

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name:	Thacker Residence	Builder:	EWPL INC
Address:	939 SW Roanoake Terr	Permitting Office:	
City, State:	Fort White, FL	Permit Number:	
Owner:	Greg & Judy Thacker	Jurisdiction Number:	22000
Climate Zone:	North		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 36.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft²)	2068 ft²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 36.0 kBtu/hr
(or Single or Double DEFAULT)	7a. (Dble Default) 118.0 ft²		HSPF: 7.20
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 118.0 ft²	c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 216.0(p) ft	a. Electric Resistance	Cap: 50.0 gallons
b. N/A			EF: 0.92
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Frame, Wood, Adjacent	R=13.0, 260.0 ft²	(HR-Heat recovery, Solar	
b. Frame, Wood, Exterior	R=13.0, 1900.0 ft²	DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits	
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=30.0, 2068.0 ft²	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 140.0 ft		
b. N/A			

Glass/Floor Area: 0.11 Total as-built points: 27080
Total base points: 32061

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [Signature]

DATE: 10-16-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **939 SW Roanoake Terr, Fort White, FL,**

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

WATER HEATING & CODE COMPLIANCE STATUS**Residential Whole Building Performance Method A - Details**

ADDRESS: 939 SW Roanoake Terr, Fort White, FL,

PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X	Credit = Total Multiplier
3		2635.00	7905.0	50.0	0.92	3		1.00	2635.00	1.00 7905.0
				As-Built Total:						7905.0

CODE COMPLIANCE STATUS							
BASE				AS-BUILT			
Cooling Points	+	Heating Points	+ Hot Water Points = Total Points	Cooling Points	+	Heating Points	+ Hot Water Points = Total Points
12074		12082	7905 32061	7947		11228	7905 27080

PASS

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 939 SW Roanoake Terr, Fort White, FL,

PERMIT #:

BASE				AS-BUILT						
Winter Base Points: 19257.6				Winter As-Built Points: 20399.2						
Total Winter X Points	System = Multiplier	Heating Points		Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier	X Credit Multiplier	= Heating Points	
19257.6	0.6274	12082.2		(sys 1: Electric Heat Pump 36000 btuh ,EFF(7.2) Ducts:Unc(S),Unc(R),Int(AH),R6.0 20399.2 1.000 (1.069 x 1.169 x 0.93) 0.474 1.000 11228.2 20399.2 1.00 1.162 0.474 1.000 11228.2						

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 939 SW Roanoake Terr, Fort White, FL,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2068.0	12.74	4742.3	Double, Clear	E	1.5	7.0	44.0	18.79	1.03	848.8
				Double, Clear	E	5.0	7.0	54.0	18.79	1.20	1217.0
				Double, Clear	E	5.0	10.0	13.3	18.79	1.12	280.6
				Double, Clear	E	5.0	3.0	6.7	18.79	1.42	177.7
				Double, Clear	S	1.5	2.0	5.3	13.30	2.27	160.7
				Double, Clear	N	1.5	2.0	5.3	24.58	1.01	133.0
				Double, Clear	W	0.0	0.0	66.0	20.73	1.00	1368.1
				Double, Clear	W	1.5	6.0	20.0	20.73	1.02	424.3
				Double, Clear	W	5.0	5.0	12.0	20.73	1.17	290.2
				As-Built Total:				226.7	4900.3		
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	260.0	3.60	936.0	Frame, Wood, Adjacent	13.0		260.0	3.30	858.0		
Exterior	1900.0	3.70	7030.0	Frame, Wood, Exterior	13.0		1900.0	3.40	6460.0		
Base Total: 2160.0 7966.0				As-Built Total:		2160.0		7318.0			
DOOR TYPES Area X BWPM = Points				Type			Area X WPM = Points				
Adjacent	20.0	11.50	230.0	Exterior Insulated			40.0	8.40	336.0		
Exterior	112.0	12.30	1377.6	Exterior Insulated			72.0	8.40	604.8		
				Adjacent Insulated			20.0	8.00	160.0		
Base Total: 132.0 1607.6				As-Built Total:		132.0		1100.8			
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	2068.0	2.05	4239.4	Under Attic	30.0		2068.0	2.05 X 1.00	4239.4		
Base Total: 2068.0 4239.4				As-Built Total:		2068.0		4239.4			
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	216.0(p)	8.9	1922.4	Slab-On-Grade Edge Insulation	0.0		216.0(p)	18.80	4060.8		
Raised	0.0	0.00	0.0								
Base Total: 1922.4				As-Built Total:		216.0		4060.8			
INFILTRATION Area X BWPM = Points						Area X WPM = Points					
2068.0 -0.59 -1220.1						2068.0 -0.59		-1220.1			

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: 939 SW Roanoake Terr, Fort White, FL,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 28302.8				Summer As-Built Points: 26605.0						
Total Summer Points	X	System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier	X Credit Multiplier	= Cooling Points	
28302.8		0.4266	12074.0	(sys 1: Central Unit 36000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 26605	1.00	(1.09 x 1.147 x 0.91)	0.263	1.000	7946.7	
				26605.0	1.00	1.138	0.263	1.000	7946.7	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 939 SW Roanoake Terr, Fort White, FL,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area				Type/SC Overhang Ornt Len Hgt Area X SPM X SOF = Points							
.18	2068.0	20.04	7459.7	Double, Clear	E	1.5	7.0	44.0	42.06	0.94	1736.7
				Double, Clear	E	5.0	7.0	54.0	42.06	0.61	1378.7
				Double, Clear	E	5.0	10.0	13.3	42.06	0.73	406.9
				Double, Clear	E	5.0	3.0	6.7	42.06	0.41	115.8
				Double, Clear	S	1.5	2.0	5.3	35.87	0.57	108.1
				Double, Clear	N	1.5	2.0	5.3	19.20	0.76	77.5
				Double, Clear	W	0.0	0.0	66.0	38.52	1.00	2542.6
				Double, Clear	W	1.5	6.0	20.0	38.52	0.91	703.7
				Double, Clear	W	5.0	5.0	12.0	38.52	0.53	245.2
				As-Built Total:							

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.6

The higher the score, the more efficient the home.

Greg & Judy Thacker, 939 SW Roanoake Terr, Fort White, FL,

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a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 36.0 kBtu/hr
(or Single or Double DEFAULT)	7a. (Dble Default) 118.0 ft ²		HSPF: 7.20
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 118.0 ft ²	c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 216.0(p) ft	a. Electric Resistance	Cap: 50.0 gallons
b. N/A			EF: 0.92
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Frame, Wood, Adjacent	R=13.0, 260.0 ft ²	(HR-Heat recovery, Solar	
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e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=30.0, 2068.0 ft ²	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 140.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.0)

Floor Plan Including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (Including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan Including:

- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

Roof System:

- a) Truss package including:
 - 1. Truss layout and truss details signed and sealed by FI. Pro. Eng.
 - 2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout Including:
 - 1. Rafter size, species and spacing
 - 2. Attachment to wall and uplift
 - 3. Ridge beam sized and valley framing and support details
 - 4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections Including:

- a) Masonry wall
 - 1. All materials making up wall
 - 2. Block size and mortar type with size and spacing of reinforcement
 - 3. Lintel, tie-beam sizes and reinforcement
 - 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
 - 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
 - 6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
 - 7. Fire resistant construction (if required)
 - 8. Fireproofing requirements
 - 9. Shoe type of termite treatment (termicide or alternative method)
 - 10. Slab on grade
 - a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
 - 11. Indicate where pressure treated wood will be placed
 - 12. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
 - a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
 - b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
- b) Floor joist size and spacing
- c) Girder size and spacing
- d) Attachment of joist to girder
- e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- b) Ceiling fans
- c) Smoke detectors
- d) Service panel and sub-panel size and location(s)
- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms

HVAC Information

- a) Manual J sizing equipment or equivalent computation
- b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

*****Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.
2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
(386) 758-1058 (Toilet facilities shall be provided for construction workers)
4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is \$50.00
6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

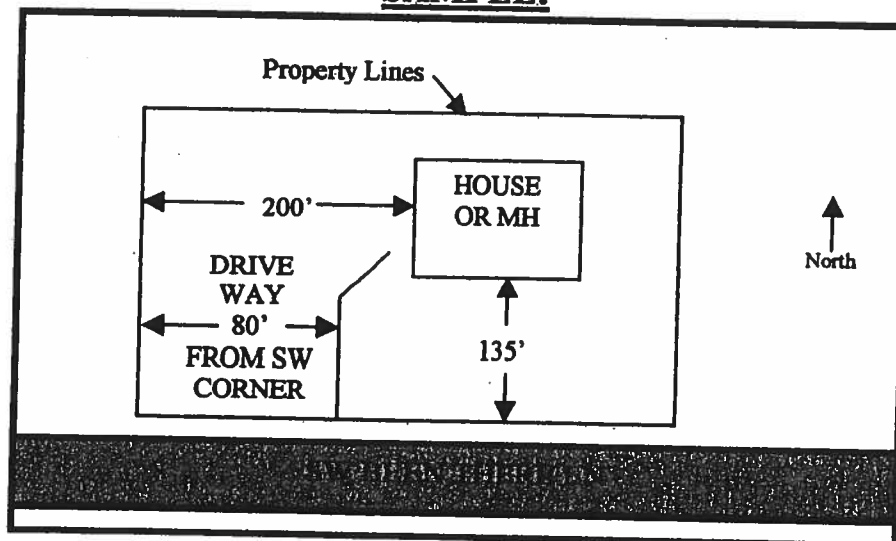
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123") FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
 - a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
 - b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
 - c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

Residential System Sizing Calculation

Summary

Greg & Judy Thacker
939 SW Roanoake Terr
Fort White, FL

Project Title:
Thacker Residence

Code Only
Professional Version
Climate: North

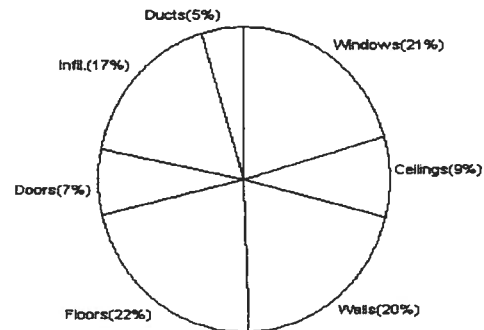
10/16/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)			
Winter design temperature	31 F	Summer design temperature	93 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	39 F	Summer temperature difference	18 F
Total heating load calculation	31144 Btuh	Total cooling load calculation	30003 Btuh
Submitted heating capacity	36000 Btuh	Submitted cooling capacity	36000 Btuh
Submitted as % of calculated	115.6 %	Submitted as % of calculated	120.0 %

WINTER CALCULATIONS

Winter Heating Load (for 2068 sqft)

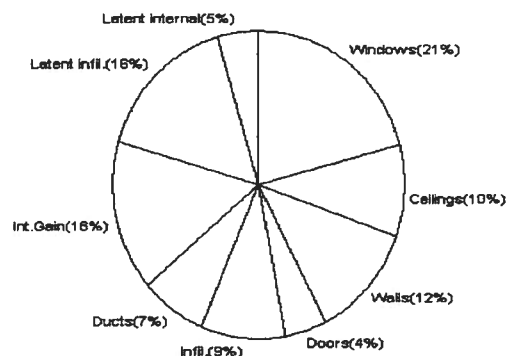
Load component		Load	
Window total	227 sqft	6415	Btuh
Wall total	2160 sqft	6306	Btuh
Door total	132 sqft	2241	Btuh
Ceiling total	2068 sqft	2688	Btuh
Floor total	216 ft	6826	Btuh
Infiltration	121 cfm	5186	Btuh
Subtotal		29661	Btuh
Duct loss		1483	Btuh
TOTAL HEAT LOSS		31144	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2068 sqft)

Load component		Load	
Window total	227 sqft	6279	Btuh
Wall total	2160 sqft	3576	Btuh
Door total	132 sqft	1338	Btuh
Ceiling total	2068 sqft	2937	Btuh
Floor total		0	Btuh
Infiltration	138 cfm	2735	Btuh
Internal gain		4800	Btuh
Subtotal(sensible)		21665	Btuh
Duct gain		2167	Btuh
Total sensible gain		23832	Btuh
Latent gain(infiltration)		4791	Btuh
Latent gain(internal)		1380	Btuh
Total latent gain		6171	Btuh
TOTAL HEAT GAIN		30003	Btuh



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY:

DATE: 10-16-06

System Sizing Calculations - Winter

Residential Load - Component Details

Greg & Judy Thacker
939 SW Roanoake Terr
Fort White, FL

Project Title:
Thacker Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

10/16/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	44.0	28.3	1245 Btuh
2	2, Clear, Metal, DEF	N	54.0	28.3	1528 Btuh
3	2, Clear, Metal, DEF	N	13.3	28.3	377 Btuh
4	2, Clear, Metal, DEF	N	6.7	28.3	189 Btuh
5	2, Clear, Metal, DEF	E	5.3	28.3	151 Btuh
6	2, Clear, Metal, DEF	W	5.3	28.3	151 Btuh
7	2, Clear, Metal, DEF	S	66.0	28.3	1868 Btuh
8	2, Clear, Metal, DEF	S	20.0	28.3	566 Btuh
9	2, Clear, Metal, DEF	S	12.0	28.3	340 Btuh
Window Total					6415 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	260	1.6	416 Btuh
2	Frame - Exterior	13.0	1900	3.1	5890 Btuh
Wall Total					6306 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		40	18.3	733 Btuh
2	Insulated - Exter		72	18.3	1320 Btuh
3	Insulated - Adjac		20	9.4	188 Btuh
Door Total					2241 Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	2068	1.3	2688 Btuh
Ceiling Total					2688 Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	216.0 ft(p)	31.6	6826 Btuh
Floor Total					6826 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.35	20680(sqft)	121	5186 Btuh
	Mechanical			0	0 Btuh
Infiltration Total					5186 Btuh

Totals for Heating	Subtotal	29661 Btuh
	Duct Loss(using duct multiplier of 0.05)	1483 Btuh
	Total Btuh Loss	31144 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Greg & Judy Thacker
939 SW Roanoake Terr
Fort White, FL

Project Title:
Thacker Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 18.0 F

10/16/2006

Window	Type	Panes/SHGC/U/InSh/ExSh	Ornt	Overhang		Window Area(sqft)			HTM		Load	
	Len			Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded			
1	2, Clear, DEF, N, N	N		1.5	7	44.0	0.0	44.0	22	22	968	Btuh
2	2, Clear, DEF, N, N	N		5	7	54.0	0.0	54.0	22	22	1188	Btuh
3	2, Clear, DEF, N, N	N		5	10	13.3	0.0	13.3	22	22	293	Btuh
4	2, Clear, DEF, N, N	N		5	3	6.7	0.0	6.7	22	22	147	Btuh
5	2, Clear, DEF, N, N	E		1.5	2	5.3	2.3	3.0	22	72	268	Btuh
6	2, Clear, DEF, N, N	W		1.5	2	5.3	2.3	3.0	22	72	268	Btuh
7	2, Clear, DEF, N, N	S		0	0	66.0	0.0	66.0	22	37	2442	Btuh
8	2, Clear, DEF, N, N	S		1.5	6	20.0	20.0	0.0	22	37	440	Btuh
9	2, Clear, DEF, N, N	S		5	5	12.0	12.0	0.0	22	37	264	Btuh
Window Total						227					6279 Btuh	
Walls	Type			R-Value		Area			HTM		Load	
1	Frame - Adjacent			13.0		260.0			1.0		270 Btuh	
2	Frame - Exterior			13.0		1900.0			1.7		3306 Btuh	
Wall Total						2160.0					3576 Btuh	
Doors	Type					Area			HTM		Load	
1	Insulated - Exter					40.0			10.1		406 Btuh	
2	Insulated - Exter					72.0			10.1		730 Btuh	
3	Insulated - Adjac					20.0			10.1		203 Btuh	
Door Total						132.0					1338 Btuh	
Ceilings	Type/Color			R-Value		Area			HTM		Load	
1	Under Attic/Dark			30.0		2068.0			1.4		2937 Btuh	
Ceiling Total						2068.0					2937 Btuh	
Floors	Type			R-Value		Size			HTM		Load	
1	Slab-On-Grade Edge Insulation			0.0		216.0 ft(p)			0.0		0 Btuh	
Floor Total						216.0					0 Btuh	
Infiltration	Type			ACH		Volume			CFM=		Load	
	Natural			0.40		20680			138.1		2735 Btuh	
	Mechanical								0		0 Btuh	
	Infiltration Total									138		2735 Btuh

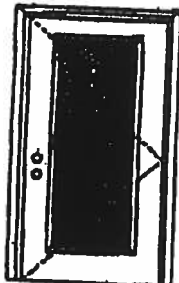
Internal gain	Occupants	Btuh/occupant	Appliance	Load
	6	X 300 +	3000	4800 Btuh

X
Glazed Inswing Unit

COP WL EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Third Party Review Certificate #2078470
per COPWL Report Validation (copy)
#2078470-001 provides additional
information - Available from the ITLWH
website (www.masonite.com), the
Masonite website (www.masonite.com)
or the literature included with this report.

Single Door
Maximum Unit Size = 3'0" x 6'8"

Design Pressure
+50.5/-50.5

(Limited water pressure special threshold design is used.)

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistance requirements for a specific building design and geographic location is determined by ASCE 7-referenced, state or local building codes specify the values required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



120, 130 Series



140 Series



160 Series



220 Series

1/2 GLASS:



100 Series*



100, 100 Series*



120 Series*



200 Series*

12 RL, 20 RL, 24 RL
Series*

167 Series*



100 Series



220 Series

*This glass kit may also be used in the following door styles: 0-panel; 2-panel with center; 2-panel 0-panel; 2-panel 0-panel with center.

Entergy
Entry Systems

June 17, 2003

Our continuing program of product improvement makes specifications, design and product
detail subject to change without notice.



Exclusively from
Masonite
Masonite International Corporation

X
Glazed Inswing Unit

COP WL FN4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES: 3/4 GLASS:



444 Series



410 Series



460 Series

FULL GLASS:



100 Series



114, 120, 140
Series



150 Series



140 Series



200 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade SCCO PA202.

Door panels constructed from 28-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom and rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE SCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L. Baithaz

State of Florida, Professional Engineer
Kurt Baithaz, P.E. - License Number 58533



This State Review Certificate #0072447C and CO-1701 Report Underlines Miami Dade Code 4470-201 (Provides additional information - available from the FBC/WH website (www.fbcwh.com). See Masonite website (www.masonite.com) or the Masonite technical portal.

Entergy
Entry Systems

June 17, 2002

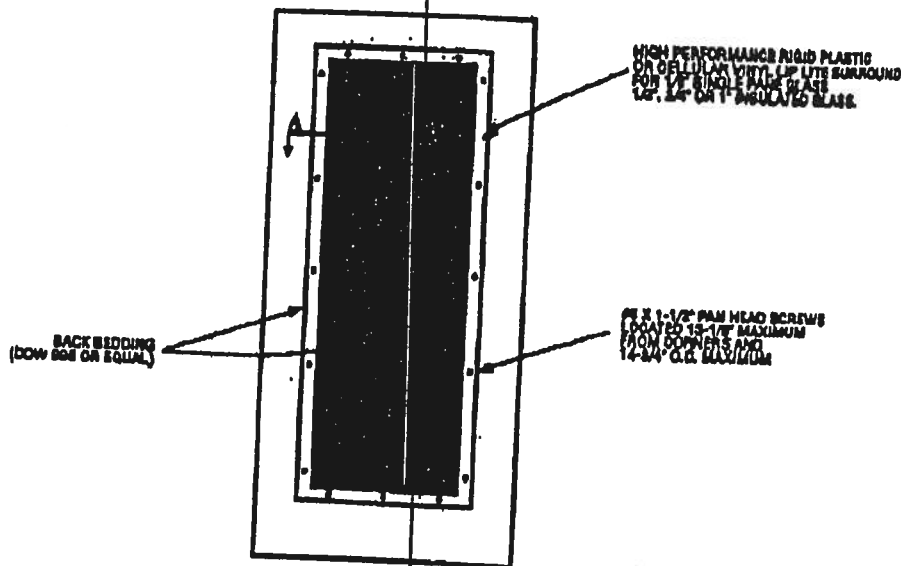
Our continuing program of product improvement studies specifications, design and product field subject to change without notice.



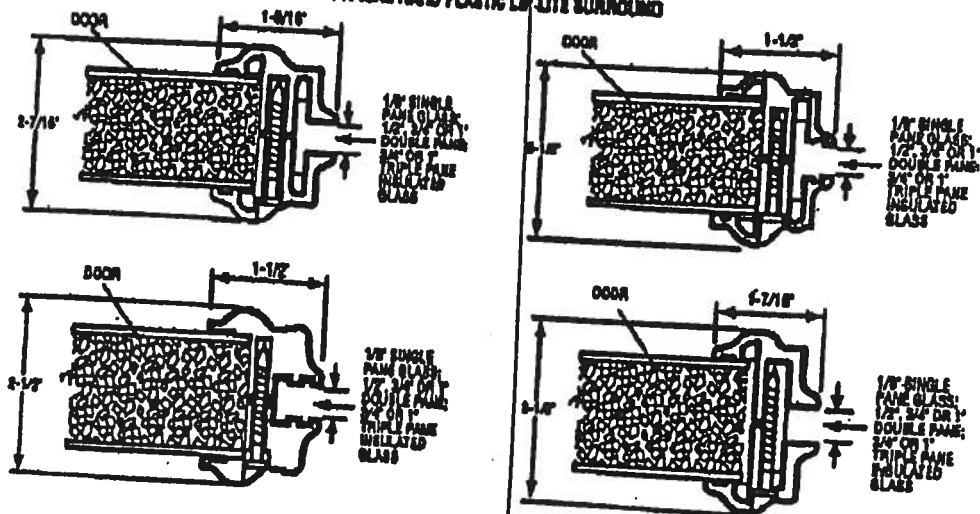
Exclusively from
Masonite
Masonite International Corporation

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A TYPICAL RIGID PLASTIC LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Service or approved validation service.

Warrick Masonry
Not Only Review Certificate #P0254475, #P0254476, #P0254477 and CDF/WHI Masonry
Reports #P0254475-C01, C02, C03; #P0254476-C01, C02, C03; #P0254477-C01, C02, C03
Additional information - available from the ITC/WHI website (www.intertek.com), the ITC/WHI
website (www.masonry.com) or the Masonry Technical Guide.

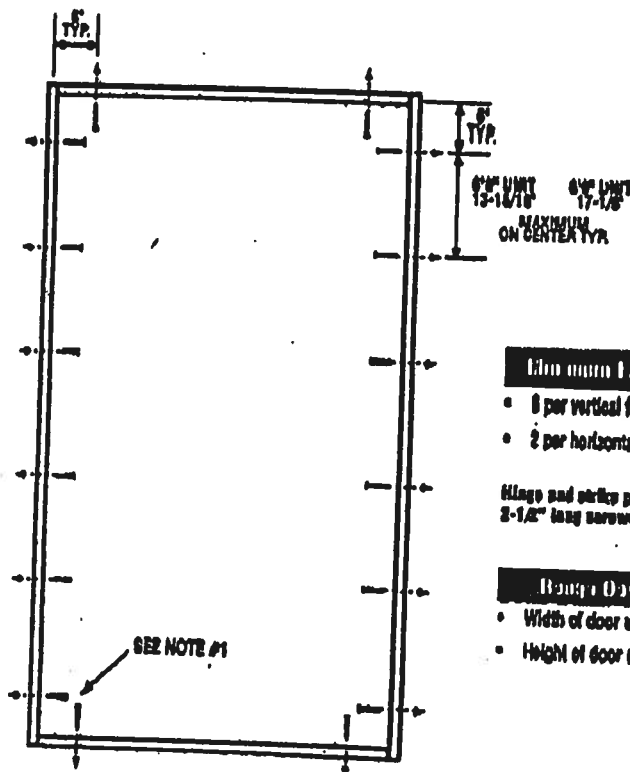
JUNE 17, 2002
Our ongoing process of product improvement creates specifications,
design and product detail changes to improve without notice.

PREQUALIFIED
Exceeding Every Detail

Manufactured by
Masonite
Masonite International Corporation

X
Unit

MID-WL-WIA0001-02

SINGLE DOOR**Minimum Fastener Count:**

- 8 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 3-1/2\"

Required Opening (RO):

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

SEE NOTE #1



This Data Review Certificate (DRC) is issued by the International Building Products Corporation (IBPC) and is valid for the products listed below. For additional information, please contact the IBPC at (800) 451-1111 or visit our website at www.ibpc.com.

Latching Hardware:

- Compliance requires that GRADE 8 or better (ANSI/BHMA A158.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT B248°, B286°, B341°, B348, B361° or B388**
Compliance requires that 6\"

*Based on required Design Pressure - see COP sheet for details.

Notes:

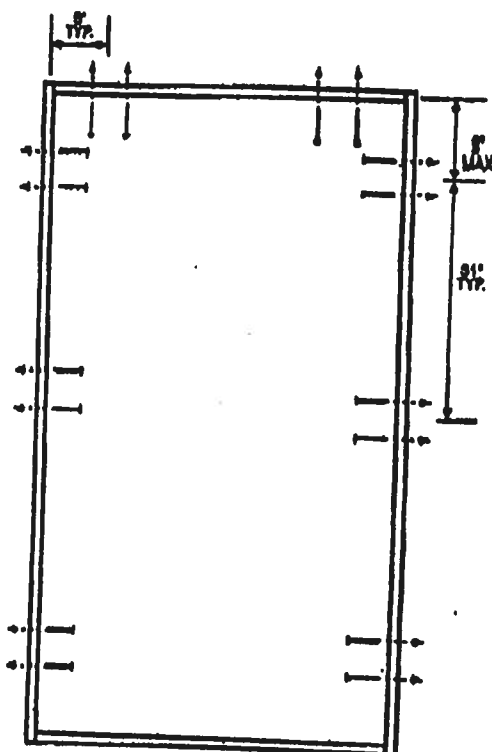
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16\"
2. The wood screw single shear design values come from Table 11.3A of ANSI/APA PRG 900 for southern pine lumber with a side member thickness of 1-1/4\"
3. Wood bolts by others, must be anchored properly to transfer loads to the structure.

March 10, 2009
Our products are made of natural materials and are subject to change without notice.

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0\" height and smaller
- 6 per vertical framing member for heights greater than 7'0\"
- 4 per horizontal framing member

Slips and strike plates require two 2-1/2\" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2\"
- Height of door unit plus 1/4\"

W Test Data Review Certificate #30284477, #30284478, #30284479 and COP-Plus Based Validation Matrix #3028447A-301, GGL, DCR, GCL, #3028447B-301, GGL, DCR, GCL, #3028447C-301, GCL, DCR, GCL provides additional information - available from the ITA/WH website (www.ita-wh.com), the document includes <http://www.masonite.com> or the Masonite technical series.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0240\", 0285\", 3241\", 3248, 3291\" or 3298**
Compliance requires that 5\" GRADE 1 (ANSI/BHMA A156.18) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

*Based on required Design Pressure - see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AP & PA NDS for southern pine lumber with a side member thickness of 1-1/4\" and achievement of minimum embedment of 1-1/4\".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2004
For installation purposes of existing units/doors making specifications, design and printed form subject to change without notice.

 **Masonite**

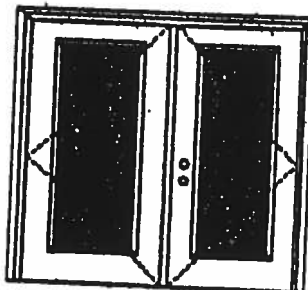
XX

Glazed Outswing Unit

COP-WL-FN4162 02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



The Door People Corporation 40705-4470
 5000 The Door People Building
 40705-4470-001 (714) 944-4470
 Information - provided from the (714) 944-4470
 website (www.masonite.com). See
 Masonite website (www.masonite.com)
 or the Masonite technical center.

Note:
 Units of other sizes are covered by this
 report as long as the panels used do not
 exceed 3'0" x 6'8".

Double Door
 Maximum unit size - 6'0" x 6'8"

Design Pressure
+50.5/-50.6

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-edition, code or local building codes specify the action required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:



120 Series



132, 134 Series



130 Series



600 Series



622 Series

1/2 GLASS:



104 Series*



106, 108 Series*



120 Series*



200 Series*



12 RL, 23 RL, 24 RL Series*



107 Series*



108 Series



304 Series

*This glass fill may also be used in the following door styles: 5-panel, 6-panel with porch, 6-panel, 6-panel, 6-panel, 6-panel with porch.

Entergy
 Entry Systems

July 17, 2003
 Our marketing program of product representation makes specification, design and product
 your subject to change without notice.



Exclusively from
Masonite
 Masonite International Corporation

XX

Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS**APPROVED DOOR STYLES:**
3/4 GLASS:

401 Series



418 Series



430 Series

FULL GLASS:

100 Series



114, 120, 122 Series



128 Series



140 Series



200 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BDDO PA202.

Door panels constructed from 24-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip like surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BDDO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L. Balph

State of Florida, Professional Engineer
Kurt Balphazer, P.E. - License Number 55533



Test Data Review Certificate #30564470
and COP/PAI Permit Violation Waiver
#202447C-001 is valid according
to inspection & analysis done by T&M
Associates (www.tandm.com), the
Miami-Dade County (www.miamidade.com)
or the Miami-Dade County

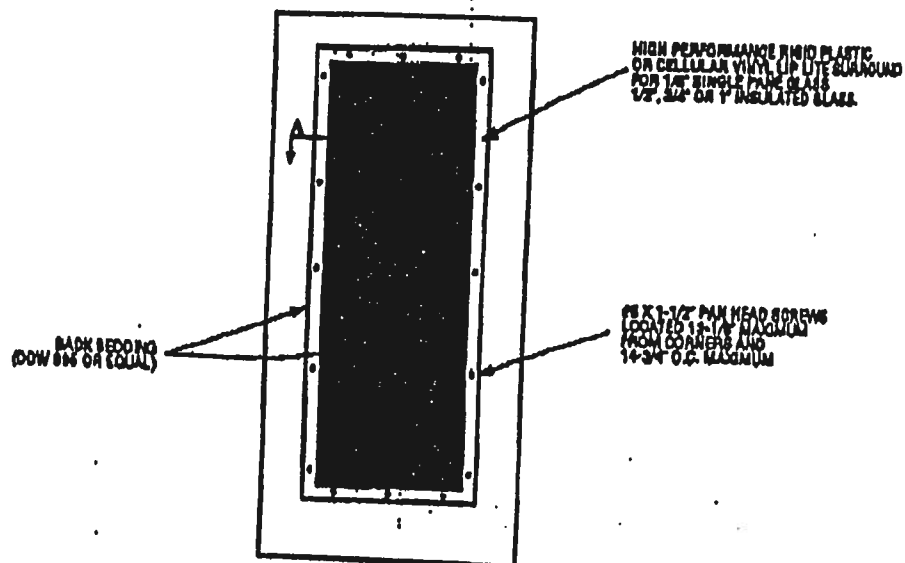
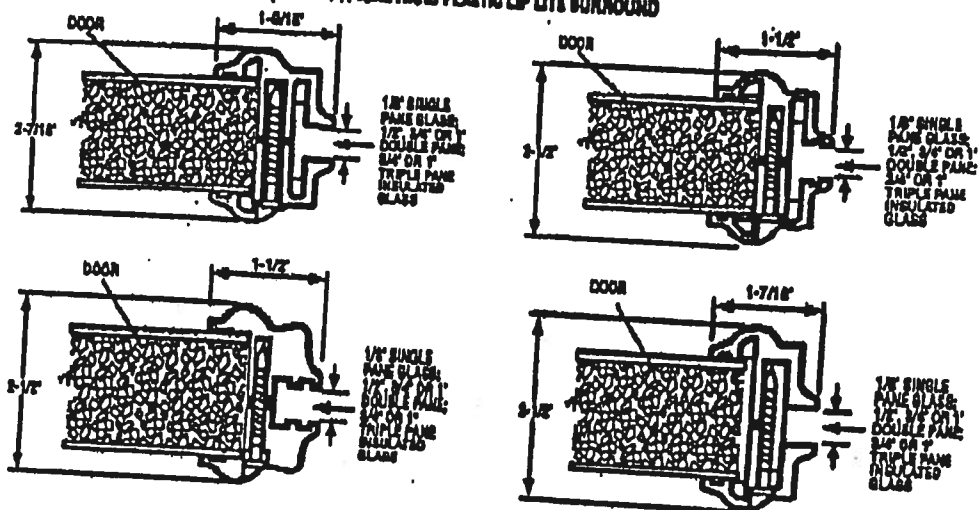
Entergy
Entry Systems

June 17, 2004
Our continuing effort of product innovation, quality, design and product
and service to Florida's utility needs.



Masonite
Masonite International Corporation

MAD-WI-MA0011-02

GLASS INSERT IN DOOR
OR SIDELITE PANELSECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND

*Glass inserts to be sub-listed by Interlink Testing Services/ETL Service or approved validation service.

Masonite Heavy Duty Code Review Certificates #5028447A; #5028447B; #5028447C and COPPING Report Validation Certificate #5028447A-901, 001, 002; #5028447B-901, 001, 002; #5028447C-901, 001, 002. Additional information - available from the IFI ETL website (www.primetech.com), the Masonite website (www.masonite.com) or the Masonite technical dealer.

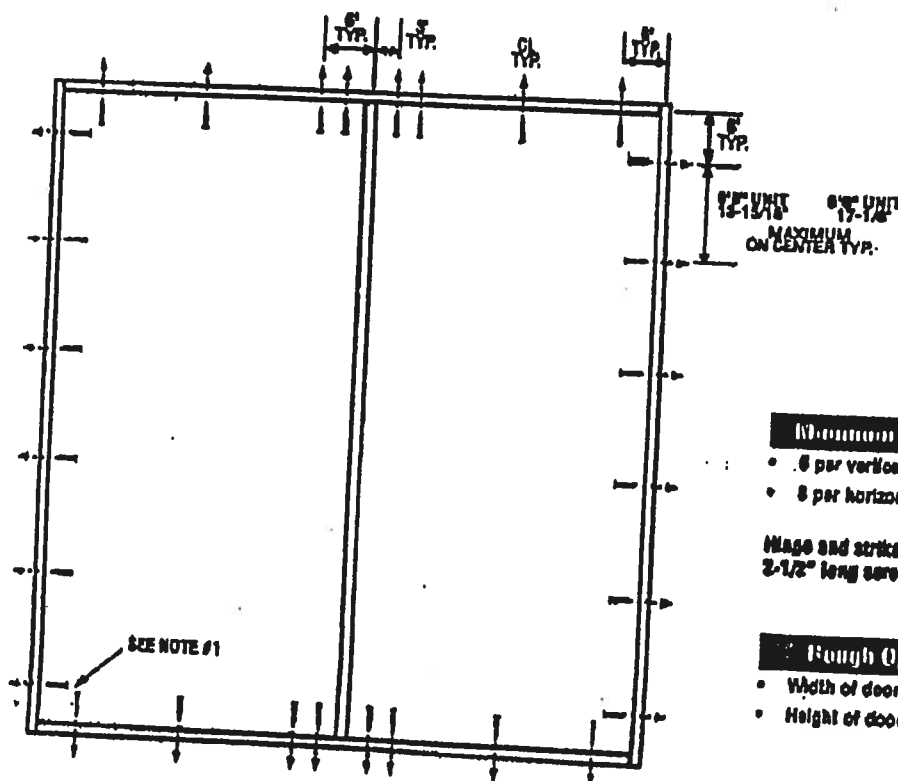
JUNE 17, 2002
For marketing purposes all product representations contain representations.
Prices and product data subject to change without notice.

PREMDORA
Premium Quality Doors

Established firm
Masonite
Masonite International Corporation

XX
Unit

MID-WL MA0002-02

DOUBLE DOOR**Minimum Fastener Count**

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Masonite Masonry VIN ONE BATTERY OPERATOR #0028447A; #0028447B; #0028447C and COPY/TOE Speed Vending Machine #0028447D-001, 002, 003, 004; #0028447E-001, 002, 003, 004; #0028447F-001, 002, 003, 004 provides additional information - available from the ITW VIN website (www.vinone.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 8247*, 8287*, 8242*, 8247, 8282* or 8287**
Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

*Based on required Design Pressure - see COP sheet for details.

Notes:

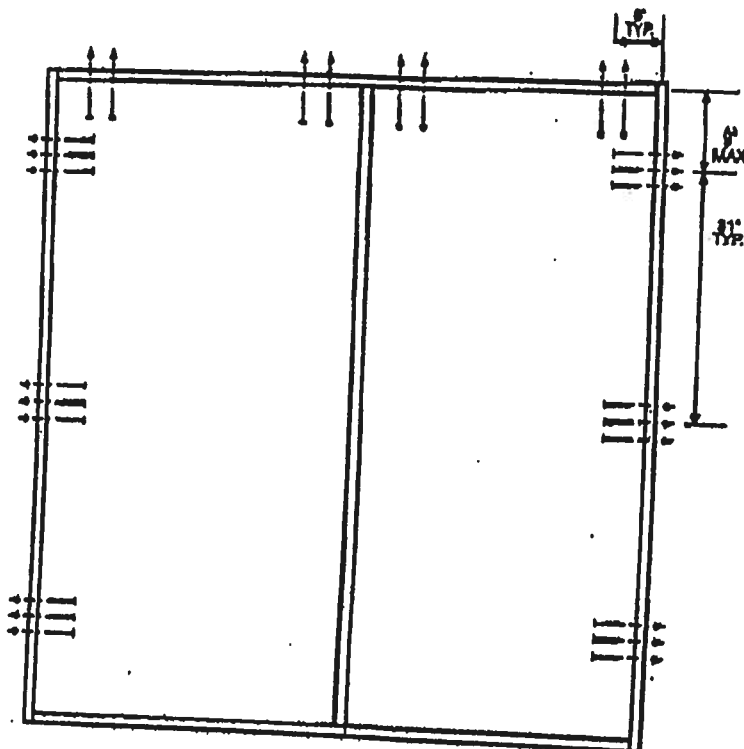
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 480 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/APA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Data Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 16, 2000
For continuing progress of product improvement making specifications, design and product detail subject to change without notice.

XX
Unit

MID WL MA0002 02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warranted Material
The Data Review Certificate #30254478, #30254479, #3025447C and COP/WHI R2004 Version Matrix #3025447A-001, 002, 003, 004; #3025447B-001, 002, 003, 004; #3025447C-001, 002, 003, 004 provides additional information - available from the ITB-001 website (www.elsevier.com), the Elsevier website (www.elsevier.com) or the Masonite Technical Center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 0247, 0257, 0242, 0247, 0252 or 0257
Compliance requires that 6" GRADE-1 (ANSI/BHMA A156.18) surface bolts be installed on latch side of active door panel - (1) at top and (1) at bottom.

*Based on required Design Pressure - see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 450 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/APA NDS for southern pine lumber with a side member thickness of 1-1/4" and a cleavage of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003
Our technical programs of product improvement and specifications, design and product development are subject to change without notice.

Masonite



MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 SINGLE HUNG (FIN & FLANGE)
165 SINGLE HUNG (FIN & FLANGE)
BB165/740/744 FIXED (FIN & FLANGE)

- Test Reports
 - 165 Single Hung
 - #CTLA-787W (Fin)
 - #CTLA-787W-1 (Flange)
 - 740/744 Single Hung
 - #01-40351.03 (Fin)
 - #01-40351.04 (Flange)
 - 165/740/744 Fixed
 - #NCTL-310-0005-2.1 (Fin)
 - # NCTL-310-0005-5.1 (Flange)
 - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

**AAMA/NWWDA 101/LS.2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

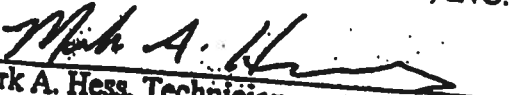
SERIES/MODEL: 740/744

TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft ²
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf
Deglazing	-70.8 psf
Forced Entry Resistance	Passed
	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.


Mark A. Hess, Technician

MAH:baw

Allen N. Reeves
15 FEBRUARY 2002



THIS FENESTRATION PRODUCT COMPLIES* WITH THE
NEW FLORIDA BUILDING CODE
FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS,
EXPOSURE "B" (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST),
AND WALL ZONE "5" (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER ASTM E1300, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE*
DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT.
THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- | | |
|---------------|--------------------|
| • 2'- 6" WIDE | DP + 40.0 / - 55.4 |
| • 3'- 0" WIDE | DP + 40.0 / - 48.5 |
| • 4'- 0" WIDE | DP + 40.0 / - 40.3 |

THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND
AIR INFILTRATION PER ATTACHED AAMA PERFORMANCE LABEL. BE ADVISED THAT
IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT
MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO
MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686



DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/IS.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/IS.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.



Allen Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance

4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"	0.29" max.
	@ 45.0 psf (negative)	0.97"	0.29" max.

* Exceeds L/175 for deflection, but meets all other test requirements.

4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

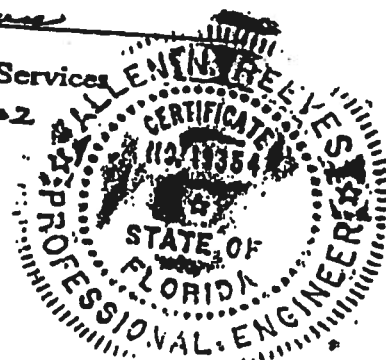
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves
Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002



Test Specimen Description: (Continued)**Drainage:** Sloped sill.**Reinforcement:** No reinforcement was utilized.**Installation:** The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.**Test Results:**

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft ²	0.30 cfm/ft ² max.
<i>Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/I.S. 2-97 for air infiltration.</i>			
2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds) @ 15.0 psf (positive) @ 15.0 psf (negative)	0.86"* 0.81"*	0.29" max. 0.29" max.
<i>Note: * Exceeds L/175 for deflection, but meets all other test requirements.</i>			
2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 22.5 psf (positive) @ 22.5 psf (negative)	0.01" <0.01"	0.20" max. 0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Top rail		
	Bottom rail	0.06"/12% 0.06"/12%	0.50"/100% 0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	
	Right stile	0.03"/6%	

Allen N. Reese
15 FEBRUARY 2002

Test Specimen Description: (Continued)

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

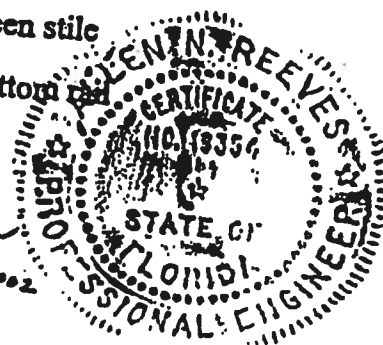
Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen H. Ream
15 FEBRUARY 2002





Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATT) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

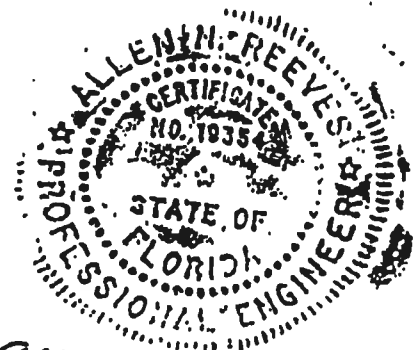
Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

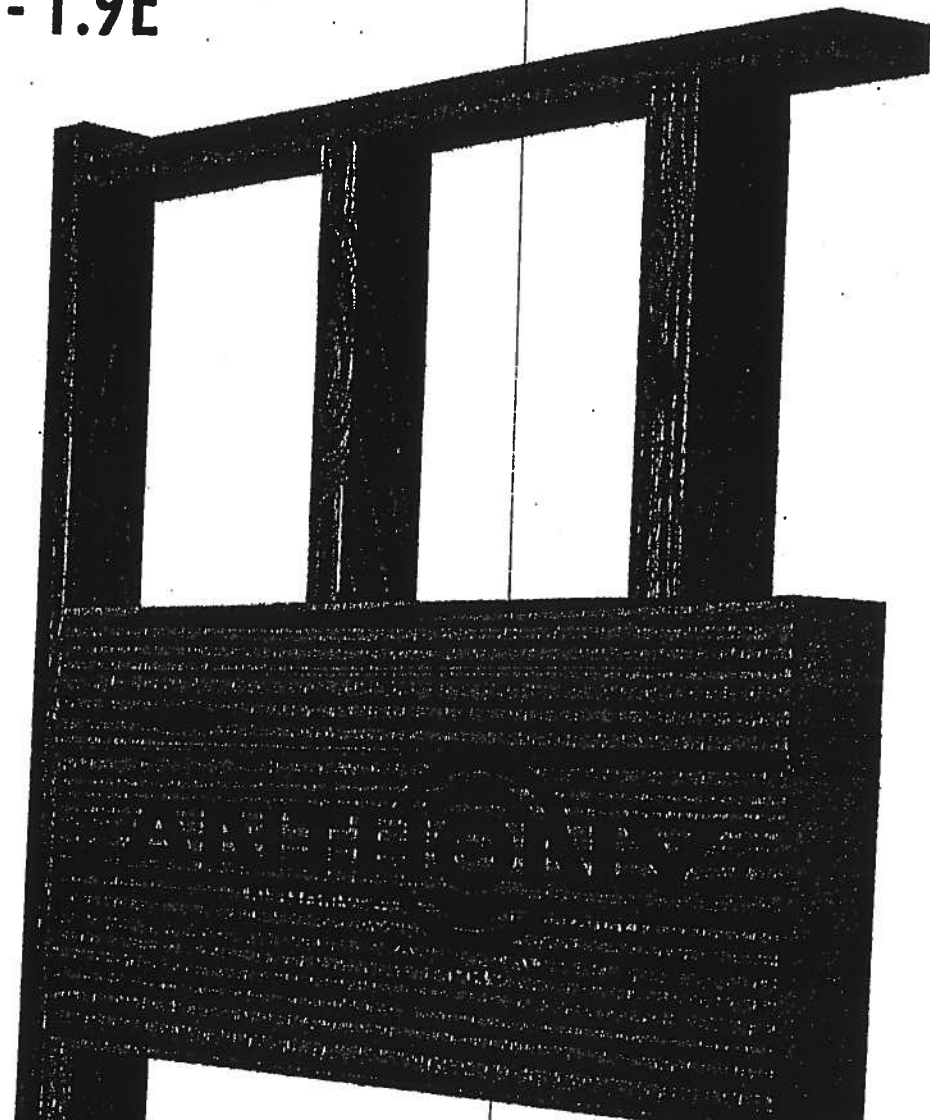
130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testatl.com



Allen N. Reeves

Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- ◆ Less Expensive than LVL or PSL
- ◆ Lighter than Steel, LVL or PSL
- ◆ Pre-Cut Lengths
- ◆ Renewable Resource
- ◆ Cambered or Non-cambered
- ◆ 3-1/2" Width to Match Framing
- ◆ One Piece - No Nail Laminating
- ◆ Lifetime Warranty

**Garage Header
Sizing Tables**

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING:

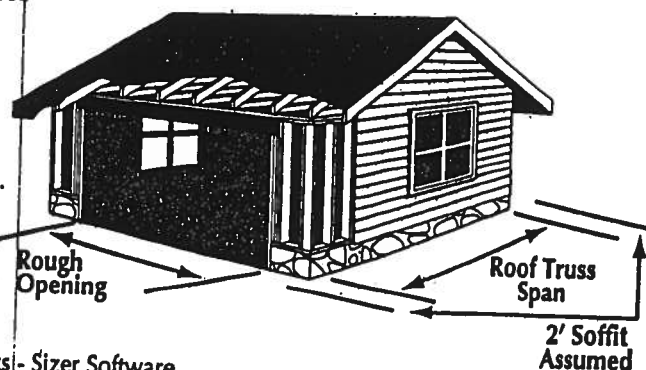
1/2 ROOF SPAN

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	9-3/4	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8		9-3/4		
8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
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8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
Non-shaded sizes require two trimmers (3" bearing).
Shaded sizes require three trimmers (4.5" bearing).
Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP 30F_b POWER BEAM® literature or AFP's WoodWorks® - Sizer Software.



Anthony POWER HEADER®

26F_b - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896	1216	1573								
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975	1322									
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony POWER HEADER®

26F_b - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) = 2600

COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) = 740

HORIZONTAL SHEAR (F_v) = 225

MODULUS OF ELASTICITY (MOE) = 1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b , shall be modified by the Volume Factor, C_v , as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for POWER HEADER®.

GARAGE HEADER COMPARISONS

810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

For more information on POWER HEADER®,
or other laminated structural products from
Anthony Forest Products Company please call
1-800-221-2326 or FAX at 870-862-6502.

POWER HEADER® is a trademark of

Anthony Forest Products Company

Post Office Box 1877 • El Dorado, Arkansas 71731

Internet address: [http:// www.anthonyforest.com](http://www.anthonyforest.com)

e-mail: info@anthonyforest.com

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- EXISTING SPARE COUNTERBALANCE
TORSION SPRING COUNTERBALANCE



16'-0" MAX WIDTH

ALLIUM CARROTUS
14 G

SECRET

**VERTICAL
TRACK, Q6 GAL**

LOWEST BRACKET APPROX. 3" FROM FLOOR, AND BRACKET NEAR THE HORIZONTAL C OF THE BOTTOM SECTION AND BRACKET NEAR THE TOP OF THE BOTTOM SECTION

BEST REPORTS ON FILE

CAUSE INDEX

STUBS 7448, EXTENDER STEEL	487" NOM GAS TESTED
STUBS 7825, EXCIBOR STEEL	487" NOM Δ
STUBS 7524, EXTERIOR STEEL	482" NOM Δ
STUBS 7670, WDM WOODS	

SAICO
GENERAL AMERICAN DOR COMPANY
5058 BASEL AVE ROAD
MONTGOMERY, 1 ALA 36108

DESIGN LOAD	+200 PSF & -200 PSF
TEST LOAD	+300 PSF & -300 PSF

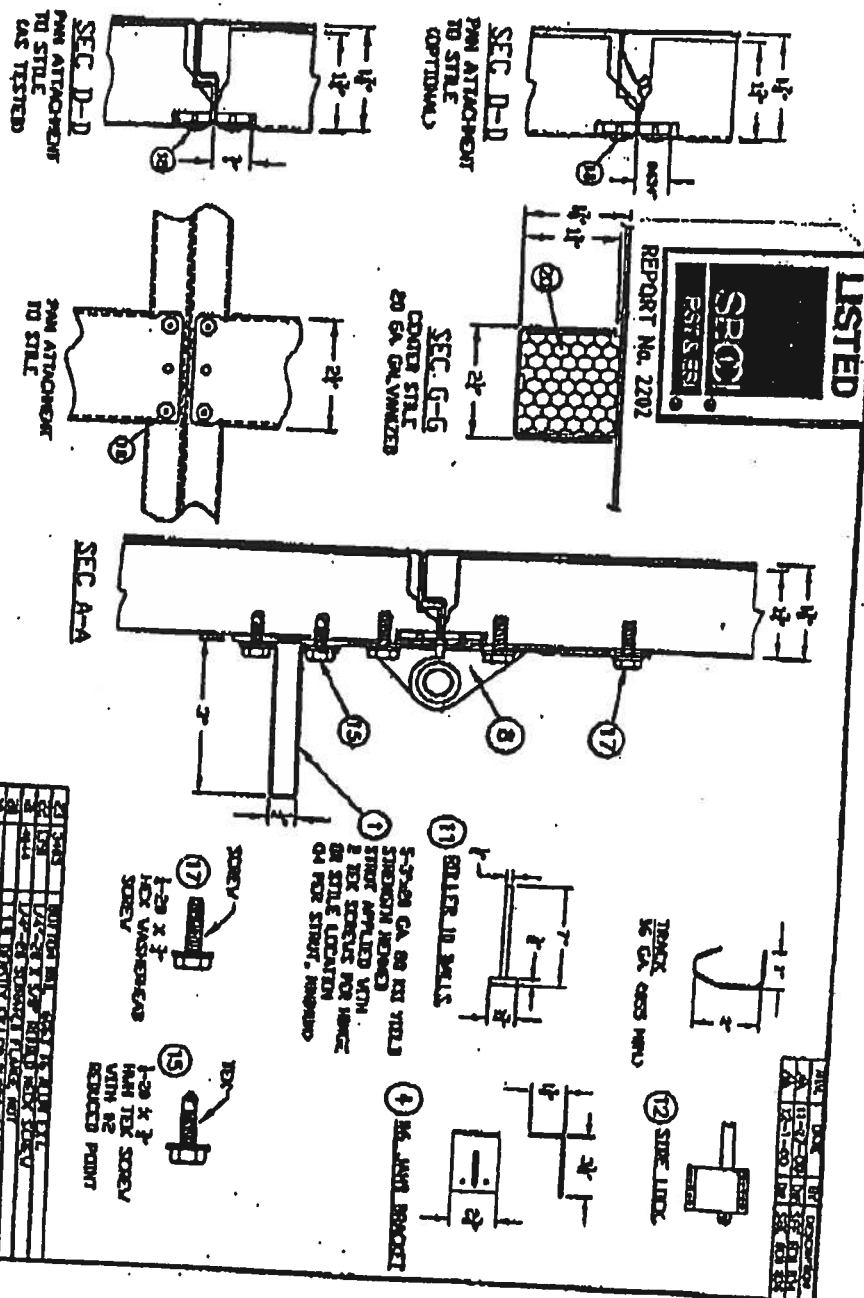
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RESTRICTED

REPORT No. 2202

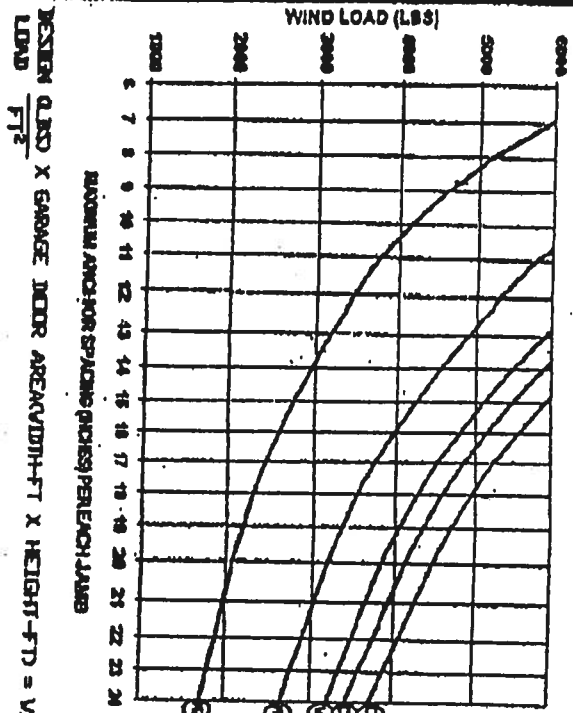
8EAL
PE No. 024280

on the drawing only
the product
the design
the construction
the material

REV	DATE	BY	LOCATION
A-A	01-10-01	DM	REC. CEN. RM

[illegible]

WIND LOAD VS ANCHOR SPACING



- ① CONCRETE MASONRY
HEAVY REINFORCED
WIND ANCHOR
1-5/8" EMBEDMENT
3/4" DIA.
- ② CONCRETE MASONRY
HEAVY REINFORCED
WIND ANCHOR
1-5/8" EMBEDMENT
3/4" DIA.
- ③ CONCRETE MASONRY
HEAVY REINFORCED
WIND ANCHOR
1-5/8" EMBEDMENT
3/4" DIA.
- ④ CONCRETE MASONRY
HEAVY REINFORCED
WIND ANCHOR
1-5/8" EMBEDMENT
3/4" DIA.

- ① ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH THE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER HARDWARE POSTS.
- ② ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SOCR STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION SSTO 10, CURRENT EDITION.
- ③ ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.
- ④ WIND FRAME BUILDING STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAYERS OF 2X6 PRESSURE TREATED SOUTHERN PINE OR GRADE OR BETTER WALL STUDS CONTINUOUS FROM FOOTING TO ROOFLINE TOP PLATE.
- ⑤ REINFORCED CONCRETE OR CONCRETE MASONRY WALLS SHALL BE ANCHORED TO SLAB OR GROUND AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS, ANCHOR SPACING AND EMBEDMENT SHALL BE AS SHOWN IN CONCRETE MASONRY UNIT (CMU) WALLS WITH ASTM C90 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI. GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
- ⑥ EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.
- ⑦ ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4".
- ⑧ LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.
- ⑨ WASHERS ARE REQUIRED ON ALL FASTENERS.
- ⑩ THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 12' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.
- ⑪ FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

EXAMPLE

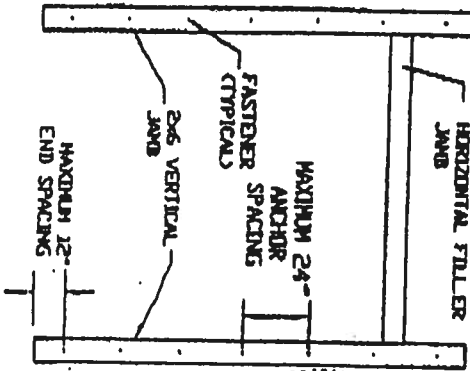
30 LBS X 66 FT WIDE X 8 FT HIGH = 3040 LBS

① USE 22" SPACING

② USE 21" SPACING

③ USE 19" SPACING

SEE NOTE IN FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



SEAL

PE NO. 024280

ENGINEER

NASER R. KEYMAN

3/6/2002

SEAL

PE NO. 024280

ENGINEER

NASER R. KEYMAN

3/6/2002

GENERAL AMERICAN DOOR COMPANY	
3000 WASHINGTON ROAD	
HARTFORD, CT 06108	
DATE: 3-21-97	REVISED BY: JLV
FOR DOOR STRUCTURE ATTACHMENT	
FOR WIND LOADING GARAGE DOORS	
AL0550	REVISION NUMBER



ELK



**PRESTIQUE®
HIGH DEFINITION®**



RAISED PROFILE™

**Prestique Plus *High Definition*
and Prestique Gallery Collection™**

Product size	13½" x 39¾"	50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.
Exposure	5½"	
Pieces/Bundle	16	
Bundles/Square	4/98.5 sq.ft.	
Squares/Pallet	11	

Raised Profile

Product size	13½" x 38¾"	30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.
Exposure	5½"	
Pieces/Bundle	22	
Bundles/Square	3/100 sq.ft.	
Squares/Pallet	16	

Prestique I *High Definition*

Product size	13½" x 39¾"	40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.
Exposure	5½"	
Pieces/Bundle	16	
Bundles/Square	4/98.5 sq.ft.	
Squares/Pallet	14	

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/FLX™

Size: 12" x 12"
Exposure: 6½"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Prestique *High Definition*

Product size	13½" x 38¾"	30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.
Exposure	5½"	
Pieces/Bundle	22	
Bundles/Square	3/100 sq.ft.	
Squares/Pallet	16	

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakeswood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood.
Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

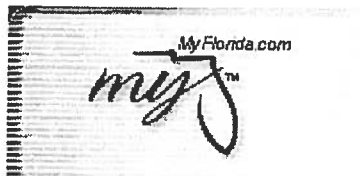
*See actual limited warranty for conditions and limitations.
**Check for product availability.

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. Fasteners

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

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10:18:45 AM 12/7/2006

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[Forgot My ID](#)[view Continuing Ed](#) [Term Glossary](#) [Online Help](#)**Licensee Details****Licensee Information**

Name: **ESCALANTE, HUGO (Primary Name)**
Main Address: **EWPL INC (DBA Name)**
P.O. BOX 280
FORT WHITE Florida 32038
County: **COLUMBIA**

License Mailing:

License Location:

License Information

License Type: **Professional Engineer**
Rank: **Cert Residential**
License Number: **CE1326967**
Status: **Current,Active**
License Date: **11/24/2003**
Expires: **11/24/2008**

Special Qualifications	Qualification Effective
Qualified Business License Required	11/24/2003

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LATERAL TOE-NAIL DETAIL

ST-TOENAIL

MiTek Industries, Chesterfield, MO Page 1 of 1

NOTES:

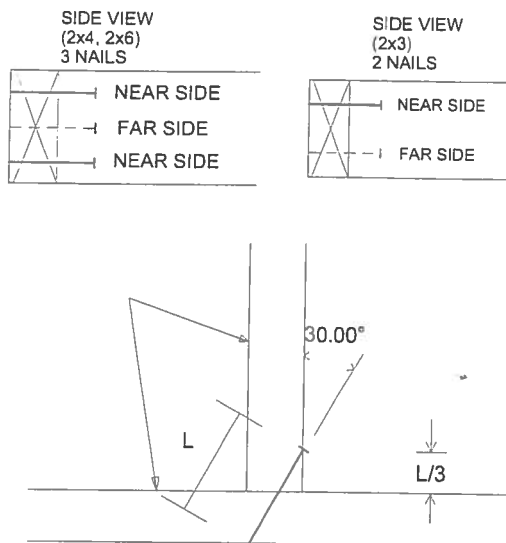
- TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
- THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

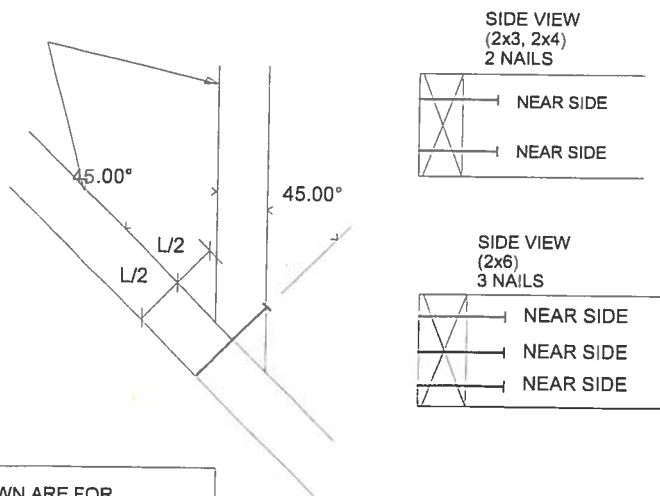
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

SQUARE CUT



45 DEGREE ANGLE BEVEL CUT

This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.

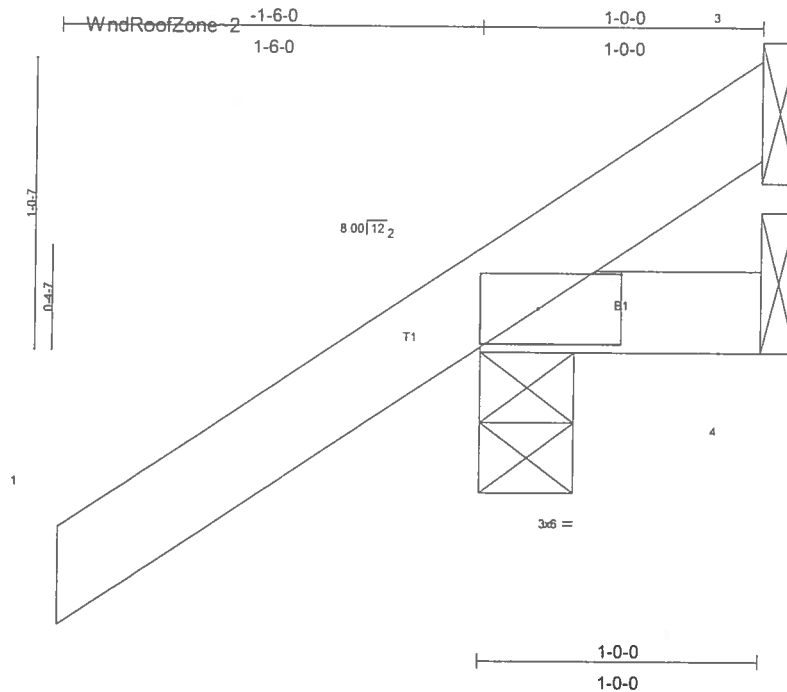


VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer.

IAN 03 2007

Job L217950	Truss CJ1	Truss Type JACK	Qty 6	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 08:54:49 2006 Page 1		



Scale = 1/8"

Plate Offsets (X,Y): [2.0-3.9,0.1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.16	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-4-0, 4=14/Mechanical, 3=-41/Mechanical
Max Horz 2=94(load case 5)
Max Uplift 2=-201(load case 5), 4=-11(load case 3), 3=-41(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=70(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-55/48
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Refer to girder(s) for truss to truss connections
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 201 lb uplift at joint 2, 11 lb uplift at joint 4 and 41 lb uplift at joint 3.

LOAD CASE(S) Standard

Job L217950	Truss CJ3	Truss Type JACK	Qty 6	Ply 1	THACKER RESIDENCE
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Builders FirstSource, Lake City, FL 32055

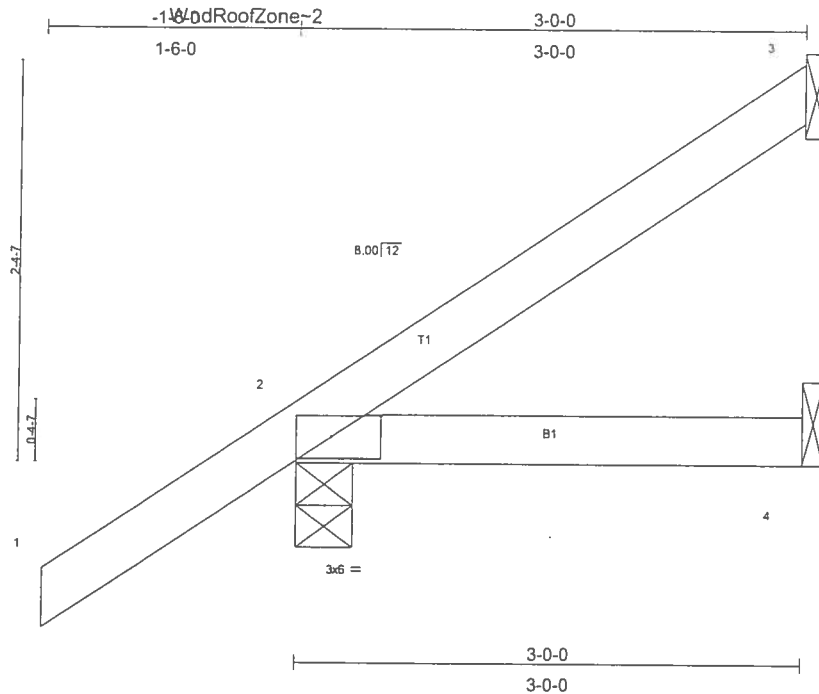
Job Reference (optional)
6.300 s Apr 19 2006 MiTek Industries, Inc. Fn Nov 17 08:54:52 2006 Page 1

Plate Offsets (X,Y): [2:0-3-9:0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.18	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TP12002								
								Weight: 13 lb	

LUMBERTOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2**BRACING**TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.**REACTIONS** (lb/size) 3=48/Mechanical, 2=233/0-4-0, 4=42/Mechanical

Max Horz 2=154(load case 5)

Max Uplift 3=47(load case 5), 2=-177(load case 5), 4=-33(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-60/19

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3, 177 lb uplift at joint 2 and 33 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L217950	Truss CJ5	Truss Type JACK	Qty 2	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 08:54:56 2006 Page 1		

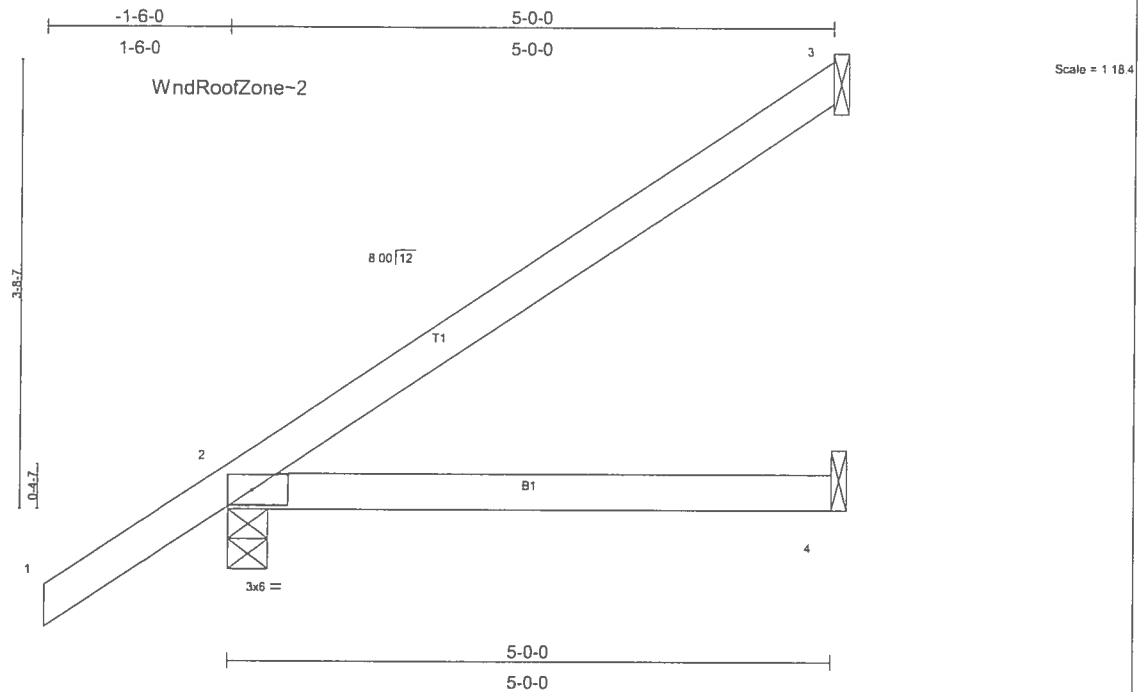


Plate Offsets (X,Y): [2-0-3-9-0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	V/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL)	0.09	2-4	>671	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.24	Vert(TL)	0.07	2-4	>784	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 19 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=306/0-4-0, 4=72/Mechanical
Max Horz 2=215(load case 5)
Max Uplift 3=-121(load case 5), 2=-198(load case 5), 4=-56(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-100/50
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 3, 198 lb uplift at joint 2 and 56 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L217950	Truss EJ4	Truss Type MONO TRUSS	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 08 55 24 2006 Page 1		

Scale = 1/4" = 1'-0"

Plate Offsets (X,Y): [2.0-3.9,0.1-8]					
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl
TCLL 20.0	2-0-0	TC 0.18	Vert(LL) 0.02	2-4	>999
TCDL 7.0	Plates Increase 1.25	BC 0.10	Vert(TL) -0.01	2-4	>999
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL) -0.00	3	n/a
BCDL 5.0	Rep Stress Incr YES	(Matrix)			
	Code FBC2004/TP1/2002				
			PLATES	GRIP	
			MT20	244/190	
			Weight: 14 lb		

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No 2	TOP CHORD Structural wood sheathing directly applied or 3-4-8 oc purlins.
BOT CHORD 2 X 4 SYP No 2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=61/Mechanical, 2=246/0-4-0, 4=47/Mechanical
 Max Horz 2=165(load case 5)
 Max Uplift 3=63(load case 5), 2=180(load case 5), 4=37(load case 3)

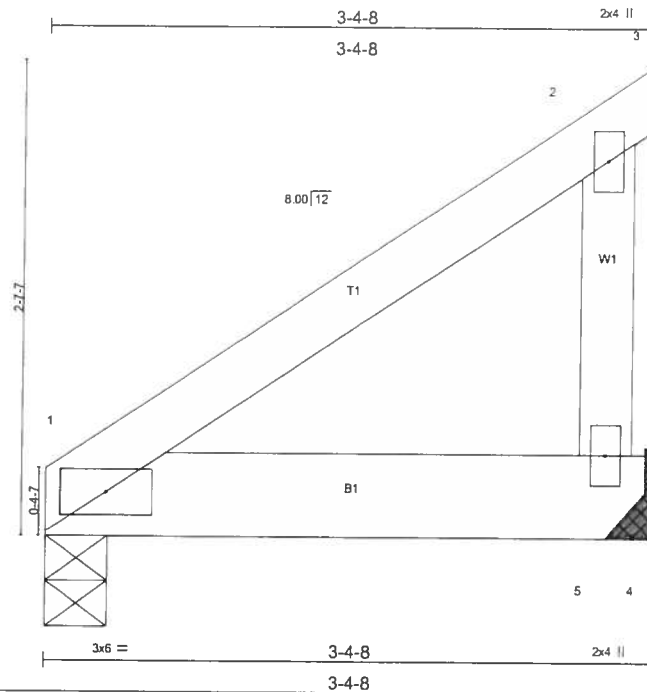
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=63/26
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX
 2 = 0.12

NOTES
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) All bearings are assumed to be SYP No 2 crushing capacity of 565.00 psi
 3) Refer to girder(s) for truss to truss connections.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3, 180 lb uplift at joint 2 and 37 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L217950	Truss EJ4G	Truss Type MONO TRUSS	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MITek Industries, Inc. Fri Nov 17 08:56:44 2006 Page 1		



Scale = 1/12.2

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.12	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.47	Vert(LL) -0.02 1-5 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.03	Vert(TL) -0.03 1-5 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.00 n/a n/a		
	Code FBC2004/TPI2002			Weight: 17 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No 2
 BOT CHORD 2 X 6 SYP No 1D
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-4-8 oc purlins
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=967/0-4-0, 5=1169/Mechanical
 Max Horz 1=101(load case 4)
 Max Uplift 1=-339(load case 4), 5=-496(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-72/36, 2-3=-2/0
 BOT CHORD 1-5=0/0, 4-5=0/0
 WEBS 2-5=-83/102

JOINT STRESS INDEX
 1 = 0.04, 2 = 0.05 and 5 = 0.06

NOTES

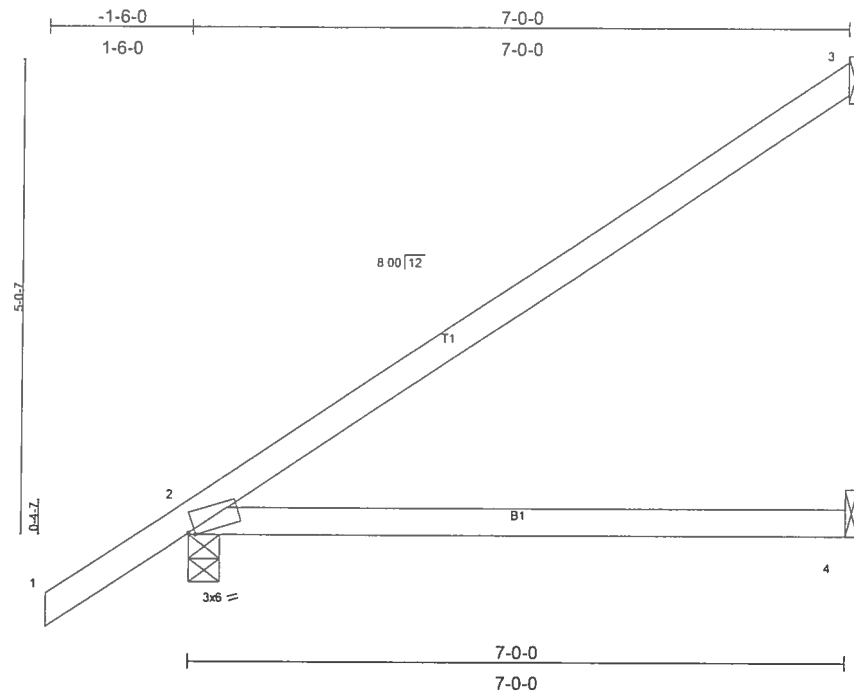
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Refer to girder(s) for truss to truss connections
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 339 lb uplift at joint 1 and 496 lb uplift at joint 5.
- 5) Girder carries tie-in span(s): 30-0-0 from 0-0-0 to 3-4-8
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54, 2-3=-14, 1-4=-615(F=-585)

Job	Truss	Truss Type	Qty	Ply	THACKER RESIDENCE
L217950	EJ7	MONO TRUSS	25	1	Job Reference (optional)

6 300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 08:57:11 2006 Page 1



Scale = 1.23 6
Camber = 1/16 in

Plate Offsets (X,Y): [2.0-0-12,Edge]									
LOADING (psf)	SPACING 2-0-0		CSI	DEFL in (loc) l/defl L/d				PLATES	GRIP
TCLL 20.0	Plates Increase 1.25		TC 0.45	Vert(LL) -0.14	2-4	>599	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25		BC 0.38	Vert(TL) -0.22	2-4	>362	180		
BCLL 10.0	Rep Stress Incr YES		WB 0.00	Horz(TL) -0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=164/Mechanical, 2=385/0-4-0, 4=109/Mechanical
Max Horz 2=277(load case 5)
Max Uplift3=-165(load case 5), 2=-142(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-126/73
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.75

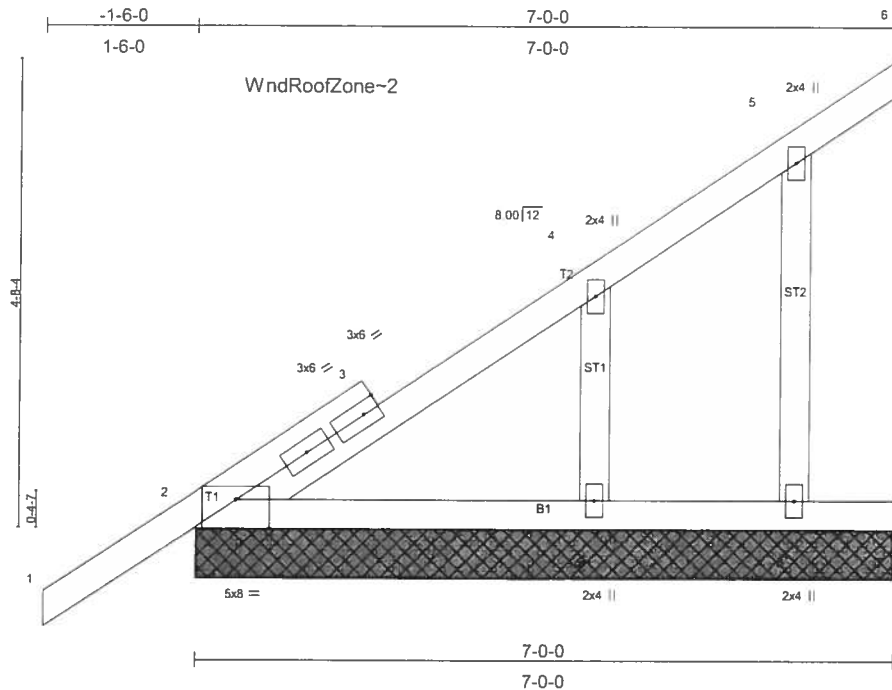
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 165 lb uplift at joint 3, 142 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 03, 2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job L217950	Truss EJ7G	Truss Type GABLE	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 08:58:16 2006 Page 1		



Scale = 1/2" = 1'-0"

Plate Offsets (X,Y): [2.0-4.0,0.3-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.18	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.09	Vert(TL)	0.00	1	n/r	90		
BCLL 10.0	Rep Stress Incr	NO	WB 0.05	Horz(TL)	-0.00	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 36 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS

(lb/size) 2=275/7'-0"-0, 6=317/7'-0"-0, 7=177/7'-0"-0, 9=346/7'-0"-0, 8=85/7'-0"-0
 Max Horz 2=263(load case 5)
 Max Uplift 2=-89(load case 5), 6=-23(load case 5), 9=-151(load case 5), 8=-77(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-185/16, 3-4=-176/72, 4-5=-73/10, 5-6=-16/14
 BOT CHORD 2-9=0/0, 8-9=0/0, 7-8=0/0
 WEBS 4-9=-227/180, 5-8=-64/91

JOINT STRESS INDEX

2 = 0.48, 3 = 0.00, 3 = 0.19, 3 = 0.19, 4 = 0.11, 5 = 0.05, 8 = 0.05 and 9 = 0.10

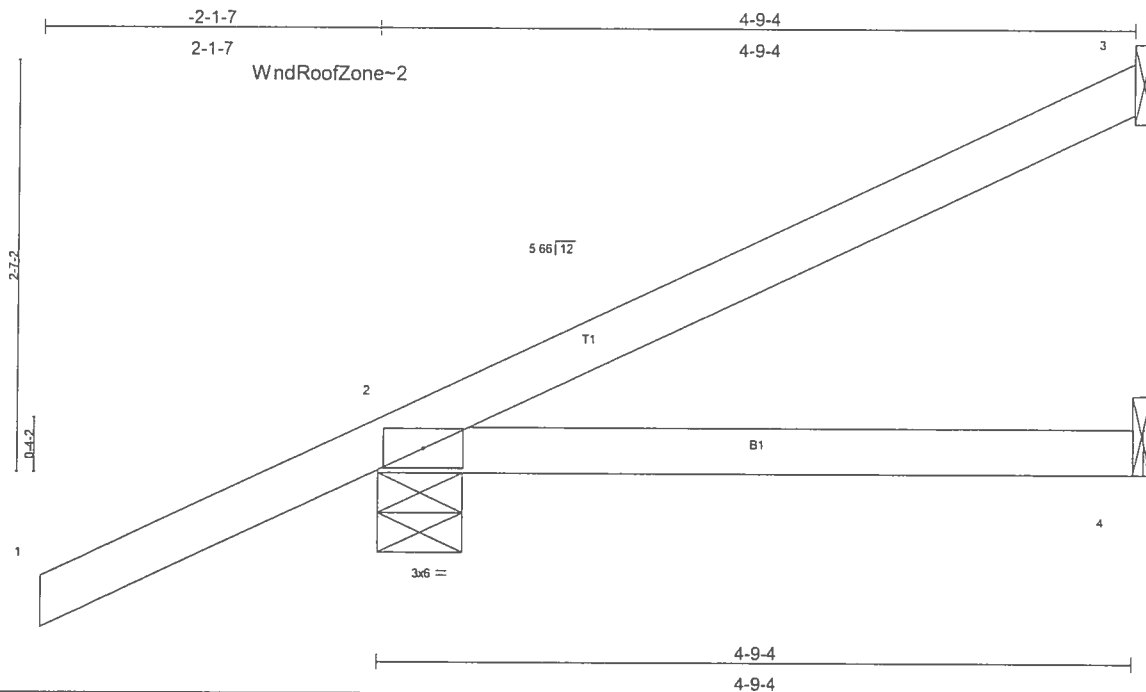
NOTES

- 1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2'-0" oc.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2, 23 lb uplift at joint 6, 151 lb uplift at joint 9 and 77 lb uplift at joint 8.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-6=-64(F=-10), 2-7=-30

Job L217950	Truss HJ4	Truss Type JACK	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09 00:37 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	Vert(LL) 0.03	2-4	>999	240		MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.09	Vert(TL) -0.02	2-4	>999	180			
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL) -0.00	3	n/a	n/a			
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=63/Mechanical, 2=243/0-6-7, 4=53/Mechanical
Max Horz 2=119(load case 4)
Max Uplift 3=42(load case 4), 2=231(load case 4), 4=45(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=40/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) All bearings are assumed to be SYP No 2 crushing capacity of 565.00 psi
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 3, 231 lb uplift at joint 2 and 45 lb uplift at joint 4.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

LOAD CASE(S) Standard

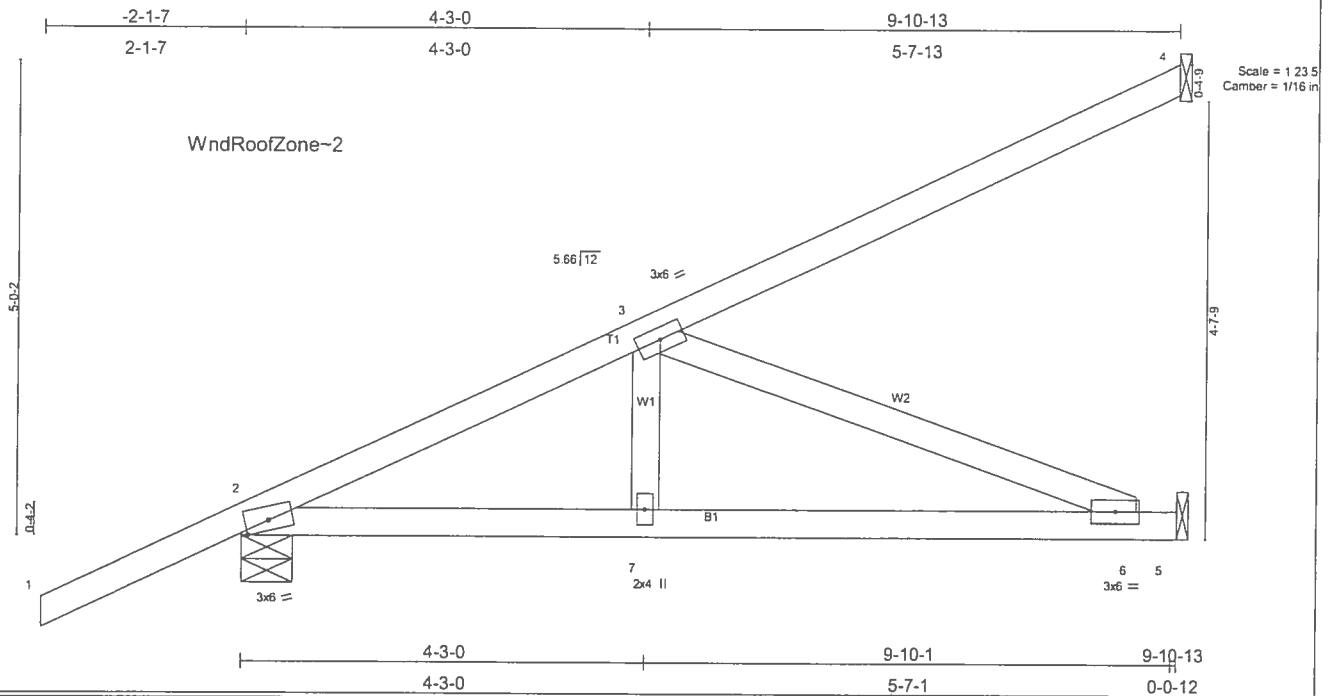
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)
Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-3=-64(F=-5, B=-5), 2=0(F=15, B=15)-to-4=-36(F=-3, B=-3)

Job L217950	Truss HJ9	Truss Type MONO TRUSS	Qty 2	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 Mittek Industries, Inc. Fri Nov 17 09:02:08 2006 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.60	Vert(LL) -0.10	6-7 >999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.58	Vert(TL) -0.17	6-7 >673	180		
BCLL 10.0	Rep Stress Incr NO	WB 0.41	Horz(TL) 0.01	5 n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)					
						Weight: 45 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-5-0 oc bracing.

REACTIONS (lb/size) 4=266/Mechanical, 2=488/0-6-6, 5=387/Mechanical
 Max Horz 2=343(load case 4)
 Max Uplift 4=-267(load case 4), 2=-306(load case 4), 5=-215(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/48, 2-3=-741/249, 3-4=-149/83
 BOT CHORD 2-7=-502/656, 6-7=-502/656, 5-6=0/0
 WEBS 3-7=-96/210, 3-6=-705/539

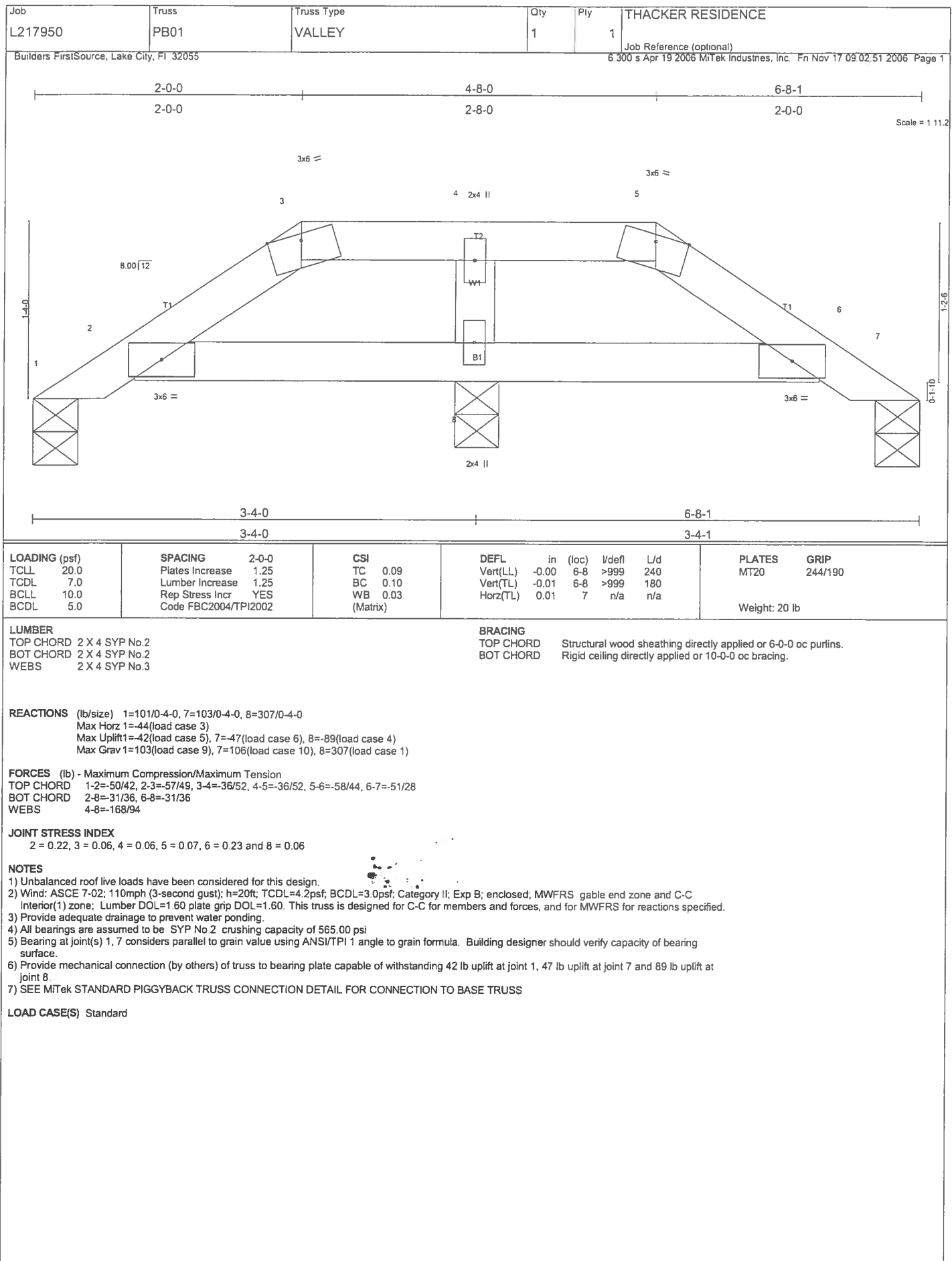
JOINT STRESS INDEX
 2 = 0.86, 3 = 0.23, 6 = 0.19 and 7 = 0.15

NOTES

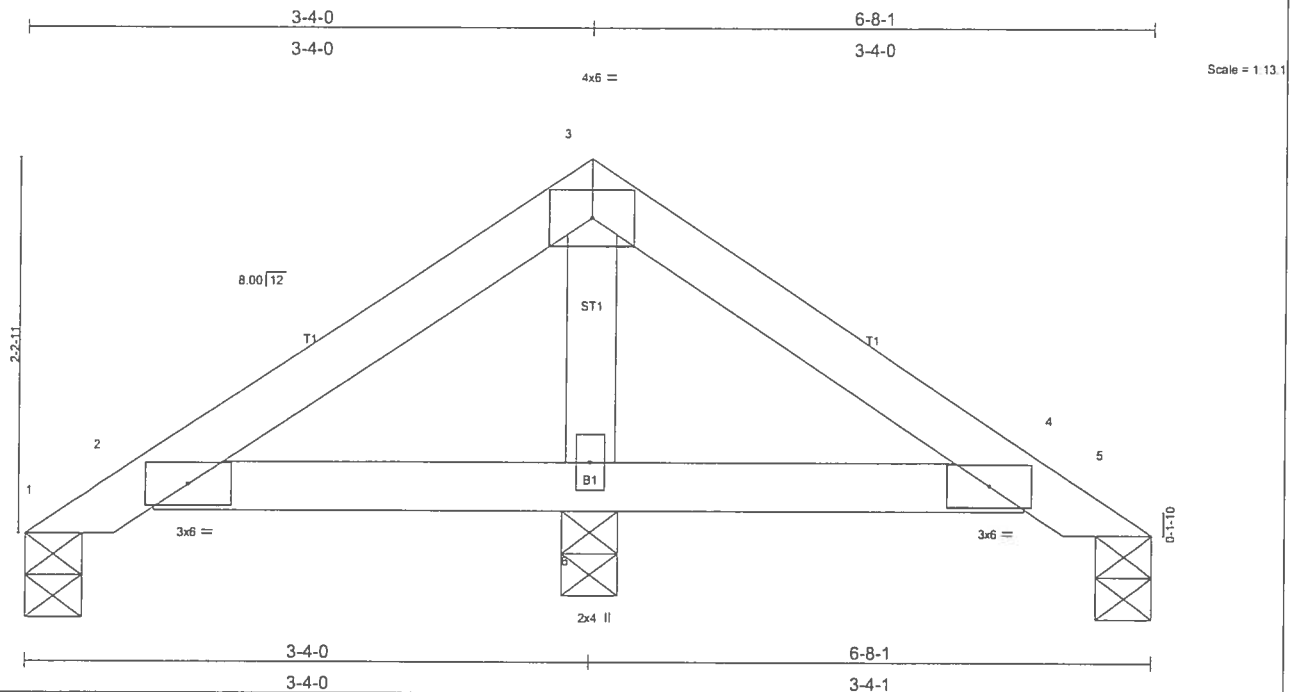
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 4, 306 lb uplift at joint 2 and 215 lb uplift at joint 5.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54
 Trapezoidal Loads (plf)
 Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)



Job L217950	Truss PB02	Truss Type VALLEY	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:03:16 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.09	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.08	Vert(LL) -0.00 4-6 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.05	Vert(TL) -0.01 2-6 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 5 n/a n/a		
	Code FBC2004/TPI2002			Weight: 22 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=43/0-4-0, 5=43/0-4-0, 6=425/0-4-0
 Max Horz 1=73(load case 4)
 Max Uplift 1=15(load case 6), 5=30(load case 3), 6=138(load case 5)
 Max Grav 1=62(load case 9), 5=62(load case 10), 6=425(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-63/68, 2-3=-53/165, 3-4=-41/165, 4-5=-29/20
 BOT CHORD 2-6=-96/79, 4-6=-96/79
 WEBS 3-6=-302/141

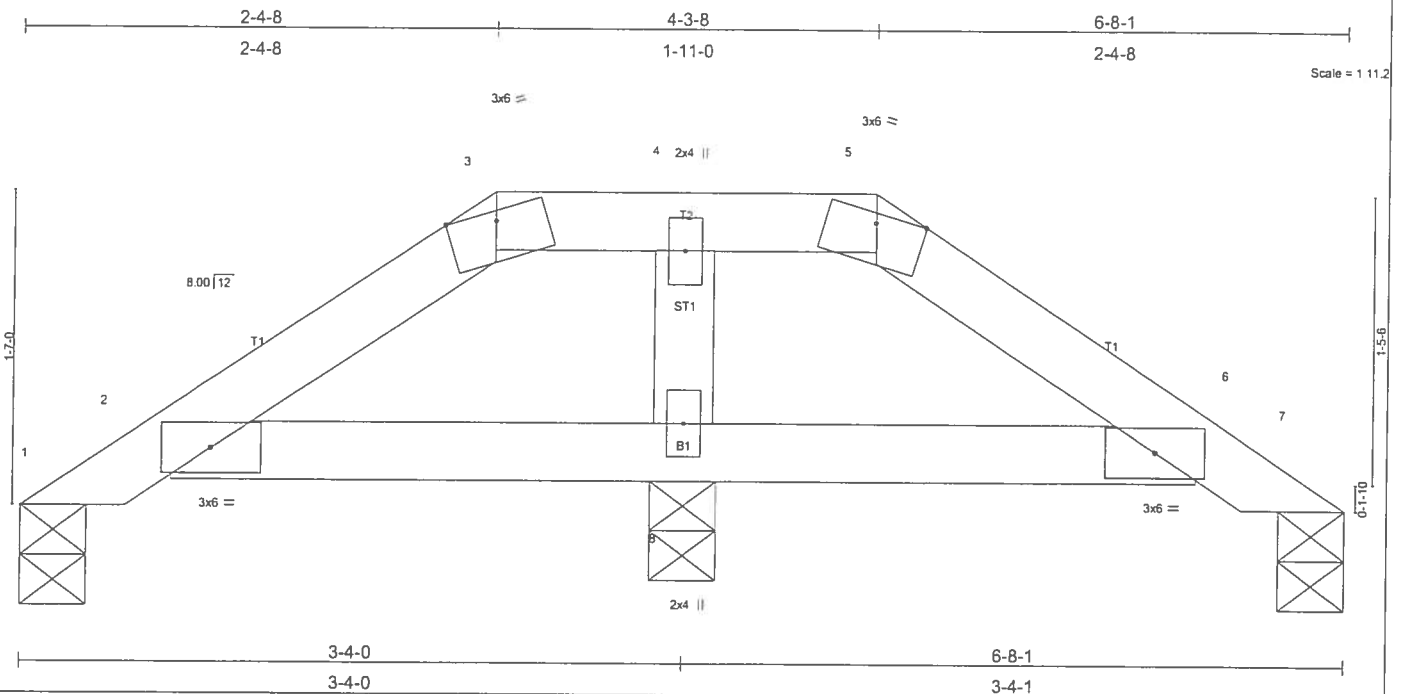
JOINT STRESS INDEX
 2 = 0.19, 3 = 0.20, 4 = 0.19 and 6 = 0.11

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 30 lb uplift at joint 5 and 138 lb uplift at joint 6.
- SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Job L217950	Truss PB03	Truss Type VALLEY	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:03:47 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.11	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.09	Vert(LL) -0.00 6-8 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.03	Vert(TL) -0.01 6-8 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 7 n/a n/a		
	Code FBC2004/TPI2002			Weight: 20 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=86/0-4-0, 7=86/0-4-0, 8=339/0-4-0
 Max Horz 1=-52(load case 3)
 Max Uplift 1=-38(load case 5), 7=-45(load case 6), 8=-81(load case 5)
 Max Grav 1=93(load case 9), 7=93(load case 10), 8=339(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-45/48, 2-3=-16/45, 3-4=0/42, 4-5=0/42, 5-6=-16/30, 6-7=-45/27
 BOT CHORD 2-8=-21/29, 6-8=-21/29
 WEBS 4-8=-200/91

JOINT STRESS INDEX
 2 = 0.23, 3 = 0.08, 4 = 0.07, 5 = 0.08, 6 = 0.23 and 8 = 0.07

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 1, 45 lb uplift at joint 7 and 81 lb uplift at joint 8.
- SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Job L217950	Truss T01	Truss Type MONO HIP	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:05:29 2006 Page 1		

-1-6-0	7-0-0	12-0-8	16-11-4	21-10-0	26-8-12	31-7-8	36-8-0	38-2-0
1-6-0	7-0-0	5-0-8	4-10-12	4-10-12	4-10-12	4-10-12	5-0-8	1-6-0
								Scale = 1/8" = 1'-0"
								Camber = 1/4" = 1'-0"

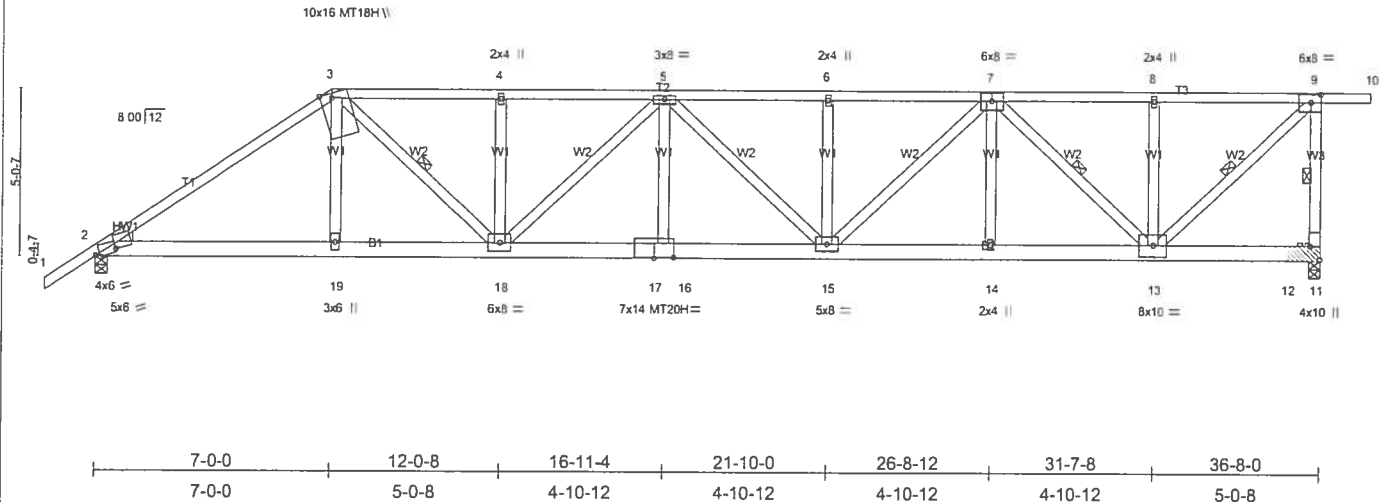


Plate Offsets (X,Y): [2-0-7-9,0-0-4], [2-0-1-8,Edge], [3-0-1-13,Edge], [9-0-3-8,0-3-0], [11-Edge,0-3-8], [17-0-7-0,0-0-4]									
LOADING (psf)		SPACING		CSI		DEFL		PLATES	
TCLL	20.0	Plates Increase	2-0-0	TC	0.90	in (loc)	l/def	L/d	GRIP
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(LL)	0.46	15-16	>939
BCLL	10.0	Rep Stress Incr	NO	WB	0.77	Vert(TL)	-0.69	15-16	>634
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Horz(TL)	0.15	11,	n/a
								Weight: 251 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.2 "Except"
 W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3
 W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-2-11 oc bracing
 WEBS 1 Row at midpt 9-11, 3-18, 7-13, 9-13

WEDGE
 Left: 2 X 4 SYP No.3

REACTIONS (lb/size) 11=3437/0-4-1 (0-4-0 + bearing block), 2=3223/0-4-0
 Max Horz 2=232(load case 3)
 Max Uplift 11=2089(load case 2), 2=1725(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-5219/2942, 3-4=-5915/3591, 4-5=-5914/3592, 5-6=-6363/3945, 6-7=-6363/3945, 7-8=-3084/1967, 8-9=-3084/1967, 9-10=0/0
 9-11=-3256/2090
 BOT CHORD 2-19=-2558/4235, 18-19=-2569/4262, 17-18=-3952/6626, 16-17=-3952/6626, 15-16=-3952/6626, 14-15=-3135/5212, 13-14=-3135/5212,
 12-13=-88/58, 11-12=-88/58
 WEBS 3-19=-373/843, 3-18=-1571/2247, 4-18=-555/678, 5-18=-983/658, 5-16=0/346, 5-15=-362/230, 6-15=-561/620, 7-15=-937/1590, 7-14=0/350
 7-13=-2939/1798, 8-13=-572/635, 9-13=-2558/4179

JOINT STRESS INDEX
 2 = 0.85, 2 = 0.80, 3 = 0.80, 4 = 0.39, 5 = 0.57, 6 = 0.35, 7 = 0.92, 8 = 0.36, 9 = 0.90, 11 = 0.68, 11 = 0.00, 12 = 0.00, 12 = 0.00, 13 = 0.90, 14 = 0.34, 15 = 0.75, 16 = 0.00, 17 = 0.78, 18 = 0.84 and
 19 = 0.27

NOTES

- 1) 2 X 6 SYP No.1D bearing block 12" long at jt. 11 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2089 lb uplift at joint 11 and 1725 lb uplift at joint 2.
- 7) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 410 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-9=-117(F=-63), 9-10=-54, 2-19=-30, 11-19=-65(F=-35)
 Concentrated Loads (lb)
 Vert: 19=-539(F)

Job L217950	Truss T02	Truss Type SPECIAL	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:06:03 2006 Page 1		

1-6-0	4-8-7	9-0-0	15-2-11	21-5-5	27-8-0	31-7-8	36-8-0	38-2-0
1-6-0	4-8-7	4-3-9	6-2-11	6-2-11	6-2-11	3-11-8	5-0-8	1-6-0
								Scale = 1/67.3 Camber = 3/16 in

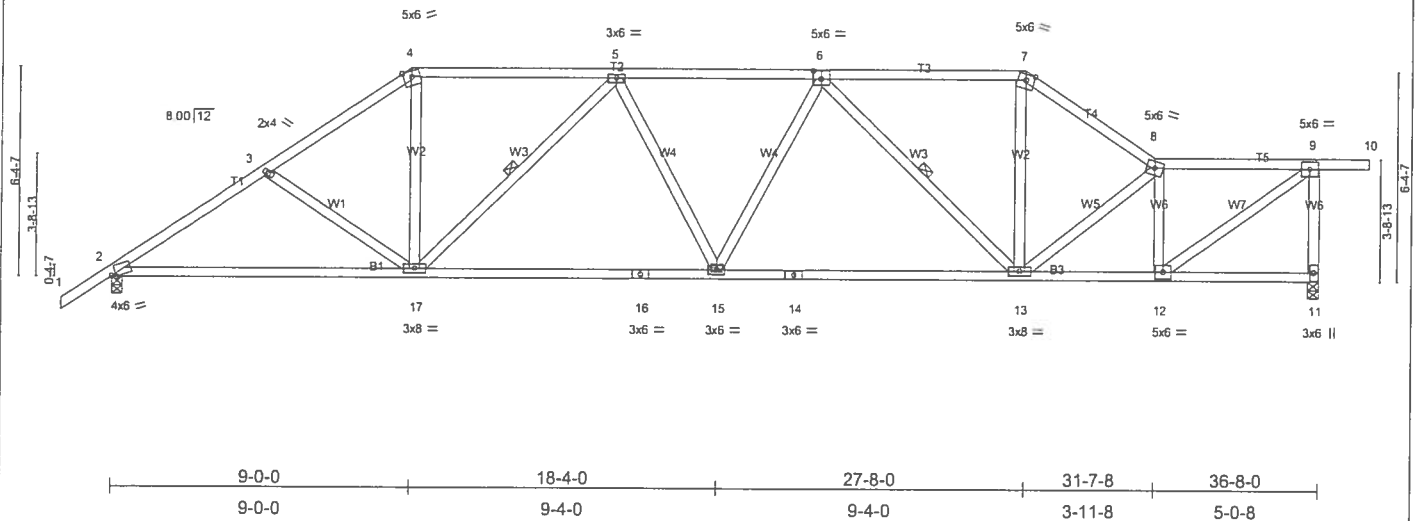


Plate Offsets (X,Y): [2-0-1-9,0-0-10], [6-0-3-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.61	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.67	Vert(LL) -0.27 13-15 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.70	Vert(TL) -0.45 13-15 >972 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.11 11 n/a n/a		
	Code FBC2004/TPI2002			Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-3 oc purlins, except end verticals
BOT CHORD Rigid ceiling directly applied or 6-4-0 oc bracing.
WEBS 1 Row at midpt 5-17, 6-13

REACTIONS (lb/size) 11=1616/0-4-0, 2=1617/0-4-0
Max Horz 2=296(load case 4)
Max Uplift 11=-646(load case 4), 2=-518(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-2320/790, 3-4=-2135/773, 4-5=-1745/694, 5-6=-2290/881, 6-7=-1739/675, 7-8=-2108/751, 8-9=-1821/629, 9-10=0/0, 9-11=-1526/651
BOT CHORD 2-17=-860/1866, 16-17=-1001/2220, 15-16=-1001/2220, 14-15=-963/2217, 13-14=-963/2217, 12-13=-631/1873, 11-12=-50/55
WEBS 3-17=-173/207, 4-17=-224/857, 5-17=-740/460, 5-15=-29/200, 6-15=-9/206, 6-13=-743/438, 7-13=-255/844, 8-13=-204/130, 8-12=-1210/442, 9-12=-704/2194

JOINT STRESS INDEX
2 = 0.87, 3 = 0.34, 4 = 0.53, 5 = 0.45, 6 = 0.55, 7 = 0.54, 8 = 0.56, 9 = 0.71, 11 = 0.32, 12 = 0.71, 13 = 0.57, 14 = 0.81, 15 = 0.45, 16 = 0.78 and 17 = 0.57

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 646 lb uplift at joint 11 and 518 lb uplift at joint 2.

LOAD CASE(S) Standard

Job L217950	Truss T03	Truss Type SPECIAL	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:06:31 2006 Page 1		

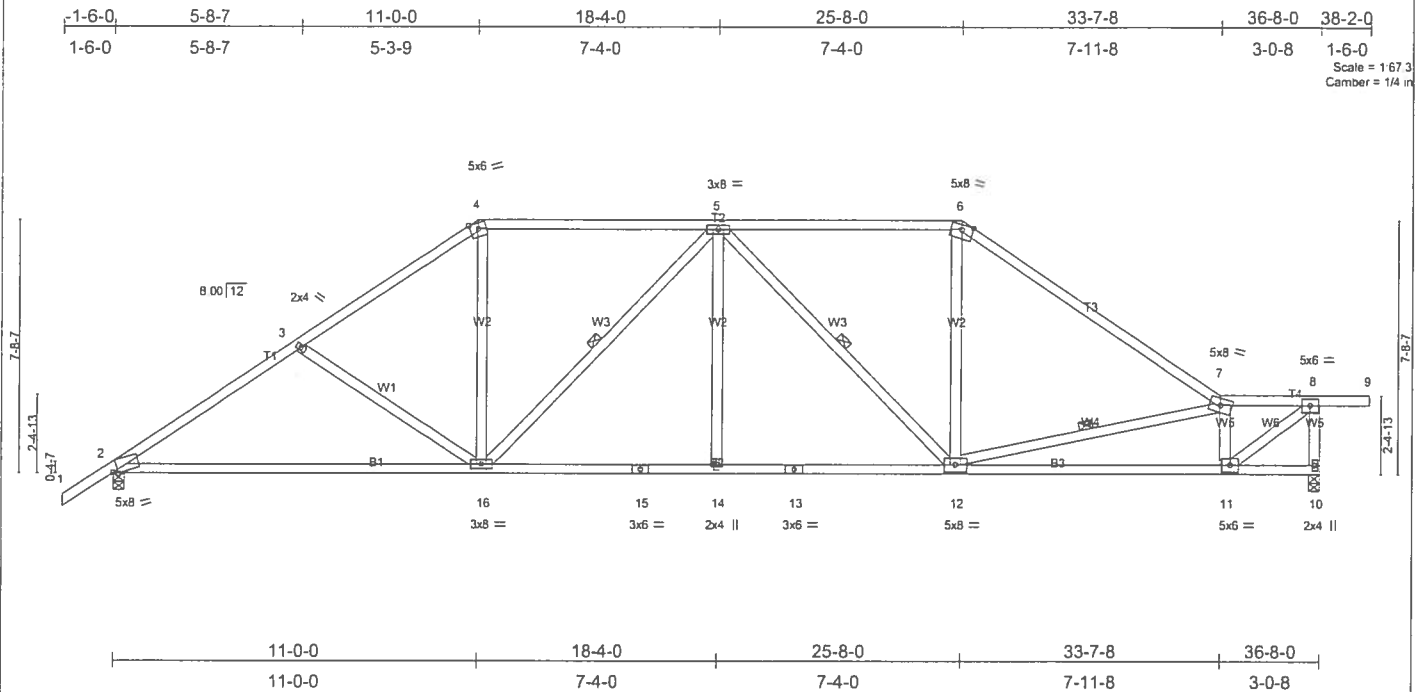


Plate Offsets (X,Y): [2:0-1:13,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.47	Vert(LL)	-0.37	2-16	>999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.84	Vert(TL)	-0.64	2-16	>683		
BCLL 10.0	Rep Stress Incr	YES	WB 0.79	Horz(TL)	0.10	10	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 213 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-6-10 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
 WEBS 1 Row at midpt 5-16, 5-12, 7-12

REACTIONS (lb/size) 10=1616/0-4-0, 2=1617/0-4-0
 Max Horz 2=318(load case 4)
 Max Uplift 10=451(load case 4), 2=536(load case 5)

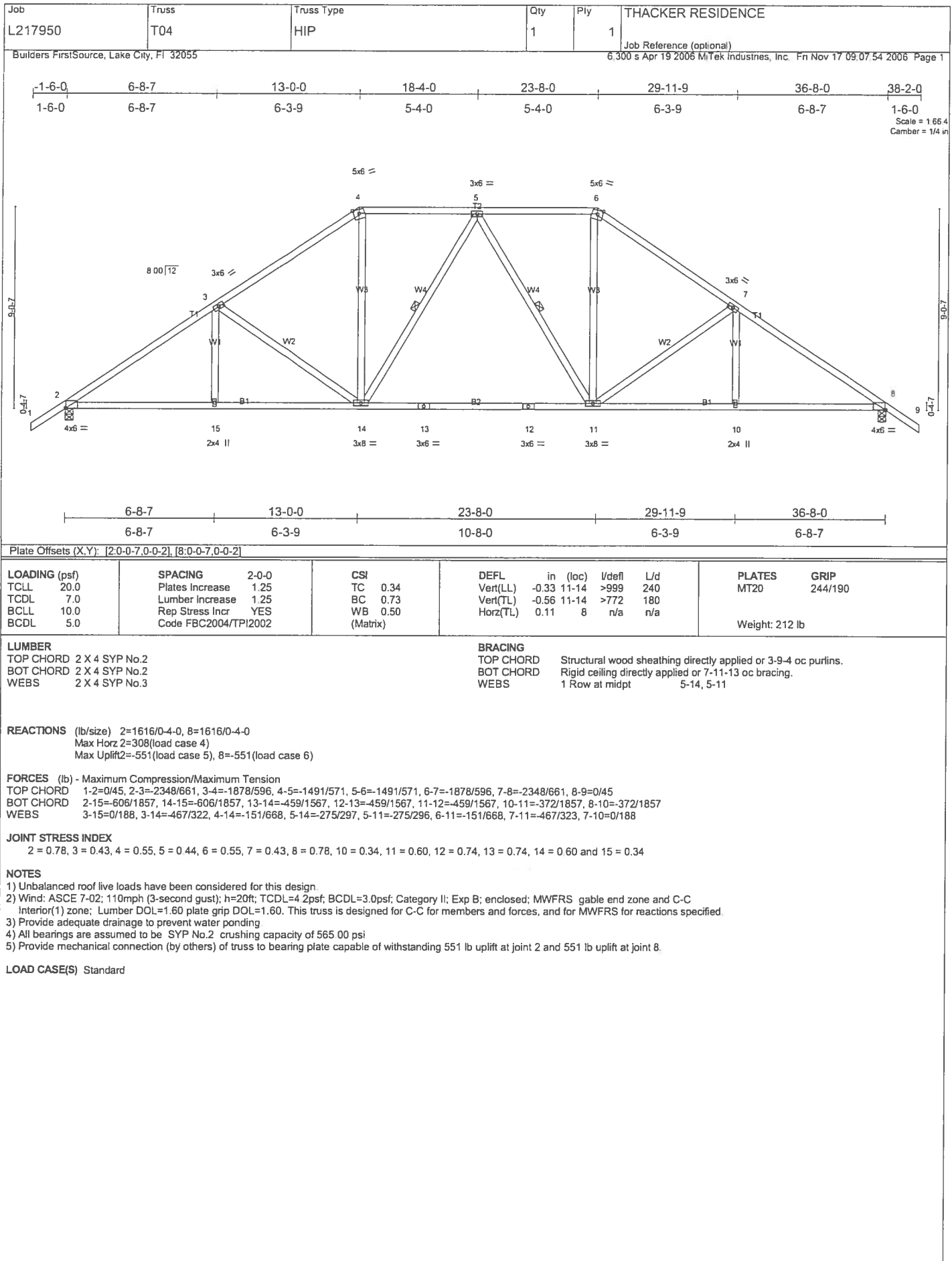
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=-2267/666, 3-4=-2023/624, 4-5=-1630/581, 5-6=-1634/599, 6-7=-2060/618, 7-8=-1944/598, 8-9=0/0, 8-10=-1603/481
 BOT CHORD 2-16=-755/1828, 15-16=-719/1903, 14-15=-719/1903, 13-14=-719/1903, 12-13=-719/1903, 11-12=-555/2035, 10-11=-35/58
 WEBS 3-16=-262/269, 4-16=-140/734, 5-16=-492/350, 5-14=0/173, 5-12=-509/354, 6-12=-164/661, 7-12=-436/234, 7-11=-1316/492, 8-11=-730/2459

JOINT STRESS INDEX
 2 = 0.69, 3 = 0.34, 4 = 0.64, 5 = 0.57, 6 = 0.87, 7 = 0.64, 8 = 0.79, 10 = 0.59, 11 = 0.79, 12 = 0.31, 13 = 0.76, 14 = 0.34, 15 = 0.69 and 16 = 0.57

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565 00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 451 lb uplift at joint 10 and 536 lb uplift at joint 2.

LOAD CASE(S) Standard



Job L217950	Truss T05	Truss Type HIP	Qty 4	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 M/Tek Industries, Inc. Fri Nov 17 09:32:30 2006 Page 1		

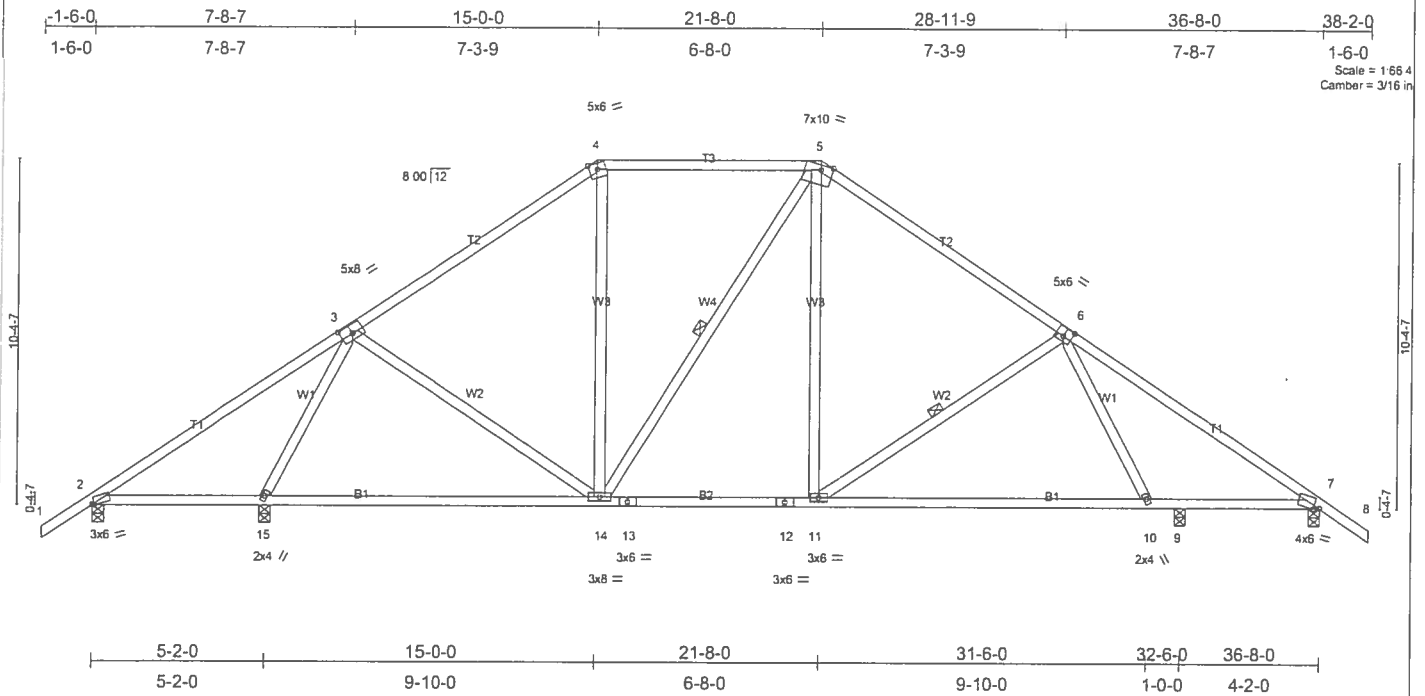


Plate Offsets (X,Y): [2.0-0-12,Edge], [3.0-4-0.0-3-0], [5.0-4-0,Edge], [6.0-3-0.0-3-4], [7.0-2-1,0-0-10]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)
TCLL 20.0	Plates Increase	1.25	TC 0.41	Vert(LL)	-0.30 10-11
TCDL 7.0	Lumber Increase	1.25	BC 0.82	Vert(TL)	-0.51 10-11
BCLL 10.0	Rep Stress Incr	YES	WB 0.90	Horz(TL)	0.04 7
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)		
				L/d	>999 240
					>647 180
					n/a n/a
Weight: 213 lb					

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-9-5 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except: 6-0-0 oc bracing: 2-15.
 WEBS 1 Row at midpt 5-14, 6-11

REACTIONS (lb/size) 2=141/0-4-0, 15=1654/0-4-0, 7=1031/0-4-0, 9=406/0-4-0
 Max Horz 2=354(load case 3)
 Max Uplift 2=201(load case 5), 15=523(load case 5), 7=343(load case 6), 9=306(load case 6)
 Max Grav 2=178(load case 9), 15=1654(load case 1), 7=1031(load case 1), 9=406(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=-174/362, 3-4=-1099/411, 4-5=-821/407, 5-6=-1238/439, 6-7=-1533/259, 7-8=0/45
 BOT CHORD 2-15=200/233, 14-15=-291/463, 13-14=-154/942, 12-13=-154/942, 11-12=-154/942, 10-11=-153/1194, 9-10=-41/1169, 7-9=-41/1169
 WEBS 3-15=-1499/540, 3-14=-109/486, 4-14=-72/232, 5-14=-322/174, 5-11=-108/440, 6-11=-325/262, 6-10=-56/278

JOINT STRESS INDEX
 2 = 0.82, 3 = 0.65, 4 = 0.74, 5 = 0.71, 6 = 0.78, 7 = 0.90, 10 = 0.34, 11 = 0.35, 12 = 0.60, 13 = 0.54, 14 = 0.59 and 15 = 0.61

NOTES

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 201 lb uplift at joint 2, 523 lb uplift at joint 15, 343 lb uplift at joint 7 and 306 lb uplift at joint 9.
- 6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Job L217950	Truss T07	Truss Type HIP	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:35:10 2006 Page 1		

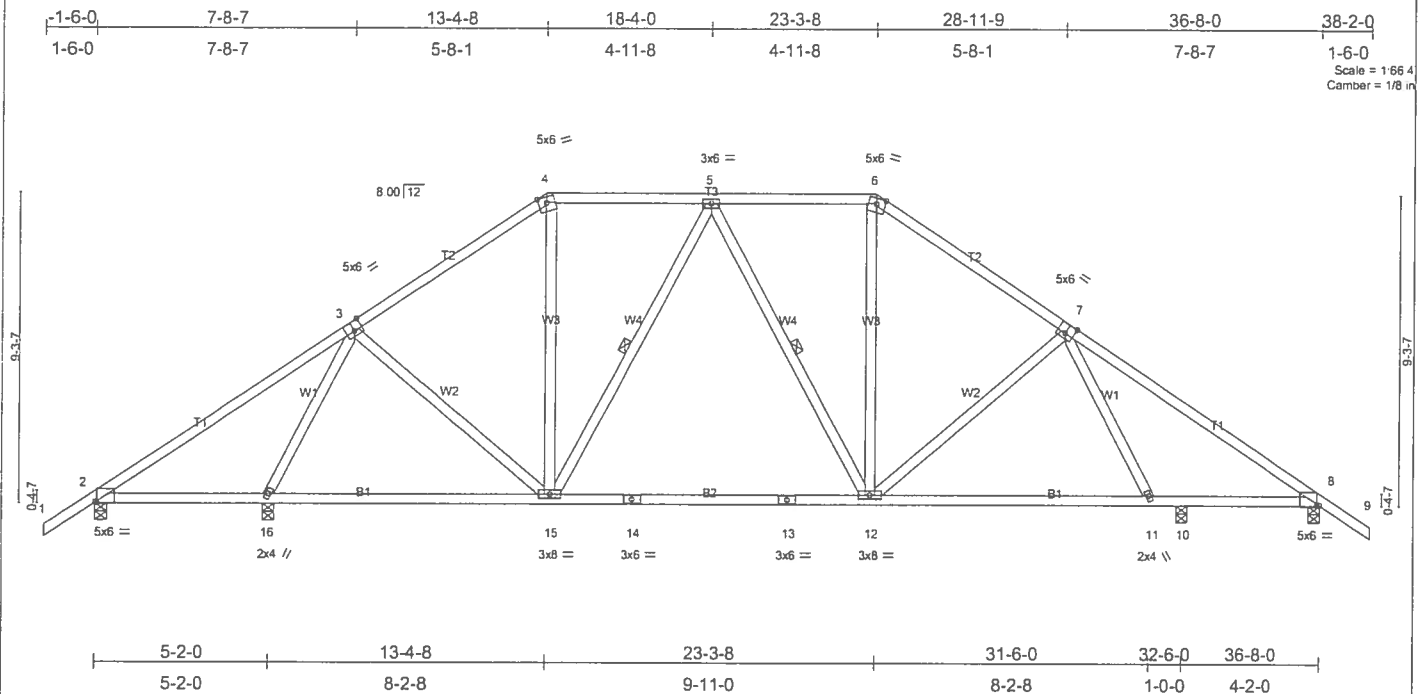


Plate Offsets (X,Y): [2:0-0-14,Edge], [3:0-3-0,0-3-4], [7:0-3-0,0-3-4], [8:0-0-14,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.43	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.77	Vert(LL) 0.06 2-16 >944 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.92	Vert(TL) -0.27 11-12 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 8 n/a n/a		
	Code FBC2004/TP12002				
				Weight: 217 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
WEBS 6-0-0 oc bracing: 2-16.
1 Row at midpt 5-15, 5-12

REACTIONS (lb/size) 2=153/0-4-0, 16=1625/0-4-0, 8=929/0-4-0, 10=524/0-4-0
Max Horz 2=317(load case 3)
Max Uplift 2=207(load case 5), 16=506(load case 5), 8=343(load case 6), 10=295(load case 6)
Max Grav 2=184(load case 9), 16=1625(load case 1), 8=929(load case 1), 10=524(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-179/381, 3-4=-1066/402, 4-5=-810/383, 5-6=-990/437, 6-7=-1278/448, 7-8=-1343/258, 8-9=0/45
BOT CHORD 2-16=-218/232, 15-16=-286/472, 14-15=-310/969, 13-14=-310/969, 12-13=-310/969, 11-12=-139/1115, 10-11=-39/1008, 8-10=-39/1008
WEBS 3-16=-1532/500, 3-15=-148/509, 4-15=-90/292, 5-15=-405/268, 6-12=-87/382, 7-12=-190/227, 7-11=-239/250, 5-12=-74/230

JOINT STRESS INDEX
2 = 0.64, 3 = 0.74, 4 = 0.39, 5 = 0.45, 6 = 0.39, 7 = 0.74, 8 = 0.64, 11 = 0.62, 12 = 0.61, 13 = 0.43, 14 = 0.43, 15 = 0.61 and 16 = 0.62

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No 2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 207 lb uplift at joint 2, 506 lb uplift at joint 16, 343 lb uplift at joint 8 and 295 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L217950	Truss T08	Truss Type SPECIAL	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:36:28 2006 Page 1		

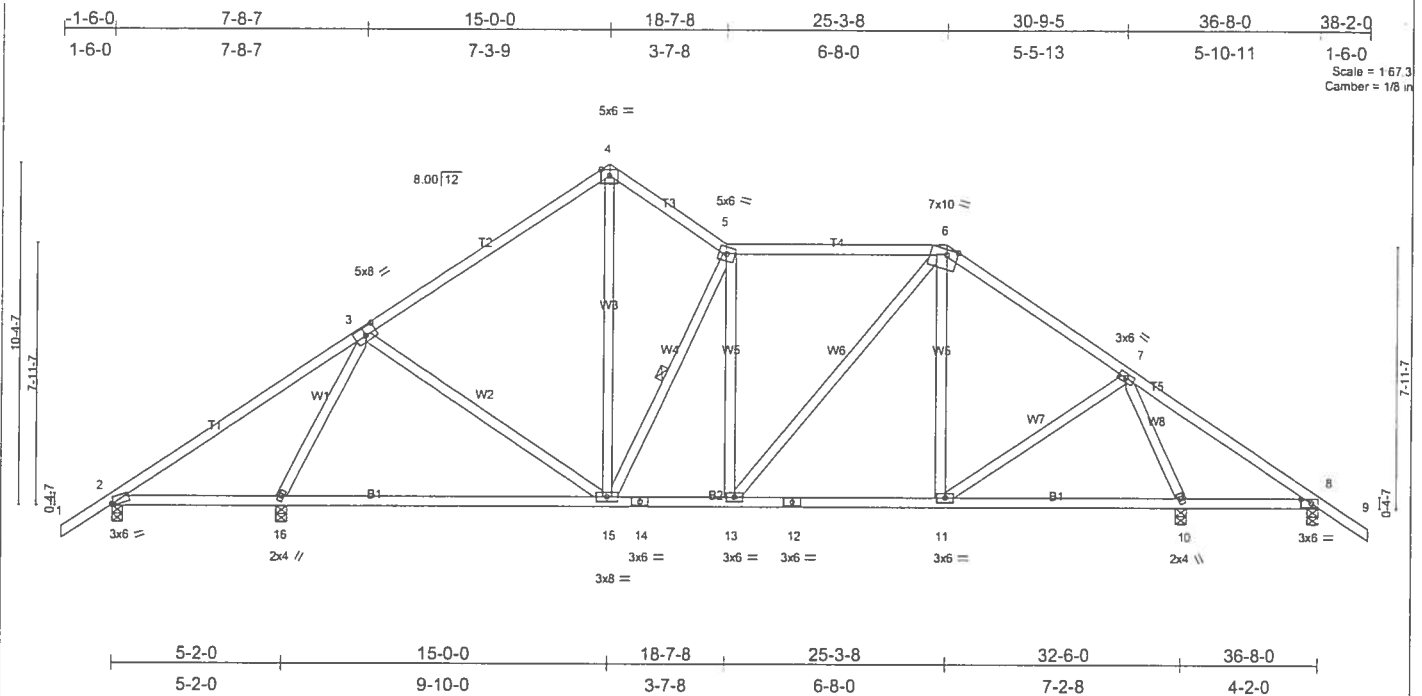


Plate Offsets (X,Y): [2:0-0-12,Edge], [3:0-4-0,0-3-0], [6:0-4-0,Edge], [8:0-3-9,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.42	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.52	Vert(LL) -0.18 15-16 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.81	Vert(TL) -0.31 15-16 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.02 10 n/a n/a		
	Code FBC2004/TPI2002			Weight: 224 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-11-1 oc purlins
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-15

REACTIONS (lb/size) 2=150/0-4-0, 16=1514/0-4-0, 10=1452/0-4-0, 8=116/0-4-0

Max Horz 2=354(load case 4)
Max Uplift 2=180(load case 5), 16=533(load case 5), 10=507(load case 6), 8=199(load case 6)
Max Grav 2=206(load case 9), 16=1514(load case 1), 10=1452(load case 1), 8=130(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-234/352, 3-4=-948/433, 4-5=-873/440, 5-6=-959/477, 6-7=-998/436, 7-8=-63/334, 8-9=0/45
BOT CHORD 2-16=-190/276, 15-16=-241/408, 14-15=-236/962, 13-14=-236/962, 12-13=-151/764, 11-12=-151/764, 10-11=-37/355, 8-10=-202/191
WEBS 3-16=-1354/549, 3-15=-53/400, 4-15=-314/620, 5-15=-660/394, 5-13=-134/109, 6-13=-128/300, 6-11=-65/147, 7-11=-168/503,
7-10=-1355/524

JOINT STRESS INDEX

2 = 0.79, 3 = 0.68, 4 = 0.49, 5 = 0.47, 6 = 0.55, 7 = 0.48, 8 = 0.48, 10 = 0.54, 11 = 0.35, 12 = 0.39, 13 = 0.40, 14 = 0.54, 15 = 0.63 and 16 = 0.55

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No 2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 2, 533 lb uplift at joint 16, 507 lb uplift at joint 10 and 199 lb uplift at joint 8.

LOAD CASE(S) Standard

Job L217950	Truss T09	Truss Type SPECIAL	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:37:01 2006 Page 1		

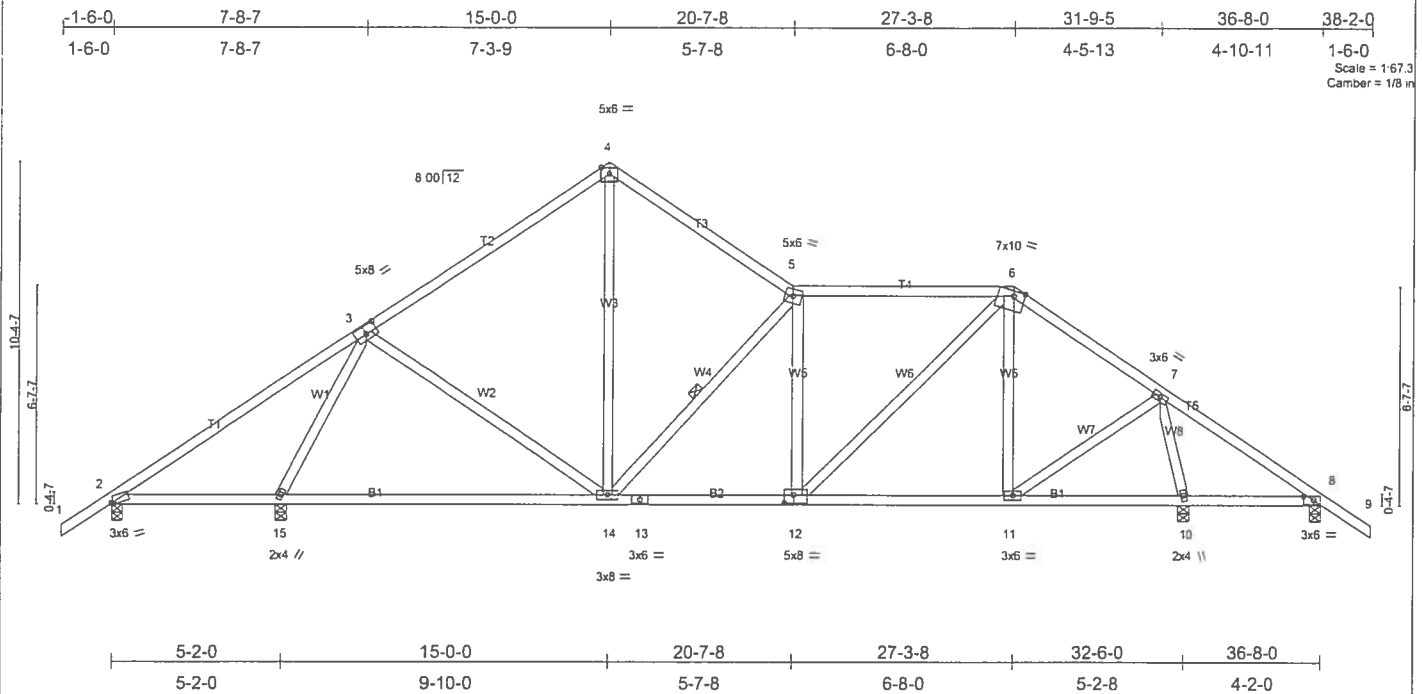


Plate Offsets (X,Y): [2.0-0.12,Edge], [3.0-4.0,0-3-0], [6.0-4.0,Edge], [8.0-3.9,0-1-8], [12.0-3-4,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.18 14-15	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.54	Vert(TL) -0.32 14-15	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.83	Horz(TL) 0.02 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)				Weight: 215 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-14

REACTIONS (lb/size) 2=117/0-4-0, 15=1545/0-4-0, 10=1496/0-4-0, 8=74/0-4-0

Max Horz 2=-354(load case 3)
Max Uplift 2=-114(load case 5), 15=-475(load case 5), 10=-473(load case 6), 8=-136(load case 6)
Max Grav 2=184(load case 9), 15=1545(load case 1), 10=1496(load case 1), 8=95(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-236/409, 3-4=-927/430, 4-5=-890/412, 5-6=-1125/516, 6-7=-892/392, 7-8=-79/390, 8-9=0/45
BOT CHORD 2-15=-238/292, 14-15=-225/375, 13-14=-224/1127, 12-13=-224/1127, 11-12=-145/687, 10-11=-7/76, 8-10=-260/190
WEBS 3-15=-1387/556, 3-14=-65/422, 4-14=-257/551, 5-14=-693/404, 5-12=-264/163, 6-12=-195/606, 6-11=-273/156, 7-11=-202/794, 7-10=-1373/509

JOINT STRESS INDEX

2 = 0.81, 3 = 0.69, 4 = 0.53, 5 = 0.71, 6 = 0.48, 7 = 0.54, 8 = 0.30, 10 = 0.51, 11 = 0.47, 12 = 0.46, 13 = 0.57, 14 = 0.57 and 15 = 0.57

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 2, 475 lb uplift at joint 15, 473 lb uplift at joint 10 and 136 lb uplift at joint 8.

LOAD CASE(S) Standard

Job L217950	Truss T10	Truss Type SPECIAL	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:37:29 2006 Page 1		

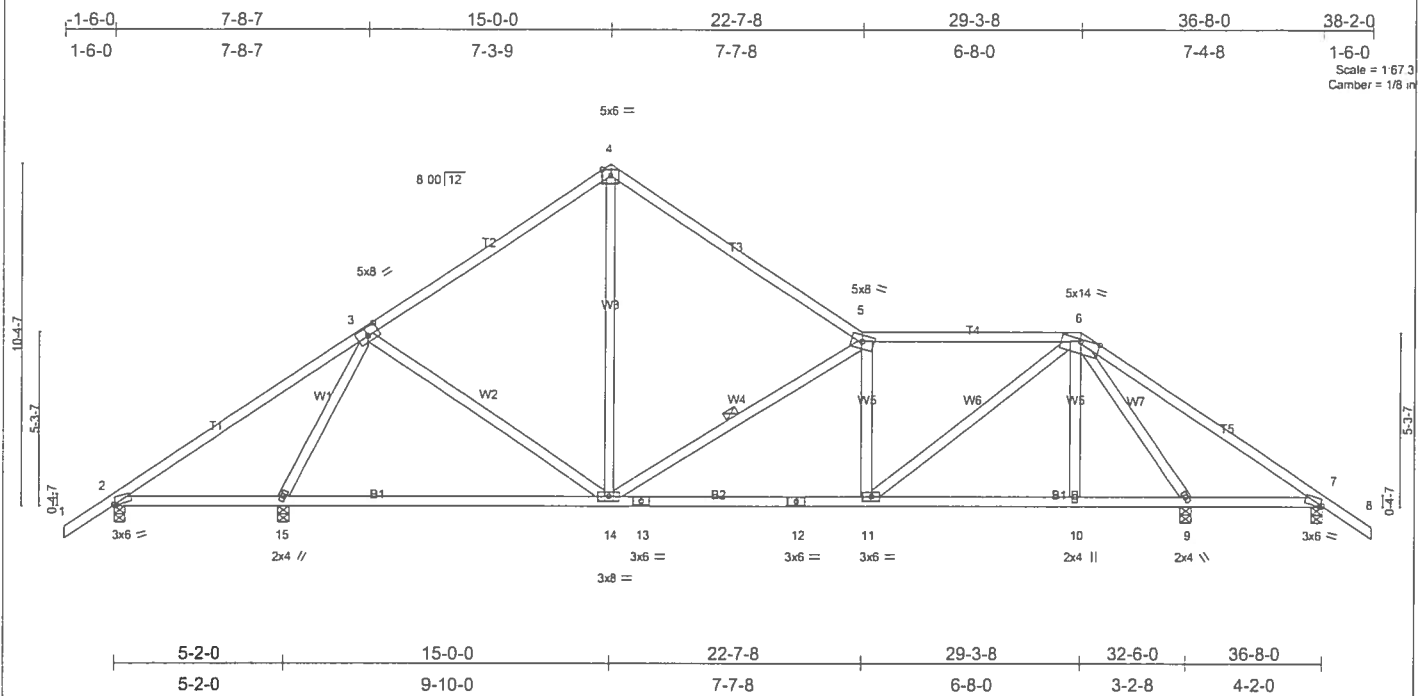


Plate Offsets (X,Y): [2-0-0-13,0-0-6], [3-0-4-0,0-3-0], [7-0-0-12,Edge]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.43	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.52	Vert(LL) 0.06 2-15 >974 240	Weight: 207 lb	
BCLL 10.0	Rep Stress Incr YES	WB 0.96	Vert(TL) -0.29 14-15 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.03 9 n/a n/a		

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 5-14

REACTIONS (lb/size) 2=65/0-4-0, 15=1614/0-4-0, 9=1448/0-4-0, 7=105/0-4-0
 Max Horz 2=-354(load case 3)
 Max Uplift 2=-162(load case 5), 15=-554(load case 5), 9=-511(load case 6), 7=-192(load case 6)
 Max Grav 2=150(load case 9), 15=1614(load case 1), 9=1448(load case 1), 7=128(load case 10)

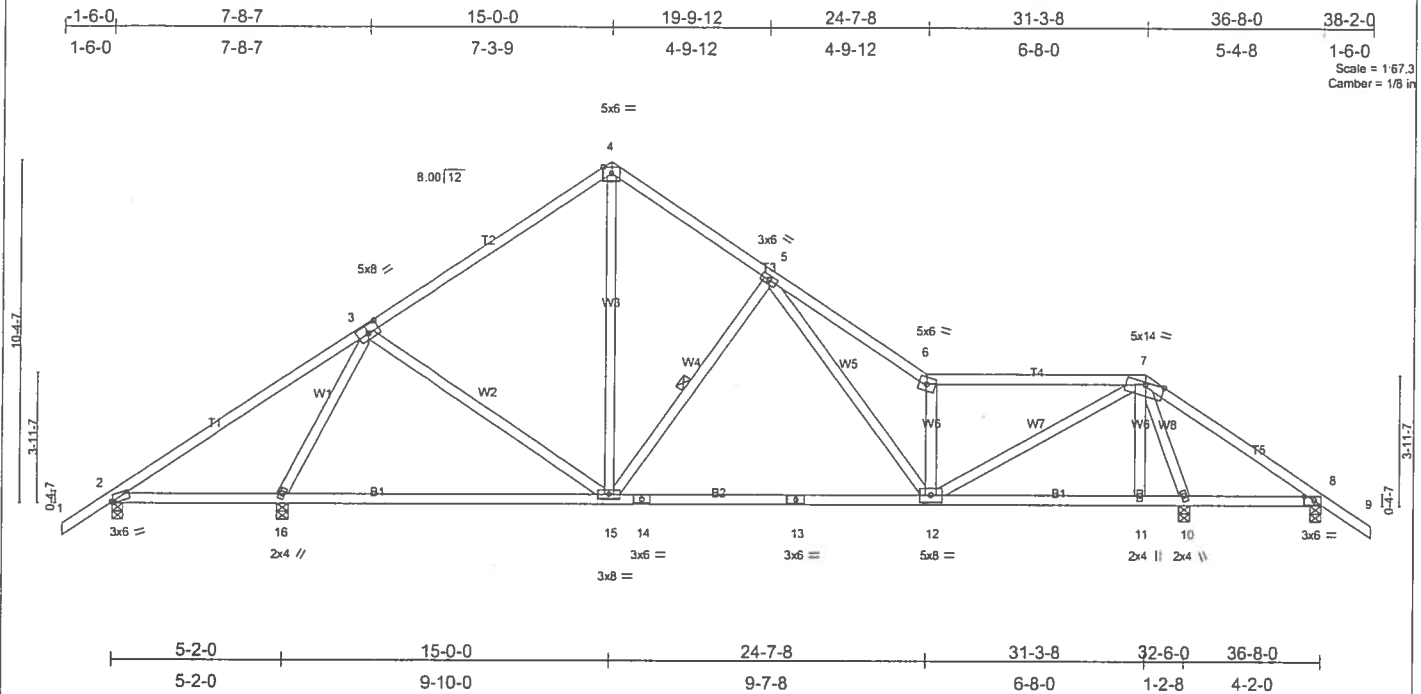
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=-237/506, 3-4=-921/427, 4-5=-921/385, 5-6=-1354/576, 6-7=-79/429, 7-8=0/45
 BOT CHORD 2-15=-319/312, 14-15=-213/330, 13-14=-291/1365, 12-13=-291/1365, 11-12=-291/1365, 10-11=-100/590, 9-10=-101/587, 7-9=-259/221
 WEBS 3-15=-1468/570, 3-14=-72/477, 4-14=-190/487, 5-14=-828/468, 5-11=-376/217, 6-11=-297/961, 6-10=0/142, 6-9=-1529/491

JOINT STRESS INDEX
 2 = 0.87, 3 = 0.69, 4 = 0.68, 5 = 0.76, 6 = 0.33, 7 = 0.78, 9 = 0.76, 10 = 0.34, 11 = 0.57, 12 = 0.53, 13 = 0.66, 14 = 0.57 and 15 = 0.60

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 2, 554 lb uplift at joint 15, 511 lb uplift at joint 9 and 192 lb uplift at joint 7.

LOAD CASE(S) Standard

Job L217950	Truss T11	Truss Type SPECIAL	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:37:58 2006 Page 1		



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.55	Vert(LL)	-0.21 12-15	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.50	Vert(TL)	-0.35 12-15	>930	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.90	Horz(TL)	0.01 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 5-15

REACTIONS (lb/size)	2=-3/0-4-0, 16=1628/0-4-0, 10=1950/0-4-0, 8=-343/0-4-0
Max Horz 2=354(load case 4)	
Max Uplift 2=-174(load case 10), 16=-559(load case 5), 10=-721(load case 6), 8=-343(load case 1)	
Max Grav 2=113(load case 9), 16=1628(load case 1), 10=1950(load case 1), 8=61(load case 5)	

FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD 1-2=0/45, 2-3=-223/632, 3-4=-821/378, 4-5=-763/370, 5-6=-1622/679, 6-7=-1240/459, 7-8=-369/1113, 8-9=0/45	
BOT CHORD 2-16=-421/355, 15-16=-199/298, 14-15=-118/846, 13-14=-118/846, 12-13=-118/846, 11-12=-206/220, 10-11=-209/218, 8-10=-835/434	
WEBS 3-16=-1498/576, 3-15=-60/473, 4-15=-221/441, 5-15=-479/347, 5-12=-344/751, 6-12=-1165/581, 7-12=-515/1657, 7-11=0/81, 7-10=-1914/663	

JOINT STRESS INDEX	
2 = 0.80, 3 = 0.70, 4 = 0.50, 5 = 0.52, 6 = 0.72, 7 = 0.58, 8 = 0.61, 10 = 0.73, 11 = 0.34, 12 = 0.76, 13 = 0.58, 14 = 0.55, 15 = 0.58 and 16 = 0.61	

NOTES	
1) Unbalanced roof live loads have been considered for this design.	
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.	
3) Provide adequate drainage to prevent water ponding.	
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi	
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 2, 559 lb uplift at joint 16, 721 lb uplift at joint 10 and 343 lb uplift at joint 8.	

LOAD CASE(S) Standard	
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Job L217950	Truss T12	Truss Type SPECIAL	Qty 1	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6 300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 04 15:51:31 2007 Page 1			

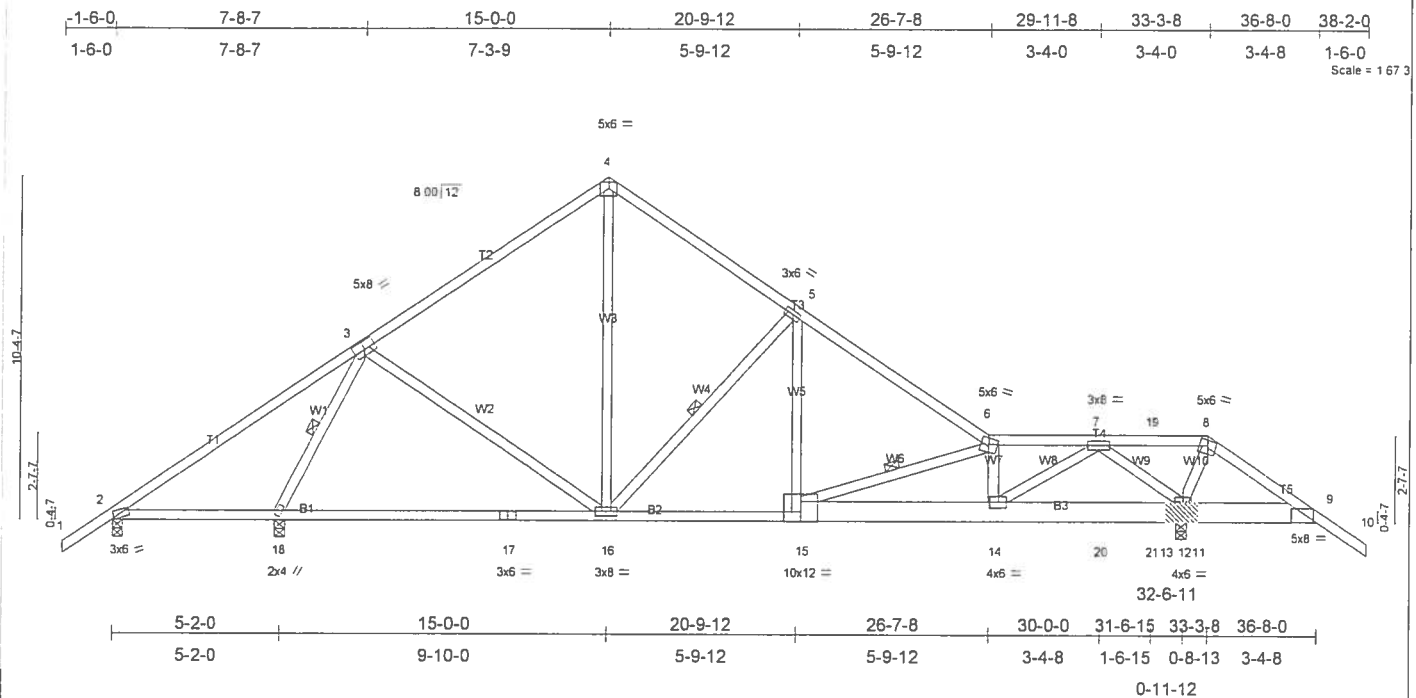


Plate Offsets (X,Y) [2-0-0-13,0-0-6], [3-0-4-0,0-3-0], [9-0-4-0,0-1-9]													
LOADING (psf)		SPACING		2-0-0		CSI		DEFL		PLATES		GRIP	
TCLL 20.0		Plates Increase		1.25		TC 0.56		in (loc) l/defl L/d		MT20		244/190	
TCDL 7.0		Lumber Increase		1.25		BC 0.67		Vert(LL) -0.19 16-18 >999 240					
BCLL 10.0		Rep Stress Incr		NO		WB 0.65		Vert(TL) -0.33 16-18 >999 180					
BCDL 5.0		Code FBC2004/TP12002				(Matrix)		Horz(TL) 0.03 12 n/a n/a					
										Weight: 233 lb			

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-4-9 oc purlins.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
B3 2 X 8 SYP 2400F 2.0E	WEBS 1 Row at midpt 3-18, 5-16, 6-15
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=-127/0-4-0, 18=1961/0-4-0, 12=3525/0-4-3 (0-4-0 + bearing block)
 Max Horz 2=-353(load case 2)
 Max Uplift 2=-297(load case 9), 18=-781(load case 4), 12=-2136(load case 5)
 Max Grav 2=66(load case 3), 18=1961(load case 1), 12=3525(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=-385/840, 3-4=-987/465, 4-5=-954/452, 5-6=-1781/707, 6-7=-3313/1527, 7-19=-310/266, 8-19=-310/266, 8-9=-339/290, 9-10=0/48
 BOT CHORD 2-18=-593/465, 17-18=-160/325, 16-17=-160/325, 15-16=-403/1398, 14-15=-1436/3343, 14-20=-816/1628, 20-21=-816/1628, 13-21=-816/1628, 12-13=-816/1628, 11-12=-223/269, 9-11=-223/269
 WEBS 3-18=-1851/825, 3-16=-166/659, 4-16=-298/613, 5-16=-982/576, 5-15=-336/852, 6-15=-2068/1102, 6-14=-437/178, 7-14=-931/2039, 7-12=-2044/967, 8-12=-132/167

JOINT STRESS INDEX
 2 = 0.83, 3 = 0.72, 4 = 0.51, 5 = 0.65, 6 = 0.60, 7 = 0.98, 8 = 0.23, 9 = 0.16, 11 = 0.00, 11 = 0.00, 12 = 0.44, 12 = 0.00, 13 = 0.00, 13 = 0.00, 14 = 0.83, 15 = 0.65, 16 = 0.63, 17 = 0.37 and 18 = 0.75

NOTES

- 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 12 attached to front face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners. Bearing is assumed to be SYP.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02, 110mph (3-second gust), h=20ft; TCDL=4.2psf, BCDL=3.0psf, Category II, Exp B, enclosed, MWFRS gable end zone, cantilever right exposed, porch left exposed, Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2, 781 lb uplift at joint 18 and 2136 lb uplift at joint 12.
- Girder carries hip end with 3-4-8 right side setback, 31-6-15 left side setback, and 3-4-8 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1169 lb down and 965 lb up at 30-0-0, and 89 lb down and 67 lb up at 33-3-8, and 829 lb down and 631 lb up at 31-6-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-54, 4-6=-54, 6-19=-54, 8-19=-68(F=-14), 8-10=-54, 2-21=-30, 11-21=-38(F=-8), 9-11=-30
 Concentrated Loads (lb)
 Vert 11=-89(F) 20=-1169(F) 21=-829(F)

Job L217950	Truss T13	Truss Type COMMON	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:42:50 2006 Page 1		

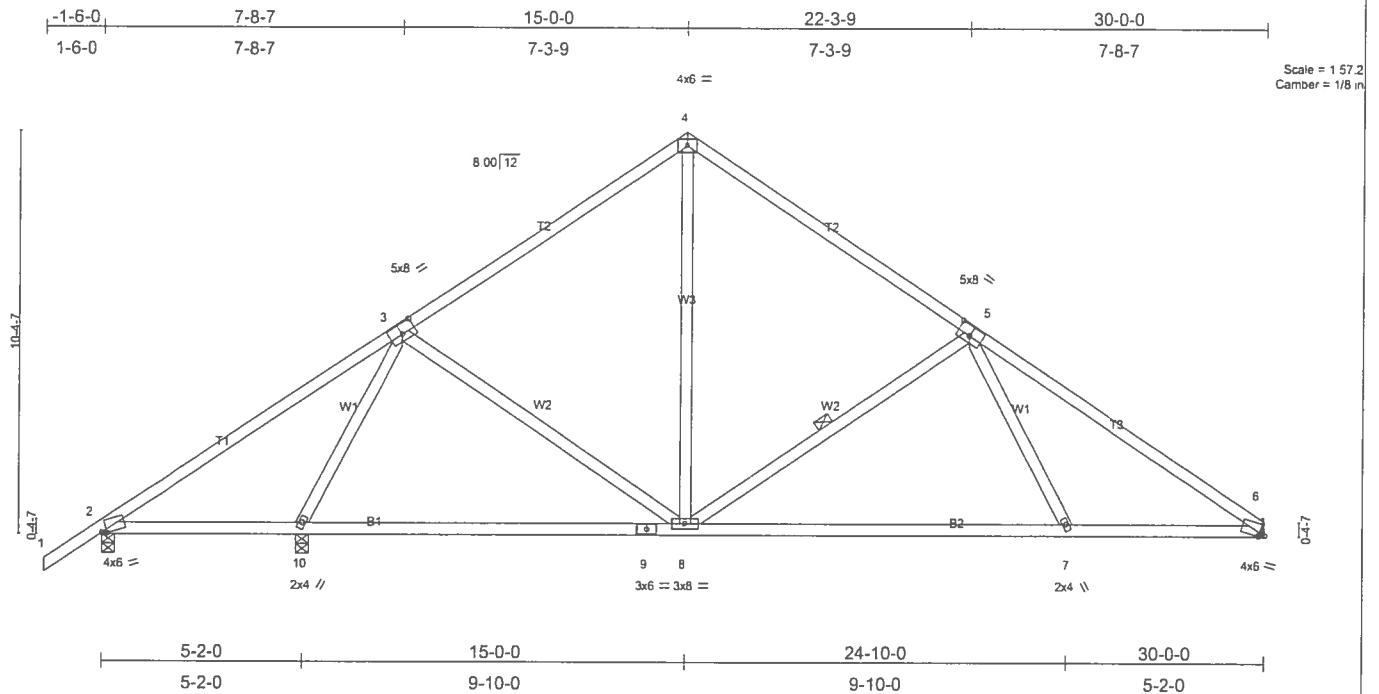


Plate Offsets (X,Y): [2:0-1-9,0-0-10], [3:0-4-0,0-3-0], [5:0-4-0,0-3-0], [6:0-1-9,0-0-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.17	7-8	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.58	Vert(TL)	-0.29	7-8	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.76	Horz(TL)	0.03	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 158 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
WEBS 6-0-0 oc bracing: 2-10, 5-8
1 Row at midpt

REACTIONS (lb/size) 2=147/0-4-0, 10=1430/0-4-0, 6=1007/Mechanical

Max Horz 2=371(load case 4)
Max Uplift 2=-120(load case 5), 10=-446(load case 5), 6=-335(load case 6)
Max Grav 2=198(load case 9), 10=1430(load case 1), 6=1007(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-160/369, 3-4=-853/392, 4-5=-853/361, 5-6=-1552/468
BOT CHORD 2-10=-206/229, 9-10=-226/359, 8-9=-226/359, 7-8=-264/1049, 6-7=-262/1192
WEBS 3-10=-1276/519, 3-8=-63/378, 4-8=-179/431, 5-8=-546/407, 5-7=0/322

JOINT STRESS INDEX

2 = 0.89, 3 = 0.69, 4 = 0.69, 5 = 0.69, 6 = 0.89, 7 = 0.52, 8 = 0.57, 9 = 0.61 and 10 = 0.52

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 2, 446 lb uplift at joint 10 and 335 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L217950	Truss T14	Truss Type COMMON	Qty 6	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.300 s Apr 19 2006 Mittek Industries, Inc. Fri Nov 17 09 44:05 2006 Page 1

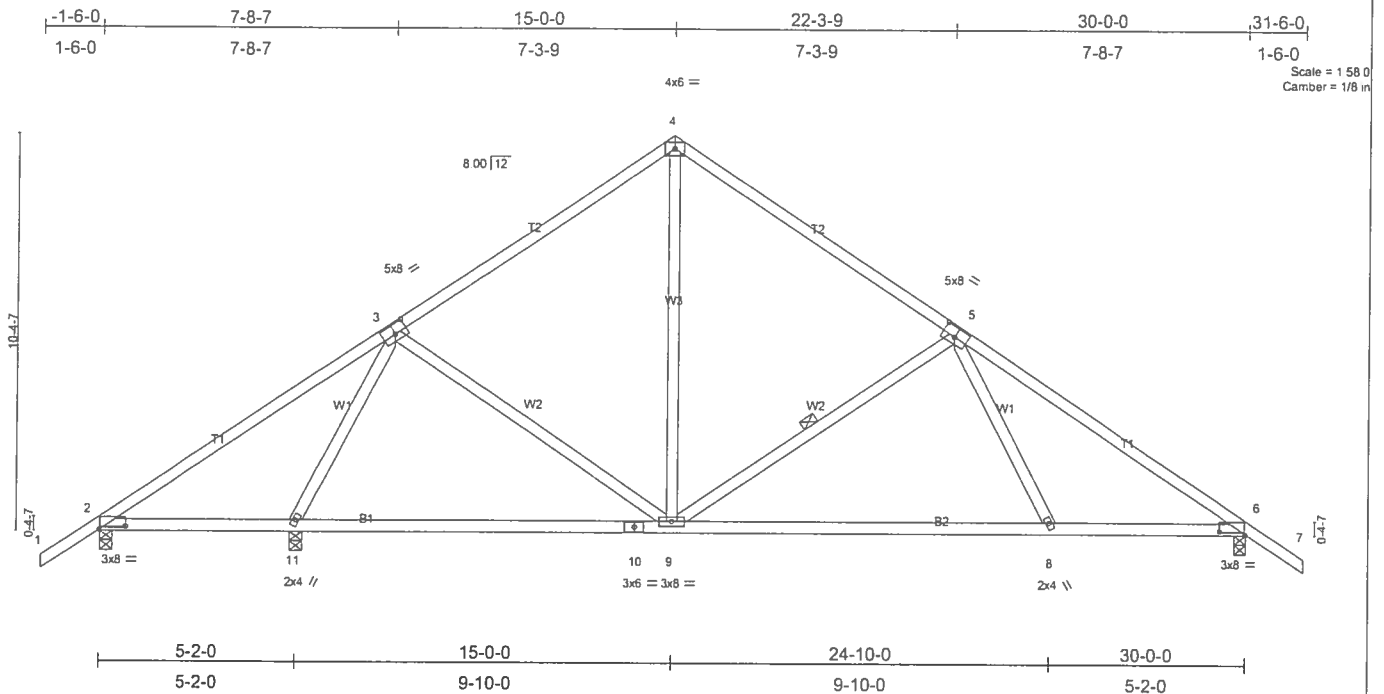


Plate Offsets (X,Y): [2:0-8-3,0-0-14], [3:0-4-0,0-3-0], [5:0-4-0,0-3-0], [6:0-8-3,0-0-14]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL)	-0.17	8-9	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.56	Vert(TL)	-0.30	8-9	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.76	Horz(TL)	0.03	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
									Weight: 161 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
WEBS 6-0-0 oc bracing: 2-11.
1 Row at midpt 5-9

REACTIONS (lb/size) 2=149/0-4-0, 11=1425/0-4-0, 6=1099/0-3-8

Max Horz 2=-354(load case 3)
Max Uplift 2=-128(load case 5), 11=-435(load case 5), 6=-436(load case 6)
Max Grav 2=199(load case 9), 11=1425(load case 1), 6=1099(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-150/366, 3-4=-850/395, 4-5=-850/363, 5-6=-1533/447, 6-7=0/44
BOT CHORD 2-11=-203/260, 10-11=-214/359, 9-10=-214/359, 8-9=-214/1038, 6-8=-202/1174
WEBS 3-11=-1271/507, 3-9=-58/375, 4-9=-180/427, 5-9=-534/390, 5-8=0/309

JOINT STRESS INDEX

2 = 0.70, 3 = 0.69, 4 = 0.73, 5 = 0.69, 6 = 0.70, 8 = 0.52, 9 = 0.57, 10 = 0.61 and 11 = 0.52

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 2, 435 lb uplift at joint 11 and 436 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L217950	Truss T15	Truss Type MONO HIP	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6:300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:44:48 2006 Page 1		

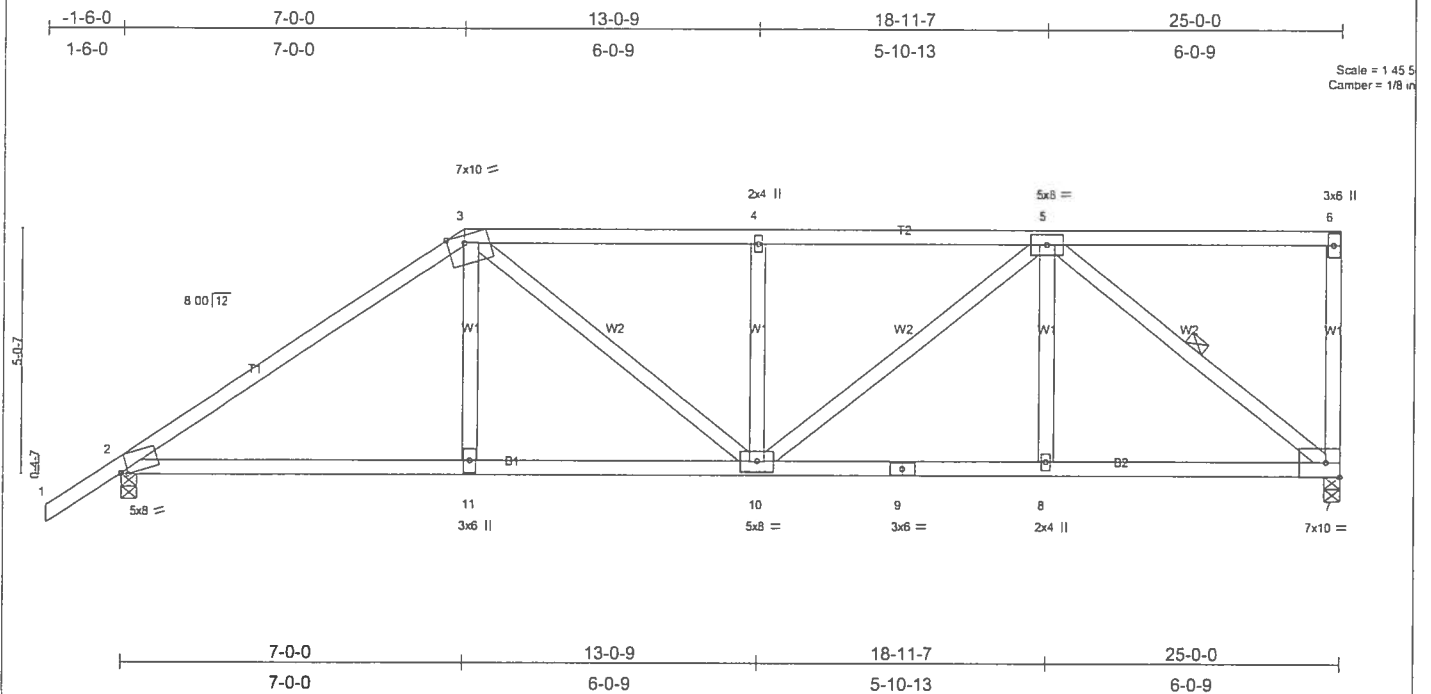


Plate Offsets (X,Y): [2-0-1-13,Edge], [3-0-4-0,Edge]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCDL 20.0	2-0-0	TC 0.85	in (loc) l/defl L/d	MT20	244/190
BCLL 7.0	Plates Increase 1.25	BC 0.76	Vert(LL) -0.17 10-11 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.84	Vert(TL) -0.29 10-11 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.10 7 n/a n/a		
	Code FBC2004/TP12002			Weight: 138 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-11-15 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-0-15 oc bracing
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=2302/0-4-0, 2=2146/0-4-0
 Max Horz 2=281(load case 4)
 Max Uplift 7=1402(load case 2), 2=1155(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=-3289/1801, 3-4=-3162/1887, 4-5=-3161/1888, 5-6=-83/53, 6-7=-324/345
 BOT CHORD 2-11=-1544/2633, 10-11=-1554/2659, 9-10=-1390/2306, 8-9=-1390/2306, 7-8=-1390/2306
 WEBS 3-11=-358/857, 3-10=-567/637, 4-10=-694/814, 5-10=-658/1097, 5-8=0/385, 5-7=-2853/1716

JOINT STRESS INDEX
 2 = 0.83, 3 = 0.88, 4 = 0.47, 5 = 0.67, 6 = 0.65, 7 = 0.50, 8 = 0.34, 9 = 0.98, 10 = 0.51 and 11 = 0.28

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1402 lb uplift at joint 7 and 1155 lb uplift at joint 2.
- 5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 410 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-6=-117(F=-63), 2-11=-30, 7-11=-65(F=-35)
 Concentrated Loads (lb)
 Vert: 11=-539(F)

Job L217950	Truss T16	Truss Type MONO HIP	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6 300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:45:01 2006 Page 1		

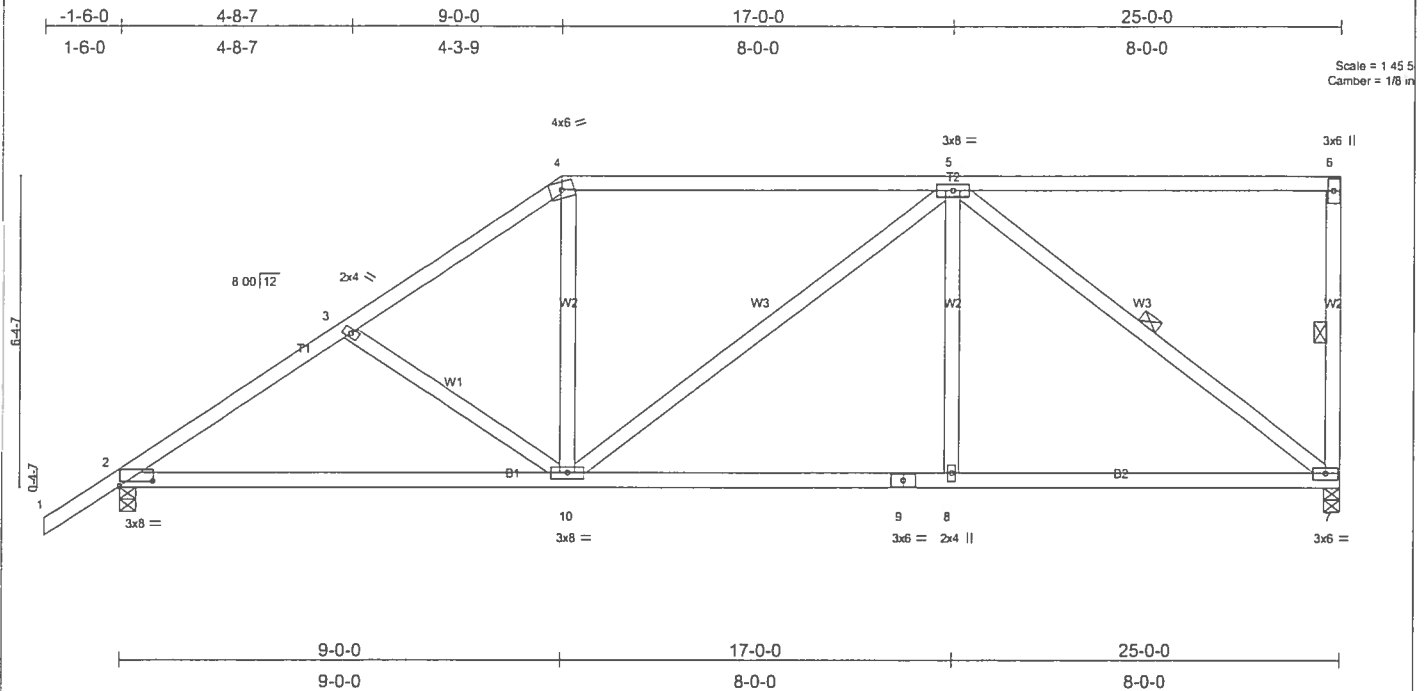


Plate Offsets (X,Y): [2.0-8-3.0-1-2]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25		TC 0.63	Vert(LL) -0.15	2-10	>999	240		MT20	244/190
TCDL 7.0	Lumber Increase 1.25		BC 0.46	Vert(TL) -0.26	2-10	>999	180			
BCLL 10.0	Rep Stress Incr YES		WB 0.55	Horz(TL) 0.04	7	n/a	n/a			
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 142 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 8-8-14 oc bracing.
 WEBS 1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 7=1034/0-4-0, 2=1130/0-4-0
 Max Horz 2=341(load case 5)
 Max Uplift 7=-396(load case 3), 2=-384(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=-1469/400, 3-4=-1281/368, 4-5=-1029/351, 5-6=-49/21, 6-7=-192/157
 BOT CHORD 2-10=-521/1170, 9-10=-360/986, 8-9=-360/986, 7-8=-360/986
 WEBS 3-10=-180/206, 4-10=-6/336, 5-10=-93/157, 5-6=0/230, 5-7=-1186/433

JOINT STRESS INDEX
 2 = 0.78, 3 = 0.34, 4 = 0.67, 5 = 0.57, 6 = 0.49, 7 = 0.64, 8 = 0.34, 9 = 0.42 and 10 = 0.57

NOTES
 1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II, Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Provide adequate drainage to prevent water ponding.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 396 lb uplift at joint 7 and 384 lb uplift at joint 2.

LOAD CASE(S) Standard

Job L217950	Truss T17	Truss Type HIP	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:45:18 2006 Page 1

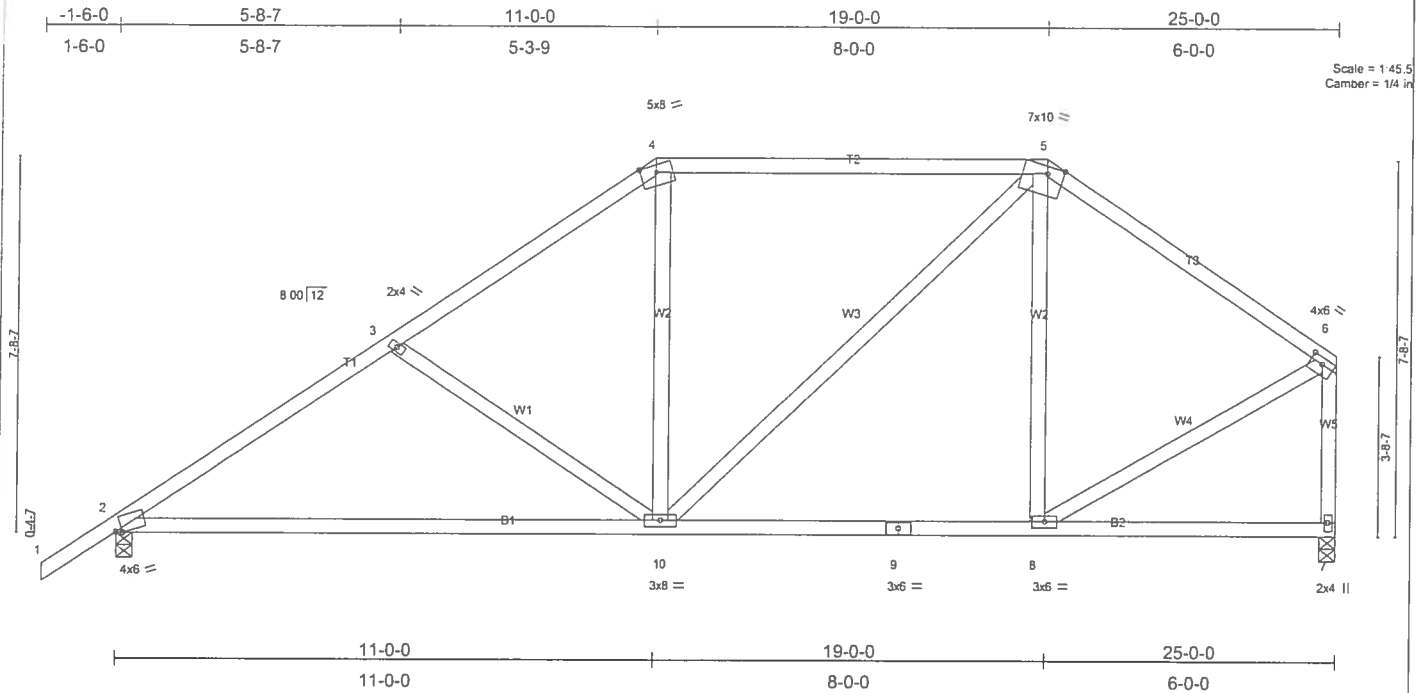


Plate Offsets (X,Y): [2.0-1.9,0.0-10], [5.0-4.0,Edge], [6.0-3.0,0.1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.49	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.65	Vert(LL) -0.33 2-10 >909 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.37	Vert(TL) -0.56 2-10 >528 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 7 n/a n/a		
	Code FBC2004/TPI2002			Weight: 143 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-13 oc bracing.

REACTIONS (lb/size) 2=1130/0-4-0, 7=1034/0-4-0
Max Horz 2=277(load case 5)
Max Uplift 2=412(load case 5), 7=285(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1416/443, 3-4=-1167/368, 4-5=-912/379, 5-6=-884/286, 6-7=-945/304
BOT CHORD 2-10=-479/1129, 9-10=-198/672, 8-9=-198/672, 7-8=-36/46
WEBS 3-10=-268/273, 4-10=-22/239, 5-10=-185/384, 5-8=-184/170, 6-8=-268/728

JOINT STRESS INDEX
2 = 0.86, 3 = 0.34, 4 = 0.69, 5 = 0.79, 6 = 0.82, 7 = 0.84, 8 = 0.42, 9 = 0.33 and 10 = 0.57

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 412 lb uplift at joint 2 and 285 lb uplift at joint 7.

LOAD CASE(S) Standard

Job L217950	Truss T18	Truss Type HIP	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, Fl 32055					Job Reference (optional) 6.300 s Apr 19 2006 Mittek Industries, Inc. Fri Nov 17 09:45:30 2006 Page 1

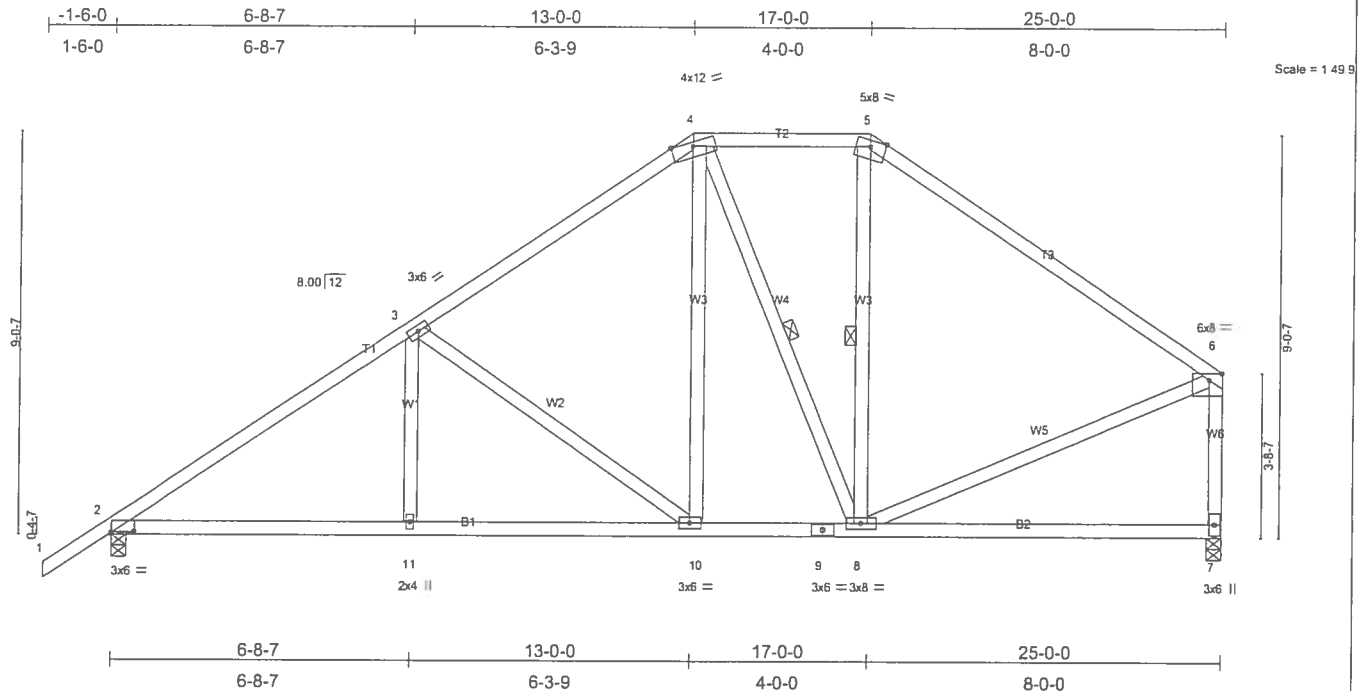


Plate Offsets (X,Y): [2-0-6-3-0-0-6], [6-0-3-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.92	Vert(LL)	-0.09	7-8	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.46	Vert(TL)	-0.15	7-8	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.57	Horz(TL)	0.03	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
Weight: 157 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-9-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-0 oc bracing.
WEBS 1 Row at midpt 4-8, 5-8

REACTIONS (lb/size) 2=1130/0-4-0, 7=1034/0-4-0
Max Horz 2=311(load case 4)
Max Uplift 2=-420(load case 5), 7=-299(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-1510/431, 3-4=-993/363, 4-5=-676/348, 5-6=-930/315, 6-7=-907/315
BOT CHORD 2-11=-476/1169, 10-11=-476/1169, 9-10=-221/745, 8-9=-221/745, 7-8=-74/117
WEBS 3-11=0/233, 3-10=-526/326, 4-10=-164/423, 4-8=-274/201, 5-8=-77/230, 6-8=-182/608

JOINT STRESS INDEX

2 = 0.71, 3 = 0.43, 4 = 0.68, 5 = 0.69, 6 = 0.60, 7 = 0.52, 8 = 0.64, 9 = 0.38, 10 = 0.35 and 11 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 420 lb uplift at joint 2 and 299 lb uplift at joint 7.

LOAD CASE(S) Standard

Job L217950	Truss T20	Truss Type GABLE	Qty 1	Ply 1	THACKER RESIDENCE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Nov 17 09:48:49 2006 Page 1		

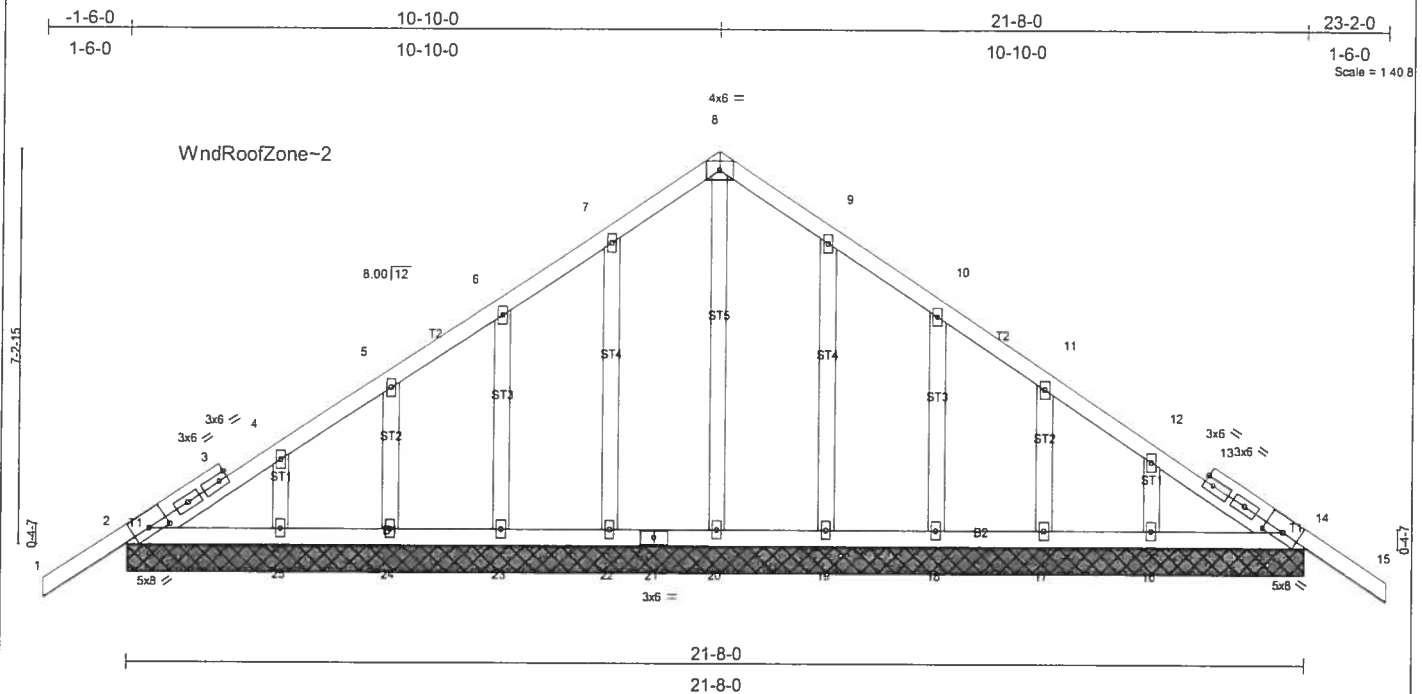


Plate Offsets (X,Y): [2.0-4.5,0-1-12], [14.0-4.5,0-1-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	V/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.16	Vert(LL)	-0.01	15	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.04	Vert(TL)	-0.01	15	n/r	90		
BCLL 10.0	Rep Stress Incr	NO	WB 0.12	Horz(TL)	0.01	14	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 131 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 2=248/21-8-0, 14=248/21-8-0, 20=169/21-8-0, 22=187/21-8-0, 23=191/21-8-0, 24=179/21-8-0, 25=226/21-8-0, 19=187/21-8-0, 18=191/21-8-0, 17=179/21-8-0, 16=226/21-8-0
 Max Horz 2=245(load case 4)
 Max Uplift 2=-101(load case 5), 14=-124(load case 6), 22=-103(load case 5), 23=-114(load case 5), 24=-120(load case 5), 25=-79(load case 5), 19=-100(load case 6), 18=-115(load case 6), 17=-119(load case 6), 16=-85(load case 6)
 Max Grav 2=248(load case 1), 14=248(load case 1), 20=169(load case 1), 22=191(load case 9), 23=191(load case 1), 24=179(load case 9), 25=226(load case 1), 19=191(load case 10), 18=191(load case 1), 17=179(load case 10), 16=226(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/51, 2-3=-192/143, 3-4=-185/152, 4-5=-150/146, 5-6=-99/140, 6-7=-54/167, 7-8=-53/209, 8-9=-53/207, 9-10=-54/145, 10-11=-53/73, 11-12=-56/52, 12-13=-99/64, 13-14=-104/55, 14-15=0/51
 BOT CHORD 2-25=-33/182, 24-25=-33/182, 23-24=-33/182, 22-23=-33/182, 21-22=-33/182, 20-21=-33/182, 19-20=-33/182, 18-19=-33/182, 17-18=-33/182, 16-17=-33/182, 14-16=-33/182
 WEBS 8-20=-141/10, 7-22=-132/115, 6-23=-129/127, 5-24=-124/128, 4-25=-150/102, 9-19=-132/112, 10-18=-129/128, 11-17=-124/127, 12-16=-150/107

JOINT STRESS INDEX
 2 = 0.41, 3 = 0.00, 3 = 0.15, 3 = 0.15, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.28, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.00, 13 = 0.15, 13 = 0.15, 14 = 0.41, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.15, 22 = 0.34, 23 = 0.34, 24 = 0.34 and 25 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 2, 124 lb uplift at joint 14, 103 lb uplift at joint 22, 114 lb uplift at joint 23, 120 lb uplift at joint 24, 79 lb uplift at joint 25, 100 lb uplift at joint 19, 115 lb uplift at joint 18, 119 lb uplift at joint 17 and 85 lb uplift at joint 16.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

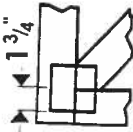
LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-8=-64(F=-10), 8-15=-64(F=-10), 2-14=-30

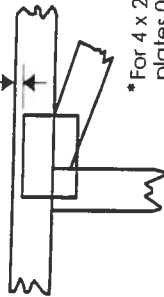
Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seal.



0-1/16"



* For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

* This symbol indicates the required direction of slots in connector plates.



* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

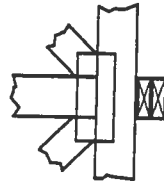
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



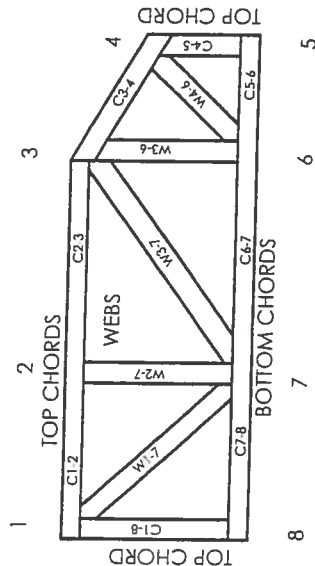
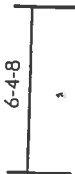
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPII: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths

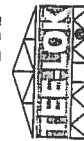


JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A



Mitek Engineering Reference Sheet: MIL-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPII.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPII.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.