

**CONNECTOR TABLE**

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600	-	-
400	290	H3	4-131"x1 1/2"	4-131"x1 1/2"
625	540	H2.5A	5-131"x1 1/2"	5-131"x1 1/2"
1040	1015	H10A	9-148"x1 1/2"	9-148"x1 1/2"
645	515	LTS12-20	6-148"x1 1/2"	6-148"x1 1/2"
990	850	MTS12-30	7-148"x1 1/2"	7-148"x1 1/2"
1415	1215	HTS16-30	8-148"x1 1/2"	8-148"x1 1/2"
1235	1235	LSTA21	6-148"x1 1/2"	6-148"x1 1/2"
1640	1460	MSTA24	9-148"x1 1/2"	9-148"x1 1/2"
1030	1030	CS20	7-148"x1 1/2"	7-148"x1 1/2"
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
555	535	SP1	4-148"x3"	4-148"x3"
1100	905	SP2	6-148"x3"	6-148"x3"
1280	1100	SP4@8	12-148"x1 1/2"	wrap under or over plate
771	771	LSTA24	10-148"x1 1/2"	wrap under or over plate
1235	1235	LSTA24	14-148"x1 1/2"	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemmwall	To Stud / Post	Anchor
2145	1835	DTT22	6-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-162"x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Stemmwall	To Post	Anchor
1900	1900	ABU44Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
2475	1900	ABU6Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2145	1935	DTT22	6-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	18-162"x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900	1900	ABU44Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
2475	1900	ABU6Z	12-162"x3 1/2"	5/8"x12" Drill & Epoxy

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

**THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR. RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)**

UD	UD	UD	UD
(1) 2x4 @ 16" O.C.	(1) 2x4 @ 12" O.C.	(1) 2x6 @ 16" O.C.	(1) 2x6 @ 12" O.C.
(1) 2x4 @ 16" O.C.	(1) 2x4 @ 12" O.C.	(1) 2x6 @ 16" O.C.	(1) 2x6 @ 12" O.C.

**GRADE & SPECIES TABLE**

Grade	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**SILL PLATE SPANS FOR 10'-0" WALL HEIGHT**

DESIGN WIND SPEED	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED WITH THE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO CONSULT TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END. 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMpressive STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI. WELDED WIRE REINFORCED SLAB: 8" x 6" W/ 4 # 4, F<sub>y</sub> = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.F.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 1.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C1119. SUPPLIER TO PROVIDE ASTM C1119 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.N.C.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, UNIONS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROTTED CMU.

**BUILDER'S RESPONSIBILITY:**

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FIBER REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED. LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**DESIGN CRITERIA & LOADS:**

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
<b>WINDLOADS</b>	
BASIC WIND SPEED (ASCE 7.22, 38 GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C&amp;C DESIGN PRESSURES:</b> SEE TABLE	
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOMS	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
<b>SOIL BEARING CAPACITY</b>	1500 PSF
<b>FLOOD ZONE</b>	THIS BUILDING IS NOT IN THE FLOOD ZONE

**COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)**

EFFECTIVE WIND AREA (FT <sup>2</sup> )	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Va/d)	-27.8(Va/d)
0 - 20	+42.6(Va/d)	-46.2(Va/d)
		+25.6(Va/d) -34.2(Va/d)

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**JOB NUMBER:**  
260123

**S-1**  
OF 2 SHEETS

Bryan Zecher Homes, Inc.

**HASKEW POOL HOUSE**

PROJECT ADDRESS:  
SW Custom Made Circle  
Lake City, Florida

Mark Disoway FL PE 53915

This item has been digitally signed and sealed by Mark Disoway P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

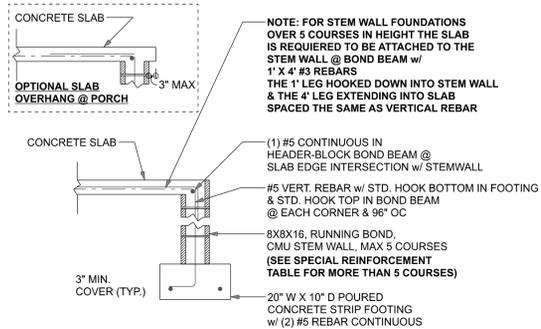
**TALL STEM WALL TABLE:**  
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96

**THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12', CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING**

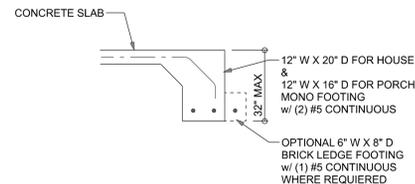
**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI/ASCE Section	Specific Requirements
1.4A	Compressive strength 8" block bearing walls $F_m = 1500$ psi
2.1	Mortar ASTM C 270, Type N, UNO
2.2	Grout ASTM C 476, admixtures require approval
2.3	CMU standard ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 10"x16" column block
2.3	Clay brick standard ASTM C 216-02, Grade SW, Type FBS, S <sub>1</sub> S <sub>2</sub> 7.5"x11.12"
2.4	Reinforcing bars, #3 - #11 ASTM 615, Grade 40, $F_y = 40$ ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection Joint reinforcement in walls exposed to moisture or wet ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

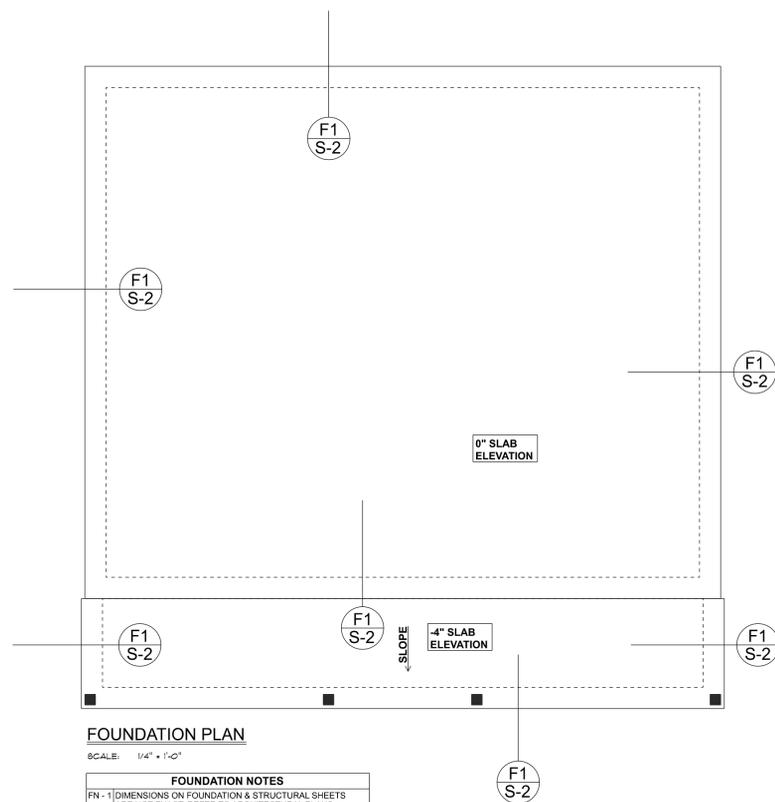


**F1 S-2** OPTIONAL STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



**F1 S-2** MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"

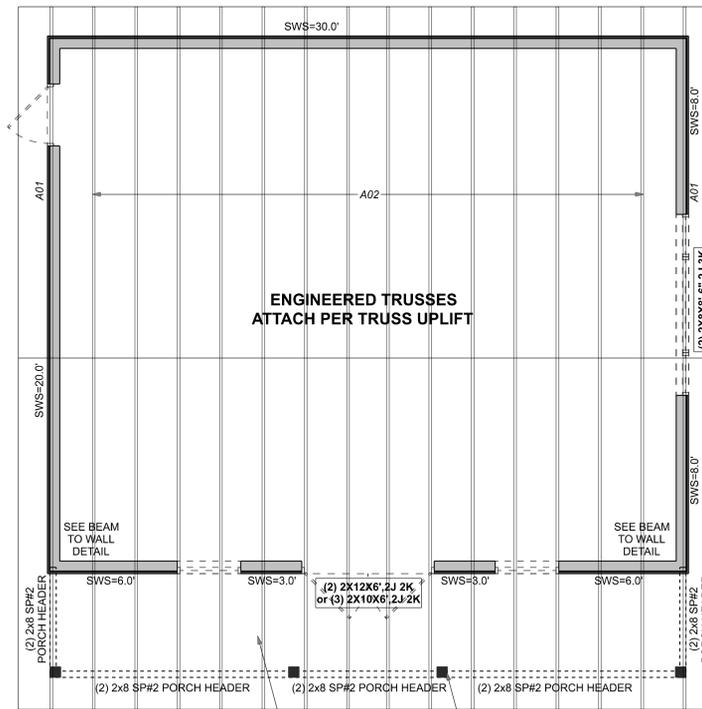


**FOUNDATION NOTES**

FN-1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

FN-2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.

FN-3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ (0.6x) 1.41:4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITIC-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITIC-TREATMENT METHOD CAN BE USED INSTEAD).



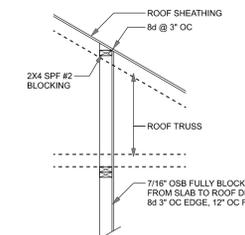
**STRUCTURAL PLAN NOTES**

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**ACTUAL vs REQUIRED SHEARWALL**

	TRANSVERSE	LONGITUDINAL
ACTUAL	14256 LBF	19008 LBF
REQUIRED	9786 LBF	6979 LBF

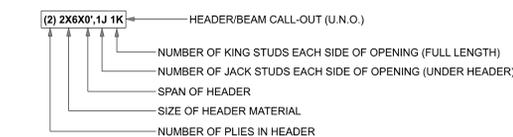
CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND CO. JOB #26-3450



**UNLESS NOTED OTHERWISE (MINIMUM REQUIERMENTS) \*\*\*SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS\*\*\***

BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X8 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

**HEADER LEGEND**



Bryan Zecher Homes, Inc

HASKEW POOL HOUSE

PROJECT ADDRESS:  
SW Custom Made Circle  
Lake City, Florida

Mark Disosway FL PE 53915  
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**S-2**  
OF 2 SHEETS