	2/2009		Be Prominently Posted	uilding Permit		PERMIT 000027846
APPLICANT	VINCE D	ICHARDSON	be I folimiently I osted	PHONE	755-5779	000027840
ADDRESS	692	SW ARLINGTON	LBLVD	LAKE CITY	133-3117	FL 32025
OWNER		1 & BONNIE ROBB		PHONE	497-4252	
ADDRESS	418	SW HILLTOP TE		FT. WHITE		FL 32038
CONTRACTOR		ICE RICHARDSON		PHONE	497-4252	· <del>-</del>
LOCATION OF	PROPER	TY 47S, TL	HERLONG RD, TR HILL	TOP, 4TH LOT ON LE	FT	_
TYPE DEVELO	OPMENT	SCREEN ENCL	OSURE EST	TIMATED COST OF CO	ONSTRUCTION	5580.00
HEATED FLOO	OR AREA	W <u>=</u>	TOTAL ARE	A	HEIGHT	STORIES
FOUNDATION		WA	LLS R	OOF PITCH	F	LOOR
LAND USE & 2	ZONING	A-3		MAX	K. HEIGHT	N
Minimum Set B			Γ-FRONT 30.00	REAR	25.00	SIDE 25.00
	200 CHONG STONE (1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 19					25.00
NO. EX.D.U.	0	FLOOD ZONE	N/A	DEVELOPMENT PER	MIT NO.	
PARCEL ID	10-6S-16-	03815-153	SUBDIVISION	CARDINAL FARM	MS	
LOT <u>53</u>	BLOCK	PHASE	UNIT	TOTA	AL ACRES	
			110	1/mc.	Richards	w w
Culvert Permit N	lo.	Culvert Waiver	Contractor's License Num		Applicant/Owne	
EXISTING		X09-152	cs	<u> </u>	HD	N
Driveway Conne	ection	Septic Tank Number	er LU & Zonin	ig checked by App	proved for Issuar	nce New Resident
COMMENTS:	NOC ON	FILE				
					Check # or C	Cash 2157
			BUILDING & ZONIN	IG DEPARTMENT		
Temporary Power	er		BUILDING & ZONIN Foundation	IG DEPARTMENT		Cash 2157 (footer/Slab)
Temporary Power	er			IG DEPARTMENT	ONLY	
Temporary Powe		FOR B date/app. by	Foundation Slab	date/app. by	ONLY  Monolithic	(footer/Slab)  date/app. by  2/Nailing
Under slab roug	h-in plumb	FOR B  date/app. by  bing  date/a	Foundation Slab app. by		ONLY  Monolithic	(footer/Slab) date/app. by
	h-in plumb	FOR B  date/app. by  ing  date/a	Foundation Slab papp. by	date/app. by	ONLY  Monolithic	(footer/Slab)  date/app. by  2/Nailing
Under slab roug	h-in plumb date/ap	date/app. by sing date/app. by p. by	Foundation Slab app. by nsulation date	date/app. by  date/app. by	ONLY  Monolithic	(footer/Slab)  date/app. by  g/Nailing  date/app. by
Under slab roug  Framing  Rough-in plumb	date/ap	FOR B  date/app. by  ing  date/a	Foundation Slab app. by date	date/app. by  date/app. by	Monolithic Sheathing	(footer/Slab)  date/app. by  g/Nailing  date/app. by
Under slab roug	date/ap	date/app. by sing date/a p. by slab and below wood	Foundation Slab app. by date	date/app. by  date/app. by  e/app. by  Elate/app. by	Monolithic Sheathing	(footer/Slab)  date/app. by  g/Nailing  date/app. by  date/app. by
Under slab roug  Framing  Rough-in plumb	date/ap	date/app. by sing date/app. by p. by	Foundation Slab app. by sulation date	date/app. by  date/app. by  e/app. by  Elate/app. by	Monolithic Sheathing ectrical rough-in	(footer/Slab)  date/app. by  g/Nailing  date/app. by
Under slab roug  Framing  Rough-in plumb  Heat & Air Duct  Permanent power	date/ap ing above s	date/app. by  date/a  date/app. by  date/a  p. by  slab and below wood  ate/app. by  te/app. by	Foundation Slab app. by nsulation date floor Peri. beam (Linte	date/app. by  date/app. by  e/app. by  Elate/app. by	Monolithic Sheathing	(footer/Slab)  date/app. by  g/Nailing  date/app. by  date/app. by
Under slab roug  Framing  Rough-in plumb  Heat & Air Duct  Permanent powe  Pump pole	date/ap ing above s	date/app. by  date/a  date/a  date/a  p. by  slab and below wood  ate/app. by  te/app. by  Utility Pole	Foundation Slab app. by nsulation date floor  Peri. beam (Linte C.O. Final  M/H tie do	date/app. by  date/app. by  e/app. by  Elate/app. by  date/app. by  date/app. by	Monolithic Sheathing ectrical rough-in Pool Culvert	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Under slab roug  Framing  Rough-in plumb  Heat & Air Duct  Permanent powe  Pump pole	date/ap ing above s t date date	date/app. by  date/a  date/a  date/a  p. by  slab and below wood  ate/app. by  te/app. by  Utility Pole	Foundation Slab app. by nsulation date floor Peri. beam (Linte	date/app. by  date/app. by  e/app. by  Elate/app. by  date/app. by  date/app. by  date/app. by  eate/app. by  pwns, blocking, electricit	Monolithic Sheathing ectrical rough-in Pool Culvert	date/app. by
Under slab roug  Framing  Rough-in plumb  Heat & Air Duct  Permanent powe  Pump pole  da	date/ap ing above s t date/app. by	date/app. by  date/a  date/a  date/a  p. by  slab and below wood  ate/app. by  te/app. by  Utility Pole	Foundation Slab app. by nsulation date floor C.O. Final M/H tie do ate/app. by	date/app. by  date/app. by  e/app. by  Elate/app. by  date/app. by  date/app. by	Monolithic  Sheathing  ectrical rough-in  Pool  Culvert  y and plumbing	date/app. by
Under slab roug  Framing  Rough-in plumb  Heat & Air Duct  Permanent powe  Pump pole  da	date/ap ing above s t date/app. by	date/app. by  date/a  date/app. by  date/a  p. by  slab and below wood  ate/app. by  te/app. by  Utility Pole  date/app. by	Foundation Slab app. by nsulation date floor C.O. Final M/H tie do ate/app. by	date/app. by  date/app. by  e/app. by  ate/app. by  date/app. by  date/app. by  ate/app. by  owns, blocking, electricit  date/app. by	Monolithic  Sheathing  ectrical rough-in  Pool  Culvert  y and plumbing	date/app. by
Under slab roug  Framing  Rough-in plumb  Heat & Air Duct  Permanent powe  Pump pole  da  Reconnection	date/ap ing above s t da r da tte/app. by	date/app. by  date/a  date/app. by  slab and below wood  ate/app. by  te/app. by  Utility Pole  date/app. by  30.00	Foundation Slab app. by nsulation date floor C.O. Final M/H tie do ate/app. by RV	date/app. by  date/app. by  e/app. by  ate/app. by  date/app. by  date/app. by  ate/app. by  owns, blocking, electricit  date/app. by	Monolithic  Sheathing  ectrical rough-in  Pool  Culvert  y and plumbing  Re-roof	date/app. by
Under slab roug  Framing  Rough-in plumb  Heat & Air Duct  Permanent powe  Pump pole  da  Reconnection  BUILDING PER	date/ap ing above s t date/app. by date/app. by	date/app. by  date/a  date/a  date/a  date/a  p. by  slab and below wood  ate/app. by  te/app. by  te/app. by  date/app. by  ZONING	Foundation  Slab  app. by  Insulation  date  Peri. beam (Linte  C.O. Final  M/H tie do  ate/app. by  RV  CERTIFICATION FEE	date/app. by  date/app. by  Elate/app. by  date/app. by  date/app. by  date/app. by  bowns, blocking, electricit  date/app. by  FIRE FEE \$ 0.00	Monolithic  Sheathing  ectrical rough-in  Pool  Culvert  y and plumbing  Re-roof  SURCHARG  WAS	(footer/Slab)  date/app. by  g/Nailing  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  The fee \$ 0.00  TE FEE \$

PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

# **Columbia County Building Permit Application**

For Office Use Only Application # 0905 - 48 Date Received 5/27 By Dermit # 27846 Soning Official Date 6/2/09 Flood Zone NA Land Use A-3 Zoning A-3
FEMA Map # Elevation MFE River Plans Examiner MD Date 4 - 4 - 9
Comments
□ NOC d'EH de Deed or PA Site Plan = State Road Info = Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Auth. from Contractor □ F W Comp. letter
IMPACT FEES: EMS Fire Corr Road/Code
School = TOTAL
Septic Permit No. 1-09-0152 in Fix 800 Fax 386-755-7889
Name Authorized Person Signing Permit VINCL Hichardson Phone 386-755-5779
Address Le92 5W Arlington Blvd. Lake City, F1 32085
Owners Name William Bonnie Robbins Ft White Phone 386-497-4252
911 Address 418 SW Hilltop Terr. Lake City, Fla. 32038
Contractors Name Richardson Aluminum, LC Phone 386-755-5779
Address 1092 Sw Arlington Blvd. Lake City F1 32025
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address Cow rence E. Bennett
Mortgage Lenders Name & Address
Circle the correct power company — FL Power & Light — Clay Elec. — Suwannee Valley Elec. — Progress Energy
Property ID Number 10-65-16-03815-153 HEstimated Cost of Construction 5580.00/kg
Subdivision Name Cardina Farms Lot 53 Block Unit Phase
Driving Directions 47 South, Turn ( on herlong rd. Turn ( on
Hilltop terr. 2-story on (R) 418 S.W Hilltop Terr
Number of Existing Dwellings on Property
Construction of Pool Enclosure Total Acreage 10 Lot Size 500X872
Do you need a - <u>Culvert Permit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drive</u> Total Building Height
Actual Distance of Structure from Property Lines - Front 195 Side 217 Side 255 Rear 455
Number of Stories Heated Floor Area O Total Floor Area Roof Pitch Custom
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

#### Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION: An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

<u>TIME LIMITATIONS OF PERMITS:</u> Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

	olalilaid ooc	my for obtaining this building Permit.
Wea		
Owners Signature	-	
CONTRACTORS AFFIDAVIT: By my signature I under written statement to the owner of all the above writtens Building Permit including all application and permit including all applications.	tten respons	ibilities in Columbia County for obtaining
Contractor's Signature (Permitee)		tor's License Number bia County ency Card Number_ <u>OOO    O</u> Alum   Viny OOO    I F AMING   before me this <u>21</u> day of <u>May</u> 20 b
Affirmed under penalty of perjury to by the Contractor a	nd subscriber	before me this 27 day of All (1)
Personally known or Produced Identification		day of 10000 20 6
Sanba A Sillata	SEAL:	SANDRA H. TILLOTSON MY COMMISSION # DD 629530 EXPIRES: January 31, 2011
State of Florida Notary Signature (For the Contractor)		Bonded Thru Budget Notary Services

# Bennett Engineering Group, Inc. Lawrence E. Bennett, P.E.

315 Herbert Street Port Orange, FL 32129 386-767-4774 fax: 386-767-6556

January 1, 2009

#### TO ALL BUILDING DEPARTMENTS

Re:

Master File Engineering

"ALUMINUM STRUCTURES DESIGN MANUAL" 2004 Florida Building Code with 2006 Supplements

Dear Building Official/Plans Examiner:

This is to certify that the following contractor/company is hereby authorized to use my "ALUMINUM STRUCTURES DESIGN MANUAL" during the year 2009.

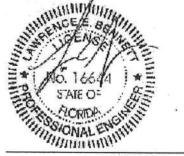
Authorization is on a January to January basis regardless of the edition of the manual. This authorization also applies to contractor master file drawings, "ONE PERMIT ONLY" drawings or any "site specific" drawings that I may furnish the contractor.

The following contractor/company is hereby added to my 2009 MASTERFILE LIST:

Vince Richardson Richardson Aluminum LLC 692 SW Arlington Blvd Lake City, FL 32025

Should you have any questions, please contact me at your convenience.

Sincerely,



Lawrence E. Bennett, P.E. #16644

# **Columbia County Property** Appraiser DB Last Updated: 4/27/2009

# 2009 Preliminary Values

Tax Record

Property Card

Interactive GIS Map

Print

**Owner & Property Info** 

Parcel: 10-6S-16-03815-153 HX

<<	Prev
----	------

Search Result: 2 of 2

Owner's Name	ROBBINS WILLIAM N & BONNIE M					
Site Address	HILLTOP					
Mailing Address	418 SW HILLTOP TERR FT WHITE, FL 32038					
Use Desc. (code)	improved A (005000)					
Neighborhood	011616.00 Tax District 3					
UD Codes	MKTA02	Market Area	02			
Total Land Area	10.010 ACRES					
Description	LOT 53 CARDINAL FARMS UNREC: COMM SE COR OF SEC 11 & RUN W 5311.34 FT, N 1995.16 FT, W 60.18 FT, N 642.99 FT, N 637.72 FT TO POB, RUN W 872.34 FT, N 500.08 FT, E 872.34 FT, S 500.08 FT TO POB. WD 1022-1211.					

# **GIS Aerial**

# **Property & Assessment Values**

Mkt Land Value	cnt: (1)	\$6,412.00
Ag Land Value	cnt: (1)	\$1,802.00
Building Value	cnt: (1)	\$267,034.00
XFOB Value	cnt: (1)	\$10,000.00
Total Appraised Value		\$285,248.00

Just Value		\$363,948.00		
Class Value		\$285,248.00		
Assessed Value		\$285,248.00		
Exemptions	(code: HX)	\$50,000.00		
Total Taxable Value	County: \$235,248.0 City: \$235,248 Other: \$235,248.0 School: \$260,248			

# Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
7/29/2004	1022/1211	WD	V	Q		\$50,000.00

# **Building Characteristics**

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value		
1	SINGLE FAM (000100)	2006	Common BRK (19)	3265	4191	\$267,034.00		
	Note: All S.F. calculations are based on exterior building dimensions.							

# **Extra Features & Out Buildings**

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0030	BARN,MT	2005	\$10,000.00	0001000.000	25 x 40 x 0	(000.00)

# Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000100	SFR (MKT)	0000001.000 AC	1.00/1.00/1.00/1.00	\$6,412.50	\$6,412.00
006200	PASTURE 3 (AG)	0000009.010 AC	1.00/1.00/1.00/1.00	\$200.00	\$1,802.00
009910	MKT.VAL.AG (MKT)	0000009.010 AC	1.00/1.00/1.00/1.00	\$0.00	\$80,502.00

HUNNIE BHANNUN, CFC COLUMBIA COUNTY TAX COLLECTOR ZUUI DEAL COINIE NOTICE OF AD VALOREM TAXES AND NON-AD VALOREM ASSESSMENTS TAX ACCOUNT NUMBER ESCROW CD **ASSESSED VALUE EXEMPTIONS** TAXABLE VALUE MILLAGE CODE R03815-153 297,336 25,000 272,336 003

R

0002877 02 AV 0.437 \*\*AUTO T3 0 0810 32038-123 ROBBINS WILLIAM N & BONNIE M 418 SW HILLTOP TERR FT WHITE FL 32038

SEE INSERT FOR IMPORTANT INFO AND TELEPHONE NUMBERS WWW.COLUMBIATAXCOLLECTOR.COM 10-6S-16 5000/5000 10.01 acres LOT 53 CARDINAL FARMS UNREC: COMM SE COR OF SEC 11 & RUN W 5311.34 FT, N 1995.16 FT, W 60.18 FT, N 642.99 FT, See Tax Roll for extra legal.

AD VALOREM TAXES							
TAXING AUTHORITY	MILLAGE RATE (DOL	LARS PER \$1,000 OF TAX	ABLE VALUE)	TAXES LEVIED			
COO1 BOARD OF COUNTY COMMISSIONERS	7.8530	25,000	272,336	2,138.65			
DISCRETIONARY LOCAL CAPITAL OUTLAY	.7600 4.7800 2.0000	25,000 25,000	272,336 272,336	206.98 1,301.77 544.67			
Y SR SUWANNEE RIVER WATER MGT DIST	.4399	25,000 25,000 25,000	272,336 272,336 272,336	119.80 550.66			
IDA COLUMBIA COUNTY INDUSTRIAL	.1240	25,000	272,336	33.77			

TOTAL MILLAGE 17.9789 AD VALOREM TAXES \$4,896.30

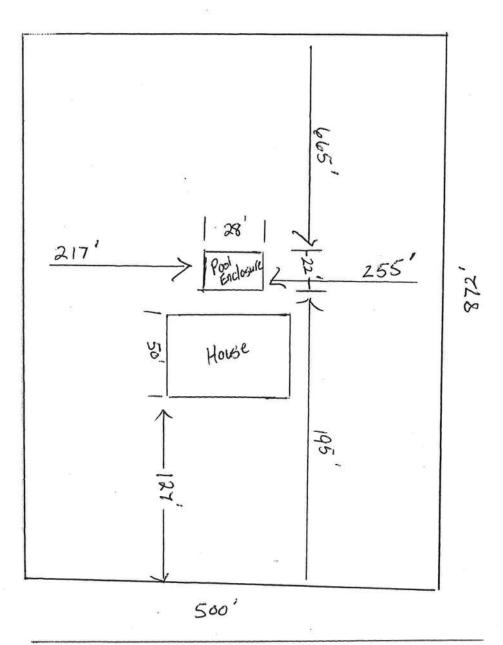
NON	-AD VALOREM ASSESSMENTS	
LEVYING AUTHORITY	RATE	AMOUNT
FFIR FIRE ASSESSMENTS		146.58
GGAR SOLID WASTE - ANNUAL		201.00

LEVYING AUTHORITY	HAIL	AMOUNT
FFIR FIRE ASSESSMENTS		146.58
GGAR SOLID WASTE - ANNUAL		201.00

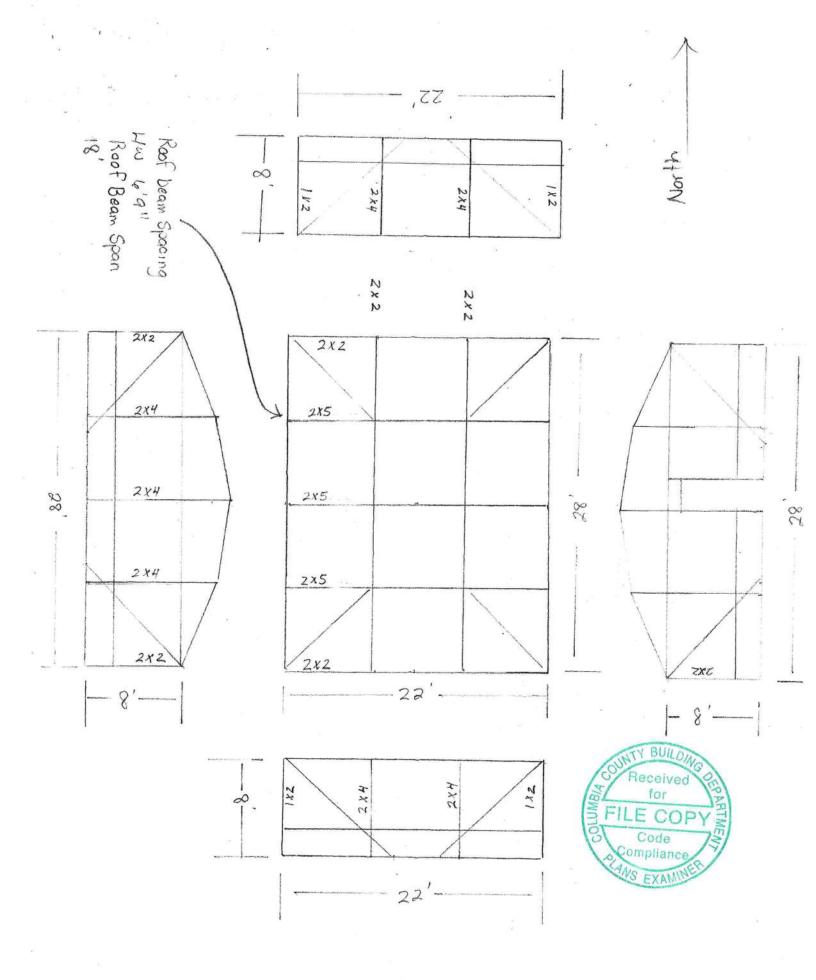
	PAY ONLY ONE AMO	UNT IN YELLOW SHAD	ED AREA	NON-AD VALOREM A	SSESSMENTS	\$347.50			
(	COMBINED TAXES AN	D ASSESSMENTS	\$5,24	1.7 . OO	PAY ONLY ONE AMOUNT	See reverse side for important information.			
(	IF PAID BY PLEASE PAY	Nov 30 5,034.12	Dec 31 5,086.56	Jan 31 5,139.00	Feb 29 5,191.44	Mar 31 5,243.88			

PAID BY





Hill top Terr.



# SECTION 1 SCREENED ENCLOSURES

# General Notes and Specifications:

- The following structures are designed to be married to site built block or wood frame DCA approved modular structures of adequate structural capacity. The contractor / home owner shall verify that the host structure is in good condition and of sufficient strength to hold the proposed addition.
- 2. If the owner or contractor has a question about the host structure, the owner (at his own expense) shall hire an architect, engineer, or a certified home inspection company to verify host structure capacity.
- The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
- 4. Spans are for enclosures with mean roof heights less than 30'. For greater heights, consult engineer.
- Connections to fascia shall be limited to overhangs shown in table 1.11 or less unless site specific engineering is provided.
- 6. The proper structural name for a chair rail or top rail of an enclosure is a girt. Thus the terminology shall be interchangeable.
- Screws that penetrate the water channel of the super gutter shall have ends clipped off for safety of cleaning gutter and the heads of screws through the gutter into the fascia shall be caulked.
- 8. Section 7 contains span tables and attachment details for pans and composite panels.
- 9. When using TEK screws in lieu of S.M.S., longer screws must be used to compensate for drill head.
- 10. An additional super gutter strap or ferrule is required to be located near the midpoint of the beam spacing. Straps shall be attached to each truss / rafter tail when a 2" sub-fascia does not exist. Straps at the beam are not required when straps are placed @ each truss / rafter tail and spacing of straps does not exceed 2'-0".
- 11. Super or extruded gutter details are applicable to all widths of super or extruded gutters, and gutters may be substituted. Gutter straps and/or ferrules shall be the width of the inside and outside of the super or extruded gutter respectively. The center of the knee braces shall not be more than 6" above the top of the super or extruded gutter.
- 12. If the sub-fascia is 3/4", and the sub-fascia is in good repair, a 3/4" P.T.P. strip the width of the fascia may be added to the existing sub-fascia by attaching the plywood with (2) 16d x 3" common nails or (2) #8 x 3" screws. This gives the equivalent of a 2" fascia.
- Spans may be interpolated between values but not extrapolated outside values.
- 14. All 2" X 4" and larger purlins shall have an internal or external angle clip or screw boss to fasten the bottom of the purlin to the beam.
- 15. Load width and / or panel spacing used in determining spans / heights is measured from center to center of the members. EXAMPLE:
  - Screen panel A is 6' center to center. Screen panel B is 7' center to center. The load width of the frame member between panel A and B is (6'/2 + 7'/2) = 6.5' or 6'-6". The distance, spacing or load width is not measured between frame members as that would add 2" to the
  - load width if figured that way.
- 16. For Design Check List and Inspection Guides for Screened Enclosures, see Appendix (Section 10).
- 17. All aluminum extrusions shall meet the strength requirements of ASTM B221 after powder coating.
- 18. Other shapes than those shown in Section 8 with State Product Approvals may be used with the details of this section so long as the shapes are compatible with the details.
- All aluminum shall be ordered as to the alloy and hardness after heat treatment and paint is applied.
   Example: 6063-T6 after heat treatment and paint process.

# Section 1 Design Statement:

The structures designed for Section 1 are framing systems with screen roofs & walls and loads have been determined by wind tunnel test that include any negative internal pressure coefficient. Since these structures are open, the negative internal pressure coefficient is considered to be 0.00. The design loads used are from Chapter 20 of the 2004 Florida Building Code w/ 2006 Supplements. The loads assume a mean roof height of less than 30'; roof slope of 0° to 20°; I = 0.87 for 100 MPH and 0.77 for 110 or higher. All loads are based on 20 / 20 screen or larger. Multiply wall heights by 1.10 for members controlled by bending(b) and 1.07 for members controlled by deflection(d) when using 18 / 14 screen. All pressures shown in the below table are in PSF (#/SF). All framing components are considered to be 6063-T6 alloy. For components of 6005-T5 and 6061-T6 multiply spans by 1.13.

# General Notes and Specifications for Section 1 Tables:

# SECTION 1 Uniform Loads for Structures with Screen Roof & Walls

	Basic		Exposure 'B'		Exposure 'C'					
Wind Velocity MPH	Wind Pressure	Roofs	Windward Walls	Leeward Walls	Roofs	Windward Walls	Leeward Walls			
100	13	3	12	10	5	17	13			
110	14	4	13	9	5	18	14			
120	17	4	15	13	6	21	17			
123	18	4.3	15.9	13.3	6.3	22.2	17.6			
130	20	5	18	14	7	25	19			
1401 & 2	23	6	21	15	8	29	23			
150	26	7	24	18	9	33	27			

Loads per table 2002.4

Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

#### **Conversion Table 1A**

# Wind Zone Conversion Factors for Screen Roof or Wall Frame Members From 120 MPH Wind Zone to Others; Exposure 'B'

	Ro	ofs	Walls				
Wind Zone MPH	Applied Load #/ SF	Conversion Factor	Applied Load #/ SF	Conversion Factor			
100	3	1.15	12	1.12			
110	110 4		13	1.07			
120	4	1.00	15	1.00			
123	4.3	0.96	15.9	0.97			
130	5	0.89	18	0.91			
1401 & 2	1401 & 2 6		21	0.85			
150	7	0.76	24	0.79			

Note:

Multipliers are for wall loads only.

Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

# Conversion Table 1B Load Conversion Factors Based on Mean Roof Height from Exposure "B" to "C" & "D"

	Expos	sure "B" to	"C"	Exposure "B" to "D"						
Mean Roof Height*	Load Conversion	Span I	Multiplier	Load Conversion	Span Multiplier					
	Factor B		Deflection	Factor	Bending	Deflection				
0 - 15'			0.94	1,47	0.83	0.88				
15' - 20'	1.29	0.88	0.92	1.54	0.81	0.87				
20' - 25'	1.34	0.86	0.91	1.60	0.79	0.86				
25' - 30'	1.40			1.66	0.78	0.85				
30' - 40'	0.00		0.90	1.61	0.79	0.85				

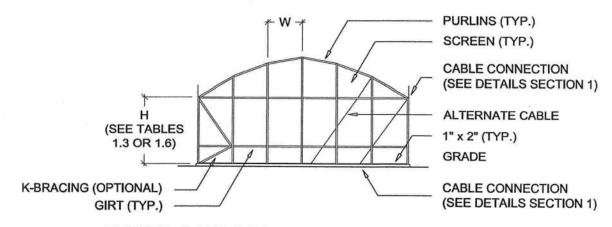
<sup>\*</sup> Use larger mean roof height of host structure or enclosure

Values are from ASCE 7-02

Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

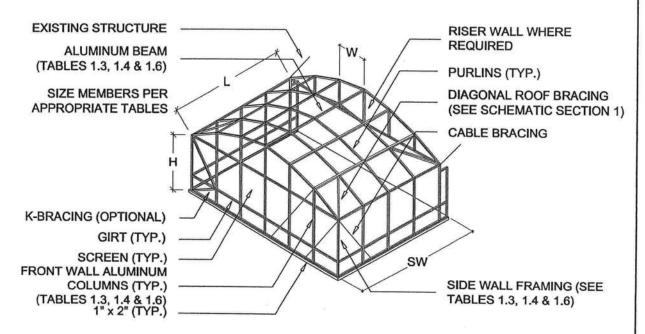
# Conversion Example (Convert span for Exposure "B" to "C"):

If max span found from span tables for Exposure "B" = 31-11" = 31.92' and the mean roof height of the structure is 0-15' then multiply span by 0.91 the span for Exposure "C" is 31.92' \* 0.91 = 29.05' = 29'-1"



# **TYPICAL DOME ROOF - FRONT WALL ELEVATION**

SCALE: N.T.S.



# **TYPICAL DOME ROOF - ISOMETRIC**

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

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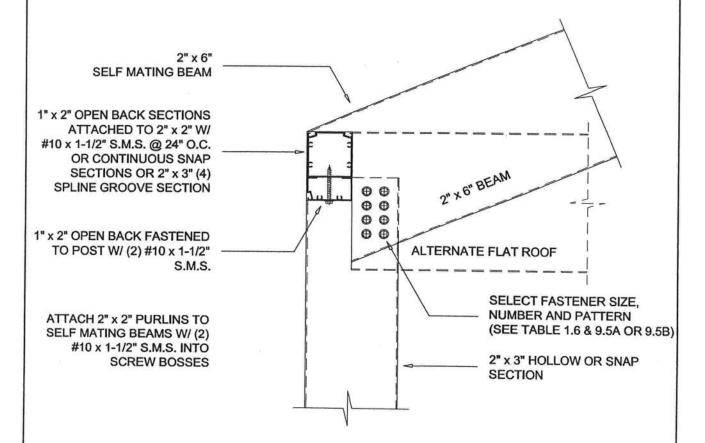
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**SECTION 1** 

MINIMUM POST SIZES REQUIRED FOR EACH BEAM SIZE (SEE TABLE 1.6)



# SLOPING BEAM TO UPRIGHT CONNECTION DETAIL (PARTIAL LAP)

SCALE: 3" = 1'-0"

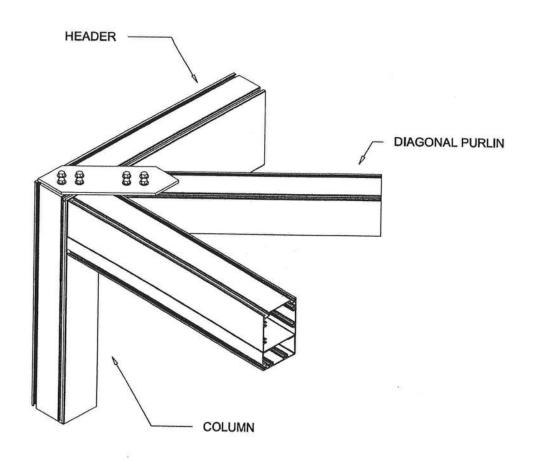
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# WIND BRACE CONNECTION DETAIL

SCALE: 3" = 1'-0"

#### NOTES:

- 1. Wind bracing shall be provided at each side wall panel when enclosure projects more than three panels from host structure. Structures of four or more panels shall be spaced for even number of panels for opposing wind bracing.
- 2. Cut brace parts with min. 12" lap of larger and smaller brace.
- 3. Cut receiving channel with angle.

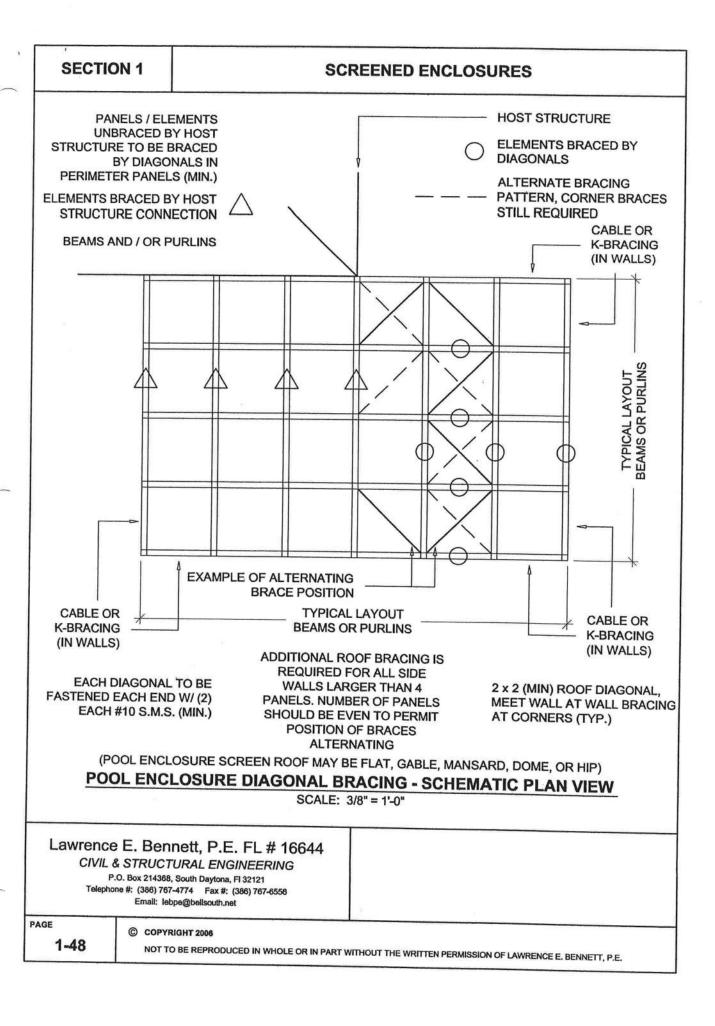
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# CABLE BRACING

# **General Notes and Specifications:**

- The following shall apply to the installation of cables as additional bracing to DIAGONAL bracing for pool enclosures:
  - a) FRONT WALL CABLES 7 x 19 STAINLESS STEEL

CABLE DIAMETER	TOTAL ALLOWABLE WALL AREA *
3/32"	233 Sq. Ft. / PAIR OF CABLES
1/8"	445 Sq. Ft. / PAIR OF CABLES

<sup>\*</sup> TOTAL WALL AREA = 100% OF FRONT WALL + 50% OF ONE SIDE WALL

EXAMPLE:

FRONT WALL AREA @ 100% (8' x 32') = 256 Sq. Ft. SIDE WALL AREA @ 50% (8' x 20') = 80 Sq. Ft. TOTAL WALL AREA = 336 Sq. Ft.

233 Sq. Ft. x 2 sets = 466 Sq. Ft. > 336 Sq. Ft.; thus two sets of 3/32" cables is required.

b) SIDE WALL CABLES - 7 x 19 STAINLESS STEEL

CABLE DIAMETER	SIDE WALL CABLE **
3/32"	ONE PER 233 Sq. Ft. OF WALL
1/8"	ONE PER 445 Sq. Ft. OF WALL

<sup>\*\*</sup> SIDE WALL CABLES ARE NOT REQUIRED FOR SIDE WALLS LESS THAN 233 Sq. Ft.

c) To calculate the required pair of cables for free standing pool enclosures use 100% of each wall area & 50% of the area of one adjacent wall.

#### NOTES:

- 1. Where wall height is such that a girt is required between the top or eave rail and the chair rail, (i.e. a mid-rise girt), then the front wall shall have two cable pairs and they shall be attached to the top rail and the mid-rise rail. If more than one additional girt is required between the top or eave rail and the chair rail, then there shall be an additional front wall cable pair at that girt also.
- 2. Side walls do not require cables until the side wall area is greater than 233 Sq. Ft.. The side wall cable may be attached at the mid-rise girt or the top rail.
- 3. Standard rounding off rules apply. ie: if the number of cables calculated is less than 2.5 pairs use two cables; if the number of cables calculated is 2.5 pairs or greater use 3 pairs of cables.
- 4. Additional roof bracing is required for all side walls larger than 4 panels. Number of panels shall be even and position shall be alternating.

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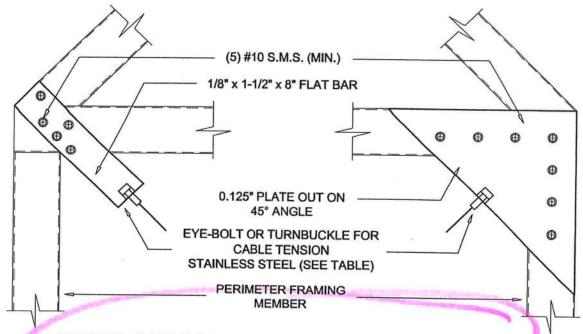
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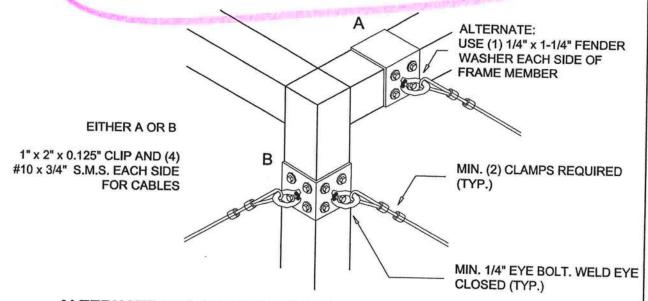
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# **SECTION 1**



# TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1 SCALE: 3" = 1'-0"



# ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

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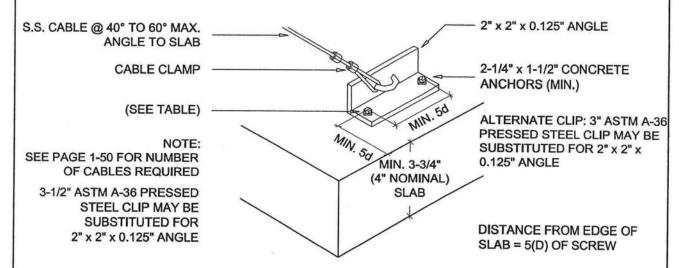
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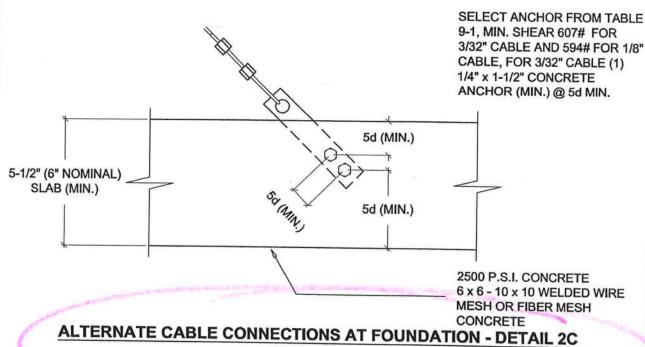
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**SECTION 1** 



# **ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B**

SCALE: 3" = 1'-0"



SCALE: 3" = 1'-0"

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# **SECTION 1 SCREENED ENCLOSURES** PURLINS ANCHORED W/ CLIPS OR #10 SCREWS THROUGH PURLINS INTO **SCREW BOSSES** 0 0 0 0 0 0 0 0 EAVE RAILS SHALL BE STITCHED W/ #10 x 1-1/2" SMS @ 6" FROM EACH END AND 24" OC MAX. GIRTS ANCHORED W/ CLIPS OR THROUGH #10 SCREWS INTO SCREW BOSSES FRONT AND SIDE BOTTOM RAILS ATTACHED TO CONCRETE W/ 1/4" x 2-1/4" CONCRETE / MASONRY ANCHORS @ PRIMARY & 1" x 2" OR 1" x 3" SECONDARY ANGLES OR @ 6" FROM EACH POST AND 24" O.C. MAX. AND WALLS MIN. 1" FROM EDGE OF CONCRETE

# **PURLIN & CHAIR RAIL DETAIL**

SCALE: 3" = 1'-0"

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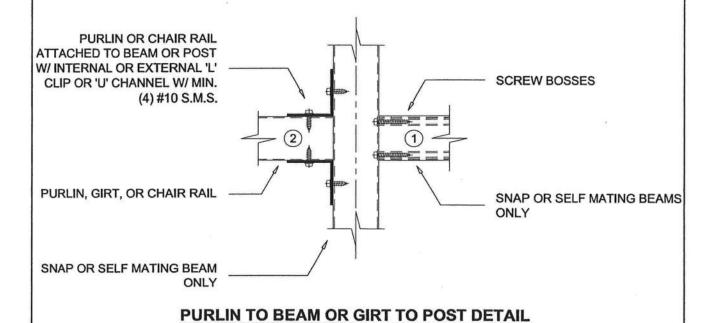
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**SECTION 1** 



TOR WALLS LESS THAN 6'-8" FROM TOP OF PLATE TO CENTER OF BEAM CONNECTION OR BOTTOM OF TOP RAIL THE GIRT IS DECORATIVE AND SCREW HEADS MAY BE REMOVED AND INSTALLED IN PILOT HOLES

SCALE: 3" = 1'-0"

OF THE CONNECTION MUST BE STRAPPED FROM GIRT TO POST WITH 0.050" x 1-3/4" x 4" STRAP AND (4) #10 x 3/4" S.M.S. SCREWS TO POST AND GIRT

IF GIRT IS ON BOTH SIDES OF THE POST THEN STRAP SHALL BE 6" LONG AND CENTERED ON THE POST AND HAVE A TOTAL (12)  $\pm$ 10 x 3/4" S.M.S.

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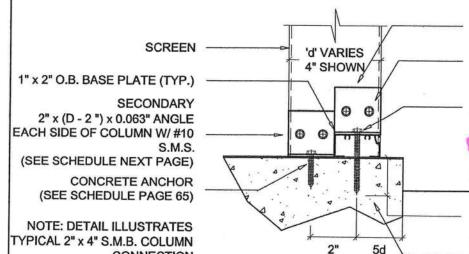
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# **SECTION 1**

# SCREENED ENCLOSURES



2" x 2" x 0.063" PRIMARY ANGLE EACH SIDE

#10 x 3/4" S.M.S. EACH SIDE (SEE SCHEDULE NEXT PAGE)

5d\* MINIMUM EDGE DISTANCE FROM EXTERIOR OF COLUMN TO OUTSIDE EDGE OF SLAB

BOLTØ	* 5d DISTANCE	4d
1/4"	1-1/4"	1"
3/8"	1-7/8"	1-1/2"

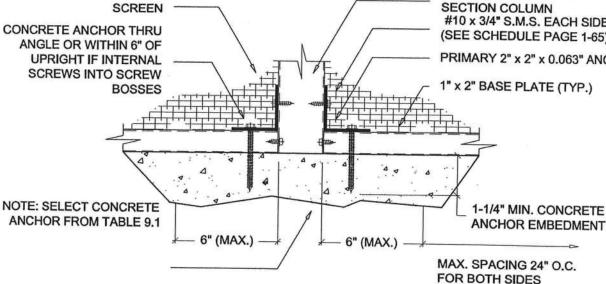
#### GRADE

1-1/4" MIN. CONCRETE ANCHOR EMBEDMENT 2500 P.S.I. CONCRETE OR ALTERNATE 2" x \_\_ WOOD DECK

TYPICAL S.M. OR SNAP SECTION COLUMN #10 x 3/4" S.M.S. EACH SIDE (SEE SCHEDULE PAGE 1-65)

PRIMARY 2" x 2" x 0.063" ANGLE

1" x 2" BASE PLATE (TYP.)



(MIN.) (MIN.)

SIDE VIEW

# **FRONT VIEW**

# 2" x 4" OR LARGER SELF MATING OR SNAP SECTION POST TO DECK DETAILS

SCALE: 3" = 1'-0"

#### NOTE:

- 1. FOR SIDE WALLS OF 2" x 4" OR SMALLER ONLY ONE ANGLE IS REQUIRED.
- 2. PREDRILL PAVERS W/ MIN. 1/4" MASONRY BIT.

CONNECTION

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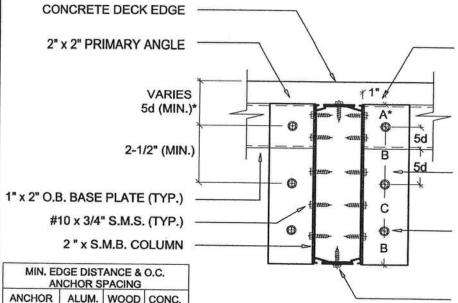
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**SECTION 1** 





SCREEN \* ABSOLUTE MINIMUM EDGE OF CONCRETE TO C.O. FASTENER = 5d

SECONDARY 2" x 2" x 0,063" ANGLE (SEE SECONDARY ANGLE ANCHOR SCHEDULE AND SECTION 9)

CONCRETE ANCHORS INTO PRIMARY AND SECONDARY **ANGLES** 

S.M.S. STITCHING SCREWS @ 24" O.C. FOR S.M.B. (SEE TABLE 1.6 FOR SIZE)

# TOP VIEW POST TO DECK DETAIL SCALE: 3" = 1'-0"

# **Primary and Secondary Anchor Schedule**

Column	Sec		y Angle		Maximum Number and Spacing Anchors											
Size	Angle	Number of Anch		nchors	chors 1/4"						16"	1	3/8"			
48	Length "L"	1/4"	5/16"	3/8"	#	"A"	"B"	"C"	#	"A"	"B"	"C"	#	T "A"	"B"	"C"
2 x 4	2"	4	4	4	4	1"	1"	1"	4	1"	1"	1"	- <del>"</del>	49	48	4"
2 x 5	3"	4	4	4	4	1"	1-1/2"	-	4	1"	1-1/2"	-	<del>-</del>	1 411	1 4 4 10 11	1"
2 x 6	4"	4	4	4	4	1"	2"	-	1	4"	2"	-	4	1-	1-1/2"	-
2 x 7	5"	6	4	4	6	1"	5/8"	1-7/8"		1 40		-	4	1"	2"	-
2 x 8	6"		1		<u> </u>	1		-	4	1"	2-1/2"	-	4	1"	2-1/2"	-
-		6	4	4	6	1"	5/8"	2-3/8"	4	1"	3"	- 1	4	1"	3"	
2 x 9	7"	6	6	4	6	1"	5/8"	2-7/8"	6	1"	13/16"	2-7/9"	1	1"	_	
2 x 10	8"	8	6	6	8	1"	5/8"	211	6	411			4	-	3-1/2"	-
xample:						-	3/0		0	1"	13/16"	3-3/16"	6	1"	3/4"	3-1/4"

5/16"

3/8"

Calculate the number of anchors required: 1.5 x beam span / 2 x beam spacing x roof wind pressure (PSF) = total #; if 1.5 x 30<sup>1</sup>/2 x 6<sup>1</sup> x 10 PSF = = 1350# and 1/4" x 1/4" Tapcon in tension @ 5d = 427# / ea. (see table 9.1)

then 1350# / 427# / ea. = 3.16 ea. use (3) ea., secondary angle not required

Actual Edge Distance Example:

2-1/2d

5/8"

25/32" 1-1/4"

4d

15/16" 1-1/2" 1-7/8"

5d

1-1/4"

1-9/16"

From edge of concrete to fastener = 2" / dia. of 0.25" = 8d

Note:

For attachment to wood deck substitute wood fasteners for concrete fasteners & calculate the required number of fasteners using tables from section 9.

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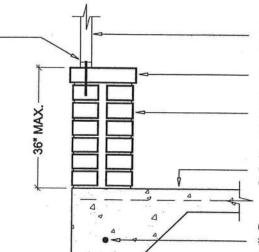
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**SECTION 1** 

1/4" x 6" RAWL TAPPER THROUGH 1" x 2" AND ROWLOCK INTO FIRST COURSE OF BRICKS

ALTERNATE CONNECTION OF SCREENED ENCLOSURE FOR BRICK OR OTHER NON-STRUCTURAL KNEE WALL 1" WIDE x 0.063" THICK STRAP @ EACH POST FROM POST TO FOOTING W/ (2) #10 x 3/4" S.M.S. STRAP TO POST AND (1) 1/4" x 1-3/4" CONCRETE ANCHOR TO SLAB OR

**FOOTING** 



ALUMINUM FRAME SCREEN WALL

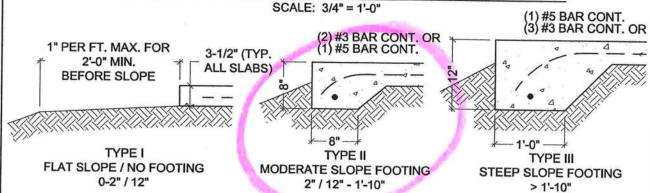
CAP BRICK

BRICK KNEEWALL TYPE 'S' MORTAR REQUIRED FOR LOAD BEARING BRICK WALL

4" (NOMINAL) PATIO CONCRETE SLAB (SEE NOTES CONCERNING FIBER MESH)

(3) #3Ø BARS OR (1) #5Ø BAR W/ 2-1/2" COVER (TYP.)

# **BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS**



# Notes for all foundation types:

- 1. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified prior to placing slab by field soil test (soil penetrometer) or a soil testing lab.
- 2. The slab / foundation shall be cleared of debris, roots and compacted prior to placement of concrete.
- 3. No footing is required except when addressing erosion until the slab width in the direction of the primary beams exceeds the span per table on page 1-69, then a type II slab is required under the load bearing wall only unless the side wall exceeds 16' in height or the enclosure is in a "C" exposure catagory in which case a type II footing is required.
- 4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 10 x 10 welded wire mesh or crack control fiber mesh; Fibermesh® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. All slabs / footings shall be allowed to cure for 7 days before installing anchors.
- 5. If local codes require a minimum footing use Type II footing or footing section required by local code. Local codes govern.

# **SLAB-FOOTING DETAILS**

SCALE: 3/4" = 1'-0"

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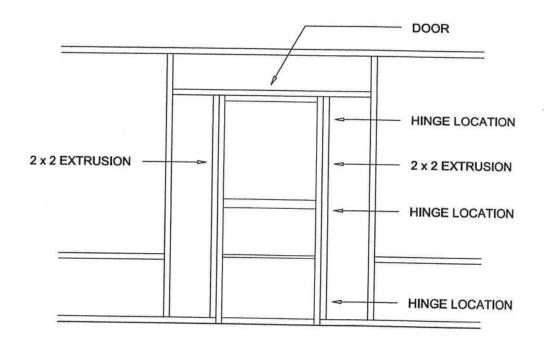
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#### NOTES:

- 1. Door to be attached to structure with minimum two (2) hinges.
- 2. Each hinge to be attached to structure with minimum four (4) #12 x 3/4" S.M.S..
- 3. Each hinge to be attached to door with minimum three (3)  $\#12 \times 3/4$ " S.M.S..
- 4. Bottom hinge to be mounted between 10 inches and 20 inches from ground.
- 5. Top hinge to be mounted between 10 inches and 20 inches from top of door.
- 6. If door location is adjacent to upright a 1"  $\times$  2"  $\times$  0.044" may be fastened to upright with #12  $\times$  1" S.M.S. at 12" on center and within 3" from end of upright.

# TYPICAL SCREEN DOOR CONNECTION DETAIL

SCALE: N.T.S.

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Table 1.1 120

Allowable Spans for Primary Screen Roof Frame Members

Aluminum Alloy 6063 T-6

For Wind Zones up to 120 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

					libutary	Load	d Width	M. =	Beam S	oacin	g										
Hollow Sections	3'-0	3'-0"		4'-0"		5'-0"		6'-0"		11	8'-0"		9'-0"								
	Allo	wabl	e Span '	L' /	Point Lo	ad (P	) or Unif	orm l	Load (U)	, ben	ding (b).	defle	ection (d	(d)							
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb		Pb	4'-5"	Pb	4'-5"	Pb	4'-5"								
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb							
2" x 2" x 0.090"	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb							
2" x 3" x 0.045"	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb							
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb							
2" x 5" x 0.062"	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-4"	Ud		Uď	-	Ud		Ud							
	Tributary Load Width 'W' = Beam Spacing											100									

	Tributary Load Width 'W' = Beam Spacing													
Self Mating Sections	3'-0"		4'-0'		5'-0"			6'-0"			8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b)									ding (b).	deflection (d)			
2" x 4" x 0.044 x 0.100"	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	_	Pd
2" x 5" x 0.050" x 0.100"	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	15'-9"	Ud		Üd
2" x 6" x 0.050" x 0.120"	20'-4"	Pd	20'-4"	Pd	20'-4"	Pd	20'-3"	Ud	19'-3"	Ud	-	Ud		Ud
2" x 7" x 0.055" x 0.120"	24'-9"	Pd	24'-9"	Pd	24'-6"	Ud	23'-1"	Ud	21'-11"	Ud	-			Ud
2" x 8" x 0.072" x 0.224"	34'-2"	Pd	32'-9"	Ud	30'-5"	Ud	28'-7"	Ud	27'-2"	Ud			24'-11"	200
2" x 9" x 0.072" x 0.224"	39'-3"	Pd	35'-11"	Ud	33'-4"	Ud	31'-5"	Ud	29'-10"	Ud	28'-6"	Ud	-	Ud
2" x 9" x 0.082" x 0.310"	42'-5"	Ud	38'-7"	Ud	35'-10"	Ud	33'-8"	Ud	31'-11"	Ud	30'-7"	Ud	-	Ud
2" x 10" x 0.092" x 0.369"	49'-3"	Ud	44'-9"	Ud	41'-7"	Ud	39'-1"	Ud	37'-2"	Ud	35'-6"	-		Ud

				7	ributary	Load	Width '	W' =	Beam Sp	acin	g			
Snap Sections	3'-0	"	4'-0		5'-0		6'-0		7'-0		8'-0	10	9'-0	"
	Allo	wabl	e Span '	L' /	Point Lo	ad (P	) or Unif	orm I	Load (U)	ben	ding (b).	defle	ection (d	)
2" x 2" x 0.044"	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	TPd
2" x 3" x 0.045"	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd		Dd	7'-6"	1 Dd
2" x 4" x 0.045"	10'-8"	Pd	10'-8"	Pd							10'-8"			Pd
2" x 6" x 0.062"	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	21'-5"	Ud	20'-5"	Ud	19'-6"	114	18'-0"	Ud
2" x 7" x 0.062"	26'-8"	Pd	26'-8"	Pd	25'-9"	Ud	24'-3"	Ud	23'-0"	Ud	22'-0"	114	241.2"	114

#### Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
- 3. Span is measured from center of beam and upright connection to fascia or wall connection.
- 4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 5. Tables are based on a maximum wall height of 16' including a 4' max, mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
- 6. Spans may be interpolated.
- 7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii. Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 9'-1"

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Table 1.3 110 Allowable Post / Upright Heights for Primary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 #/sq. ft.

			1	rib	utary Lo	ad	Width 'W	"=	Upright	Spa	cing			_
<b>Hollow Sections</b>	3'-0'	•	4'-0"		5'-0"		6'-0"		7'-0"	_	8'-0"	•	9'-0'	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	7'-5"	d	6'-5"	b	5'-8"	Ь	5'-1"	Ь	4'-8"	Ь	4'-3"	Б	3'-11"	Fb
2" x 2" x 0.050"	7'-10"	d	7'-1"	b	6'-3"	Ь	5'-8"	ь	5'-2"	ь	4'-9"	b	4'-5"	b
2" x 2" x 0.090"	8'-11"	d	8'-2"	d	7'-10"	d	7'-1"	Ь	6'-7"	ь	6'-1"	ь	5'-9"	15
2" x 3" x 0.045"	8'-4"	d	7'-7"	d	7'-9"	d	6'-11"	d	6'-5"	d	5'-11"	ĥ	5'-6"	15
2" x 4" x 0.050"	11'-2"	ь	9'-7"	Ь	8'-6"	Ь	7'-9"	ь	7'-1"	ь	6'-7"	ь	6'-1"	b
2" x 5" x 0.062"	17'-3"	b	14'-10"	b	13'-2"	b	11'-11"	b	11'-0"	b	10'-3"	Б	9'-7"	Ь

				Γrlb	utary Lo	ad \	Width 'W	/' ==	Upright	Spa	cing			
Self Mating Sections	3'-0"		4'-0"		5'-0'		6'-0"		7'-0'	-	8'-0"		9'-0"	
			Allo	wab	le Helgh	t "H	"/ bend	ilng	(b), def	lect	lon (d)	_		
2" x 4" x 0.044 x 0.100"	11'-11"	d	10'-10"	d	10'-0"	d	9'-5"	Ь	8'-8"	Ь	8'-0"	Ь	7'-6"	Тb
2" x 5" x 0.050" x 0.100"	14'-9"	d	13'-5"	d	12'-5"	d	11'-7"	Ь	10'-8"	Ь	9'-11"	ь	9'-4"	1 <sub>b</sub>
2" x 6" x 0.050" x 0.120"	17'-3"	d	15'-8"	d	14'-4"	Ь	13'-1"	Ь	12'-0"	Ь	11'-3"	ь	10'-6"	h
2" x 7" x 0.055" x 0.120"	19'-8"	d	17'-6"	b	15'-7"	Ь	14'-2"	ь	13'-1"	<u> </u>	121.22	h	11'-5"	1 h
2" x 8" x 0.072" x 0.224"	24'-4"	d	22'-1"	d	20'-6"	d	19'-4"	d	18'-4"	d	17'-6"	ă	16'-10"	12
2" x 9" x 0.072" x 0.224"	26'-8"	d	24'-3"	d	22'-6"	d	21'-2"	d	20'-1"	d	19'-3"	ď	18'-2"	<u> </u>
2" x 9" x 0.082" x 0.310"	28'-8"	d	26'-0"	d	24'-2"	d	22'-9"	d	21'-7"	d	20'-8"	d	19'-10"	a
2" x 10" x 0.092" x 0.369"	33'-3"	d	30'-3"	d	28'-1"	d	26'-5"	d	25'-1"	d	23'-11"	d	23'-1"	d

				Trib	utary Lo	ad	Width 'V	V'= (	Jpright S	Spa	cing			
Snap Sections	3'-0'		4'-0"		5'-0"		6'-0"		7'-0"	-	8'-0"	i i	9'-0'	
24 2 22 2 2 2 2 2 2 3 3 3 3 2 2 2 2 2 2			Allo	wab	e Helgh	t "H	"/ bend	ding	(b), def	lect	ion (d)			_
2" x 2" x 0.044"	6'-7"	d	5'-11"	d	5'-7"	d					4'-5"	Ь	4'-1"	Th
2" x 3" x 0.045"	8'-10"	d	8'-1"	d	7'-6"	d	6'-11"	ь		ь	5'-9"	h	5'-3"	15
2" x 4" x 0.045"	11'-2"	d	10'-2"	d	9'-2"	Ь	8'-2"	b	7'-5"	b	6'-9"	b	6'-2"	15
2" x 6" x 0.062"	18'-3"	d	16'-7"	d	15'-5"	d	14'-6"	ď		d		ā		12
2" x 7" x 0.062"	20'-7"	d	18'-9"	d	17'-5"	d		d			14'-10"			

#### Notes:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select upright length 'H'.
- Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 4. Site specific engineering required for pool enclosures over 30' in mean roof height.
- 5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
- 6. Chair ralls of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
- 7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
- 8. Spans may be interpolated.
- 9. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

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Table 1.4 110 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members

Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 # / sq. ft.

A. Sections As Horizontals Fastened To Posts With Clips

			1	rib	utary Lo	ad	Width 'W	"=	Upright S	Spa	cing			_
<b>Hollow Sections</b>	3'-0"		4'-0"	-	5'-0"		6'-0"		7'-0"		8'-0"		9'-0'	,
		-	llowable	He	lght "H'	or	Span "L'	"1	bending	(b),	deflecti	on	(d)	
2" x 2" x 0.044"	7'-5"	d	6'-5"	b	5'-8"	b	5'-1"	Ь	4'-8"	Ь	4'-3"	Тb	3'-11"	Tb
2" x 2" x 0.050"	7'-10"	d	7'-1"	b	6'-3"	b	5'-8"	Ь	5'-2"	ь	4'-9"	Ь	4'-5"	h
2" x 2" x 0.090"	8'-11"	d	8'-2"	d	7'-10"	d	7'-1"	ь	6'-7"	ь	6'-1"	b	5'-9"	15
3" x 2" x 0.045"	8'-4"	d	7'-4"	ь	6'-6"	ь	5'-10"	ь	5'-4"	b	4'-11"	ħ	4'-7"	b
3" x 2" x 0.070"	9'-5"	d	8'-6"	d	7'-9"	ь	7'-0"	b	6'-5"	b	5'-11"	h	5'-7"	15
2" x 3" x 0.045"	8'-4"	d	7'-7"	d	7'-9"	d	6'-11"	d	6'-5"	ď	5'-11"	h	5'-6"	늉
2" x 4" x 0.050"	11'-2"	Ь	9'-7"	b	8'-6"	h	7'-9"	h	7'-1"	h	6'-7"	냚	6'-1"	Б
2" x 5" x 0.062"	17'-3"	ь	14'-10"	Ь	13'-2"	ь	11'-11"	b	11'-0"	ь	10'-3"	6	9'-7"	6

			Trib	utary Lo	oad	Width '\	N'=	Upright \$	Spa	cing			
3'-0'	•	4'-0'	• 1	5'-0'	•	6'-0	**	7'-0"	,	8'-0		9'-0	**
	A	llowabl	e He	ight "H	or	Span "L	."1	bending	(b)	deflect	ion	(d)	
6'-7"	d	5'-11"	d	5'-7"	d	5'-3"	Id	4'-10"	b	4'-5"	b	4'-1"	Th
		3'-0" A	3'-0" 4'-0' Allowabl	3'-0" 4'-0" Allowable He	3'-0" 4'-0" 5'-0' Allowable Height "H'	3'-0" 4'-0" 5'-0" Allowable Height "H" or	3'-0" 4'-0" 5'-0" 6'-0 Allowable Height "H" or Span "I	3'-0" 4'-0" 5'-0" 6'-0" Allowable Height "H" or Span "L" /	3'-0" 4'-0" 5'-0" 6'-0" 7'-0' Allowable Height "H" or Span "L" / bending	3'-0" 4'-0" 5'-0" 6'-0" 7'-0"  Allowable Height "H" or Span "L" / bending (b),	Allowable Height "H" or Span "L" / bending (b), deflect	3'-0" 4'-0" 5'-0" 6'-0" 7'-0" 8'-0"  Allowable Height "H" or Span "L" / bending (b), deflection	

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

		127		Trib	utary Lo	ad '	Width 'W	/' =	Upright :	Spa	cing			
<b>Hollow Sections</b>	3'-0'		4'-0'		5'-0'		6'-0"		7'-0"	<b>WEST COLUMN</b>	8'-0"	1	9'-0'	_
		-	llowable	e He	lght "H'	or	Span "L	"1	bending	(b)	deflecti	on (		_
3" x 2" x 0.045"	9'-7"	Ь	8'-3"	Ь	7'-3"	b					5'-6"	Ть	5'-1"	Tb
3" x 2" x 0.070"	11'-5"	b	9'-10"	Ь	8'-8"	b	7'-10"	Ь		ь	6'-8"	b	6'-3"	늄
2" x 3" x 0.045"	11'-2"	d	9'-9"	ь	- I was a second and the second and	_	7'-10"	_		b	6'-8"	b	6'-2"	15
2" x 4" x 0.050"	12'-6"	Ь	10'-9"	b		ь		-	7'-11"	b	7'-4"	h	6'-10"	15
2" x 5" x 0.062"	19'-3"	b	16'-7"	b					12'-4"	ь	11'-6"	Б	10'-9"	늄

		Tributary Load Width "W"= Upright Spacing												_
Snap Sections	3'-0"		4'-0'		5'-0"		6'-0"		7'-0"		8'-0"		9'-0	
		A	llowabl	e He	ight "H"	or	Span "L"	ī	bending	(b).	deflection	on	(d)	_
2" x 2" x 0.044"	8'-10"	d	7'-8"	b	6'-9"	b	6'-0"	ь	5'-5"	Ь	4'-11"	Ь	4'-7"	Ti

#### Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040",
- 2. Using screen panel width 'W' select girt lengths.
- 3. Site specific engineering required for pool enclosures over 30' in mean roof height.
- 4. Span/height is to be measured from center of beam and upright connection to fascia or wall connection.
- 5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential gardrails provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8'-0" o.c.
- 6. Girt spacing shall not exceed 6'-8".
- 7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
- 8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
- 9. Spans/heights may be interpolated.
- 10. To convert spans/heights to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

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Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams To Wall Uprights or Beam Splicing

Beam/Upright	Upright or	Minimum Purlin, Girt	Notes	Minimu	ım Number of	Screws*	Beam Stitching
or Post	Post/Beam	& Knee Brace Size		#8 x 1/2"	#10 x 1/2"	#12 x 1/2"	Screw at 24" OC
2 x 4 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#10
2 x 5 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#8
2 x 6 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	10	8	6	#10
2 x 7 SMB	2 x 4 SMB or H	2" x 3" x 0.044"	Full Lap	14	12	10	#12
2 x 8 SMB	2 x 5 SMB or H	2" x 3" x 0.044"	Full Lap	16	14	12	#14
2 x 9 SMB	2 x 6 SMB	2" x 3" x 0.045"	Full Lap	18	16	14	#14**
2 x 9 SMB *	2 x 7 SMB	2" x 4" x 0.050"	Full Lap	20	18	16	#14**
2 x 10 SMB	2 x 8 SMB	2" x 5" x 0.050"	Full Lap	20	18	16	#14**

Screw Size	Minimum Distance and	Spacing of Screws	Gusset Plate Thic	ckness
	Edge To Center	Center To Center	Beam Size	Thickness
#8	5/16"	5/8"	2" x 7" x 0.055" x 0.120"	0.063"
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	0.125"
#12	1/2"	1"	2" x 9" x 0.072" x 0.224"	0.125"
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0306"	0.190"
5/16"	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	0.250"
3/8"	1"	2"		0.200

<sup>\* 0.082&</sup>quot; wall thickness, 0.310" flange thickness

#### Connection Example:

2" x 7" beam & 2" x 5" at beam & gusset plate, (14) #8 x 1/2" sms & upright & gusset plate

#### (14) #8 x 1/2" sms ea. side of beam & upright.

- 1. Connection of 2" x 6" to 2" x 4" shall use a full lap cut or 1/16" gusset plate.
- 2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
- 3. The number of screws is based on the maximum allowable moment of the beam.
- 4. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
- 5. Hollow splice connections can be made provided the connection is approved by the engineer.
- 6. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of
- 7. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
- 8. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.224" s.m.b. w/ 2" x 6" x 0.050 x 0.120" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)

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<sup>\*\* (1)</sup> Stitching screw at 16" O.C. max.

Table 1.7

Minimum Size Screen Enclosure Knee Braces and Anchoring Required

Aluminum 6063 T-6

Brace Length*	Extrusion	Anchoring System
0' - 2'-0"	2" x 2" x 0.044"	2" H-Channel With (3) #10 x 1/2" each leg of channel
To 3'-0"		2" H-Channel With (3) #10 x 1/2" each leg of channel
To 6'-0"	2" x 4" x 0.044" x 0.125"	2" H-Channel With (4) #10 x 1/2" each leg of channel

<sup>\*</sup> Knee brace length shall be the horizontal and vertical length @ a 45° angle from the center of the connection to the face of the beam or upright.

Note:

- 1. For required knee braces greater than 4'-6" contact engineer for specifications and details.
- Cantilever beam detail shown on page 1-40 shall be used for transom wall to host structure attachment when knee brace length exceeds 6'-0".

Table 1.8 K-Bracing Fastening Schedule

		Number	of #10 x 3/4" S.M.S.	Required	
Maximum Wall Width =	Corner Post @ Top	Diagonals (K) per End	Intermediate Post @ Chair Rall	Corner Post	Plate to Sole Plate
20'-0"	2	2	4	2	2
30'-0"	2	2	4	2	2
40'-0"	3	4	6	2	2
50'-0"	4	5	8	3	2
60'-0"	6	7	12	3	3

Use front wall width when determining number of s.m.s. for the side wall K-bracing.

Use side wall width when determining number of s.m.s. for the front and / or back wall K-bracing.

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# NOTICE OF COMMENCEMENT

PERMIT NUMBER:
STATE OF: FLORIDA COUNTY OF: Columbia CITY OF: Lake City
THE UNDERSIGNED HEREBY gives notice that improvement/s) will be made to cortain seed and
with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.
DESCRIPTION OF PROPERTY
LOT: 53 BLOCK: SECTION: TOWNSHIP: RANGE:
TAN PARCEL NUMBER: 10 - 105 - 110 - 103 - 115 - 152 11V
SUBDIVISION: CAIQING FORMS
STREET ADDRESS: 4185W Hilliam Terr.
SENEIGHE DESCRIPTION OF IMPROVEMENTS.
TO CONSTRUCT: Pool Enclosure
OWNER INFORMATION.
OWNER NAME: William / Bonnie Robbins
ADDRESS: 418 5.14) Hillton Terr
SIAIE TU ZID CODE. A 2/4 ZIV
Ft white
INTEREST IN PROPERTY:
FEE SIMPLE TITLEHOLDER NAME:
FEE SIMPLE TITLEHOLDER ADDRESS:
(if other than owner)
CONTRACTOR NAME: Vince Richardson / Richardson Aluminum LLC.  ADDRESS: 492 SW Arlington Blvd PHONE NUMBER: 386-755-5779  CITY: Lake City STATE: Fla ZIP CODE: 32025  BONDING COMPANY:
ADDRESS: PHONE NUMBER:
STATE:ZIP CODE·
LENDER NAME:
ADDRESS:PHONE NUMBER:
CITY: STATE: ZIP CODE:
Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as
, and the control of
NAME: ADDRESS: In addition to himself. Owner designators
o mor designates
to receive a copy of Lienor's Notice as provided in Section 743 40/4/4
Expiration date is one (1) year from date of recording unless a different date is specified.
SIGNATURE OF OWNER: Wall
SWORN to and subscribed by
SWORN to and subscribed before me this 27 day of May , A.D. 2004, 2009
Notary Public: Columbia Danela Wellow SANDRA H. TILLOTSON
My commission Expires: Jan 31 2011  **SANDRA H. TILLOTSON MY COMMISSION # DD 629530 EXPIRES: January 31, 2011 Bonded Thru Budget Notary Services



# **COLUMBIA COUNTY, FLORIDA**

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Hea Classification	Parcel Number
Hea Classification SCREEN ENCLOSHRE	Parcel Number 10-6S-16-03815-153
Eiro: 0.00	Building permit No.
	No. 000027846

Permit Holder VINCE RICHARDSON Waste:

Location: 418 SW HILLTOP TERR, FT. WHITE, FL

Date: 06/05/2009

Owner of Building WILLIAM & BONNIE ROBBINS

Total:

0.00

**POST IN A CONSPICUOUS PLACE** (Business Places Only)

**Building Inspector**