ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TYV8228Z0129103627

Truss Fabricator: Anderson Truss Company

Job Identification: 10-020--Fill in later ISAAC/ADAMS -- , **

Truss Count: 16

Model Code: Florida Building Code 2007 and 2009 Supplement

Truss Criteria: FBC2007Res/TPI-2002(STD)
Engineering Software: Alpine Software, Version 9.02.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-05 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: A1101505-GBLLETIN-

#	Ref Description	Drawing#	Date
1	62181 A	10029001	01/29/10
2	62182AGE	10029002	01/29/10
3	62183 H8A	10029003	01/29/10
4	62184H10A	10029004	01/29/10
5	62185 H11A	10029005	01/29/10
6	62186 AM	10029007	01/29/10
7	62187 H4108T	10029001	01/29/10
8	62188 AT	10029002	01/29/10
9	62189 H10108T	10029003	01/29/10
10	62190 H8108T	10029004	01/29/10
11	62191H6108T	10029005	01/29/10
12	62192CJ3T	10029006	01/29/10
13	62193 CJ1	10029008	01/29/10
14	62194HJ4108T	10029006	01/29/10
15	62195 EJ4108T	10029007	01/29/10
16	62196EJ4108	10029008	01/29/10







Seal Date: 01/29/2010

-Truss Design Engineer-Doug Fleming Florida License Number: 66648 1950 Marley Drive Haines City, FL 33844

Bot Webs 2x4 SP #3 :Stack Chord SC1 2x4 SP #2 Dense::Stack Chord chord chord chord 2x4 SP chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense

SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load

See DWGS All015050109 & GBLLETIN0109 for more requirements

Dropped top chord braced at 24" o.c. intervals. Attach stack chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord chord in notchable area using 3x6. Stacked top chord must NOT be notched or cut in area (NNL) interface, plate length perpendicular to chord length. Splice top o.c. intervals. Attach stacked top

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

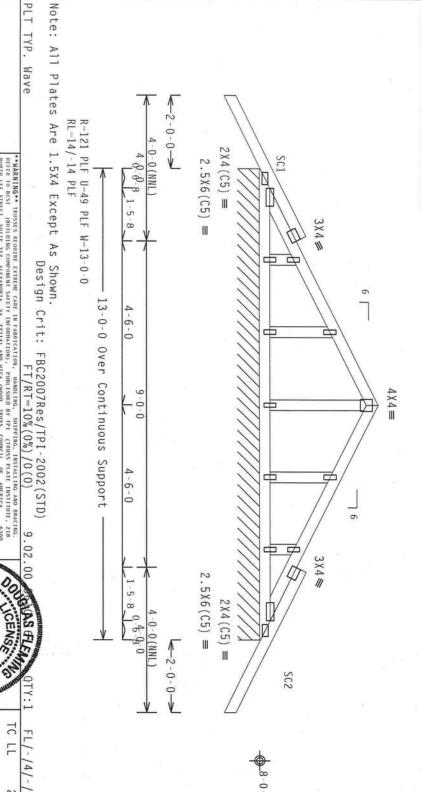
Wind reactions based on MWFRS pressures

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord

must not be cut or notched. In lieu of structural panels use purlins to brace 1C @

Deflection meets L/240 live and L/180 total load

Bottom chord checked for 10.00 psf non-concurrent live load



HROPH LET STREET, SHITE 312, ALEXANDRIA, YA, 22314) AND NICA (MODO TRUSS CONSCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, H. 1. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORHING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCCURAL PARELS AND BOTTOM CHORD SHALL HAVE NENT SAFETY INF (TRUSS PLATE INSTITUTE, 218

BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN; ANY FAILURE TO FOT JO FARRICATURG, HANDLING, SHIPPHOE, INSTALL ING. A BRACHEM OF T DESIGN CONTORNS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN CONNECTOR BY LATES ARE FANDS OF 20/18/1660 (M.)/55//K) ASTM ASSS GRADE FALTS TO FACIF TACE OF TRUSS AND. UNLESS OTHERHISE LOCATED ON THE **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR NOS (MATIONAL DESIGN SPIC, NY AFADA) AND TPI.
YSZYN ASTM ASSO BRADE 4050 (W. KJH. SS) GALV.
L BE PER ANNEL AS OF TPI1-2007 SEC. 3.
A CENGIMEENTAN EXSENDISHILITY SOLELY FOR THE RR. BUILD THE TRUSS IN CONFORMANCE WITH STEEL. APPLY SHALL NOT

ANY INSPECTION OF PLATES FOLIOND BY (1) SMALL BE PER ANN DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN SHOWN. THE SUITABILLITY AND USE OF THIS COMPONENT BULLDING DESIGNER PER ANSI/IPI 1 SEC. 2. DASTRICTLY OF THE A SEAL ON THIS

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL Cq 78



24.0" 40.0 10.0 1.25 10.0 PSF 0.0 PSF PSF PSF PSF JREF -SEQN-DATE REF HC-ENG DRW HCUSR8228 10029002 R8228-1TYV8228Z01 JB/DF 83822 01/29/10 62182

Scale = .375"/Ft.

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ $24\mbox{\ensuremath{^{\circ}}}\xspace$ 0C.

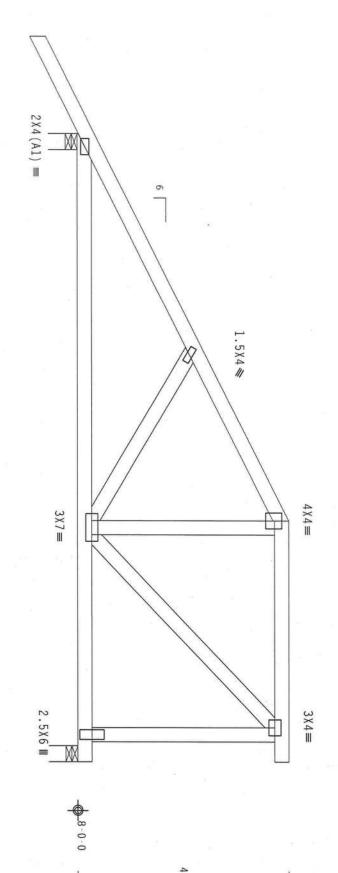
 $\ensuremath{\mathsf{MWFRS}}$ loads based on trusses located at least 7.50 ft. from roof edge.

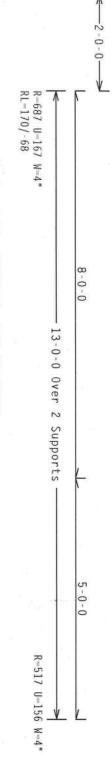
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load.





Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0)

TYP.

Wave

MARNING TRUSSES REDUTRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, BETER TO BOSS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FPT (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 132, ALEXANDRIA, VA, 2214) ABO UTCA (POOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANG, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERNISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED RIGHD CELLING.

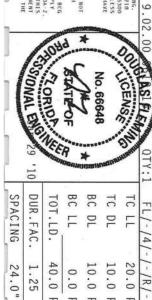
IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY OFFICIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORANCE WITH FPI; OR FARECATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (MATIGNAL DESIGN SPEC, BY ALRAY) AND TRI. ITH BEGE CONNECTOR PLAIRS ARE HANDE OF ZO/18/156A (M.H/SSY), ASTM AGSS GRANE BO/SO (M.K./M.SSY), AND AGAIN. STEEL, APPLY PLAIRS TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION FER IDMANINGS 160A-Z ANY INSPECTION OF PLAIRS FOLLOWED BY (1) SHALL BE PER MANEX AS OF TRIZ-ZOOZ SEC.3. AS SEAL ON THIS DEALHS INDICATES ACCEPTANCE OF REPESSIONAL ENGLIEERING RESPONSIBILITY SOFTHE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

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ALPINE

Haines City, FL 33844 FL CQ 100 778



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SP	DUF	TOT	BC LL	ВС	TC	TC LL
SPACING	10 DUR.FAC.	TOT.LD.	 	DL	DL	F
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
		PSF	PSF	PSF	PSF	PSF
JREF		SEQN-	HC-E	DRW	DATE	REF
1		•	NG	нси	0,00	R8
TYV82		83830	HC-ENG JB/DF	SR8228	01/2	228-
JREF - 1TYV8228Z01		30		DRW HCUSR8228 10029003	01/29/10	REF R8228- 62183
			4	ω	- 1	

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" σ 0C.

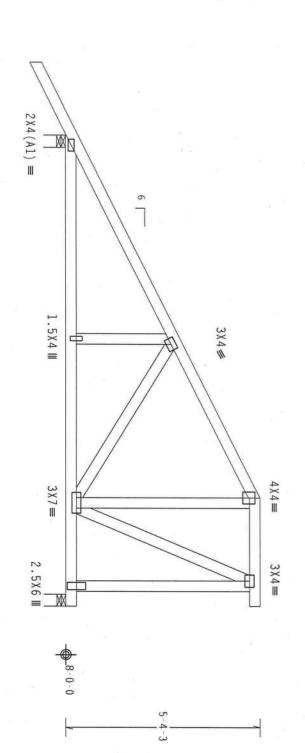
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

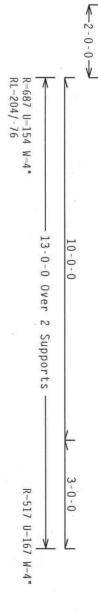
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/)=0.18

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load.





Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0) 9.02.

TYP. Wave

***HAKRN ING** HRUSES REQUIRE EXTREME CARE IN FARMICATION, TADDALERM, SOUTH CONTROL PARTIES FOR THE AMERICA, 218 ARTHER TO BOSS (QUINCING COMPONENT SAFETY INFORMATION), PORLISHED BY THE (FRUSE YARIE INSTITUTE, 218 AND STATE 112, ALLEANDRIA, VA. ZZZIA) AND NITA (WOOD TRUSS COUNCIL OF AMERICA, 6200 EXITERETS LAWE, MODISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERCONNING THESE FUNCTIONS. UNICESS OTHERWISE INDICATED TO PENORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

IMPORTANT URBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. HC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TO: OR FARELCHAITG, LANGILLON, SHEPPING, INSTALLING A BRACING OF TRUSSES.

THE BCG. TOWORDS WITH APPLICABLE PROVISIONS OF MS (MATIONAL DESIGN SPEC. MY AFRA) AND TPI. THE BCG. CONNECTOR PLATES ARE MADE OF 20/18/1664 (M.H/SS)/X ASTN A653 GRADE 40/50 (M.K/M.SS) GALV. SIEEL APPLY PLATES TO LACH TACE OF TRUSS AND. UNLESS OTHERSISE LOCATED ON THIS DESIGN, POSITION PER ORWAINGS 160A-Z.

CONNECTOR PLATES ARE MADE OF 20/18/166A (P.H.)SS/K) ASHA A653 GAME 40/50 (M. K.H., SS) GAVE, STEEL, APPLY PLATES TO LACH TACE OF TRUSS AND.

PLATES TO LACH TACE OF TRUSS AND.

MISS OTHERS IS CONTINUED ON THIS DESIGN. POSITION FOR BOARDING 160A-Z

ANY INSPECTION OF PLATES TOLLOWED BY (1) SMALL BE PER ANNEX A3 OF TPI1-2002 SEC. 3.

A SEAL ON THIS DANAING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SULELY FOR THE BRUSS COMPONENT DESIGN SHOWN.

HE SULTABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2.

TW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL CQ 78

CENS No. 66648 BC LL BC DL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-24.0" 40.0 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF PSF

SEQN-

HC-ENG

JB/DF 83841 DRW HCUSR8228 10029004

JREF -

1TYV8228Z01

REF DATE

01/29/10

Scale = .375"/Ft. REF R8228- 62184

SPACING

24.0"

JREF -

Top chord 2x4 SP #
Bot chord 2x4 SP #
Webs 2x4 SP # #2 Dense #2 Dense #3

Roof overhang supports 2.00 psf soffit load

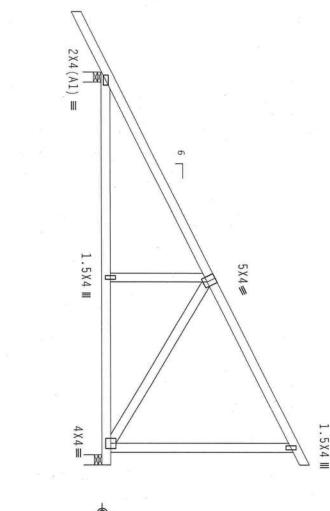
Bottom chord checked for 10.00 psf non-concurrent live load.

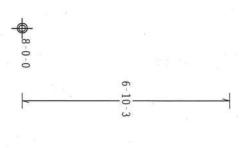
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load.







Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0)

TYP.

Wave

WARNING TRUSSES REDUTRE E REFER TO BCSI (BULLDING COMPA NORTH LEE SIREET, SUITE 312, AU ENTERPRISE LANE, MADISON, WI OTHERWISE INDICATED TOP CHORD \$ ORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE ALEXANDRIA, VA. 22314) AND NICA (MOOD TRUSS COUNCIL OF AMERICA. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. TRUSS PLATE INSTITUTE, 218 UNLESS

ANY INSPECTION OF PLATES FOLIDIED BY (1) SHALL BE PER ANNEX AS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONDED ESSON SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT FOR BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. **IMPORTANT***UNBRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE DEG. THE SHALL HID BE RESPONSIBLE ON ANY DEVIATION FROM THIS DESIGN. ANY FALLURG TO BUILD THE TRUSS IN CONFORMACE WITH PET: ON FARRICATING, AND FLICALL FROM SHALLING, SHEPPING, METALLING & BRACH METALLING TO BUILD THE TRUSS IN CONFORMACE WITH THE BEG. THE TRUSS TO BE TH IS DESIGN. POSITION PER OR
TP11-2002 SEC.3. A
IBILITY SOLELY FOR THE IR A SEAL ON THIS

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ALPINE

Haines City, FL 33844 FL Cl 278

BUILDING IS THE RESPONSIBILITY OF THE STANDING SOUCENS IN O Con Con No. 666 QTY:6

JREF - 1TYV8228Z01	JRE	4	24.0"	SPACING	-
			1.25	10 DUR.FAC.	29 · 10
N- 83876	SEQN-	PSF	40.0 PSF	TOT.LD.	ORINE
HC-ENG JB/DF	HC-I	PSF	0.0 PSF	BC LL	OF R
DRW HCUSR8228 1002900	DRW	PSF	10.0 PSF	BC DL	**************************************
01/29/10	DATE	PSF	10.0 PSF	TC DL	*****
REF R8228- 62186	REF	PSF	20.0 PSF	TC LL	SAM

10029007

Scale =.3125"/Ft.

Top chord :W10 chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 :W2 2x8 S 10 2x4 SP #2 Dense: SP #1 Dense:

Roof overhang supports 2.00 psf soffit load

In lieu of structural panels use purlins to brace all flat TC @ $24\,\text{{\tt "}}$ OC.

Deflection meets L/240 live and L/180 total load

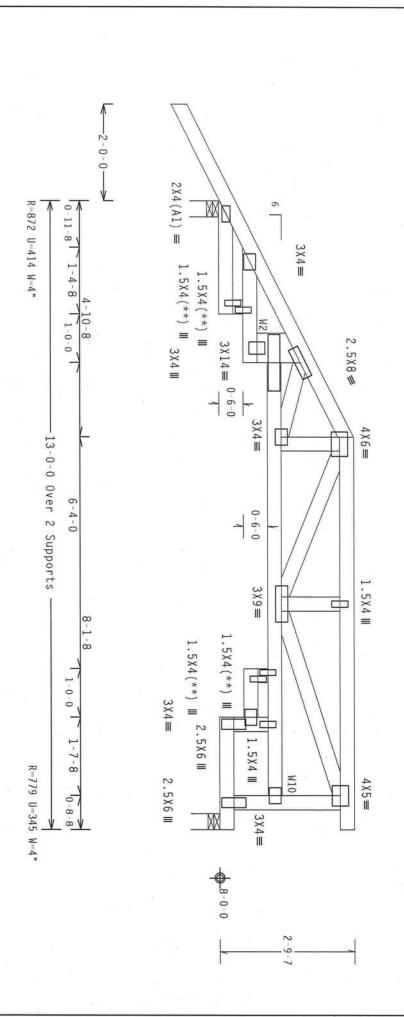
Laterally brace Bottom Chord above filler at 24" oc including a lateral brace at chord ends.

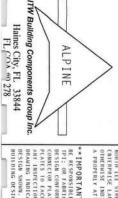
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

#1 hip supports 4-10-8 jacks with no webs





REFER TO BCSI (BUILDING NORTH LEE STREET, SUITE 3. ENTERPRISE LANE, MADISON, OTHERWISE INDICATED TOP CO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE COMPONENT SAFETY ING. INSTALLING AND BRACING. (TRUSS PLATE INSTITUTE, 218 DUNCIL OF AMERICA, 6300 UNLESS

Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=10%(0%)/0(0)

9.02

FL/-/4/-/-/R/-

Scale = .5"/Ft.

R8228- 62187

01/29/10

TYP.

Wave

IMPORTANT*PURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OF FARTCATURE, MANDLING, SHIPPING, INSTALLING & BRACIE OF TRUSSES.
BESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY AFAPA) AND IPI. ITW BCG

TE 40/60 (H. K/H.SS) GALV. STEEL APPLY
TIS DESIGN, POSITION PER DRAWINGS 160A-Z.
BILLIV

PII-2002 SEC.3. A SEAL ON THIS ILLITY SOLELY FOR THE TRUSS COMPONENT BUILDING IS THE RESPONSIBILITY OF THE

OS/ONAL ENGINEE CENS BC DL TC DL TC LL DUR.FAC. BC LL SPACING TOT.LD. 40.0 10.0 24.0" 10.0 20.0 PSF 1.25 0.0 PSF PSF PSF PSF SEQN-DATE JREF -REF HC-ENG DRW HCUSR8228 10029001

TCE / DF 84286

Top chord 2x4 SP + Bot chord 2x4 SP + Webs 2x4 SP + p #2 Dense p #2 Dense p #3 :W2 2x6 SP #2:

Roof overhang supports 2.00 psf soffit load

Bottom chord checked for 10.00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load.

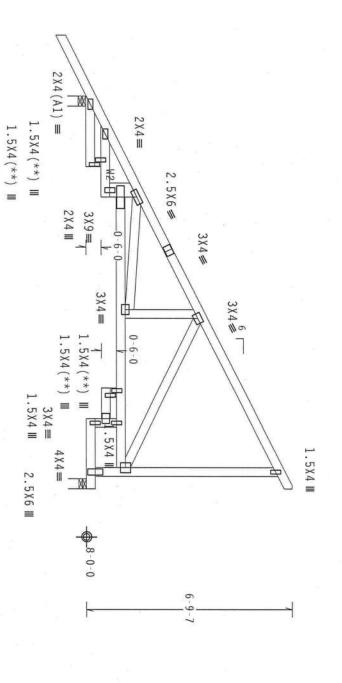
Laterally brace Bottom Chord above filler at 24" oc. including a lateral brace at chord ends.

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/)=0.18

Wind reactions based on MWFRS pressures

MWFRS loads based on trusses located at least 7.50 ft. from roof



R=687 U=130 W=4" RL=254/-87 -11-81-4-8 1-0-0 Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0) 13-0-0 Over 2 Supports 6-4-0 9.02.00 -0-0 1-7-8 d-8-8 R-517 U-183 W-4"

WARNING IRBUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RELIER TO BEST (BUILDING COMPONENT SAFETY IMPORATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 213 NORTH LEE SHEET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (MOOD TRUSS COUNCIL OR AMERICA, 6300 ERREBERSE LAKE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLDSCALED TOP COMED SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

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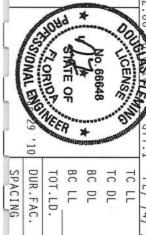
Wave

IMPORTANTQUENTSH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH THE CONTRACTOR OF FARELGALING, HANDLING, SHAPPLY, HANDLING, A BRACHER OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY AFRAYA AND TPL. THE GO CONNECTOR PLATES ARE MORE OF 20/18/1660 (M.M./SS/K), ASTM A653 GRADE 40/60 (M.K.M.SSE), GALV. STELL APPLY PLATES TO EACH FACE OF TRUSS AND, (MLISS OTHERWISE LOCATED ON THIS DESIGN, POSITION FOR DRAMINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWER OF THE SESSIONAL ENGLIEF HANDLE AS OFFICE OF THE SESSIONAL FOR THE THE DESIGN OF THE TRUSS COMPONENT DRAWN HANDLING INCOMES ACCEPTANCE OF PROFESSIONAL ENGLIEF HANDLE AS OFFICE OF THE RESPONSIBILLITY OF THE DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI I SEC. 2.

ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278

ALPINE



10			Marian	Market Street	
DUR.F	T0T.1	BC LI	BC DI	TC DI	TC LL
AC.	.D.				68
1.2	40.0	0.0	10.0	10.0	20.0
	PSF	PSF	PSF	PSF	20.0 PSF
1	SEON	HC-EN	DRW	DATE	REF
4		NG TCE	HCUSR82	0.1	R8228
	1202	E/DF	28 10029002	1/29/10	REF R8228- 62188
	10 DUR.FAC. 1.25	TOT.LD. 40.0 PSF DUR.FAC. 1.25	BC LL 0.0 PSF TOT.LD. 40.0 PSF DUR.FAC. 1.25	BC DL 10.0 PSF BC LL 0.0 PSF TOT.LD. 40.0 PSF DUR.FAC. 1.25	TC DL 10.0 PSF BC DL 10.0 PSF BC LL 0.0 PSF TOT.LD. 40.0 PSF DUR.FAC. 1.25

Scale = .3125"/Ft.

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3 :W2 2x6 SP #2:

Roof overhang supports 2.00 psf soffit load

In lieu of structural panels use purlins to brace all flat TC 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live

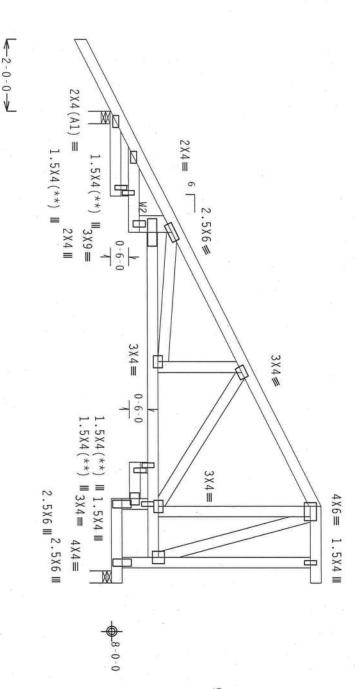
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

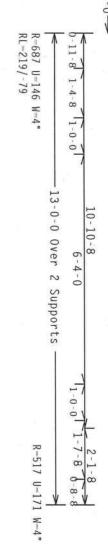
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.





Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0)

TYP.

Wave

ENTERPRISE LANE, MAD CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE . SHIPPING, INSTALLING AND BRACING.
BY TPI (TRUSS PLATE INSTITUTE, 218
TRUSS COUNCIL OF AMERICA, 6300 UNLESS

TMPORTANTTURNISM A COPY OF THIS DESIGN: ANY FAILURE TO BUILD HEE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN: ANY FAILURE TO BUILD HEE HE PE: OR FARRICATING, MANDLING, SHIPPING, HESTAILING A BRACING OF HUSSES.

DESIGN CONFIDENCY HE APPLICABLE PROVISIONS OF HDS (MAIDMAL DESIGN SPEC, BY AFT CONNECTOR PLAIRS, ARE HABED (* ZO/18) 7600, (4) H/SS/N, ASTH ASS) GRADE 40/60 (4). N CONTRACTOR. ITW BCG. INC. SHALL NOT BUILD THE TRUSS IN CONFORMANCE WITH

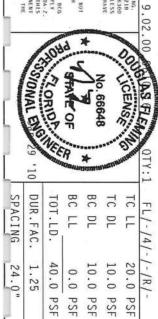
AMING INDICATES ACCEPTANCE OF F BUILDING IS THE RESPONSIBILITY OF THE SPEC, BY AFAPA) AND TPI.

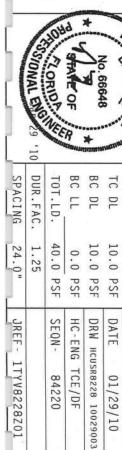
10/60 (H. K/H.SS) GALV. STEEL APPLY
DESIGN, POSITION PER DRAHINGS 160A-Z SEAL ON THE

ITW Building Components Group Inc. Haines City, FL 33844 FL 77 79 278

DESIGNER PER ANSI/TPI 1 SEC

ALPINE





REF

62189

Scale = .375"/Ft. R8228-

Top chord 2x4 Bot chord 2x4 chord 2x4 SP chord 2x4 SP Webs 2x4 SP) #2 Dense) #2 Dense) #3 :W2 2x6 SP #2:

Roof overhang supports 2.00 psf soffit load

In lieu of structural panels use purlins to brace all flat TC @ $24\mbox{\,{}^{"}}$ OC.

Bottom chord checked for 10.00 psf non-concurrent live

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Laterally brace Bottom Chord above filler at 24" oc. including a lateral brace at chord ends.

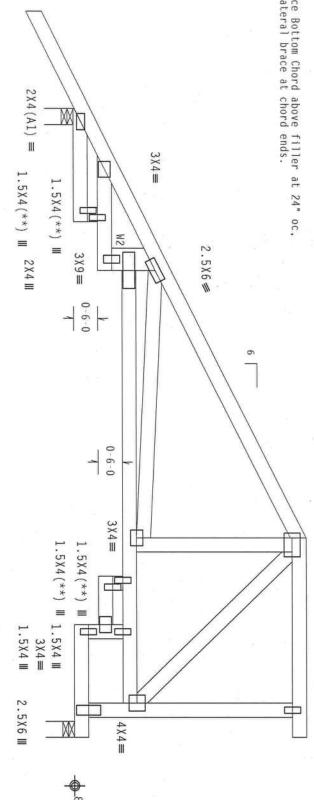
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

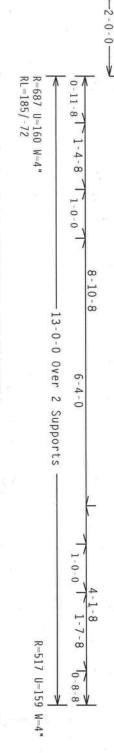
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load.

4 X 6 ≡ 1.5X4 Ⅲ





Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0) 9.02.00

FL/-/4/-/-/R/-

Scale =.5"/Ft. R8228-

TYP.

Wave

REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FOT (TRUSS PLATE INSTITUTE, 2108 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 27314) AND NECK (4000) TRUSS COUNCEL OF AMERICA, 63000 ENTERPRISE LAME, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE EURCITONS, UNLESS OTHERWISE HIDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED. HANDLING, SHIPPING, INSTALLING AND BRACING, PUBLISHED BY IPI (TRUSS PLATE INSTITUTE, 218 FCA (HOOD TRUSS COUNCIL OF AMERICA, 6300

IMPORTANT unusisu a copy of inis design to the installation contractor.

**REPRONSIBLE TOR ANY OFVIATION FROM THIS DESIGN, ANY FALURE TO BUILD THE TRU

**PI: OR FARRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE TRU

**RESIGN CONFIGERS HITH APPLICABLE PROVISIONS OF HIS SEAL SHARD ADDRESS AFEA OF AFA

**CONNECTOR PLATES ARE MADE OF \$70/18/16/AG, (H.H.YSX), ASIA MASS GRADE 40/40 (H.K.) PLATES TO EACH FACE OF TRUSS AND ON CONTRACTOR. ITH BCG, INC. SHALL NOT BUILD THE TRUSS IN CONFORMANCE WITH

DRAWING INDICATES ACCEPTANCE OF PROF DESIGNER PER ANSI/IPI WHIS DESIGN, POSITION PER DRAHING MONTH. ON THE CONTROL PER DRAHING MONTH OF THE CONTROL PER DRAHING MONTH ON THE CONTROL PER DRAHIN TPI1-2002 SEC.3. A SEAL ON THIS BILITY SOLELY FOR THE TRUSS COMPONENT BUILDING IS THE RESPONSIBILITY OF THE

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL 27 40 278



TCE / DF 84239

01/29/10 62190

Top chord chord 2x4 SP chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3 :W2 2x6 SP #2:

Roof overhang supports 2.00 psf soffit load

In lieu of structural panels use purlins to brace all flat TC @ $24\ensuremath{^{\prime\prime}}\xspace$ 0C.

Bottom chord checked for 10.00 psf non-concurrent live

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

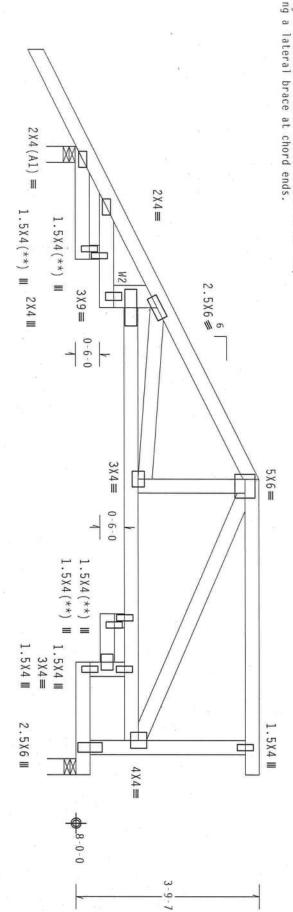
Laterally brace Bottom Chord above filler at $24\,^{\circ}$ oc. including a lateral brace at chord ends.

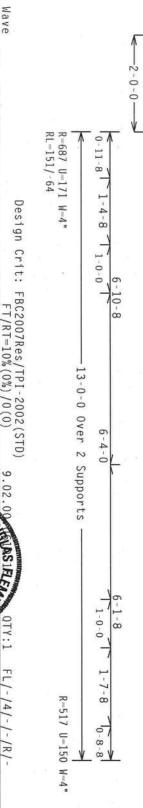
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/ $^{\prime}$)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load





REFER TO BEST (BULLDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB HORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WIGH (MOOD TRUSS COUNCIL OF AMERICA, 6300 EHTERPRISE LAME, MADISOM, WI 53719) FOR SAFETY PRACTICES PRIOR TO EBEFORMING THESE UNCTIONS. UNLESS OHERWISE TROTCATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION.
REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION). SHIPPING, INSTALLING AND BRACING.
TPI (TRUSS PLATE INSTITUTE, 218
USS COUNCIL OF AMERICA, 630 PLT

TYP.

IMPORTANT**UBMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR: ITW ECG. THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH THIS DESIGN, OF TABLEATING, AND THIS DESIGN COMPORTS OF THE THIS DESIGN COMPORTS HITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY ATAPA) AND THIS DESIGN COMPORTS HITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY ATAPA) AND THIS DESIGN COMPORTS HITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY ATAPA) AND THIS DESIGN FOR THE APPLY PLATES TO EACH FACE OF TRUSS AND, UNITES OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2 PLATES TO EACH FACE OF TRUSS AND. UNITES OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2

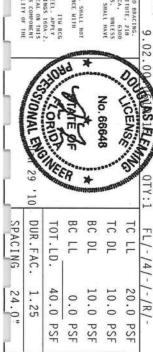
DESIGN SHOWN. THE SUITABILITY AND USE OF BUILDING DESIGNER PER ANSI/TPI I SEC. 2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING SIGN SPEC, BY AFAPA) AND IP1. IIW BCG
IADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY
THIS DESIGN, POSITION PER DRAWINGS 160A-Z.

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL 2012/19 278

BUILDING IS THE RESPONSIBILITY OF THE SEAL ON THIS



SEQN-

JREF - 1TYV8228Z01

HC-ENG

TCE / DF 84258

DRW HCUSR8228 10029005

DATE REF

01/29/10

Scale =.5"/Ft.

R8228- 62191

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 10-020--Fill in later ISAAC/ADAMS -- . ** . CJ3T)

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load

Roof overhang supports 2.00 psf soffit load

Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails (0.162"x3.5"), toe nailed at Top chord. 2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord.

2X4(A1) = \mathbb{M} 1.5X4 Ⅲ 1.5X4 Ⅲ 3 X 4 ≡ Фr R=19 U=3

R=54 U=21 9-6-11

8-0-0 8-6-0

-2-0-0->

R-317 U-86 W-4" RL-85/-49 3-0-020ver03 Supports

Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0)

TYP.

Wave

PARAMATAN STRUCTURE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.

REFER TO SHEET (BUILDING COMPONENT SAFETY HAPDMAITON), PUBLISHED BY TPI (FRUSS PLAIE INSTITUTE, 218

WORTH LEE SIREE, SUITE 137, ALEXANDRIA, VA, 22713) AND WICK, (4000) TRUSS COMPCIL OF AMERICA, 6300

ENTERPRIS LAME, MADISON, MI 55719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE HODICATED TOP CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

TMPORTANT*UNNISH A CORP OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE NGG. NING. SHALL NOT BE RESPONSIBLE FOR ANY DEFINATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMACE WITH PI: OR FAMILICATING, MAINTHING, SUPPRING. HISTAIL HOLD, A BRACHE OF BRUSSES. HE TRUSS IN COMPORMACE WITH PI: IT HE GET CONNECTED BY ALTER ARE PAGE OF 20/189/560. HE SHALL SO THE SHALL BESTON A SHEED, AND FIT. IT HE GET CONNECTED BY ALTER ARE PAGE OF 20/189/560. HE HISTS OTHERAUSE LOCATED ON THIS DESIGN. POSITION PEE DRAWINGS 160A-Z. ANY INSPECTION OF PAGE STOLUGHED BY OUT SHALL HE FER AMER AS OF THIS JOBS SEC. 3. OF THE SHALL SHAL

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/IPI I SEC. 2.

9.02.00 ON BLASS ITES OSIONAL ENGINEE CENSE QTY:2 10 BC DL TC DL DUR.FAC. BC LL TC LL SPACING TOT.LD. FL/-/4/-/-/R/-

40.0

PSF PSF

SEQN-

83620

0.0

HC-ENG

JB/DF

1.25

20.0 PSF

Scale =.5"/Ft.

R8228- 62192

DATE REF

01/29/10

10.0 PSF 10.0 PSF

DRW HCUSR8228 10029006

24.0"

JREF -

1TYV8228Z01

ITW Building Components Group Inc. Haines City, FL 33844 FL 201 #9 278 ALPINE

Top chord 2x4 SP Bot chord 2x4 SP #2 Dense #2 Dense

Roof overhang supports 2.00 psf soffit load

Bottom chord checked for 10.00 psf non-concurrent live load.

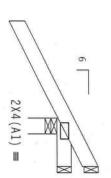
Provide (2) 16d common nails (0.162"x3.5"), toe nailed at Top chord. 2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load.

R=-110 Rw=72 U=103







R=-35 Rw-31 U=36

1-0-0 Over 3 Supports R=361 U=151 W=4" RL=50/-42

Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0)

9.02.00

QTY:2

FL/-/4/-/-/R/-

Scale = .5"/Ft.

R8228- 62193

TYP.

Wave

ENTERPRISE LANE, MADISON, THERHISE INDICATED TOP CHOSED SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

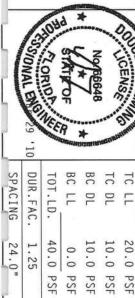
IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. BE RESPONSIBLE FOR ANY DEPLATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUE FROM FRANCE OF THIS CONTRACTOR. AND THIS CONTRACTOR. AND THIS CONTRACTOR. AND THIS CONTRACTOR FRANCE OF THE APPLICABLE PROVISIONS OF THIS (MAINTAIN CRESSON SPEC. N MAINTEN OF THE CONTRACTOR PLATES ARE MADE OF ZO/18/15/6M, CHAPSSEY, ASTHAMASS TO BEACH 40/50 (M. M. PASSEY). ASTHAMASS THE ADDRESS OF THE APPLICABLE PROVISIONS OF THE MAINTEN OF THE ACTION PLATES OF THE APPLICABLE PROVISIONS OF THE ACTION OF DRAHING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING TION CONTRACTOR. ITW BCG, INC. SHALL NOT BUILD THE TRUSS IN COMPORMANCE WITH TRUSSES.

DESIGN SHOWN. ITE. ANSI/TPI 1 SEC. ANY BUILDING IS THE RESPONSIBILITY OF THE SIGN SPEC. BY AFAPA) AND TPI. I'M BCG
NADE 40/60 (M. K/H. SS) GALV. SIEEL. APPLY
THIS DESIGN. POSITION PER DRAWINGS 160A-2
OF TPII-2002 SEC.3. A SEAL ON THIS

ITW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL CO 278



PSF

HC-ENG

JB/DF 83624

SEQN-

JREF -

1TYV8228Z01

DATE REF

01/29/10

DRW HCUSR8228 10029008

Top chord 2x4 SP #
Bot chord 2x4 SP #
Webs 2x4 SP # Deflection meets L/240 live and L/180 total load. Hipjack supports 4-10-8 setback jacks with no webs. (10-020-Fill in later ISAAC/ADAMS --ITW Building Components Group Inc. TYP. Haines City, FL 33844 FL Cox 49 278 ALPINE Wave #2 Dense #2 Dense #3 **IMPORTANI** TREMESS A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THY BCG, IRC. SMALL NOT THE RESPONSIBLE FOR ANY DEVIATION PROM HIS DESIGN. ANY FALLING TO BUILD THE FURS IN COMPERMANCE WITH FPI: OR FARBICATING, MADDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

BESIGN CONFIDENCE WITH APPLICABLE PROVISIONS OF BUS (AATIONAL DESIGN SPEC, BY MAPA) AND IPP.

CONNECTOR PLATES ARE MADE OF TO/189/JORGA (M. H/SS/S), ASTH AGS GRADE TO/500 (M. K/M/SS) DALY. SIEEL: APPLY CONNECTOR PLATES ARE MADE OF TO/189/JORGA (M. H/SS/S). ASTH AGS GRADE TO/500 (M. K/M/SS) DALY. SIEEL: APPLY ENTERPRISE LANE, MADISON, BUILDING DESIGNER PER ANSI/IPI I SEC. ARTHG INDICATES ACCEPTANCE OF F N. WI 53719) FOR SAFETY PRACTICE'S PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE * Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0) HJ4108T) 2X4(A1) =4.24 R=385 U=432 W=5.657" W NAL DESIGN SPEC. BY AFAPA) AND IPI.

ITH 8EG
A653 GRADE 40/60 (M. K/H.SS) GALV. STEEL. APPLY
FED ON THIS DESIGN, POSITION PER BRANTINGS 16GA-Z
NEX A3 OF IPI1-2002 SEC.3.

A SEAL ON THIS NANDLING, SHIPPING, INSTALLING AND BRACING, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 CA (MOOD TRUSS COUNCIL OF AMERICA, 6300 3-3-10 OF TPI1-2002 SEC.3. A SEAL ON THIS
DWS181LITY SOLELY FOR THE TRUSS COMPONENT
ANY BUILDING IS THE RESPONSIBILITY OF THE -6-10-12 Over 1.5X4 III 1.5X4 III 3 \ 4 = 中 Φ 3 Supports 1 - 5 - 0110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18 Provide Wind reactions based on MWFRS pressures. Provide 1.5X4 Ⅲ 9.02.00 3 X 4 ≡ 3X4= (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord O LICENS 2-2-3 0.66648 4 X 4 == 10 R-108 U-52 R-145 U-123 BC DL TC DL DUR.FAC. BC LL TC LL SPACING TOT.LD. FL/-/4/-/-/R/-40.0 10.0 20.0 24.0" 1.25 10.0 PSF 0.0 8-0-0 9-0-0 10-5-10 PSF PSF PSF PSF DATE REF JREF- 1TYV8228Z01 SEQN-DRW HCUSR8228 10029006 HC-ENG Scale = .5"/Ft. R8228- 62194 TCE / DF 01/29/10 84173

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

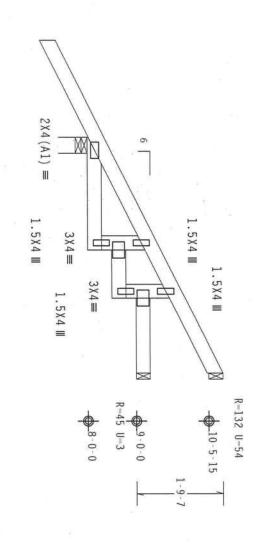
(2) 16d common nails (0.162"x3.5"), toe nailed at Top chord. (2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord.

Provide Provide

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



-2-0-0-> R-373 U-88 W-4" RL-117/-57 -4-10-8 Over 2-4-0 1-0-0 3 Supports 1-6-8

Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0)

9.02.00

QTY:3

FL/-/4/-/-/R/-

20.0 PSF

REF

R8228- 62195

01/29/10

Scale =.5"/Ft.

TYP.

Wave

NORTH LEE STREET, SHITE 31 ENTERPRISE LANE, MADISON, OTHERWISE INDICATED TOP CH OTHERHISE TROITED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION CHORD SHALL HAVE A PROPERLY ATTACHED REGION CHORD SHALL HAVE *WARNING** TRUSSES ATION, HANDLING, SHIPPING, INSTALLING AND BRACING. 10N), PUBLISHED BY IPI (TRUSS PLATE INSTITUTE, 218 AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 630

IMPORTANT**UNRMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE PROSS IN COMPORMANCE WITH TP: OR FARELCHING. MINDFULDS. SHEPPING. INSTALLING & BRACING OF TRUSSES.

DISTOR CONTRONS WITH APPLICABLE PROVISIONS OF MOS (WALTONAL DESIGN SPEC, BY AFAPA) AND TPI. BLACK CONNECTOR PLATES ARE MADE OF ZO/189/166A W.H.SSY, ASTH ASS JARANE 409/60 (W. X.M.SS) AGAINE 409/60 (W. X.M.SS)

ANY INSPECTION OF PLATES FOLICHED BY (1) SMALL BE PER A DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERI DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPON BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. IN ACC. BY ARRA) AND PPL.

IN ACC.

IN ACC. BY ARRA) AND PPL.

IN ACC. SIEEL. APPLY

IN IS DESIGH, POSITION PER DRAMINGS 160A-Z

OF PPI-2002 SEC.3.

OF PPI-2002 SEC.3.

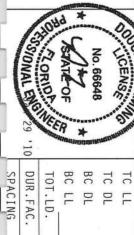
ONSIBILITY SOLELY FOR THE TRUSS COMPONENT

ONSIBILITY OF THE

ITW Building Components Group

ALPINE

Haines City, FL 33844 FL 79 278



40.0 1.25 24.0" 10.0 PSF 10.0 PSF 0.0 PSF PSF JREF - 1TYV8228Z01 SEQN-DATE DRW HCUSR8228 10029007 HC-ENG

TCE/DF

84179

(10-020--Fill in later ISAAC/ADAMS --** EJ4108)

Top chord 2x4 SP Bot chord 2x4 SP #2 Dense #2 Dense

Roof overhang supports 2.00 psf soffit load

Bottom chord checked for 10.00 psf non-concurrent live load.

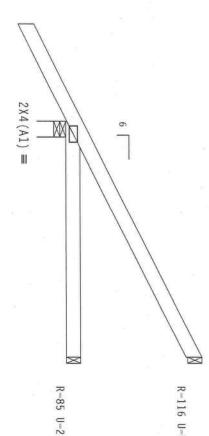
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

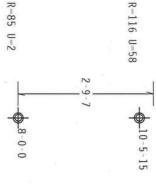
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

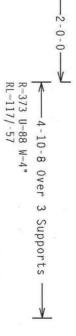
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load

Provide Provide (2) 16d common nails(0.162"x3.5"), toe nailed (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. at Bot chord.







Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0)

TYP.

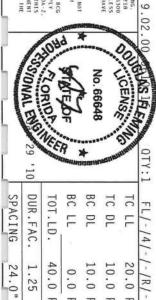
Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FARRICATION, JANGUIGE, SHIPPING, INSTALLING AND BRACING.
RETER TO REST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY DIT (TRUSS PLATE INSTITUTE, 218
MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND WITCA (MODO TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LAME, MODISON, HI 55719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OHIGHNISE INDICATED TOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE
A PROPERLY ATTACHED REGID CELLING.

DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group Inc. Haines City, FL 33844 FL Co. 49 278

ALPINE



10.0 20.0

PSF PSF

DATE

01/29/10

REF

R8228- 62196

Scale =.5"/Ft.

10.0 PSF 0.0

DRW HCUSR8228 10029008

1.25 40.0

PSF PSF

SEQN-

84183

HC-ENG

TCE/DF

24.0"

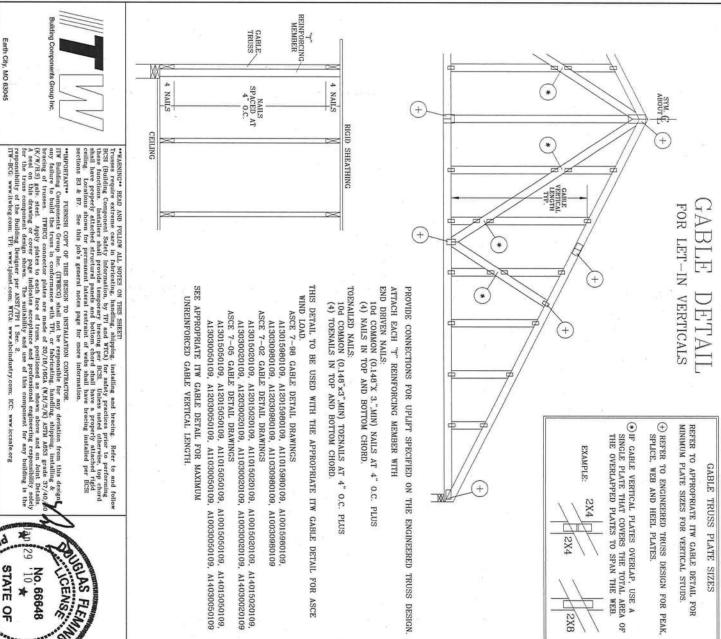
JREF -

AT EACH END. DIAGONAL BRACE FOR 600# BRACE IS USED. DOUBLED ERTICAL LENGTH MAY BE DIAGONAL BRACE OPTION: MAX GABLE VERTICAL LENGTH CH END. MAX WEB LENGTH IS 14'. Earth City, MO 63045 IN TABLE ABOVE VERTICAL LENGTH SHOWN SPACING | SPECIES WHEN DIAGONAL 12" O.C. 16 24 O.C. O.C. ASCE CONNECT DIAGONAL AT MIDPOINT OF VERTICAL GABLE VERTICAL CONNECT SPF SPF SPF DFL DFL DFI SP ~ -05: GRADE STANDARD STANDARD STANDARD STANDARD STANDARD STANDARD STUD STUD #3 STUD STUD WEB **HUPDEFNAT*** FIRBUSH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

IT Building Components Group Inc. (ITREGO shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TFL or fabricating, and integrated the trusses. ITREGO connector plates are made of 20/16/160A (***EH/25/K) ASTM AGGS apack 37/40/60 (***K/W/K) gats, steel. Apply plates to each face of truss, positioned as shown above and on John Details A seal on this drawing or cover page indicates acceptance and professional agineering responsibility and use of this component for any building is the responsibility and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2 of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2 of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2 of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2 of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2 of this component for any building is the #3 #2 #3 #3 #3 #3 BRACE ""wakeN(not" READ AND FOLLOW ALL NOTES ON THIS SHEET;
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow
FUS; (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing
these functions. Installers shall provide temporary bracing per BCS. Unless noted otherwise, top chord
shill have properly attached structural panets and bottom chord shall have a properly attached rigid
shill be actions shown for permanent lateral restricts of webs shall have bracing installed per BCSI
sections B3 & BT. See this job's general notes page for more information. 110 GABLE TRUSS 3' 10' 4 0 MPH GROUP A (1) 1X4 "L" BRACE * (1) 2X4 "L" BRACE * 2X4 STUD, #3 OR BETTER DIAGONAL OR DOUBLE CUT (AS SHOWN) AT WIND BRACE: SINGLE UPPER END GROUP B GROUP A SPEED, සු ගු GABLE 10' 0' 10 0 9' 7" 10, LE STUD , 15' ME/ GROUP B MEAN HEIGHT, ENCLOSED, REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH ABOUT C 18 REINFORCEMENT DETAIL (2) 2X4 "L" BRACE ** GROUP A 000000000 0 9 10 10 10 10 00000 CONTINUOUS BEARING 2X4 #2N OR BETTER GROUP B GROUP A 9. 9. 9 4 8 8 10 10 (1) 2X6 "L" BRACE . 10, 12, 13 14 114' 0" 114' 0 14 14 29 Nq₀66648 000 **(** STATE OF GROUP B 13 14 14 12 14 14 10 25 00000000 0 0 0 GROUP A (2) 2X6 "L" BRACE ** 11 14' 0" 14 14' 0" 14 14 14' 0" * 1.00, 0 0 MAX. MAX. GROUP B 14 14' 0" 14' 0" 14' 0" 14. 14. EXPOSURE TOT. SPACING 0 0 0 LD. ATTACH EACH "L" BRACE WITH 10d NAILS.
(0.188 "x3" min)
(0.188 "x3" min)

** FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

*** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. GABLE END SUPPORTS LOAD FROM 4' 0"
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB LIVE LOAD DEFLECTION CRITERIA IS L/240. SPRUCE-PINE-FIR
#1 / #2 STANDARD
#3 STUD PLYWOOD OVERHANG. DOUGLAS FIR-LARCH BRACING GROUP SPECIES GABLE 60 SOUTHERN PINE GREATER THAN 4' 0", BUT LESS THAN 11' 6" 24.0" GREATER THAN 11' 6' VERTICAL LENGTH LESS THAN 4' 0" REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. GABLE VERTICAL PLATE SIZES STANDARD PSF STUD #2 TRUSS C, DRWG DATE REF GROUP HEM-FIR GROUP Kzt DETAIL NOTES DOUGLAS FIR-LARCH A11015050109 1/1/09 ASCE7-05-GAB11015 Ħ A: SOUTHERN PINE 11 3 2 NO SPLICE 1X4 OR 2X3 STANDARD AND GRADES: HEM 2.5X4 STUD 1.00 42 3X4 STANDARD STUD



TOENAIL "I" REINFORCEMENT ATTACHMENT DETAIL "T" REINFORCING MEMBER OR REINFORCING ENDNAIL

APPROPRIATE ITW GABLE DETAIL) TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON

WEB LENGTH INCREASE W/ "T" BRACE MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

30 FT	90 MPH	15 FT	90 MPH	30 FT	100 MPH	15 FT	100 MPH	30 FT	110 MPH	15 FT	110 MPH	30 FT	120 MPH	15 FT	120 MPH	30 FT	130 MPH	15 FT	130 MPH	30 FT	140 MPH	15 FT	140 MPH	AND MRH
2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	"T" REINF. MBR. SIZE
30 %	20 %	20 %	20 %	40 %	10 %	30 %	20 %	50 %	10 %	40 %	10 %	40 %	10 %	50 %	10 %	50 %	2 01	50 %	10 %	50 %	2 01	50 %	10 %	"T'

ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT, Kzt = 1.00
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4 MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH $1.10 \times 6' \ 7'' = 7' \ 3''$ "T" BRACE INCREASE (FROM ABOVE) = 10%
(1) 2X4 "L" BRACE LENGTH = 6' 7" = 1.10

REF

GBLLETINO109 1/1/09 LET-IN VERT

* MAX SPACING DUR. FAC. MAX TOT. LD ANY 60 PSF 24.0" DRWG DATE

ORIOTAL ENGINE