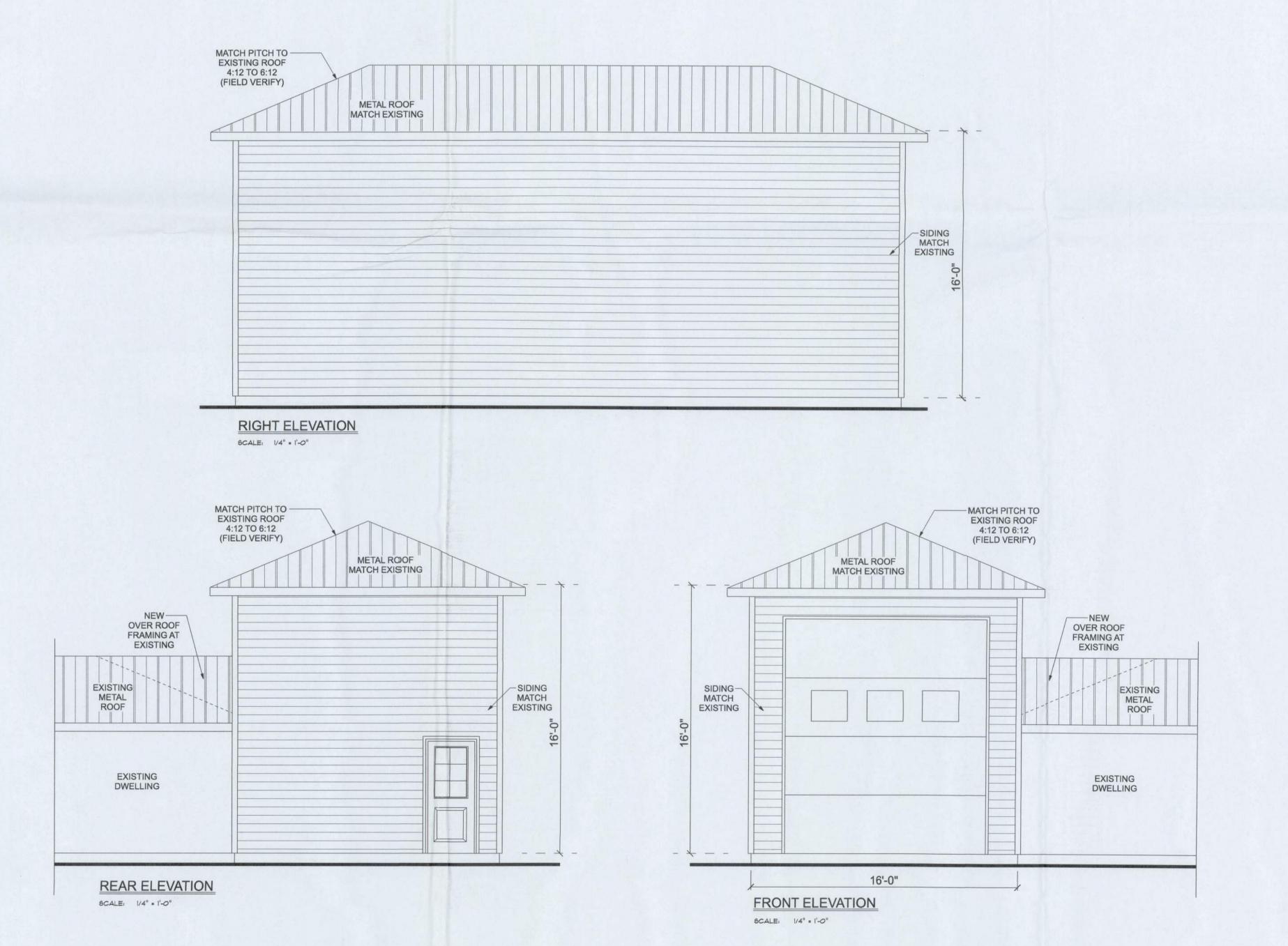
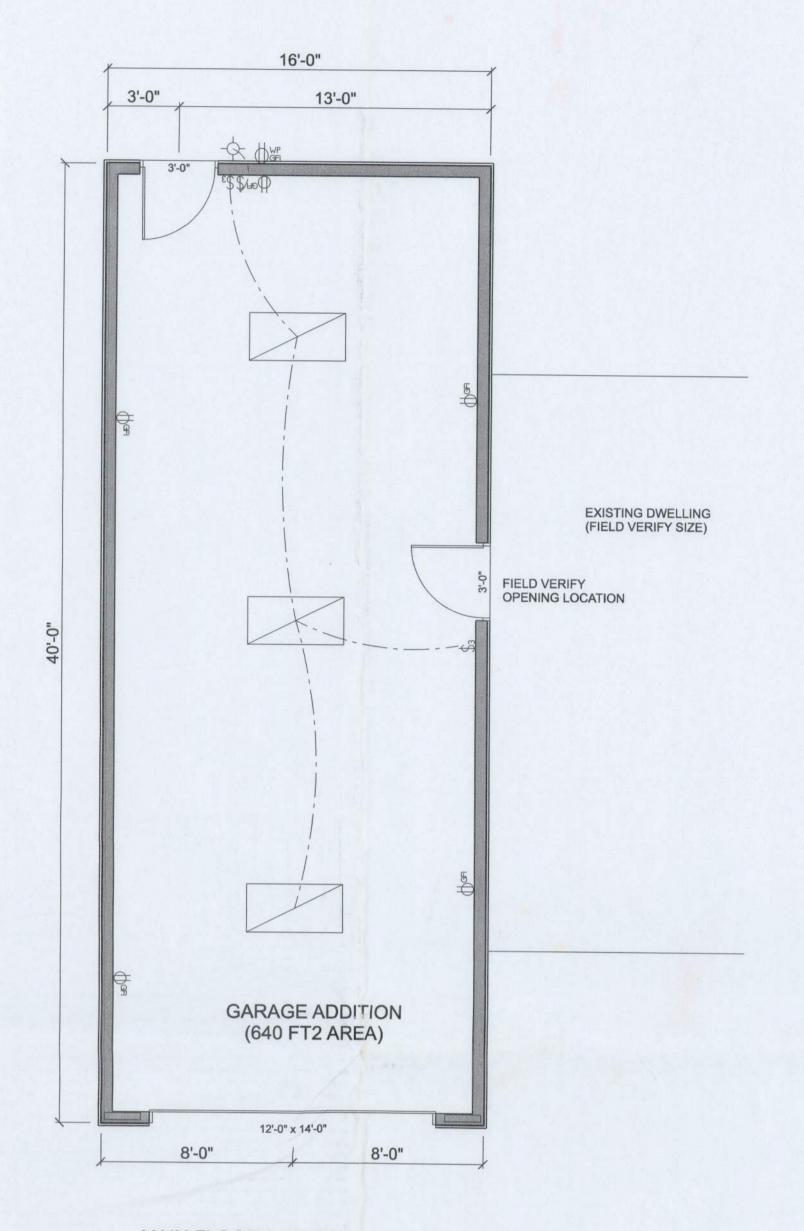


| Π | ELECTRICAL LEGEND |
|--------------|---|
| | CEILING FAN (PRE-WIRE FOR LIGHT KIT) |
| QD | DOUBLE SECURITY LIGHT |
| | 2X4 FLUORESCENT LIGHT FIXTURE |
| 0 | RECESSED CAN LIGHT |
| → | BATH EXAUST FAN WITH LIGHT |
| ₩ | BATH EXAUST FAN |
| - | LIGHT FIXTURE |
| Ф | DUPLEX OUTLET |
| Φ | 220v OUTLET |
| ⊕ GFI | GFI DUPLEX OUTLET |
| • | SMOKE DETECTOR |
| \$ | WALL SWITCH |
| \$3 | 3 WAY WALL SWITCH |
| \$4 | 4 WAY WALL SWITCH |
| ₩P/GFI | WATER PROOF GFI OUTLET |
| ∇ | PHONE JACK |
| 0 | TELEVISION JACK |
| 9 | GARAGE DOOR OPENER |
| ⊕CM | CARBON MONOXIDE ALARM |





MAIN FLOOR LAYOUT

SCALE: 1/4" = 1'-0"

R302.5.1 Opening protection: Openings from a private garage directly into a room used for sleeping purposes shall not be permittedl. Other openings between the garage and residence shall be equipped with solid wood dloors not less than 1 3/8 inches in thickness, solid or honeycomb-core steel doors not less than 1 3/8 inches thick, or 20-minute fire-rated doors, equipped TABLE R:302.6 DWELLING/GARAGE SEPARATION: SEPARATION Not less than 1/2-inch gypsum board From the residence and attics or equivalent applied to the garage side From all habitable rooms above Not less than 5/8-inch Type X gypsum board the garage Structure(s) supporting floor/ceiling Not less than 1/2-inch gypsum board or equivalent

assemblies used for separation required by this section Garages located less than 3 feet from a dwelling unit on the same lot applied to the interior side of exterior walls that are within this area

ROOF PLAN NOTES:

REQUIRED ATTIC ACCESS:

REQUIRED ATTIC ACCESS:
BUILDINGS WITH COMBUSTIBLE CEILING OR ROOF CONSTRUCTION SHALL HAVE ATTIC ACCESS OPENING TO ATTIC AREAS THAT EXCEED 30 SQUARE FEET AND HAVE A VERTICAL HEIGHT OF 30" OR GREATER. THE VERTICAL HEIGHT SHALL BE MEASURED FROM THE TOP OF THE CEILING FRAMING MEMBERS TO THE UNDERSIDE OF THE ROOF FRAMING MEMBERS. THE ROUGH-FRAMED OPENING SHALL NOT BE LESS THAN 22" x 30" AND SHALL BE LOCATED IN A HALLWAY OR OTHER READILY ACCESSIBLE LOCATION. WHEN LOCATED IN A WALL, THE OPENING SHALL BE A MIN. OF 22" WIDE x 30" HIGH. WHEN THE ACCESS IS LOCATED IN A CEILING, MIN. UNOBSTRUCTED HEADROOM IN THE ATTIC SPACE SHALL BE 30" AT SOME POINT ABOVE THE ACCESS MEASURED VERTICALLY FROM THE BOTTOM OF CEILING FRAMING MEMBERS.

SEE SECTION M1305.1.3 FROM ACCESS REQUIREMENTS WHERE MECHANICAL EQUIPMENT IS LOCATED IN ATTICS

REQUIRED ROOF VENTILATION:

ENCLOSED ATTIC AND ENCLOSED RAFTER SPACES FROMED WHERE CEILINGS ARE APPLIED DIRECTLY TO THE UNDERSIDE OF ROOF RAFTERS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENING PROTECTED AGAINST THE ENTRANCE OF RAIN. VENTILATION OPENINGS SHALL HAVE A LEAST DIMENSION OF 1/16" MIN. AND 1/4" MAX. VENTILATION OPENINGS HAVING A LEAST DIMENSION LARGER THAN 1/4" SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE CLOTH SCREENING, HARDWARE CLOTH, OR SIMILAR MATERIAL WITH OPENINGS HAVING A LEAST DIMENSION OF 1/16" MIN. AND 1/4" MAX. OPENINGS IN ROOF FRAMING MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF SEC. R802.1.8. REQUIRED VENTILATION OPENINGS SHALL OPEN DIRECTLY TO OUTSIDE AIR

MINIMUM ROOF VENT AREA:

THE MINIMUM NET FREE VENTILATING AREA SHALL BE 1/150 OF THE AREA OF THE VENTED SPACE.

EXCEPTION: THE MINIMUM NET FREE VENTILATION AREA SHALL BE 1/300 OF THE VENTED SPACE PROVIDED ONE OR MORE OF THE FOLLOWING

CONDITIONS ARE MET:

1. IN CLIMATE ZONES 6, 7 AND 8, A CLASS I OR II VAPOR RETARDER IS INSTALLED ON THE WARM-IN-WINTER SIDE OF THE CEILING.

2. AT LEAST 40 PERCENT AND INOT MORE THAN 50 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE ATTIC OR RAFTER SPACE, UPPER VENTILATORS SHALL BE LOCATED NO MORE THAN 3 FEET BELOW THE RIDGE OR HIGHEST POINT OF THE SPACE, MEASURED VERTICALLY, WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS. WHERE THE LOCATION OF WALL OR RICOF FRAMING MEMBERS CONFLICTS WITH THE INSTALLATION OF UPPER VENTILATORS, INSTALLATION MORE THAN 3 FEET BELOW THE RIDGE OR HIGHEST POINT OF THE SPACE SHALL BE PERMITTED. 3 FEET BELOW THE RIDGE OR HIGHEST POINT OF THE SPACE SHALL BE PERMITTED.



DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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> JOB NUMBER: 250490

> > OF 3 SHEETS

ROOF OVER FRAMING & BRACING DETAIL

VALLEY ROOF PLAN MEMBER LEGEND

= = = TRUSS UNDER VALLEY FRAMING ===== VALLEY RAFTER OR RIDGE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

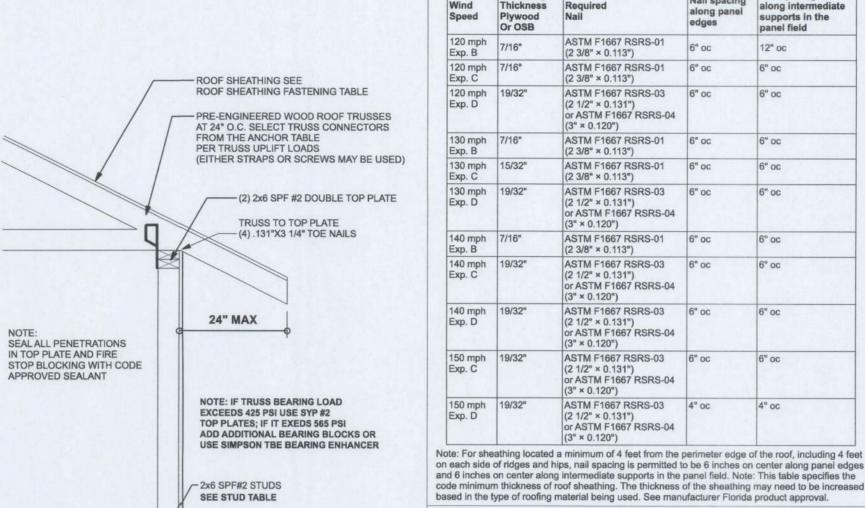
| 1 | 2X4 RAFTERS TO RIDGE | 3 -16d OR 6131 x 3" TOE NAILS |
|----|--|--|
| 2 | CRIPPLE TO RIDGE | 3 - 16d OR 6131 x 3" FACE NAILS |
| 3 | CRIPPLE TO RAFTERS | 3 - 16d OR 6131 x 3" FACE NAILS |
| 4 | RAFTER TO SLEEPER OR BLOCKING | 6 -16d OR 12131 x 3" TOE NAILS |
| 5 | SLEEPER TO TRUSS | 4 - 16d OR 8131 x 3" FACE NAILS EACH TRUSS |
| 6 | RIDGE BOARD TO ROOF BLOCK | 3 -16d OR 6131 x 3" TOE NAILS |
| 7 | RIDGE BOARD TO TRUSS | 3 -16d OR 6131 x 3" TOE NAILS |
| 8 | PURLIN TO TRUSS (TYP.) | 3 -16d OR 6131 x 3" NAILS |
| 8 | PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN) | 4 -16d OR 8131 x 3" NAILS |
| 9 | TRUSS TO BLOCKING | 3 -16d OR 6131 x 3" END NAILS |
| 10 | CRIPPLE TO TRUSS | 3 -16d OR 6131 x 3" FACE NAILS |
| 11 | CRIPPLE TO PURLIN | 3 -16d OR 6131 x 3" FACE NAILS |

GENERAL NOTES

- ENCLOSED BUILDING

MAXIMUM RAFTER SPANS 6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2. 16ft2 IN ZONES 2 & 3 , 24ft2 IIN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 16ft2 OR 2'-0" X 8'-0" SPAN = 16ft2) PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS. THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS: -SPANS (DISTANCS BETWEIEN HEELS) 40'-0" OR LESS - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS -MAXIMUM WIND SPEED: 130 MPH MAXIMUM MEAN ROOF HEIGHT: 30 FEET MAXIMUM TOTAL LOADING: 40 nsf - MEETS FBC / ASCE 7-16 WIND REQUIREMENTS EXPOSURE CATEGORY "C;", I = 1.0, Kzt = 1.0

CRIPPLE, BRACING, & BLOCKING NOTES -2X4 CONTINUOUS LATERAL IBRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED W/ 2 - 10d NAILS OR :2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQURE TWO CLB's OR BOTH FACES W/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLIE CAN FACE RIDGE OR RAFTER,
AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDIS AND LATERAL BRACING IS NOT USED APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHE:RWISE.



ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

(TYP.) CORNER FRAMING

WOOD FRAME

Attach king stud to top plate | w/ (2) Simpson SDWC15450

< 895 Attach king stud to top plate Attach king stud to bottom plate

w/ (2) Simpson SDWC15600 w/ (3) Simpson SDWC15450

Option Uplift Top Connection

#4 < 1455 MSTA24, 18-10d header to jacks

#5 < 1800 (2) MSTA24, 18-10d header to jacks DTT2Z #6 < 2910 (2) MSTA24, 18-10d header to jacks HTT4

HEADER SCREWS TABLE

Bottom Connection

w/ (1) Simpson SDWC15600 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer

HEADER STRAP TABLE

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT MAX. SPANS FOR SPF #2

130 MPH EXP. C 5'-2" 7'-9" 7'-7" 11'-3" HEIGHTS (H) SILL SPAN SHALL BE

WIND SPEED (1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6

#3 < 1235 LSTA24, 14-10d wrap over plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer

Attach king stud to bottom plate

must be located within 6" of king stud @ all door locations

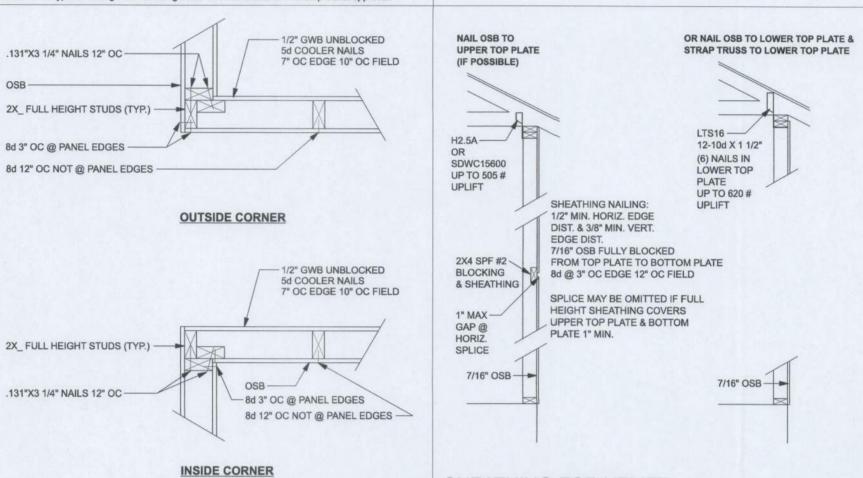
must be located within 6" of king stud @ all door locations

must be located within 6" of king stud @ all door locations

TABLE A-3.23B

1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer

LSTA24, 14-10d wrap under plate



- ENGINEERED TRUSSES ATTACH PER TRUSS UPLIFT -TRUSS TO TOP PLATE (4) .131"X3 1/4" TOE NAILS SEE TABLE FOR -STRAP OR SCREW -(2) 2X_ SPF #2 TOP PLATE & SEE TABLE FOR SPACING SEE STUD TABLE SEE TABLE FOR ----2X_PT SP #2 PLATE STRAP OR SCREW 1/2" X 10" ANCHOR BOLT 2" X 2" X 1/8" WASHER & SEE TABLE FOR SPACING @ SEE TABLE FOR SPACING & 8" FROM CORNERS

SHEATHING FOR UPLIFT

ATTACHMENT DETAILS

ONE STORY WOOD FRAME

CONNECTOR TABLE

To Plate

5-8dx1 1/2"

9-10d1 1/2"

6-10d1 1/2"

7-10d1 1/2"

12-10d1 1/2"

6-10d

14-10d

To Post

18-16dx2 1/2"

18-16dx2 1/2"

To One Member

To Truss/Rafter

5/8"x12" Drill & Epoxy

5/8"x7" Drill & Epoxy

5/8"x7" Drill & Epoxy

5-8dx1 1/2"

Uplift SP Uplift SPF Truss Connector

Uplift SP Uplift SPF Strap Ties

Uplift SP Uplift SPF Stud Plate

LTS12-20

MTS12-30

HTS20-30

MSTA24

LSTA24

Uplift SP Uplift SPF Holdowns @ Mono

Uplift SP Uplift SPF Post Bases @ Mono

Uplift SP Uplift SPF Holdowns @ Stemwall To Stud / Post

Uplift SP Uplift SPF Post Bases @ Stemwall To Post

| Uplift on wall | Top Connection | Bottom Connection | Anchor Bolt Spacing |
|----------------|---|--|---------------------|
| 227 plf | SP2 @ 32" OC | SP1 @ 32" OC | 48" OC |
| 454 plf | SP2 @ 16" OC | SP1 @ 16" OC | 32" OC |
| 223 plf | (2) SDWC 15600 @ 48" OC | (3) SDWC15450 @ 48" OC | 48" OC |
| 336 plf | (2) SDWC 15600 @ 32" OC | (3) SDWC15450 @ 32" OC | 48" OC |
| 309 plf | LSTA24, 14-10d @ 48" OC Wrap Under Plate | LSTA24, 14-10d @ 48" OC Wrap Over Plate | 48" OC |
| 465 plf | LSTA24, 14-10d @ 32" OC Wrap Under Plate | LSTA24, 14-10d @ 32" OC Wrap Over Plate | 32" OC |

(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

TYPICAL HEADER STRAPING OR SCREWS DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

OF PLATE JOINT

~7/16" O.S.B. WALL SHEATHING

UPLIFT ATTACHMENT DETAILS

-4" CONCRETE FLOOR SLAB REINFORCED

WITH 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS AT 1 1/2" DEPTH OR

BARRIER WITH 6" LAPS SEALED WITH POLY TAPE OVER TERMITE-TREATED

FIBER MESH CONCRETE, 6-MIL POLY VAPOR

NOTE: USE 1/2" x 6" TITEN HD

IN PLACE OF WET-SET ANCHORS AS NEEDED

8d COMMON NAILS 3" OC EDGE, 12" OC FIELD

AND COMPACTED FILL

1/2" X 10" ANCHOR BOLT

@ 32" OC & 8" FROM CORNERS

3" X 3" X 1/4" WASHER

-2X PT SP #2 PLATE

ONE STORY WALL SECTION

SEE "STRUCTURAL PLAN NOTES" ---

. OTHER STRAPING OR SCREWS LESS OR MORE

FOR (U.N.O) STRAPING OR SCREWS

IS NOTED ON STRUCTURAL PLANS

(6) .131"X3 1/4" TOE NAILED -

OR BACK NAILED THRU

KING STUD INTO HEADER

SHEATHING MUST BE NAILED TO TOP PLATES

w/ 8d 3" OC (NAILING MAY BE STAGGERED) & SHEATHING NAILED TO HEADER

ALL HEADER JACK & KING STUDS SHALL BE FASTENED TO EACH OTHER w/ (2) ROWS

- WINDOW SILL PLATE

(PER TABILE BELOW)

TOE NAIL ENDS OF EACH PLY W

2x6 = (6) .131" x 3.25" NAILS

CRIPPLES OF REQUIRED

w/ (2) ROWS OF 8d @ 6" OC

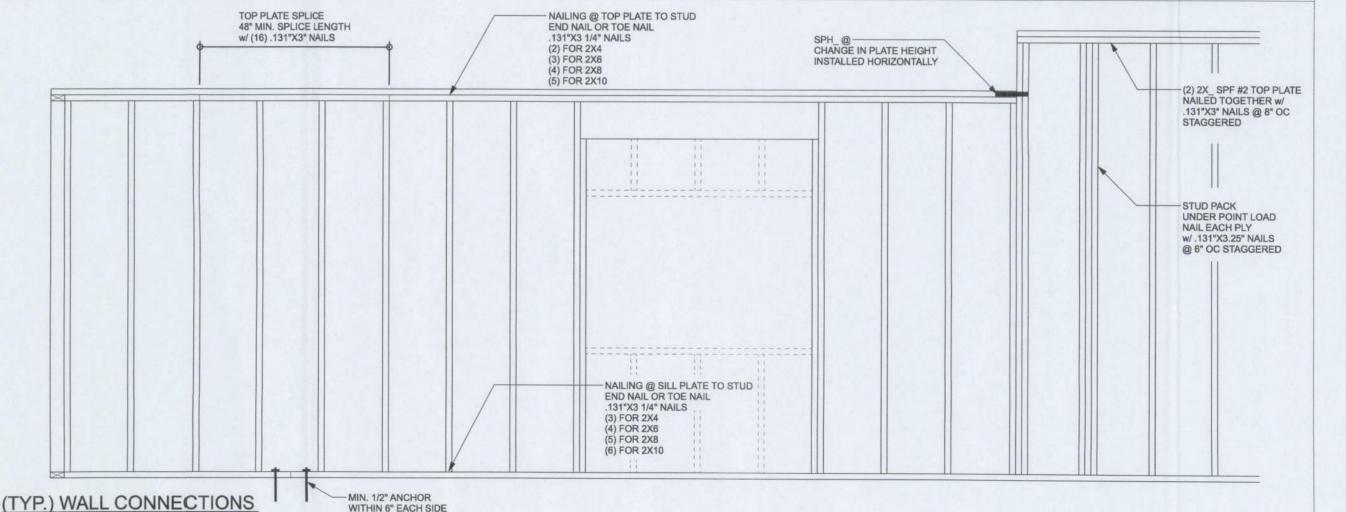
10d @ 8" OC STAGGERED

ONE STORY WOOD FRAME

SCALE: 3/4" = 1'-0"

FULLY BLOCKED

SEE SHEATHING FOR



WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END. SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS

GENERAL NOTES:

TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S 9-10d1 1/2" DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER 6-10d1 1/2 LY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS ASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING 7-10d1 1/2 WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR 12-10d1 1/2' REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS To Other Member To Plate VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE) 4-10d CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. 6-10d wrap under or over pla WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE wrap under or ove

REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS 8-SDS 1/4"x1 1/2" 1/2"x12" Titen HD 1/2"x12" Titen HD FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 To Stud / Post Anchor TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATION: 8-SDS 1/4"x1 1/2" 1/2"x6" Titen HD IBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 1/2"x12" Titen HD CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL 5/8"x12" Drill & Epoxy

ONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL E CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND YPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. MMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN

ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,

LL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE

RUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED OAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY: THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN: THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE T REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS: THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE O STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.) TO 10'-1" STUD HEIGHT (1) 2x4 @ 16" OC (1) 2x4 @ 12" OC TO 11'-2" STUD HEIGHT (1) 2x6 @ 16" OC TO 15'-7" STUD HEIGHT

TO 17'-3" STUD HEIGHT

| GRA | DE & SPECIES TA | BLE | |
|------|-----------------|------|-----|
| | | Fb | E |
| 2x8 | SP #2 | 925 | 1.4 |
| 2x10 | SP #2 | 800 | 1.4 |
| 2x12 | SP #2 | 750 | 1.4 |
| GLB | 24F-V3 SP | 2600 | 1.9 |
| LSL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MICROLAM | 2950 | 2.0 |
| PSL | PARALAM | 2900 | 2.0 |

(1) 2x6 @ 12" OC

TRUSS SHEETS.

| BUILDING CODE | 8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023) | | |
|---|--|--|--|
| CODE FOR DESIGN LOADS | ASCE 7-22 | | |
| WINDLOADS | | | |
| BASIC WIND SPEED (ASCE 7-22, 3S GUST) | 130 MPH | | |
| WIND EXPOSURE (BUILDER MUST FIELD VERIFY) | С | | |
| TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY) | I and the second | | |
| RISK CATEGORY | II | | |
| ENCLOSURE CLASSIFICATION | ENCLOSED | | |
| INTERNAL PRESSURE COEFFICIENT | 0.18 | | |
| ROOF ANGLE | 7-45 DEGREES | | |
| MEAN ROOF HEIGHT | 30 FT | | |
| C&C DESIGN PRESSURES | SEE TABLE | | |
| FLOOR LOADING | | | |
| ROOMS OTHER THAN SLEEPING ROOM | 40 PSF LIVE LOAD | | |
| SLEEPING ROOMS | 30 PSF LIVE LOAD | | |
| ROOF LOADING | | | |
| FLAT OR < 4:12 | 20 PSF LIVE LOAD | | |
| 4:12 TO < 12:12 | 16 PSF LIVE LOAD | | |
| 12:12 & GREATER | 12 PSF LIVE LOAD | | |
| SOIL BEARING CAPACITY | 1500 PSF | | |
| FLOOD ZONE | THIS BUILDING IS NOT IN THE FLOOD ZON | | |

| EFFECTIVE WIND AREA (FT2) | ZONE 4 INTERIOR | | | ZONE 5 END 4' FROM AL OUTSIDE CORN | |
|------------------------------|--------------------|-------------------------|------|--|-------------|
| 0 - 20 | +25.6(Vasd |) -27.8(\ | asd) | +25.6(Vasd) | -34.2(Vasd) |
| 0 - 20 | +42.6(Vult |) -46.2(\ | ult) | +42.6(Vult) | -57(Vult) |
| GARAGE DOOF | DESIGN PR | ESSURI | S 13 | 0 MPH (EXP C | () |
| 9x7 GARAGE DOOR | | +22.6(Vasd) -25.5(Vasd) | | - | |
| 16x7 GARAGE DOOR | ₹ +2 | 1.7(Vasd) | -24. | 1(Vasd) | |

No/53915 2025-05-30

Mark Disosway FL PE 53915

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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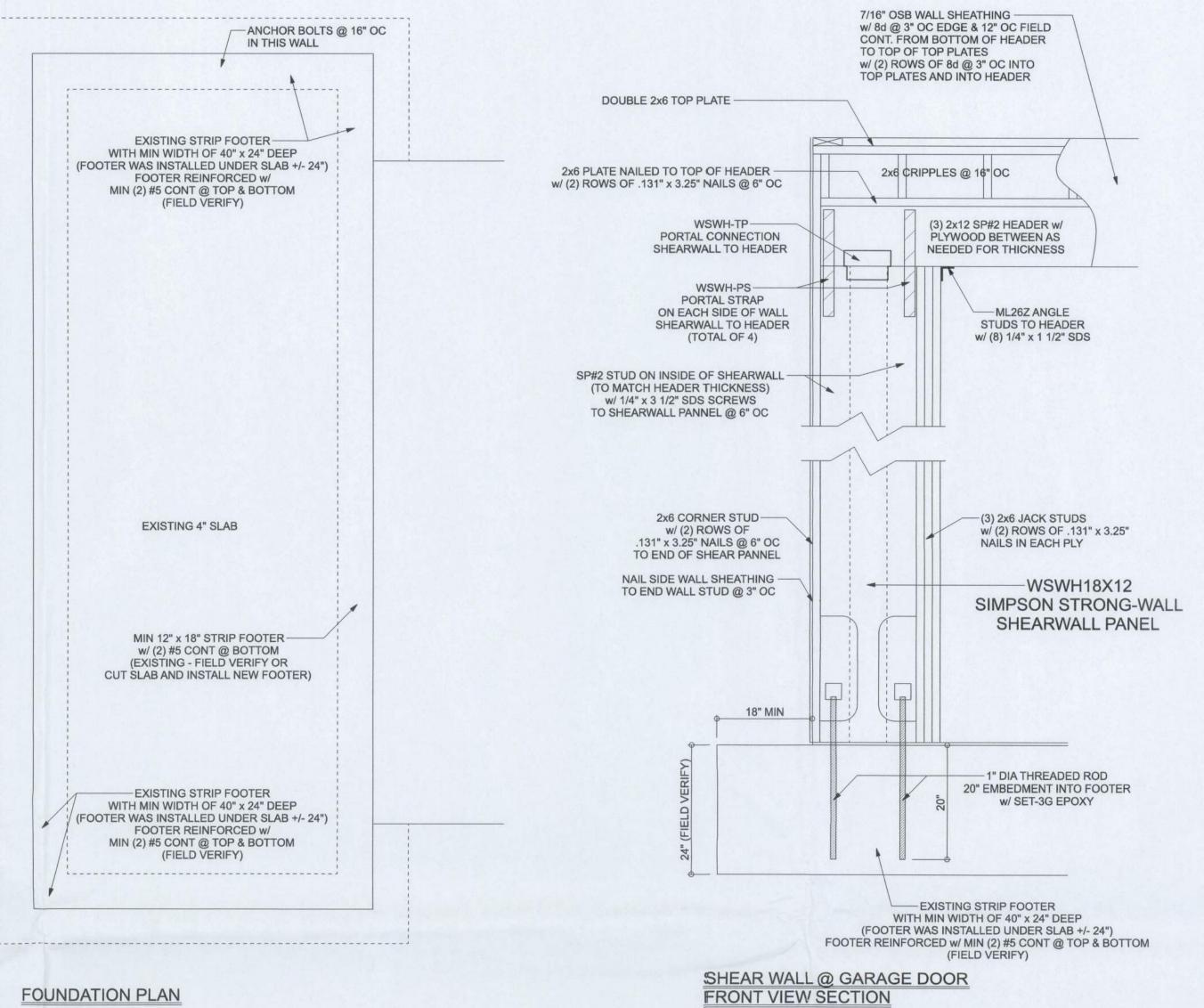
permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E. 163 SW Midtown Place Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

> JOB NUMBER: 250490

S-1 OF 3 SHEETS

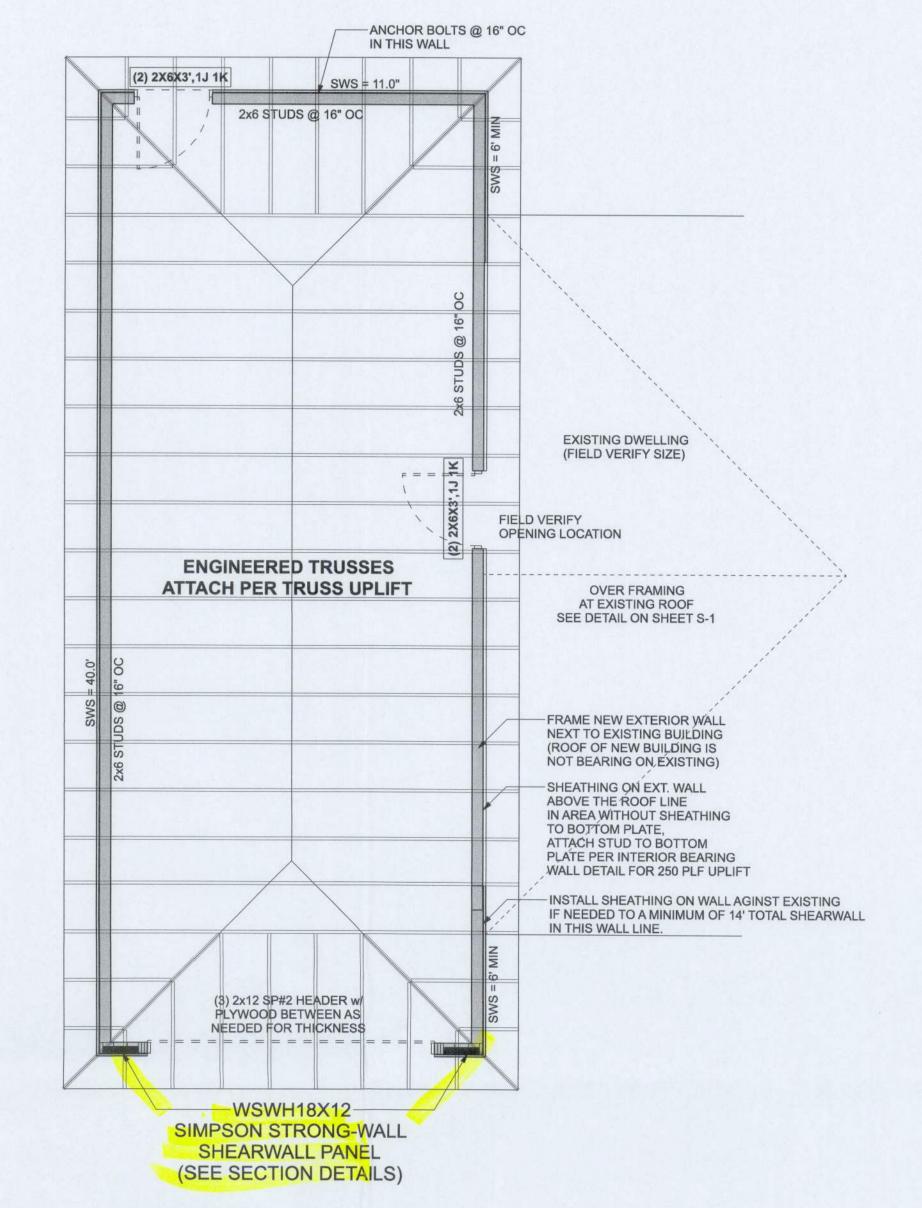


FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN. CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING FN - 2 IN ALL AREAS BY REVIEWINGTHE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN I - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)

> 2x6 SP#2 RAFTERS @ 24" OC 2x8 SP#2 HIP BOARD w/ CS20 TENSION STRAP @ 24" OC w/ H2.5a CLIP & (4) .131" x 3.25" TOE-NAILS TO TOP PLATE RAFTER TO HIP AND (4) .131" x 3.25" WITH (6) 10d @ EACH END TOE-NAILS TO RIDGE / HIP (2) 2x10 SP#2 CEILING JOISTS w/ (10) .131" x 3.25" NAILS TO RAFTER @ EACH END 2x10 SP#2 CEILING JOISTS @ 24" OC w/ (10) .131" x 3.25" NAILS TO RAFTER @ EACH END 2x6 SP#2 RAFTERS @ 24" OCw/ H2.5a CLIP & (4) .131" x 3.25" TOE-NAILS TO TOP PLATE -2x8 SP#2 RIDGE BOARD AND (4) .131" x 3.25" w/ CS20 RIDGE TENSION STRAP @ 24" OC TOE-NAILS TO RIDGE / HIP WITH (6) 10d @ EACH END CONVENTIONALLY FRAMED ROOF SYSTEM AS ALT. TO TRUSSES



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

ACTUAL vs REQUIRED SHEARWALL LONGITUDUNAL TRANSVERSE 9570 LBF 21384 LBF ACTUAL

STRUCTURAL PLAN NOTES

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

9069 LBF 9069 LBF 4021 LBF

HEADER LEGEND

(2) 2X6X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) 4 4 4 4 4 - NUMBER OF KING STUDS EACH SIDE OF OPENING (FULL LENGTH) NUMBER OF JACK STUDS EACH SIDE OF OPENING (UNDER HEADER) SPAN OF HEADER

SIZE OF HEADER MATERIAL

NUMBER OF PLIES IN HEADER

UNLESS NOTED OTHERWISE (MINIMUM REQUIERMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS*** BEAM / HEADERS (SIZE) ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO) HEADERS (JACK & KING STUDS) ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO) HEADERS (STRAPING) ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.) JACK STUDS UNDER GIRDER TRUSS USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

Mark Disosway FL PE 53915



2025-05-30 DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.

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OF 3 SHEETS

S-2

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL 1 mm

MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY

| | ACI530.1-02 Section | Specific Requirements |
|---------|--|---|
| 1.4A | Compressive strength | 8" block bearing walls F'm = 1500 psi |
| 2.1 | Mortar | ASTM C 270, Type N, UNO |
| 2.2 | Grout | ASTM C 476, admixtures require approval |
| 2.3 | CMU standard ASTM C 90-02, Normal weight, Hollo medium surface finish, 8"x8"x16" runi bond and 12"x12" or 16"x16" column block | |
| 2.3 | Clay brick standard | ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5" |
| 2.4 | Reinforcing bars, #3 - #11 | ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5) |
| 2.4F | Coating for corrosion protection | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS |
| 2.4F | Coating for corrosion protection Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet mel ties not completely embedded in mortar of grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS | |
| 3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval. |
| 3.3.E.7 | Movement joints | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings. |