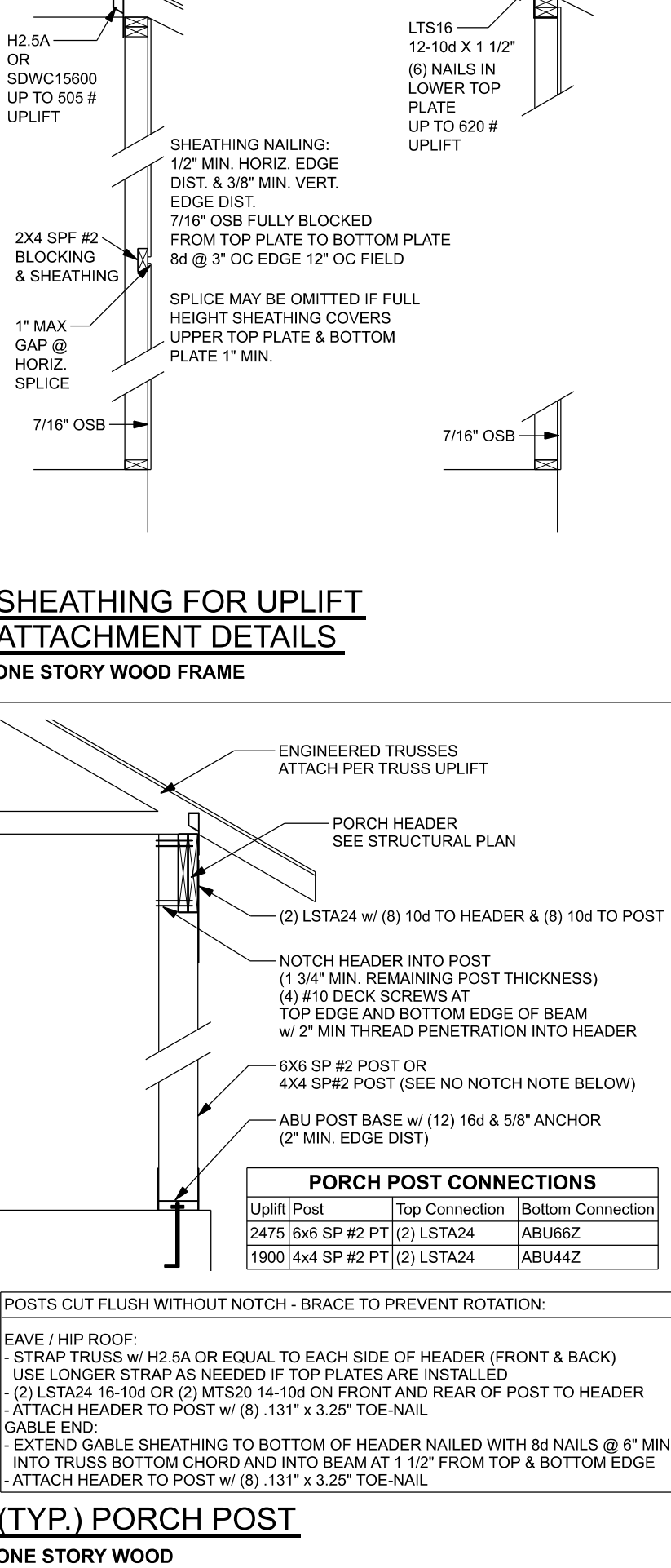
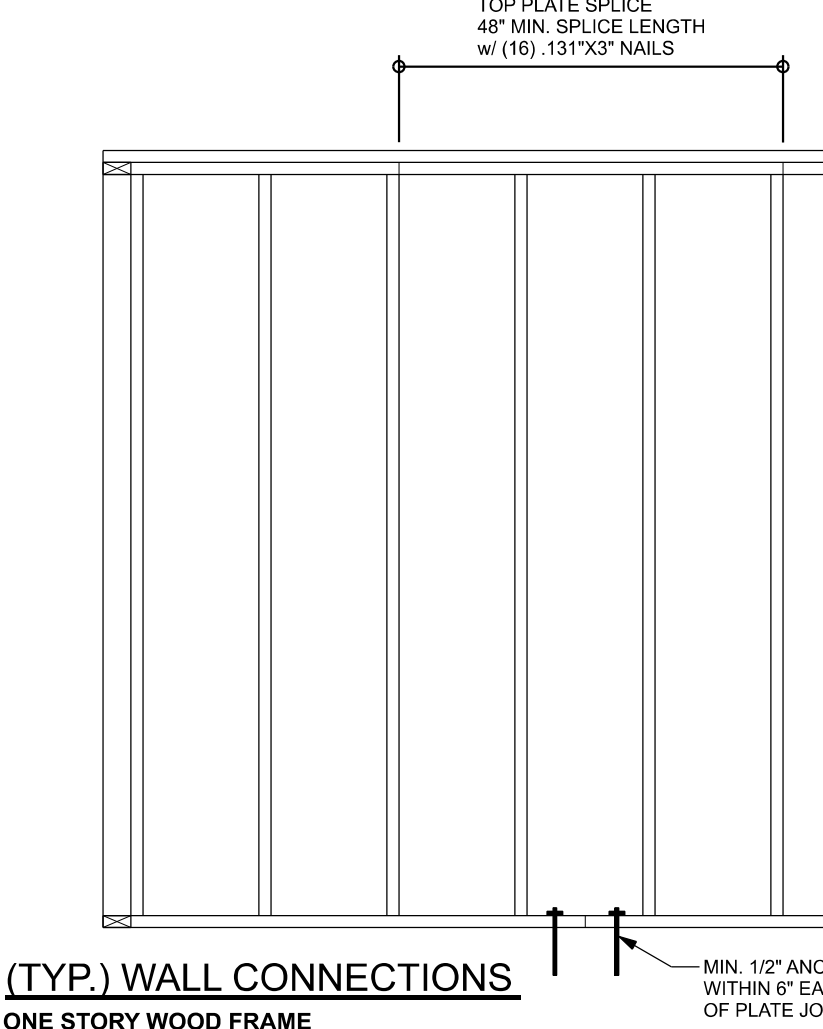
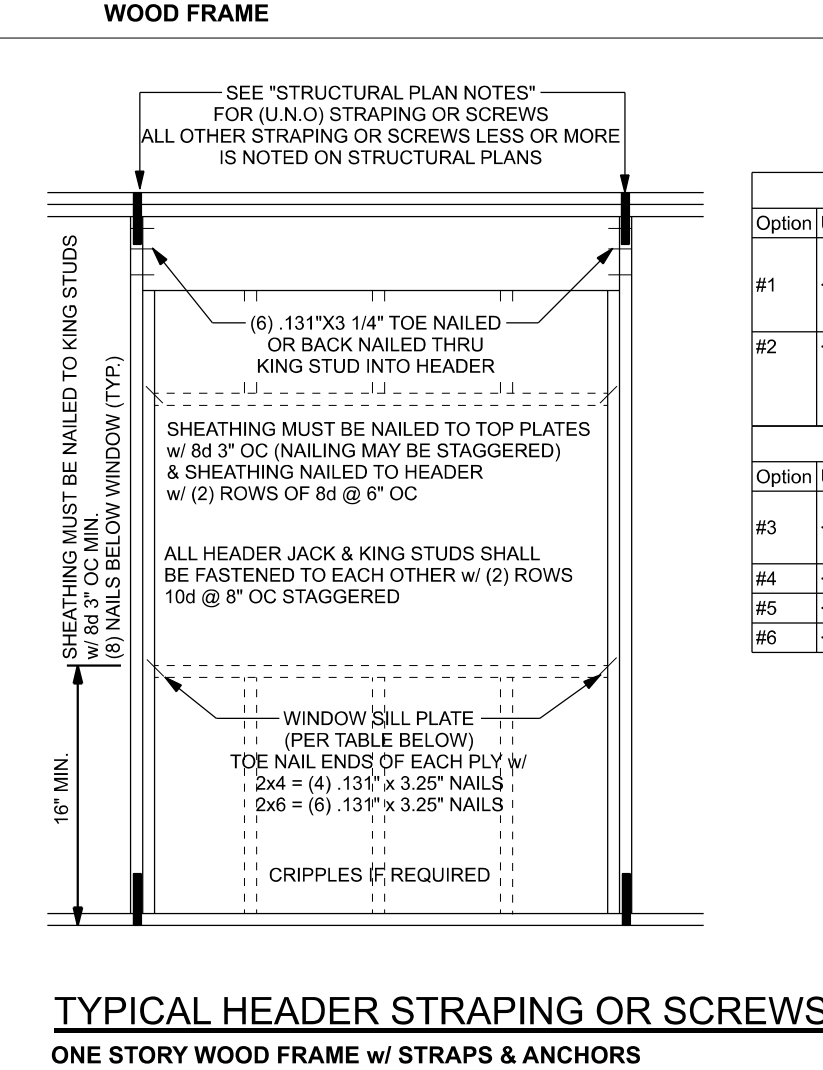
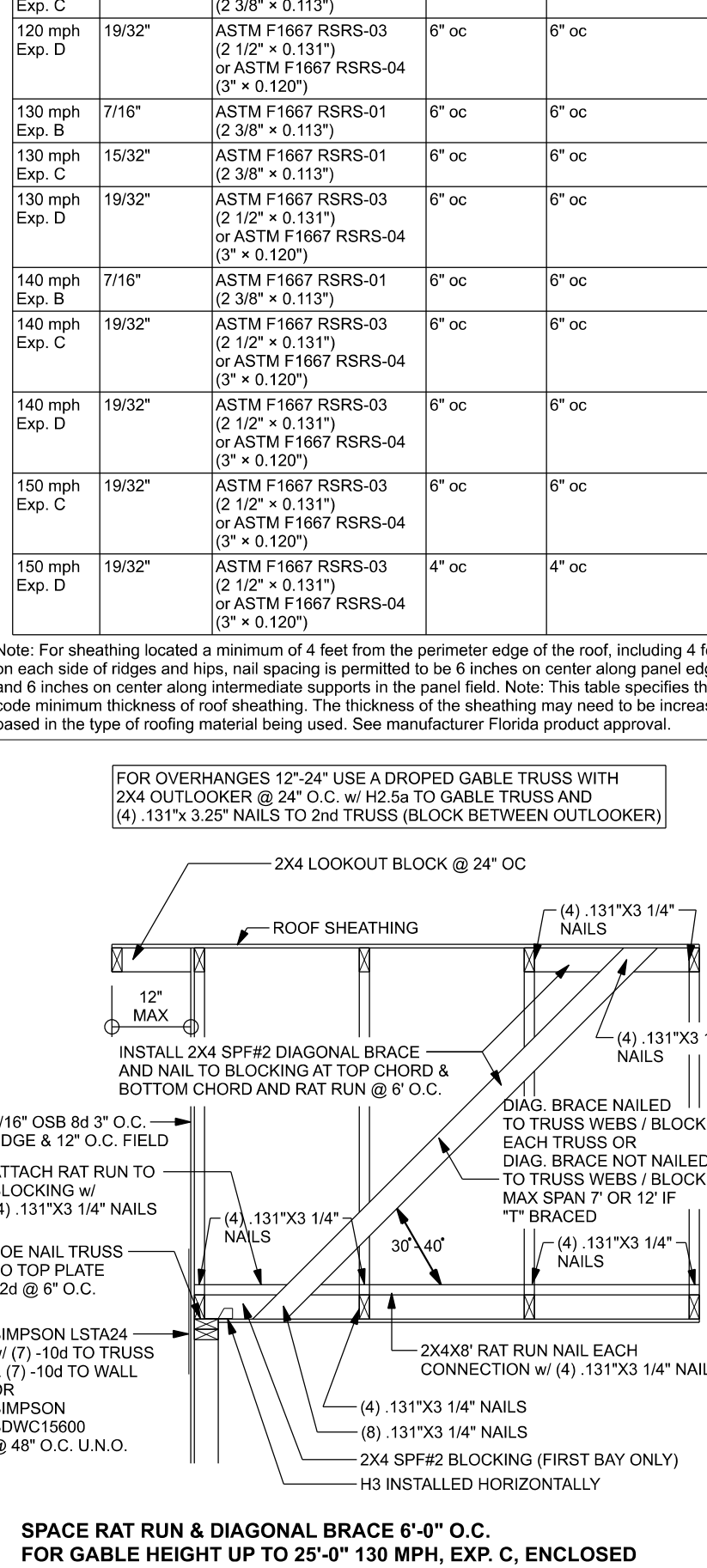
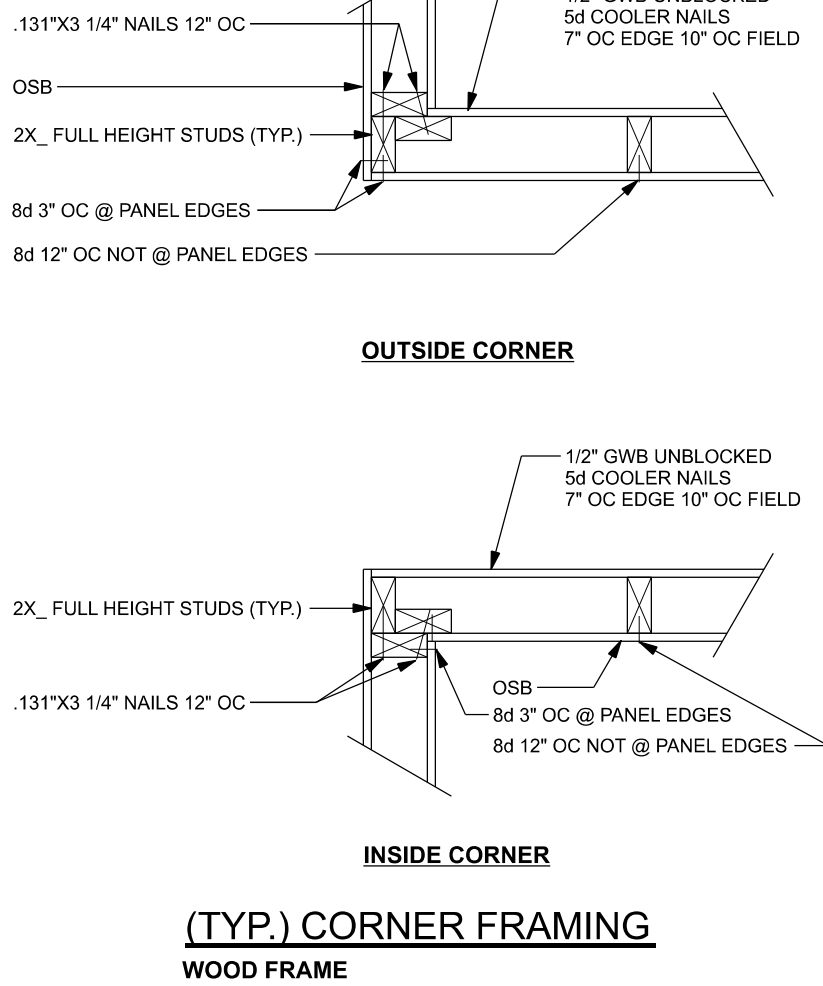
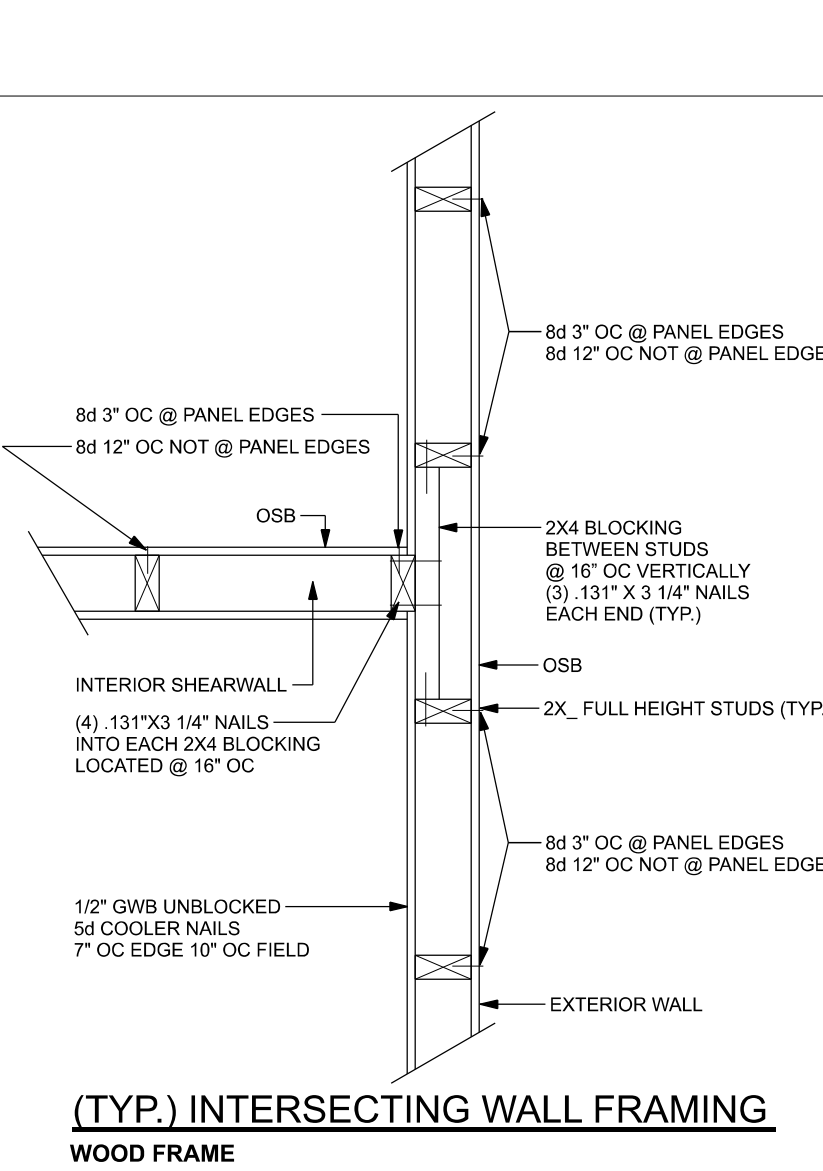
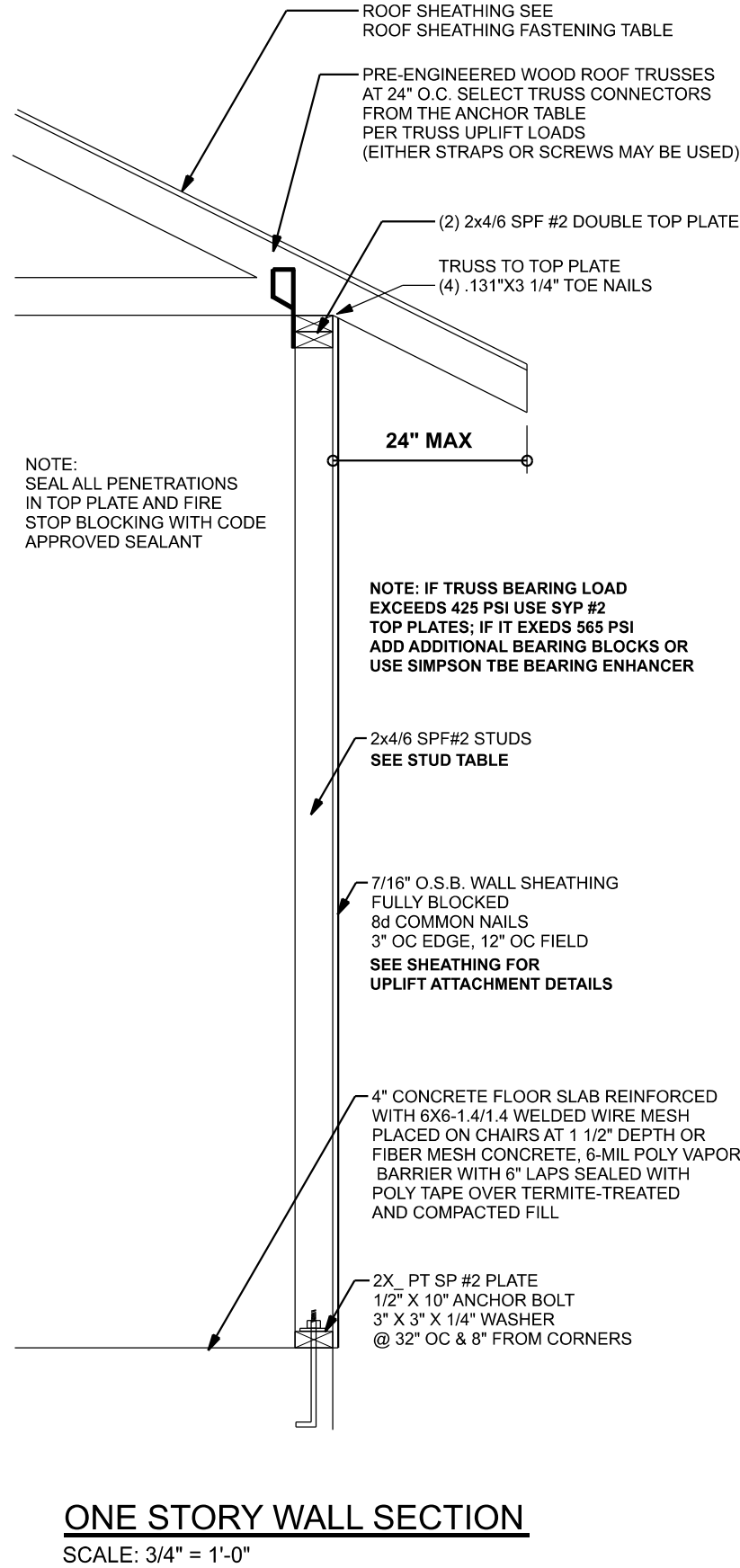




HEADER SCREWS TABLE

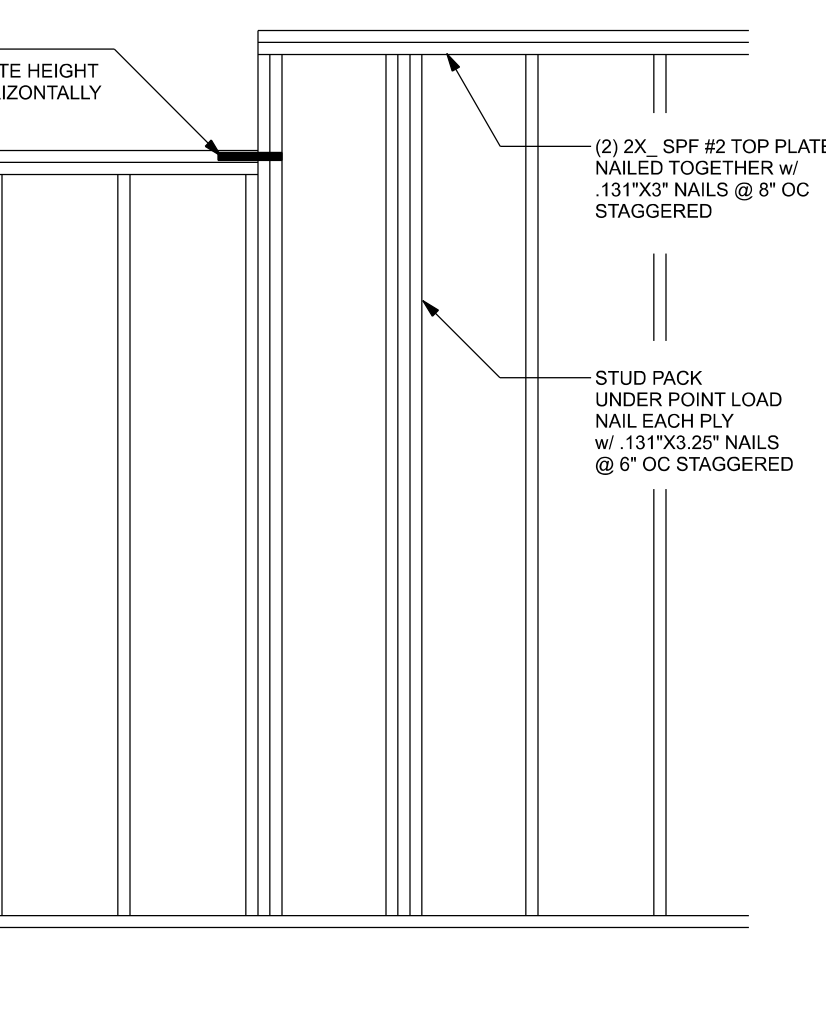
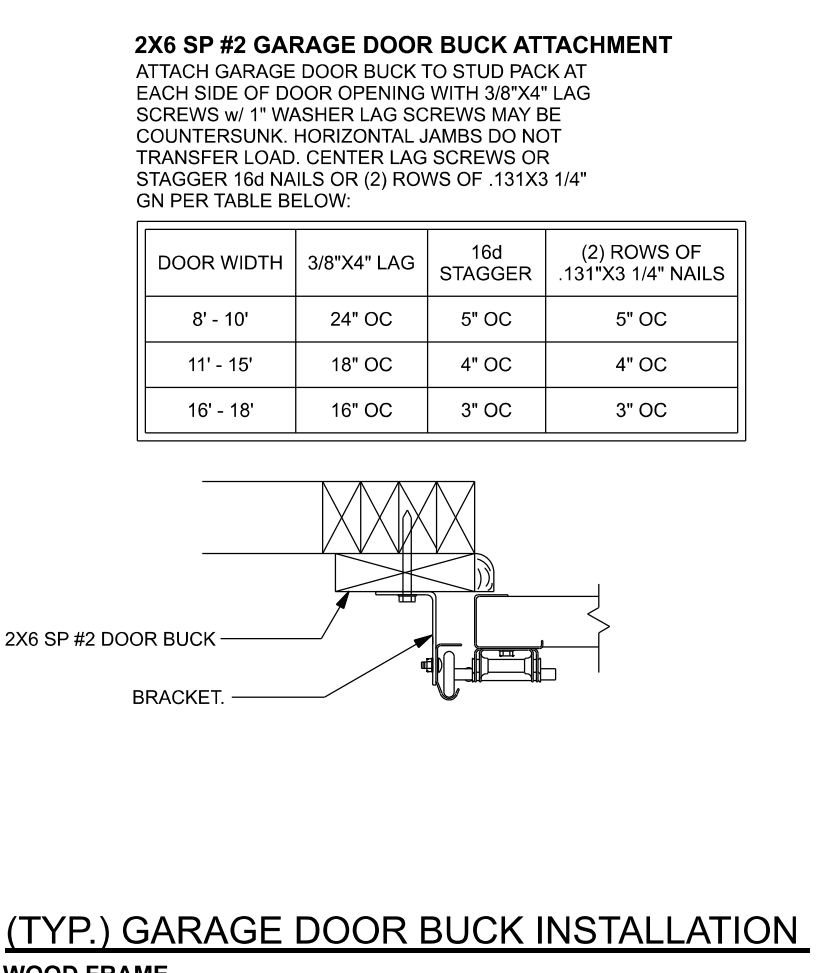
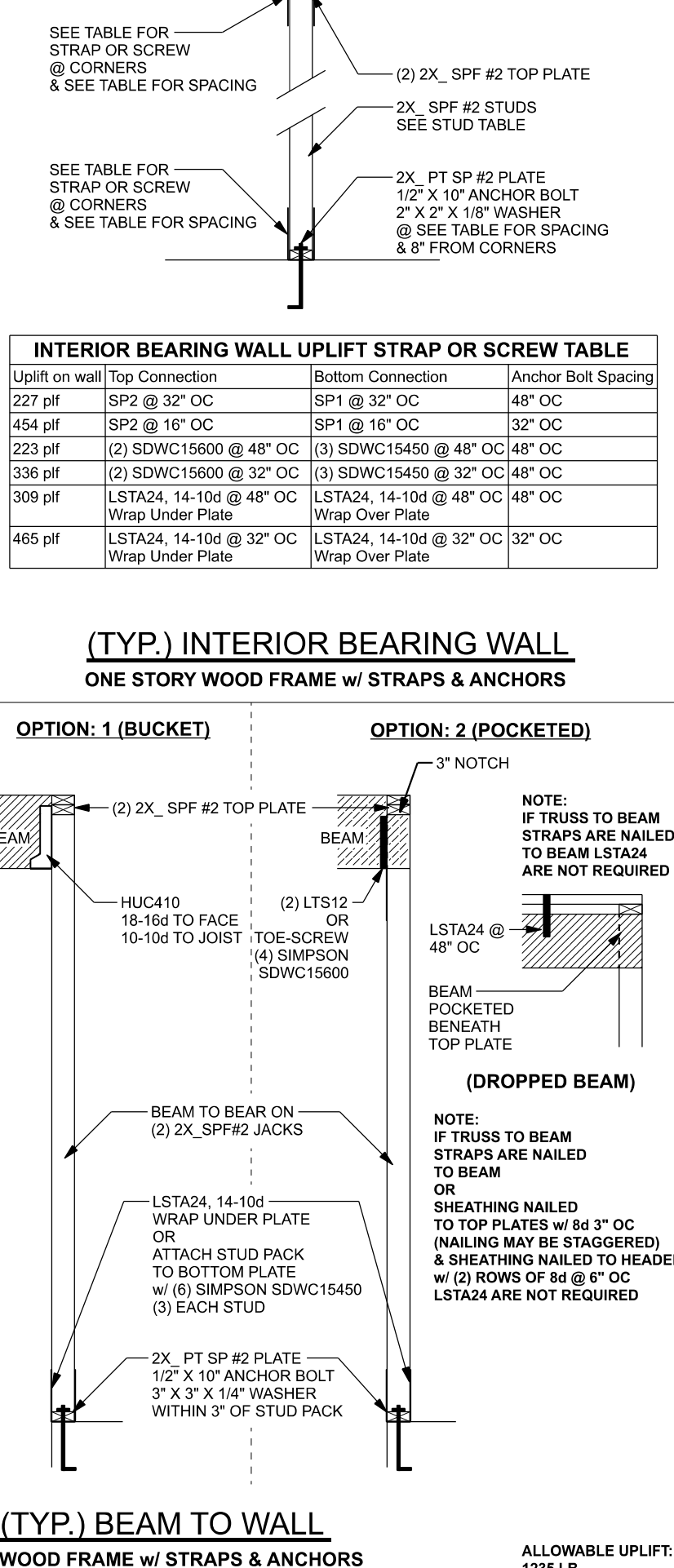
Option	Uplift	Top Connection	Bottom Connection
#1	< 510	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450
#2	< 895	Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450

HEADER STRAP TABLE

Option	Uplift	Top Connection	Bottom Connection
#3	< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
#4	< 1455	MSTA24, 18-10d header to studs	DTT22
#5	< 1800	MSTA24, 18-10d header to studs	DTT22
#6	< 2910	MSTA24, 18-10d header to studs	HTT4

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-2.3
WIND SPEED	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6	
130 MPH EXP. C	5'-2" 7'-9" 7'-7" 11'-3"	



720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP		Uplift SPF	Strap Ties	To One Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP		Uplift SPF	Stud Plate Ties	To Stud
585	535	SP1	6-10d	4-10d
1065	905	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP		Uplift SPF	Holdowns @ Stewall	To Stud / Post
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16d12 1/2"	1/2"x12" Titen HD
Uplift SP		Uplift SPF	Holdowns @ Mono	To Stud / Post
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16d12 1/2"	1/2"x12" Titen HD
Uplift SP		Uplift SPF	Post Bases @ Stewall	To Post
1900	1900	ABU4Z	12-16d	5/8"x12" Drill & Epoxy
2475	2475	ABU6Z	12-16d	5/8"x12" Drill & Epoxy
Uplift SP		Uplift SPF	Post Bases @ Mono	To Post
1900	1900	ABU4Z	12-16d	5/8"x12" Drill & Epoxy
2475	2475	ABU6Z	12-16d	5/8"x12" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD LOAD TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WIND LOADS, 150 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR ZONE FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXCEPT 16" O.C. x 0.8 x 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, TRUSS LAYOUT, TRUSS BRACING, TRUSS CONNECTIONS, TRUSS REACTION LOADS, TRUSS BRACING LOCATIONS, TRUSS BRACING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE BUILDER SHALL FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN AND SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F8 x 8KSI, WELED WIRE REINFORCEMENT FABRIC W/ W14 CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.175 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ENGINEER'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI. ALL LAP SPLICES 40" DB (25' FOR 45 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-05, UNO.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE SITED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN ONITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, TRUSS LAYOUT, TRUSS BRACING, TRUSS CONNECTIONS, TRUSS REACTION LOADS, TRUSS BRACING LOCATIONS, TRUSS BRACING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE BUILDER SHALL FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



Bryan Zecher Homes, Inc.
Deborah Taylor Res.
PROJECT ADDRESS:
01-35-16-00127-013
COLUMBIA COUNTY, FLORIDA

FL PE 53915
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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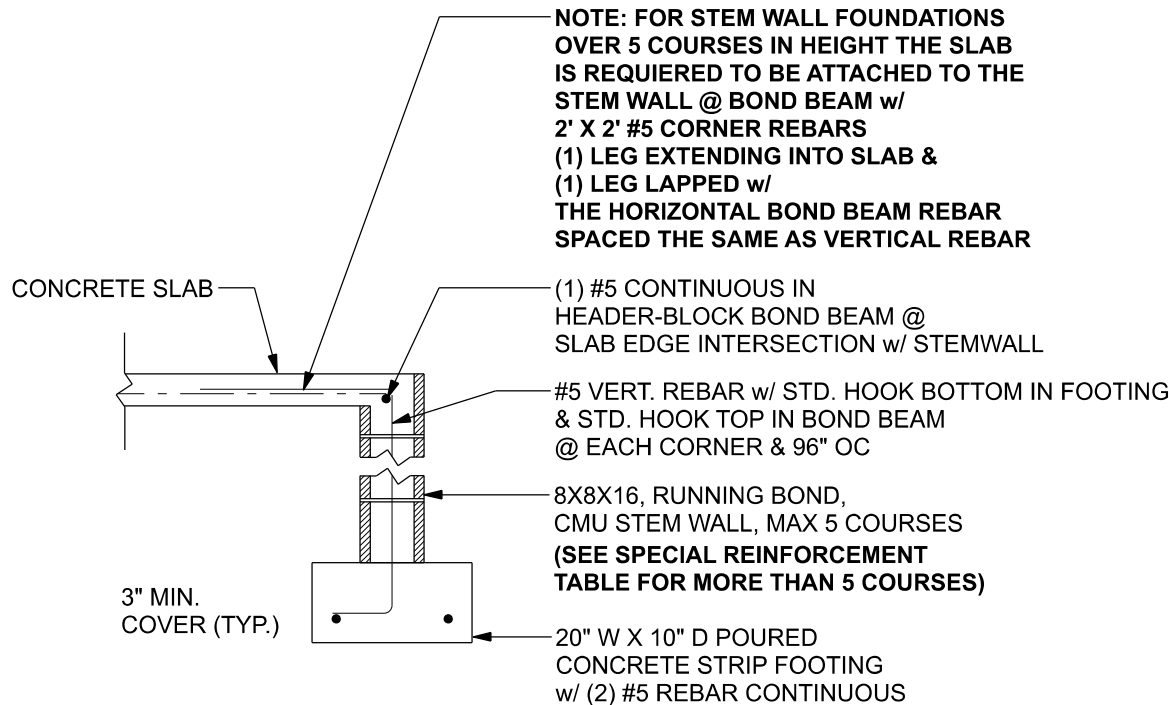
JOB NUMBER:
241023
S-1
OF 3 SHEETS

TALL STEM WALL TABLE: The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.							
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

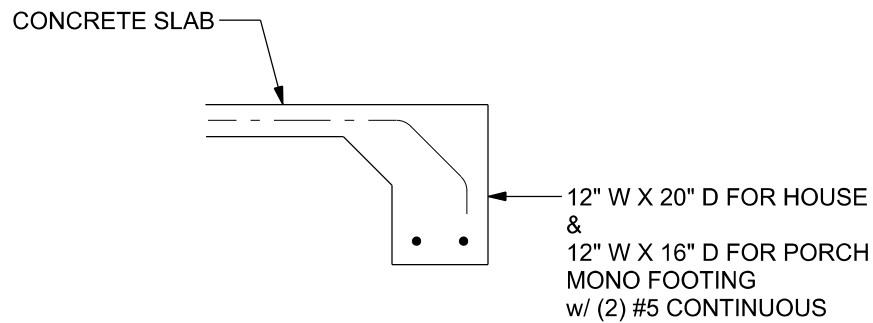
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, F_y = 40 ksi. Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A325, Class 500, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

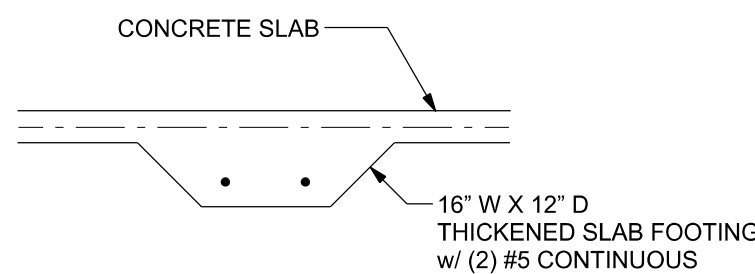
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



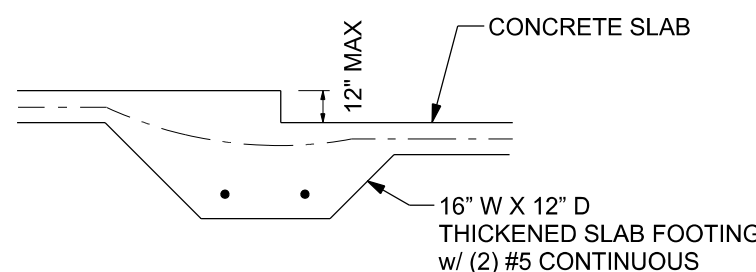
F1 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



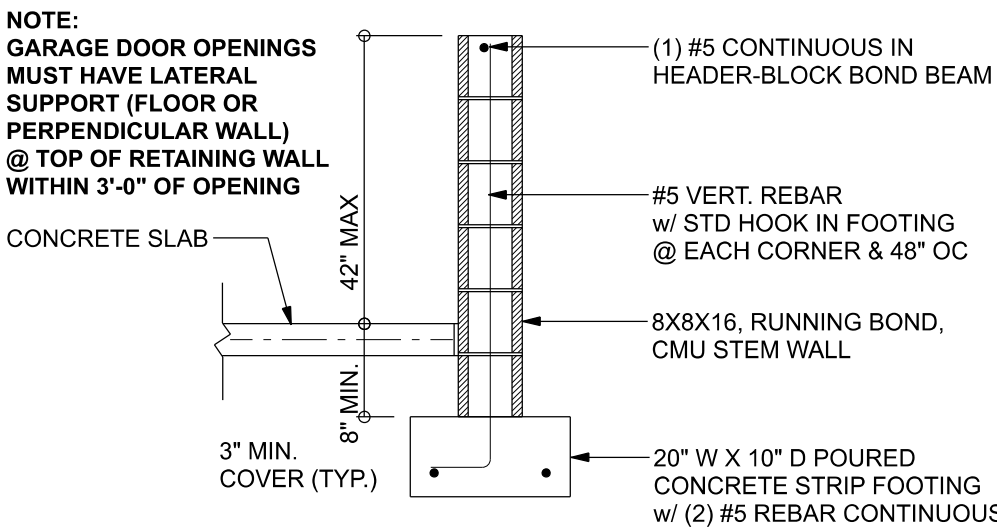
F1.1 S-2 MONOLITHIC FOOTING
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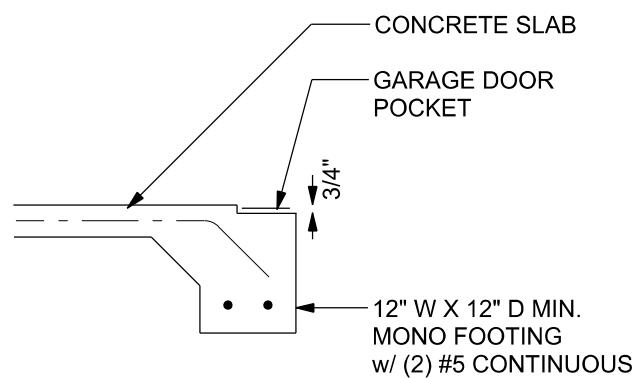
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



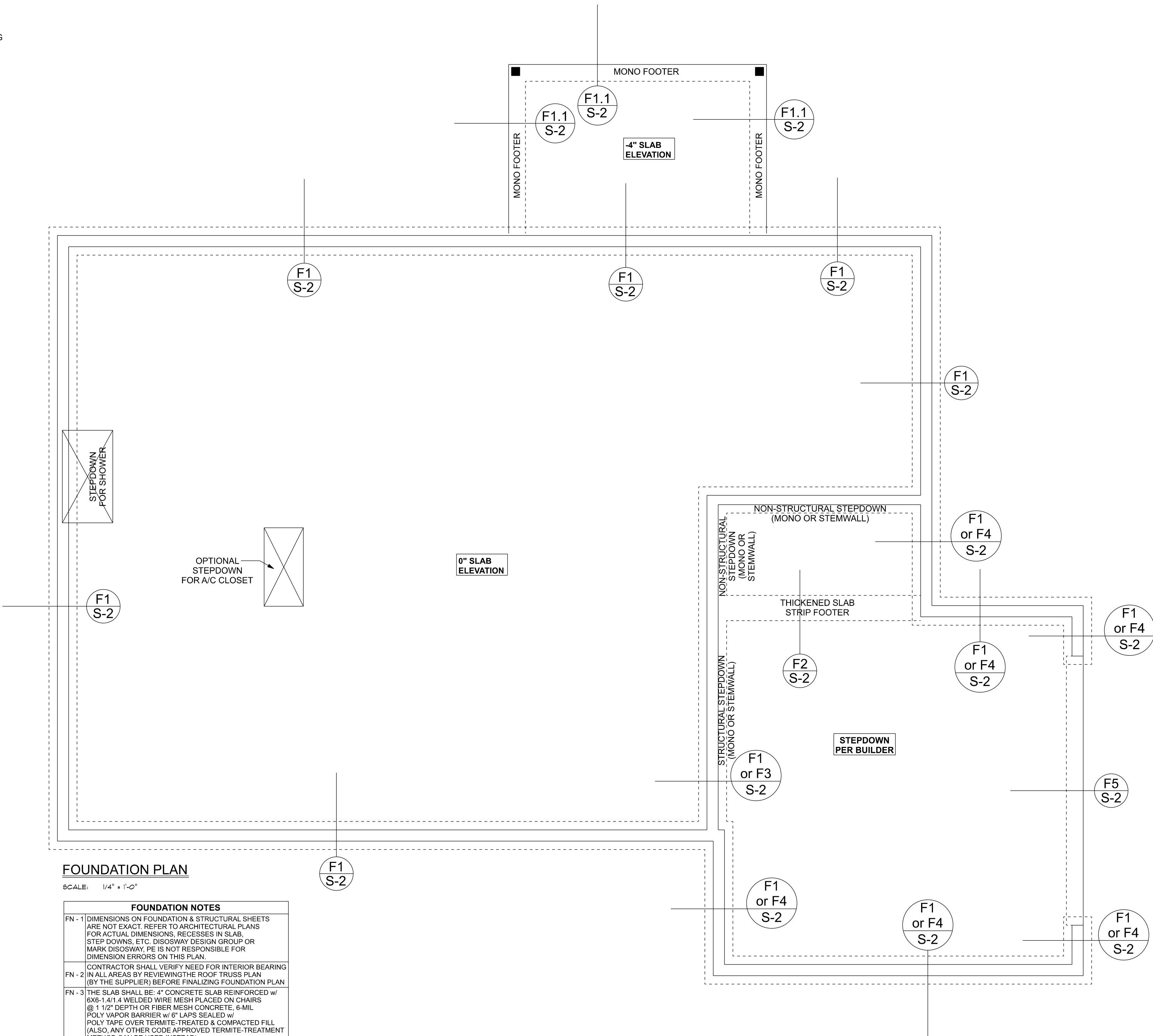
F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 OPTIONAL STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION NOTES

- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
- FN - 3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SCALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)

Bryan Zecher Homes, Inc.

Deborah Taylor Res.

PROJECT ADDRESS:
01-35-15-00127-013
COLUMBIA COUNTY, FLORIDA

FL PE 53915

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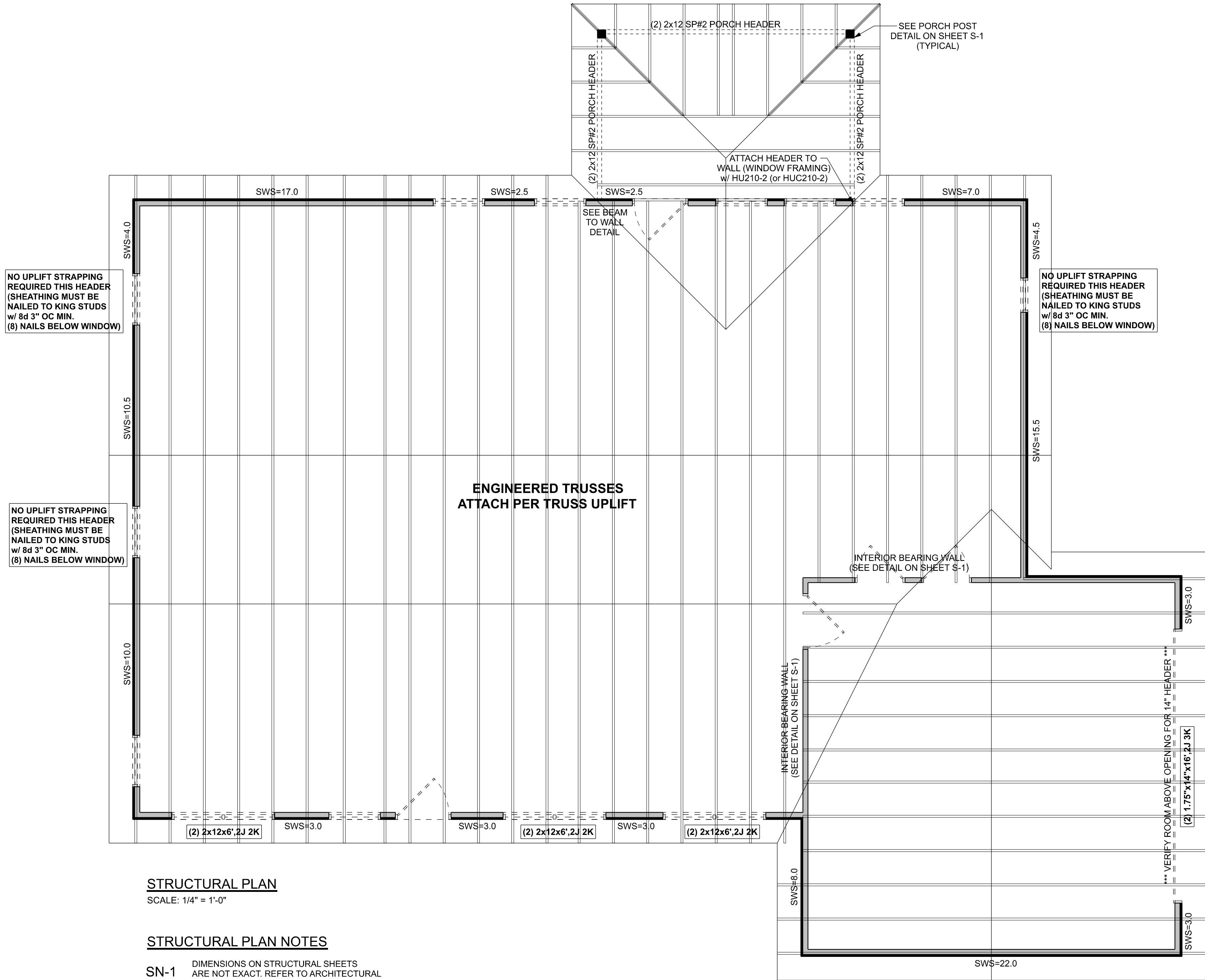
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JOB NUMBER:
2411023

S-2
OF 3 SHEETS



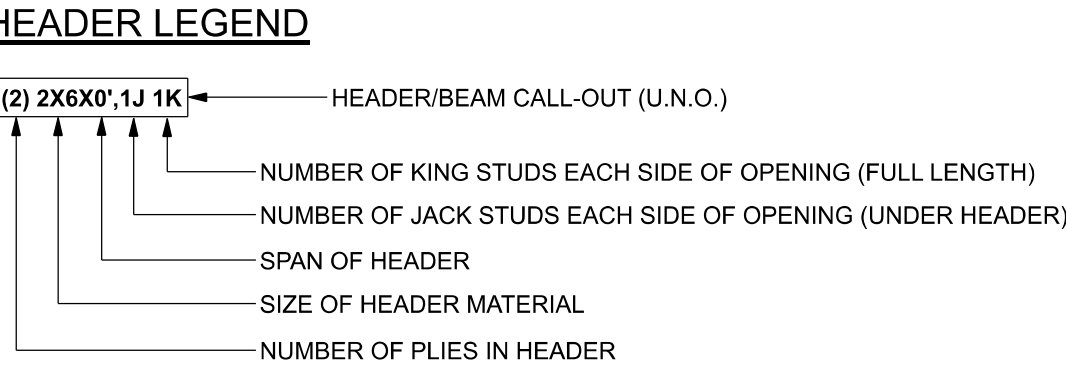
STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND COMPANY, INC JOB #24-1377

UNLESS NOTED OTHERWISE (MINIMUM REQUIERMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD



ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDUNAL
ACTUAL	23999 LBF	23760 LBF
REQUIRED	21513 LBF	19857 LBF

Bryan Zecher Homes, Inc

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PROJECT ADDRESS:
01-3S-15-00127-013
COLUMBIA COUNTY, FLORIDA

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JOB NUMBER:
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S-3
OF 3 SHEETS