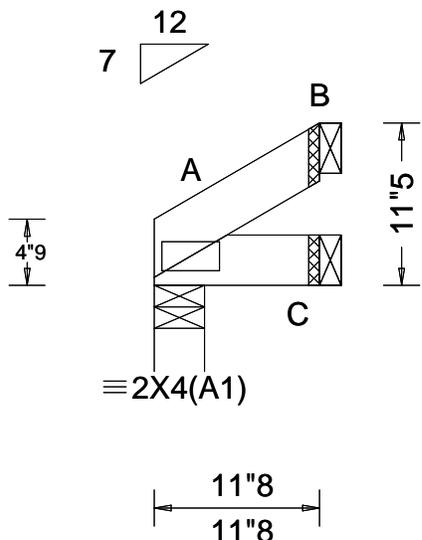
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				Loc	R+	/R-	/Rh	/Rw	/U	/RL																						
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D	63	-	-	/39	-	-																										
C	86	-	-	/53	/50	-																										
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.																																



COA #0 278
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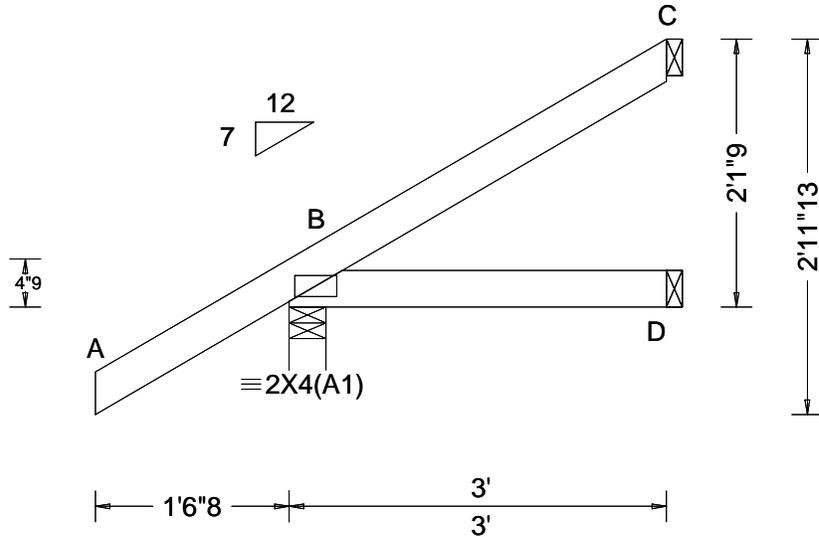
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 A - - HORZ(TL): 0.000 A - - Creep Factor: 2.0 Max TC CSI: 0.013 Max BC CSI: 0.007 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>47</td> <td>/-</td> <td>/-</td> <td>/28</td> <td>/-</td> <td>/18</td> </tr> <tr> <td>C</td> <td>15</td> <td>/-</td> <td>/-</td> <td>/10</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>B</td> <td>22</td> <td>/-</td> <td>/-</td> <td>/15</td> <td>/14</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) C Brg Wid = 1.5 Min Req = - B Brg Wid = 1.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	47	/-	/-	/28	/-	/18	C	15	/-	/-	/10	/-	/-	B	22	/-	/-	/15	/14	/-
				Loc		Gravity			Non-Gravity																													
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A	47	/-	/-	/28	/-	/18																																
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Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.																																						



COA #0 278
04/04/2022
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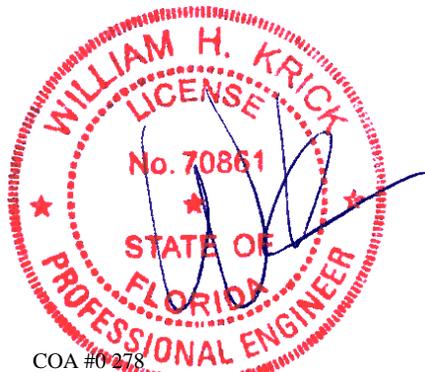
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	B	269	/-	/-	/196	/22	/61
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	D	50	/-	/-	/32	/-	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 B - -	C	62	/-	/-	/37	/25	/-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.001 B - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5 (Truss)						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.199	D Brg Wid = 1.5 Min Req = -						
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.066	C Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	FT/RT:20(0)/10(0)	Max Web CSI: 0.000	Bearing B is a rigid surface.						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.02.00.1005.17	Members not listed have forces less than 375#						
	Loc. from endwall: not in 9.00 ft	WAVE								
	GCp1: 0.18									
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

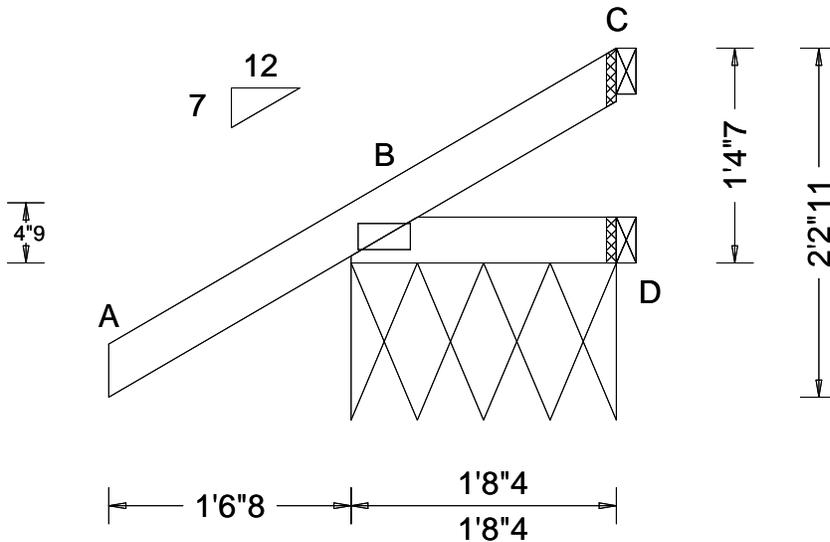


COA #0218

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.056 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 144 /- /- /111 /27 /35 D 22 /-1 /- /20 /6 /- C 2 /- /- /26 /22 /- Wind reactions based on MWFRS D Brg Wid = 20.3 Min Req = - D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

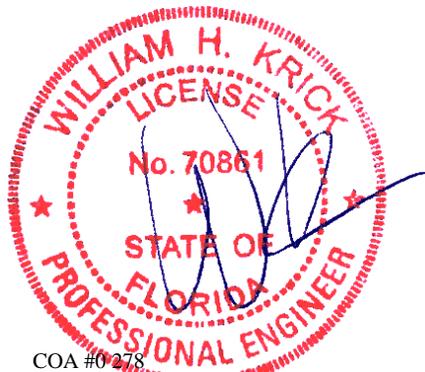
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

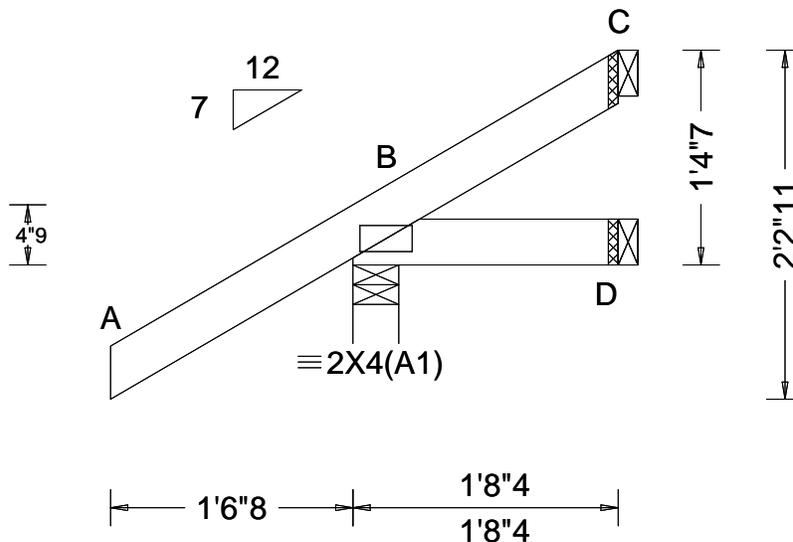


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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.258 Max BC CSI: 0.056 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1216.15	▲ Maximum Reactions (lbs) Gravity <table border="1"> <thead> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>243</td> <td>/-</td> <td>/-</td> <td>/187</td> <td>/45</td> <td>/59</td> </tr> <tr> <td>D</td> <td>22</td> <td>/-1</td> <td>/-</td> <td>/20</td> <td>/6</td> <td>/-</td> </tr> <tr> <td>C</td> <td>2</td> <td>/-</td> <td>/-</td> <td>/26</td> <td>/22</td> <td>/-</td> </tr> </tbody> </table> Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#	Loc	R+	/R-	/Rh	/Rw	/U	/RL	B	243	/-	/-	/187	/45	/59	D	22	/-1	/-	/20	/6	/-	C	2	/-	/-	/26	/22	/-
				Loc	R+	/R-	/Rh	/Rw	/U	/RL																						
B	243	/-	/-	/187	/45	/59																										
D	22	/-1	/-	/20	/6	/-																										
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Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.																																

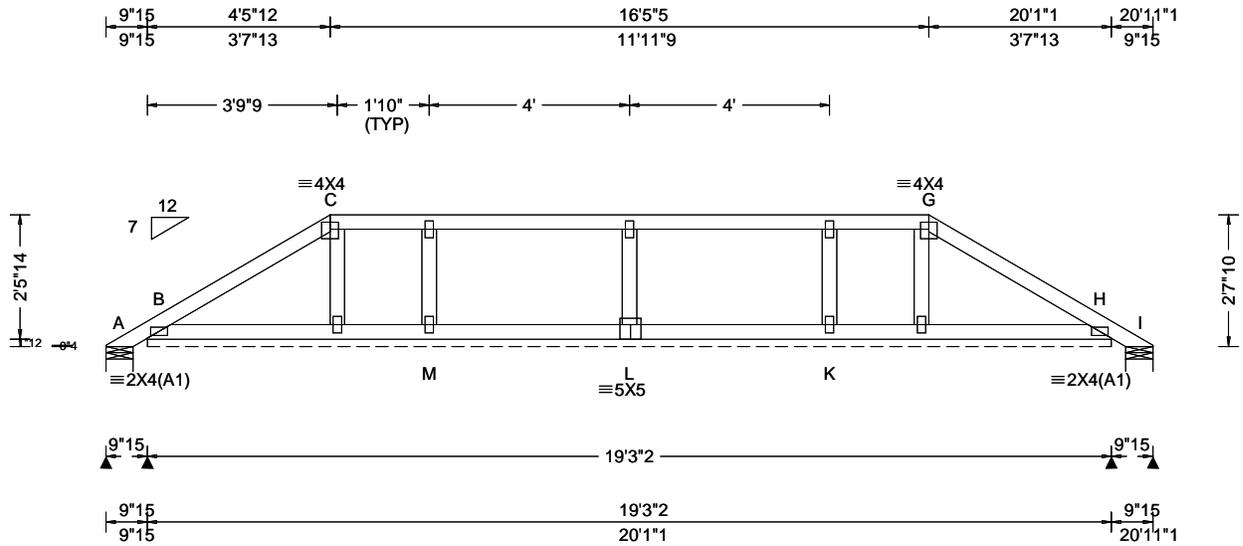


COA #0278

04/04/2022
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.33 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.49 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.002 B 999 180 HORZ(LL): 0.001 H - - - HORZ(TL): 0.002 H - - - Creep Factor: 2.0 Max TC CSI: 0.184 Max BC CSI: 0.051 Max Web CSI: 0.057 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-54 /- /56 /84 /68 B* 77 /- /- /49 /20 /- I - /-54 /- /22 /50 /- B /-128 M /-222 L /-286 K /-222 H /-125 Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 231 Min Req = - I Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.



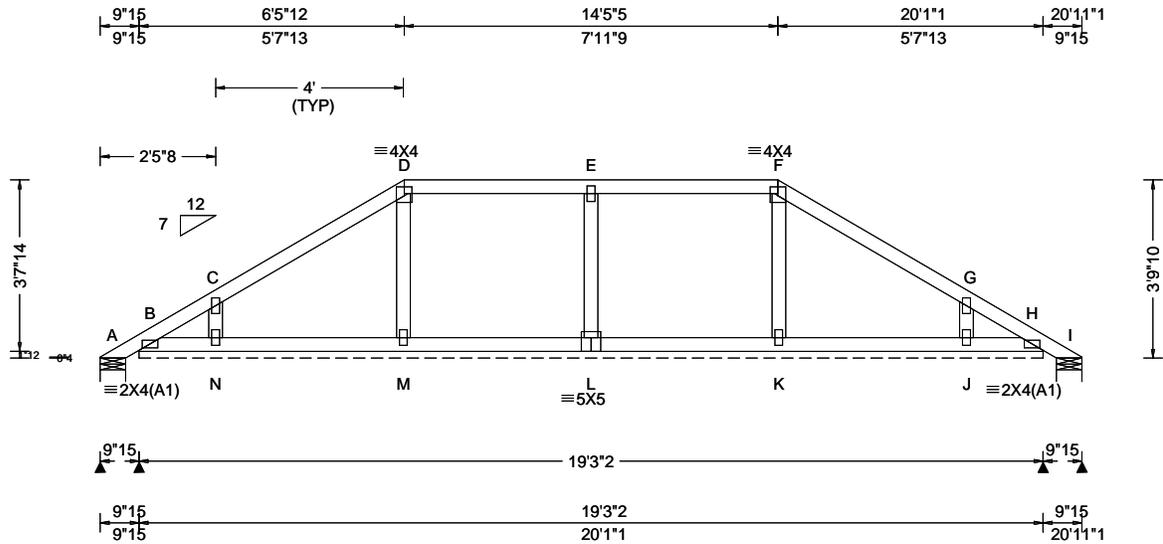
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SEQN: 57111 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: PB02	Cust: R215 JRef: 1Xec2150005 T34 DrwNo: 094.22.1026.21563 AK / WHK 04/04/2022
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.91 ft TCCL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.66 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 E 999 240 VERT(CL): 0.002 E 999 180 HORZ(LL): 0.001 G - - HORZ(TL): 0.001 G - - Creep Factor: 2.0 Max TC CSI: 0.235 Max BC CSI: 0.052 Max Web CSI: 0.084 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>32</td> <td>/-</td> <td>/-</td> <td>/64</td> <td>/44</td> <td>/101</td> </tr> <tr> <td>B*</td> <td>68</td> <td>/-</td> <td>/-</td> <td>/46</td> <td>/13</td> <td>/-</td> </tr> <tr> <td>I</td> <td>32</td> <td>/-</td> <td>/-</td> <td>/26</td> <td>/7</td> <td>/-</td> </tr> <tr> <td>L</td> <td colspan="6">/-111</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	32	/-	/-	/64	/44	/101	B*	68	/-	/-	/46	/13	/-	I	32	/-	/-	/26	/7	/-	L	/-111					
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Purlins

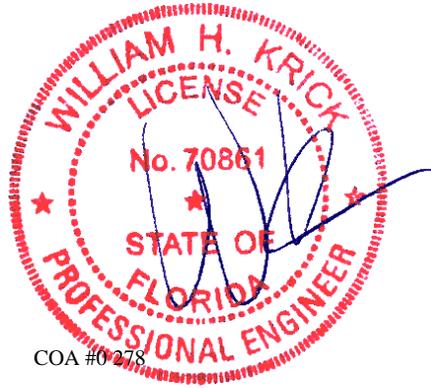
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.

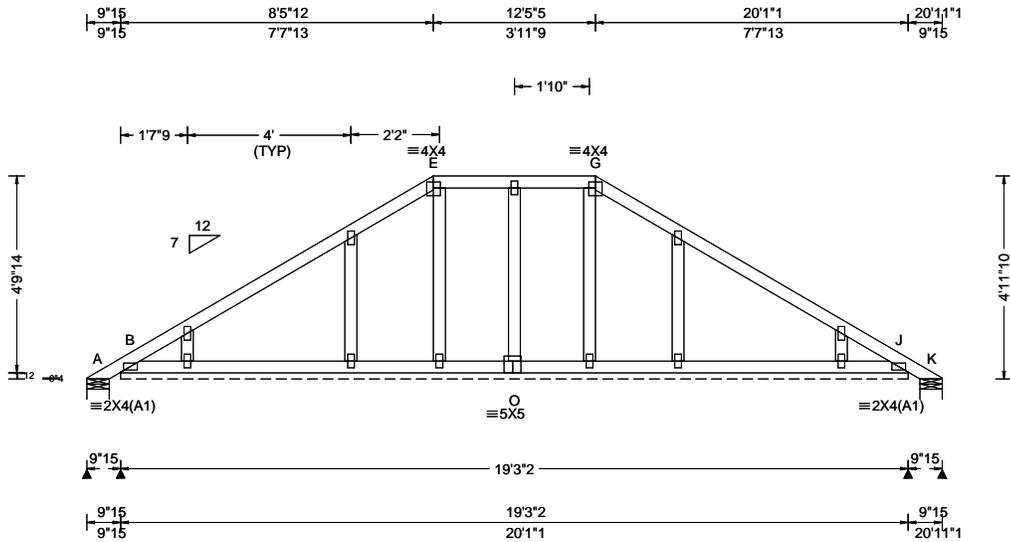


COA #0278

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.73 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.49 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.001 D 999 180 HORZ(LL): 0.001 H - - HORZ(TL): 0.002 H - - Creep Factor: 2.0 Max TC CSI: 0.137 Max BC CSI: 0.045 Max Web CSI: 0.063 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 27 /- /- /82 /63 /136 B* 68 /- /- /48 /8 /- K 27 /- /- /23 /5 /- Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 231 Min Req = - K Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & K are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.

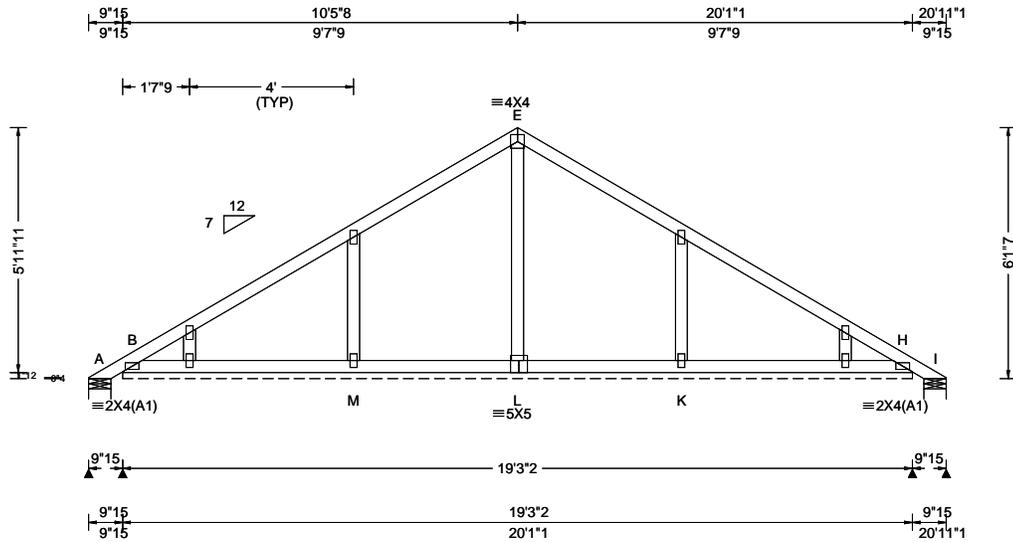


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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.07 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.49 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 E 999 240 VERT(CL): 0.002 F 999 180 HORZ(LL): 0.002 F - - HORZ(TL): 0.003 F - - Creep Factor: 2.0 Max TC CSI: 0.215 Max BC CSI: 0.054 Max Web CSI: 0.113 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 25 /- /- /90 /82 /167 B* 68 /- /- /48 /13 /- I 25 /- /- /13 /6 /- M /-144 K /-144 Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 231 Min Req = - I Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.

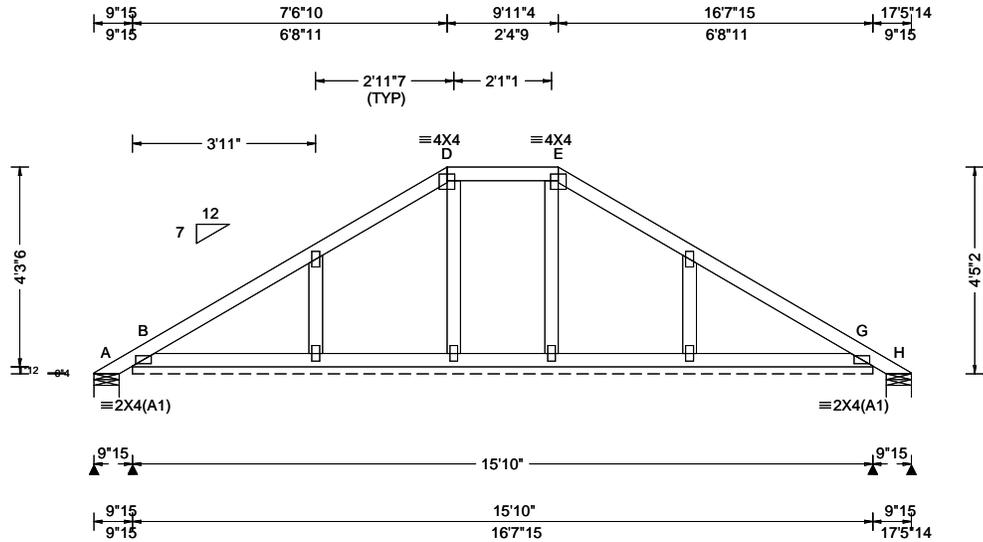


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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 40.00 NCBC LL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.96 ft TC DL: 5.0 psf BC DL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.32 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.002 B 999 180 HORZ(LL): 0.001 G - - HORZ(TL): 0.002 G - - Creep Factor: 2.0 Max TC CSI: 0.134 Max BC CSI: 0.056 Max Web CSI: 0.051 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-35</td> <td>/-</td> <td>/78</td> <td>/95</td> <td>/121</td> </tr> <tr> <td>B*</td> <td>77</td> <td>/-</td> <td>/-</td> <td>/54</td> <td>/10</td> <td>/-</td> </tr> <tr> <td>H</td> <td>-</td> <td>/-35</td> <td>/-</td> <td>/18</td> <td>/35</td> <td>/-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 190 Min Req = - H Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & H are a rigid surface. Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-35	/-	/78	/95	/121	B*	77	/-	/-	/54	/10	/-	H	-	/-35	/-	/18	/35	/-
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B*	77	/-	/-	/54	/10	/-																																
H	-	/-35	/-	/18	/35	/-																																

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Refer to DWG PB160160118 for piggyback details.



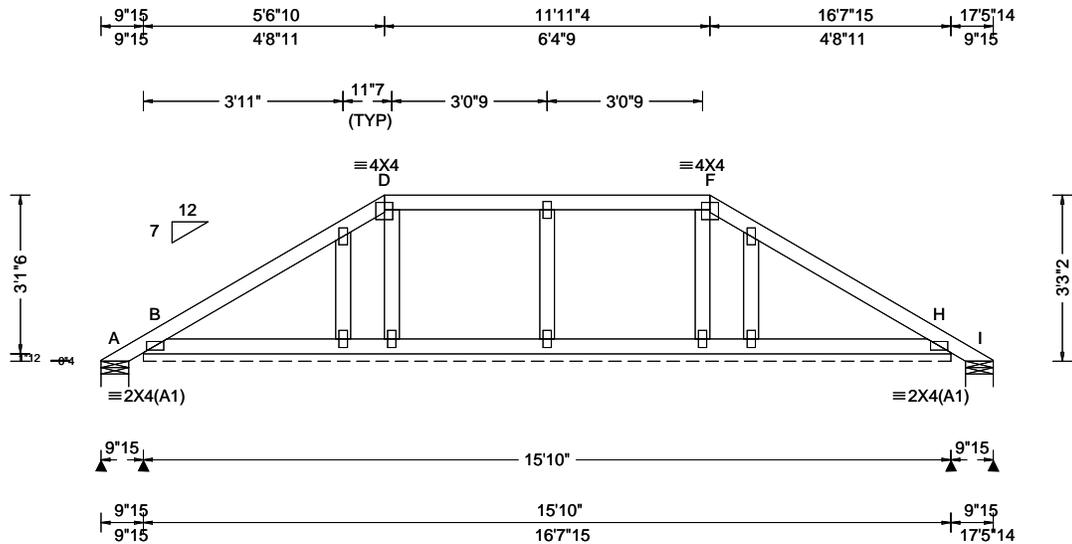
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SEQN: 57104 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: PB06	Cust: R215 JRef: 1Xec2150005 T58 DrwNo: 094.22.1026.31273 AK / WHK 04/04/2022
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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.37 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.32 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 H 999 240 VERT(CL): 0.002 H 999 180 HORZ(LL): 0.001 H - - HORZ(TL): 0.002 H - - Creep Factor: 2.0 Max TC CSI: 0.147 Max BC CSI: 0.052 Max Web CSI: 0.056 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-42</td> <td>/-</td> <td>/63</td> <td>/83</td> <td>/88</td> </tr> <tr> <td>B*</td> <td>77</td> <td>/-</td> <td>/-</td> <td>/52</td> <td>/10</td> <td>/-</td> </tr> <tr> <td>I</td> <td>-</td> <td>/-42</td> <td>/-</td> <td>/19</td> <td>/39</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-42	/-	/63	/83	/88	B*	77	/-	/-	/52	/10	/-	I	-	/-42	/-	/19	/39	/-
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
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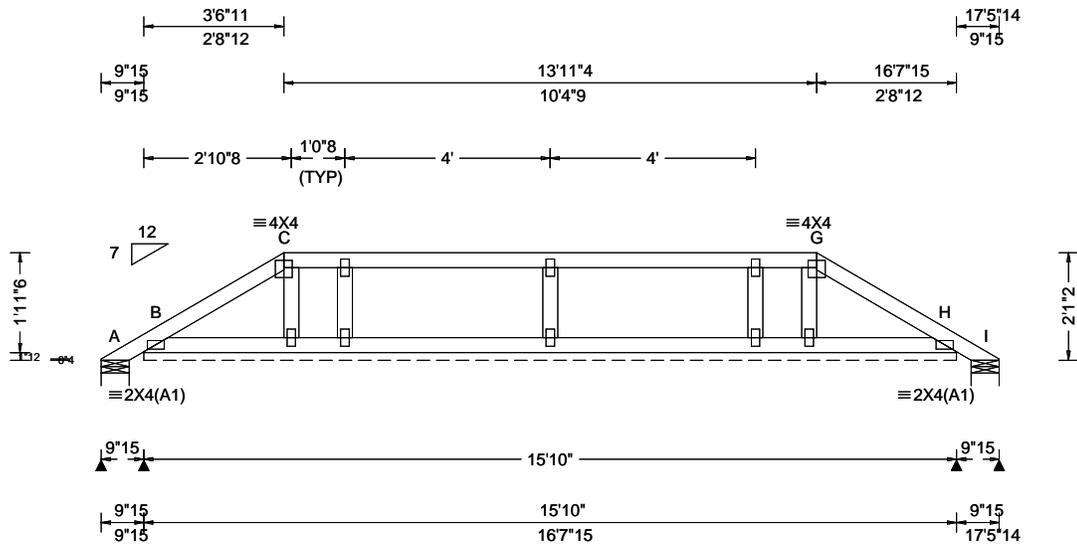
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SEQN: 57106 FROM:	COMN Ply: 1 Qty: 1	Job Number: 22-7193 Poirrier Res Truss Label: PB07	Cust: R215 JRef: 1Xec2150005 T80 DrwNo: 094.22.1026.33893 AK / WHK 04/04/2022
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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 40.00 NCBC LL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.79 ft TC DL: 5.0 psf BC DL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.32 ft Loc. from endwall: not in 13.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 H 999 240 VERT(CL): 0.001 H 999 180 HORZ(LL): 0.001 H - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.186 Max BC CSI: 0.051 Max Web CSI: 0.051 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL
				A - /-13 /- /35 /38 /55 B* 74 /- /- /48 /10 /- I - /-13 /- /7 /10 /- Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 189 Min Req = - I Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

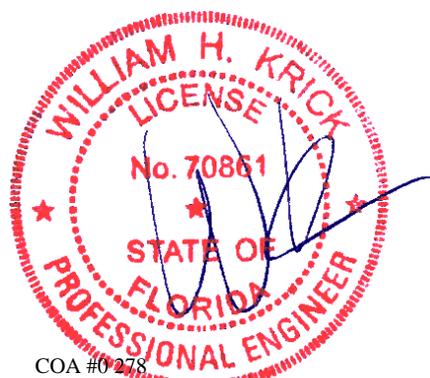
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Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

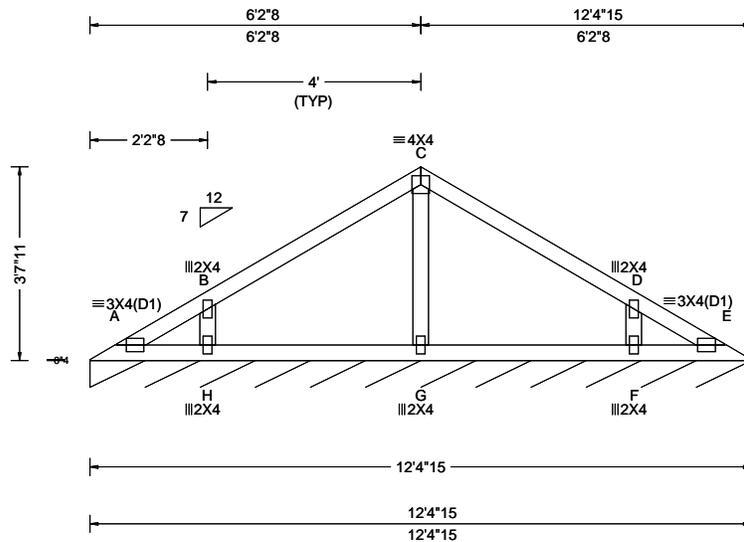
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.

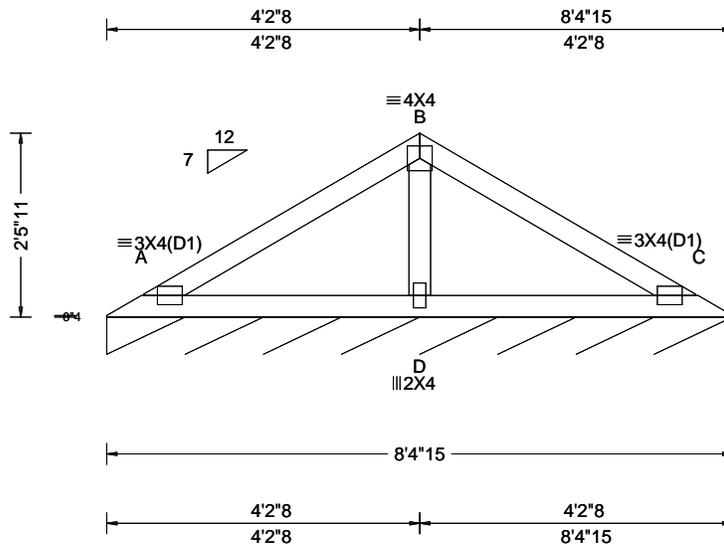


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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.07 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.008 A 999 240 VERT(CL): 0.016 A 999 180 HORZ(LL): -0.004 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.224 Max BC CSI: 0.194 Max Web CSI: 0.080 VIEW Ver: 21.02.00.1005.17	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL C* 83 /- /- /41 /13 /7 Wind reactions based on MWFRS C Brg Wid = 100 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 260 -403

Lumber

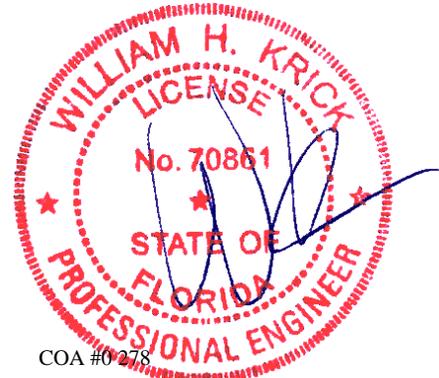
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.

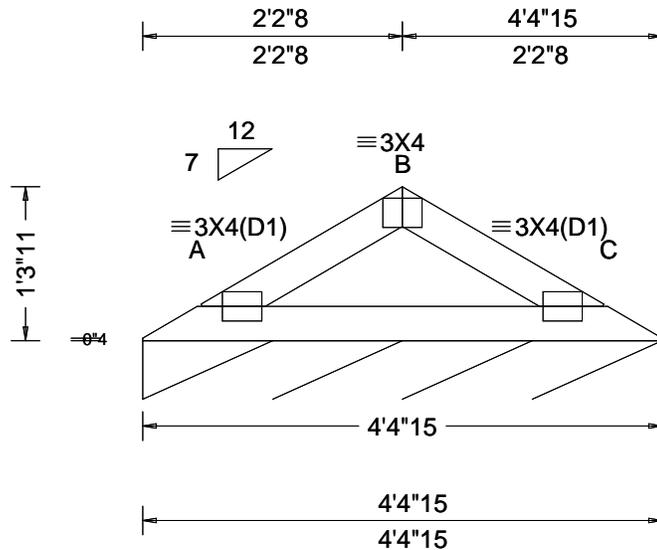


COA #0218
 04/04/2022

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 A 999 240	C*	82	/-	/-	/39	/9	/6
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.009 A 999 180	Wind reactions based on MWFRS						
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 C - -	C Brg Wid = 53.0 Min Req = -						
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.004 C - -	Bearing A is a rigid surface.						
NCBCLL: 10.00	Mean Height: 17.65 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	Members not listed have forces less than 375#						
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.092							
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.117							
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.000							
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.02.00.1005.17							
	Loc. from endwall: not in 4.50 ft	WAVE								
	GCp: 0.18									
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

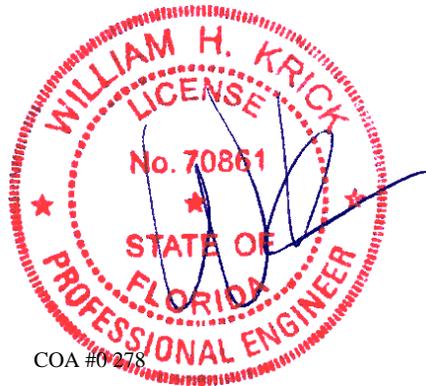
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0278

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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

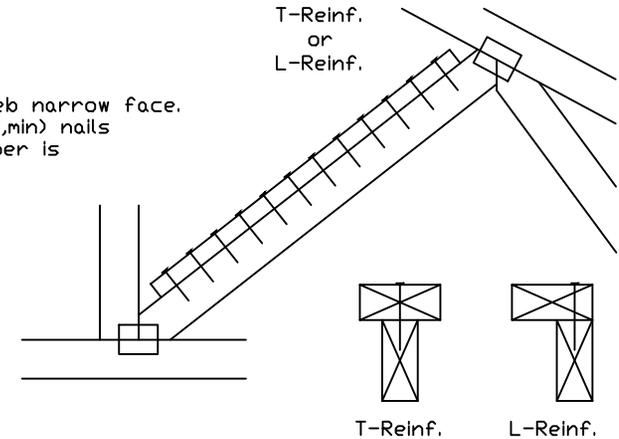
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6 or 2x4	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

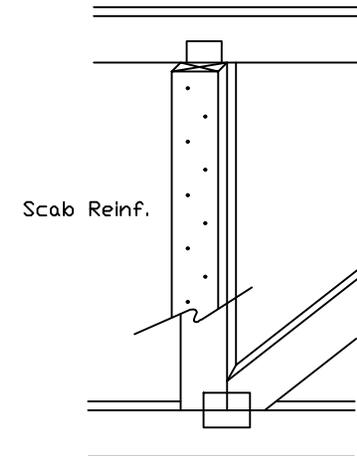
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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Glenview, IL 60025

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CGA #0278 04/04/2022
Florida Certificate of Product Approval #FL 1999

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

REF CLR Subst.
DATE 01/02/19
DRWG BRCLBSUB0119

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

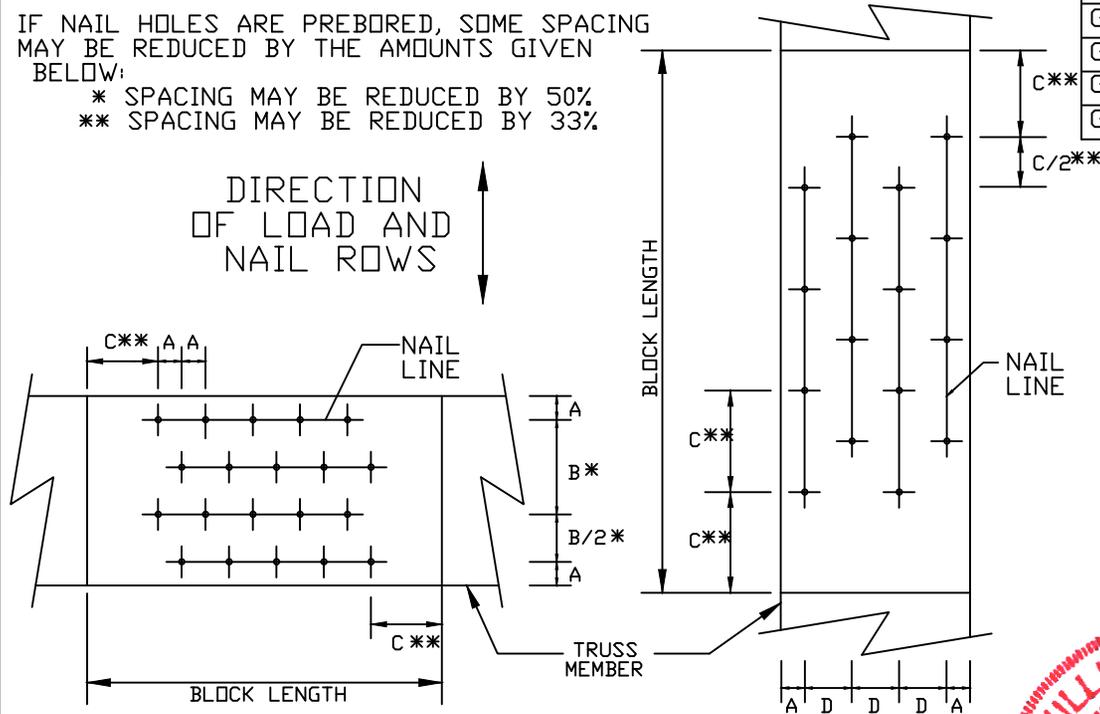
IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

* SPACING MAY BE REDUCED BY 50%

** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

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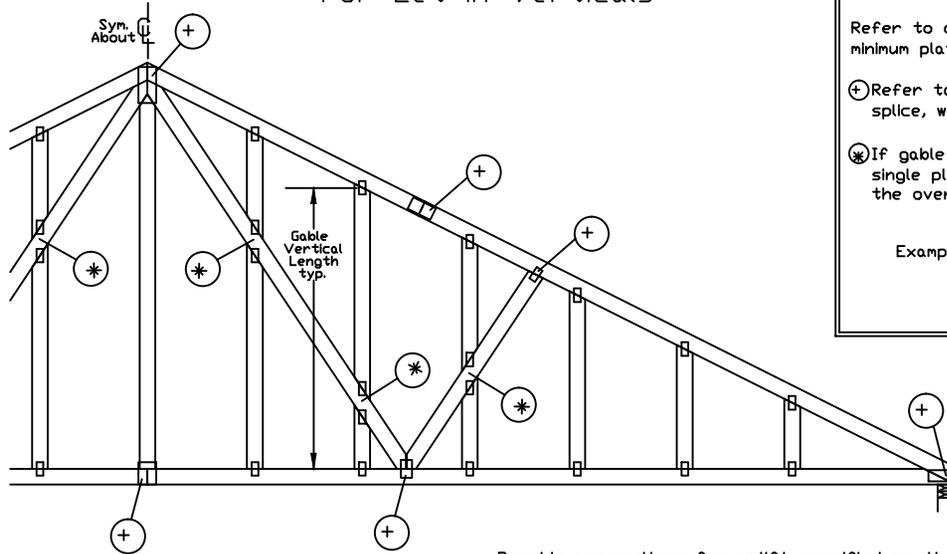


COA #0278 04/04/2022

Florida Certificate of Product Approval #FL 1999

REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014

Gable Detail For Let-in Verticals

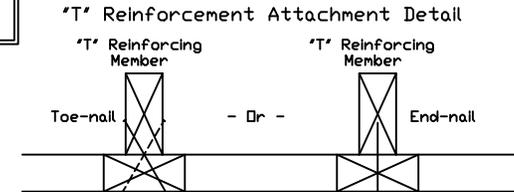


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- ⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.
 Attach each 'T' reinforcing member with
 End Driven Nails:
 10d Common (0.148"x3",min) Nails at 4' o.c. plus
 (4) nails in the top and bottom chords.

Toenailed Nails:
 10d Common (0.148"x3",min) Toenails at 4' o.c. plus
 (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings
 A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
 A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
 A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
 A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
 A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
 A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
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 S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
 S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
 S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum panel for Gable vertical length.

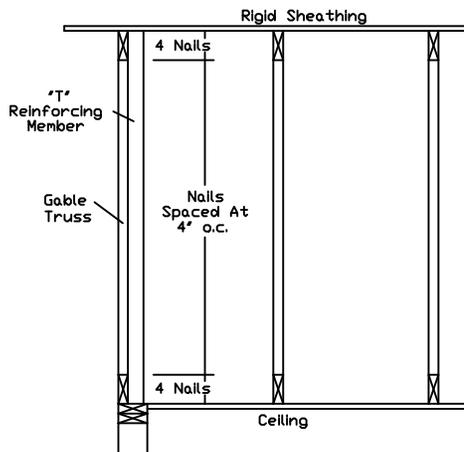
To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.
 'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
 ASCE 7-10 Wind Speed = 120 mph
 Mean Roof Height = 30 ft, Kzt = 1.00
 Gable Vertical = 24' o.c. SP #3
 'T' Reinforcing Member Size = 2x4
 'T' Brace Increase (From Above) = 30% = 1.30
 (1) 2x4 'L' Brace Length = 8' 7"
 Maximum 'T' Reinforced Gable Vertical Length
 1.30 x 8' 7" = 11' 2"



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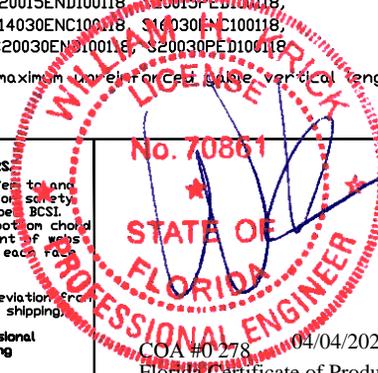
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

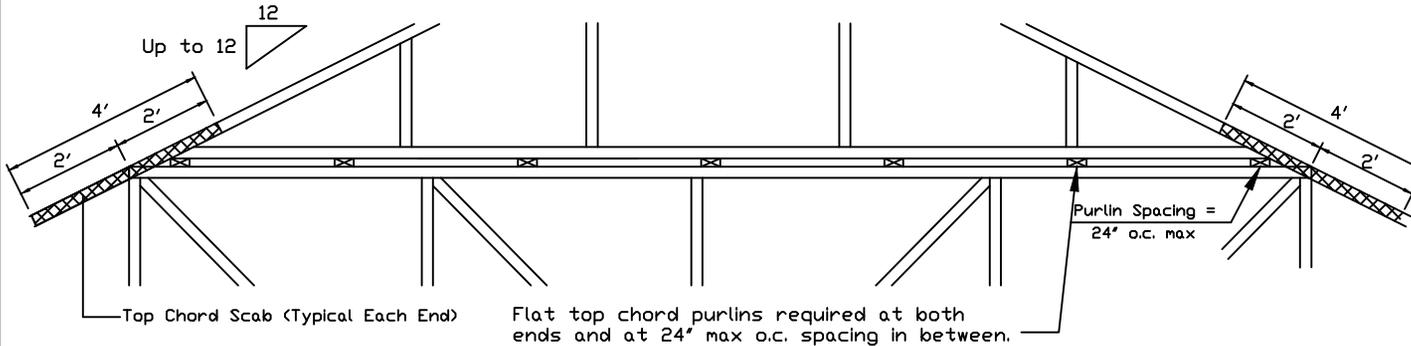
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
 Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

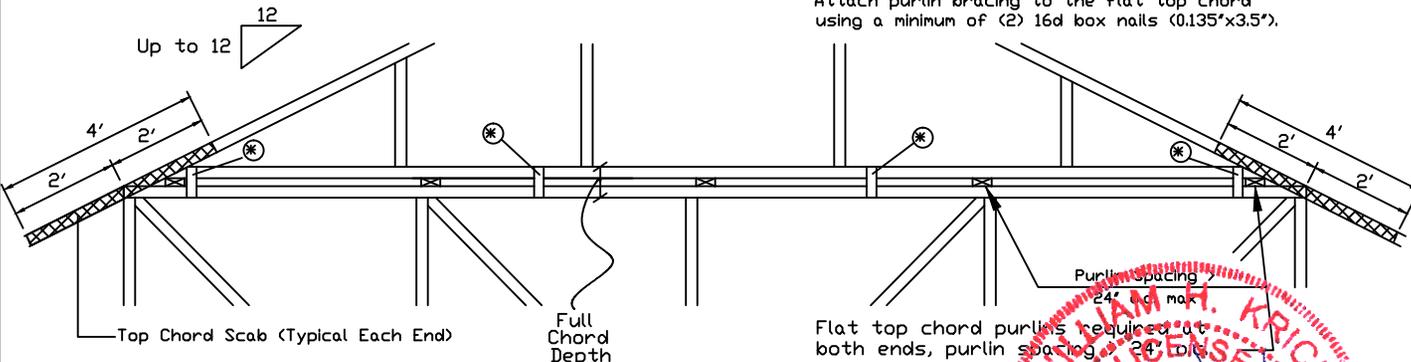


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

- Trulox**
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
- APA Rated Gusset**
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
- 2x4 Vertical Scabs**
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
- 28PB Wave Piggyback Plate**
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.



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 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
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 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

No. 70861
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 COA #0278 04/04/2022
 Florida Certificate of Product Approval #FL 1999
 SPACING 24.0"

REF PIGGYBACK
 DATE 01/02/2018
 DRWG PB160160118

SPACING 24.0"

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 535# connection or with (1) Simpson H2.5A or equivalent connector for
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

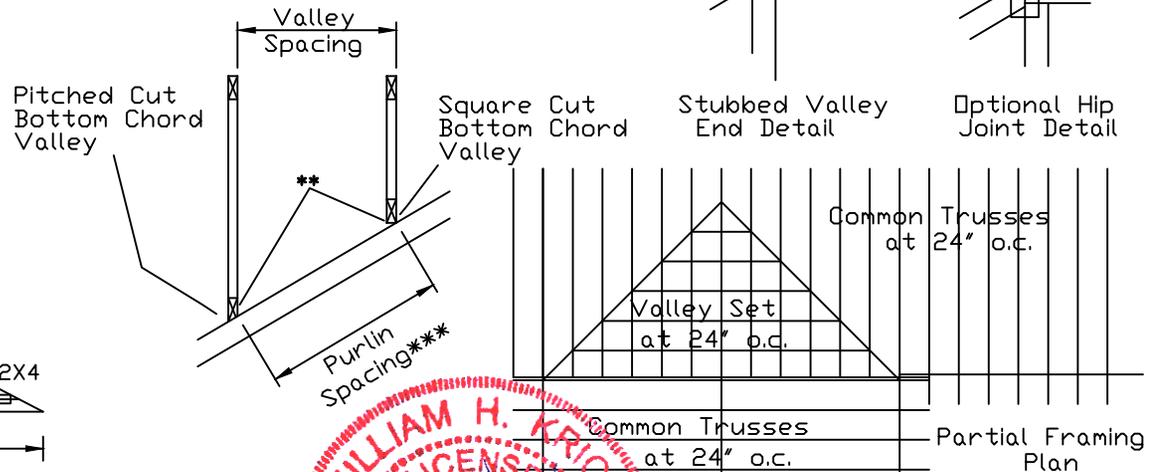
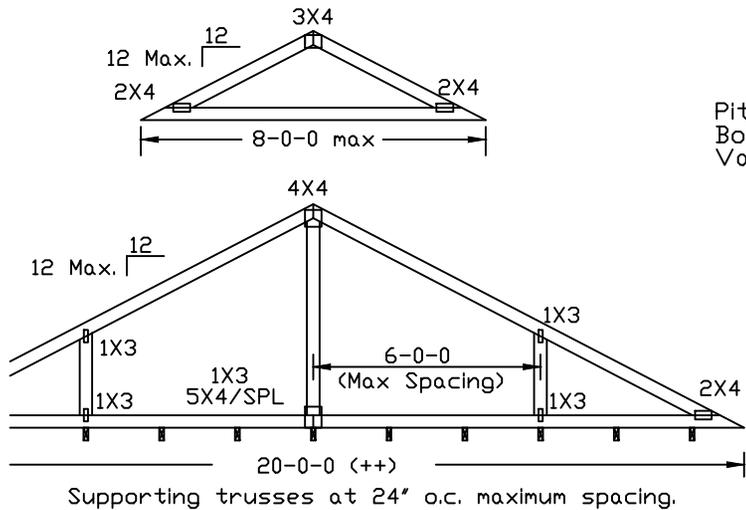
Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

Or
 Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

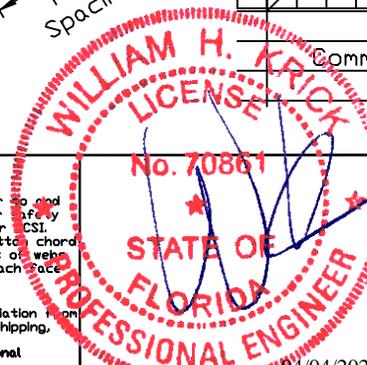
Or
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



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COA #0278 04/04/2022
 Florida Certificate of Product Approval

TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING	FL 1999	24.0"			

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 170 mph for SP (G = 0.55, min.),
 155 mph for DF-L (G = 0.50, min.), or
 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

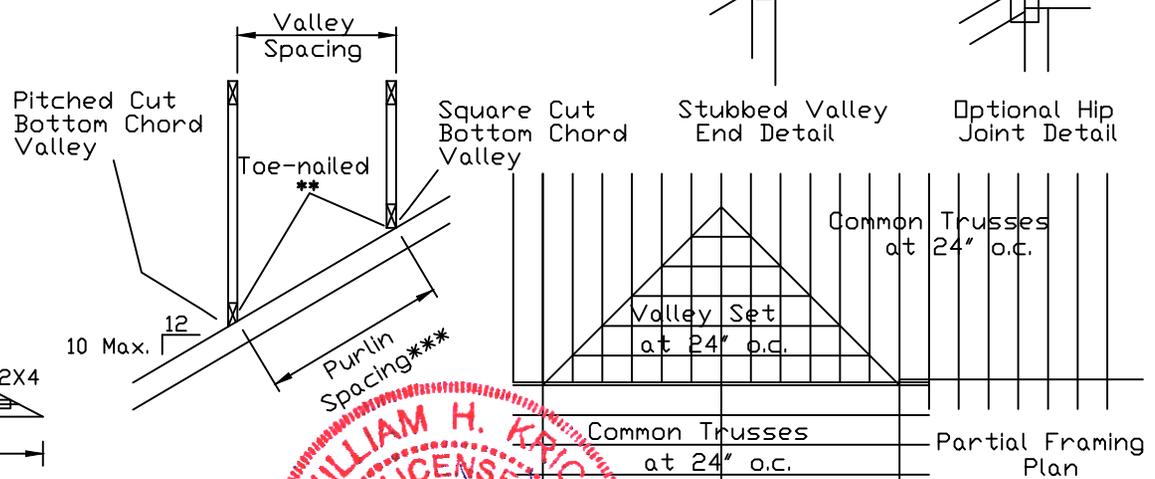
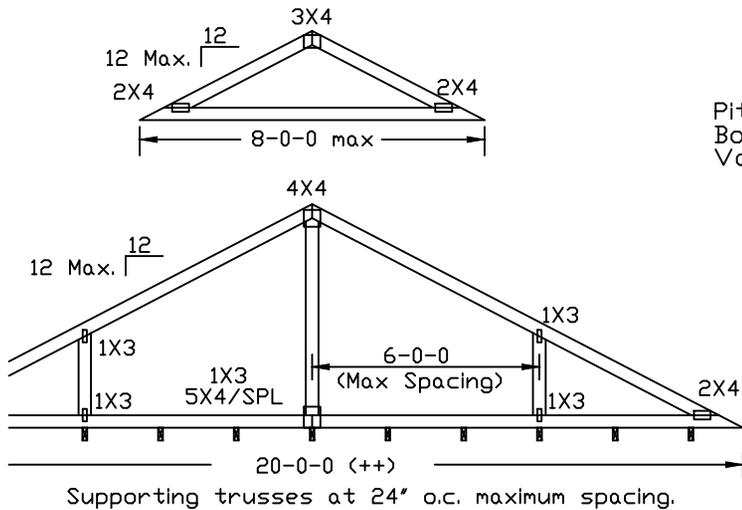
Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

- Or
- Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
- Or
- By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".



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TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10PSF	DRWG	VALTN160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING			24.0"		

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