



FRONT ELEVATION
SCALE: 1/4" = 1'-0"

ROOF NOTES

R - 1 ALL OVERHANGS 18" UNLESS OTHERWISE NOTED
24" MAX

REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
2828 S.F. / 300 x 50% = 5 S.F. RIDGE VENT AREA REQUIRED
45 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT
2828 S.F. / 300 x 50% = 5 S.F. SOFFIT VENT AREA REQUIRED
167 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

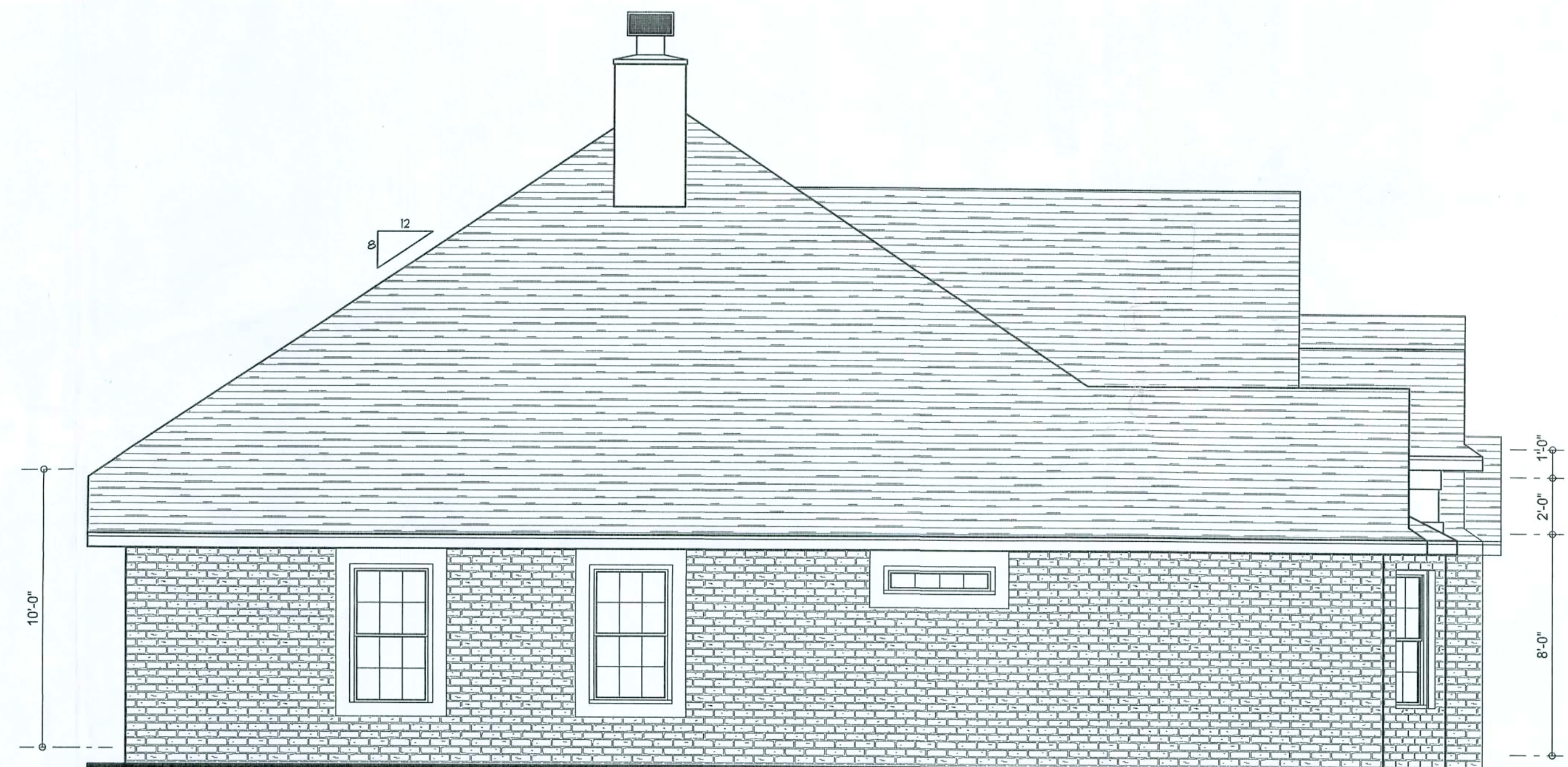
1. RIDGE VENTS = 16 IN²/FT (.11 FT²/FT)
2. OFF-RIDGE VENTS = .70 FT² PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN²/FT (.03 FT²/FT)

REVISIONS

SOFTPLAN
ARCHITECTURAL SOFTWARE



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Discoway,
P.E. No. 53915, POB 868 Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions approximate scaled
dimensions. Refer all questions to
Mark Discoway, P.E. for resolution.
Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering,
comply with section 1609, Florida building code
2004, to the best of my knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISCOWAY
P.E. 53915

Mark Discoway
01DEC05
SEAL

**SPARKS
CONSTRUCTION**

Spec House Lot #2
Rolling Meadows S/D

ADDRESS:
Lot #2 Rolling Meadows S/D
Columbia County Florida

Mark Discoway P.E.
P.O. Box 868

Lake City, Florida 32026
Phone: (386) 754 - 5419

Fax: (386) 269 - 4871

windloadengineer@bellsouth.net

PRINTED DATE:
November 30, 2005

DRAWN BY: Ben Sparks

CHECKED BY:

FINALS DATE:
30 / Nov / 05

JOB NUMBER:
511234

DRAWING NUMBER

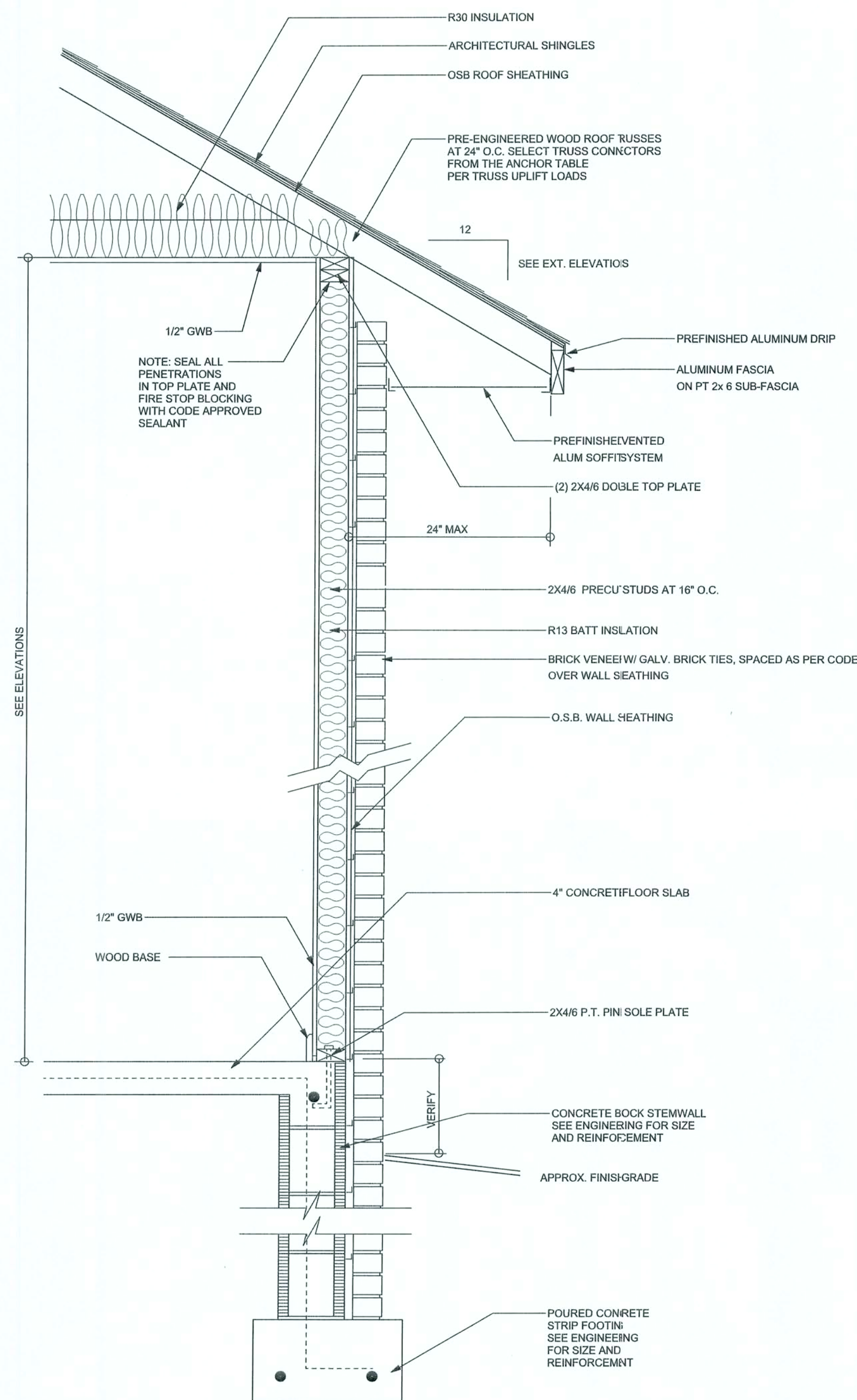
A-1

OF 6 SHEETS

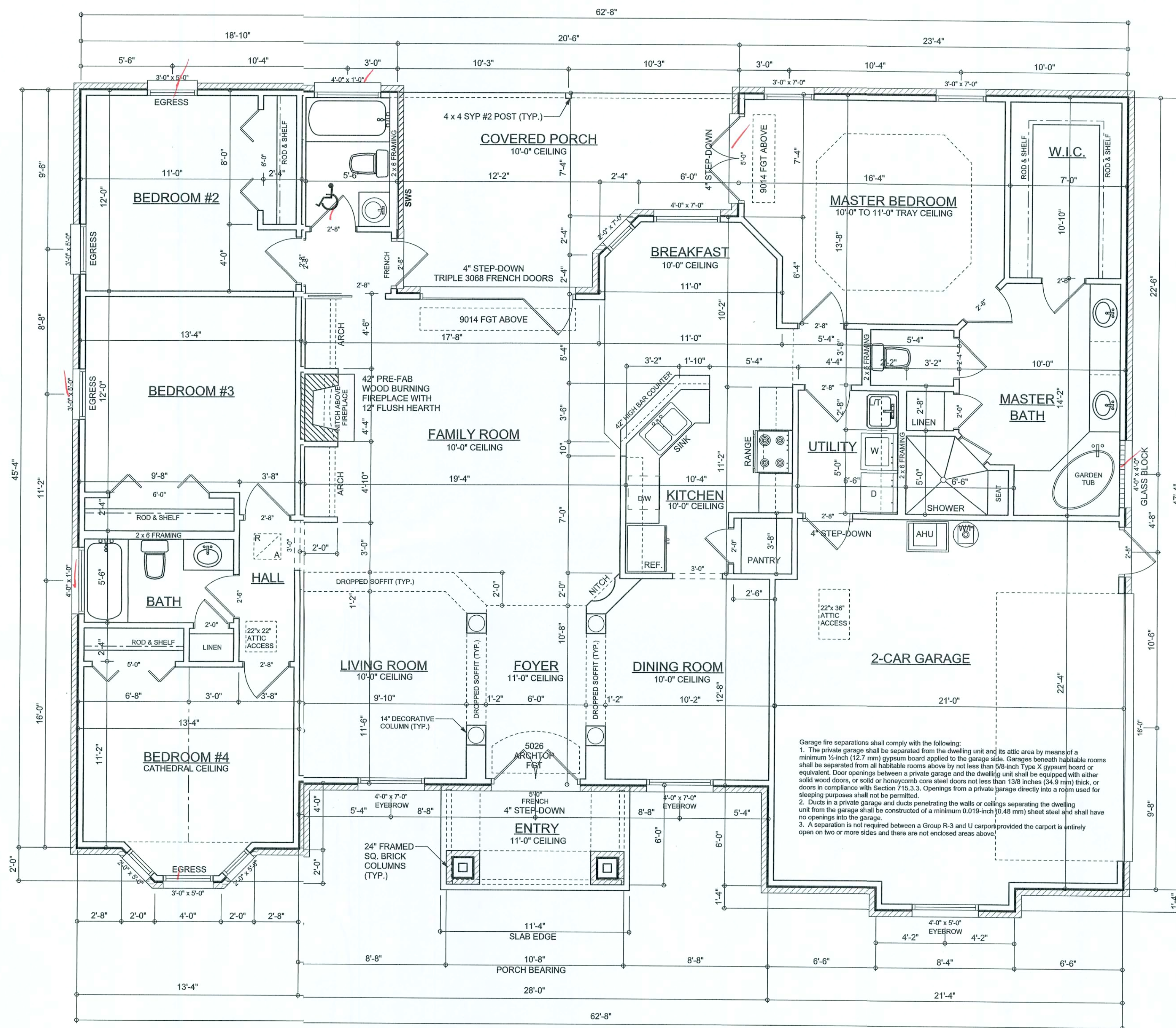
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TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
SCALE: 1\"/>



FLOOR PLAN

SCALE: 1/4\"/>

NOTE: ALL CEILING HEIGHTS SHALL BE 8'-0\"/>

Garage fire separations shall comply with the following:
1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 1 3/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.

AREA SUMMARY

LIVING AREA	2199	S.F.
GARAGE AREA	492	S.F.
COVERED PORCH AREA	193	S.F.
ENTRY PORCH AREA	64	S.F.
TOTAL AREA	2948	S.F.

WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 53915, P.O. Box 868, Lake City, FL
32056, 386-754-5419

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MARK DISOWAY
P.E. 53915

Mark Disoway
01/05/05
SEAL

SPARES CONSTRUCTION

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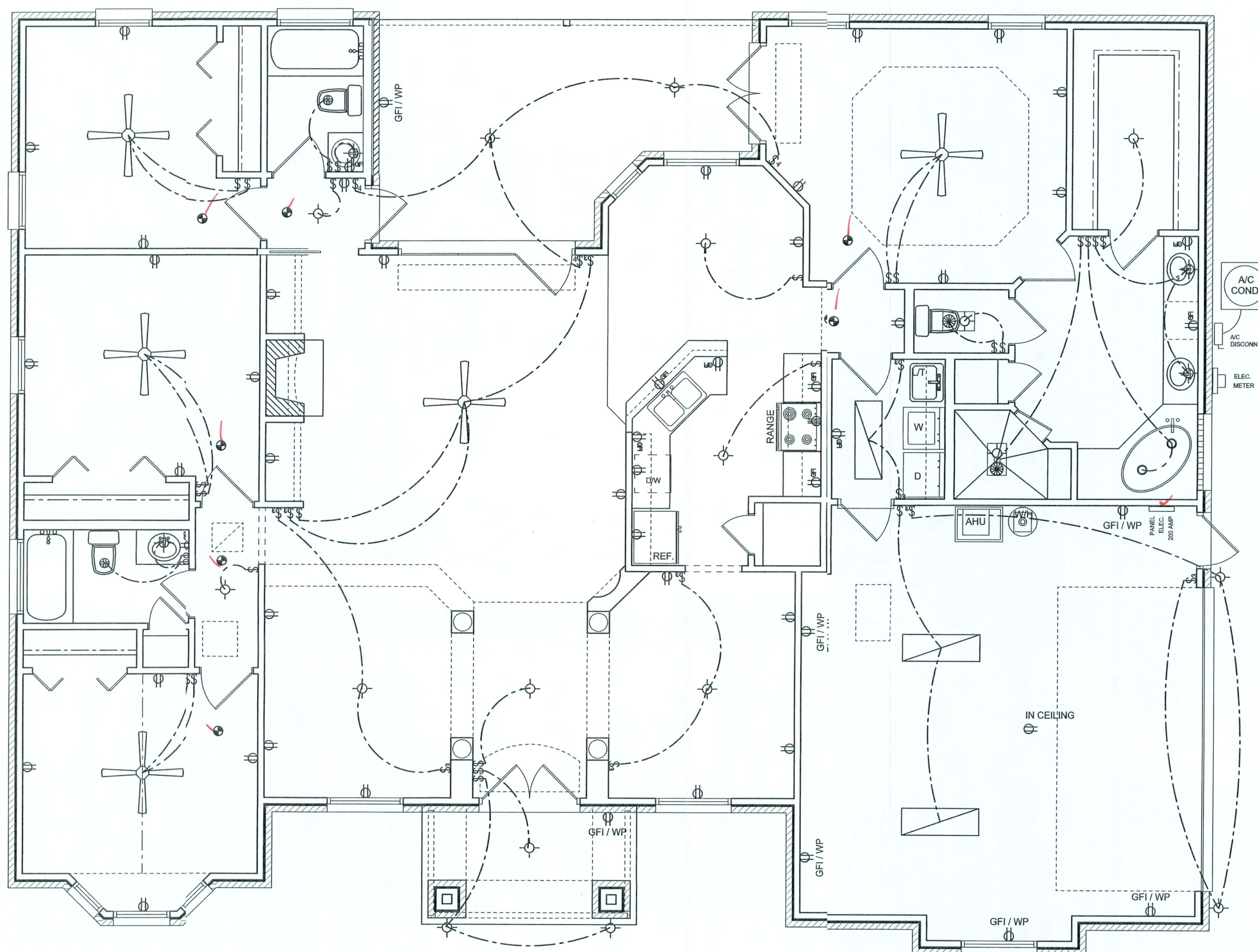
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OF 6 SHEETS

REVISIONS

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ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2x4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway,
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32056, 386-754-5419

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MARK DISOSWAY
P.E. 53915

Mark Disosway
01DEC05
S.E.A.L.

SPARKS CONSTRUCTION

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51'234

DRAWINGNUMBER

A-3

OF 6 SHEETS

REVISIONS



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, TRUSS LAYOUT, TRUSS BRACING, TRUSS CONNECTIONS, TRUSS UPLIFT AND REACTION LOADS FOR PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO FURNISH TRUSS ENGINEERING FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT CONNECTION 4x15L EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 60KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCED STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI, ALL LAP SPLICES 48" DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-VSP, $F_b = 2.4$ ksi, $E = 1800$ ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOKED APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 5 FRAMING MEMBERS, WITH PANEL EDGES MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DESIGN OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A 30" ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED RING BEAM OR 10" IN GROUTED CML.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x .064", WITH 5/8" BOLTS TO BE 3" x 3" x .064", WITH 3/4" BOLTS TO BE 3" x 3" x .064", WITH 7/8" BOLTS TO BE 3" x 3" x .064", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, TRUSS LAYOUT, TRUSS BRACING, TRUSS CONNECTIONS, TRUSS UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004, SECTION 1609

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

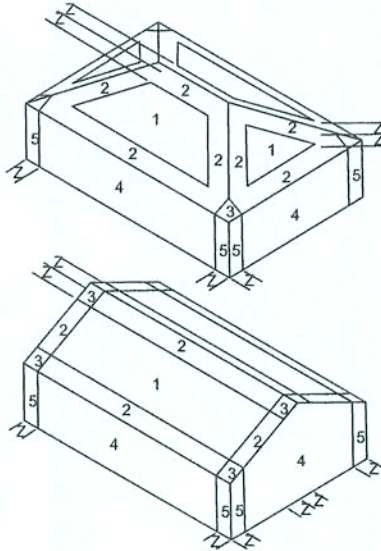
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING, 1609.6)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (FBC TABLE 1609.6 B&C)

Zone	Effective Wind Area (ft ²)		
	10	100	
1	19.9	-21.8	18.1
2	19.9	-25.5	18.1
2 Othg		-40.6	-40.6
3	19.9	-25.5	18.1
3 Othg		-68.3	-42.4
4	21.8	-23.6	18.5
5	21.8	-29.1	18.5
Doors & Windows Worst Case (Zone 5, 10 ft ²)	21.8	-29.1	
8x7 Garage Door	19.5	-22.9	
16x7 Garage Door	18.5	-21.0	

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)



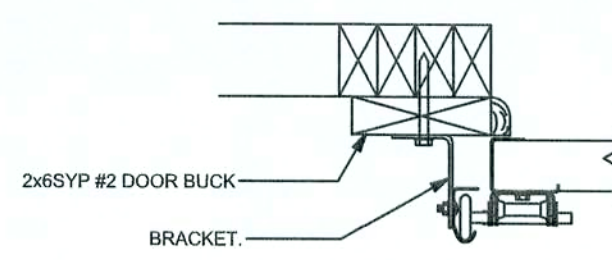
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

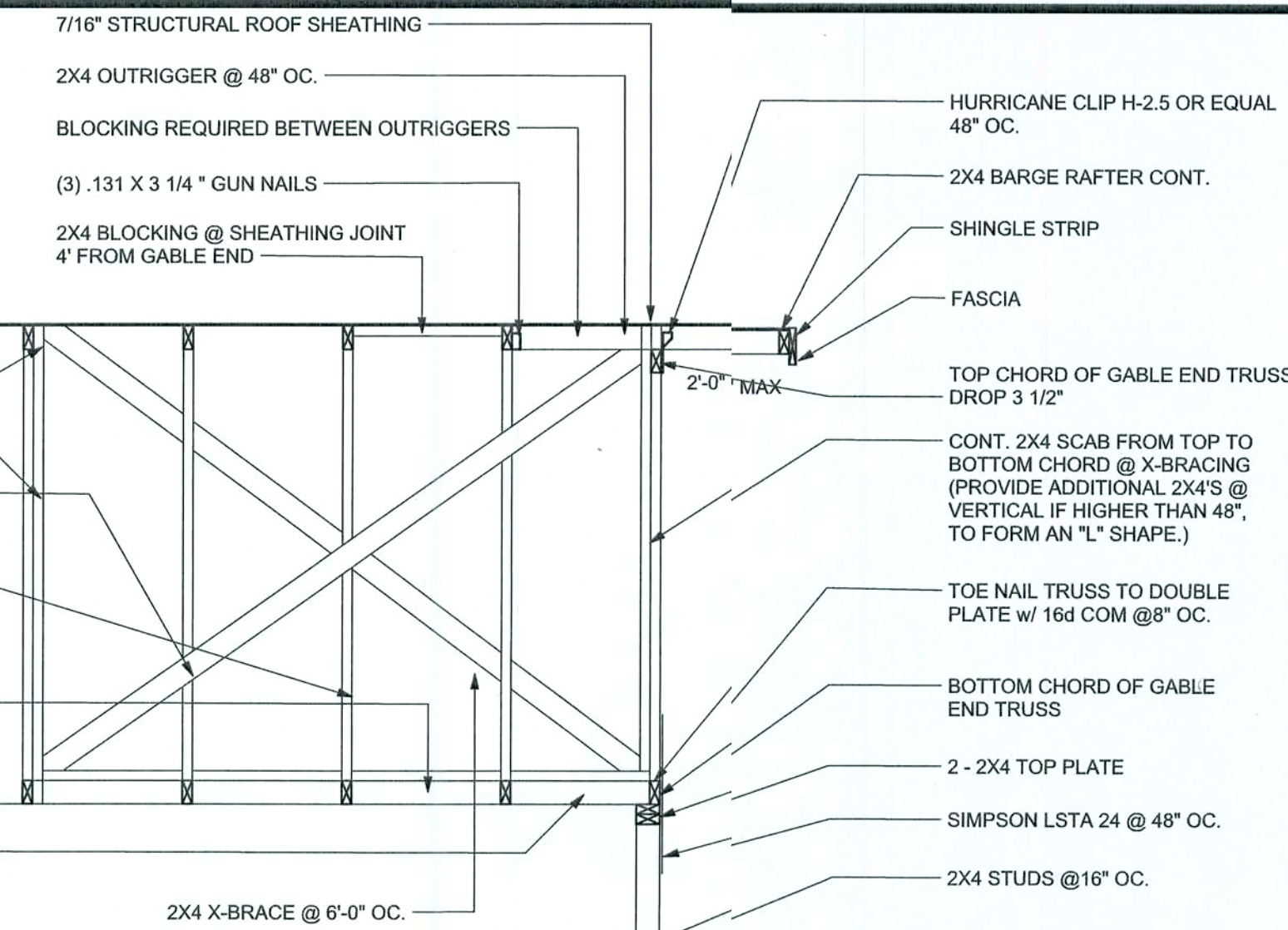
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GR PER TABLE BELOW.

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GR
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



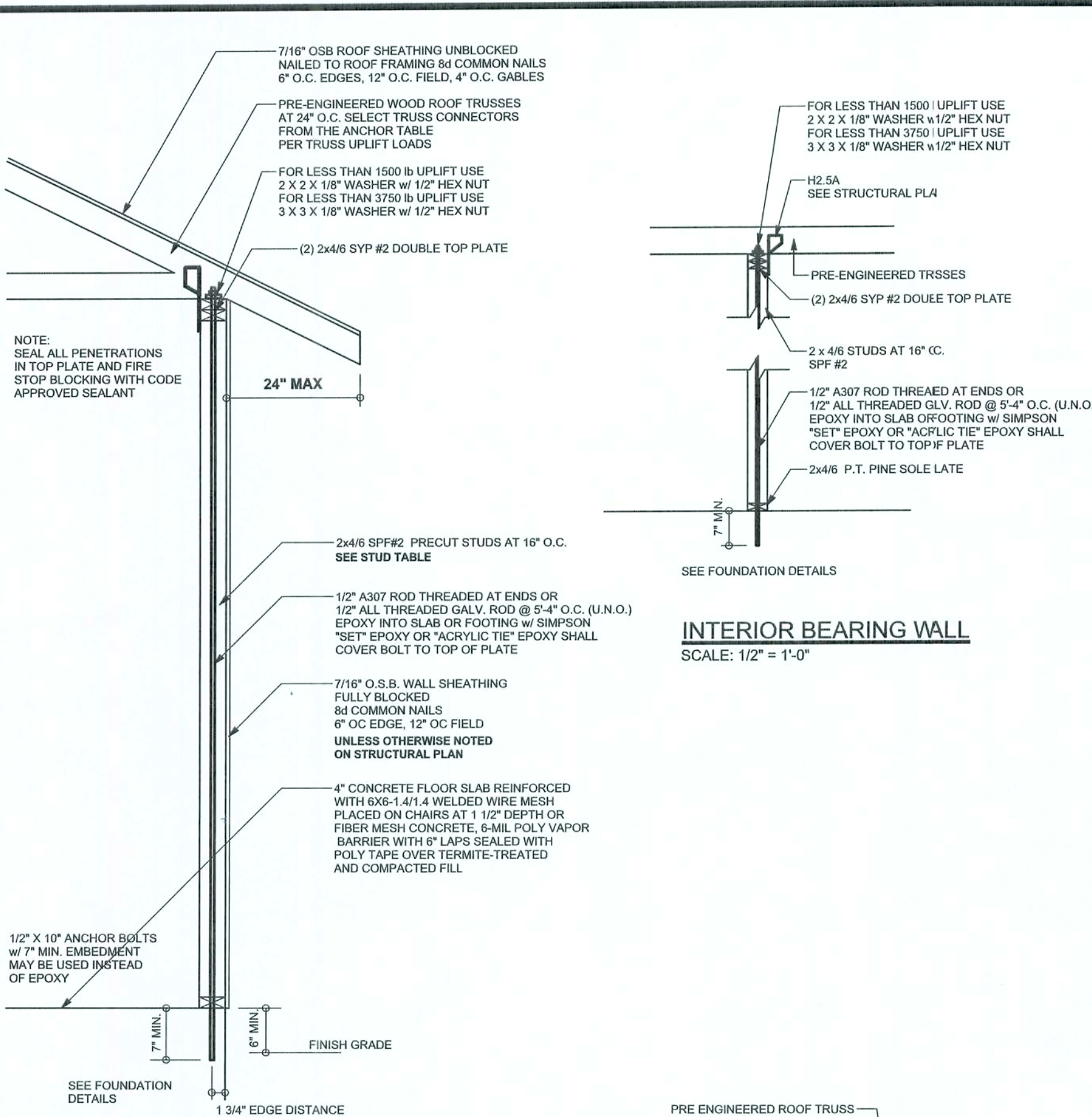
GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.



TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

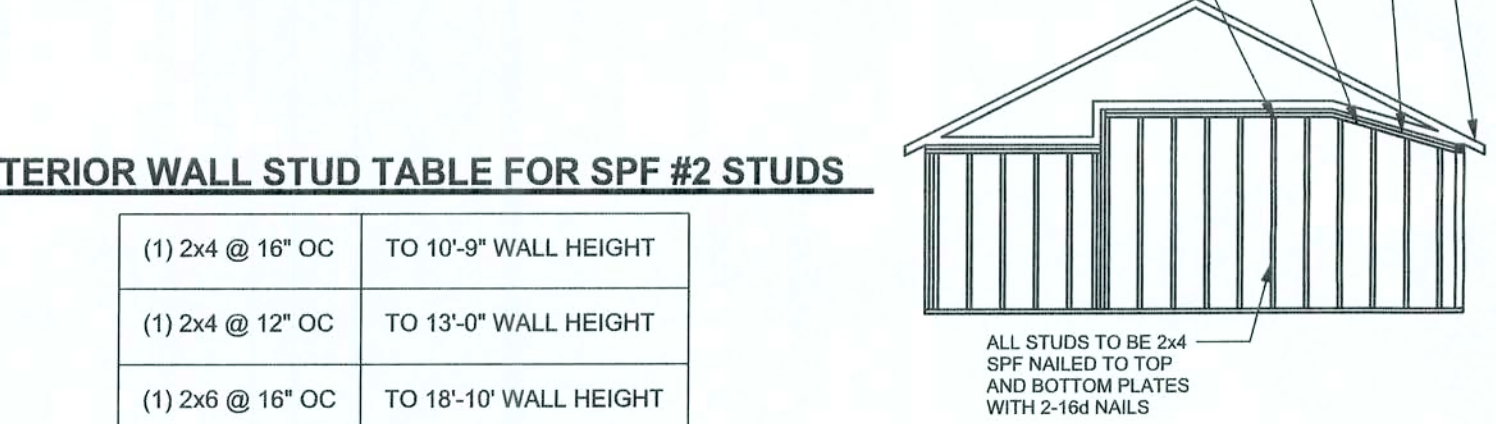


ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

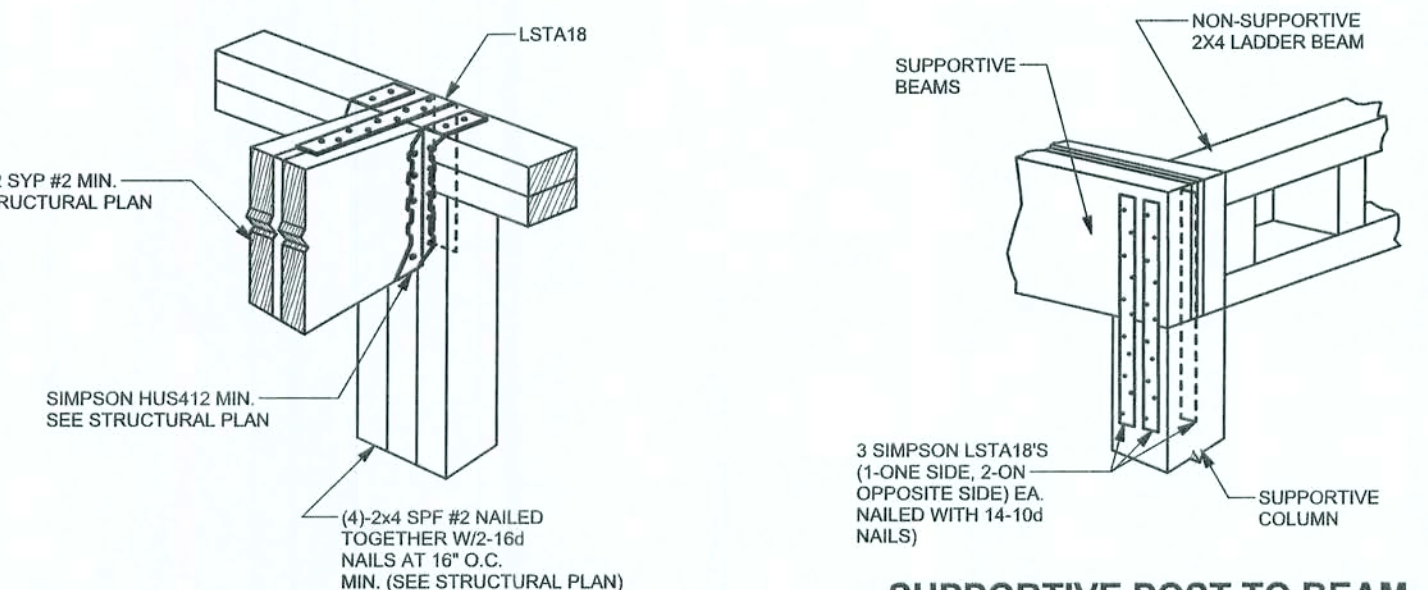
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" WALL HEIGHT



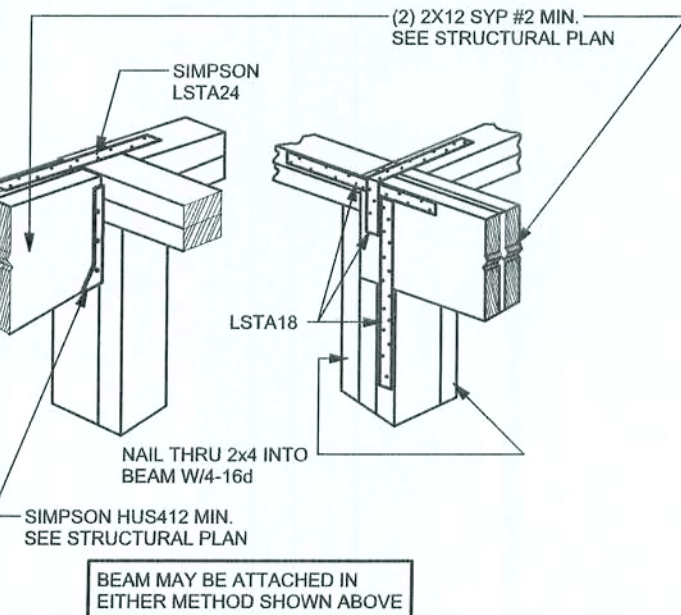
CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.

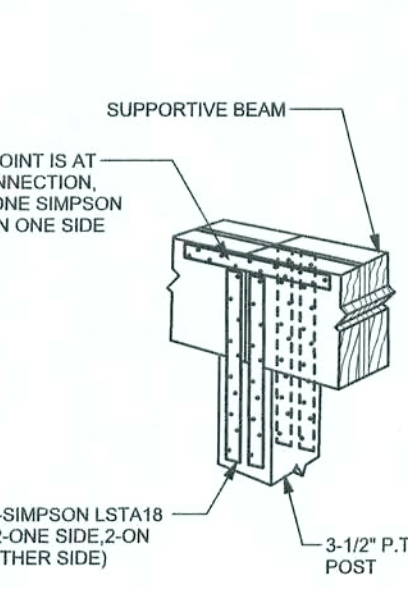


BEAM CORNER CONNECTION. DETAIL

SCALE: N.T.S.

SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.

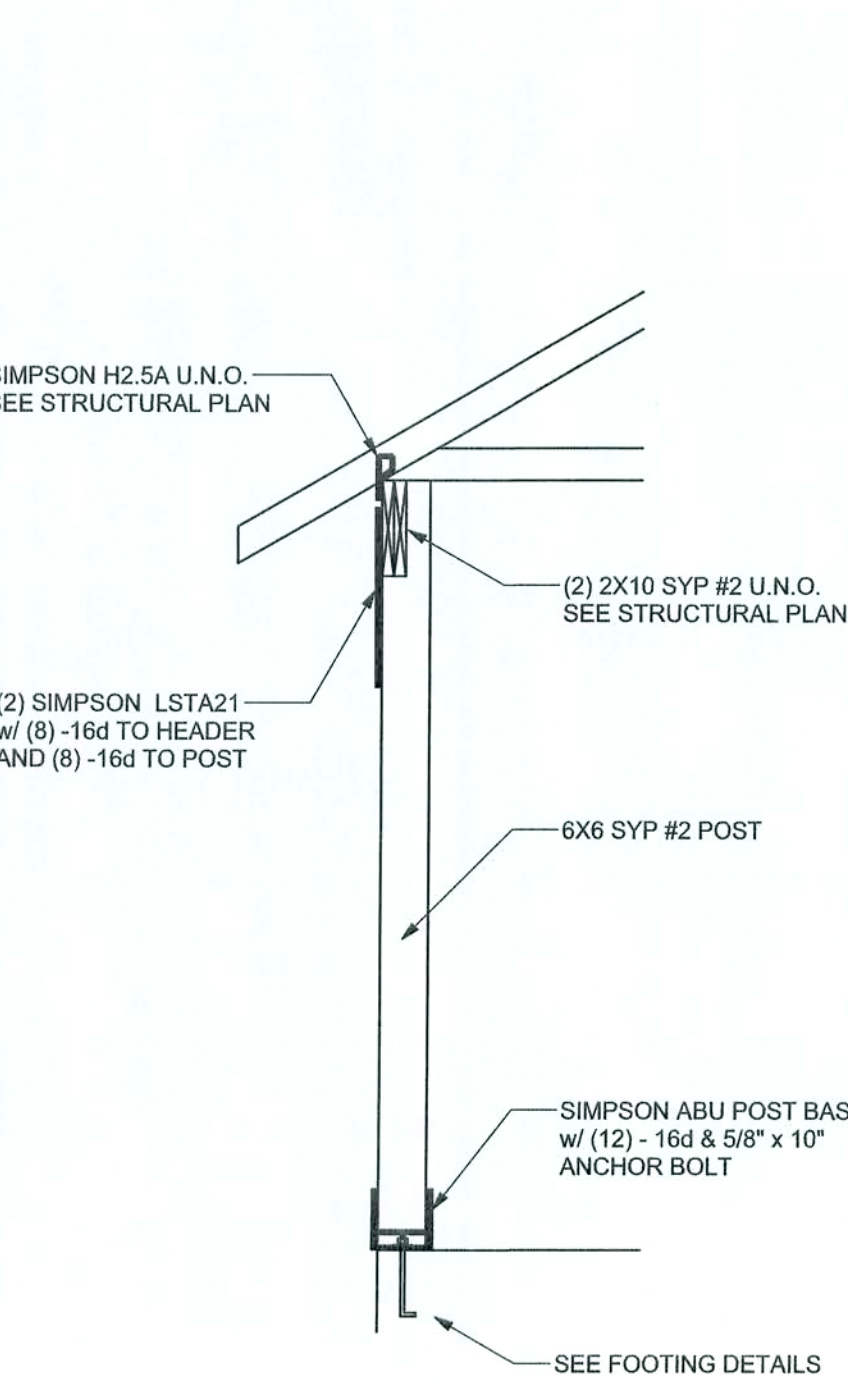


SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

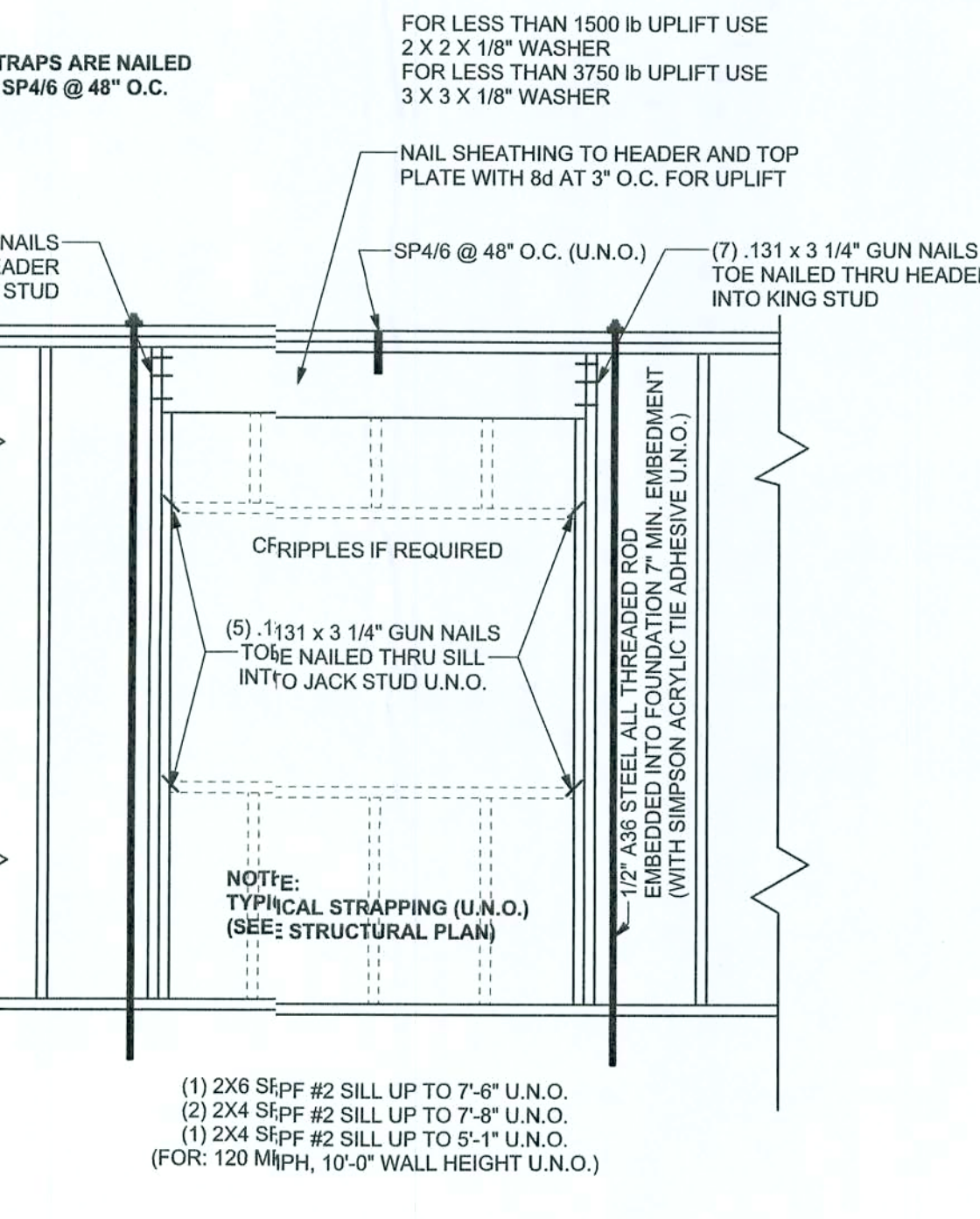
TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"



TYPICAL 1 STORY+ HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"



WINDLOAD ENGINEER: Mark Discovay, P.E. No.53915, F08 688, Lake City, FL 32056, 386-754-5419

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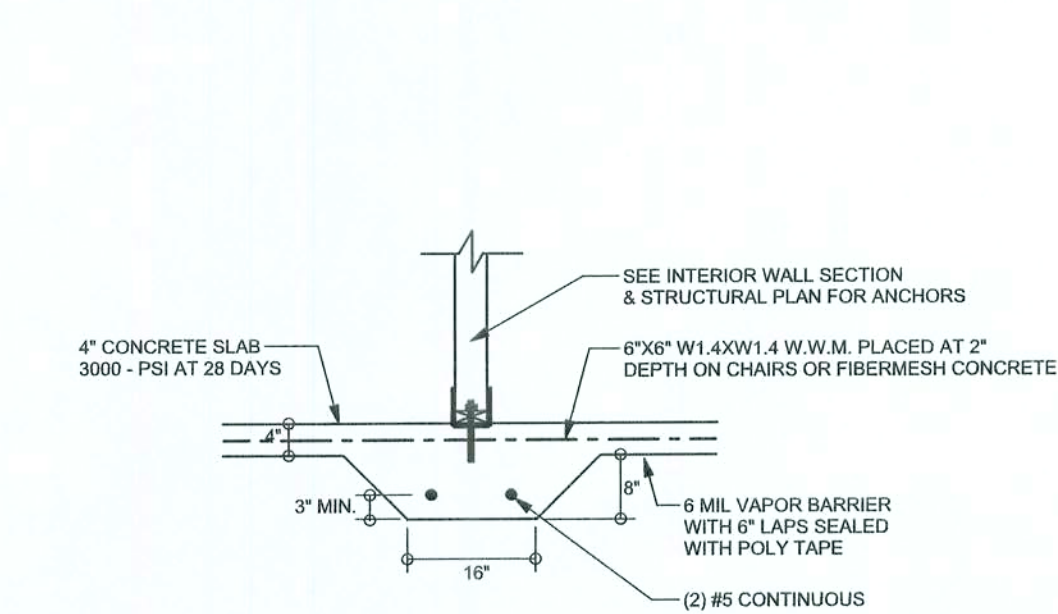
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DRAWING NUMBER

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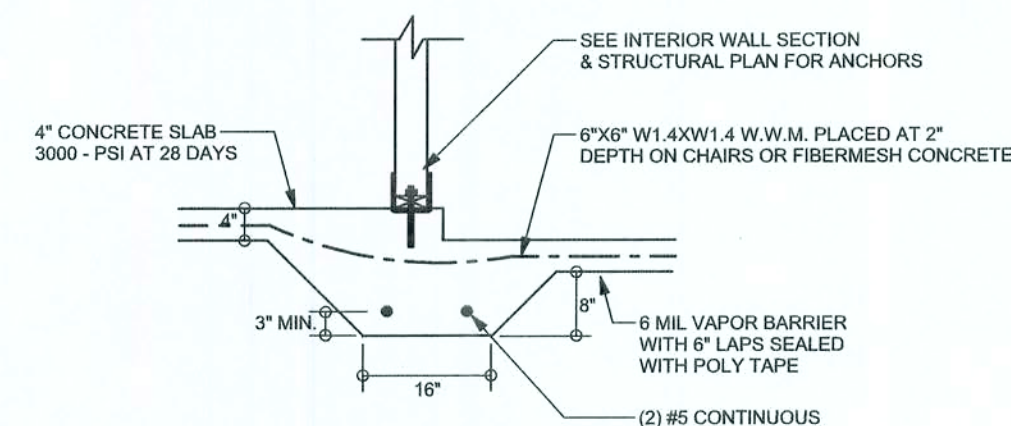
OF 6 SHEETS

REVISIONS

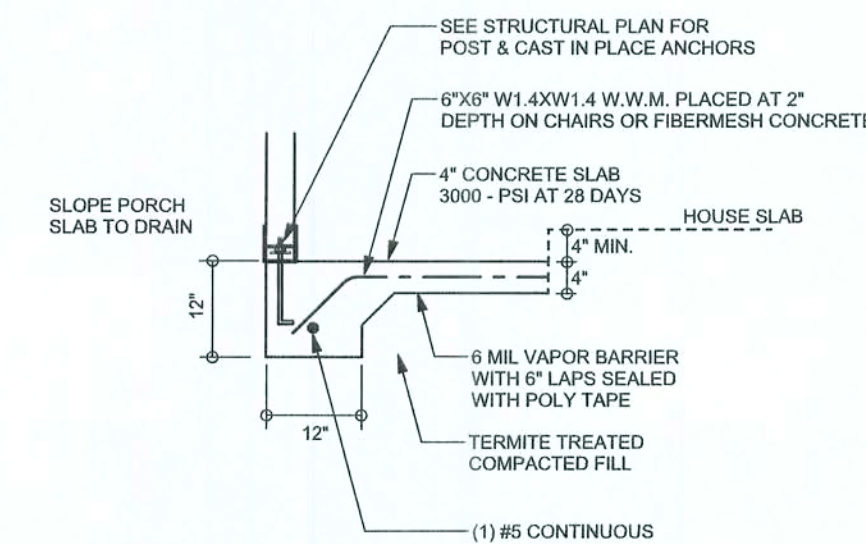
SOFTWARE
ARCHITECTURAL DESIGN SOFTWARE



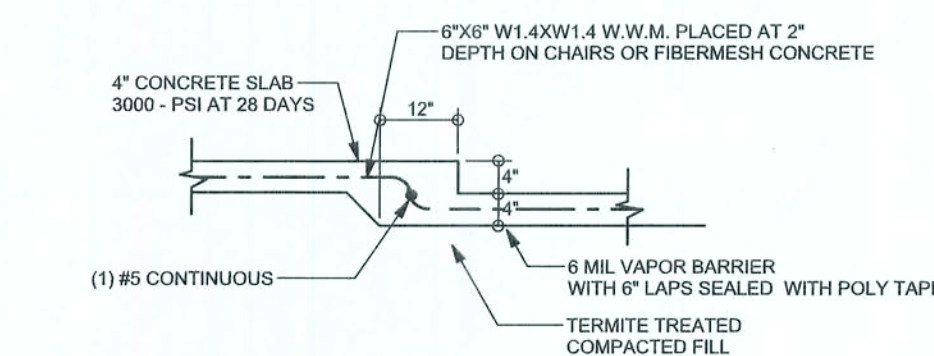
F2
S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



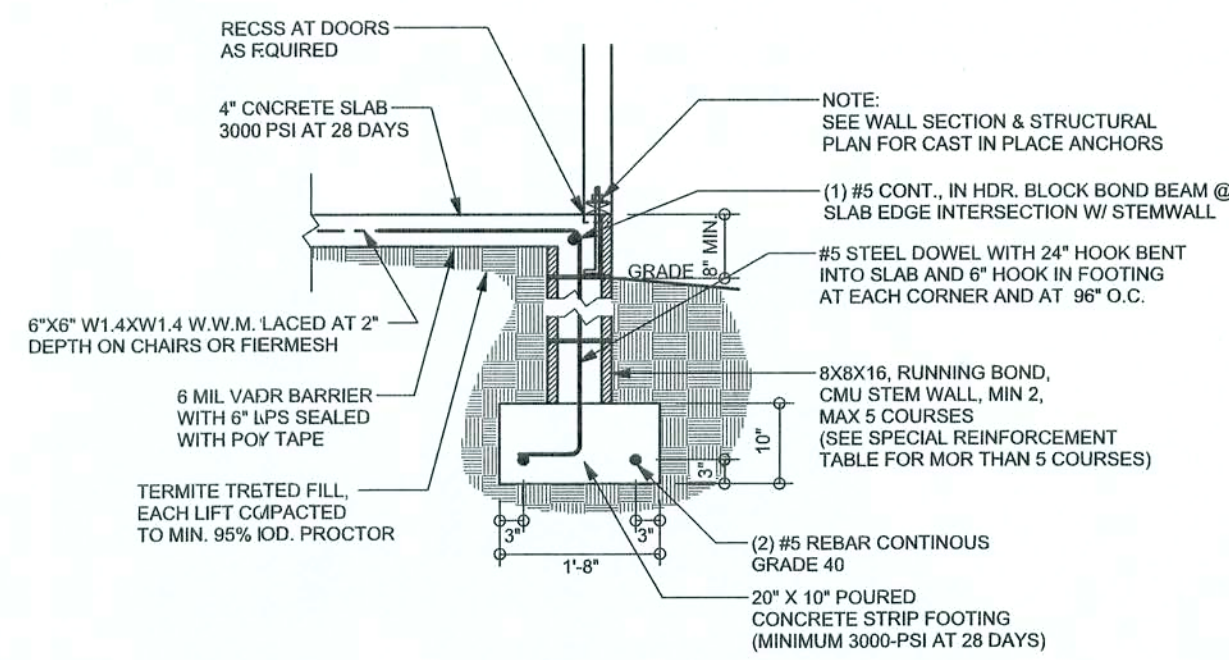
F3
S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



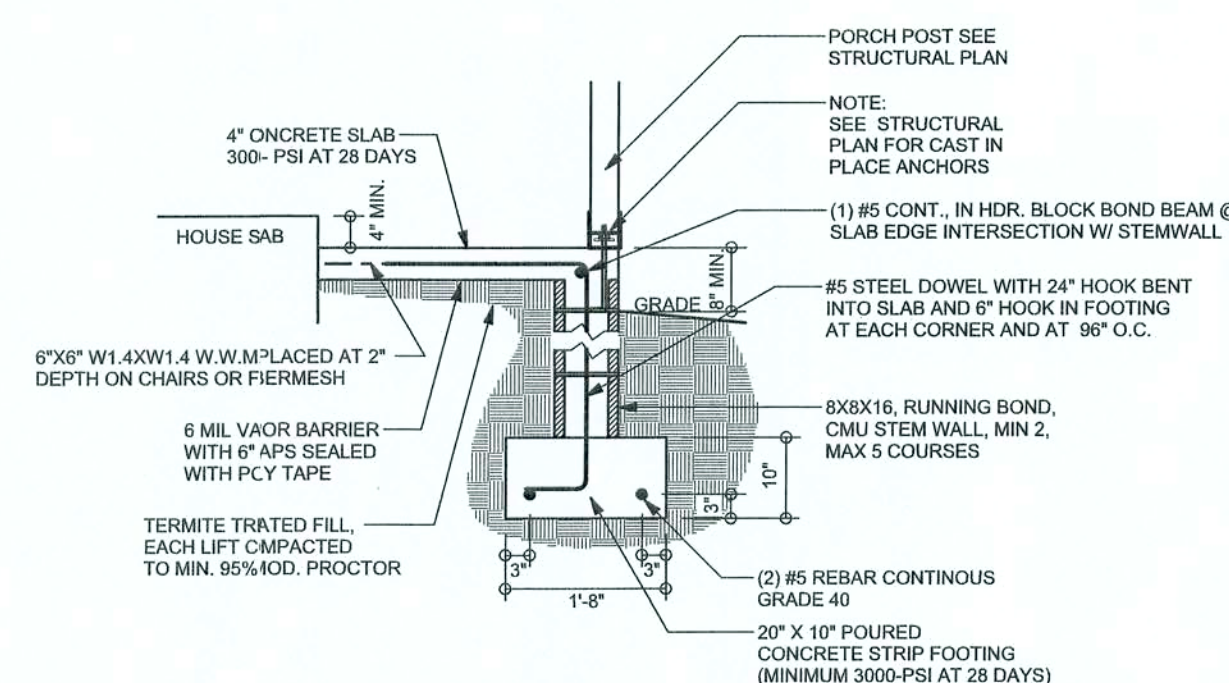
F5
S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



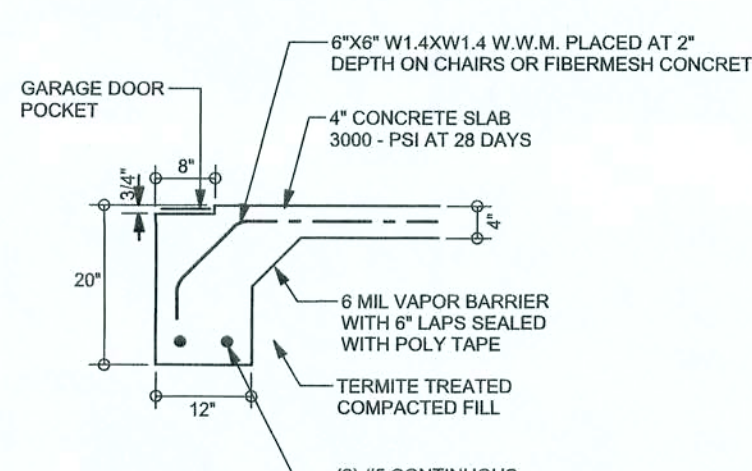
F6
S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



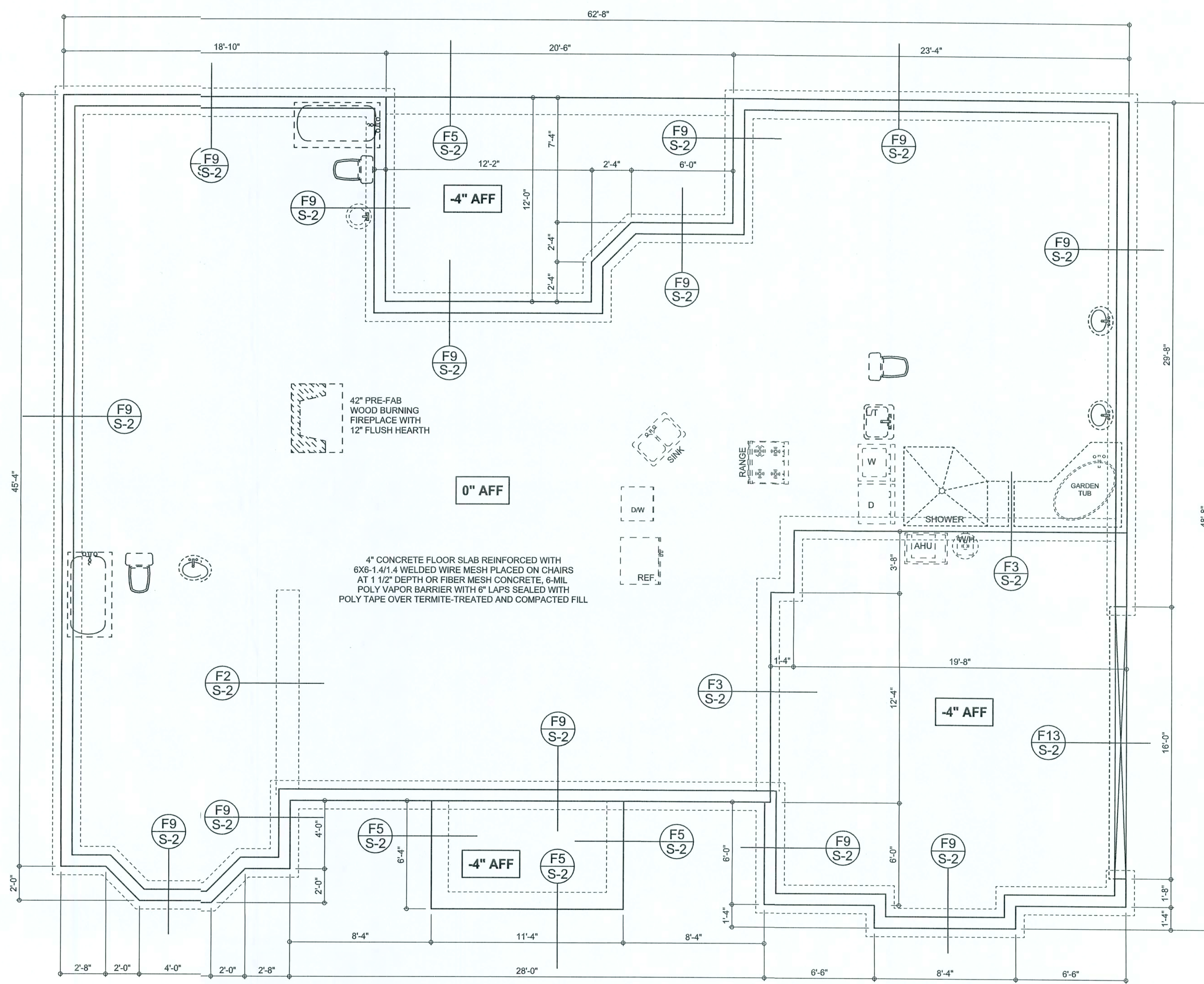
F9
S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F12
S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F13
S-2 ALT. STEM WALL GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 3/16" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Discway,
P.E. No. 53915, P.O. Box 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Discway, P.E. for resolution.
Do not proceed without clarification.

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permission and consent of Mark Discway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section 1609, Florida building code
2004, to the best of my knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISWAY
P.E. 53915

Mark Discway
0-DEC-05
SEAL

SPARKS
CONSTRUCTION

Spec House Lot #2
Rolling Meadows S/D

ADDRESS:
Lot #2 Rolling Meadows S/D
Columbia County Florida

Mark Discway P.E.
P.O. Box 868
Lake City, Florida 32026
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871
windloadengineer@bellsouth.net

PRINTED DATE:
November 30, 2005

DRAWN BY: Ben Sparks
CHECKED BY:

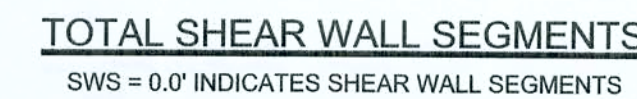
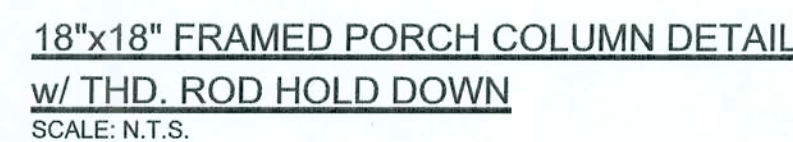
FINALS DATE:
30 / Nov / 05

JOB NUMBER:
511234

DRAWING NUMBER
S-2

OF 6 SHEETS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



	REQUIRED	ACTUAL
TRANSVERSE	39.3'	74.5'
LONGITUDINAL	38.2'	60.1'

SN-1	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
SN-2	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
SN-3	DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
SN-4	PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC511-03, BC511-B1, BC511-B2, & BC511-B3, BC511-B1, BC511-B2, & BC511-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

● INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD

⊗ INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

Diagram illustrating the components of a beam call-out box:

- HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE (JOB #L95241)

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, P.OB 868, Lake City, FL 32056, 386-754-5419

Dimensions: stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that all applicable portions of the plan, relating to wind engineering comply with section 610, Florida building code 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location

MARK DISOWAY
PE 53915

Mark Disoway

SEA/

Spec House Lot #2
Rolling Meadows S/D

ADDRESS:
Lot #2 Rolling Meadows S/D
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S-3

F 6 SHEETS