ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1T7X8228Z0104093834

Truss Fabricator:	Anderson Truss Company
Job Identification:	7-051--Mike Todd Construction KELLY RES. -- , **
Truss Count:	26
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Versions 7.36, 7.25.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 06/04/2007

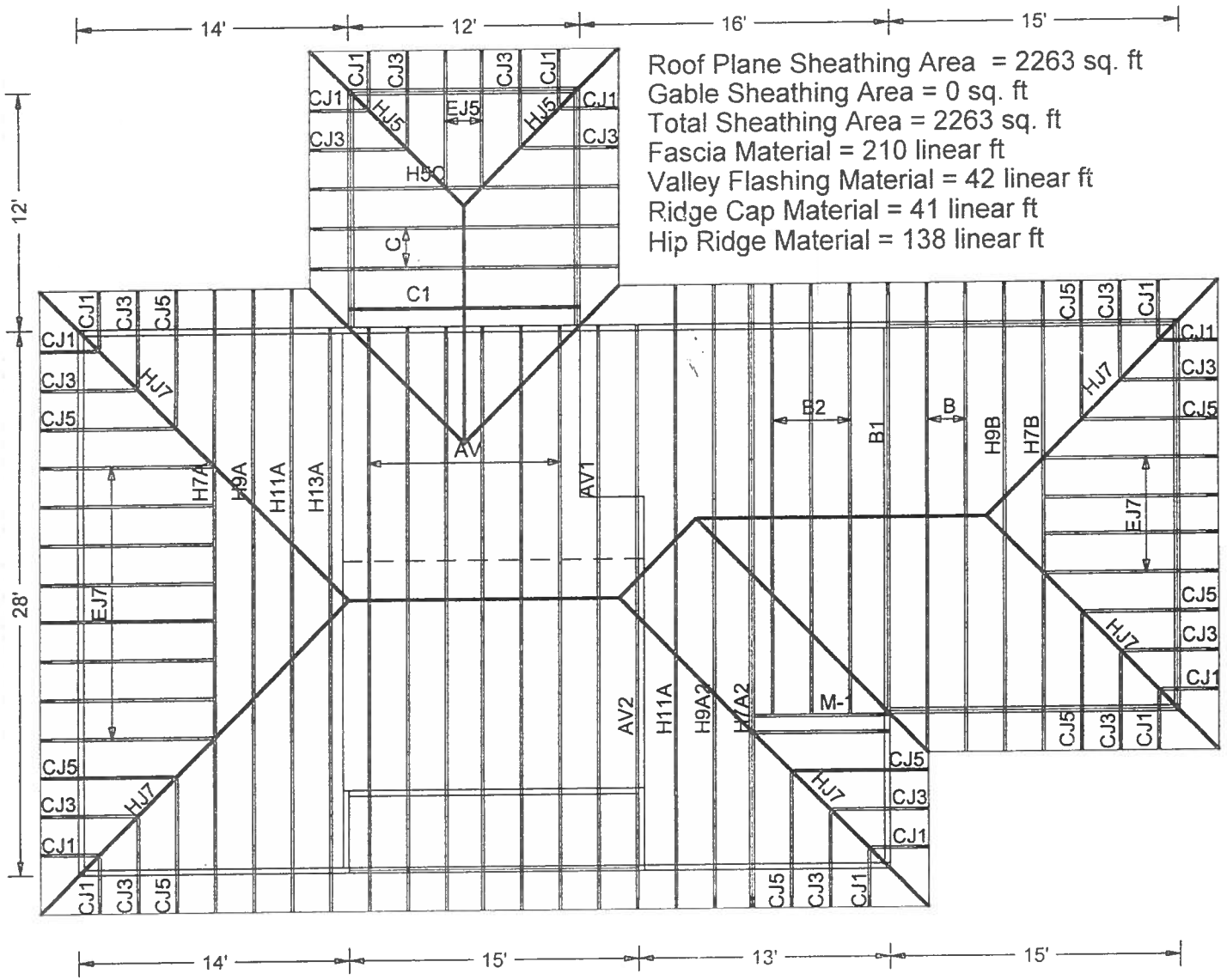
-Truss Design Engineer-
Arthur R. Fisher
Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-

#	Ref Description	Drawing#	Date
1	26081--H7A	07155001	06/04/07
2	26082--H9A2	07155002	06/04/07
3	26083--H11A	07155003	06/04/07
4	26084--H9A	07155004	06/04/07
5	26085--H13A	07155005	06/04/07
6	26086--H7A2	07155013	06/04/07
7	26087--AV	07155014	06/04/07
8	26088--AV1	07155015	06/04/07
9	26089--AV2	07155016	06/04/07
10	26090--H7B	07155018	06/04/07
11	26091--H9B	07155019	06/04/07
12	26092--B	07155020	06/04/07
13	26093--B1	07155021	06/04/07
14	26094--B2	07155011	06/04/07
15	26095--H5C	07155026	06/04/07
16	26096--C	07155023	06/04/07
17	26097--C1	07155022	06/04/07
18	26098--CJ1	07155006	06/04/07
19	26099--HJ7	07155010	06/04/07
20	26100--HJ5	07155025	06/04/07
21	26101--CJ3	07155007	06/04/07
22	26102--EJ5	07155024	06/04/07
23	26103--CJ5	07155008	06/04/07
24	26104--EJ7	07155017	06/04/07
25	26105--M-1	07155012	06/04/07
26	26106--EJ7S	07155009	06/04/07



Roof Plane Sheathing Area = 2263 sq. ft
Gable Sheathing Area = 0 sq. ft
Total Sheathing Area = 2263 sq. ft
Fascia Material = 210 linear ft
Valley Flashing Material = 42 linear ft
Ridge Cap Material = 41 linear ft
Hip Ridge Material = 138 linear ft

#7-051 Mike Todd / Candace Kelly
05/31/07

JOB DESCRIPTION:: Mike Todd Construction / KELLY RES.

JOB NO: 7-051

PAGE NO 1 OF 1

(7-051 - Mike Todd Construction KELLY RES. -- , ** - H7A)

Top chord 2x4 SP #2 Dense :T2 2x6 SP #2:
Bot chord 2x6 SP #2
Webs 2x4 SP #3

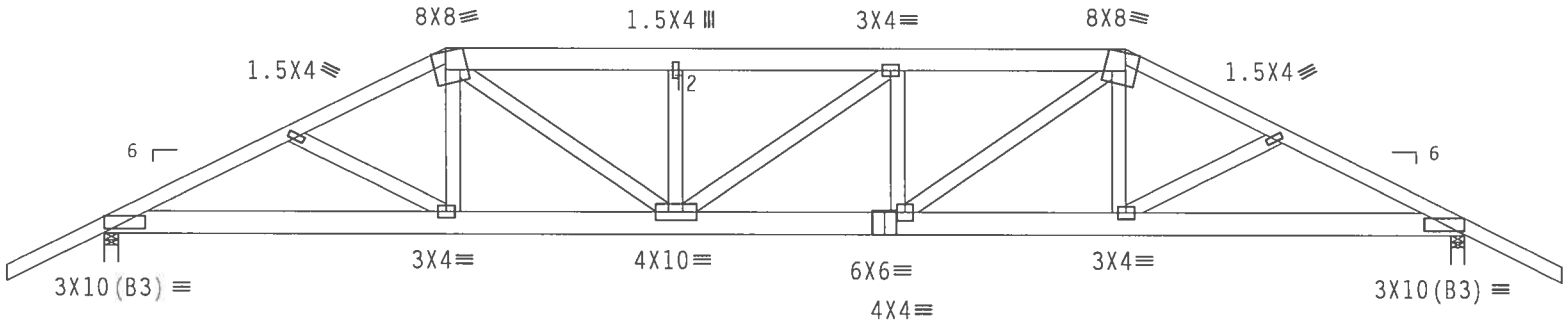
Wind reactions based on MWFRS pressures.

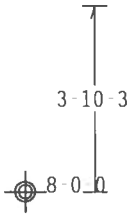
#1 hip supports 7-0-0 jacks with no webs.

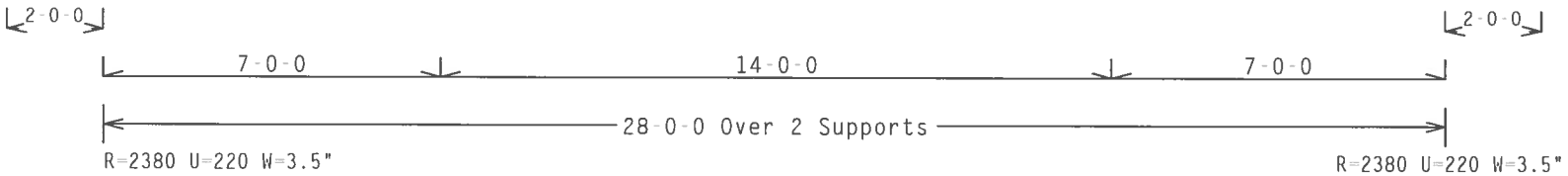
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.







PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 6 '07

TC LL	20.0 PSF	REF	R8228-26081
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155001
BC LL	0.0 PSF	HC-ENG	TCE/AF
TOT.LD.	40.0 PSF	SEQN-	29319
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1T7X8228Z01

(7 051 Mike Todd Construction KELLY RES. , ** H9A2)

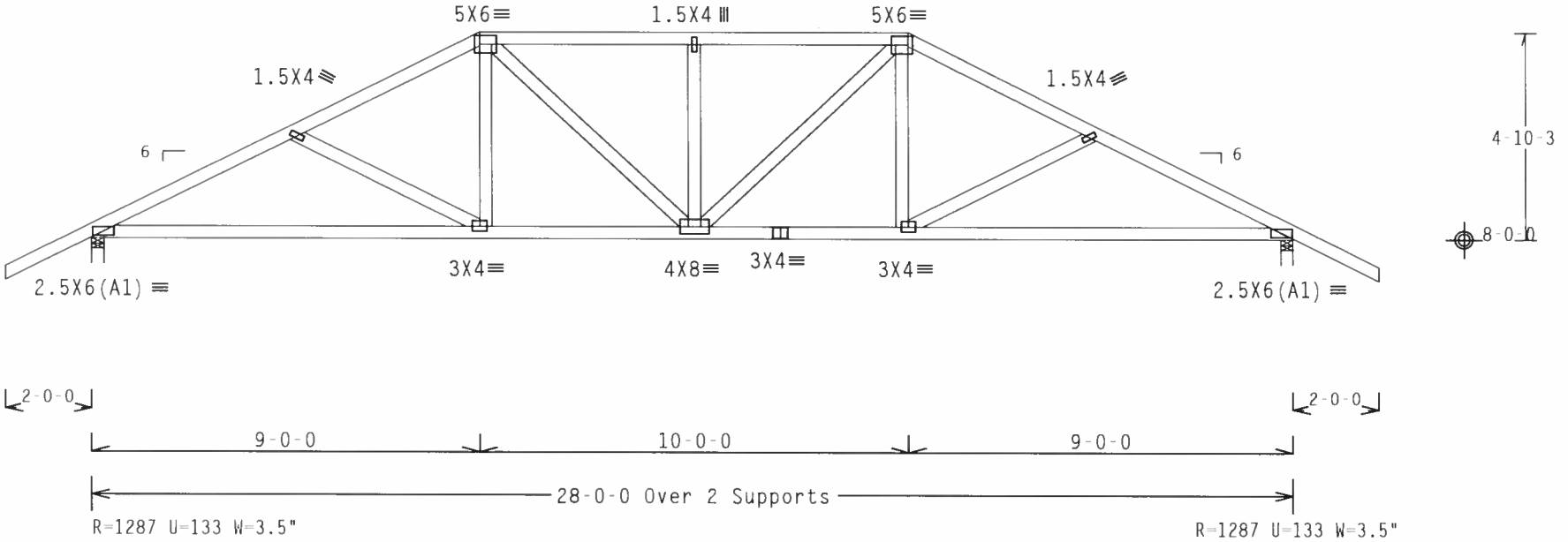
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.0 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jun 04 '07

ALPINE

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TC LL	20.0 PSF	REF	R8228- 26082
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155002
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29358
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - H11A)

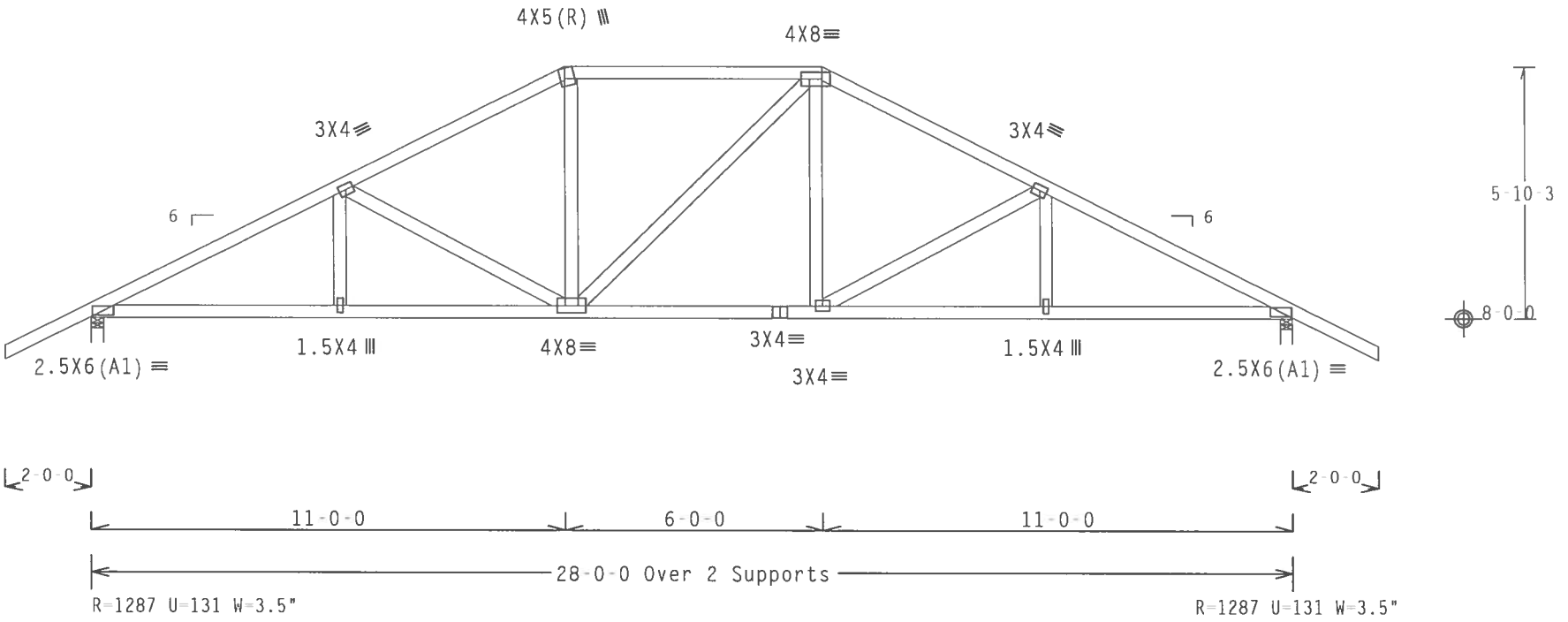
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.04 12 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26083
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155003
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29362
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - H9A)

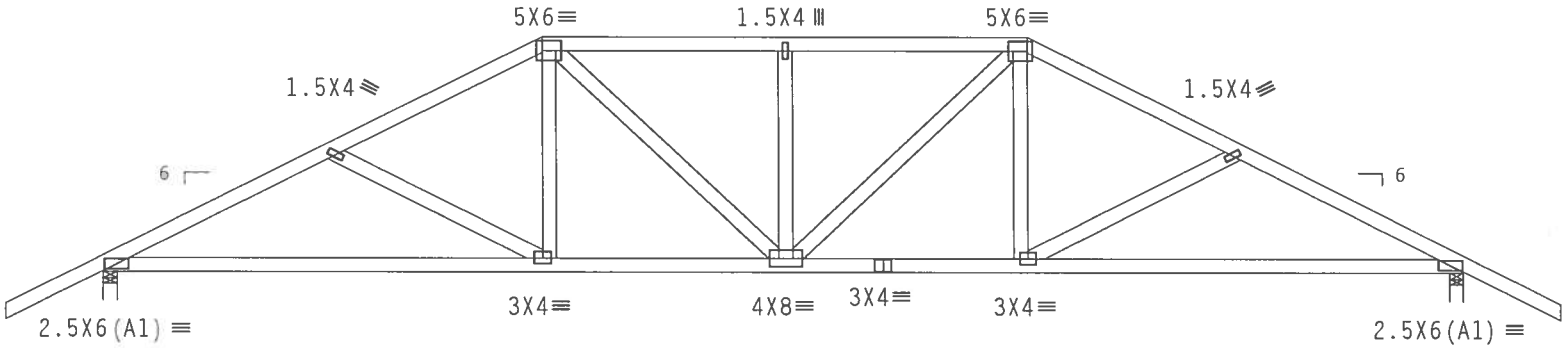
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

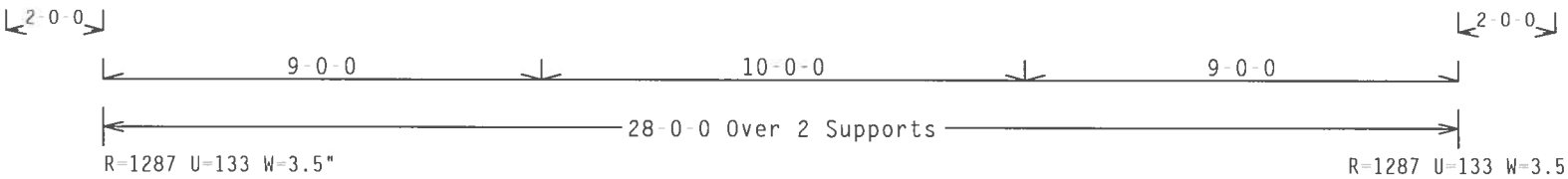
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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Jun 04 '07

TC LL	20.0 PSF	REF	R8228- 26084
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155004
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29365
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - H13A)

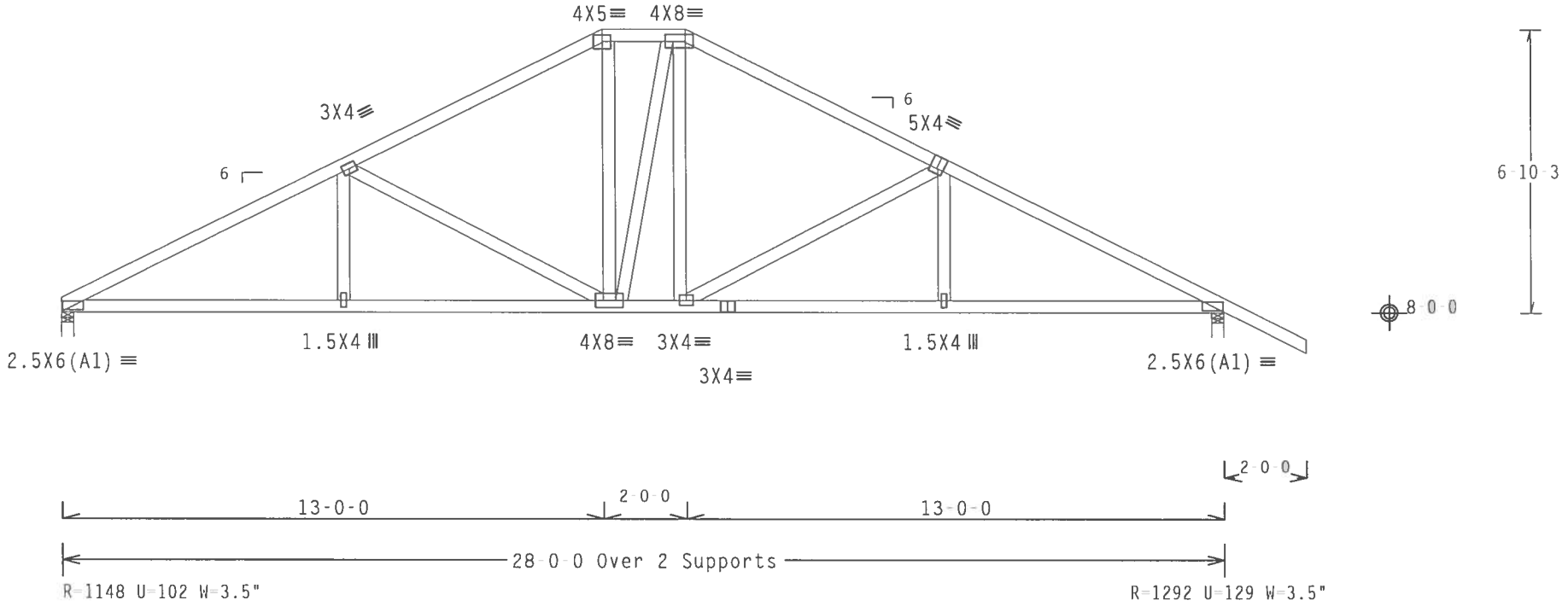
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 667

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Jun 04 07

TC LL	20.0 PSF	REF	R8228-26085
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155005
BC LL	0.0 PSF		HC-ENG TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29368
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - H7A2)

Top chord 2x4 SP #2 Dense :T2 2x6 SP #2:
Bot chord 2x6 SP #2
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 62 PLF at -2.00 to 62 PLF at 30.00
BC - From 4 PLF at -2.00 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 28.00
BC - From 4 PLF at 28.00 to 4 PLF at 30.00
TC - 446 LB Conc. Load at 7.06
BC - 440 LB Conc. Load at 7.00
BC - 1679 LB Conc. Load at 7.94

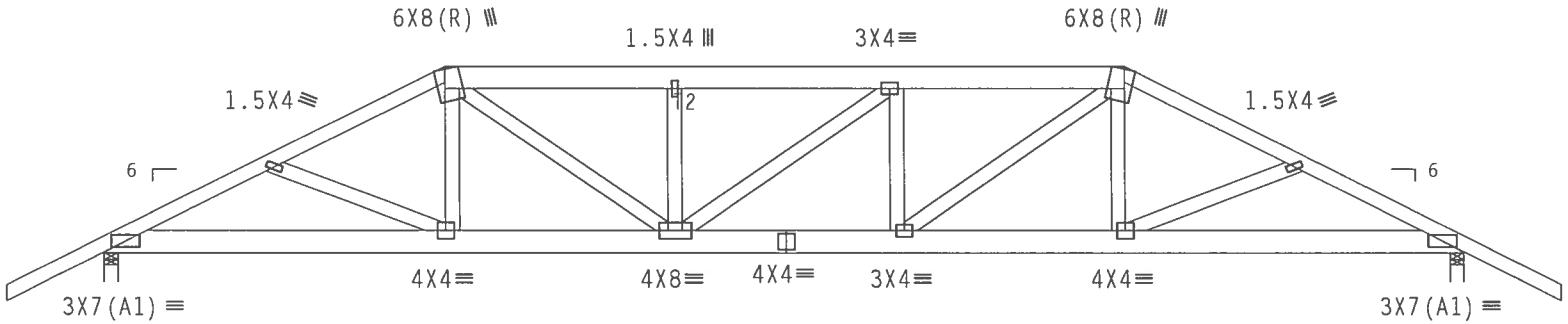
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

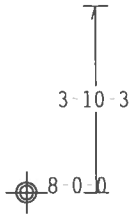
2 COMPLETE TRUSSES REQUIRED

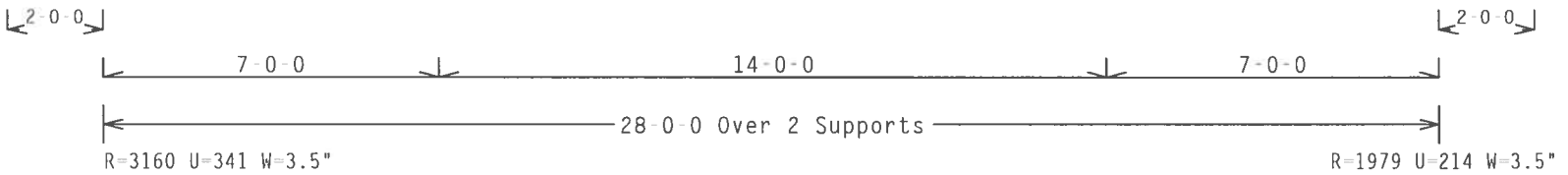
Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.







Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.25.0 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26086
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155013
BC LL	0.0 PSF	HC-ENG	TCE/AF
TOT.LD.	40.0 PSF	SEQN-	95805 REV
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - AV)

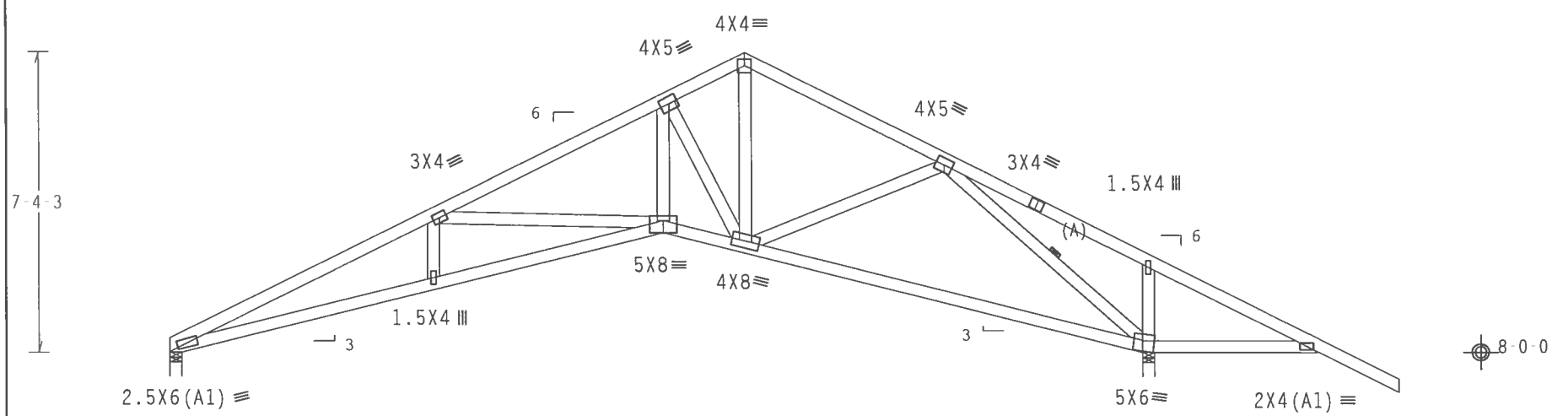
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

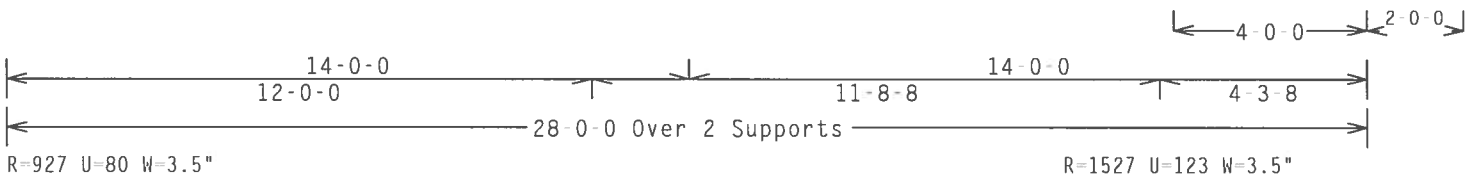
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

(A) Continuous lateral bracing equally spaced on member.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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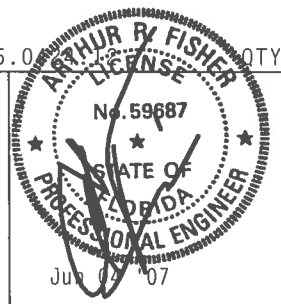


TC LL	20.0 PSF	REF	R8228-26087
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155014
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29373
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - AV1)

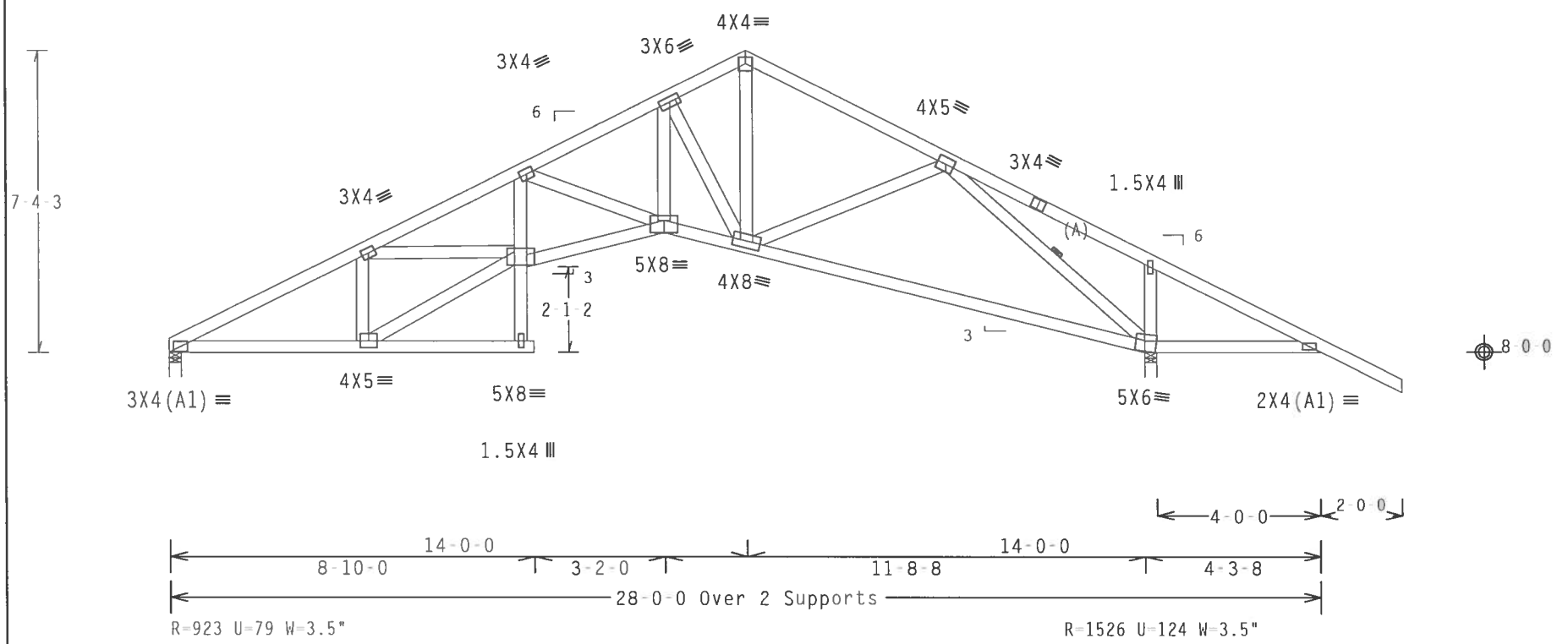
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

(A) Continuous lateral bracing equally spaced on member.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
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ARTHUR B. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun [illegible] '07

TC LL	20.0 PSF	REF	R8228- 26088
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155015
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29376
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7 051 Mike Todd Construction KELLY RES. , ** AV2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

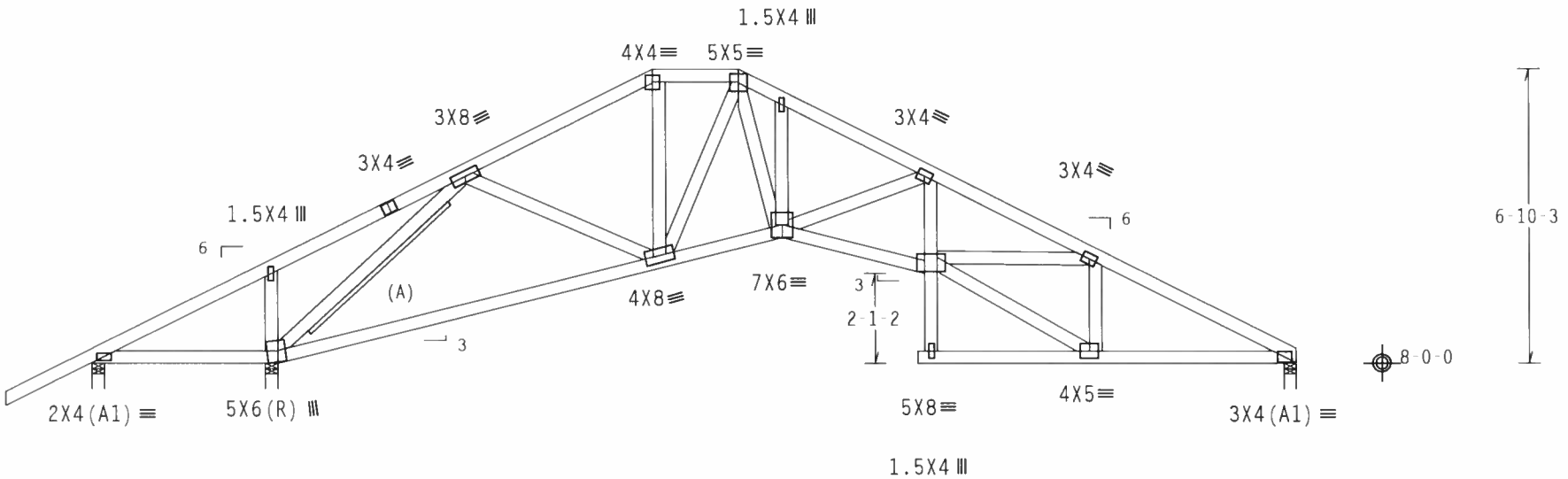
Wind reactions based on MWFRS pressures.

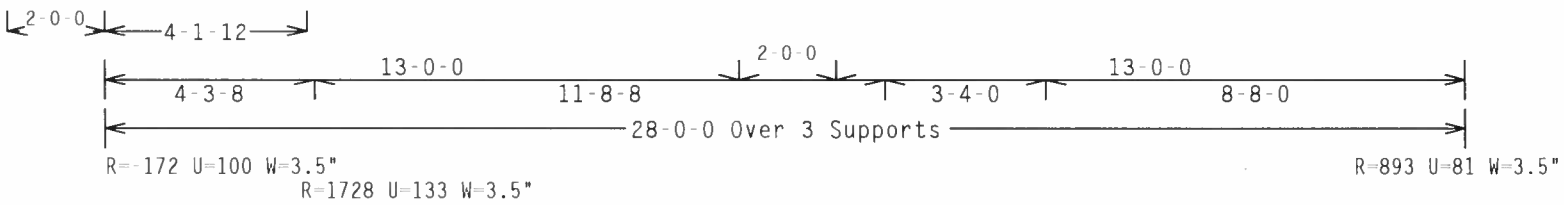
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 TY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228- 26089
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155016
BC LL	0.0 PSF		HC-ENG TCE/AF
TOT.LD.	40.0 PSF	SEQN-	29379
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7 051 Mike Todd Construction KELLY RES. , ** H7B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC	From	62 PLF	at	-2.00	to	62 PLF	at 7.00
TC	From	31 PLF	at	7.00	to	31 PLF	at 13.00
TC	From	62 PLF	at	13.00	to	62 PLF	at 22.00
BC	From	4 PLF	at	-2.00	to	4 PLF	at 0.00
BC	From	20 PLF	at	0.00	to	20 PLF	at 7.00
BC	From	10 PLF	at	7.00	to	10 PLF	at 13.00
BC	From	20 PLF	at	13.00	to	20 PLF	at 20.00
BC	From	4 PLF	at	20.00	to	4 PLF	at 22.00

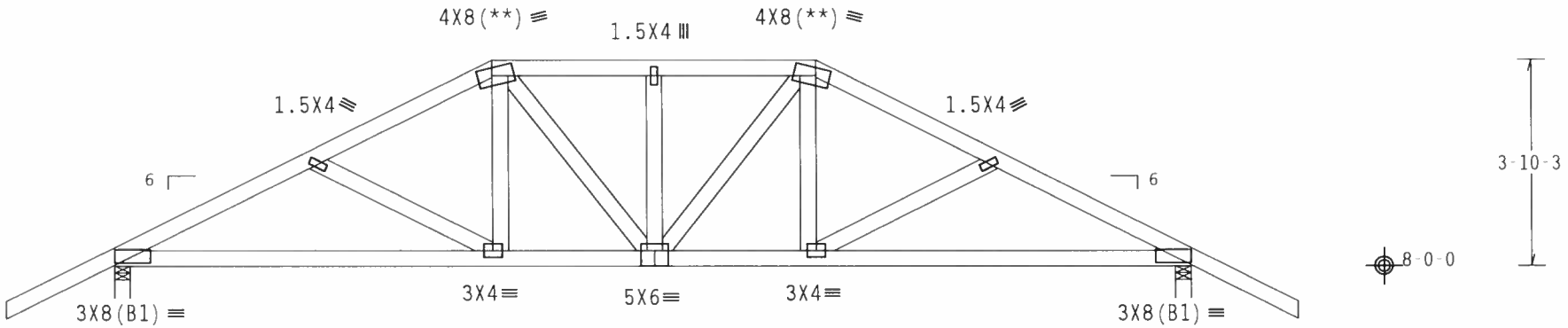
TC - 434 LB Conc. Load at 7.06, 12.94
TC - 182 LB Conc. Load at 9.06, 10.94
BC - 430 LB Conc. Load at 7.00, 13.00
BC - 77 LB Conc. Load at 9.06, 10.94

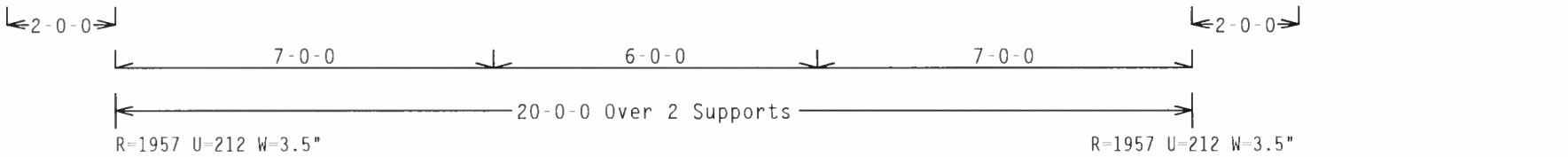
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.25.04 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

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ARTHUR A. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228- 26090
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155018
BC LL	0.0 PSF		HC-ENG TCE/AF
TOT.LD.	40.0 PSF	SEQN-	95810 REV
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - H9B)

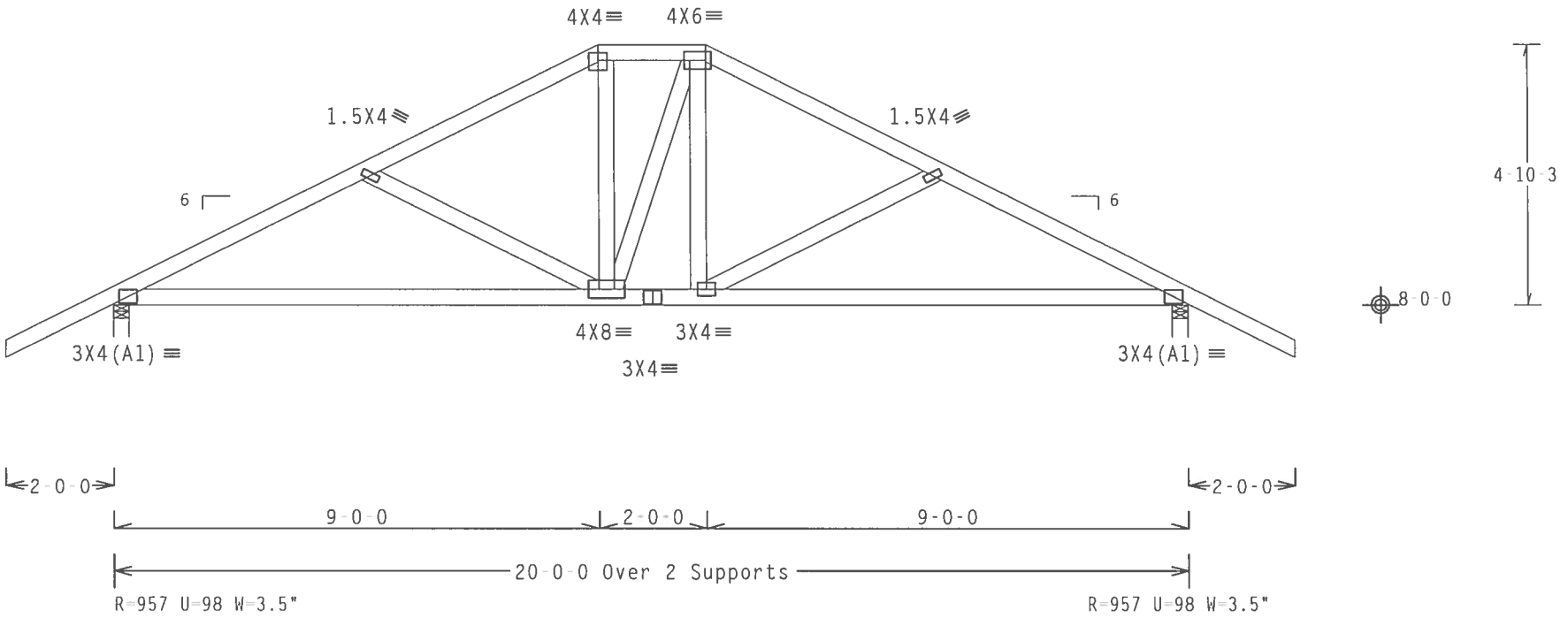
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
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TC LL	20.0 PSF	REF	R8228- 26091
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155019
BC LL	0.0 PSF		HC-ENG TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29326
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

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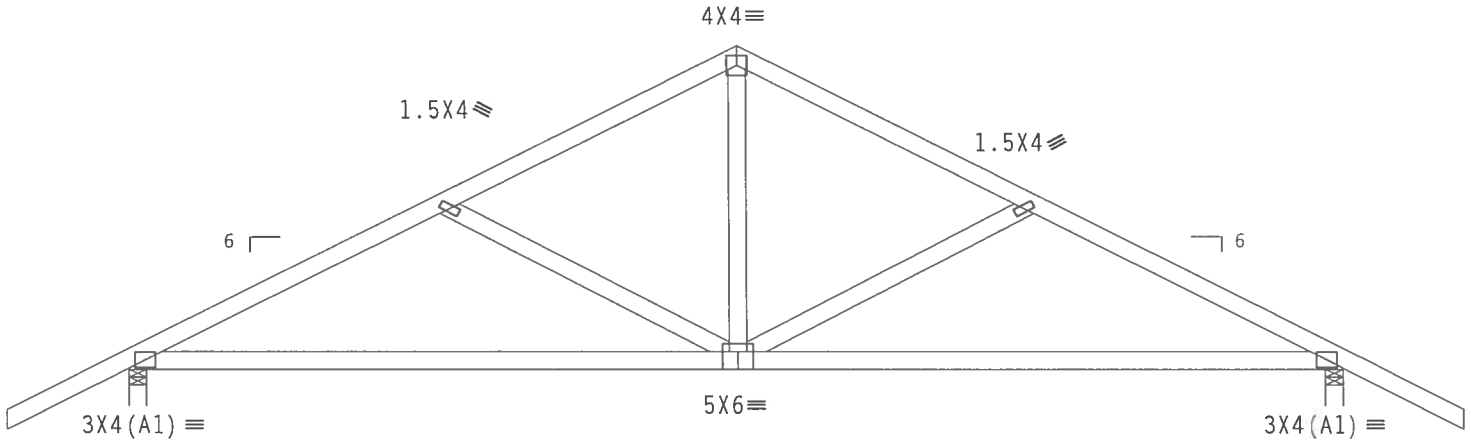

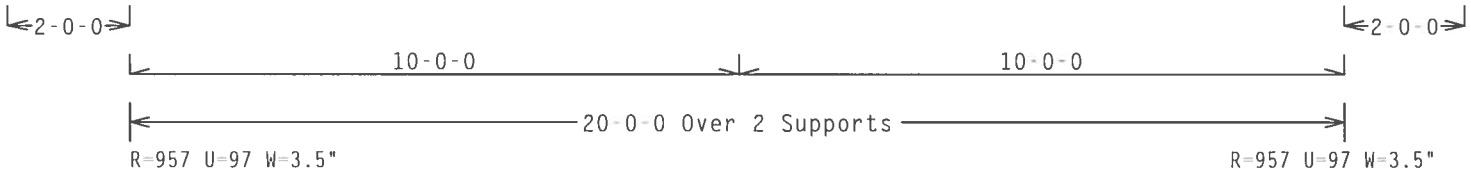

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group, Inc.
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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26092
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155020
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29329
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

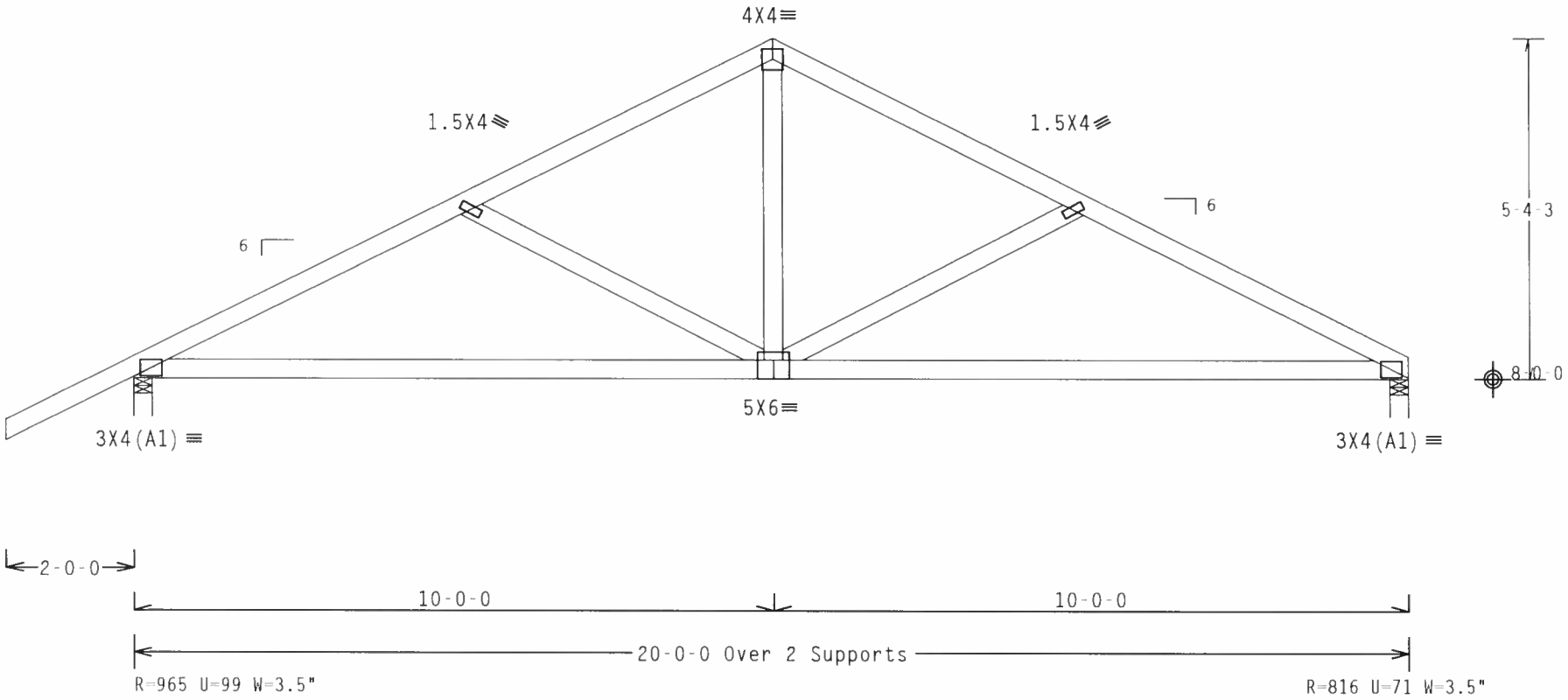
(7 051 Mike Todd Construction KELLY RES. , ** B1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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ARTHUR R. FISHER
LICENSE
No. 53687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 07 2007

TC LL	20.0 PSF	REF	R8228- 26093
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155021
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29332
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

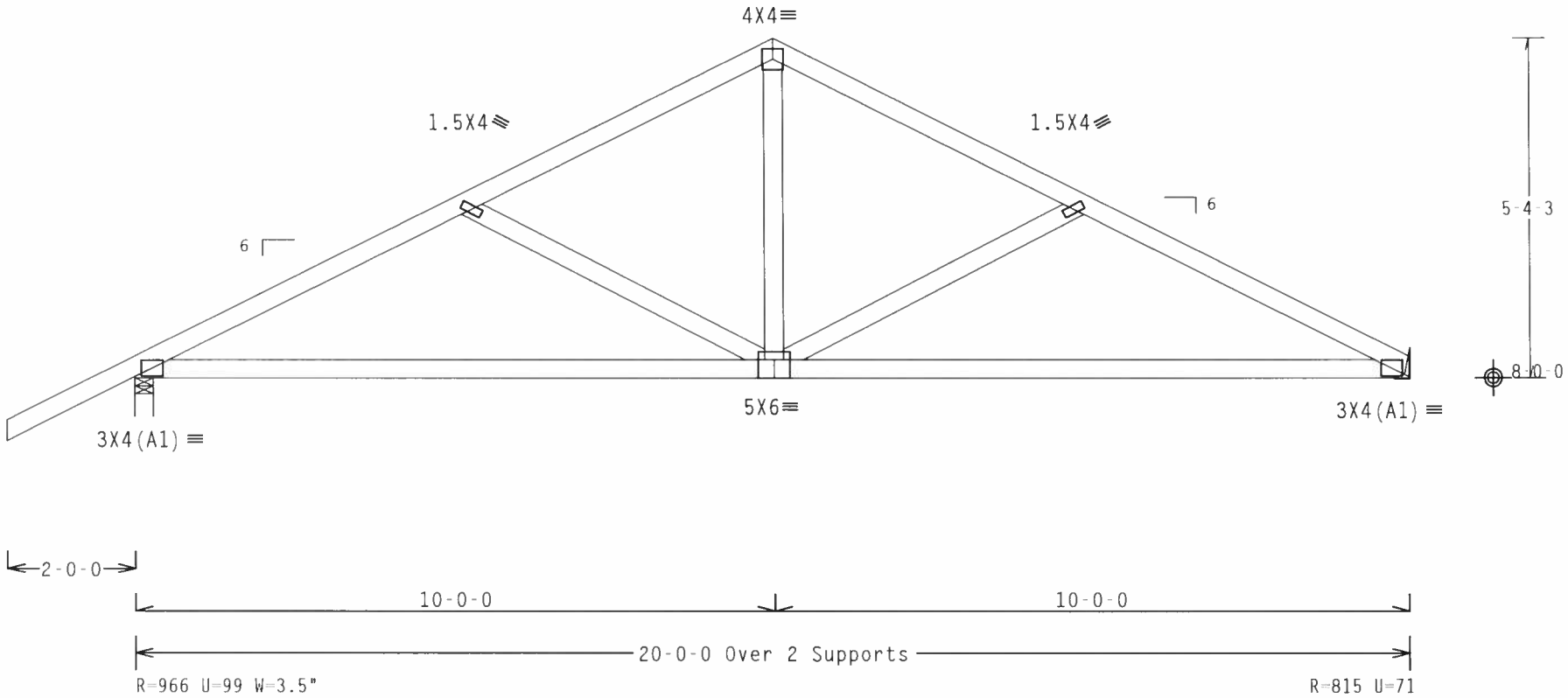
(7 051 Mike Todd Construction KELLY RES. , ** B2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.0 TY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228- 26094
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155011
BC LL	0.0 PSF		HC-ENG TCE/AF
TOT.LD.	40.0 PSF	SEQN-	29335
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7 051 Mike Todd Construction KELLY RES. , ** H5C)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

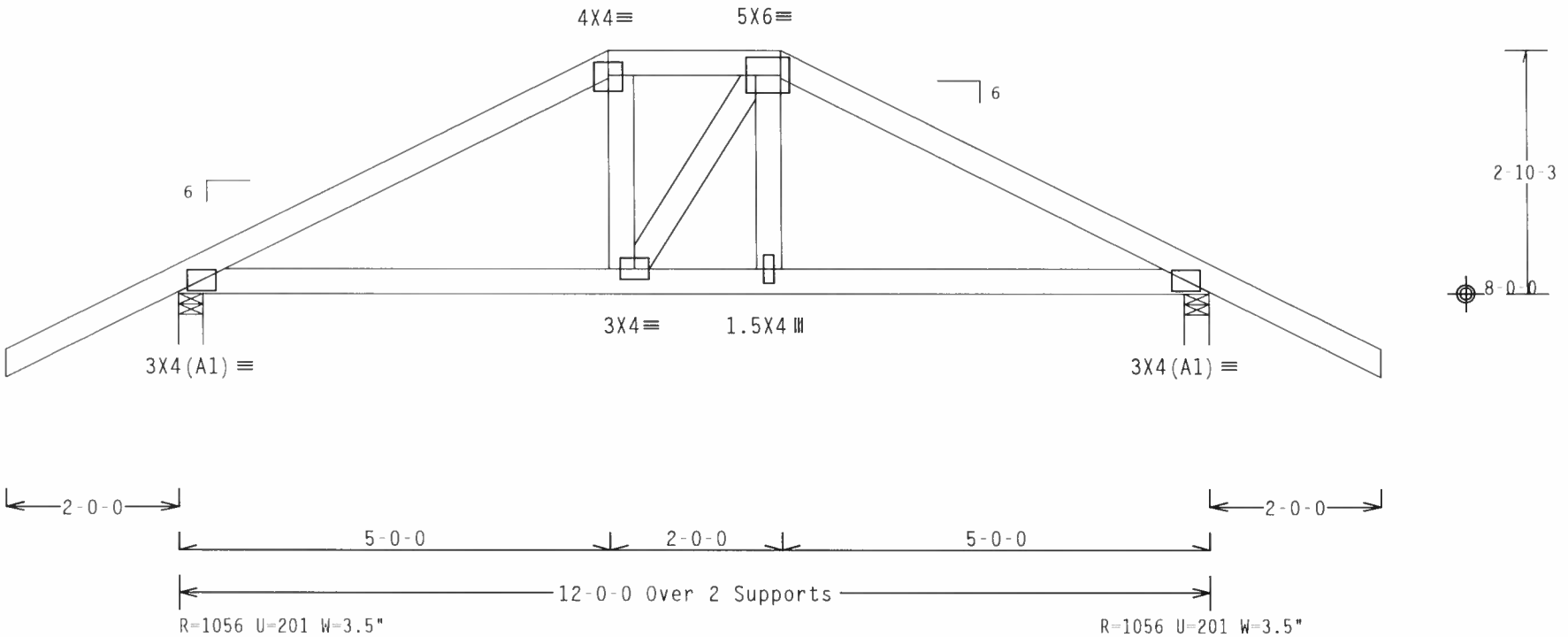
Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 62 PLF at -2.00 to 62 PLF at 5.00
TC - From 62 PLF at 5.00 to 62 PLF at 7.00
TC - From 62 PLF at 7.00 to 62 PLF at 14.00
BC - From 4 PLF at -2.00 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 12.00
BC - From 4 PLF at 12.00 to 4 PLF at 14.00
TC - 312 LB Conc. Load at 5.00, 7.00
BC - 116 LB Conc. Load at 5.00, 7.00



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
JUL 04 '07

TC LL	20.0 PSF	REF	R8228- 26095
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155026
BC LL	0.0 PSF		HC-ENG TCE/AF
TOT.LD.	40.0 PSF	SEQN-	29303
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1T7X8228Z01

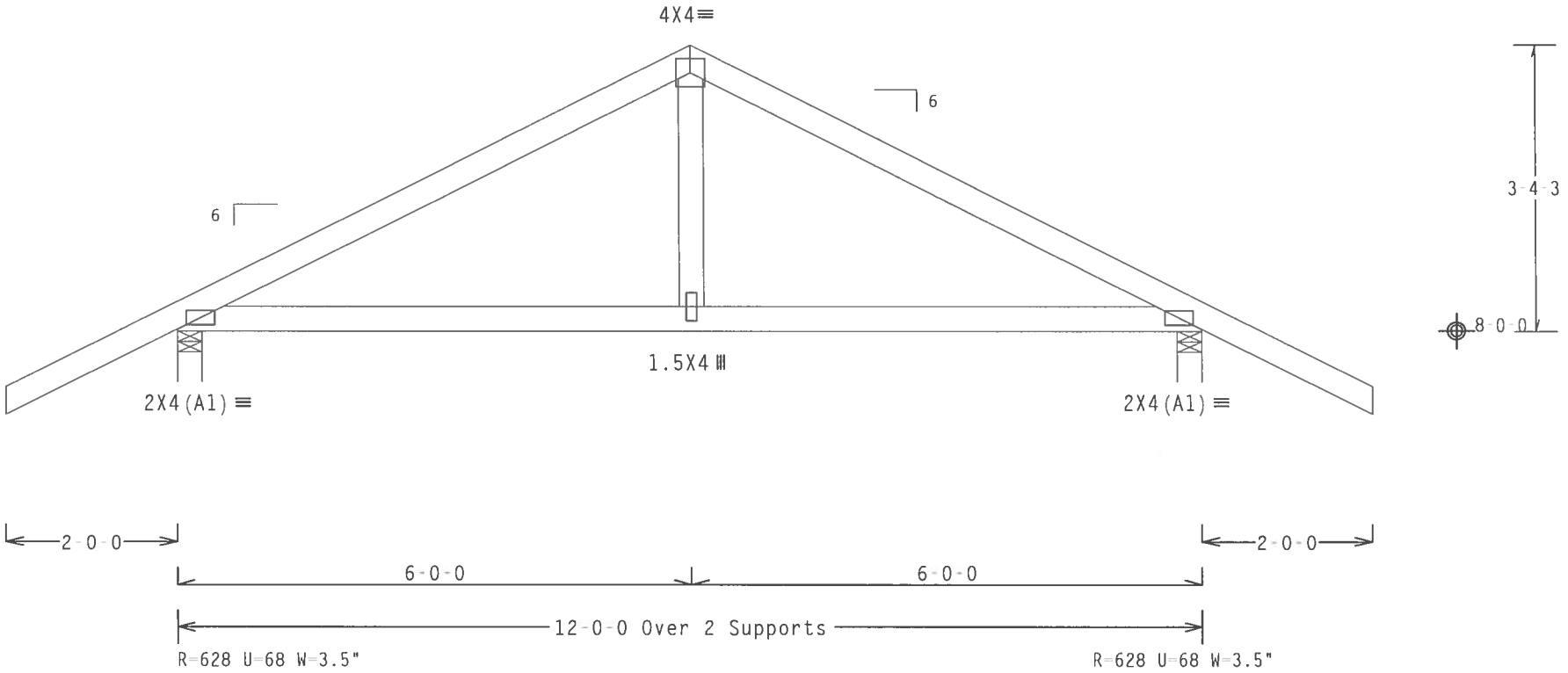
(7-051--Mike Todd Construction KELLY RES. -- , ** - C)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/-

PLT TYP. Wave

Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 07 '07

TC LL	20.0 PSF	REF	R8228-26096
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155023
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29307
DUR.FAC.	1.25	FROM	J1
SPACING	24.0"	JREF-	1T7X8228Z01

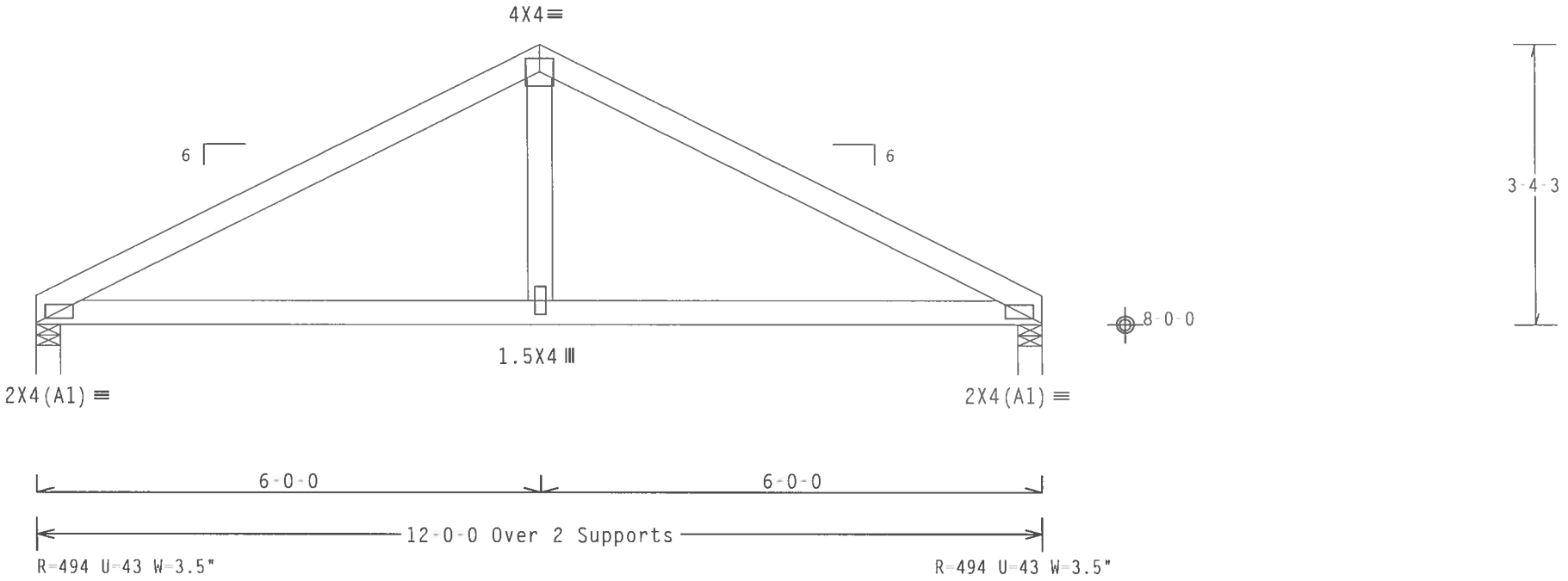
(7-051--Mike Todd Construction KELLY RES. -- , ** - C1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 07

TC LL	20.0 PSF	REF	R8228-26097
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155022
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN	29310
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF	1T7X8228Z01

(7-051 - Mike Todd Construction KELLY RES. - , ** - CJ1)

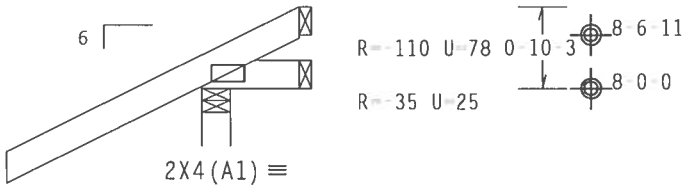
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

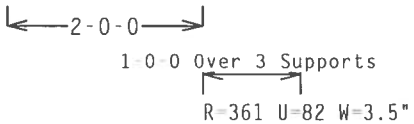
Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
Provide (2) 0.162x3.5" 16d Common toe-nails at Bottom Chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.04 TY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26098
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155006
BC LL	0.0 PSF	HC-ENG	TCE/AF
TOT.LD.	40.0 PSF	SEQN-	29283
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051 Mike Todd Construction KELLY RES. , ** HJ7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

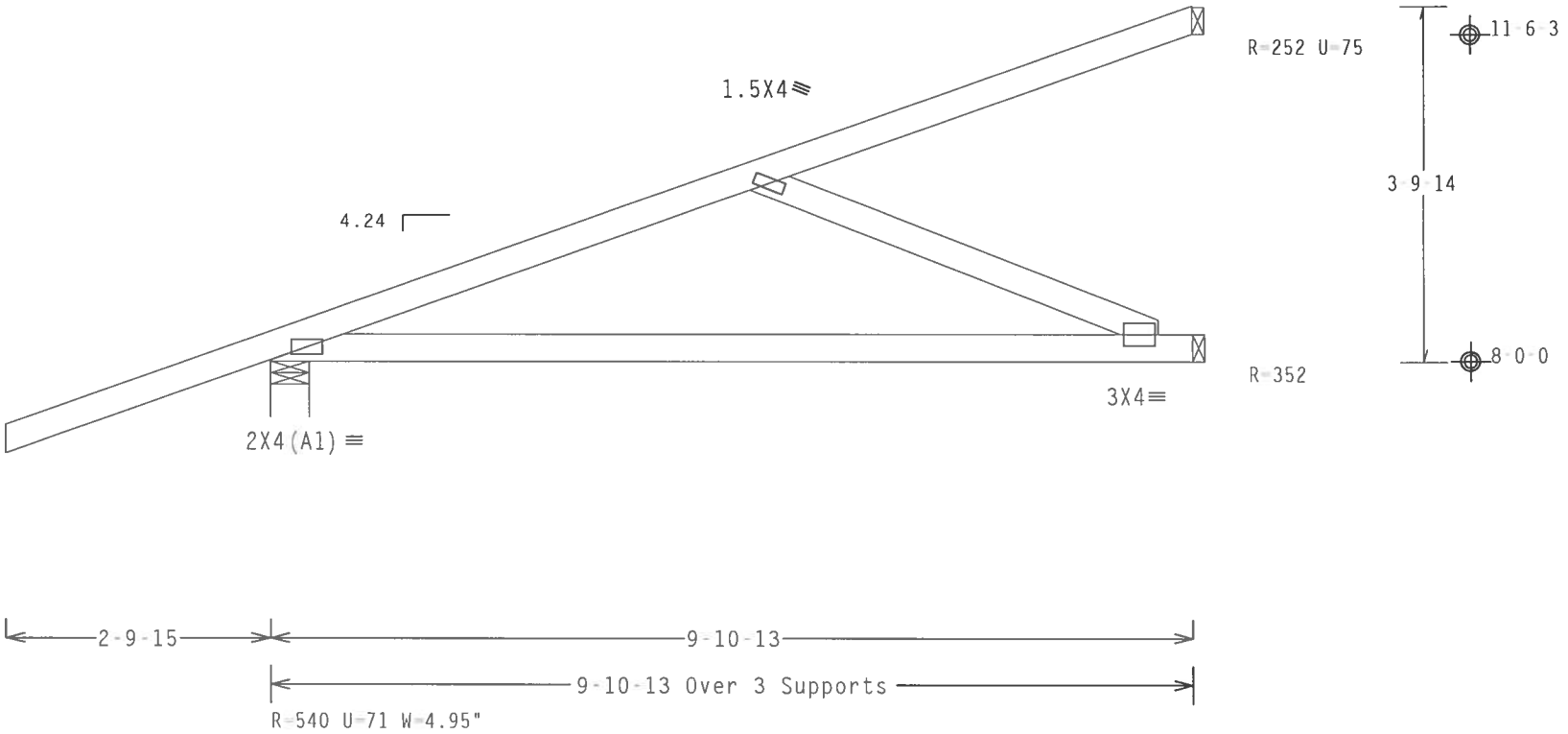
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Hipjack supports 7-0-0 setback jacks with no webs.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
Provide (3) 0.162x3.5" 16d Common toe-nails at Bottom Chord.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26099
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155010
BC LL	0.0 PSF		HC-ENG TCE/AF
TOT.LD.	40.0 PSF	SEQN-	29288
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1T7X8228Z01

(7-051 - Mike Todd Construction KELLY RES. - , ** - HJ5)

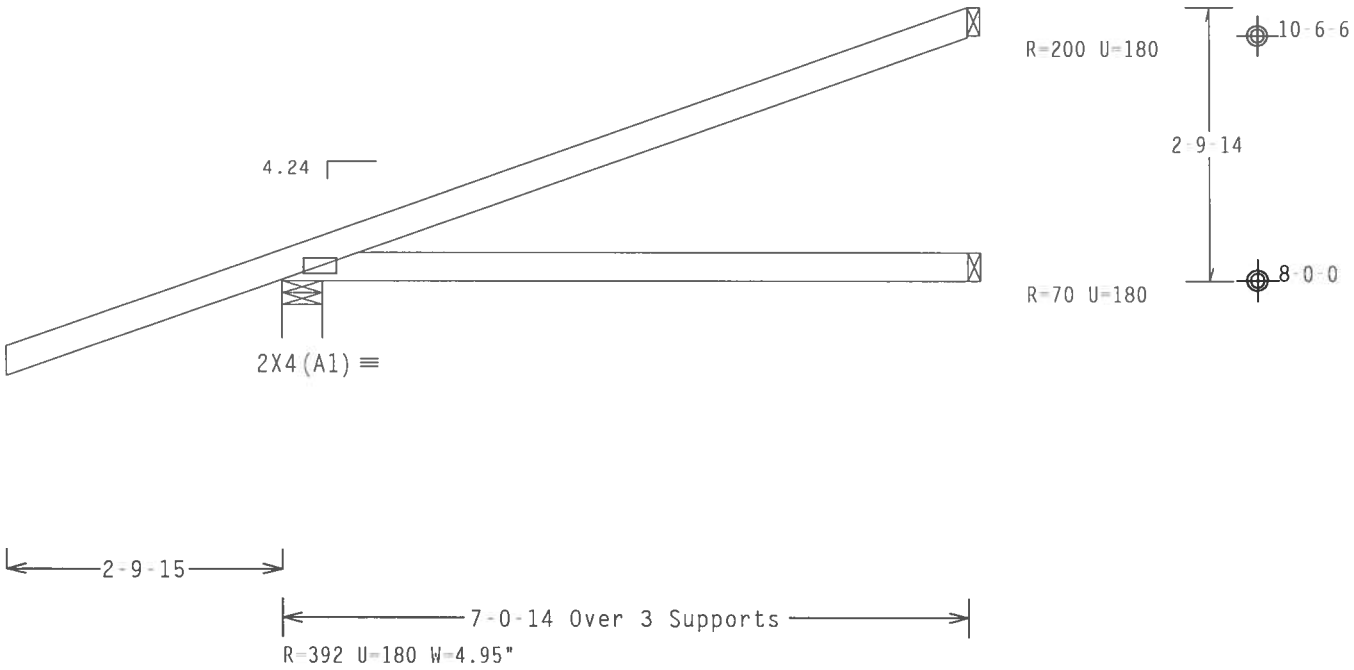
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Hipjack supports 5-0-0 setback jacks with no webs.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
Provide (2) 0.162x3.5" 16d Common toe-nails at Bottom Chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.25.04 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04,07

TC LL	20.0 PSF	REF	R8228-26100
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155025
BC LL	0.0 PSF	HC-ENG	TCE/AF
TOT.LD.	40.0 PSF	SEQN-	95814 REV
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1T7X8228Z01

(7 051 Mike Todd Construction KELLY RES. , ** CJ3)

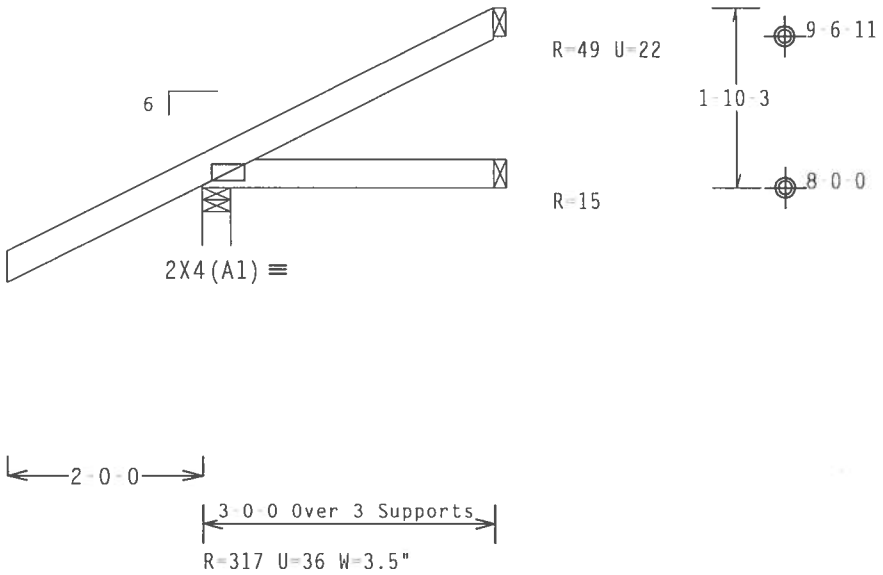
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe nails at Top Chord.
Provide (2) 0.162x3.5" 16d Common toe nails at Bottom Chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/-

PLT TYP. Wave

Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

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ARTHUR R. FISHER
LICENSE
No.39687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 04 07

TC LL	20.0 PSF	REF	R8228-26101
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155007
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29296
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - EJ5)

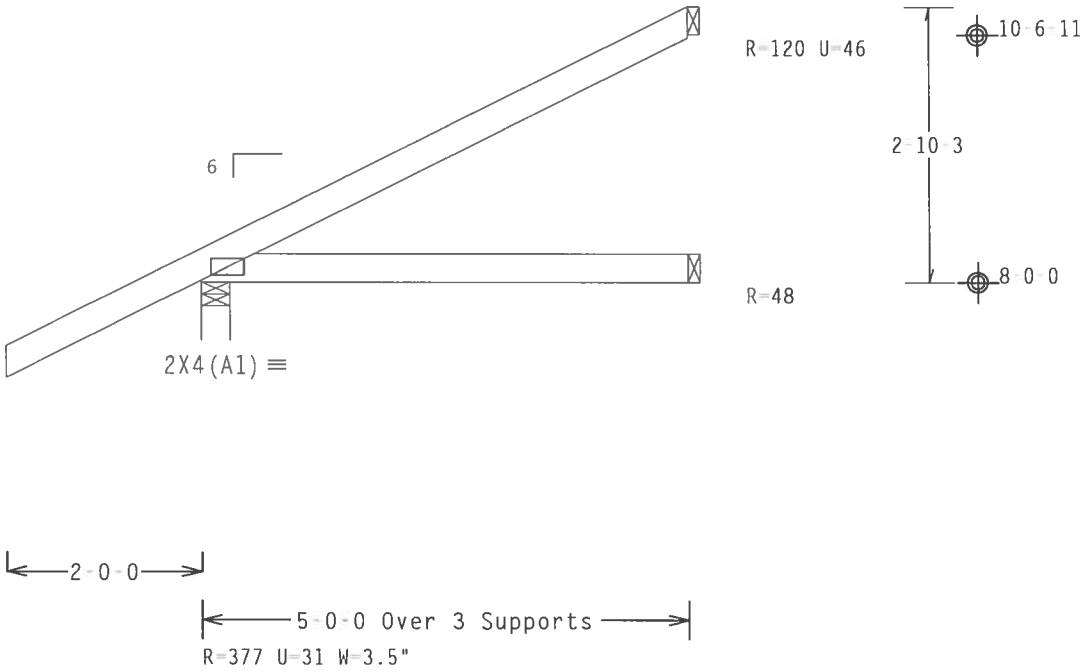
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe nails at Top Chord.
Provide (2) 0.162x3.5" 16d Common toe nails at Bottom Chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.0 TY:1 FL/-/4/-/-/R/-

PLT TYP. Wave

Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 667

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26102
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155024
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29300
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7 051 Mike Todd Construction KELLY RES. -- , ** - CJ5)

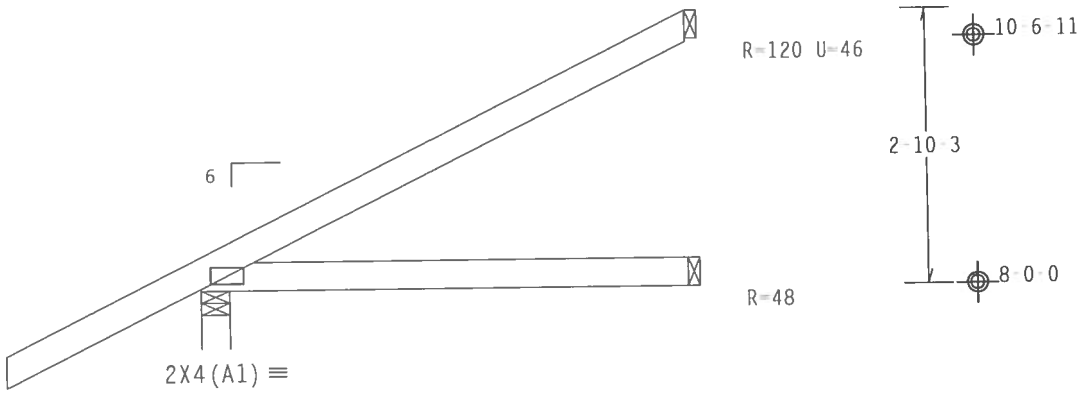
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

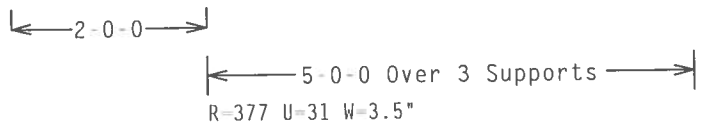
Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe nails at Top Chord.
Provide (2) 0.162x3.5" 16d Common toe nails at Bottom Chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

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ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26103
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155008
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29313
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - EJ7)

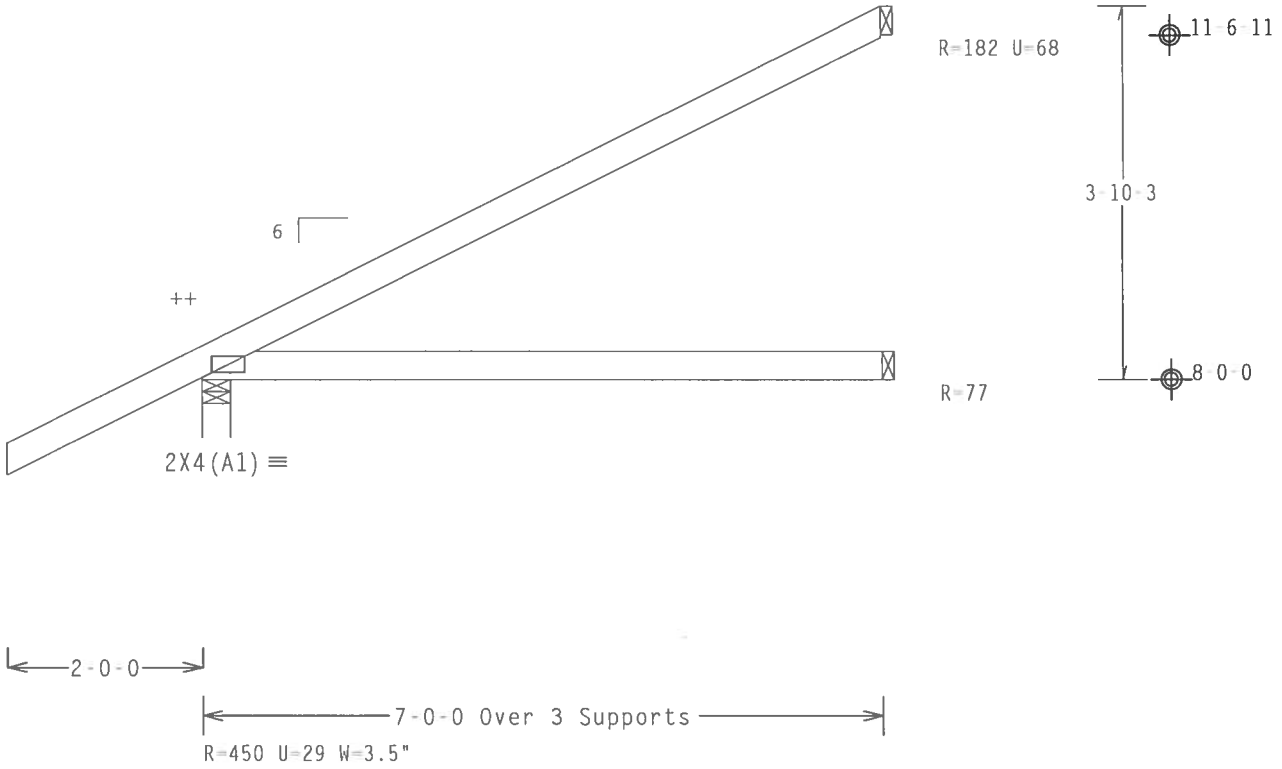
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
Provide (2) 0.162x3.5" 16d Common toe-nails at Bottom Chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.04 TY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26104
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155017
BC LL	0.0 PSF		HC-ENG TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29316
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - M-1)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

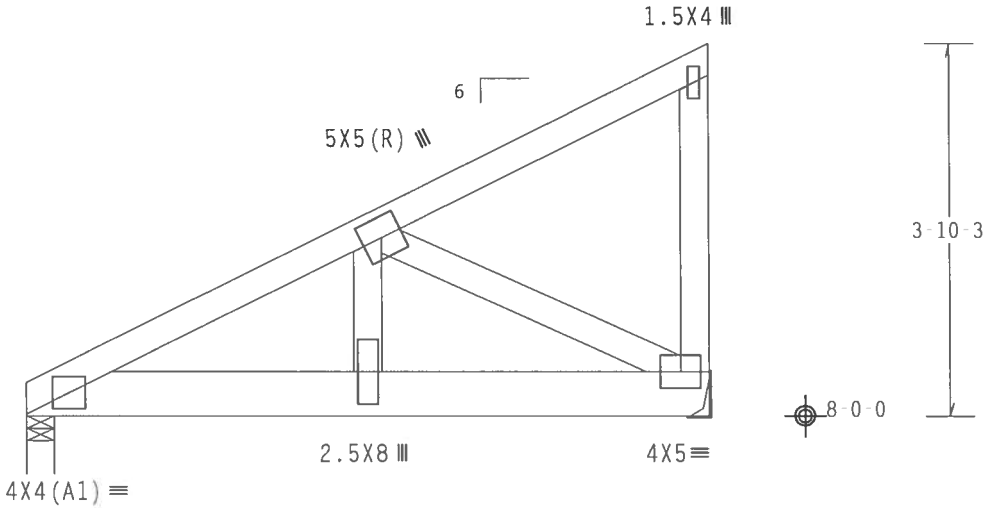
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 62 PLF at 0.00 to 62 PLF at 7.00
BC - From 20 PLF at 0.00 to 20 PLF at 7.00
PLB- 815 LB Conc. Load at (2.06,8.04), (4.06,8.04), (6.06,8.04)

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



7-0-0 Over 2 Supports

R=1342 U=122 W=3.5" R=1679 U=151

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 667

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 04 '07

TC LL	20.0 PSF	REF	R8228-26105
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155012
BC LL	0.0 PSF		HC-ENG TCE/AF
TOT.LD.	40.0 PSF	SEQN-	29338
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

(7-051--Mike Todd Construction KELLY RES. -- , ** - EJ7S)

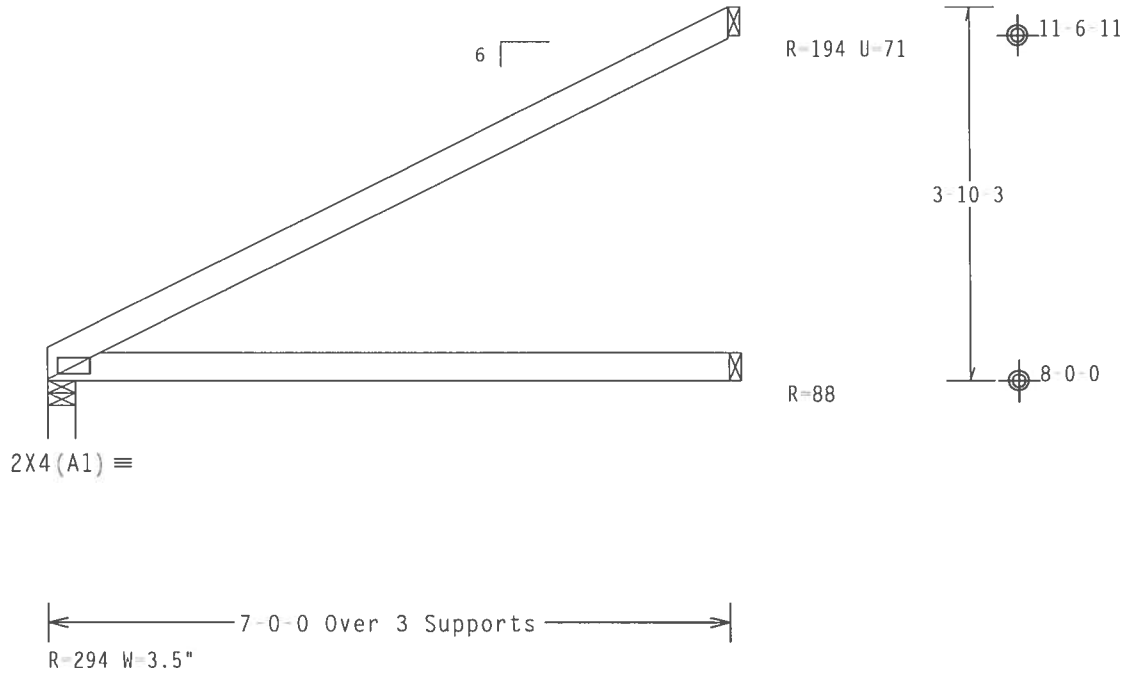
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
Provide (2) 0.162x3.5" 16d Common toe-nails at Bottom Chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.36.0 QTY:1 FL/--/4/--/--/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPII-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ARTHUR A. FISHER
LICENSE
No.59687
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
Jun 0[illegible] '07

TC LL	20.0 PSF	REF	R8228-26106
TC DL	10.0 PSF	DATE	06/04/07
BC DL	10.0 PSF	DRW	HCUSR8228 07155009
BC LL	0.0 PSF	HC-ENG	TCE/AF *
TOT.LD.	40.0 PSF	SEQN-	29351
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T7X8228Z01

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

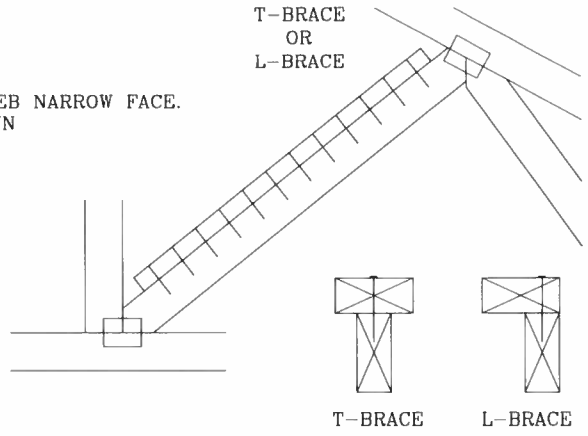
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

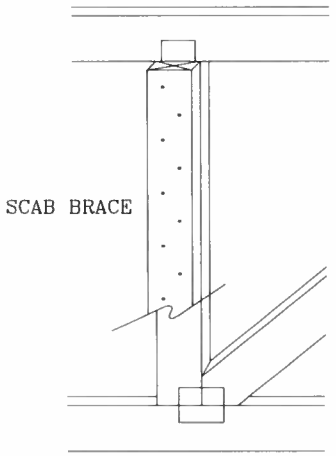
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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KARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

NICHOLAS
PAUL
GEISLER
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Road
Lake City, FL 32055
386/755-9021

FLORIDA BUILDING CODE SECTION 1609

COMPLIANCE SUMMARY

PROJECT: KELLY RESIDENCE, COLUMBIA COUNTY, FL (100 WIND ZONE)

TYPE OF CONSTRUCTION

ROOF: Hip Construction, Wood Trusses @ 24" O.C., SYP
WALLS: 2x4 Wood Studs @ 16" O.C.
FLOOR: 4" Thk. Conc. Slab, w/ Fibermesh concrete additive
FOUNDATION: Continuous Footer/Stemwall
EDGE STRIP: 3.0 ft. END ZONE: 6.0 ft.

ROOF DECKING

MATERIAL: 7/16" O.S.B.
SHEET SIZE: 48"x96" Sheets Placed Perpendicular to Roof Framing
FASTENERS: 8d Common Nails @ 5" O.C. Ends, 10" O.C. Interior

SHEAR WALLS

MATERIAL: 7/16" O.S.B. "WindStorm Sheathing"
SHEET SIZE: 48"x97 1/8" Sheets Placed Vertical
FASTENERS: 8d Common Nails @ 5" O.C. Edges, 10" O.C. Interior
DRAGSTRUT: Dbl. Top Plate Nailed w/ 16d Nails @ 16" O.C.
WALL STUDS: S-P-F Nr. 2 and better, 2x4 Studs @ 16" O.C.

HURRICANE UPLIFT CONNECTORS

TRUSS CLIPS: "Simpson" H9
WALL TENSION: 1/2" CDX plywd. w/ 8d Common Nails @ 4" O.C. Edges, 8" O.C. Interior for all exterior non-shear walls
HOLD-DOWN CONNECTORS: A307 Bolts, within 6" of corners
WALL SILL: 1/2" x 10" A.B., w/ 2" washers @ 48" o.c., 7" embedment
CORNER HOLD-DOWN DEVICE: "SIMPSON" HTT16, Ea. Corner

FOOTINGS AND FOUNDATIONS

HOUSE FOOTINGS: 20"x10" Continuous w/ 2 - #5 Rebars
HOUSE STEMWALL: 8" CMU w/ #5 Rebar Dowels Gd. 40, @ 72" O.C.
CONCRETE: Fb = 2500 p.s.i. or greater

PREPARER'S CERTIFICATION

I hereby certify that the attached Wind Load Design and Analysis calculations are in compliance with the Florida Building Code, Section 1606, to the best of my knowledge and belief.

Date: 27 Apr 2K7

Nicholas Paul Geisler, Architect AR0007005

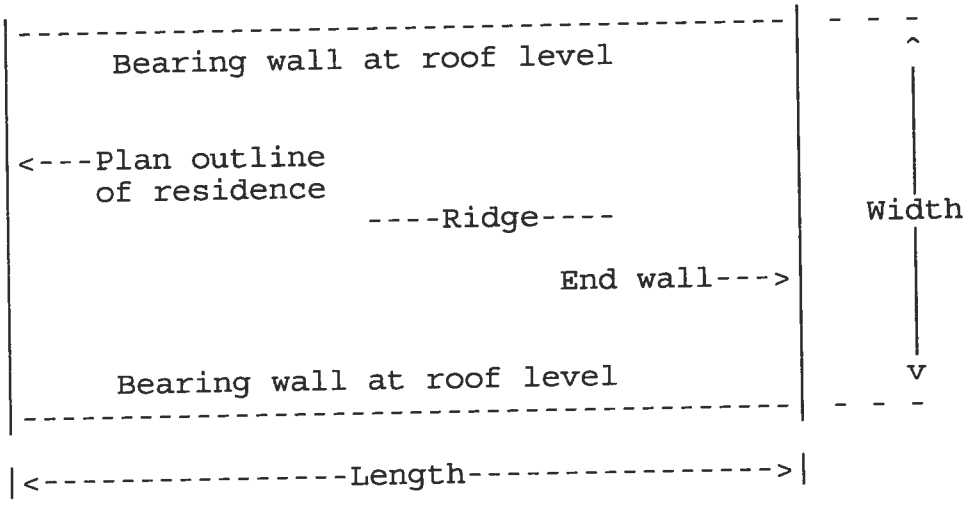
Data entry by: MT Date: 4 - 20 - 07

Project name: KELLY
Location : COLUMBIA COUNTY

R E S I D E N T I A L W I N D D E S I G N A N D A N A L Y S I S
A product of EDA Software, Inc.
Based on the Standard Building Code, 1994 edition

**** GENERAL INPUT DATA ****

Permanent construction
Simple rectangular building



Length along bearing walls out to out of studs = 57 feet
Width along end walls out to out of studs = 28 feet
Roof overhang in long direction from outer face of stud = 2 feet generally
Roof overhang at short end wall from outer face of stud = 2 feet generally
Height of exterior wall to top of plate on long side = 8 feet constant
Roof cross slope = 6 /12

Wind velocity = 110 mph

**** DEGREE OF ENCLOSURE ****

Assume that this building is an 'Enclosed building' per Code 1606.2.3.

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27 Apr 2007

**** STRUCTURAL FRAMING INPUT DATA ****

*** Roof Structural Data ***

Member number 1
Jack truss--hip-ended roof
Span length out to out of supports = 28 feet
Roof cross slope = 6 /12
Truss spacing = 24 inches
Overhang = 2 feet

Member number 2
Jack truss--hip-ended roof
Span length out to out of supports = 20 feet
Roof cross slope = 6 /12
Truss spacing = 24 inches
Overhang = 2 feet

*** Wall Structural Data ***

Spacing of wall studs = 16 inches
Total number of plates = 3
Wall stud number 1 is 8 feet high out to out of plates

COEFFICIENTS AND PRESSURES
Main Wind Force Resisting Systems

Actual pressure = Velocity pressure x Use factor x Coefficient
Wind velocity is 110 mph
Mean roof height is 11.87268 feet
Velocity pressure is 24.7 psf
Use factor is 1.0
Roof cross slope is 6 on 12, which equals 26.56505 degrees to horizontal
End zone width is 6 feet

	Coefficient	Design Pressure (psf)
End zone		
Windward wall (1E)	.7	17.29
Windward roof (2E)	-1	-24.7
Leeward roof (3E)	-1	-24.7
Leeward wall (4E)	-.95	-23.47
Overhang	-1.5	-37.06
Interior zone		
Windward wall (1)	.4	9.88
Windward roof (2)	-.75	-18.53
Leeward roof (3)	-.75	-18.53
Leeward wall (4)	-.7	-17.3
Overhang	-1.5	-37.06

ROOF LOADING--Roof Number 1 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 10.59222

Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf

ROOF LOADING--Roof Number 2 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 10.59222

Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf

ROOF LOADING--Roof Number 3 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 10.59222

Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf

ROOF LOADING--Roof Number 4 (pounds per square foot)

Roof cross slope = 6 inches per foot

Fiberglass shingles 240 # per square and 1 layer of 15 # felt	= 2.55
No insulation	
7/16 in. roof sheathing	= 1.31
2 in. x 4 in. wood trusses at 24 in. spacing	= 2.215147
Total roof unit weight on slope	= 6.075148
Cosine of roof cross slope	= .8944272
Roof unit weight on horizontal	= 6.792222
1 layer of 1/2 in. gypsum board ceiling--plain	= 2
Ceiling insulation R-30	= .5
Air-conditioning ductwork	= 1
Full lighting	= .3
Miscellaneous	= 0
Total	= 10.59222

Roof Unit Dead Load = 11 psf
Roof dead load supported generally by wall = 159.7911 plf

ROOF MEMBER DEAD LOAD REACTIONS AT BEARINGS
All values are in pounds

Roof member number 1 --Span 28 feet, Slope 6 /12, interior zone---- 319
Roof member number 2 --Span 28 feet, Slope 6 /12, end zone--------- 319
Roof member number 3 --Span 20.5 feet, Slope 6 /12, interior zone---- 240
Roof member number 4 --Span 20.5 feet, Slope 6 /12, end zone--------- 240

EXTERIOR WALL LOADING (pounds per linear foot)

Wood frame wall-- 8 ft. out to out plates

3--2 in. x 4 in. plates	= 2.865625
2 in. x 4 in. studs at 16 in. spacing	= 5.462598
R-13 Insulation	= 1.90625
Brick veneer siding	= 373.3333
1/2 in. Gypsum board--Total 1 layer---	= 16
Total	= 399.5678

Exterior Wall Unit Dead Load = 400 plf

S U M M A R Y O F H U R R I C A N E A N C H O R A N A L Y S I S

All values of forces are in pounds. Resistances have been increased for wind.
End zone width = 6 feet
Code: C = Compliance N = Non-compliance

Simpson hurricane anchors

Member 1 --Hip roof--Span 28 feet, at 24 inches oc--in interior zone:
Uplift = 771 Dead = 319 Net = 452 Model Special, Resistance = 717 C
Model H9--all nails installed per manufacturers catalog
Data supplied by operator--not from EDA database

Member 2 --Hip roof--Span 28 feet, at 24 inches oc--in end zone:
Uplift = 771 Dead = 319 Net = 452 Model Special, Resistance = 717 C
Model H9--all nails installed per manufacturers catalog
Data supplied by operator--not from EDA database

Member 3 --Hip roof--Span 20.5 feet, at 24 inches oc--in interior zone:
Uplift = 604 Dead = 240 Net = 364 Model Special, Resistance = 717 C
Model H9--all nails installed per manufacturers catalog
Data supplied by operator--not from EDA database

Member 4 --Hip roof--Span 20.5 feet, at 24 inches oc--in end zone:
Uplift = 604 Dead = 240 Net = 364 Model Special, Resistance = 717 C
Model H9--all nails installed per manufacturers catalog
Data supplied by operator--not from EDA database

**** ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM TRANSVERSE ****
Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is Oriented Strand Board, 7/16 inch thick
Sheathing has no intermediate blocking
Fasteners on panel ends are 8d nails spaced at 5 inches
Fasteners in panel interior are 8d nails spaced at 10 inches

Total lateral wind force on building = 10587 pounds
Total force transferred through diaphragm to shearwalls = 5293 pounds
Total length of shearwalls = 56 feet
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 21 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 94 plf
Allowable diaphragm force per unit length of shearwall = 251 plf

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*** Summary of Analysis ***
Roof sheathing diaphragm satisfies Code requirements.

**** ANALYSIS OF ROOF SHEATHING AS SHEAR DIAPHRAGM LONGITUDINAL ****
Shear analysis applies along supporting shearwalls.

Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is Oriented Strand Board, 7/16 inch thick
Sheathing has no intermediate blocking
Fasteners on panel ends are 8d nails spaced at 5 inches
Fasteners in panel interior are 8d nails spaced at 10 inches

Total lateral wind force on building = 4571 pounds
Total force transferred through diaphragm to shearwalls = 2285.5 pounds
Total length of shearwalls = 116 feet
MINIMUM REQUIRED TOTAL SHEARWALL LENGTH = 8.8 FT.--LOCATE EVENLY THROUGHOUT

Actual diaphragm force per unit length of shearwall = 19 plf
Allowable diaphragm force per unit length of shearwall = 251 plf

---

*** Summary of Analysis ***
Roof sheathing diaphragm satisfies Code requirements.

**** ANALYSIS OF ROOF SHEATHING FOR FASTENER WITHDRAWAL ****

Interior zone (area Ri)
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end trusses are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 656 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

Edge strip (area Si) width = 3 feet
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end trusses are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 1024 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

End zone (areas Se and C) width = 6 feet
Roof trusses are Southern Pine lumber, spaced at 24 inches
Sheathing is 7/16 inch with no intermediate blocking
Size of sheathing is 48 inches by 96 inches
Fasteners along end truss are 8d nails spaced at 5 inches
Fasteners along end wall are 8d nails spaced at 5 inches
Fasteners along int. trusses are 8d nails spaced at 10 inches
Total outward wind force on sheathing = 1417 pounds
Total withdrawal resistance of 40 nails = 3038 pounds (increased for wind)
Fastening of roof sheathing satisfies Code requirements.

**** ANALYSIS OF WALL STUDS ****

*** Analysis of Wall Stud Number 1 ***

2 in. x 4 in. single studs at 16 in. spacing
Stud height is 7.625 feet--located in interior zone
Top of studs is laterally supported by ceiling diaphragm or other method
Spruce--Pine--Fir lumber----Number 1--Number 2 grade
Sheathing is inch rated OSB, span rating 24/16

Cross-sectional area = 5.25 sq.in.
Moment of inertia = 5.359375 in.^4
Section Modulus = 3.0625 in.^3
Elastic modulus of wood stud = 1400000 in.^2

Total outward force on stud = 268 pounds
Stud moment = 255 ft-lb.

Stresses:
Stud bending vert : Actual = 1000 psi Allowable = 2415 psi (adjusted)
Stud shear : Actual = 35 psi Allowable = 112 psi (adjusted)
Stud tensile : Actual = 33 psi Allowable = 1020 psi (adjusted)
Interaction bending and tension actual/allowable stress ratio total = .4464316
Sheathing bending hor: Actual = 146 psi Allowable = 222 psi(adjusted)
Deflections:
Stud : Actual = .2226 in. Allowable = .5083 in.

*** Summary of Analysis ***

Wall structure satisfies all Code requirements.

**** ANALYSIS OF WALL STUDS ****

*** Analysis of Wall Stud Number 2 ***

2 in. x 4 in. single studs at 16 in. spacing
Stud height is 7.625 feet--located in end zone
Top of studs is laterally supported by ceiling diaphragm or other method
Spruce--Pine--Fir lumber----Number 1--Number 2 grade
Sheathing is inch rated OSB, span rating 24/16

Cross-sectional area = 5.25 sq.in.
Moment of inertia = 5.359375 in.^4
Section Modulus = 3.0625 in.^3
Elastic modulus of wood stud = 1400000 in.^2

Total outward force on stud = 309 pounds
Stud moment = 294 ft-lb.

Stresses:
Stud bending vert : Actual = 1154 psi Allowable = 2415 psi (adjusted)
Stud shear : Actual = 40 psi Allowable = 112 psi (adjusted)
Stud tensile : Actual = 33 psi Allowable = 1020 psi (adjusted)
Interaction bending and tension actual/allowable stress ratio total = .5101997
Sheathing bending hor: Actual = 169 psi Allowable = 222 psi(adjusted)
Deflections:
Stud : Actual = .2567 in. Allowable = .5083 in.

*** Summary of Analysis ***

Wall structure satisfies all Code requirements.

**** ALLOWABLE STRESS PROPERTIES ****

Base stresses (psi):
Wood:
   Bending   = 875
   Tension   = 425
   Shear   = 70
   Elastic modulus = 1400000

Adjustment factors for wood:
   Duration (Du)   = 1.6
   Wet service (Wt)   = 1
   Temperature (Tm)   = 1
   Stability (St)   = 1
   Size (Sz)   = 1.5
   Volume (Vm)   = 1
   Flat use (Fu)   = 1
   Repetitive (Rp)   = 1.15
   Curvature (Cu)   = 1
   Form (Fm)   = 1
   Shear stress (Sh) = 1

Allowable stresses (psi):
Wood:
   Bending = 2415 (Base x Du x Wt x Tm x St x Sz x Vm x Fu x Rp x Cu x Fm)
   Tension = 1020 (Base x Du x Wt x Tm x Sz)
   Shear   = 112 (Base x Du x Wt x Tm x Sh)
   Elastic modulus = 2240000 (Base x Wt x Tm)

Sheathing:
   Bending   = 222 (Base x 1.33)
   Elastic modulus = 61904.76 (Base)

T R A N S V E R S E D R A G S T R U T N A I L A N A L Y S I S

Wall framing is 2 in. x 4 in. studs
Wall stud framing lumber is Spruce--Pine--Fir
Fasteners are 16d common nails
Approximate nail spacing = 20 inches

Total lateral force on building = 10587 pounds
Force applied at top of walls = 5293 pounds
Total dragstrut length = 56 feet
Shear per unit dragstrut length = 94 pounds per linear foot

Actual shear on each nail = 156 pounds
Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

L O N G I T U D I N A L D R A G S T R U T N A I L A N A L Y S I S

Wall framing is 2 in. x 4 in. studs
Wall stud framing lumber is Spruce--Pine--Fir
Fasteners are 16d common nails
Approximate nail spacing = 20 inches

Total lateral force on building = 4571 pounds
Force applied at top of walls = 2285 pounds
Total dragstrut length = 116 feet
Shear per unit dragstrut length = 19 pounds per linear foot

Actual shear on each nail = 31 pounds
Allowable shear on each nail = 192 pounds

Dragstrut nailing satisfies Code requirements.

**** T R A N S V E R S E S H E A R W A L L A N A L Y S I S ****

Wall framing is 2 in. x 4 in. studs at 16 inch spacing
Wall stud framing lumber is Spruce--Pine--Fir
Wall shear siding is Oriented Strand Board -- 7/16 inch thick
Wall sheathing has all edges nailed
Fasteners: 8d common nails spaced along edges at 5 inch centers
Fasteners: 8d common nails spaced in interior at 10 inch centers

Total lateral force on building = 10587 pounds
Force applied at top of walls = 5293 pounds
Accumulated total shearwall length = 56 feet

Actual unit shear on shearwalls = 94 pounds per linear foot
Allowable unit shear on shearwalls = 257 pounds per linear foot

Shearwall satisfies Code requirements.

**** L O N G I T U D I N A L S H E A R W A L L A N A L Y S I S ****

Wall framing is 2 in. x 4 in. studs at 16 inch spacing
Wall stud framing lumber is Spruce--Pine--Fir
Wall shear siding is Oriented Strand Board -- 7/16 inch thick
Wall sheathing has all edges nailed
Fasteners: 8d common nails spaced along edges at 5 inch centers
Fasteners: 8d common nails spaced in interior at 10 inch centers

Total lateral force on building = 4571 pounds
Force applied at top of walls = 2285 pounds
Accumulated total shearwall length = 116 feet

Actual unit shear on shearwalls = 19 pounds per linear foot
Allowable unit shear on shearwalls = 257 pounds per linear foot

Shearwall satisfies Code requirements.

*** ANALYSIS OF OUTWARD FORCES ON WALL SHEATHING ***

Wall number 1 : Total outward wind force on sheathing = 804 pounds
: Total withdrawal resistance of 76 nails = 4240 pounds

Wall number 2 : Total outward wind force on sheathing = 927 pounds
: Total withdrawal resistance of 76 nails = 4240 pounds

**** ANALYSIS OF SHEATHING FASTENERS ****

Wall framing is Spruce--Pine--Fir lumber
Sheathing is 7/16 inch Oriented Strand Board
Sheathing extends from bottom of bottom plate to top of top plate
Fasteners are 8d common nails at 5 inch spacing

Total uniform wind uplift in first story at top of wall level = 292 plf
Uniform dead loads per linear foot:
Roof = 159.7911 plf

Total = 159.7911 plf
Total uniform dead load in first story at top of wall level = 159 plf
Net wind uplift in first story at top of wall level = 133 plf

Total uplift force on each nail = 55 pounds
Allowable shear on each nail = 97 pounds (increased for wind)
Sheathing to plate fastening satisfies all Code requirements.

**** ANALYSIS OF SHEATHING FASTENERS ****

Wall framing is Spruce--Pine--Fir lumber
Sheathing is 7/16 inch Oriented Strand Board
Sheathing extends from bottom of bottom plate to top of top plate
Fasteners are 8d common nails at 5 inch spacing

Total uniform wind uplift in first story at floor level = 292 plf
Uniform dead loads per linear foot:
Roof = 159.7911 plf
Wall = 399.5678 plf

Total = 559.3589 plf
Total uniform dead load in first story at floor level = 559 plf
Net wind uplift in first story at floor level =-267 plf

Total uplift force on each nail =-112 pounds
Allowable shear on each nail = 97 pounds (increased for wind)
Sheathing to plate fastening satisfies all Code requirements.

**** ANALYSIS OF FOUNDATION ANCHORAGE ****

Anchor bolts are 1/2 inch A307, with 2 inch round washer at 48 inch centers.

Total uniform wind uplift on foundation = 290 pounds per linear foot
Uniform dead loads in pounds per linear foot:
Roof = 159.7911 plf
Wall = 38.42197 plf

Total = 198.2131 plf
Total uniform dead load times 2/3 = 132 pounds per linear foot
Net uplift force on foundation = 158 pounds per linear foot

Total uplift force on each anchor bolt = 632 pounds
Safe tension value of each anchor bolt = 1634 pounds (increased by 1/3)
Bolt safe tension value is governed by washer failure

*** Summary of Analysis ***
Foundation anchorage satisfies all Code requirements.

**** ANALYSIS OF CORNER HOLD-DOWN REQUIREMENTS ****

Hold-down is one typical anchor bolt with washer, each wall
Normal anchor bolt spacing = 48 inches
Distance from corner to hold-down device = 6 inches
Distance from corner to first interior anchor bolt = 48 inches
Net uplift force on foundation = 158 pounds per linear foot
Tributary distance to corner device = 2.25 feet
Net uplift on corner hold-down device = 355 pounds

Uplift tension due to shearwall action in a transverse shearwall segment:
Distance from corner to hold-down device = 6 inches
Distance from corner to first interior anchor bolt = 48 inches
Total shear from shearwall segment = 319 pounds
Height of wall = 8 feet
Uniform dead load times 2/3 = 25 pounds per linear foot
Shearwall moment at bottom of wall = 2552 foot-pounds
Additional tension at corner device = 1237 pounds
Total uplift tension on corner hold-down devices = 1592 pounds
Allowable tension on corner hold-down devices = 3268 pounds

*** Summary of Analysis ***
Corner hold-down device COMPLIES with Code requirements.

**** ANALYSIS OF FOUNDATION ****

Stemwall is 8 inch concrete masonry, filled with grout, 16 inches high
Footing is 20 inches wide by 10 inches deep
Earth cover over top of footing is 4 inches

Total uniform wind uplift on foundation = 290 pounds per linear foot
Uniform dead loads in pounds per linear foot:
Roof = 159.7911 plf
Wall = 38.42197 plf

Total = 198.2131 plf
Total uniform dead load times 2/3 = 132 pounds per linear foot
Net uplift force at top of foundation = 158 pounds per linear foot
Weight of stemwall footing earth x 2/3 = 261 pounds per linear foot
Net uplift at bottom of footing = 0 pounds per linear foot

*** Summary of Analysis ***
Foundation is stable.

**** ANALYSIS OF REINFORCING STEEL ****

Grade 40 reinforcing steel, Number 5 vert. bars at 72 inch centers

Total uniform wind uplift on foundation = 290 pounds per linear feet
Uniform dead loads in pounds per linear foot:
Roof = 159.7911 plf
Wall = 38.42197 plf

Total = 198.2131 plf
Total uniform dead load times 2/3 = 132 pounds per linear foot
Net uplift force on foundation = 158 pounds per linear foot
Weight of concrete block stemwall x 2/3 = 81 pounds per linear foot
Net uplift at top of footing = 77 pounds per linear foot

Total uplift force on each re-bar = 462 pounds
Safe tension value of each re-bar = 8181 pounds (increased by 1/3)

*** Summary of Analysis ***
Reinforcing steel satisfies all Code requirements.

**** SUMMARY OF REINFORCING DATA ****

Foundation wall data:
Wall is composed of 8 inch concrete masonry, fully grouted.
Wall reinforcing is Grade 40 steel, Number 5 at 72 inch centers
Minimum required lap splice for Number 5 bar is 25 inches.
Minimum required clearance for Number 5 bar is 1.5 inches.
Wall reinf. in footing has a std. A.C.I. hook, 6 inches below top of footing.

Footing data:
Footing is continuous, 20 inches wide by 10 inches deep.
Footing concrete is 2500 psi
Footing reinforcing is Grade 40 steel, 2--#( ) longitudinal.
Minimum required splice length = 25 inches
Reinforcing steel shall have cover as follows:
    Top-------6 inches
    Sides-----3 inches
    Bottom----3 inches

Architectural Testing

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 450
TYPE: Aluminum Single Hung Window
RATING: H-C30 54 x 90; H-C45 52 x 72*

Title of Test	Results	
	Test Specimen #1	Test Specimen #2
Overall Design Pressure	30 psf	47 psf
Operating Force	20 lb max.	N/A
Air Infiltration	0.27 cfm/ft^2	N/A
Water Resistance	5.25 psf	6.0 psf
Structural Test Pressure	±45.0 psf	±70.5 psf
Deglazing	Passed	N/A
Forced Entry Resistance	Grade 10	N/A

Reference should be made to Report No. 01-37589.01 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Adam A. Fodor, Technician

AAFTg0

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testatl.com

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INCORPORATED
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-37589.01
Test Date: 06/29/00
Report Date: 09/11/00
Expiration Date: 06/29/04

Project Summary: Architectural Testing, Inc. (ATI) was contracted to witness tests on a Series/Model 450, aluminum single hung window at the MI Home Products in-plant test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1 H-C30 54 x 90; Test Specimen #2 H-C40 52 x 72*. Test specimen descriptions and results are reported herein.

General Note: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the minimum test size for the product type and class.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 450

Type: Aluminum Single Hung Window

Test Specimen #1 H-C30 54 x 90

Overall Size: 4' 6-1/2" wide by 7' 6-1/2" high

Sash Size: 4' 4" wide by 3' 9-3/4" high

Fixed Daylight Opening Size: 4' 1-1/2" wide by 3' 6-1/2" high

Screen Size: 4' 2-1/4" wide by 3' 8-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.testati.com

Test Specimen Description: (Continued)

Test Specimen #2: H-C40 52 x 72*

Overall Size: 4' 4-1/4" wide by 6' 0" high

Sash Size: 4' 2" wide by 3' 0-1/2" high

Fixed Daylight Opening Size: 3' 11-1/2" wide by 2' 9-1/2" high

Screen Size: 4' 0" wide by 2' 11" high

The following descriptions apply to all specimens.

Finish: All aluminum was painted.

Glazing Details: The lites utilized 5/8" thick sealed insulating glass units fabricated from two sheets of 3/32" thick clear annealed glass and an Intercept™ spacer system. The sash was channel glazed with a flexible gasket. The fixed lite was interior glazed onto single-sided adhesive foam tape and secured with extruded PVC glazing beads.

Weatherstripping:

Description	Quantity	Location
0.210" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Stiles
0.300" diameter by 0.187" backed foam-filled vinyl bulb gasket	Row	Bottom rail
0.400' high by 1/2" square polypile dust plug	4	One on each sash corner

Frame Construction: The main frame was constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners. The fixed meeting rail was constructed of an extruded aluminum member with coped, butted and sealed ends fastened with two screws each.

Test Specimen Description: (Continued)

Sash Construction: The sash members were constructed of thermally-broken extruded aluminum members with coped, butted and sealed corners fastened with one screw each.

Screen Construction: The screen was constructed of rolled aluminum members with plastic keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Plastic snap latch	1	Midspan of bottom rail
Block and tackle balance system	2	One per jamb
Plastic tilt latch	2	One on each end of sash meeting rail
Metal pivot bar	2	One on each end of bottom rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test unit was installed into the nominal 2" x 8" Spruce-Pine-Fir #2 wood test buck utilizing the integral nailing fin secured with 1" long galvanized roofing nails, 6" from each corner and every 18" on center. The nailing fin was also bedded in polyurethane. The exterior perimeter was blindstopped with wood members and secured with #8 x 3" screws every 24" on center.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-C30 54 x 90			
2.2.1.6.1	Operating Force	20 lbs	45 lbs max.
	Air Infiltration per ASTM E 283 (See Note #1) @ 1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.
	Water Resistance per ASTM E 547 (with and without screen) WTP – 4.5 psf	No leakage	No leakage
2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail)		
	@ 45.0 psf (exterior)	0.03"	0.22" max.
	@ 45.0 psf (interior)	0.04"	0.22" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction at 70 lbs		
	Meeting rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Test Results:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) WTP – 5.25 psf	No leakage	No leakage
Test Specimen #2: H-C40 52 X 72*			
Optional Performance			
4.3	Water Resistance per ASTM E 547 and 331 (with and without screen) WTP – 6.0 psf	No leakage	No leakage
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the fixed meeting rail) (Loads held for 33 seconds)		
	@ 47.0 psf (exterior)	0.04"	N/A
	@ 47.0 psf (interior)	0.03"	N/A
	(Loads held for 10 seconds)		
	@ 70.5 psf (exterior)	0.07"	0.21" max.
	@ 70.5 psf (interior)	0.04"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Adam A. Fodor
Technician

Bruce W. Croak
Director – Product/Physical Testing

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