DATE 07/28/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT 000027207

APPLICANT PAMELA SMITH PHONE 786.295.9296

ADDRESS 377 SW MAULDIN AVE LAKE CITY FL 32024

OWNER BRJ VENTURES,LLC. PHONE 786.229.9638

ADDRESS 219 NW MELANIE WAY LAKE CITY FL 32055

CONTRACTOR LOUIS SCHWARTZ PHONE 305 542-9193

LOCATION OF PROPERTY 41N, TL ON MOORE RD, TL ON MELANIE WAY, 3RD LOT ON LEFT.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 89700.00

HEATED FLOOR AREA 1326.00 TOTAL AREA 1794.00 HEIGHT 18.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 14-3S-16-02123-043 SUBDIVISION CHADWORTH

LOT 4 BLOCK F PHASE UNIT TOTAL ACRES 1.08

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
		CBC1253816	Pamela Smith	
Driveway Connection	**Septic Tank Number**	**LU & Zoning checked by**	**Approved for Issuance**	**New Resident**
EXISTING	08-0421	BLK	WR	Y

COMMENTS: LEGAL NON-CONFORMING LOT OF RECORD. 1 FOOT ABOVE ROAD. NOC on file.

Check # or Cash 1001

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ______ date/app. by   Foundation ______ date/app. by   Monolithic ______ date/app. by

Under slab rough-in plumbing ______ date/app. by   Slab ______ date/app. by   Sheathing/Nailing ______ date/app. by

Framing ______ date/app. by   Rough-in plumbing above slab and below wood floor ______ date/app. by

Electrical rough-in ______ date/app. by   Heat & Air Duct ______ date/app. by   Peri. beam (Lintel) ______ date/app. by

Permanent power ______ date/app. by   C.O. Final ______ date/app. by   Culvert ______ date/app. by

M/H tie downs, blocking, electricity and plumbing ______ date/app. by   Pool ______ date/app. by

Reconnection ______ date/app. by   Pump pole ______ date/app. by   Utility Pole ______ date/app. by

M/H Pole ______ date/app. by   Travel Trailer ______ date/app. by   Re-roof ______ date/app. by

---

BUILDING PERMIT FEE $ 450.00 CERTIFICATION FEE $ 8.97 SURCHARGE FEE $ 8.97

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 542.94

INSPECTORS OFFICE [signature] CLERKS OFFICE CN

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Craft 1001 - 1002

Columbia County Building Permit Application

For Office Use Only Application # 0807-40 Date Received 7/16/08 By G Permit # 27207

Zoning Official BLK Date 23.07.08 Flood Zone X Land Use A-3 Zoning A-3

FEMA Map # N/A Elevation N/A MFE 1ft above Rd River N/A Plans Examiner WR Date 7/22/08

Comments Legal Non-conforming Lot of Record

☑ NOC ☐ EH ☑ Deed or PA ☑ Site Plan ☐ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter

IMPACT FEES: EMS $29.88 Fire $78.63 Corr $409.16 Road/Code $1,046.00 /210

School $1,500.00 = TOTAL $3,063.67

Septic Permit No. 08-0421 Fax 386-755-6824

Name Authorized Person Signing Permit John or Pam Smith Phone 786-295-9296

Address 377 SW Mauldin Ave Lake City, FL 32024

Owners Name BRJ Ventures, LLC Phone 786-229-9638

911 Address 219 NW Melanie Way Lake City, FL 32055

Contractors Name Catalina Caststone Creations Inc Phone 305-542-9193

Address 9801 SW 121 St. Miami, FL 33176

Fee Simple Owner Name & Address BRJ Ventures, LLC 830 North Krome Ave Homestead FL 33030

Bonding Co. Name & Address

Architect/Engineer Name & Address Architect Mark Haddox-755-2411 / Engineer Mark Disosway

Mortgage Lenders Name & Address

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy

Property ID Number 14-3S-16-02123-043 Estimated Cost of Construction 85,000

Subdivision Name Chadworth Lot 4 Block E Unit ___ Phase ___

Driving Directions 41 North to Moore Rd, turn left, 1½ miles to Melanie Way, turn left 3rd lot on left

Number of Existing Dwellings on Property 0

Construction of Residental SFD Total Acreage 1.08 Lot Size ___

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 18'3"

Actual Distance of Structure from Property Lines - Front 105' Side 40' Side 60' Rear 144'

Number of Stories 1 Heated Floor Area 1326 Total Floor Area 1794 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Page 1 of 2 (Both Pages must be submitted together.)

Spoke to Pam 7/23/08

Revised 1-10-0

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

B.R.J Ventures L.L.C

John Smith managing member

Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

John Smith

Contractor's Signature (Permitee)

Contractor's License Number CBC1253816

Columbia County Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 16 **day of** July **20**08.

Personally known ✓ **or Produced Identification** _____

State of Florida Notary Signature (For the Contractor)

SEAL:

Notary Public State of Florida
Judi M Lowrey
My Commission DD477506
Expires 10/02/2009

Louis R. Schwartz
Catalina Caststone Creations
Lake City Office
377 SW Mauldin Ave.
Lake City, Fl. 32024
License Number: 7001785

To Whom It May Concern:
This letter is to inform you that I give permission to John J. or Pamela T. Smith to authorize work done in my name at my property located at lot 4 & 5 on Melanie Way in Lake City,Fl. County of Columbia. Parcel Id #14-3S-16-02123-043. Mr Smith will be the site supervisor at this location and can be reached at 786.299.9638.

Signature of Affiant: Louis R. Schwartz

CERTIFICATE OF ACKNOWLEDGMENT OF NOTARY PUBLIC

Sworn to (or affirmed) and subscribed before me this 8th day of June, 2007

By Louis R. Schwartz. The affiant is ✓ personally known to me,

Or ___ produced the following identification: _____

Notary Seal:

JUDY BAKER WASHINGTON
Notary Public - State of Florida
My Commission Expires Apr 28, 2011
Commission # DD 660648
Bonded Through National Notary Assn.

Judy Baker Washington
JUDY BAKER WASHINGTON
Signature of Notarial Officer

Notary Public for the State of Florida

My commission expires: _____

Application # 0707-39
Chadworth Lot 5

FLORIDA DEPARTMENT OF STATE
DIVISION OF CORPORATIONS

Detail by Entity Name

Florida Limited Liability Company

BRJ VENTURES, LLC

Filing Information

Document Number	L08000066453
FEI Number	NONE
Date Filed	07/09/2008
State	FL
Status	ACTIVE
Effective Date	07/09/2008
Last Event	LC AMENDMENT
Event Date Filed	07/24/2008
Event Effective Date	NONE

Principal Address

830 NORTH KROME AVENUE
HOMESTEAD FL 33030 US

Mailing Address

830 NORTH KROME AVENUE
HOMESTEAD FL 33030 US

Registered Agent Name & Address

LYNN, SANDRA T
830 NORTH KROME AVENUE
HOMESTEAD FL 33030 US

Manager/Member Detail

Name & Address

Title MGR

SCHWARTZ, LOUIS R
9801 SW 121ST STREET
MIAMI FL 33176 US

Title MGR

SMITH, JOHN J
377 SW MAULDIN AVENUE
LAKE CITY FL 32024

Annual Reports

No Annual Reports Filed

Document Images

Electronic Articles of Organization For Florida Limited Liability Company

L08000066453
FILED 8:00 AM
July 09, 2008
Sec. Of State
gharvey

Article I

The name of the Limited Liability Company is:

BRJ VENTURES, LLC

Article II

The street address of the principal office of the Limited Liability Company is:

830 NORTH KROME AVENUE
HOMESTEAD, FL. US 33030

The mailing address of the Limited Liability Company is:

830 NORTH KROME AVENUE
HOMESTEAD, FL. US 33030

Need new papers

Article III

The purpose for which this Limited Liability Company is organized is:

ANY AND ALL LAWFUL BUSINESS.

Article IV

The name and Florida street address of the registered agent is:

SANDRA T LYNN
830 NORTH KROME AVENUE
HOMESTEAD, FL. 33030

Having been named as registered agent and to accept service of process for the above stated limited liability company at the place designated in this certificate, I hereby accept the appointment as registered agent and agree to act in this capacity. I further agree to comply with the provisions of all statutes relating to the proper and complete performance of my duties, and I am familiar with and accept the obligations of my position as registered agent.

Registered Agent Signature: SANDRA T. LYNN

L08000066453
FILED 8:00 AM
July 09, 2008
Sec. Of State
gharvey

Article V

The name and address of managing members/managers are:

Title: MGR
LOUIS R SCHWARTZ
9801 SW 121ST STREET
MIAMI, FL. 33176 US

Article VI

The effective date for this Limited Liability Company shall be:

07/09/2008

Signature of member or an authorized representative of a member

Signature: SANDRA T. LYNN

The undersigned certifies that the foregoing resolution was adopted by the members of BRJ VENTURES, LLC on the date hereinafter set forth.

DATED: 7/9/08

Weisleder Associates, Inc.

MEMBER

Catalina Caststone Creations, Inc.

MEMBER

John J. Smith, MEMBER

Columbia County Property Appraiser

DB Last Updated: 4/15/2008

2008 Proposed Values

Tax Record | Property Card | Interactive GIS Map | Print

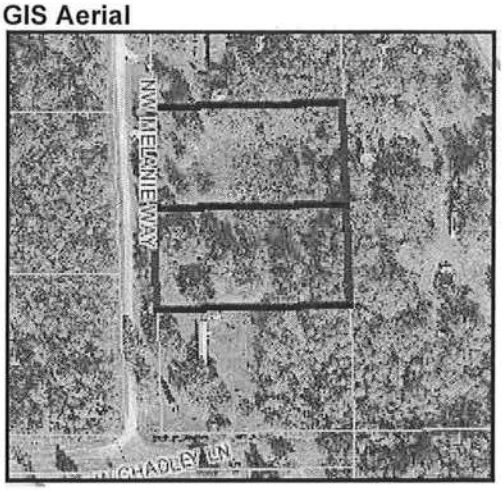
Parcel: 14-3S-16-02123-043

Search Result: 1 of 1

Owner & Property Info

Owner's Name	SCHWARTZ LOUIS R		
Site Address			
Mailing Address	9801 SW 121 STREET MIAMI, FL 33176		
Use Desc. (code)	VACANT (000000)		
Neighborhood	14316.01	**Tax District**	3
UD Codes	MKTA03	**Market Area**	03
Total Land Area	2.298 ACRES		
Description	COMM NE COR OF NW1/4 OF SE1/4, RUN S 473.50 FT FOR POB, CONT S 323.50 FT, W 310 FT, N 323.5 FT, E 310 FT TO POB. (AKA LOT 4,& 5 BLOCK F CHADWORTH S/D UNREC) ORB 611-651, WD 1051-803. POA 1115-414. WD 1115-415.		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (1)	$28,000.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$28,000.00

Just Value	$28,000.00
Class Value	$0.00
Assessed Value	$28,000.00
Exempt Value	$0.00
Total Taxable Value	$28,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
3/29/2007	1115/415	WD	V	U	01	$55,000.00
7/8/2005	1051/803	WD	V	U	08	$15,000.00
12/1/1986	611/651	WD	V	U	01	$4,770.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

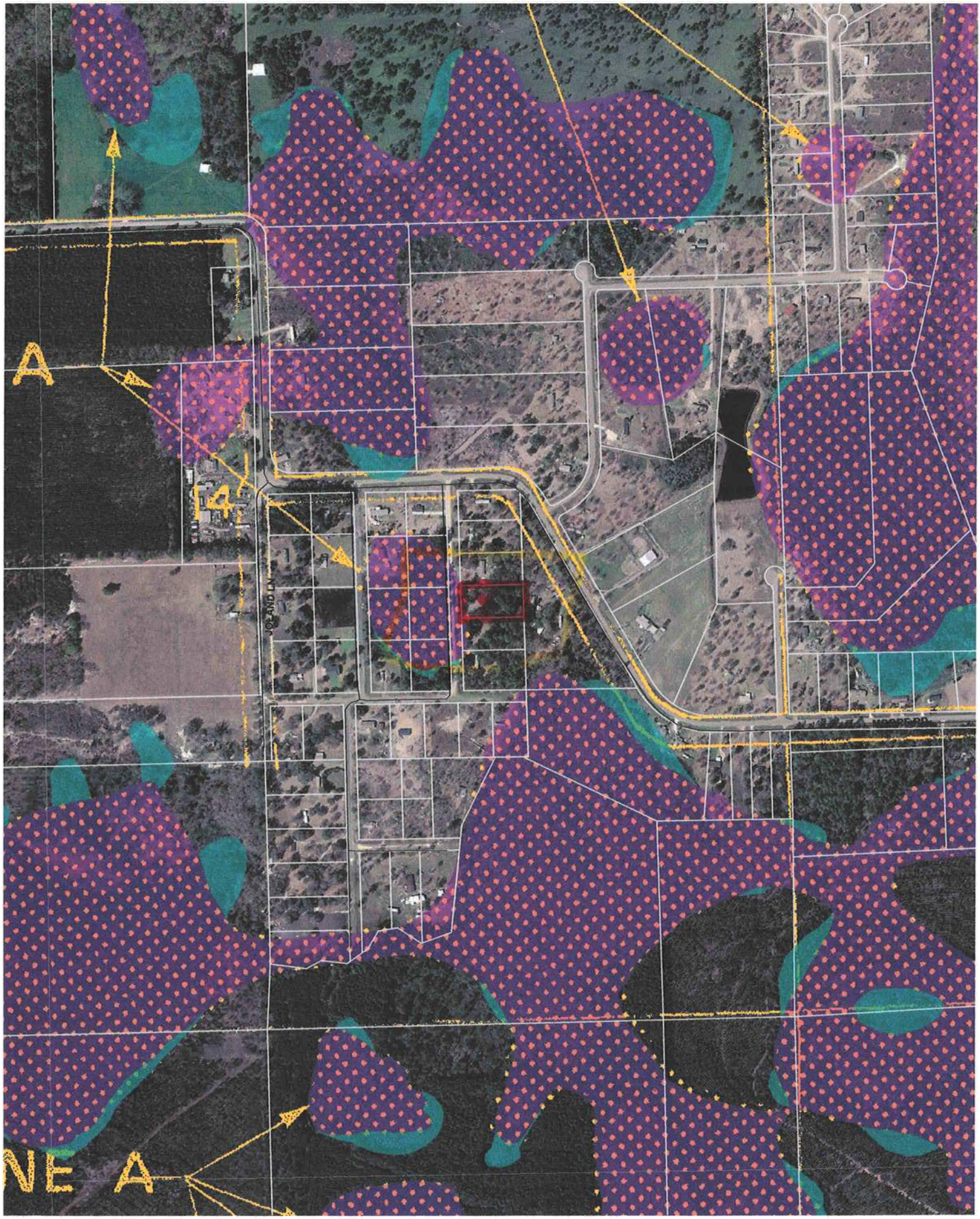
Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	2.000 LT - (2.298AC)	1.00/1.00/1.00/1.00	$14,000.00	$28,000.00

Columbia County Property Appraiser DB Last Updated: 4/15/2008

1 of 1



0807-40

Prepared by and return to:
Sandra T. Lynn, Esq.
Attorney at Law
Turner & Lynn, P.A.
830 North Krome Avenue
Homestead, FL 33030
305-247-6521

Inst:200812012934 Date:7/10/2008 Time:12:07 PM
Doc Stamp-Deed:192.50
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1154 P:686

[Space Above This Line For Recording Data]

Quit Claim Deed

This Quit Claim Deed made this 9 day of July, 2008 between **Louis R. Schwartz, a married man**, whose post office address is **9801 SW 121st Street, Miami, FL 33176**, grantor, and **BRJ Ventures, LLC, a Florida limited liability company,** whose post office address is **830 North Krome Avenue, Homestead, FL 33030**, grantee:

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum TEN AND NO/100 DOLLARS ($10.00) and other good and valuable consideration to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, does hereby remise, release, and quitclaim to the said grantee, and grantee's heirs and assigns forever, all the right, title, interest, claim and demand which grantor has in and to the following described land, situate, lying and being in **Columbia County, Florida** to-wit:

Lot 4, Block F: Commence at the NE corner of the NW 1/4 of SE 1/4, Section 14, Township 3 South, Range 16 East, Columbia County, Florida and run thence S l°20'56" E along the East line of said NW 1/4 of SE 1/4, 473.50 feet to the Point of Beginning, thence continue S l°20'56" E along said East line, 161.75 feet, thence S 89°30' W, 310.00 feet to the East line of Melanie Lane, thence N 1°20'56" W along the said East line, 161.75 feet, thence N 89°30' E, 310.00 feet to the Point of Beginning. Containing 1.08 acres, more or less.

Parcel Identification Number: 14-3S-16-02123-043

Grantor warrants that at the time of this conveyance, the subject vacant property is not the Grantor's homestead within the meaning set forth in the constitution of the state of Florida, nor is it contiguous to or a part of homestead property. Grantor's residence and homestead address is: 9801 SW 121 Street, Miami, FL 33176.

To Have and to Hold, the same together with all and singular the appurtenances thereto belonging or in anywise appertaining, and all the estate, right, title, interest, lien, equity and claim whatsoever of grantors, either in law or equity, for the use, benefit and profit of the said grantee forever.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Witness Name: SANDRA T. Lynn

Angela D. Wadsworth
Witness Name: ANGELA D. WADSWORTH

Louis R. Schwartz
Louis R. Schwartz

State of Florida
County of Miami-Dade

The foregoing instrument was acknowledged before me this 9 day of July, 2008 by Louis R. Schwartz, who [_] is personally known or [X] has produced a driver's license as identification.

[Notary Seal]

ANGELA D. WADSWORTH
Commission DD 776123
Expires May 9, 2012
Bonded Thru Troy Fain Insurance 800-385-7019

Angela D. Wadsworth
Notary Public

Printed Name: _____

My Commission Expires: _____

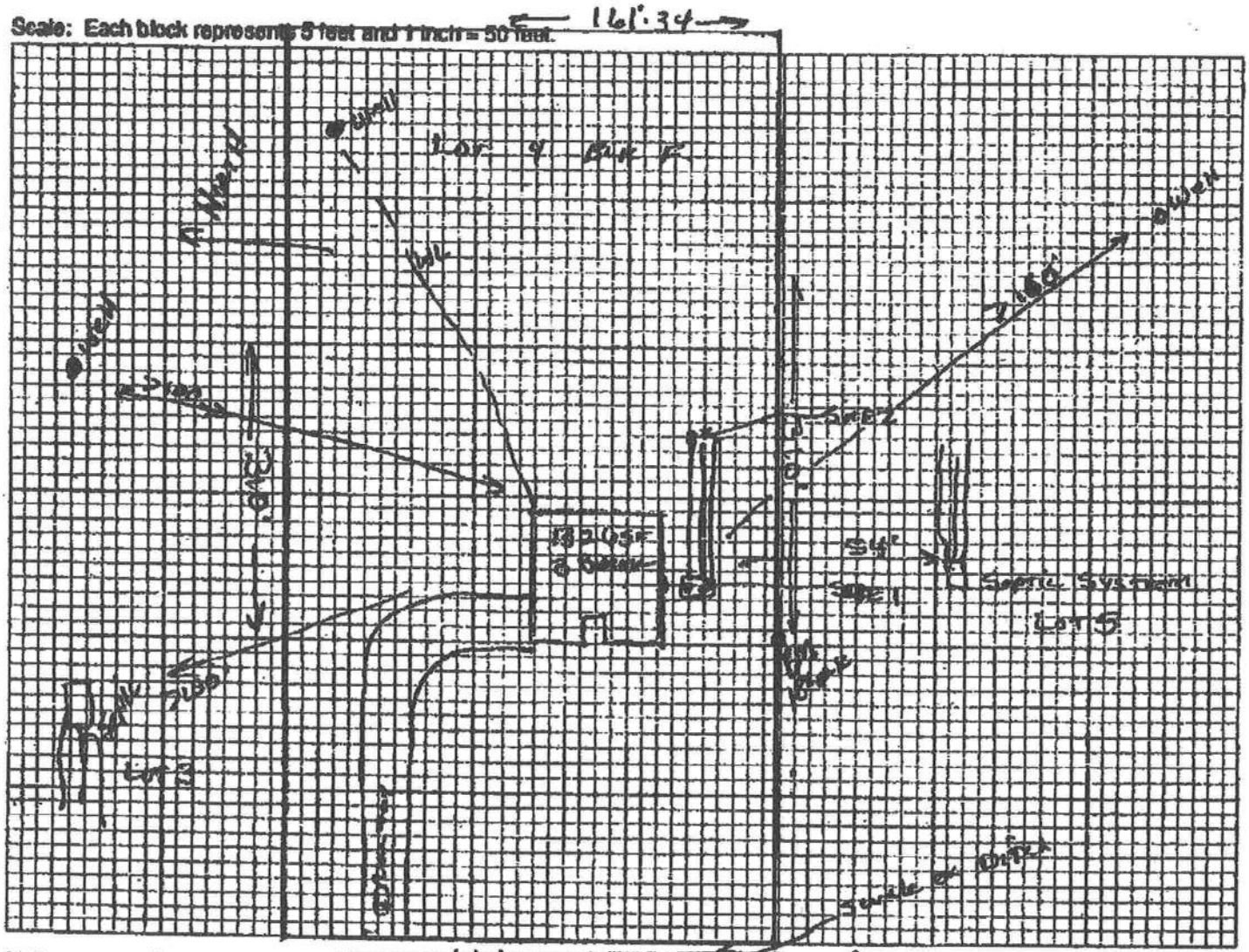
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 08-0421

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:

Lot 4 Blk F Chadworth

1.08 acres

Louis Schwartz (Pam Smith)

Site Plan submitted by: Robert W Dodd
Signature

Agent
Title

Plan Approved ✓ Not Approved ____

Date 6-10-08

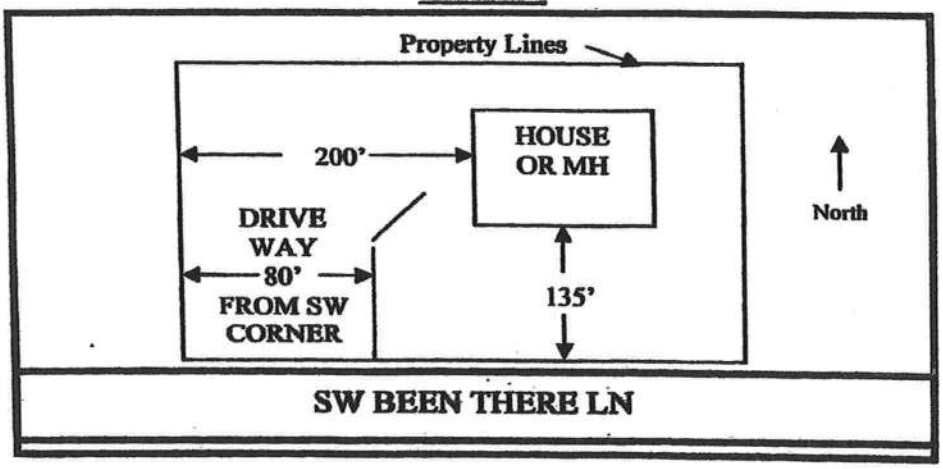
By M R D Jr Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

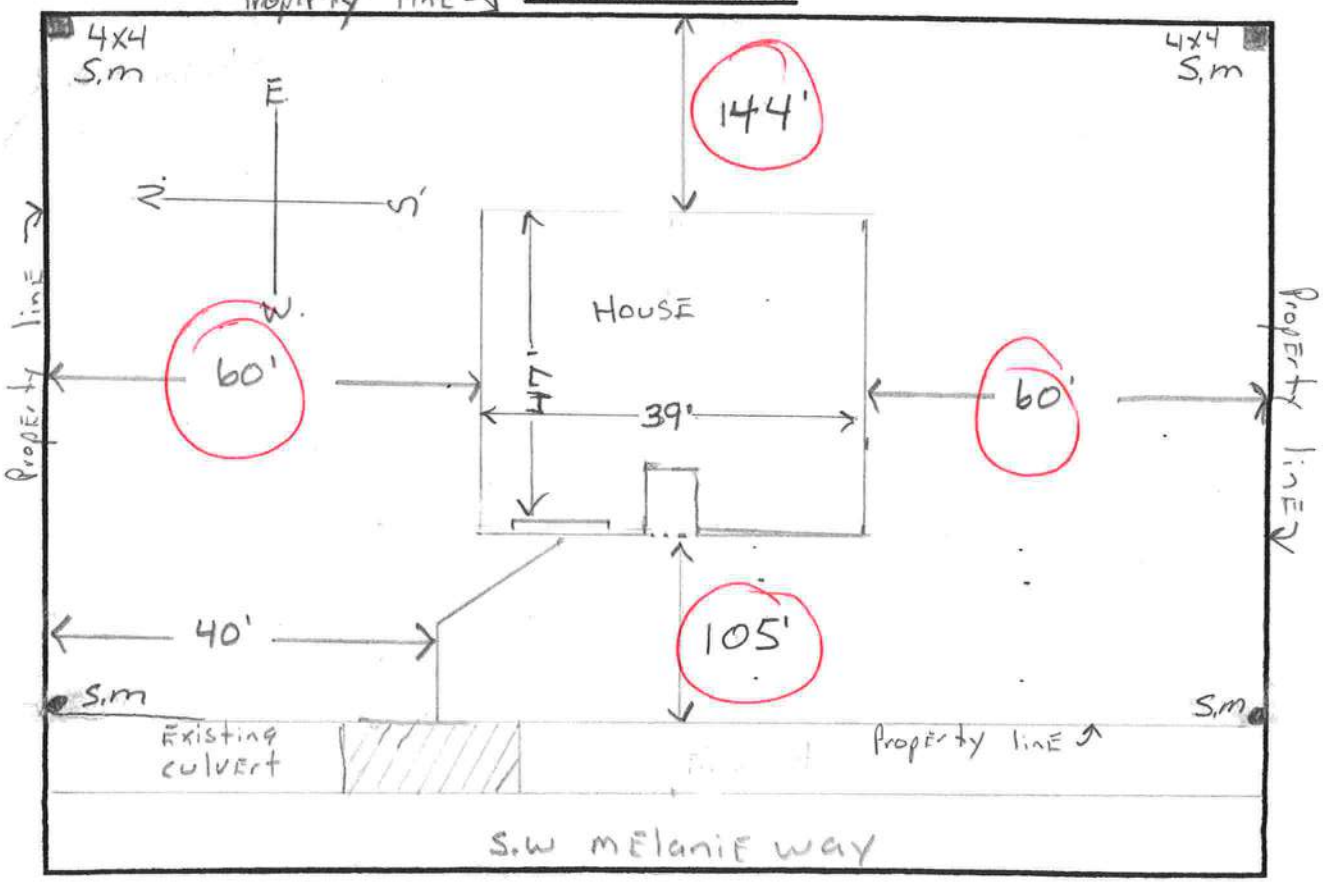
DH 4015, 10/08 (Replaces HRS-H Form 4015 which may be used)
(Stock Number: 5764-009-4015-0)

1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



SITE PLAN BOX:



COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 6/9/2008 **DATE ISSUED:** 6/10/2008

ENHANCED 9-1-1 ADDRESS:

219 NW MELANIE WAY

LAKE CITY FL 32055

PROPERTY APPRAISER PARCEL NUMBER:

14-3S-16-02123-043

Remarks:

PARENT PARCEL, LOT 4 BLOCK F CHADWORTH S/D UNREC

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

1218

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and inaccordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.
IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Tax Parcel ID Number 00-00-00-01438-002 **Permit Number** _____

1. Description of property: (legal description of the property and street address or 911 address)
798 SW Utah St. Fort White, FL. 32038

Inst:200812013301 Date:7/16/2008 Time:11:00 AM
DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1154 P:1808

2. General description of improvement: Single Family Home

3. Owner Name & Address BRJ Ventures, LLC 830 North Krome Ave Homestead FL 33030
Interest in Property _____

4. Name & Address of Fee Simple Owner (If other than owner): _____

5. Contractor Name Catalina Caststone Creations Inc. **Phone Number** 305-542-9193
Address 9801 SW 121 St. Miami, FL 33176

6. Surety Holders Name _____ **Phone Number** _____
Address _____
Amount of Bond _____

7. Lender Name _____ **Phone Number** _____
Address _____

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:
Name _____ **Phone Number** _____
Address _____

9. In addition to himself/herself the owner designates John or Pam Smith **of** 377 SW Mauldin Ave Lake City 32024 **to receive a copy of the Lien Notice as provided in Section 713.13 (1) –**
(a) 7. Phone Number of the designee 786-295-9296

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified) _____

THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN IN HIS/HER STEAD.

B.R.J Ventures L.L.C
Jud J Smith managing member.
Signature of Owner

Sworn to (or affirmed) and subscribed before day of July 16, **20** 08.

_____ **NOTARY STAMP/SEAL**
Signature of Notary

Notary Public State of Florida
Judi M Lowrey
My Commission DD477506
Expires 10/02/2009

AC# 2602644

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

SEQ# L060601008

DATE	BATCH NUMBER	LICENSE NBR
06/01/2006	058088240	CBC1253816

The BUILDING CONTRACTOR
Named below IS CERTIFIED
Under the provisions of Chapter 489 FS.
Expiration date: AUG 31, 2008

SCHWARTZ, LOUIS R
CATALINA CASTSTONE CREATIONS INC
9801 SW 121 STREET
MIAMI FL 33176

JEB BUSH
GOVERNOR

SIMONE MARSTILLER
SECRETARY

DISPLAY AS REQUIRED BY LAW

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION

CONSTRUCTION INDUSTRY LICENSING BOARD
1940 NORTH MONROE STREET
TALLAHASSEE FL 32399-0783

(850) 487-1395

CATALINA CASTSTONE CREATIONS INC
9801 SW 121 STREET
MIAMI FL 33176

STATE OF FLORIDA AC# 3230661
DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION

QB45382 05/23/07 068190750

QUALIFIED BUSINESS ORGANIZATION
CATALINA CASTSTONE CREATIONS INC

(NOT A LICENSE TO PERFORM WORK. ALLOWS COMPANY TO DO BUSINESS IF IT HAS A LICENSED QUALIFIER.)

IS QUALIFIED under the provisions of Ch. 489 FS.
Expiration date: AUG 31, 2009 L07052300635

DETACH HERE

3230661

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD SEQ# L07052300635

DATE	BATCH NUMBER	LICENSE NBR
5/23/2007	068190750	QB45382

The BUSINESS ORGANIZATION
Named below IS QUALIFIED
Under the provisions of Chapter 489 FS.
Expiration date: AUG 31, 2009
(THIS IS NOT A LICENSE TO PERFORM WORK. THIS ALLOWS COMPANY TO DO BUSINESS ONLY IF IT HAS A QUALIFIER.)

CATALINA CASTSTONE CREATIONS INC
9801 SW 121 STREET
MIAMI FL 33176

CHARLIE CRIST
GOVERNOR

DISPLAY AS REQUIRED BY LAW

HOLLY BENSON
SECRETARY

2007-08

COLUMBIA COUNTY BUSINESS TAX RECEIPT
RONNIE BRANNON, TAX COLLECTOR
RECEIPT EXPIRES 09/30/2008

RECEIPT NUMBER:
7001785

MACHINES ROOMS SEATS EMPLOYEES

BUSINESS TYPE: 000102

BUILDING CONTRACTOR

RONNIE BRANNON TAX C
2606624.0001 of 0001
DATE 08/10/2007
Oper LYC
Till 026
Paid 18.00

CATALINA CASTSTONE CREATIONS
LOUIS R SCHWARTZ
9801 SW 121 STREET
MIAMI, FL 33176-0000

SUPPLEMENTAL
X RENEWAL 18.00
NEW RECEIPT
TRANSFER

PENALTY 0.00
TOTAL 18.00

LOCATION ADDRESS: 377 SW MAULDIN AVE
LAKE CITY FL 32024-0000

X L. Ronald Schwartz, Pres

SIGN AND RETURN WITH PAYMENT

I SWEAR THAT THIS APPLICATION FOR RECEIPT IS MADE FOR THE BUSINESS OR PROFESSION INDICATED HEREON AND IS TRUE AND CORRECT.

THE APPLICATION MUST COMPLY WITH STATE AND LOCAL ORDINANCE INCLUDING ZONING

0000001800 0000001800 00000000000003359 1001 7

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (386) 752-1854
FAX (386) 755-7022
904 NW MAIN BLVD.
LAKE CITY, FLORIDA 32055

February 19, 2008

Notice To All Contractors:

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results. All wells will have a pump & tank combination that will be sufficient enough for each situation.

If you have any questions please feel free to call our office.

Thank You ,

Donald D. Hall

COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ———110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☐	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. *The following information must be shown as per section 1603.1.4 FBC* a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, *ASCE 7* and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
☑	☐	**Elevations including:** a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation

d) Location, size and height above roof of chimneys.
e) Location and size of skylights
f) Building height
e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned.
b) Shear walls identified.
c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
d) Show safety glazing of glass, where required by code.
e) Identify egress windows in bedrooms, and size.
f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel.

Roof System:

a) Truss package including:
   1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
   2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:
   1. Rafter size, species and spacing
   2. Attachment to wall and uplift
   3. Ridge beam sized and valley framing and support details
   4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
   1. All materials making up wall
   2. Block size and mortar type with size and spacing of reinforcement
   3. Lintel, tie-beam sizes and reinforcement
   4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
   5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
   6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
   7. Fire resistant construction (if required)
   8. Fireproofing requirements
   9. Shoe type of termite treatment (termiticide or alternative method)
   10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
   11. Indicate where pressure treated wood will be placed
   12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment
g) Arc Fault Circuits (AFCI) in bedrooms
h) Exhaust fans in bathroom

HVAC information

a) Energy Calculations (dimensions shall match plans)
b) Manual J sizing equipment or equivalent computation
c) Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

FORM 600A-2004 EnergyGauge® 4.21

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**219 Melanie way**	Builder:	**Glenn I. Jones Inc.**
Address:	**219 NW Melanie Way**	Permitting Office:	22/004 Columbia
City, State:	**Lake City, fl 32055**	Permit Number:	27207
Owner:	**219 NW Melanie**	Jurisdiction Number:	22/000
Climate Zone:	**North**		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	No	
6.	Conditioned floor area (ft²)	1326 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. (Dble, U=0.7) 45.0 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 153.0 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 146.3(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=14.8, 1289.3 ft²	
b.	Frame, Wood, Adjacent	R=11.0, 324.7 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 1325.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH(Sealed):Garage	Sup. R=6.0, 142.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 41.5 kBtu/hr SEER: 14.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 42.0 kBtu/hr HSPF: 8.40	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons EF: 0.92	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.12 Total as-built points: 19165 Total base points: 23340 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 1-14-08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Project Summary
Entire House
Glenn I. Jones Inc.

Job: 219 NW Melanie
Date:
By:

Project Information

For: 219 NW Melanie
219 NW Melanie Way, Lake City, fl 32055

Notes:

Design Information

Weather: Jacksonville Cecil Field NAS, FL, US

Winter Design Conditions

Outside db	34 °F
Inside db	68 °F
Design TD	35 °F

Summer Design Conditions

Outside db	94 °F
Inside db	75 °F
Design TD	19 °F
Daily range	M
Relative humidity	50 %
Moisture difference	41 gr/lb

Heating Summary

Structure	23498 Btuh
Ducts	1175 Btuh
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	24673 Btuh

Sensible Cooling Equipment Load Sizing

Structure	24778 Btuh
Ducts	2478 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.99
Equipment sensible load	27092 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	***1326***	***1326***
Volume (ft³)	13253	13253
Air changes/hour	1.00	0.50
Equiv. AVF (cfm)	221	110

Latent Cooling Equipment Load Sizing

Structure	3085 Btuh
Ducts	0 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	3085 Btuh
Equipment total load	30178 Btuh
Req. total capacity at 0.70 SHR	3.2 ton

Heating Equipment Summary

Make	Carrier
Trade	Base 13 Puron HP
Model	25HBA342A30
ARI ref no.	0
Efficiency	8.4 HSPF
Heating input	
Heating output	42000 Btuh @ 47°F
Temperature rise	28 °F
Actual air flow	1383 cfm
Air flow factor	0.056 cfm/Btuh
Static pressure	0.50 in H2O
Space thermostat	

Cooling Equipment Summary

Make	Carrier
Trade	Base 13 Puron HP
Cond	25HBA342A30
Coil	FX4CNF042
ARI ref no.	0
Efficiency	14 SEER
Sensible cooling	29050 Btuh
Latent cooling	12450 Btuh
Total cooling	41500 Btuh
Actual air flow	1383 cfm
Air flow factor	0.051 cfm/Btuh
Static pressure	0.50 in H2O
Load sensible heat ratio	0.90

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.4

The higher the score, the more efficient the home.

219 NW Melanie, 219 NW Melanie Way, Lake City, fl, 32055

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	No	
6.	Conditioned floor area (ft²)	1326 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
	a. U-factor:	Description Area	
	(or Single or Double DEFAULT)	7a. (Dble, U=0.7) 45.0 ft²	
	b. SHGC:		
	(or Clear or Tint DEFAULT)	7b. (Clear) 153.0 ft²	
8.	Floor types		
	a. Slab-On-Grade Edge Insulation	R=0.0, 146.3(p) ft	
	b. N/A		
	c. N/A		
9.	Wall types		
	a. Frame, Wood, Exterior	R=14.8, 1289.3 ft²	
	b. Frame, Wood, Adjacent	R=11.0, 324.7 ft²	
	c. N/A		
	d. N/A		
	e. N/A		
10.	Ceiling types		
	a. Under Attic	R=30.0, 1325.0 ft²	
	b. N/A		
	c. N/A		
11.	Ducts		
	a. Sup: Unc. Ret: Unc. AH(Sealed):Garage	Sup. R=6.0, 142.0 ft	
	b. N/A		

12.	Cooling systems		
	a. Central Unit	Cap: 41.5 kBtu/hr	
		SEER: 14.00	
	b. N/A		
	c. N/A		
13.	Heating systems		
	a. Electric Heat Pump	Cap: 42.0 kBtu/hr	
		HSPF: 8.40	
	b. N/A		
	c. N/A		
14.	Hot water systems		
	a. Electric Resistance	Cap: 40.0 gallons	
		EF: 0.92	
	b. N/A		
	c. Conservation credits		
	(HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits		
	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

FORM 600A-2004 EnergyGauge® 4.21

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **219 NW Melanie Way, Lake City, fl, 32055** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor, around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2004 EnergyGauge® 4.21

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **219 NW Melanie Way, Lake City, fl, 32055** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2635.00	7905.0	40.0	0.92	3	1.00	2635.00	1.00	7905.0	
				As-Built Total:						**7905.0**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
7566	**7869**	**7905**	**23340**	**5040**	**6220**	**7905**	**19165**

PASS

FORM 600A-2004 EnergyGauge® 4.21

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **219 NW Melanie Way, Lake City, fl, 32055** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1326.0	12.74	3040.8	Double,U=0.73,Clear	N	1.5	6.5	15.0	20.70	1.00	311.1	
				Double,U=0.73,Clear	E	1.5	6.5	15.0	14.99	1.03	231.7	
				Single,U=1.21,Clear	E	1.6	8.6	42.0	24.80	1.02	1062.7	
				Double,U=0.73,Clear	S	1.5	4.5	6.0	9.51	1.25	71.5	
				Double,U=0.73,Clear	S	1.5	6.5	45.0	9.51	1.09	467.9	
				Double,U=0.73,Clear	W	1.5	6.5	30.0	16.87	1.02	515.8	
				As-Built Total:				**153.0**			**2660.8**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM	=	Points	
Exterior	1289.3	3.70	4770.4	Frame, Wood, Exterior			14.8	1289.3	3.04		3919.5	
Adjacent	324.7	3.60	1168.9	Frame, Wood, Adjacent			11.0	324.7	3.60		1168.9	
Base Total:	**1614.0**		**6939.3**	**As-Built Total:**				**1614.0**			**5088.4**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM	=	Points	
Exterior	21.0	8.40	176.4	Exterior Wood				21.0	12.30		258.3	
Adjacent	18.7	8.00	149.6	Adjacent Wood				18.7	11.50		215.1	
Base Total:	**39.7**		**326.0**	**As-Built Total:**				**39.7**			**473.4**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1325.0	2.05	2716.3	Under Attic			30.0	1325.0	2.05 X	1.00	2716.3	
Base Total:	**1325.0**		**2716.3**	**As-Built Total:**				**1325.0**			**2716.3**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM	=	Points	
Slab	146.3(p)	8.9	1302.1	Slab-On-Grade Edge Insulation			0.0	146.3(p	18.80		2750.4	
Raised	0.0	0.00	0.0									
Base Total:			**1302.1**	**As-Built Total:**				**146.3**			**2750.4**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM	=	Points	
	1326.0	-0.59	-782.3					1326.0	-0.59		-782.3	

FORM 600A-2004

EnergyGauge® 4.21

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **219 NW Melanie Way, Lake City, fl, 32055** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**12542.1**	**Winter As-Built Points:**					**12906.9**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 42000 btuh ,EFF(8.4) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
12542.1	**0.6274**	**7868.9**	12906.9 **12906.9**	1.000 **1.00**	(1.069 x 1.169 x 0.95) **1.187**	0.406 **0.406**	1.000 **1.000**	6220.3 **6220.3**

FORM 600A-2004

EnergyGauge® 4.21

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **219 NW Melanie Way, Lake City, fl, 32055** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1326.0	20.04	4783.1	Double,U=0.73,Clear	N	1.5	6.5	15.0	19.85	0.95	282.2	
				Double,U=0.73,Clear	E	1.5	6.5	15.0	42.64	0.93	593.0	
				Single,U=1.21,Clear	E	1.6	8.6	42.0	48.00	0.96	1926.7	
				Double,U=0.73,Clear	S	1.5	4.5	6.0	36.45	0.78	170.4	
				Double,U=0.73,Clear	S	1.5	6.5	45.0	36.45	0.88	1439.7	
				Double,U=0.73,Clear	W	1.5	6.5	30.0	39.12	0.93	1088.7	
				As-Built Total:				**153.0**			**5600.7**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Exterior	1289.3	1.70	2191.8	Frame, Wood, Exterior			14.8	1289.3	1.32		1701.9	
Adjacent	324.7	0.70	227.3	Frame, Wood, Adjacent			11.0	324.7	0.70		227.3	
Base Total:	**1614.0**		**2419.1**	**As-Built Total:**				**1614.0**			**1929.2**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Exterior	21.0	4.10	86.1	Exterior Wood				21.0	6.10		128.1	
Adjacent	18.7	1.60	29.9	Adjacent Wood				18.7	2.40		44.9	
Base Total:	**39.7**		**116.0**	**As-Built Total:**				**39.7**			**173.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1325.0	1.73	2292.3	Under Attic			30.0	1325.0	1.73 X 1.00		2292.3	
Base Total:	**1325.0**		**2292.3**	**As-Built Total:**				**1325.0**			**2292.3**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	146.3(p)	-37.0	-5413.1	Slab-On-Grade Edge Insulation			0.0	146.3(p	-41.20		-6027.6	
Raised	0.0	0.00	0.0									
Base Total:			**-5413.1**	**As-Built Total:**				**146.3**			**-6027.6**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1326.0	10.21	13538.5					1326.0	10.21		13538.5	

FORM 600A-2004 EnergyGauge® 4.21

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **219 NW Melanie Way, Lake City, fl, 32055** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 17735.9			**Summer As-Built Points:**					**17406.0**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 41500 btuh ,SEER/EFF(14.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
			17406	1.00	(1.09 x 1.147 x 0.95)	0.244	1.000	5039.9
17735.9	**0.4266**	**7566.1**	**17406.0**	**1.00**	**1.188**	**0.244**	**1.000**	**5039.9**

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC

SERIES/MODEL: 420/430/440
PRODUCT TYPE: Aluminum Sliding Glass Door

Title	Summary of Results		
	Test Specimen #1	Test Specimen #2	Test Specimen #3
Rating	SGD-R25 182 x 96	SGD-R35 182 x 80	SGD-R40 144 x 96
Operating Force	17 lbf max.	17 lbf max.	N/A
Air Infiltration	0.23 cfm/ft^2	0.27 cfm/ft^2	N/A
Water Resistance Test Pressure	3.75/6.0/9.0 psf	6.0 psf	N/A
Uniform Load Deflection Test Pressure	±35.0 psf	±35.0 psf	+40.0 psf/-40.1 psf
Uniform Load Structural Test Pressure	±37.5 psf	±52.5 psf	+60.0 psf/-60.2 psf
Forced Entry Resistance	Grade 10	Grade 10	N/A

Reference should be made to ATI Report No. 52112.01-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	52112.01-122-47
Revision 2:	09/14/05
Test Dates:	06/30/04
Through:	08/12/04
Report Date:	08/30/04
Expiration Date:	07/02/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 420/430/440, aluminum sliding glass doors at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: SGD-R25 182 x 96; Test Specimen #2: SGD-R35 182 x 80; Test Specimen #3: SGD-R40 144 x 96. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 420/430/440

Product Type: Aluminum Sliding Glass Door

Test Specimen #1: SGD-R25 182 x 96 (XXO)

Overall Size: 15' 1-3/4" wide by 8' 0" high

Active Door Panel Size (2): 5' 0-1/2" wide by 7' 11" high

Fixed Door Panel Size: 5' 1" wide by 7' 11" high

Screen Size: 5' 0-3/8" wide by 7' 11" high

Overall Area: 121.2 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520).

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Test Specimen #2: SGD-R35 182 x 80 (OXX)

Overall Size: 15' 1-3/4" wide by 6' 8" high

Active Door Panel Size (2): 5' 0-1/2" wide by 6' 7" high

Fixed Door Panel Size: 4' 8-7/8" wide by 6' 2-5/8" high

Screen Size: 5' 0-3/8" wide by 6' 7" high

Overall Area: 101 ft^2

Reinforcement: No reinforcement was utilized.

Test Specimen #3: SGD-R40 144 x 96 (OXO)

Overall Size: 12' 0" wide by 8' 0" high

Active Door Panel Size: 3' 8-1/4" wide by 7' 10-1/2" high

Fixed Door Panel Size (2): 3' 8-3/4" wide by 7' 6-1/2" high

Screen Size: 3' 11-1/2" wide by 7' 11-3/8" high

Overall Area: 96 ft^2

Reinforcement: The active and fixed interlocking stile utilized a steel U-shaped reinforcement (Drawing #9917525). The fixed intermediate jamb utilized a steel reinforcement (Drawing #9917520). The interlock utilized an aluminum reinforcement (Drawing #SECT4237).

The following descriptions apply to all specimens.

Finish: All aluminum was painted.

Glazing Details: All glazing consisted of a single sheet of 3/16" thick clear tempered glass that was channel glazed with a wrap around rubber gasket.

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.187" backed by 0.270" high polypile with center fin	2 Rows	Stiles
1/2" wide by 1" long polypile dust plug	2 Pieces	Corner of head, jamb, and top and bottom of panel retainer
0.187" backed by 0.250" high polypile with center fin	2 Rows	Top rail
0.187" backed by 0.350" high polypile with center fin	2 Rows	Bottom rail
0.187" backed by 0.230" high polypile with center fin	1 Row	Panel interlock, screen stiles

Frame Construction: The frame was constructed of extruded aluminum. Corners were coped, butted, sealed, and fastened with two #8 x 5/8" screws. An aluminum panel adaptor was added to the screen adaptor and secured with #6 x 3/8" pan head screws located 3-1/2" from the ends and 14" on center through the screen adaptor into the panel adaptor. The jambs utilized a panel jamb retainer on the fixed panels secured to the jambs with two #6 x 1/2" screws through the retainer into the jambs. The panels were placed in the retainer and secured to the frame with two #8 x 1/2" screws located through the retainers into the panels. Three panel jamb retainers were utilized to secure the fixed panels, located at panel top and bottom and one midspan. The fixed panels also utilized an aluminum sill retainer clip located at the sill. The sill utilized an optional aluminum sill extender.

Door Panel Construction: The door panels were constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" x 3/4" screw at the bottom and two #8 x 3/4" screws at the top.

Screen Construction: The screen was constructed of extruded aluminum members. Corners were coped, butted, and fastened with one 1/4" x 3/4" screw and one #8 x 1" screw at the bottom and one #8 x 1" screw at the top.

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Locking handle	1	44" from active panel bottom
Roller assembly	2	3" from bottom rail ends
Screen locking handle	1	46" from screen bottom rail
Screen rollers	2	Corners of bottom rail

Drainage:

Description	Quantity	Location
Sloped sill	1	Sill
1/2" long drain off notches	6	Ends of vertical sill legs

Installation: The units were installed into a #2 Spruce-Pine-Fir wood test buck. The units were fastened to the test buck with two rows of #8 x 1-1/4" screws, 8" from each end and 23" on center. The exterior perimeter was sealed with silicone.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	24 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.23 cfm/ft^2	0.3 cfm/ft^2 max.
2.1.3	Water Resistance per ASTM E 547 (with and without screen) 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	15.0 psf (positive)	0.56"	See Note #2
	15.0 psf (negative)	0.57"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	22.5 psf (positive)	0.02"	0.30" max.
	22.5 psf (negative)	0.03"	0.30" max.
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%

Note #1: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.

Note #2: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 3.75 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.3	Water Resistance per ASTM E 547 (with and without screen) (with 2-5/8" Dade County sill extension) 9.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	35.0 psf (positive)	2.98"	See Note #2
	35.0 psf (negative)	2.52"	See Note #2

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: SGD-R25 182 x 96 (XXO) (Continued)			
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	37.5 psf (positive)	0.20"	0.36" max.
	37.5 psf (negative)	0.19"	0.36" max.
Test Specimen #2: SGD-R35 182 x 80 (OXX)			
2.2.1.6.1	Operating Force	17 lbf	20 lbf max.
	Breakaway force	21 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.27 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets (or exceed) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547 (with and without screen) 2.86 psf	No leakage	No leakage
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Locking stile	0.12"/24%	0.50"/100%
	Interlock stile	0.12"/24%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 842		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A6	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: SGD-R35 182 x 80 (OXX) (Continued)			
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) (with sill riser) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	35.0 psf (positive)	1.28"	See Note #2
	35.0 psf (negative)	1.33"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	52.5 psf (positive)	0.13"	0.30" max.
	52.5 psf (negative)	0.15"	0.30" max.
Test Specimen #3: SGD-R40 144 x 96 (OXO)			
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	40.0 psf (positive)	1.42"	See Note #2
	40.1 psf (negative)	1.28"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	60.0 psf (positive)	0.27"	0.37" max.
	60.2 psf (negative)	0.30"	0.37" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Digitally Signed by: Mark A. Hess

Mark A. Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MH:vlm

Revision Log

Rev. #	Date	Page(s)	Revision(s)
0	08/30/04	N/A	Original report issue
1	09/13/04	Cover page	Switch Specimens 1 and 2 / Added 430/440 to Series/Model
1	09/13/04	Page 1 and 2	Switch Specimen 1 and 2 sizes Added 430/440 to Series/Model on Page 1
1	09/13/04	Pages 4 through 7	Switch Specimen 1 and 2 test results / Specimen 2 optional performance water resistance from 3.75 psf to 6.00 psf with sill riser.
2	09/14/05	Page 2	Corrected configuration of Test Specimen #3
2	09/14/05	Page 3	Added additional Weatherstripping

Community Affairs

Product Approval

USER: Public User

Search Criteria

Code Version	2004	FL#	ALL
Application Type	ALL	Product Manufacturer	MI Windo
Category	ALL	Subcategory	ALL
Application Status	ALL	Compliance Method	ALL

Search Results - Applications

Go to Page [ ] GO

FL#	Type	Manufacturer	Validat
FL5100	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	
FL5104	New	MI Windows and Doors **Category:** Windows **Subcategory:** Double Hung	
FL5108	New	MI Windows and Doors **Category:** Windows **Subcategory:** Single Hung	
FL5418	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	
FL5438	New	MI Windows and Doors **Category:** Windows **Subcategory:** Single Hung	
FL5447	New	MI Windows and Doors **Category:** Windows **Subcategory:** Double Hung	
FL5451	New	MI Windows and Doors **Category:** Windows **Subcategory:** Horizontal Slider	
FL5483-R1 History	Revision	MI Windows and Doors **Category:** Exterior Doors **Subcategory:** Sliding Exterior Door Assemblies	
FL5513	New	MI Windows and Doors **Category:** Windows	Steven

		Subcategory: Mullions	(717) 7
FL6023	New	MI Windows and Doors **Category:** Windows **Subcategory:** Casement	
FL6024	New	MI Windows and Doors **Category:** Windows **Subcategory:** Horizontal Slider	
FL6028	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	
FL6029	New	MI Windows and Doors **Category:** Windows **Subcategory:** Single Hung	
FL6489	New	MI Windows and Doors **Category:** Windows **Subcategory:** Mullions	Steven (717) 7
FL6499	New	MI Windows and Doors **Category:** Windows **Subcategory:** Single Hung	
FL6501	New	MI Windows and Doors **Category:** Windows **Subcategory:** Double Hung	
FL6502	New	MI Windows and Doors **Category:** Windows **Subcategory:** Horizontal Slider	
FL6503	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	
FL6679	New	MI Windows and Doors **Category:** Windows **Subcategory:** Fixed	

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Codes and Standards
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Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

Product Approval

USER: Public User

Search Criteria

Code Version	2004	FL#	ALL
Application Type	ALL	Product Manufacturer	JORDAN WIND
Category	ALL	Subcategory	ALL
Application Status	ALL	Compliance Method	ALL

Search Results - Applications

FL#	Type	Manufacturer	Validat
FL1378-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Single Hung	
FL1384-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Horizontal Slider	
FL1385-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Fixed	
FL1386-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Exterior Doors **Subcategory:** Sliding Exterior Door Assemblies	
FL2685-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Mullions	Steven (717) 7
FL2946-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Awning	
FL2949-R1 History	Revision	JORDAN WINDOWS and DOORS **Category:** Windows **Subcategory:** Casement	

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard

Product Approval

USER: Public User

Search Criteria			
Code Version	2004	FL#	ALL
Application Type	ALL	Product Manufacturer	Masonit
Category	ALL	Subcategory	ALL
Application Status	ALL	Compliance Method	ALL

Search Results - Applications

FL#	Type	Manufacturer	Validated By
FL4242-R1 History	Revision	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL4334-R1 History	Revision	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL4668-R1 History	Revision	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL4904	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL4940	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL5114	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies	
FL5465	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door	

		Assemblies
FL5507	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL5508	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL6015	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL6506-R1 History	Revision	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL6509	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL7050	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies
FL7091	New	Masonite International **Category:** Exterior Doors **Subcategory:** Swinging Exterior Door Assemblies

DCA Administration

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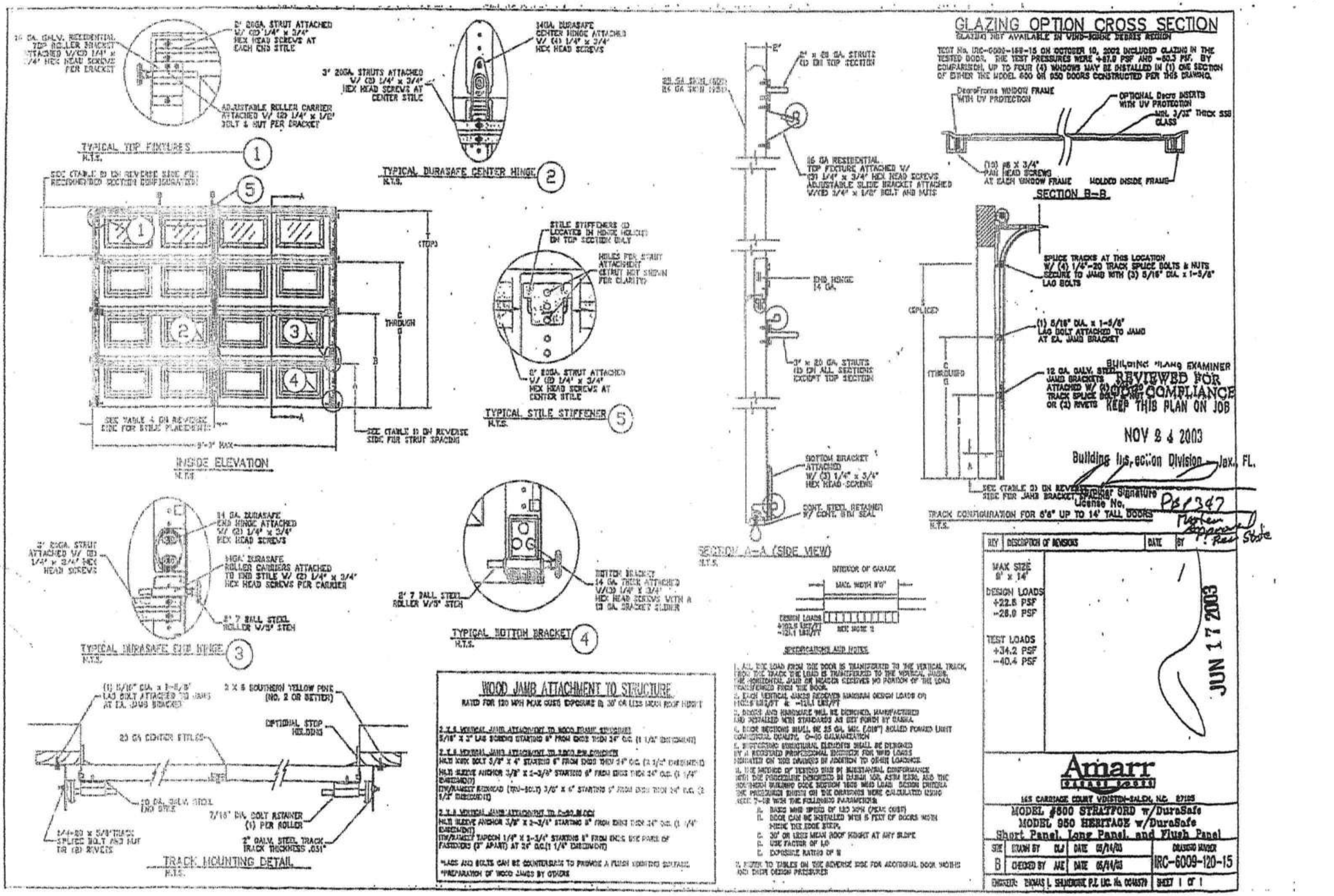
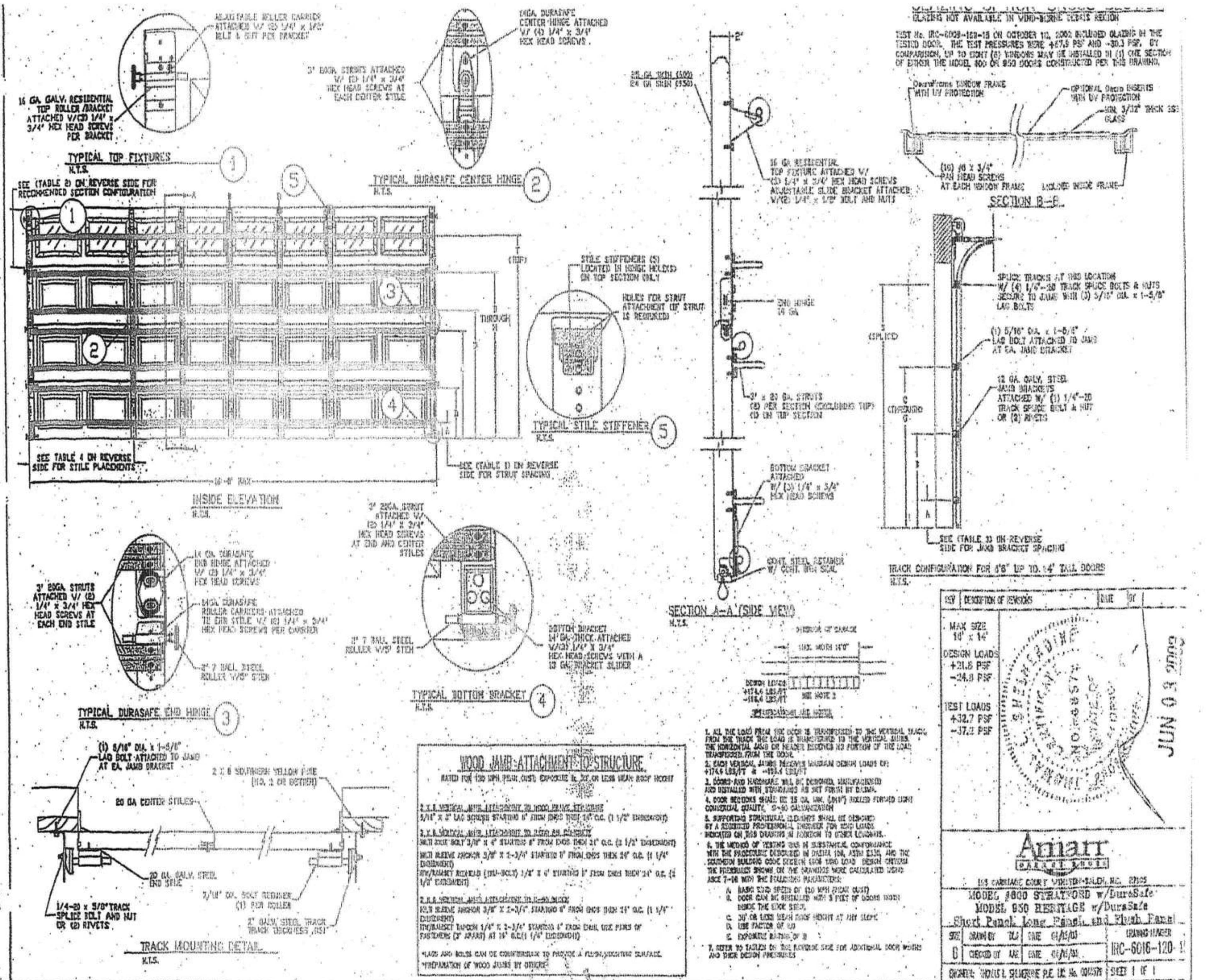
9x7


GLAZING OPTION CROSS SECTION
SECTION B-B
TYPICAL TOP FIXTURES
N.T.S.
TYPICAL DURASAFE CENTER HINGE
N.T.S.
INSIDE ELEVATION
N.T.S.
TYPICAL STILE STIFFENER
N.T.S.
TYPICAL DURASAFE END HINGE
N.T.S.
TYPICAL BOTTOM BRACKET
N.T.S.
TRACK MOUNTING DETAIL
N.T.S.
WOOD JAMB ATTACHMENT TO STRUCTURE
SECTION A-A (SIDE VIEW)
N.T.S.
SPECIFICATIONS AND NOTES
TRACK CONFIGURATION FOR 6'6" UP TO 14' TALL DOORS
N.T.S.
Building Plans Examiner
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
NOV 2 4 2003
Building Inspection Division Jax. FL.
JUN 17 2003
MAX SIZE
8' x 14'
DESIGN LOADS
+22.5 PSF
-26.0 PSF
TEST LOADS
+34.2 PSF
-40.4 PSF
Amarr
165 CARRIAGE COURT WINSTON-SALEM, N.C. 27105
MODEL #800 STRATFORD w/DuraSafe
MODEL 950 HERITAGE w/DuraSafe
Short Panel, Long Panel, and Flush Panel
IRC-6009-120-15
SHEET 1 OF 1

17

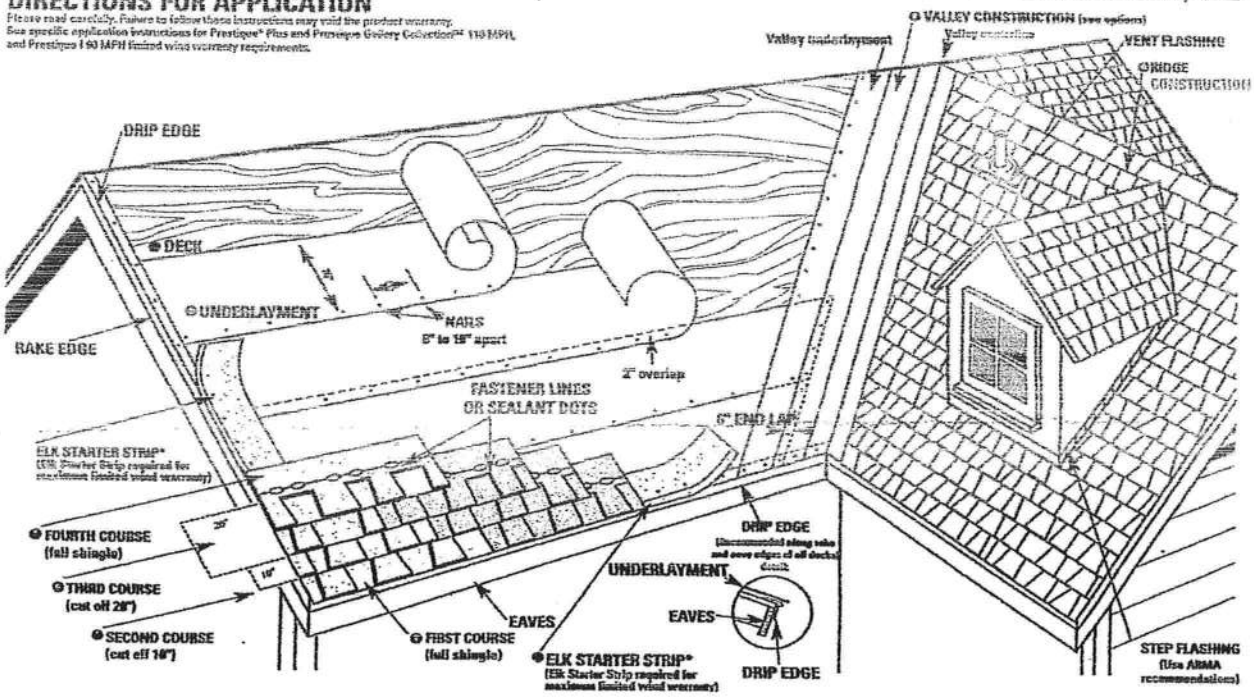

GLAZING NOT AVAILABLE IN WIND-BORNE DEBRIS REGION
BY COMPARISON, UP TO EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF EITHER THE MODEL 800 OR 950 DOORS CONSTRUCTED PER THIS DRAWING.
WITH UV PROTECTION
OPTIONAL INSERTS WITH UV PROTECTION
SECTION B-B
ADJUSTABLE ROLLER CARRIER
16 GA. GALV. RESIDENTIAL TOP ROLLER /BRACKET ATTACHED W/(3) 1/4" x 3/4" HEX HEAD SCREWS PER BRACKET
TYPICAL TOP FIXTURES
N.T.S.
SEE (TABLE 2) ON REVERSE SIDE FOR RECOMMENDED SECTION CONFIGURATION
TYPICAL DURASAFE CENTER HINGE
N.T.S.
HEX HEAD SCREWS AT EACH CENTER STILE
HEX HEAD SCREWS.
STILE STIFFENERS (5) LOCATED IN HINGE HOLE(S) ON TOP SECTION ONLY
HOLES FOR STRUT ATTACHMENT (IF STRUT IS REQUIRED)
TYPICAL STILE STIFFENER
N.T.S.
SEE TABLE 4 ON REVERSE SIDE FOR STILE PLACEMENTS
SIDE FOR STRUT SPACING
INSIDE ELEVATION
N.T.S.
16 GA. RESIDENTIAL TOP FIXTURE ATTACHED W/
END HINGE 14 GA.
3" x 20 GA. STRUTS (8) PER SECTION (EXCLUDING TOP) (1) ON TOP SECTION
BOTTOM BRACKET ATTACHED W/ (3) 1/4" x 3/4" HEX HEAD SCREWS
CONT. STEEL RETAINER W/ CONT. SEAL
SECTION A-A (SIDE VIEW)
N.T.S.
SPLICE TRACKS AT THIS LOCATION
LAG BOLTS
(SPLICED)
(1) 5/16" DIA. x 1-5/8" LAG BOLT ATTACHED TO JAMB AT EA. JAMB BRACKET
12 GA. GALV. STEEL JAMB BRACKETS
TRACK SPLICE BOLT & NUT OR (2) RIVETS
(THROUGH)
SEE (TABLE 3) ON REVERSE SIDE FOR JAMB BRACKET SPACING
TRACK CONFIGURATION FOR 6'8" UP TO 14' TALL DOORS
N.T.S.
3" 20GA. STRUTS ATTACHED W/ (2) 1/4" x 3/4" HEX HEAD SCREWS AT EACH END STILE
14 GA. DURASAFE END HINGE ATTACHED
14GA. DURASAFE ROLLER CARRIERS ATTACHED
2" 7 BALL STEEL ROLLER W/5" STEM
3" 20GA. STRUT ATTACHED W/ (2) 1/4" x 3/4" HEX HEAD SCREWS AT END AND CENTER STILES
BOTTOM BRACKET
TYPICAL BOTTOM BRACKET
N.T.S.
TYPICAL DURASAFE END HINGE
N.T.S.
(1) 5/16" DIA. x 1-5/8" LAG BOLT ATTACHED TO JAMB AT EA. JAMB BRACKET
20 GA. CENTER STILES
20 GA. GALV. STEEL END STILE
1/4-20 x 5/8" TRACK SPLICE BOLT AND NUT OR (2) RIVETS
(1) PER ROLLER
2" GALV. STEEL TRACK
TRACK MOUNTING DETAIL
N.T.S.
WOOD JAMB ATTACHMENT TO STRUCTURE
*PREPARATION OF WOOD JAMBS BY OTHERS
4. DOOR SECTIONS SHALL BE 25 GA. MIN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
E. EXPOSURE RATING OF B
7. REFER TO TABLES ON THE REVERSE SIDE FOR ADDITIONAL DOOR WIDTHS AND THEIR DESIGN PRESSURES
MAX SIZE 10' x 14'
DESIGN LOADS +21.8 PSF -24.8 PSF
TEST LOADS +32.7 PSF -37.2 PSF
JUN 0 9 2003
Amarr
MODEL 800 STRATFORD w/DuraSafe
MODEL 950 HERITAGE w/DuraSafe
Short Panel, Long Panel, and Flush Panel
SIZE
B
DRAWN BY
CHECKED BY
DATE
DRAWING NUMBER
IRC-6016-120-1
SHEET 1 OF 1

18

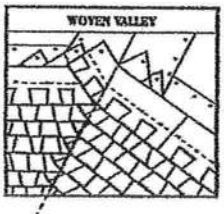
TUSCALOOSA, AL

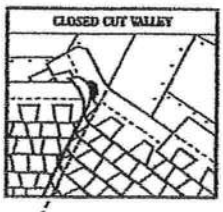
DIRECTIONS FOR APPLICATION

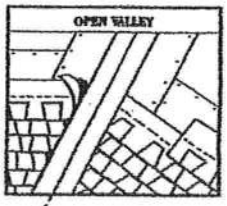
Please read carefully. Failure to follow these instructions may void the product warranty.
See specific application instructions for Prestique® Plus and Prestique Gallery Collection™ 110 MPH, and Prestique I 90 MPH limited wind warranty requirements.



VALLEY CONSTRUCTION OPTIONS (California Open and California Closed are also acceptable). NOTE: For complete ARMA valley installation details, see ARMA Residential Asphalt Roofing Manual.







DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In those cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

❶ DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" clapboard.

❷ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Field Service Department for application specifications over other decks and other slopes.

❸ STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR A STRIP SHINGLE INVERTED WITH THE HEADLAP APPLIED AT THE EAVE EDGE. With at least 4" trimmed from the end of the first shingle, start at the rake edge overhanging the eave 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

❹ FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

❺ SECOND COURSE

Start at the rake with the shingle having 10" trimmed off and continue across roof with full shingles.

❻ THIRD COURSE

Start at the rake with the shingle having 28" trimmed off and

❼ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof.

❽ VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying 18" metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

❾ RIDGE CONSTRUCTION

For ridge construction use Class "A" Seal-A-Ridge® with formula *FLX™* (See ridge package for installation instructions.)

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Always nail or staple through the fastener line or on products without fastener lines, nail or staple between and in line with sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for re-covers. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

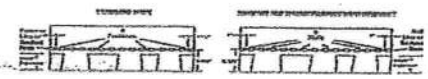
Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less.

MANSARD APPLICATIONS

Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

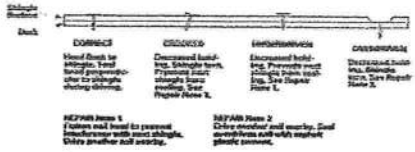
LIMITED WIND WARRANTY

- **For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.**
- **For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk**



HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

ELK

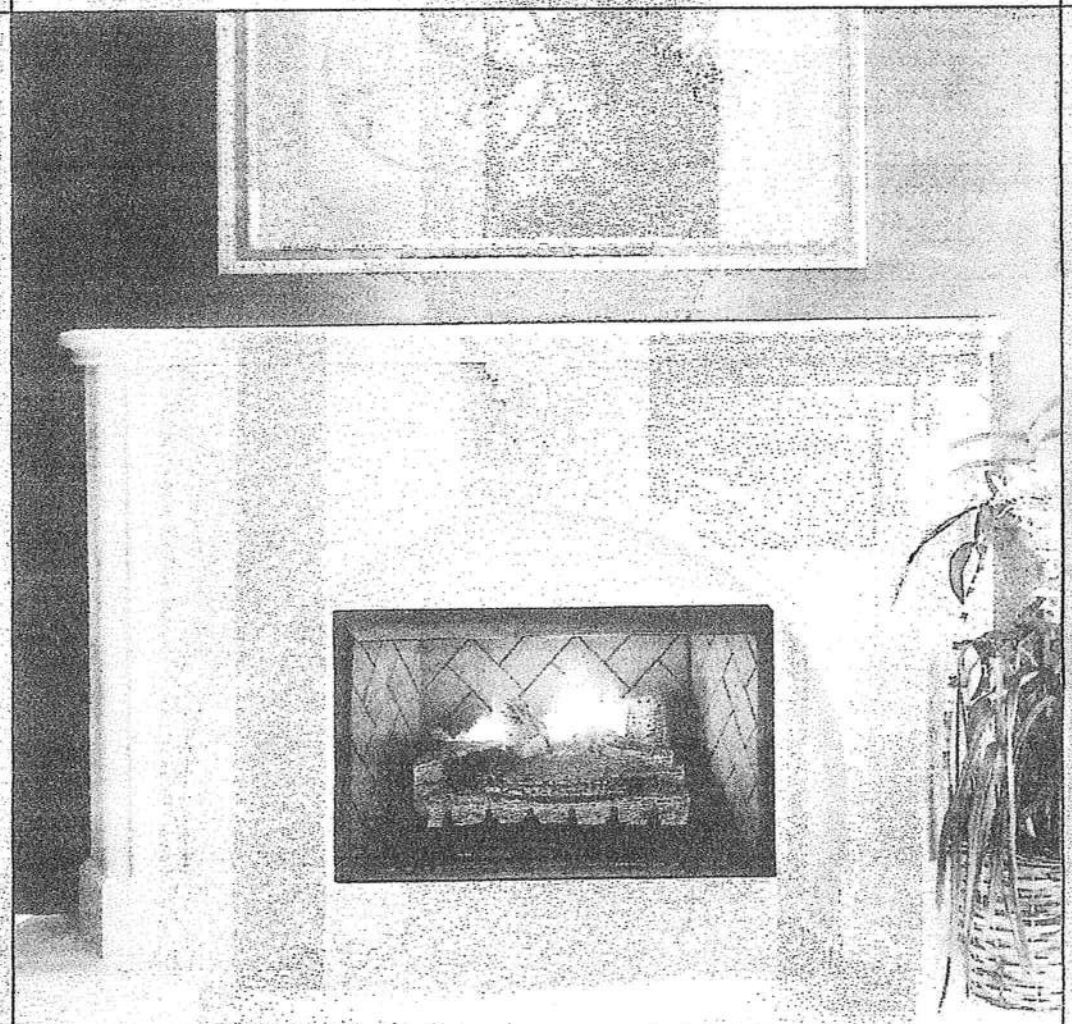
www.elkcorp.com

THE RENAISSANCE SERIES

Colonial

VENT-FREE GAS FIREPLACES

V32/36/42/50 Model Series



for builders

FIREPLACES FOR BUILDERS

Fmi

THE RENAISSANCE SERIES

Colonial

VENT-FREE GAS FIREPLACES

V32/36/42/50 Model Series

Warm Up To A High-Efficiency Colonial

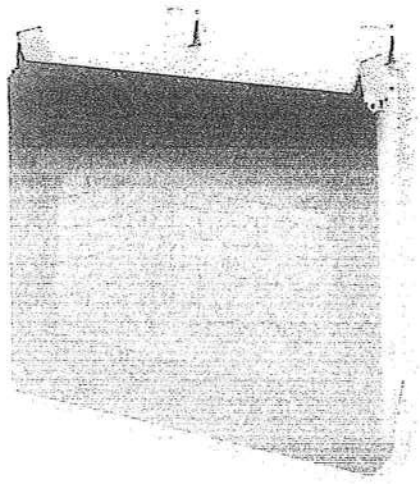
There's a growing demand for vent-free gas fireplaces because they're 99 percent energy-efficient and can be installed virtually anywhere. FMI's *Colonial* vent-free models deliver these benefits and more. They're part of our exciting new Renaissance Series, which offers a consistent look, sizing and construction across the entire line...plus beautiful new features homeowners will love!

Homeowner Highlights:

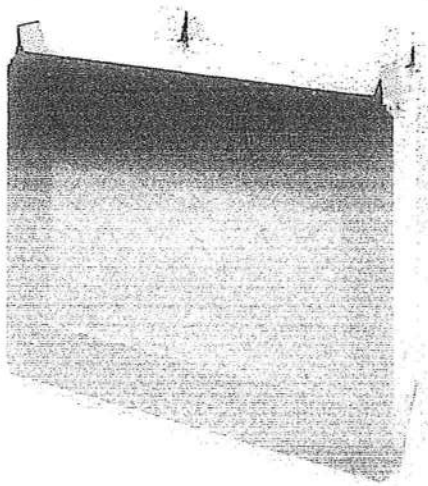
- Visual appeal—The industry's finest textured refractory brick liner (except 32") offers the attractive look of a true masonry fireplace.
- Many luxury features are standard—The Colonial comes standard with a heat deflection hood, hidden screen pockets (except 50"), stamped steel louvered panels, and other distinctive features.
- Dollar-saving efficiency—Paired with an Fmi vent free gas log heater, the systems 99% energy efficiency can provide dramatic energy savings.

Builder Benefits:

- Straight, secure installation—We've added full-length nailing flanges, and drywall stops.
- Flexibility in the field—You can quickly convert from louvered to clean face at any time (except 50").
- Economical and versatile—There's no chimney required. Can be installed virtually anywhere.



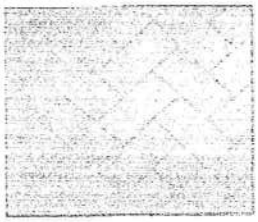
V36 is our louver-faced 36" fireplace with textured refractory brick-lined interior.



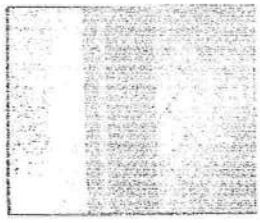
V42 is FMI's 42" louvered-face fireplace shown with optional herringbone textured refractory brick-lined interior.

32", 36", 42" & 50" Vent-Free Fireplace Models Available With The Following:

- Clean or Louver (Circulating) Faced Models Available (Clean Faced only on 50")
- Traditional Stacked and Herringbone Pattern Refractory Brick-Lined Interiors
- Solid wrap or Outside Air Ready Models

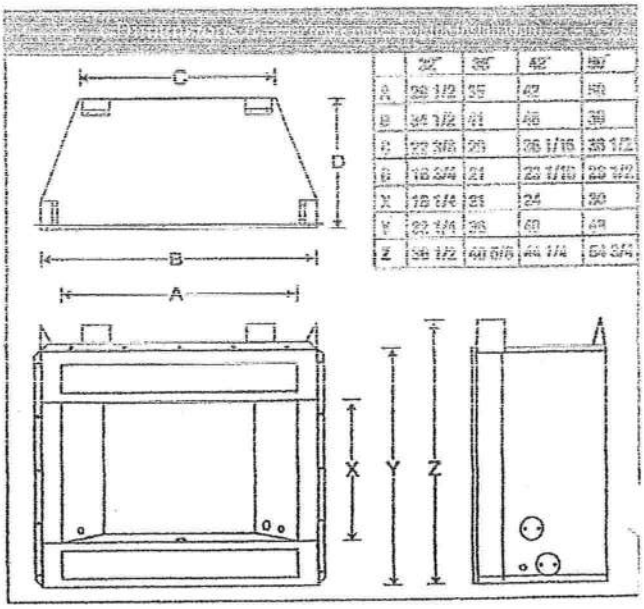


The Colonial features the industry's finest textured refractory brick lining.



You get straight, solid installation, thanks to our full-length nailing flanges and drywall stops.

- Rolled Black Louver Panels
- Louver Trim (Brushed Brass & Platinum)
- Decorative Filigree Panels (Black, Brushed Brass & Platinum)
- Perimeter Trim Kits (Black, Brushed Brass & Platinum)
- Heat Deflection Hoods (Brushed Brass & Platinum)
- Fan Kits
- Standard & Herringbone Refractory Brick Liners



	32"	36"	42"	50"
A	29 1/2	35	42	50
B	35 1/2	41	48	56
C	22 3/8	28	36 1/16	38 1/2
D	18 3/4	21	23 1/16	29 1/2
X	18 1/4	21	24	30
Y	32 1/4	36	60	62
Z	36 1/2	40 5/8	44 1/4	54 3/4

FIREPLACES FOR BUILDERS Fmi

Fmi Hearth Industries
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For more information, call (866) 328-4537

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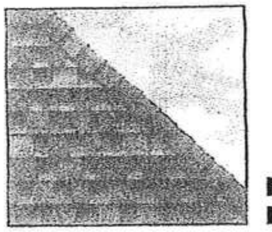
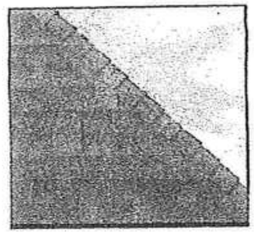


PRESTIQUE®
HIGH DEFINITION®



RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size .... 13¼" x 39⅜"
Exposure .... 5⅝"
Pieces/Bundle 16
Bundles/Square .... 4/98.5 sq.ft.
Squares/Pallet 11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size .... 13¼" x 38¾"
Exposure .... 5⅝"
Pieces/Bundle 22
Bundles/Square .... 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size .... 13¼" x 39⅜"
Exposure .... 5⅝"
Pieces/Bundle 16
Bundles/Square .... 4/98.5 sq.ft.
Squares/Pallet 14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size .... 13¼" x 38¾"
Exposure .... 5⅝"
Pieces/Bundle 22
Bundles/Square .... 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12" x 12"
Exposure: 6⅝"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood.
Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

*See actual limited warranty for conditions and limitations
**Check for product availability.

SPECIFICATIONS

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula *FLX*.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

Preparation of Roof Deck: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.52mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low slopes (4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)), use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula *FLX* with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. **These instructions are the minimum required to meet Elk application requirements.** In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

Southeast & Atlantic Office: 800.945.5551

Corporate Headquarters: 800.354.7732

Plant Location: 800.945.5545

ELK
www.elkcorp.com

DIRECTIONS FOR APPLICATION

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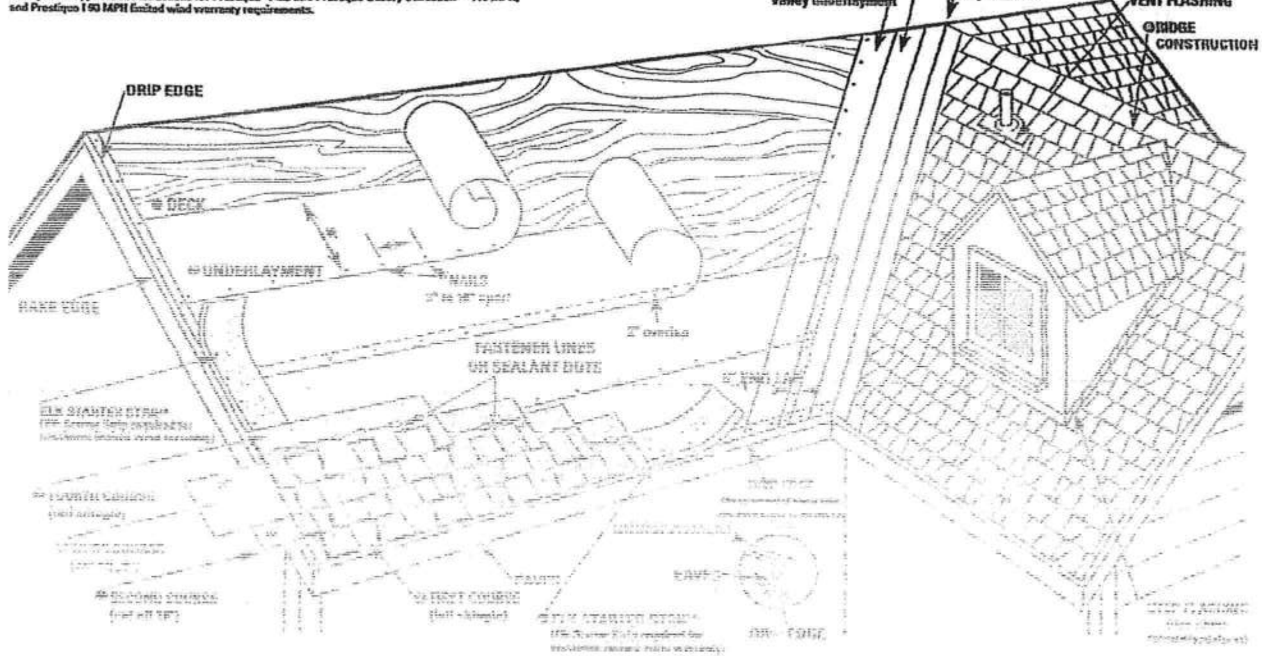
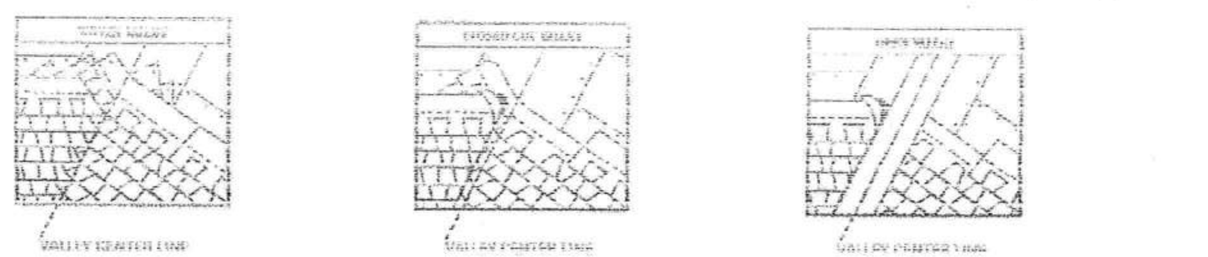
Tuscaloosa, AL





DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. *Note: It is not necessary to remove tape on back of shingle.*

DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL, OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Field Service Department for application specifications over other decks and other slopes.

STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR A STRIP SHINGLE INVERTED WITH THE HEADLAP APPLIED AT THE EAVE EDGE. With at least 4" trimmed from the end of the first shingle, start at the rake edge overhanging the eave 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

SECOND COURSE

Start at the rake with the shingle having 10" trimmed off and continue across roof with full shingles.

THIRD COURSE

Start at the rake with the shingle having 20" trimmed off and continue across roof with full shingles.

FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof.

VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying 16" metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

RIDGE CONSTRUCTION

For ridge construction use Class "A" Seal-A-Ridge® with formula FLX™ (See ridge package for installation instructions.)

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Always nail or staple through the fastener line or on products without fastener lines, nail or staple between and in line with sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12 gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

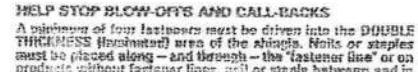
Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less.

MANSARD APPLICATIONS

Correct fastening is critical to the performance of the roof. For slopes exceeding 60" (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

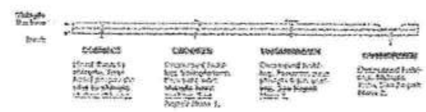
LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.



HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.

All Prestique and Raised Profile shingles have a UL® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

ELK

www.elkcorp.com

Builders FirstSource

Project Information for: L279639

Builder: Woodman Park Builders, Inc.
Address: 219 Northwest Melanie Way
... Lake City, FL 32055
County: Columbia
Truss Count: 26
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

Mark E. Haddox Florida Certified Residental Contractor License No. CRC1329442
Address: Woodman Park Builders, Inc. 4816 W U.S. Highway 90 Suite# 100 Lake City, Florida 32055

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

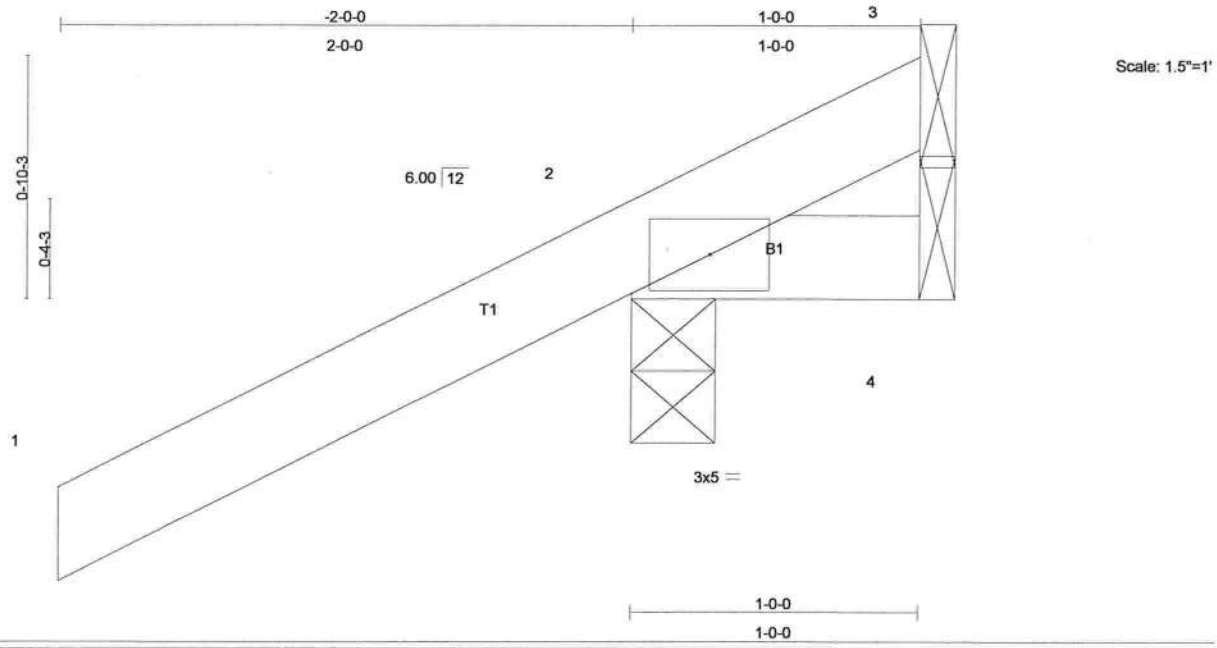
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1973297	CJ1	6/16/08
2	J1973298	CJ3	6/16/08
3	J1973299	CJ5	6/16/08
4	J1973300	EJ7	6/16/08
5	J1973301	HJ9	6/16/08
6	J1973302	T01	6/16/08
7	J1973303	T01G	6/16/08
8	J1973304	T02	6/16/08
9	J1973305	T03	6/16/08
10	J1973306	T03G	6/16/08
11	J1973307	T04	6/16/08
12	J1973308	T05	6/16/08
13	J1973309	T06	6/16/08
14	J1973310	T07	6/16/08
15	J1973311	T08	6/16/08
16	J1973312	T09	6/16/08
17	J1973313	T10	6/16/08
18	J1973314	T11	6/16/08
19	J1973315	T12	6/16/08
20	J1973316	T13	6/16/08
21	J1973317	T14	6/16/08
22	J1973318	T15	6/16/08
23	J1973319	T16	6/16/08
24	J1973320	T17	6/16/08
25	J1973321	T18	6/16/08
26	J1973322	T19	6/16/08

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973297
L279639	CJ1	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:45 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=-274(load case 6), 3=-90(load case 1)
Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.17

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 90 lb uplift at joint 3.

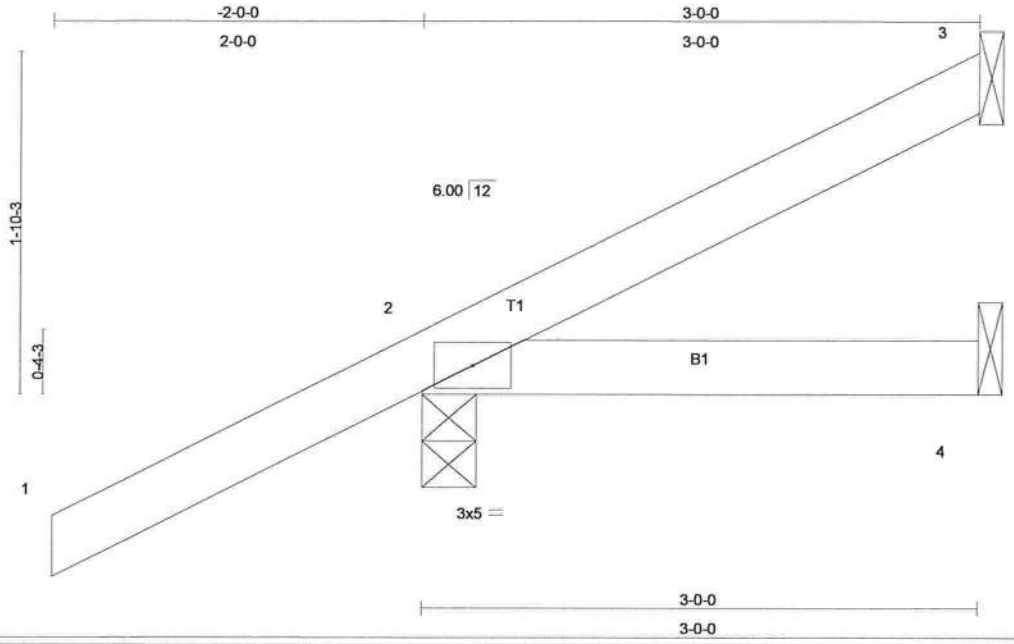
Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973298
L279639	CJ3	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:45 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift 3=-28(load case 7), 2=-203(load case 6)
Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 203 lb uplift at joint 2.

Continued on page 2

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Truss Design Engineer
Florida PE No. 34869
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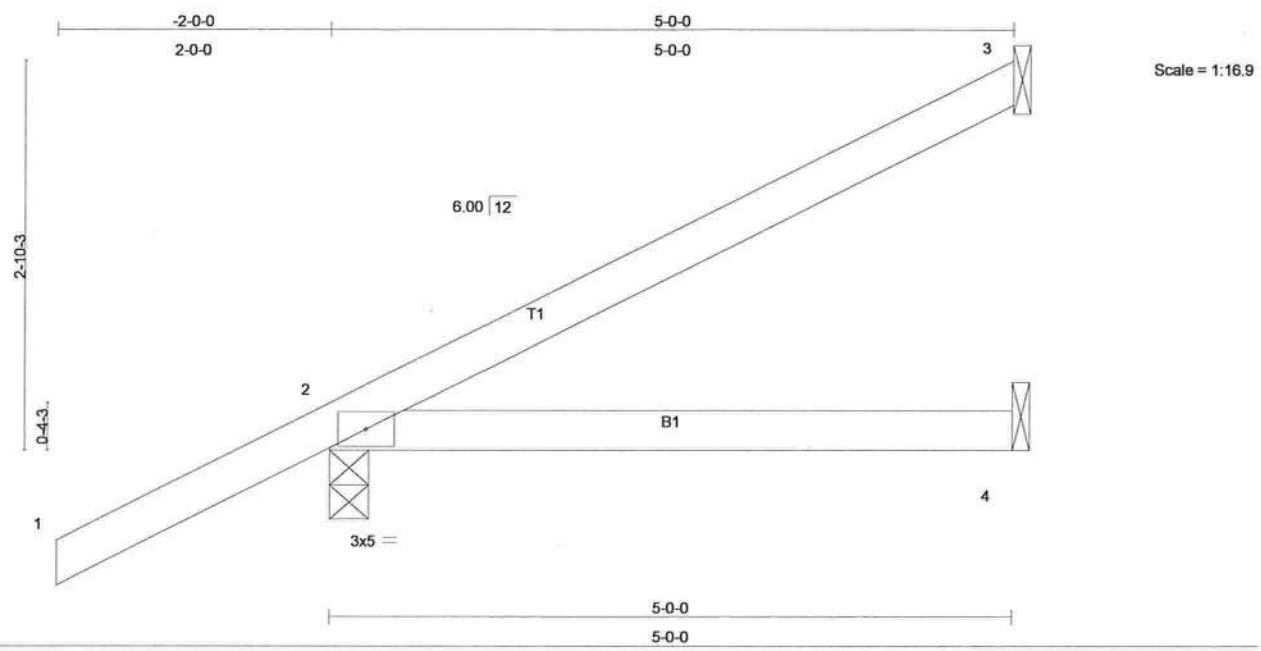
June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973299
L279639	CJ5	JACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:46 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical
Max Horz 2=178(load case 6)
Max Uplift 3=-87(load case 6), 2=-199(load case 6)
Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.17

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

Continued on page 2

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Truss Design Engineer
Florida PE No. 34869
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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973300
L279639	EJ7	JACK	25	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:47 2008 Page 1

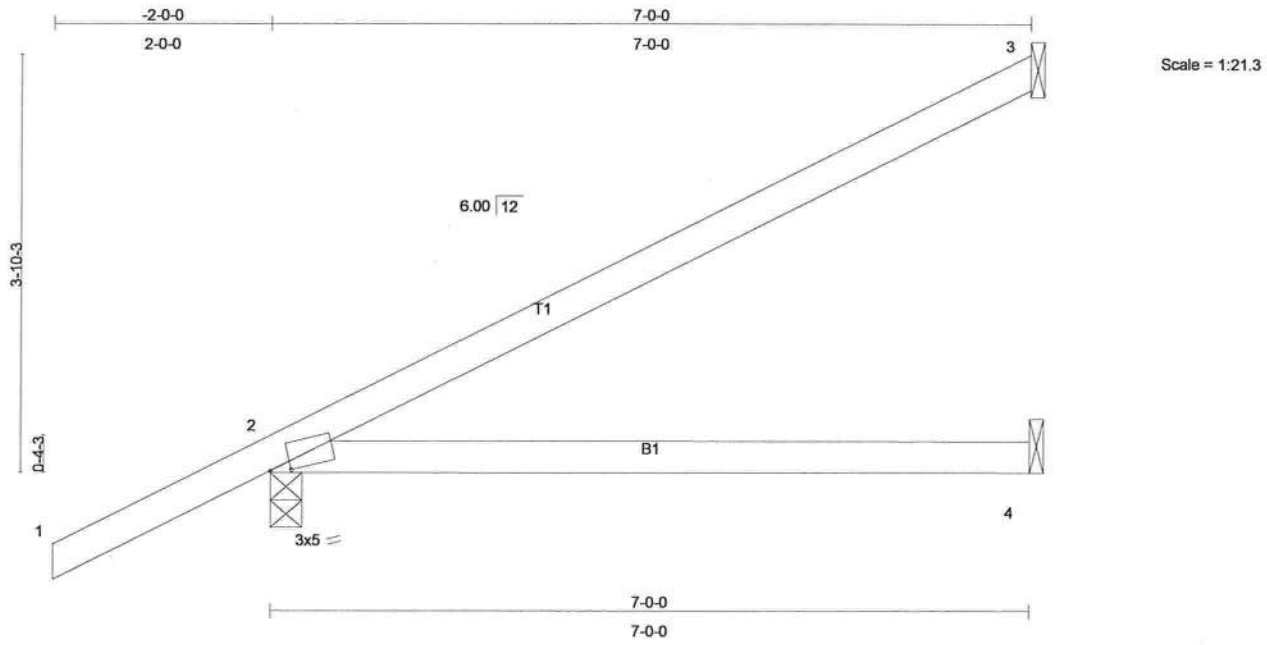

Plate Offsets (X,Y): [2:0-2-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>501	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical
Max Horz 2=161(load case 6)
Max Uplift 3=-84(load case 6), 2=-139(load case 6)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/54
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.81

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 139 lb uplift at joint 2.

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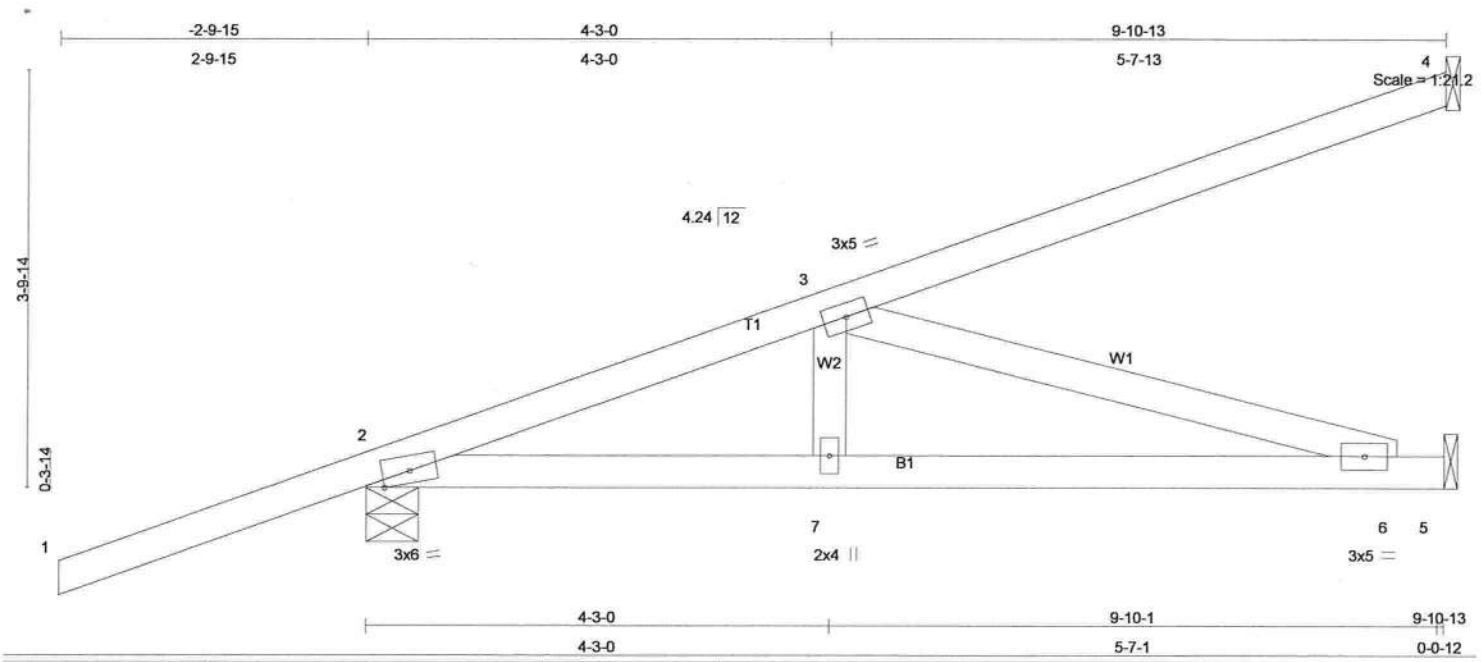
June 16,2008

Continued on page 2

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973301
L279639	HJ9	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:47 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.05	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.12	6-7	>986	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical
Max Horz 2=269(load case 3)
Max Uplift 4=-232(load case 3), 2=-281(load case 3), 5=-62(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-647/120, 3-4=-105/65
BOT CHORD 2-7=-308/599, 6-7=-308/599, 5-6=0/0
WEBS 3-7=0/190, 3-6=-624/321

JOINT STRESS INDEX
2 = 0.77, 3 = 0.18, 6 = 0.21 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 4, 281 lb uplift at joint 2 and 62 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Julius Lee
Truss Design Engineer
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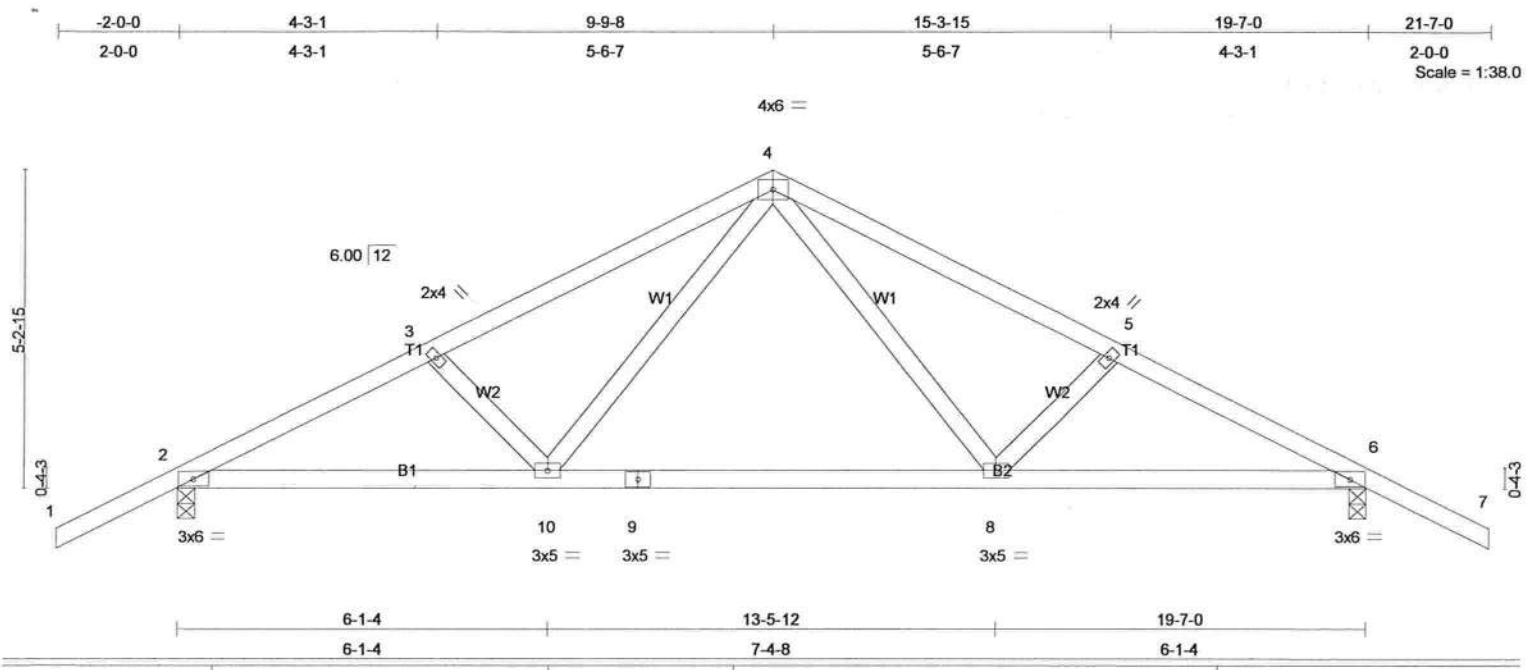
Boynton Beach, FL 33435

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973302
L279639	T01	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:07:03 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.26	8-10	>901	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.40	8-10	>572	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 94 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-9 oc bracing.

REACTIONS (lb/size) 2=955/0-3-8, 6=955/0-3-8
Max Horz 2=-94(load case 7)
Max Uplift 2=-292(load case 6), 6=-292(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1603/857, 3-4=-1456/828, 4-5=-1456/828, 5-6=-1603/857, 6-7=0/47
BOT CHORD 2-10=-608/1365, 9-10=-310/911, 8-9=-310/911, 6-8=-608/1365
WEBS 3-10=-195/185, 4-10=-287/583, 4-8=-287/583, 5-8=-195/185

JOINT STRESS INDEX
2 = 0.71, 3 = 0.34, 4 = 0.76, 5 = 0.34, 6 = 0.71, 8 = 0.42, 9 = 0.60 and 10 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 292 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1326 Coastal Bay Blvd
Boynton Beach, FL 33435

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973303
L279639	T01G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:18:03 2008 Page 1

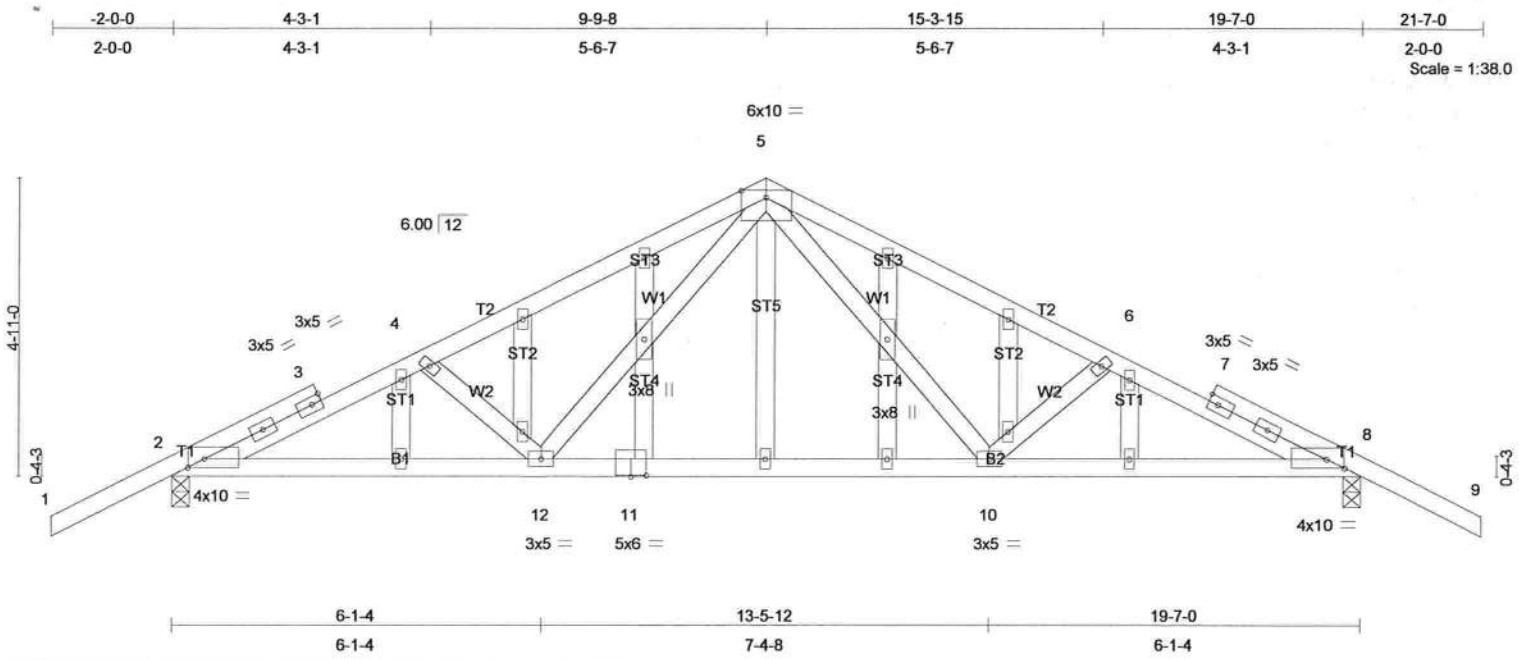


Plate Offsets (X,Y): [2:0-3-4,0-1-12], [8:0-3-4,0-1-12], [11:0-3-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.21	10-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.35	10-12	>656	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 125 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-7-9 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-9-15 oc bracing.

REACTIONS (lb/size) 2=1551/0-3-8, 8=1551/0-3-8
Max Horz 2=-102(load case 7)
Max Uplift 2=-796(load case 6), 8=-796(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-23/100, 2-3=-2682/1452, 3-4=-2575/1414, 4-5=-2272/1256, 5-6=-2272/1256, 6-7=-2575/1414, 7-8=-2682/1452, 8-9=-23/100
BOT CHORD	2-12=-1176/2364, 11-12=-612/1424, 10-11=-612/1424, 8-10=-1176/2364
WEBS	4-12=-618/422, 5-12=-401/787, 5-10=-401/787, 6-10=-618/422

JOINT STRESS INDEX

2 = 0.90, 3 = 0.00, 3 = 0.62, 3 = 0.72, 4 = 0.34, 5 = 0.48, 6 = 0.34, 7 = 0.00, 7 = 0.72, 7 = 0.62, 8 = 0.90, 10 = 0.56, 11 = 0.43, 12 = 0.56 , 13 = 0.34, 14 = 0.48, 15 = 0.34, 16 = 0.00, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.48, 24 = 0.34, 25 = 0.34, 26 = 0.34 and 27 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

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June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973304
L279639	T02	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:07:39 2008 Page 1

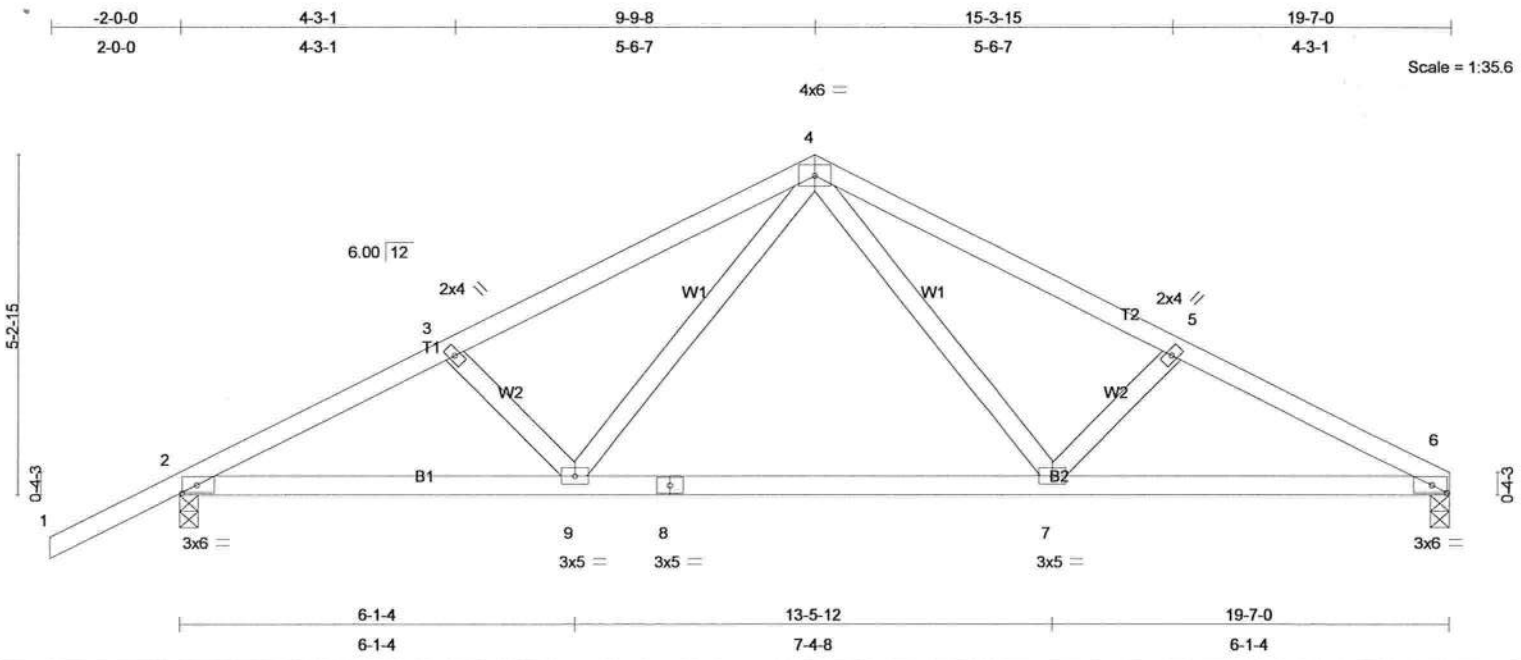

Plate Offsets (X,Y): [2:0-2-12,0-1-8], [6:0-2-12,0-1-8]

LOADING	(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.25	7-9	>921	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.40	7-9	>580	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 91 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-8-11 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 7-0-2 oc bracing.

REACTIONS (lb/size) 6=832/0-3-8, 2=961/0-3-8
Max Horz 2=107(load case 6)
Max Uplift 6=-195(load case 7), 2=-293(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-1615/878, 3-4=-1468/849, 4-5=-1495/894, 5-6=-1648/932
BOT CHORD	2-9=-704/1376, 8-9=-409/924, 7-8=-409/924, 6-7=-766/1413
WEBS	3-9=-195/187, 4-9=-282/582, 4-7=-347/619, 5-7=-217/221

JOINT STRESS INDEX

2 = 0.76, 3 = 0.34, 4 = 0.73, 5 = 0.34, 6 = 0.76, 7 = 0.45, 8 = 0.60 and 9 = 0.45

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 6 and 293 lb uplift at joint 2.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

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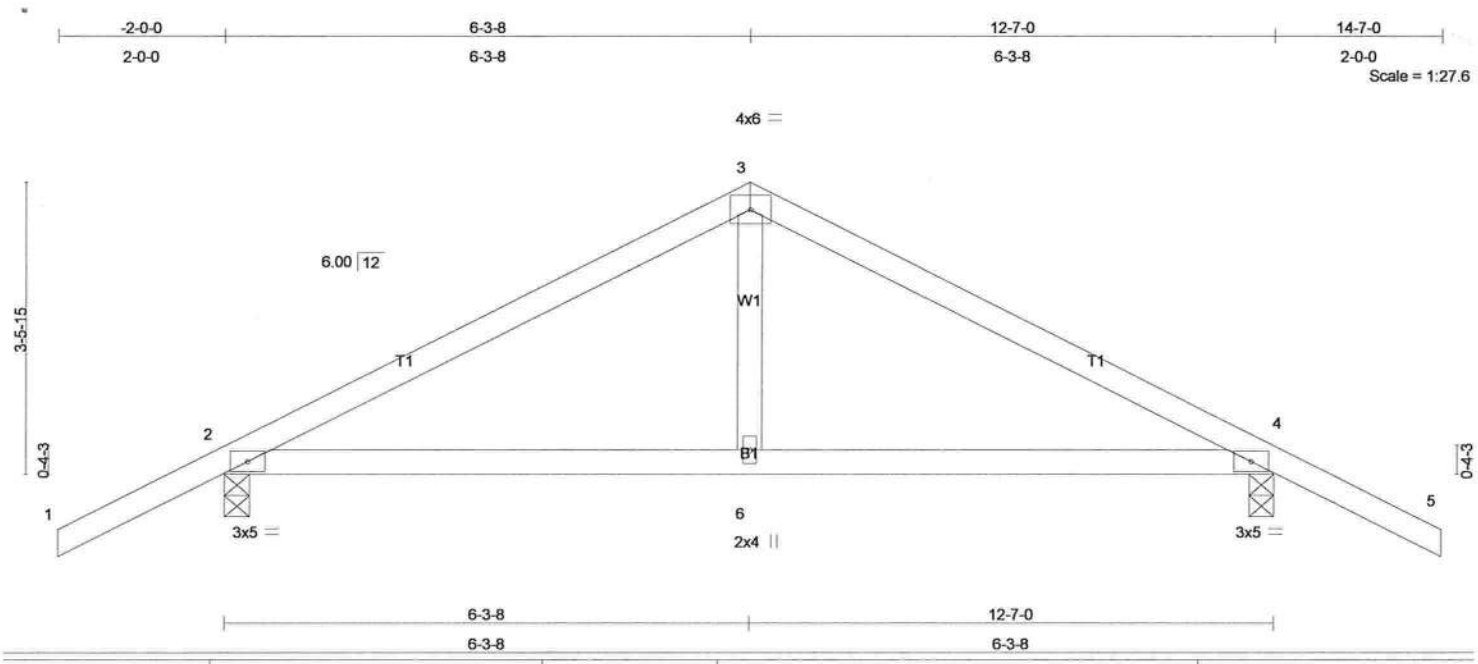
June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973305
L279639	T03	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:51 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=509/0-3-8, 4=509/0-3-8
Max Horz 2=-73(load case 7)
Max Uplift 2=-184(load case 6), 4=-184(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-537/276, 3-4=-537/276, 4-5=0/47
BOT CHORD	2-6=-66/412, 4-6=-66/412
WEBS	3-6=0/210

JOINT STRESS INDEX
2 = 0.40, 3 = 0.73, 4 = 0.40 and 6 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint 2 and 184 lb uplift at joint 4.

Continued on page 2

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973306
L279639	T03G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:52 2008 Page 1

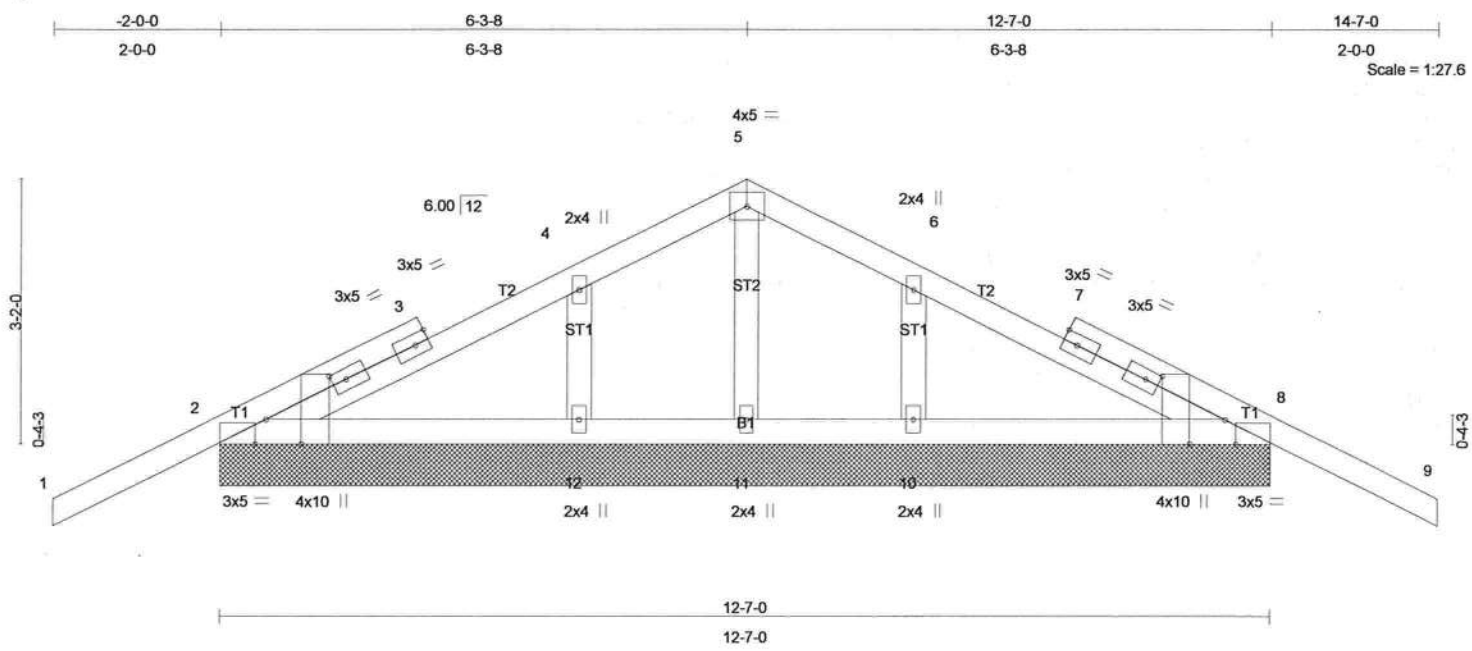

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge], [3:0-1-15,0-1-8], [7:0-1-15,0-1-8], [8:0-3-8,Edge], [8:0-1-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.03	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.05	9	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 61 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=489/12-7-0, 8=489/12-7-0, 11=206/12-7-0, 12=416/12-7-0, 10=416/12-7-0
Max Horz 2=-78(load case 7)
Max Uplift 2=-319(load case 6), 8=-332(load case 7), 11=-52(load case 6), 12=-205(load case 6), 10=-208(load case 7)
Max Grav 2=494(load case 10), 8=494(load case 11), 11=206(load case 1), 12=417(load case 10), 10=417(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-27/99, 2-3=-34/48, 3-4=-64/174, 4-5=-6/109, 5-6=-6/109, 6-7=-41/174, 7-8=-34/48, 8-9=-27/99
BOT CHORD	2-12=-71/144, 11-12=-71/144, 10-11=-71/144, 8-10=-71/144
WEBS	5-11=-206/56, 4-12=-374/295, 6-10=-374/295

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JOINT STRESS INDEX

2 = 0.78, 2 = 0.00, 3 = 0.00, 3 = 0.49, 3 = 0.49, 4 = 0.15, 5 = 0.10, 6 = 0.15, 7 = 0.00, 7 = 0.49, 7 = 0.49, 8 = 0.78, 8 = 0.00, 10 = 0.16, 11 = 0.07 and 12 = 0.16

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973307
L279639	T04	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:53 2008 Page 1

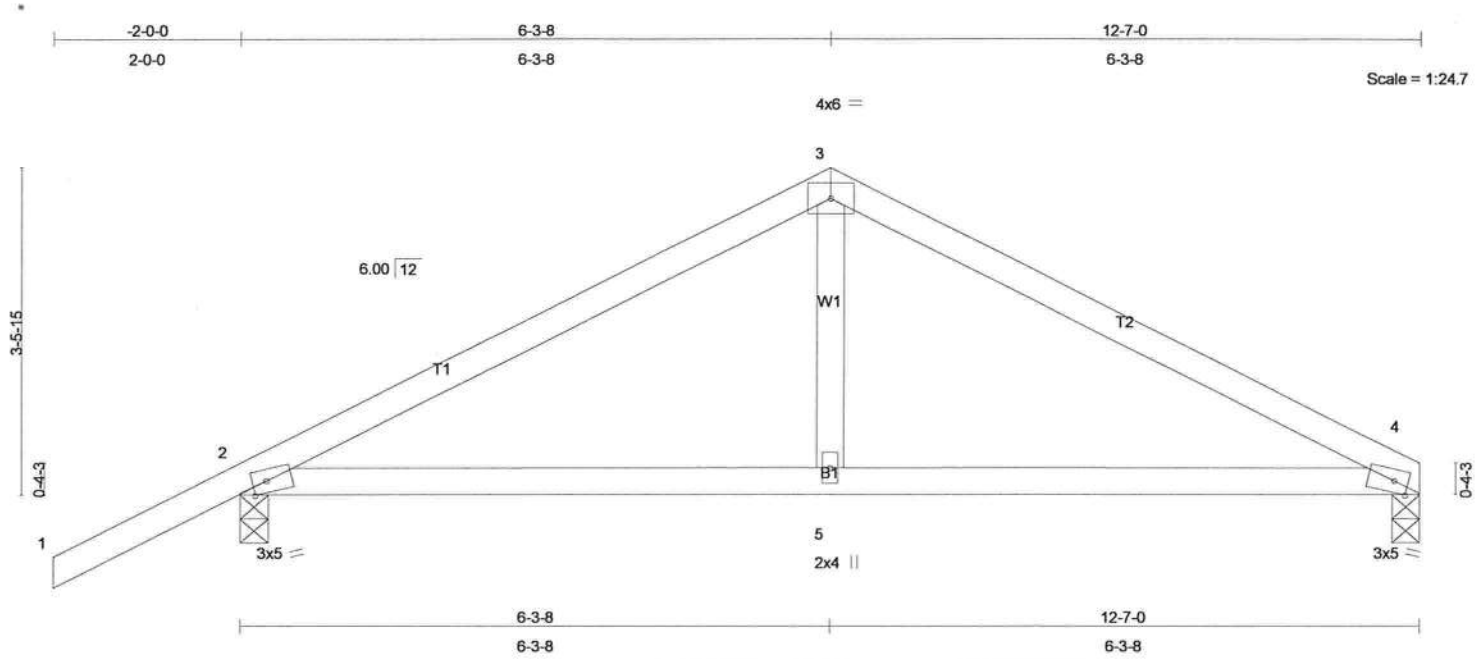

Plate Offsets (X,Y): [2:0-1-12,Edge], [4:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.05	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.07	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 48 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=383/0-3-8, 2=519/0-3-8
Max Horz 2=86(load case 6)
Max Uplift 4=-84(load case 7), 2=-186(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-568/328, 3-4=-564/320
BOT CHORD 2-5=-191/441, 4-5=-191/441
WEBS 3-5=0/214

JOINT STRESS INDEX
2 = 0.73, 3 = 0.81, 4 = 0.73 and 5 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
L279639	T05	COMMON	1	2	J1973308
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:54 2008 Page 1

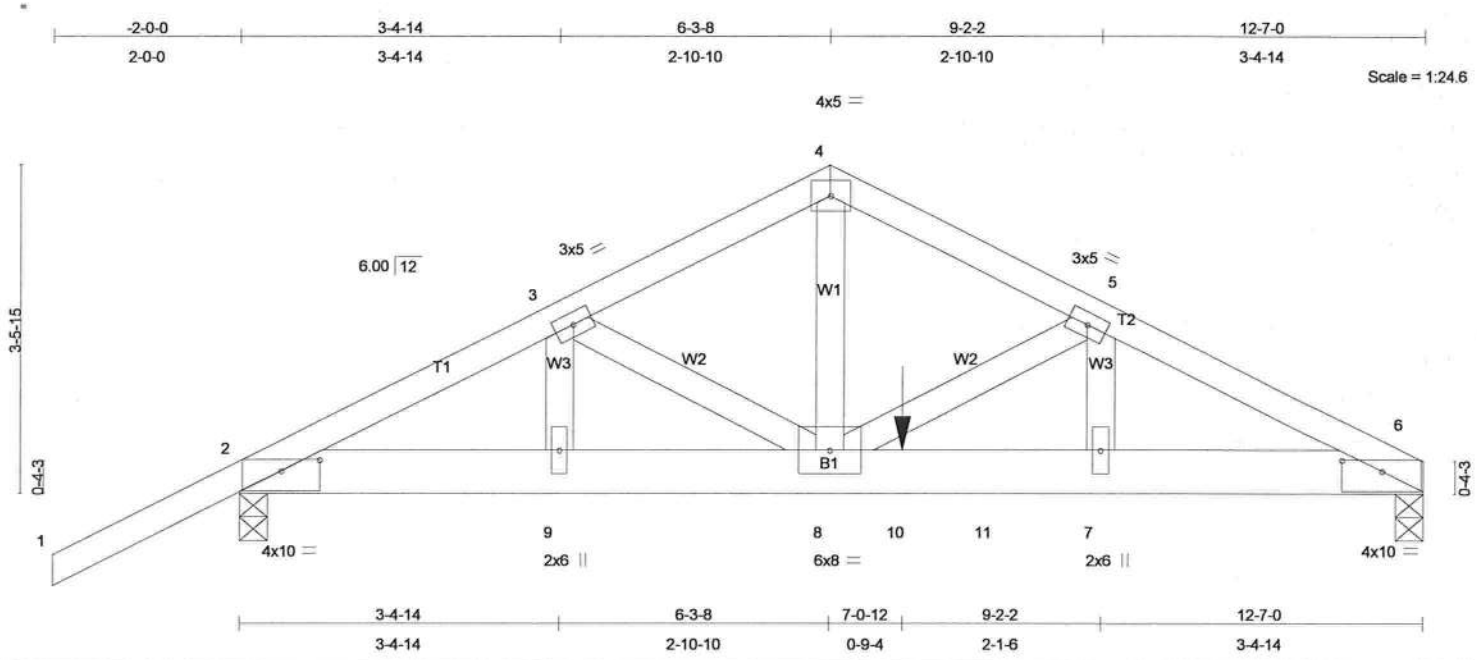

Plate Offsets (X,Y): [2:0-5-0,0-1-7], [6:0-5-0,0-1-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.06	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.11	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-4-13 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=3563/0-3-8, 2=1960/0-3-8
Max Horz 2=89(load case 5)
Max Uplift 6=-964(load case 6), 2=-585(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/51, 2-3=-3554/919, 3-4=-3847/1051, 4-5=-3843/1043, 5-6=-5951/1603
BOT CHORD	2-9=-785/3123, 8-9=-785/3123, 8-10=-1397/5275, 10-11=-1397/5275, 7-11=-1397/5275, 6-7=-1397/5275
WEBS	3-9=-410/169, 3-8=-115/445, 4-8=-864/3193, 5-8=-2145/625, 5-7=-507/1917

JOINT STRESS INDEX
2 = 0.65, 3 = 0.81, 4 = 0.75, 5 = 0.81, 6 = 0.65, 7 = 0.45, 8 = 0.39 and 9 = 0.45

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Continued on page 2

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973309
L279639	T06	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:54 2008 Page 1

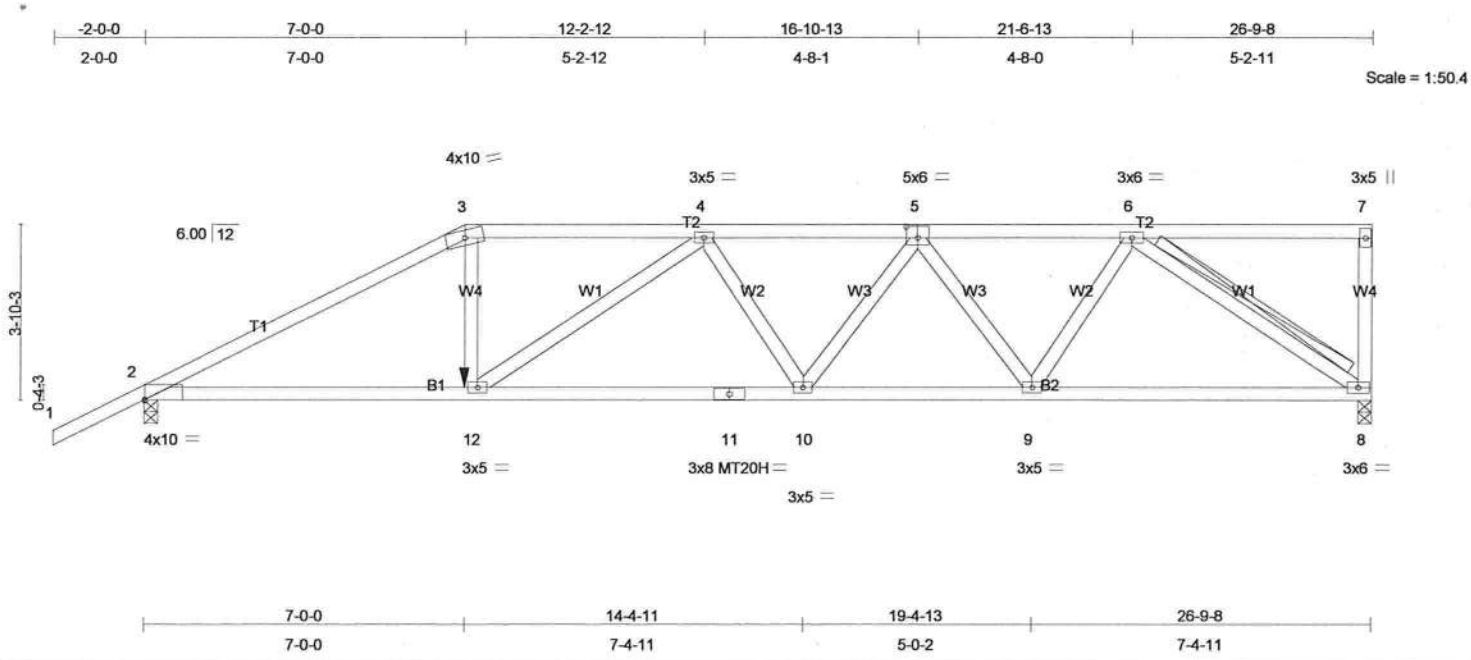

Plate Offsets (X,Y): [2:Edge,0-0-2], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.19	10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.44	10-12	>728	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-4 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=1883/0-3-8, 2=1818/0-3-8
Max Horz 2=163(load case 5)
Max Uplift 8=-649(load case 4), 2=-578(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3360/1085, 3-4=-2956/1008, 4-5=-3532/1196, 5-6=-2790/938, 6-7=-85/15, 7-8=-288/143
BOT CHORD 2-12=-997/2914, 11-12=-1273/3610, 10-11=-1273/3610, 9-10=-1162/3346, 8-9=-798/2236
WEBS 3-12=-296/996, 4-12=-799/378, 4-10=-150/148, 5-10=-60/323, 5-9=-966/389, 6-9=-270/1065, 6-8=-2625/956

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JOINT STRESS INDEX
2 = 0.76, 3 = 0.77, 4 = 0.42, 5 = 0.55, 6 = 0.79, 7 = 0.64, 8 = 0.79, 9 = 0.79, 10 = 0.42, 11 = 0.87 and 12 = 0.73

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973310
L279639	T07	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:55:55 2008 Page 1

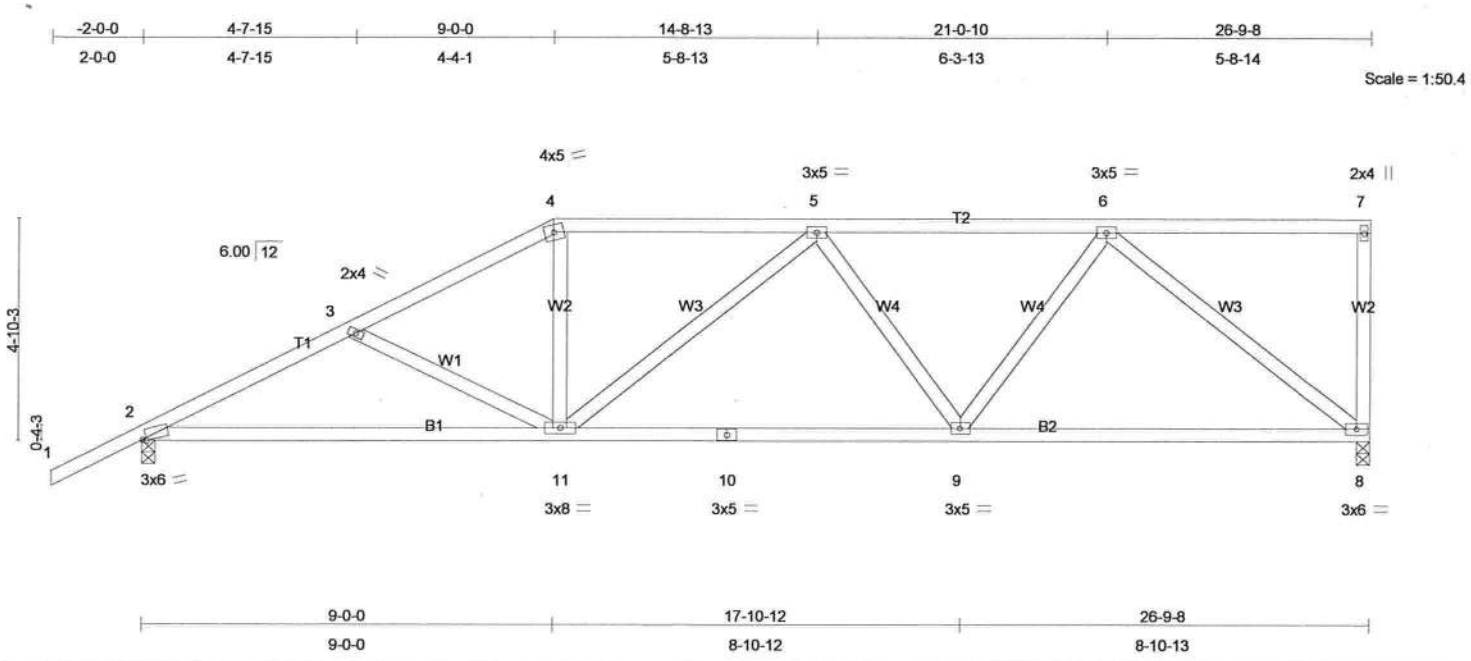

Plate Offsets (X,Y): [2:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.13	2-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.24	2-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-7 oc bracing.

REACTIONS (lb/size) 8=843/0-3-8, 2=969/0-3-8
Max Horz 2=195(load case 6)
Max Uplift 8=-229(load case 5), 2=-248(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1537/763, 3-4=-1299/656, 4-5=-1126/644, 5-6=-1073/568, 6-7=-40/7 , 7-8=-135/93
BOT CHORD 2-11=-827/1311, 10-11=-698/1238, 9-10=-698/1238, 8-9=-476/847
WEBS 3-11=-216/207, 4-11=-73/334, 5-11=-145/117, 5-9=-290/228, 6-9=-162/410, 6-8=-1052/605

JOINT STRESS INDEX
2 = 0.83, 3 = 0.33, 4 = 0.74, 5 = 0.41, 6 = 0.41, 7 = 0.82, 8 = 0.60, 9 = 0.41, 10 = 0.48 and 11 = 0.56

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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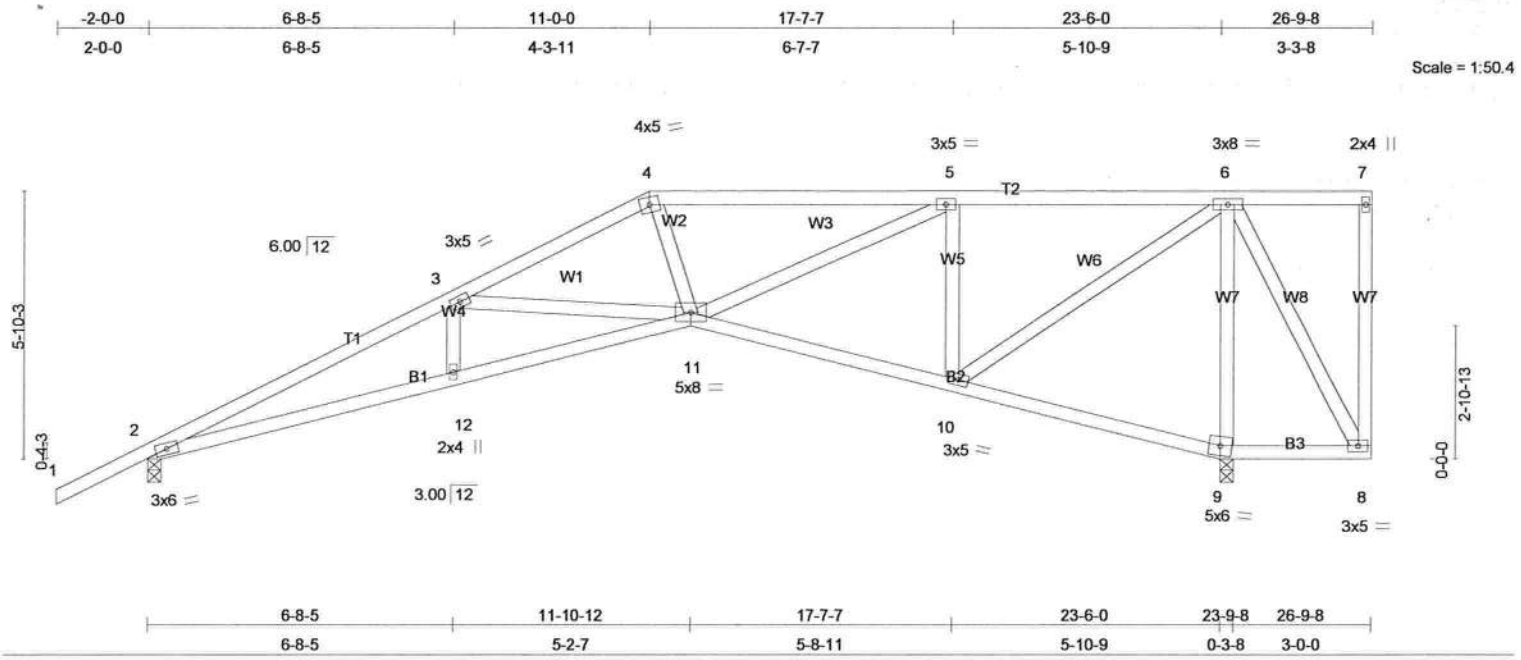
June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973311
L279639	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:09:15 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.20	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.27	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.54	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 149 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-10-12 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-6-9 oc bracing.

REACTIONS (lb/size) 2=844/0-3-8, 9=1052/0-3-8
Max Horz 2=226(load case 6)
Max Uplift 2=-237(load case 6), 9=-284(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/46, 2-3=-2229/1204, 3-4=-1600/921, 4-5=-1523/937, 5-6=-746/435, 6-7=-2/3, 7-8=-92/58
BOT CHORD	2-12=-1289/1967, 11-12=-1290/1966, 10-11=-453/776, 9-10=-105/59, 8-9=-69/37
WEBS	3-12=0/185, 3-11=-517/378, 4-11=-207/426, 5-11=-553/856, 5-10=-722/491, 6-10=-588/1015, 6-9=-966/593, 6-8=-74/139

JOINT STRESS INDEX

2 = 0.72, 3 = 0.48, 4 = 0.79, 5 = 0.56, 6 = 0.97, 7 = 0.34, 8 = 0.46, 9 = 0.45, 10 = 0.66, 11 = 0.61 and 12 = 0.34

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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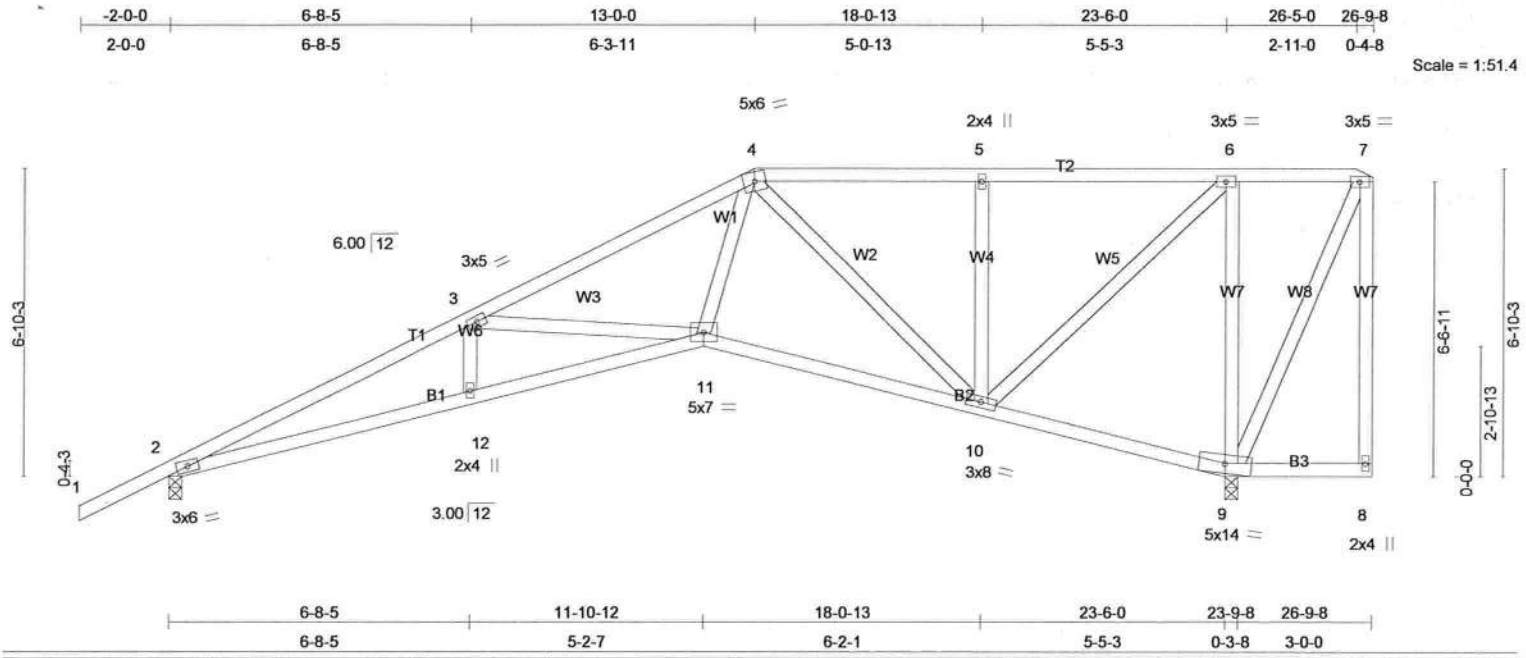
June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
L279639	T09	SPECIAL	1	1	J1973312
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:11:11 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.20	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.26	11-12	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.67	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 159 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-1 oc bracing.

REACTIONS (lb/size) 2=844/0-3-8, 9=1052/0-3-8
Max Horz 2=258(load case 6)
Max Uplift 2=-240(load case 6), 9=-281(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2257/1244, 3-4=-1533/879, 4-5=-537/327, 5-6=-537/327, 6-7=-33/65, 7-8=-9/50
BOT CHORD 2-12=-1383/1997, 11-12=-1385/1997, 10-11=-720/1079, 9-10=-92/52, 8-9=-0/1
WEBS 3-12=0/180, 3-11=-633/487, 4-11=-550/899, 4-10=-719/526, 5-10=-308/194, 6-10=-508/847, 6-9=-846/542, 7-9=-152/77

JOINT STRESS INDEX
2 = 0.73, 3 = 0.48, 4 = 0.57, 5 = 0.34, 6 = 0.59, 7 = 0.48, 8 = 0.34, 9 = 0.42, 10 = 0.84, 11 = 0.66 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973313
L279639	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:12:02 2008 Page 1

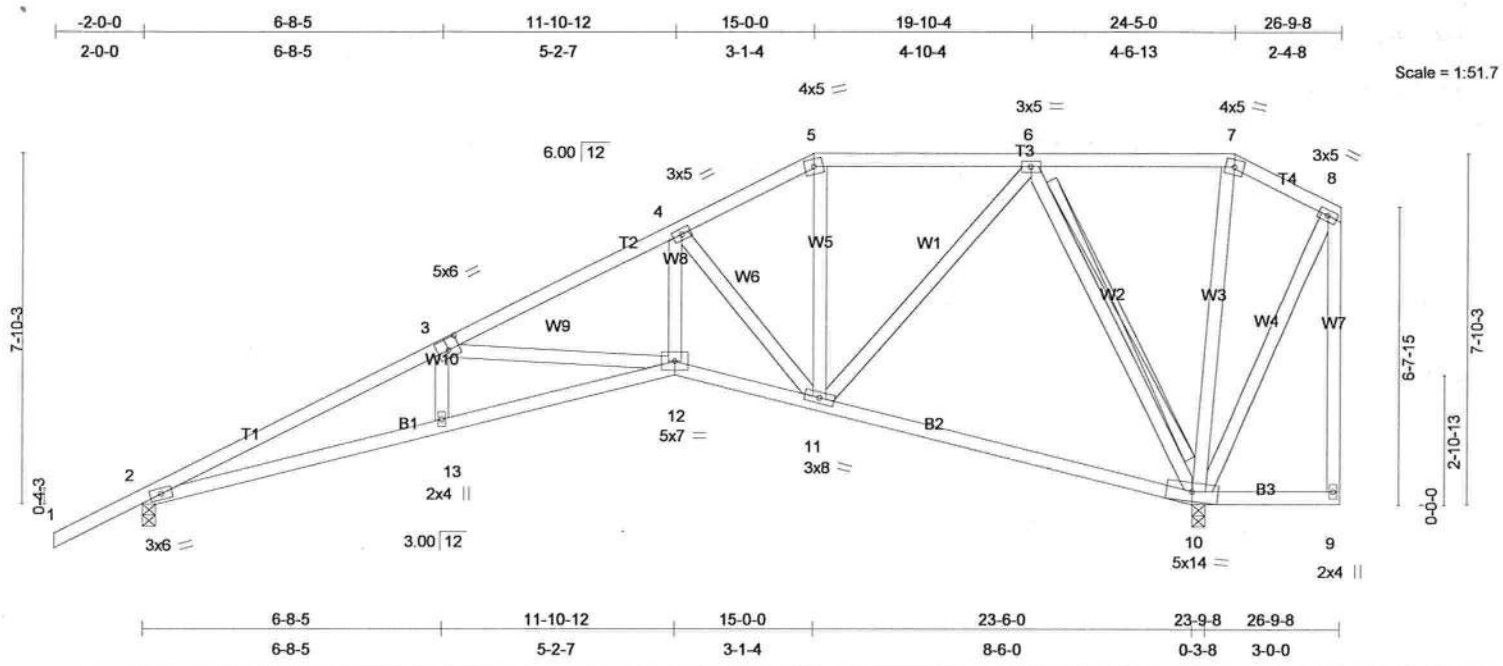

Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.20	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.27	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.17	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-7 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=844/0-3-8, 10=1052/0-3-8
Max Horz 2=267(load case 6)
Max Uplift 2=-247(load case 6), 10=-221(load case 5)
Max Grav 2=851(load case 10), 10=1052(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2281/1240, 3-4=-1579/913, 4-5=-814/494, 5-6=-699/475, 6-7=-2/72, 7-8=-19/90, 8-9=-17/27
BOT CHORD 2-13=-1369/2017, 12-13=-1371/2014, 11-12=-922/1391, 10-11=-187/324, 9-10=-3/0
WEBS 3-13=0/198, 3-12=-604/436, 4-12=-582/903, 4-11=-1021/741, 5-11=-68/183, 6-11=-371/607, 6-10=-811/525, 7-10=-230/157, 8-10=-98/79

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JOINT STRESS INDEX
2 = 0.73, 3 = 0.55, 4 = 0.79, 5 = 0.42, 6 = 0.46, 7 = 0.60, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.64, 12 = 0.67 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973314
L279639	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:12:49 2008 Page 1

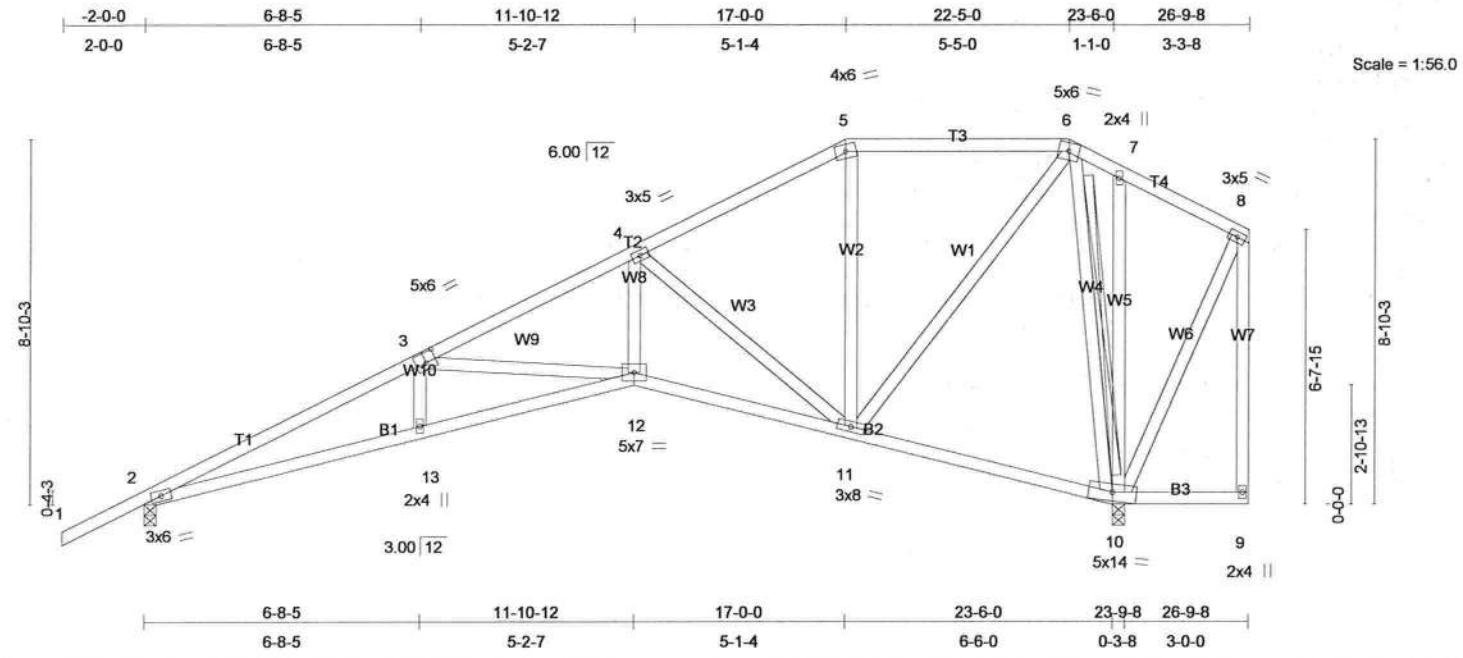

Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.21	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.28	12-13	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.18	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=844/0-3-8, 10=1052/0-3-8
Max Horz 2=279(load case 6)
Max Uplift 2=-261(load case 6), 10=-272(load case 6)
Max Grav 2=851(load case 10), 10=1052(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2275/1247, 3-4=-1589/929, 4-5=-587/361, 5-6=-472/378, 6-7=0/94, 7-8=-16/110, 8-9=-13/39
BOT CHORD 2-13=-1375/2011, 12-13=-1377/2009, 11-12=-940/1408, 10-11=-35/66, 9-10=-3/2
WEBS 3-13=0/188, 3-12=-586/424, 4-12=-573/927, 4-11=-1160/830, 5-11=-76/75, 6-11=-429/690, 6-10=-779/449, 7-10=-143/119, 8-10=-145/105

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JOINT STRESS INDEX
2 = 0.73, 3 = 0.52, 4 = 0.81, 5 = 0.60, 6 = 0.32, 7 = 0.34, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.75, 12 = 0.67 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973315
L279639	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jun 16 13:13:34 2008 Page 1

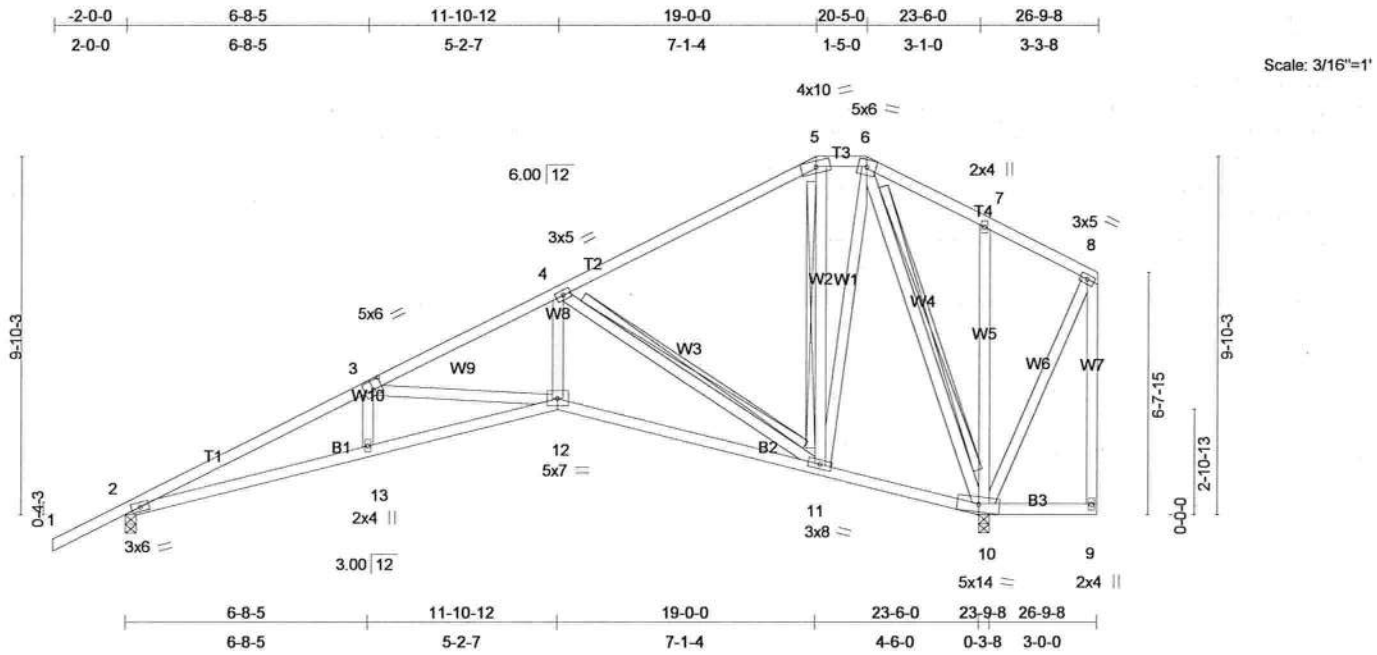

Plate Offsets (X,Y): [3:0-3-0,0-3-0]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.22	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.29	12-13	>967	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.63	Horz(TL)	0.20	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-11, 5-11, 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=850/0-3-8, 10=1046/0-3-8
Max Horz 2=291(load case 6)
Max Uplift 2=-253(load case 6), 10=-239(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-2260/1266, 3-4=-1611/970, 4-5=-418/274, 5-6=-295/322, 6-7=0/102, 7-8=-13/109, 8-9=-8/56
BOT CHORD 2-13=-1391/1996, 12-13=-1392/1996, 11-12=-987/1440, 10-11=-95/199, 9-10=-2/2
WEBS 3-13=0/174, 3-12=-543/395, 4-12=-575/942, 4-11=-1315/949, 5-11=-236/201, 6-11=-481/742, 6-10=-762/403, 7-10=-203/204, 8-10=-145/94

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JOINT STRESS INDEX
2 = 0.73, 3 = 0.47, 4 = 0.82, 5 = 0.67, 6 = 0.43, 7 = 0.34, 8 = 0.48, 9 = 0.34, 10 = 0.42, 11 = 0.93, 12 = 0.70 and 13 = 0.34

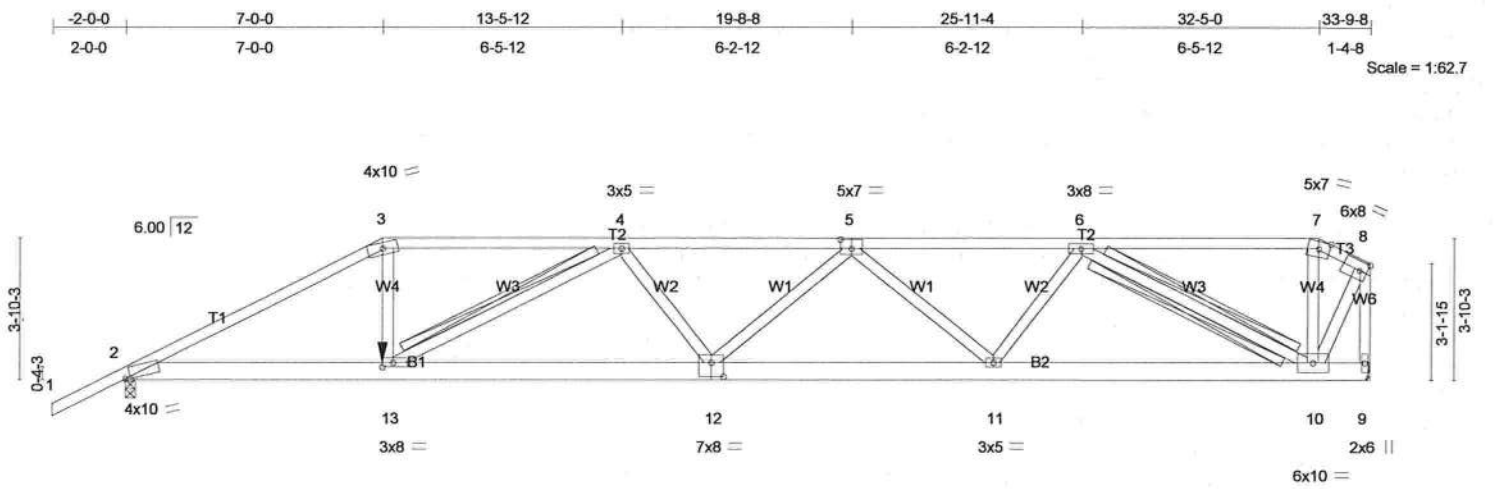
NOTES
1) Unbalanced roof live loads have been considered for this design.

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973316
L279639	T13	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:01 2008 Page 1



Simpson HTU26

Plate Offsets (X,Y): [2:0-1-11,0-0-6], [5:0-3-8,0-3-0], [12:0-4-0,0-4-8], [13:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.36	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.68	12-13	>589	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.15	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS I-Brace: 2 X 4 SYP No.3 - 6-10
T-Brace: 2 X 4 SYP No.3 - 4-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2313/0-3-8, 9=2363/Mechanical
Max Horz 2=122(load case 5)
Max Uplift 2=-716(load case 5), 9=-787(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4517/1485, 3-4=-4027/1378, 4-5=-5721/1945, 5-6=-4785/1629, 6-7=-1016/363, 7-8=-1050/329, 8-9=-2367/718
BOT CHORD 2-13=-1324/3966, 12-13=-1954/5639, 11-12=-1955/5649, 10-11=-1442/4130, 9-10=-9/32
WEBS 3-13=-418/1454, 4-13=-1930/748, 4-12=0/271, 5-12=0/182, 5-11=-1166/489, 6-11=-265/1154, 6-10=-3553/1277, 7-10=-209/197, 8-10=-738/2278

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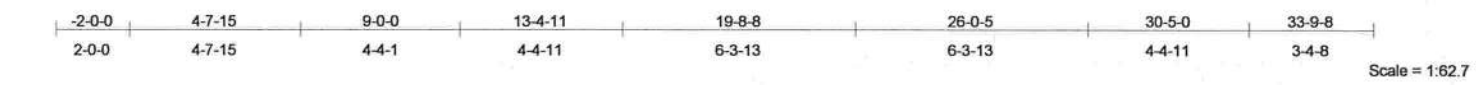
June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973317
L279639	T14	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:02 2008 Page 1



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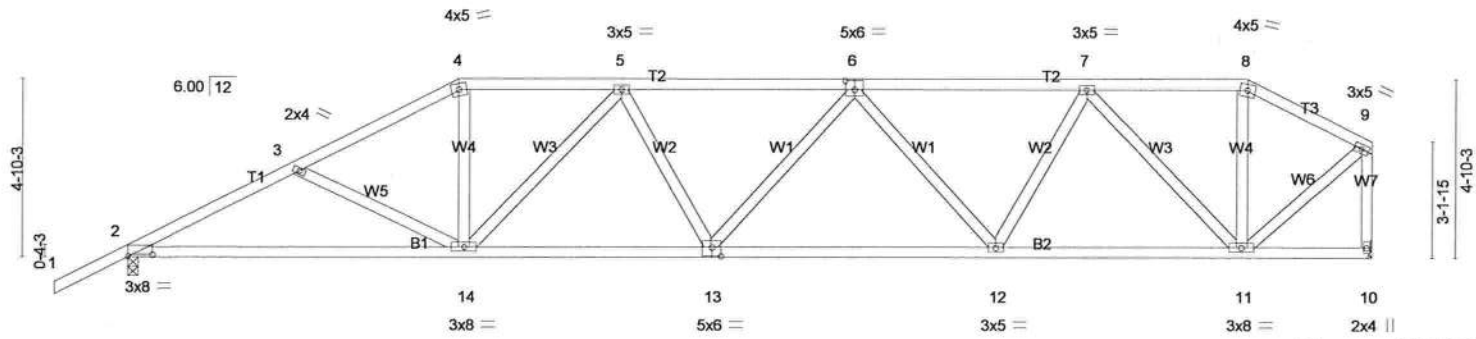

Simpson HTU26

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [6:0-3-0,0-3-0], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.16	2-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.31	2-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.09	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-4-1 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-3-6 oc bracing.

REACTIONS (lb/size) 2=1192/0-3-8, 10=1068/Mechanical
Max Horz 2=162(load case 6)
Max Uplift 2=-277(load case 6), 10=-241(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-2014/1040, 3-4=-1776/933, 4-5=-1558/894, 5-6=-1933/1073, 6-7=-1637/909, 7-8=-727/447, 8-9=-843/455, 9-10=-1051/566
BOT CHORD	2-14=-990/1734, 13-14=-992/1893, 12-13=-1019/1938, 11-12=-728/1405, 10-11=-15/17
WEBS	3-14=-216/207, 4-14=-227/535, 5-14=-566/265, 5-13=-17/150, 6-13=-64/94, 6-12=-473/299, 7-12=-205/497, 7-11=-1009/534, 8-11=-52/208, 9-11=-460/932

JOINT STRESS INDEX

2 = 0.70, 3 = 0.33, 4 = 0.63, 5 = 0.43, 6 = 0.50, 7 = 0.43, 8 = 0.38, 9 = 0.66, 10 = 0.37, 11 = 0.89, 12 = 0.43, 13 = 0.56 and 14 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

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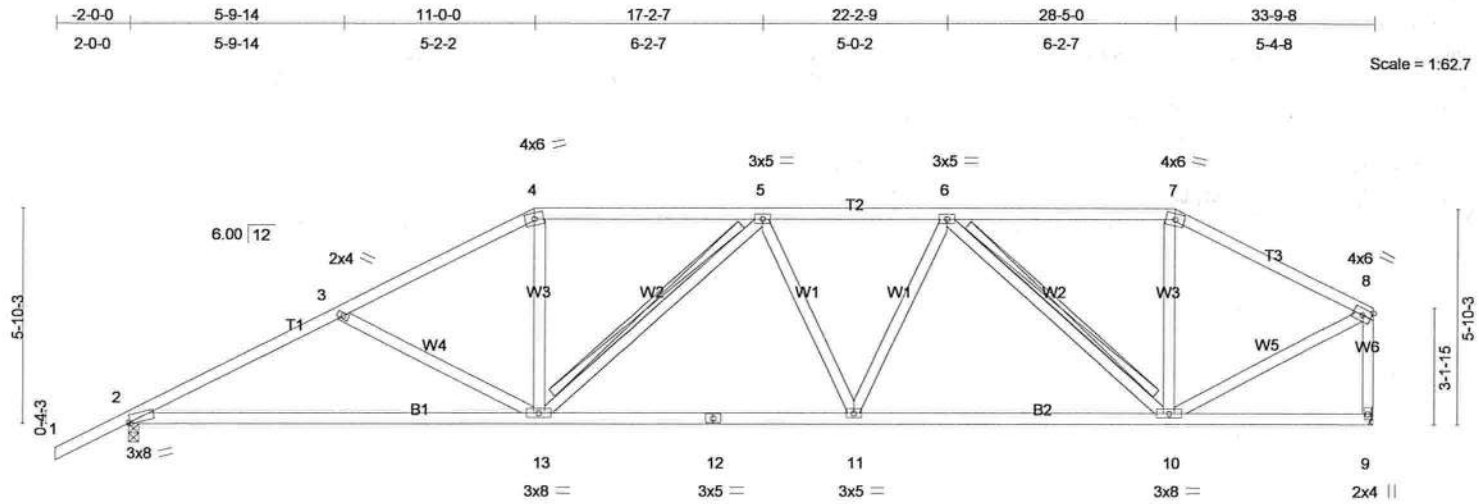
June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973318
L279639	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:03 2008 Page 1



Simpson HTU26

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.30	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.56	2-13	>713	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-14 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1192/0-3-8, 9=1068/Mechanical
Max Horz 2=173(load case 6)
Max Uplift 2=-292(load case 6), 9=-210(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1980/1046, 3-4=-1675/905, 4-5=-1453/876, 5-6=-1564/919, 6-7=-903/579, 7-8=-1061/577, 8-9=-1044/580
BOT CHORD 2-13=-985/1700, 12-13=-839/1627, 11-12=-839/1627, 10-11=-752/1465, 9-10=-34/36
WEBS 3-13=-291/278, 4-13=-152/439, 5-13=-350/150, 5-11=-166/131, 6-11=-80/278, 6-10=-803/416, 7-10=-11/229, 8-10=-469/983

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JOINT STRESS INDEX
2 = 0.88, 3 = 0.33, 4 = 0.74, 5 = 0.45, 6 = 0.45, 7 = 0.71, 8 = 0.68, 9 = 0.43, 10 = 0.90, 11 = 0.45, 12 = 0.57 and 13 = 0.56

Continued on page 2

June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973319
L279639	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:04 2008 Page 1

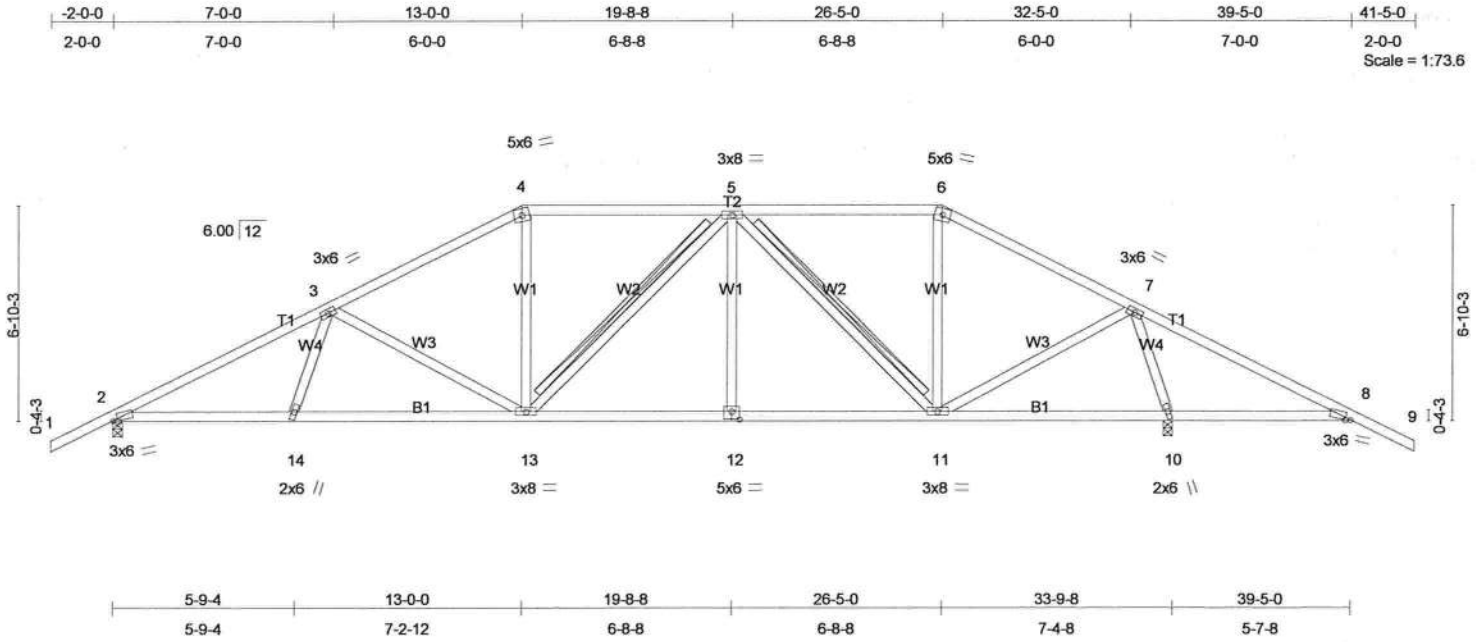

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [8:0-1-13,0-0-7], [12:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.09	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.20	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 214 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1140/0-3-8, 10=1597/0-3-8
Max Horz 2=-113(load case 7)
Max Uplift 2=-303(load case 6), 10=-535(load case 7)
Max Grav 2=1144(load case 10), 10=1597(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1900/878, 3-4=-1440/745, 4-5=-1227/734, 5-6=-769/409, 6-7=-927/389, 7-8=-846/733, 8-9=0/47
BOT CHORD 2-14=-601/1616, 13-14=-615/1591, 12-13=-272/1220, 11-12=-272/1220, 10-11=-122/538, 8-10=-578/905
WEBS 3-14=0/221, 3-13=-422/336, 4-13=-68/340, 5-13=-134/95, 5-12=0/186, 5-11=-638/388, 6-11=-6/196, 7-11=-612/970, 7-10=-1579/1193

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JOINT STRESS INDEX
2 = 0.79, 3 = 0.72, 4 = 0.54, 5 = 0.56, 6 = 0.54, 7 = 0.72, 8 = 0.79, 10 = 0.54, 11 = 0.89, 12 = 0.44, 13 = 0.89 and 14 = 0.54

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973320
L279639	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:06 2008 Page 1

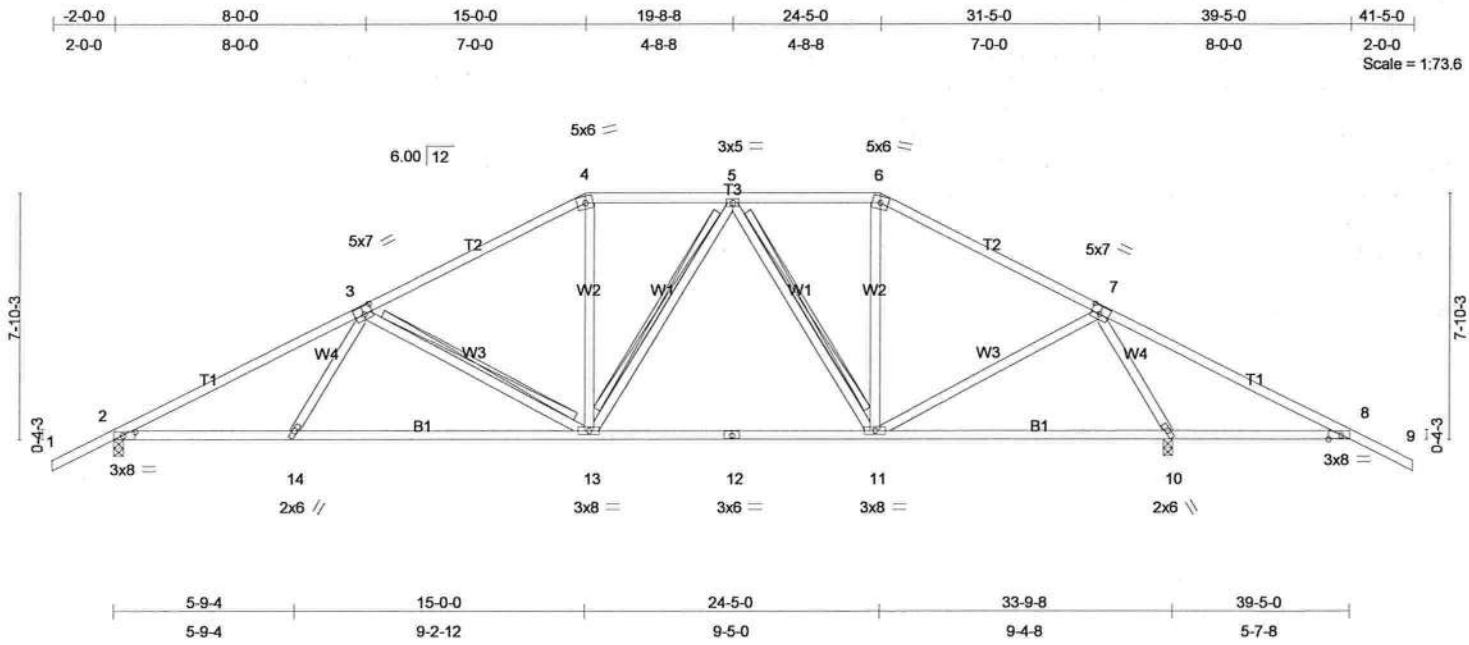

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [3:0-3-4,0-3-0], [7:0-3-4,0-3-0], [8:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.16	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.33	13-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.06	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-13, 5-13, 5-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1142/0-3-8, 10=1595/0-3-8
Max Horz 2=-125(load case 7)
Max Uplift 2=-314(load case 6), 10=-549(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1887/872, 3-4=-1315/705, 4-5=-1099/703, 5-6=-823/505, 6-7=-1011/489, 7-8=-883/781, 8-9=0/47
BOT CHORD 2-14=-582/1595, 13-14=-606/1537, 12-13=-180/1023, 11-12=-180/1023, 10-11=-13/296, 8-10=-607/950
WEBS 3-14=0/291, 3-13=-511/421, 4-13=-54/307, 5-13=-117/156, 5-11=-433/266, 6-11=-11/221, 7-11=-388/666, 7-10=-1703/1317

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JOINT STRESS INDEX
2 = 0.70, 3 = 0.84, 4 = 0.54, 5 = 0.43, 6 = 0.54, 7 = 0.84, 8 = 0.70, 10 = 0.74, 11 = 0.61, 12 = 0.43, 13 = 0.61 and 14 = 0.74

June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973321
L279639	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:07 2008 Page 1

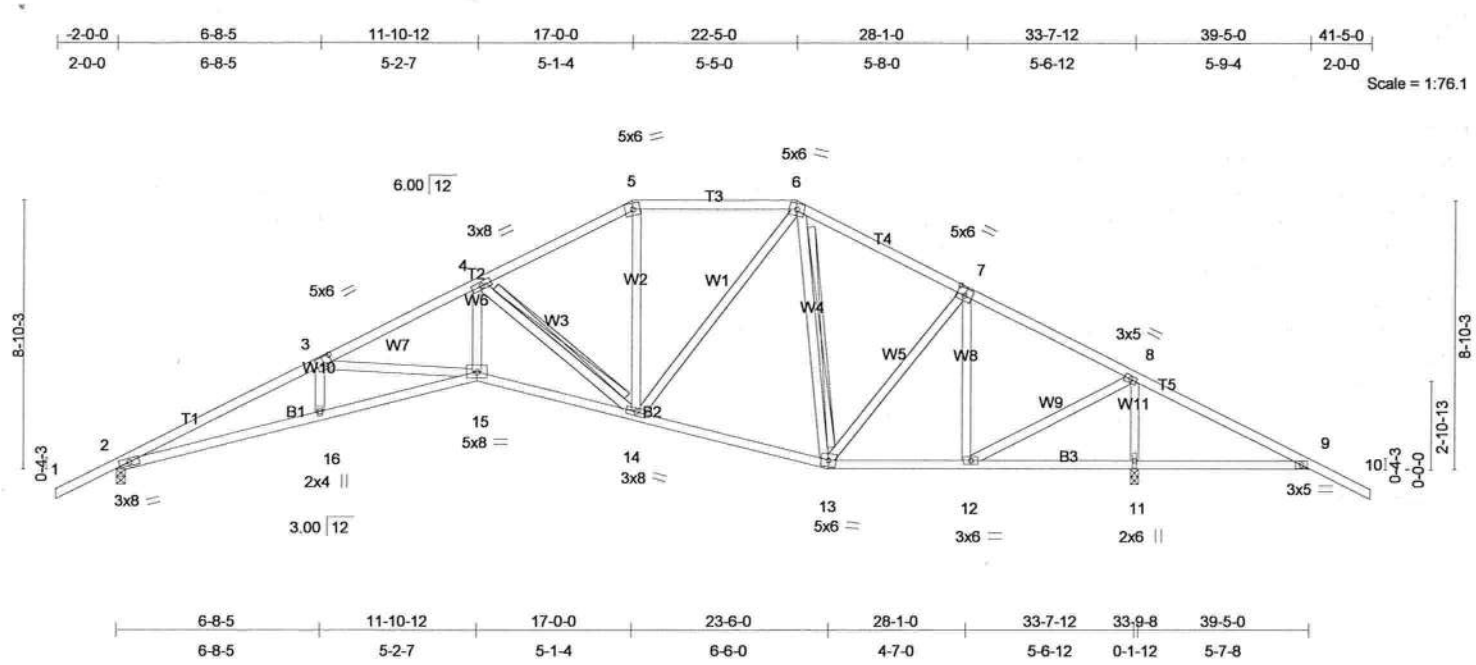

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.29	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.54	15-16	>743	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.29	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 221 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 6-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1138/0-3-8, 11=1599/0-3-8
Max Horz 2=-136(load case 7)
Max Uplift 2=-350(load case 6), 11=-566(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3441/1524, 3-4=-2805/1216, 4-5=-1362/711, 5-6=-1172/693, 6-7=-959/544, 7-8=-774/276, 8-9=-807/679, 9-10=0/47
BOT CHORD 2-16=-1211/3075, 15-16=-1213/3074, 14-15=-787/2529, 13-14=-117/868, 12-13=-83/631, 11-12=-540/858, 9-11=-540/858
WEBS 3-16=0/184, 3-15=-537/413, 4-15=-495/1451, 4-14=-1676/759, 5-14=-105/336, 6-14=-213/590, 6-13=-347/156, 7-13=-150/259, 7-12=-581/474, 8-12=-899/1318, 8-11=-1520/1150

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June 16,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973322
L279639	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Jun 16 12:56:08 2008 Page 1

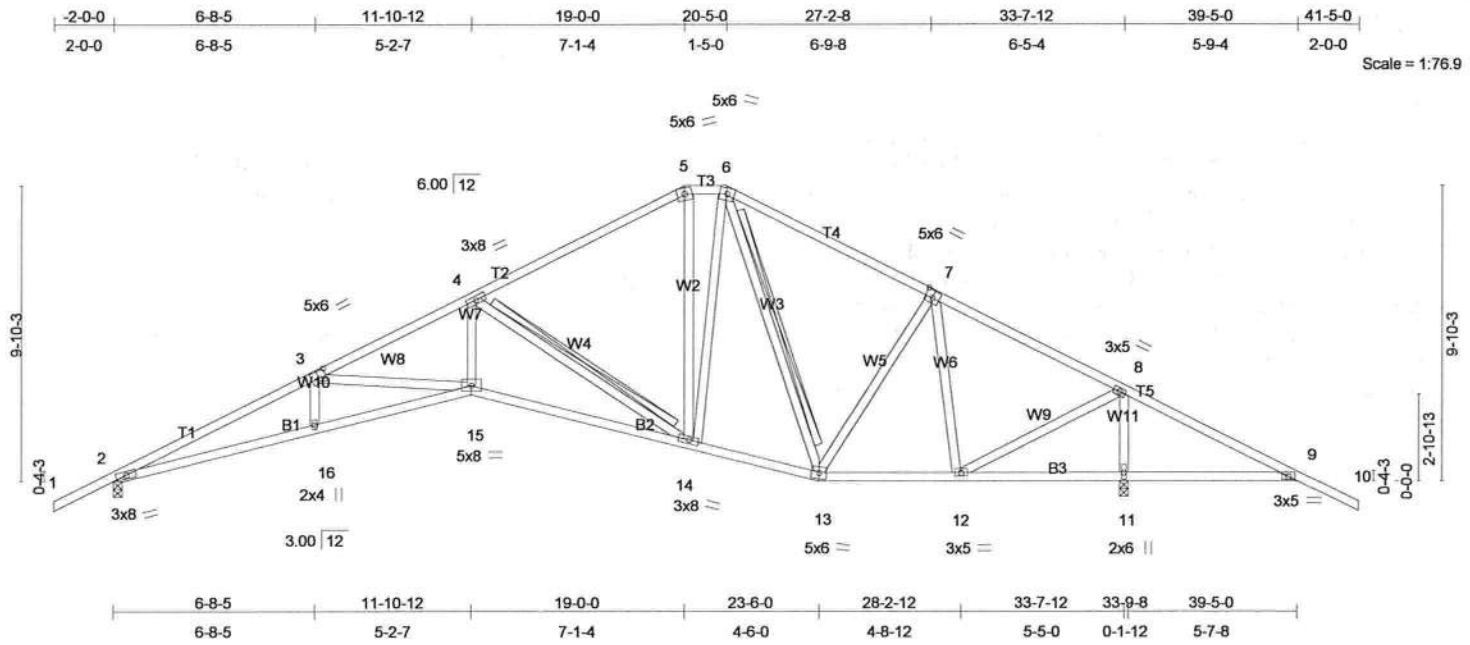

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.31	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.56	15-16	>712	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.32	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 229 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-3 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-14, 6-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1138/0-3-8, 11=1599/0-3-8
Max Horz 2=-148(load case 7)
Max Uplift 2=-331(load case 6), 11=-577(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3431/1543, 3-4=-2829/1255, 4-5=-1136/633, 5-6=-945/648, 6-7=-949/581, 7-8=-768/281, 8-9=-803/675, 9-10=0/47
BOT CHORD 2-16=-1227/3064, 15-16=-1227/3064, 14-15=-832/2561, 13-14=-82/904, 12-13=-73/693, 11-12=-535/854, 9-11=-535/854
WEBS 3-16=0/171, 3-15=-502/386, 4-15=-496/1468, 4-14=-1856/869, 5-14=-75/281, 6-14=-189/625, 6-13=-320/85, 7-13=-71/165, 7-12=-572/479, 8-12=-892/1306, 8-11=-1522/1159

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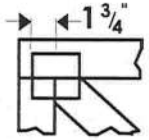
June 16,2008

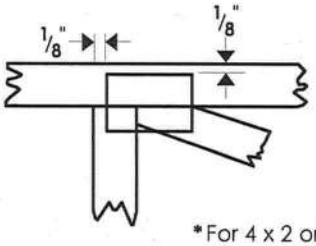
Continued on page 2

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

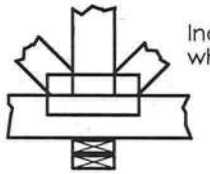
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

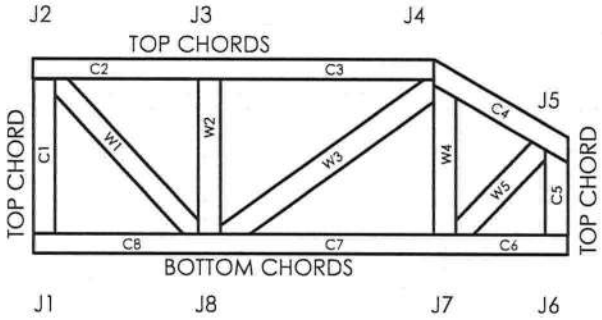
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 6"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 8"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

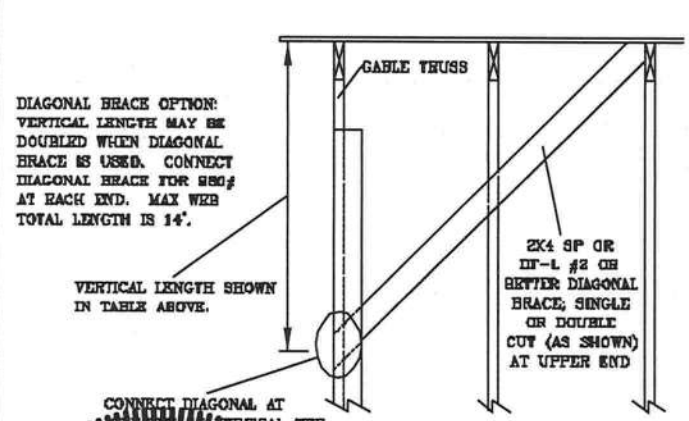
LIVE LOAD DEFLECTION CRITERIA IS L/240.

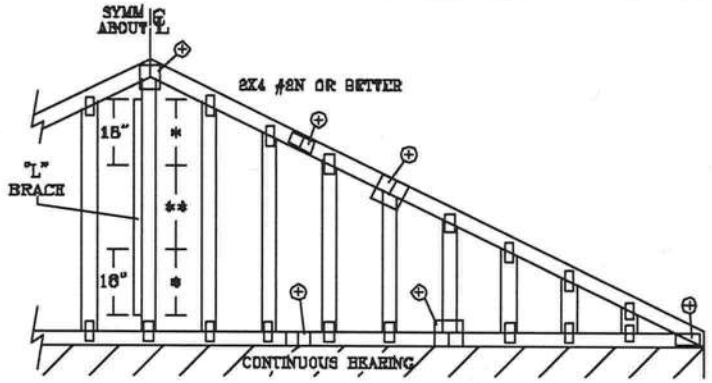
PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13030
DATE	11/26/03
DWG	MITEK STD GABLE 30' E M7
	-ENG

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

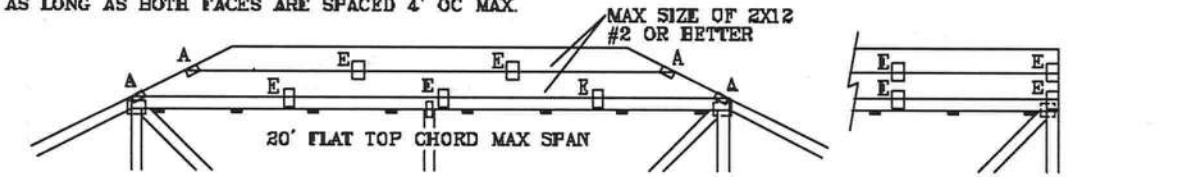
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

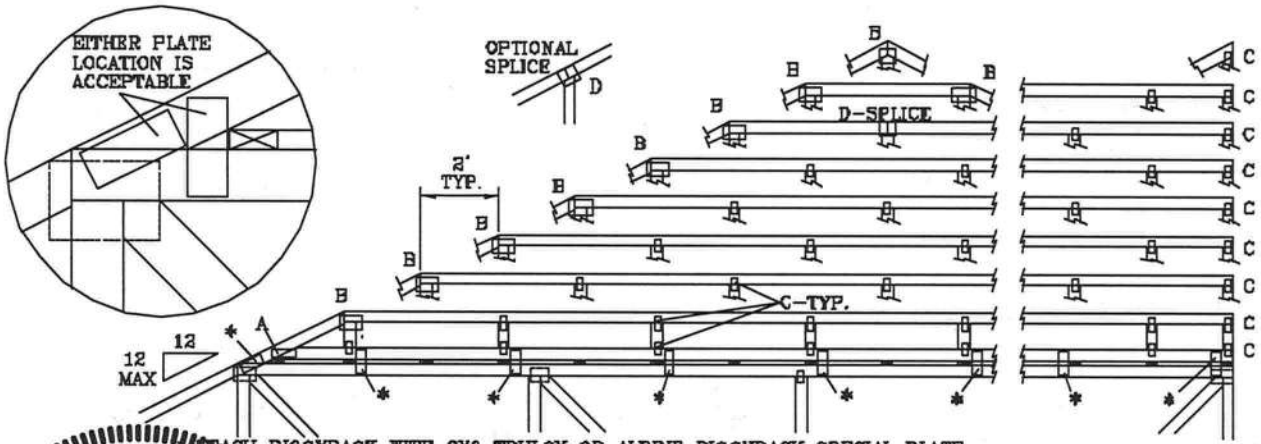
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





ATTACH PIGGYBACK WITH 3X6 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

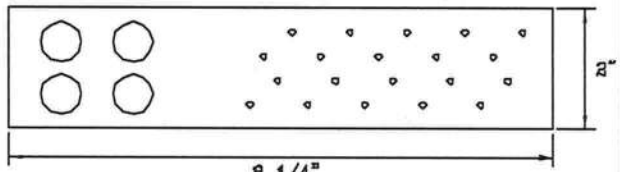
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.6X4	2.6X4	3X6
B	4X6	6X6	6X6	6X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	6X6	6X5	6X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI, 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

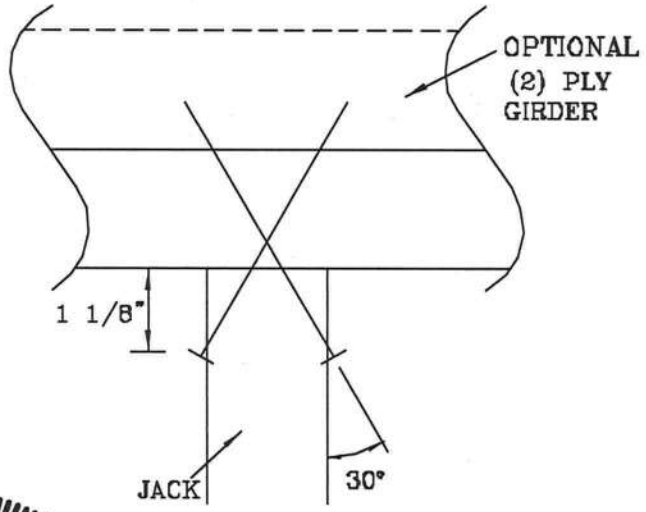
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

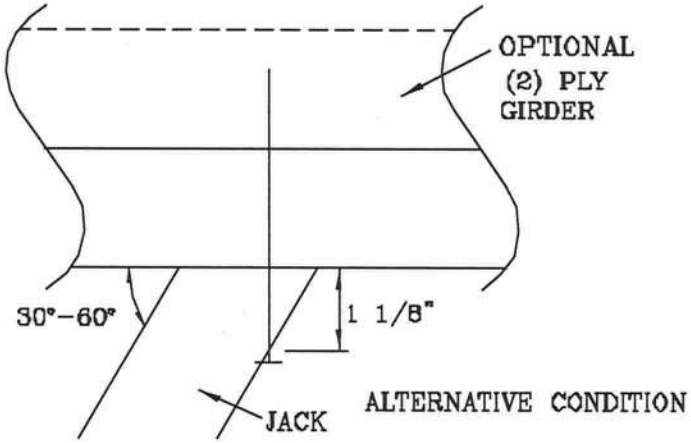
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1450 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

TRULOX CONNECTION DETAIL

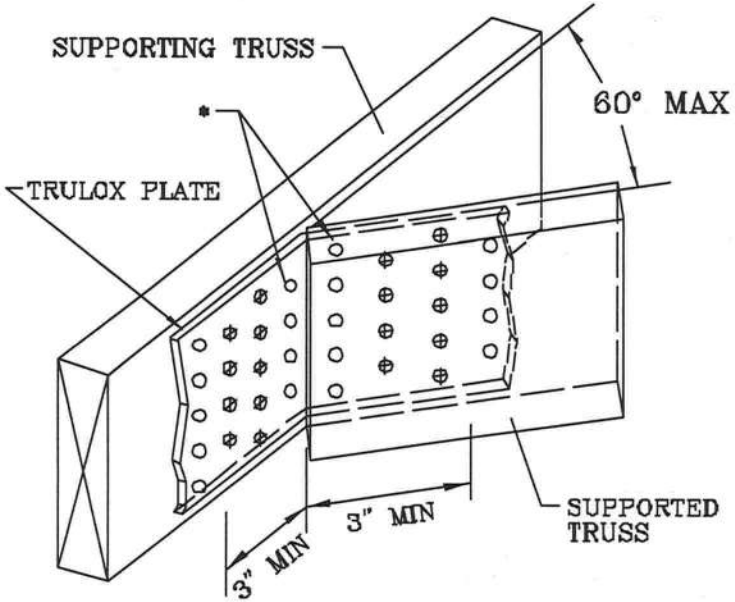
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

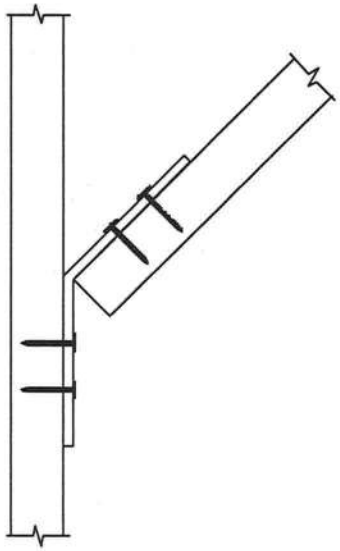
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

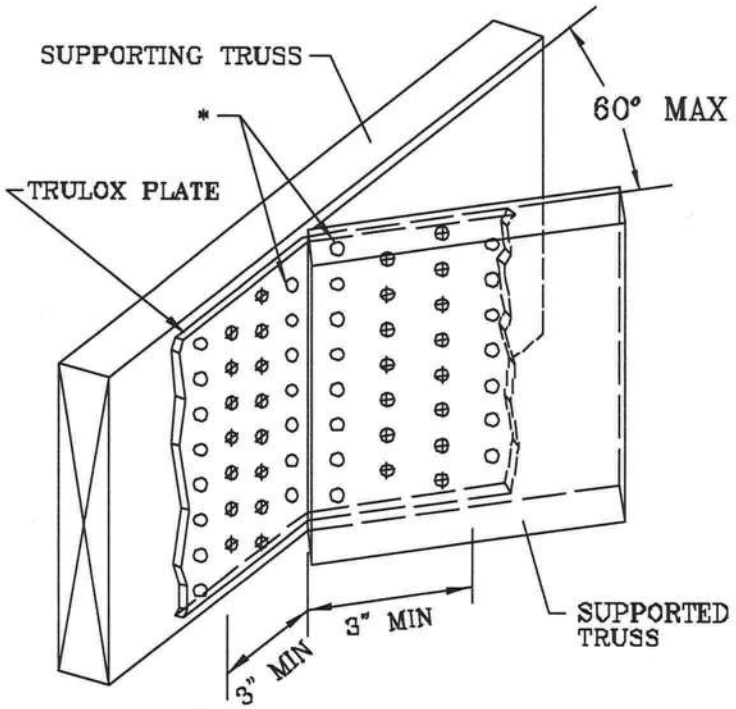
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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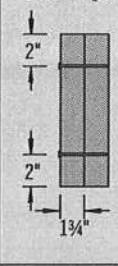
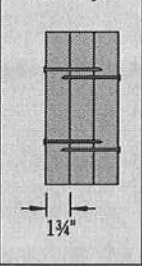
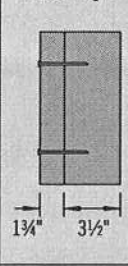
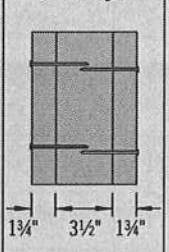
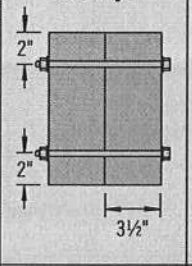
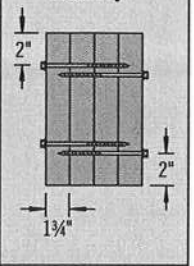
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A 	Assembly B 	Assembly C 	Assembly D 	Assembly E 	Assembly F 
			3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail[1]	2	12"	370	***280***	280	***245***		
	3	12"	555	***415***	415	***370***		
½" A307 Through Bolts[2][4]	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS ¼" x 3½"[4]	2	24"	680	***510***	510	***455***		
		19.2"	850	***640***	640	***565***		
		16"	1,020	***765***	765	***680***		
SDS ¼" x 6"[3][4]	2	24"				***455***	***465***	***455***
		19.2"				***565***	***580***	***565***
		16"				***680***	***695***	***680***
USP WS35 [4]	2	24"	480	***360***	360	***320***		
		19.2"	600	***450***	450	***400***		
		16"	715	***540***	540	***480***		
USP WS6 [3][4]	2	24"				***350***	***525***	***350***
		19.2"				***440***	***660***	***440***
		16"				***525***	***790***	***525***
3⅜" TrussLok[4]	2	24"	635	***475***	475	***425***		
		19.2"	795	***595***	595	***530***		
		16"	955	***715***	715	***635***		
5" TrussLok[4]	2	24"		***500***	500	***445***	***480***	***445***
		19.2"		***625***	625	***555***	***600***	***555***
		16"		***750***	750	***665***	***725***	***665***
6¾" TrussLok[4]	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 9⁄16" maximum.

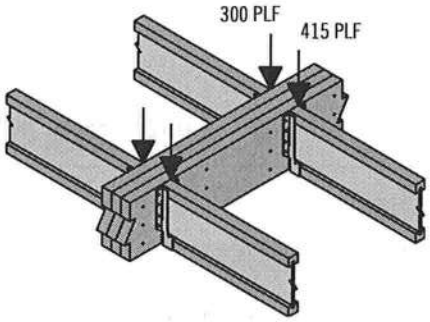
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- ***Bold Italic*** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by ½ the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of ½" bolts or SDS ¼" x 3½" screws at 19.2" on-center.

INQUIRY: Next Previous First Last Change Delete Exit
View next vendor.

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+-------------------------------------------------------------------------------+
|Vendor ID:  37170                                                              |
|Name:  BFS LAKE CITY YARD TRANSFER     Pay:  BFS LAKE CITY YARD TRANSFER       |
|Sort:  BFS LAKE CITY YARD TRANSFER    Addr:  2525 E DUVAL STREET               |
|Addr:  2525 E DUVAL STREET            Addr:                                    |
|    :                                 City:  LAKE CITY                         |
|City:  LAKE CITY                      St,Z:  FL                 32055          |
|St,Z:  FL                32055      TrCst:     Tr Markup:                     |
|Phone #:                Fax #:                Vendor Ref:                     |
|Vendor Parent ID :                Fax PO to Vendor   :  N                      |
|Vendor Status    :  A             Receive 1099?      :  N   Format :           |
|                                  Cap Vendor?        :  N                      |
|Vendor Type      :  TRANSFER      Federal ID         :                         |
|Payment Status   :  H             G/L Table ID       :  I/C                    |
|Vendor Terms Code:  NOW           Allocation Group ID:                         |
|Payment Date Code:  D             Vendor Pay Class   :                         |
|Shipping Method  :                Balance            :            0.00        |
|-------------------------------------------------------------------------------|
|                                                                               |
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SHIFT F1 - Multiple Term Codes

Cal-Tech Testing, Inc.

• Engineering
• Geotechnical
• Environmental
Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

27207

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00382
DATE TESTED: 7/30/08
DATE REPORTED: 7/30/08

PROJECT:	Lot 4 Chadworth Sub-division
CLIENT:	Catalina Caststone Creations, 377 SW Mauldin Ave., Lake City, FL 32024
GENERAL CONTRACTOR:	Catalina Caststone Creations
EARTHWORK CONTRACTOR:	Catalina Caststone Creations
INSPECTOR:	Pam Geiger

ASTM METHOD: (D-2922) Nuclear

SOIL USE: BUILDING FILL

SPECIFIED REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft^3)	MOISTURE PERCENT	DRY DENSITY (lb/ft^3)	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
1		18'E X 10'N OF SW CORNER	0-12"	124.7	11.1	112.2	07-528-1	115.9	97%
2		14'W X 16'S OF NE CORNER	0-12"	124.7	11.4	111.9	07-528-1	115.9	97%
3		8'W X 15'N OF SE CORNER	0-12"	123.5	9.5	112.8	07-528-1	115.9	97%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft^3)	OPT. MOIST.	TYPE
07-528-1		Tan Sand w/Trace of Clay	115.9	10.8	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO
SW

Reviewed By:

Date: 7/30/08
Licensed, Florida No: 57842

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 14-3S-16-02123-043 **Building permit No.** 000027207

Use Classification SFD/UTILITY **Fire:** 38.52

Permit Holder LOUIS SCHWARTZ **Waste:** 100.50

Owner of Building BRJ VENTURES,LLC. **Total:** 139.02

Location: 219 NW MELANIE WAY, LAKE CITY, FL 32055

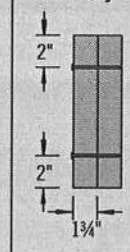
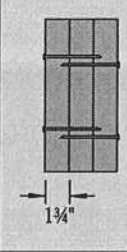
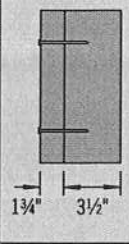
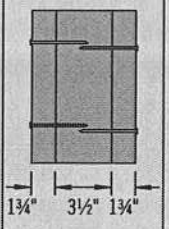
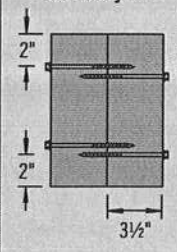
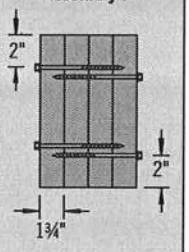
Date: 04/07/2009

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern: Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	*835*	835	*740*		
	12	2,225	*1,670*	1,670	*1,485*		
	18	3,335	*2,505*	2,505	*2,225*		
	24	4,450	*3,335*	3,335	*2,965*		
SDS Screws ¼" x 3½" or WS35 ¼" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	*1,275*	*1,860(2)*	*1,405(2)*
	6	2,870	2,150 (4)	2,150	*1,915*	*2,785(2)*	*2,110(2)*
	8	3,825	2,870 (4)	2,870	*2,550*	*3,715(2)*	*2,810(2)*
3⅜" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	*1,695*	*1,925(3)*	*1,775(3)*
	6	3,815	2,860 (4)	2,860	*2,545*	*2,890(3)*	*2,665(3)*
	8	5,090	3,815 (4)	3,815	*3,390*	*3,855(3)*	*3,550(3)*

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

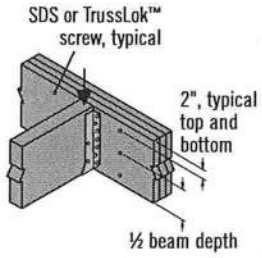
(2) 6" long screws required.

(3) 5" long screws required.

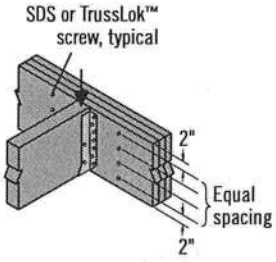
(4) 3½" and 3⅜" long screws must be installed on both sides.

Connections

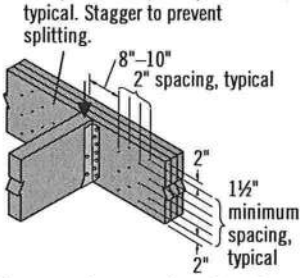
4 or 6 or Screw Connection



8 Screw Connection

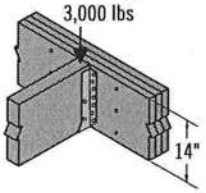


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3⅜" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

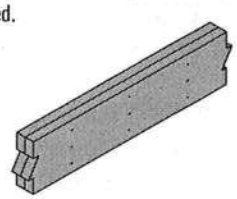
MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1¾" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3⅜" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

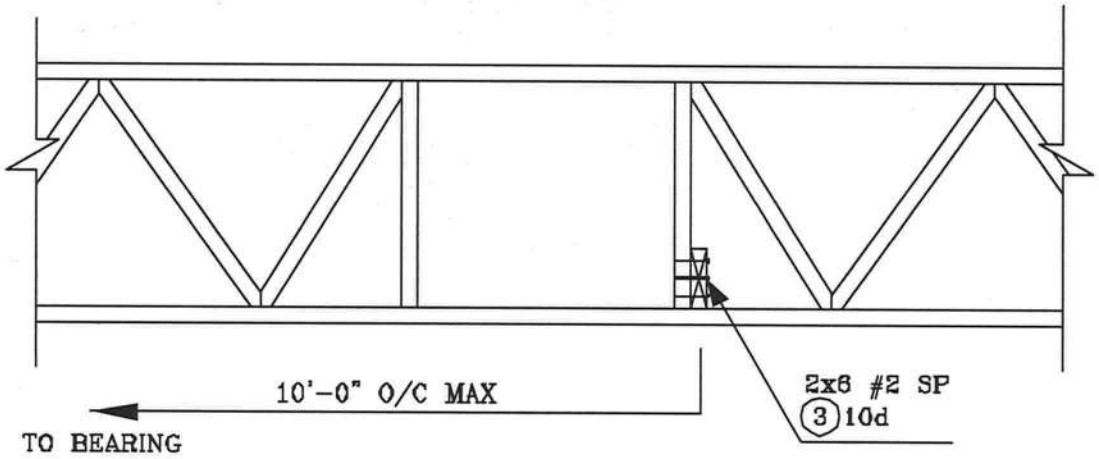
3½" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.

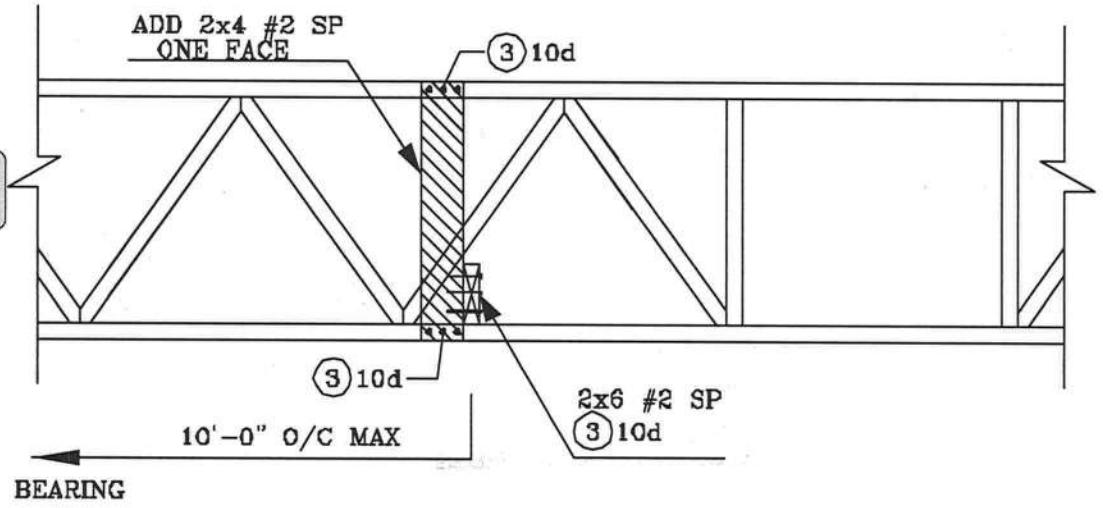


L6 ***Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"***

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

JULIUS LEE
LICENSE
NO. 34869
STATE OF FLORIDA
PROFESSIONAL ENGINEER

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

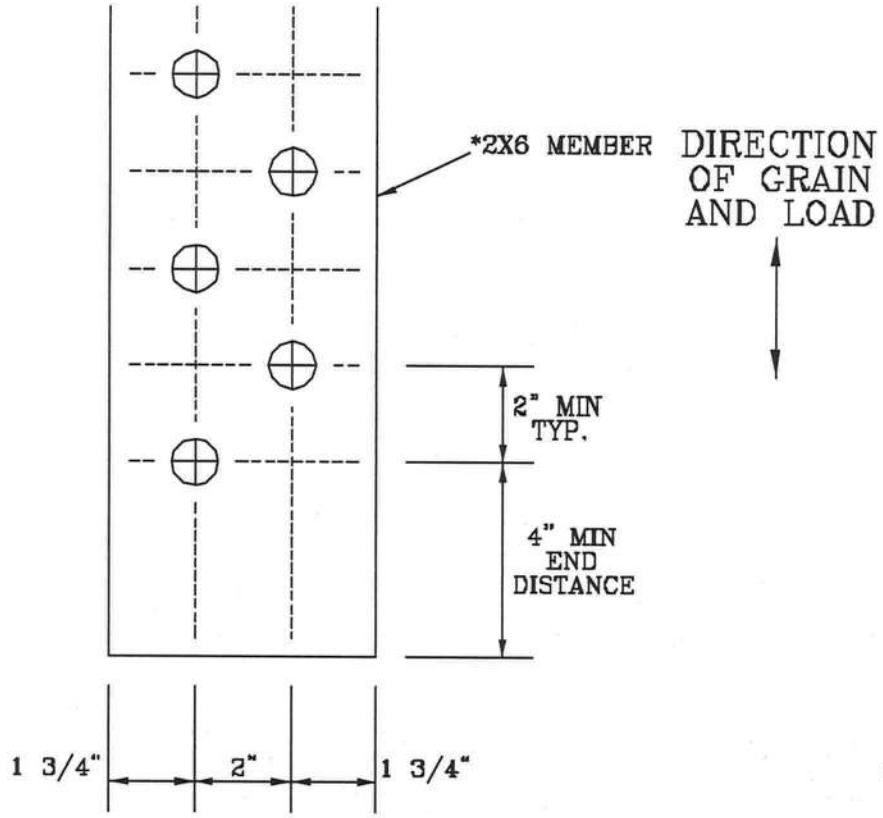
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

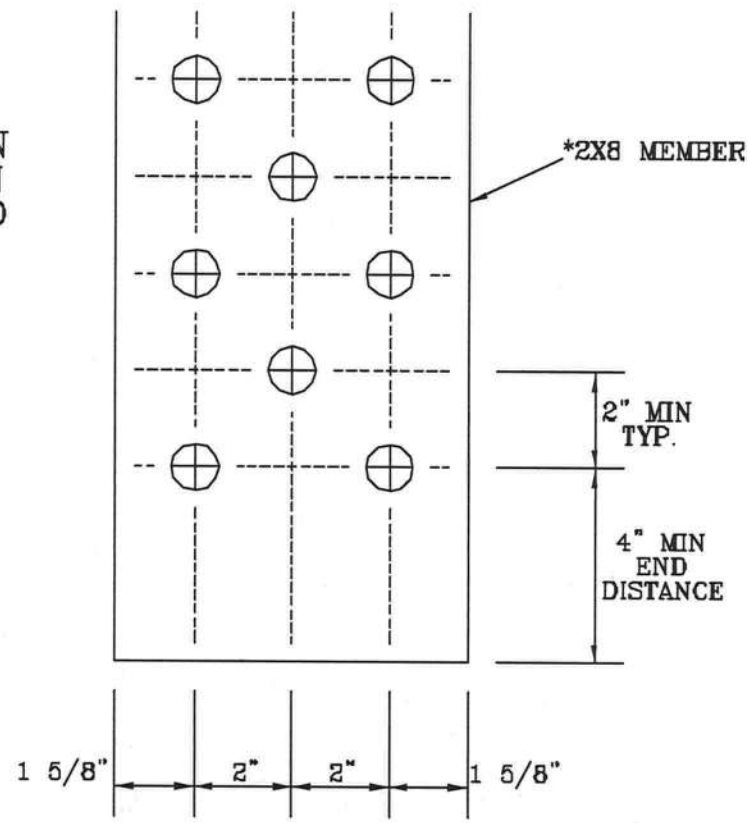
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628,016

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF BOLT SPACING
TC DL	PSF	DATE 11/26/03
BC DL	PSF	DRWG CNBOLTSP1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

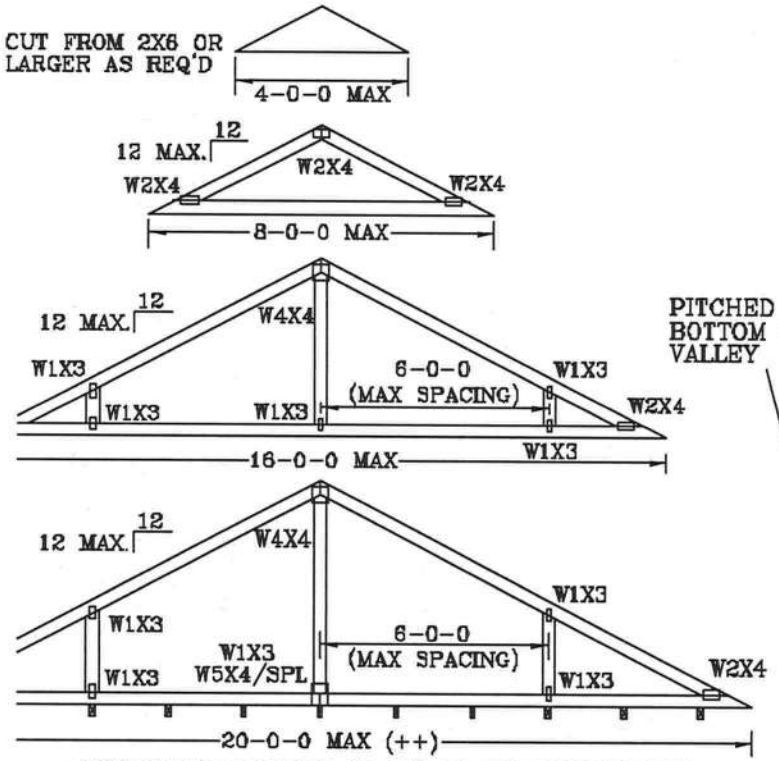
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

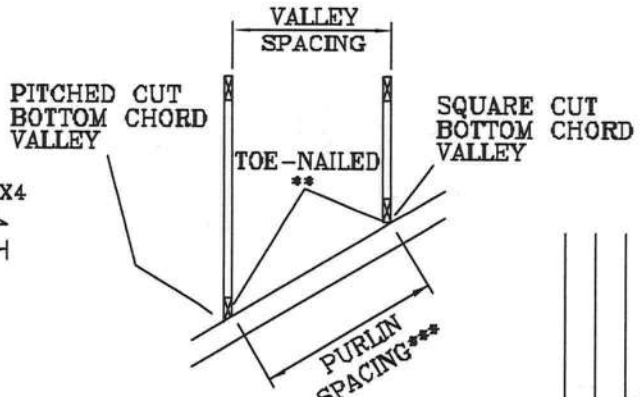
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

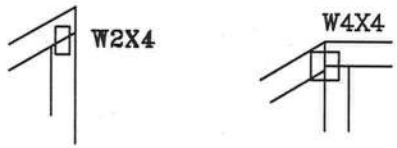
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

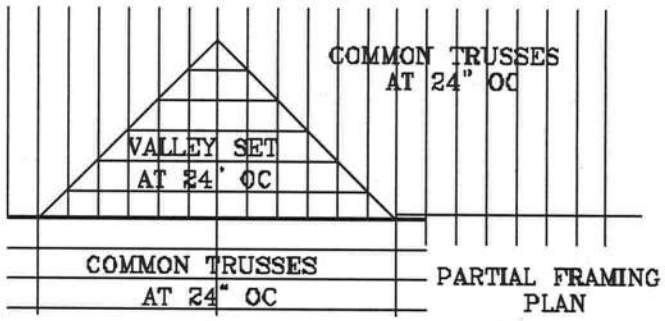
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

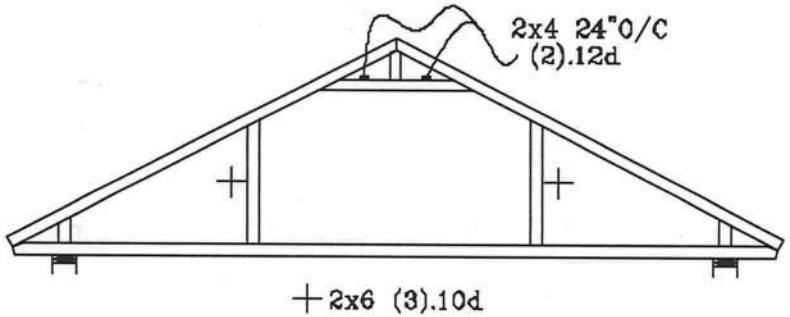
REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

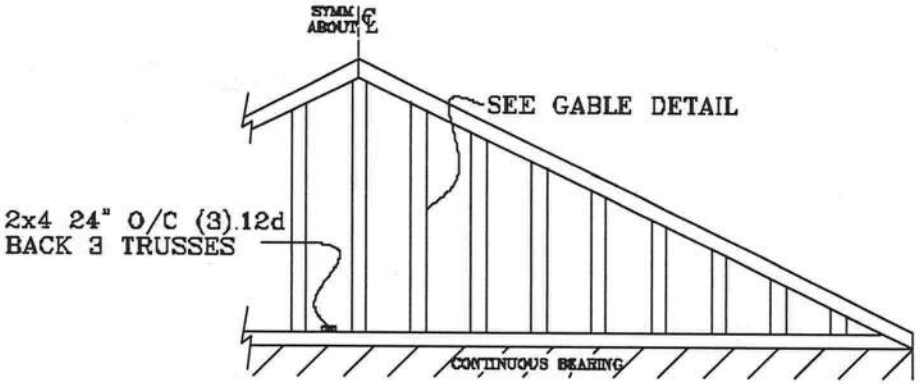
No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TYPICAL ATTIC TRUSS BRACING

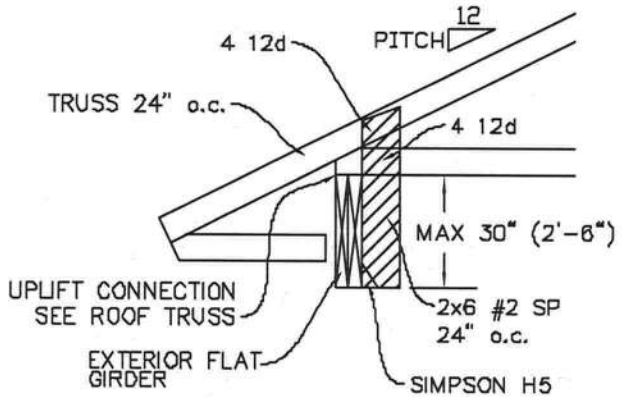


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

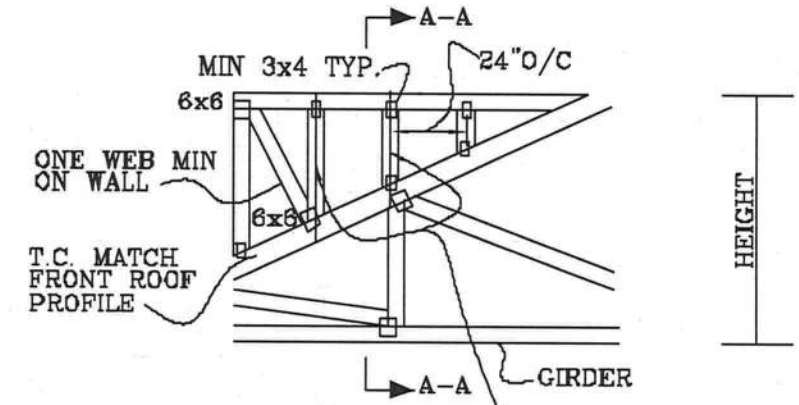
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

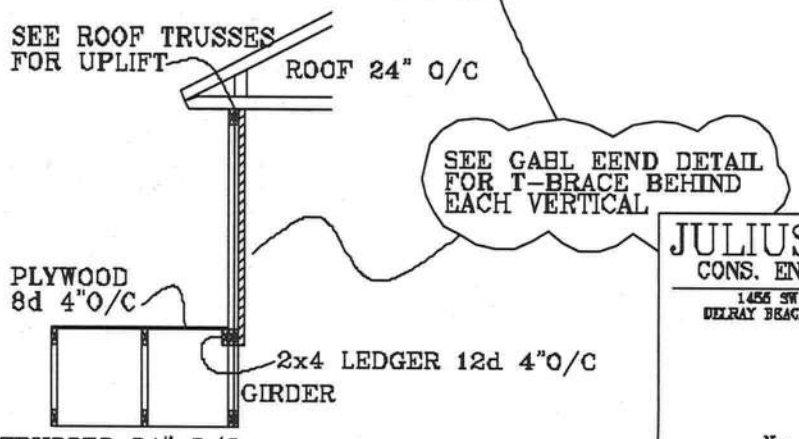


REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





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1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 6"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 6"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 5"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 8"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 8"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

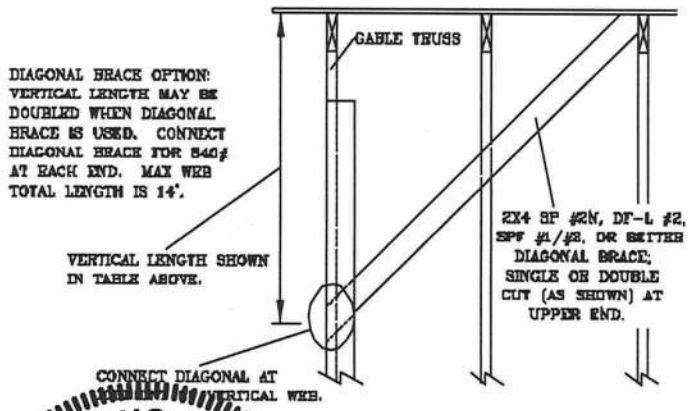
ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

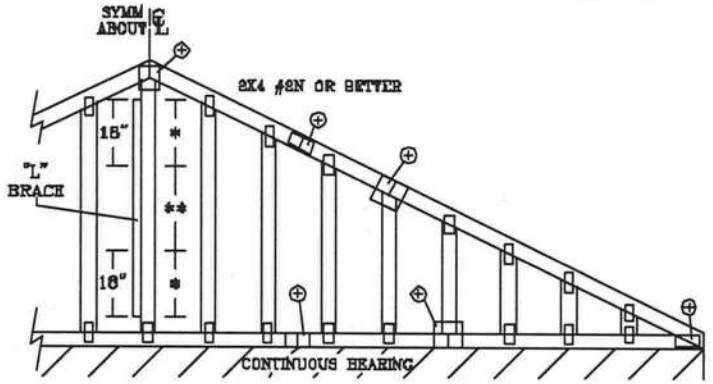
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 540# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13015
DATE	11/26/03
DRWG	MITEK STD GABLE 15 E HT
-ENG	

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973322
L279639	T19	SPECIAL	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.79, 3 = 0.49, 4 = 0.84, 5 = 0.66, 6 = 0.58, 7 = 0.65, 8 = 0.79, 9 = 0.56, 11 = 0.55, 12 = 0.81, 13 = 0.28, 14 = 0.92, 15 = 0.93 and 16 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 2 and 577 lb uplift at joint 11.

LOAD CASE(S) Standard

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June 16,2008

JOINT STRESS INDEX

2 = 0.79, 3 = 0.53, 4 = 0.83, 5 = 0.43, 6 = 0.43, 7 = 0.45, 8 = 0.79, 9 = 0.54, 11 = 0.54, 12 = 0.74, 13 = 0.42, 14 = 0.85, 15 = 0.93 and 16 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 2 and 566 lb uplift at joint 11.

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973320
L279639	T17	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2 and 549 lb uplift at joint 10.

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973319
L279639	T16	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 535 lb uplift at joint 10.

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973318
L279639	T15	HIP	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 210 lb uplift at joint 9.

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973317
L279639	T14	HIP	1	1	Job Reference (optional)

NOTES

3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 2 and 241 lb uplift at joint 10.

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973316
L279639	T13	HIP	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.81, 3 = 0.98, 4 = 0.62, 5 = 0.81, 6 = 0.84, 7 = 0.85, 8 = 0.86, 9 = 0.61, 10 = 0.92, 11 = 0.82, 12 = 0.90 and 13 = 0.91

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 716 lb uplift at joint 2 and 787 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-118(F=-64), 7-8=-118(F=-64), 2-13=-10, 9-13=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 13=-411(F)

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973315
L279639	T12	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 2 and 239 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-6=-54, 6-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973314
L279639	T11	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2 and 272 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-6=-54, 6-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973313
L279639	T10	SPECIAL	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 2 and 221 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-7=-54, 7-8=-54, 2-12=-10, 10-12=-10, 9-10=-10
   Concentrated Loads (lb)
   Vert: 9=-30(F) 8=-54(F)

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973312
L279639	T09	SPECIAL	1	1	
					Job Reference (optional)

NOTES

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2 and 281 lb uplift at joint 9.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-11=-10, 9-11=-10, 8-9=-10
   Concentrated Loads (lb)
   Vert: 8=-30(F) 7=-54(F)

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973311
L279639	T08	SPECIAL	1	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 2 and 284 lb uplift at joint 9.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-11=-10, 9-11=-10, 8-9=-10
   Concentrated Loads (lb)
   Vert: 7=-54(F) 8=-30(F)

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973310
L279639	T07	MONO HIP	1	1	Job Reference (optional)

NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 8 and 248 lb uplift at joint 2.

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973309
L279639	T06	MONO HIP	1	1	Job Reference (optional)

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 649 lb uplift at joint 8 and 578 lb uplift at joint 2.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)
   Concentrated Loads (lb)
   Vert: 12=-411(F)

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June 16,2008

NOTES

3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 964 lb uplift at joint 6 and 585 lb uplift at joint 2.
8) Girder carries tie-in span(s): 33-9-8 from 8-0-0 to 12-7-0

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 2-11=-10, 6-11=-519(F=-509)
   Concentrated Loads (lb)
   Vert: 10=-2363(F)

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973307
L279639	T04	COMMON	3	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 4 and 186 lb uplift at joint 2.

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973306
L279639	T03G	GABLE	1	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 319 lb uplift at joint 2, 332 lb uplift at joint 8, 52 lb uplift at joint 11, 205 lb uplift at joint 12 and 208 lb uplift at joint 10.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-10

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
L279639	T03	COMMON	3	1	J1973305
					Job Reference (optional)

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973304
L279639	T02	COMMON	6	1	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 2-9=-10, 7-9=-70(F=-60), 6-7=-10

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973303
L279639	T01G	GABLE	1	1	Job Reference (optional)

NOTES

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 796 lb uplift at joint 2 and 796 lb uplift at joint 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-12=-10, 10-12=-40(F=-30), 8-10=-10

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

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Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973301
L279639	HJ9	MONO TRUSS	2	1	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 1-2=-54
   Trapezoidal Loads (plf)
      Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

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June 16,2008

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH J1973299
L279639	CJ5	JACK	4	1	Job Reference (optional)

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
L279639	CJ3	JACK	4	1	J1973298
					Job Reference (optional)

LOAD CASE(S) Standard

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June 16,2008

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK - JOHN & PAM SMITH
					J1973297
L279639	CJ1	JACK	4	1	Job Reference (optional)

LOAD CASE(S) Standard

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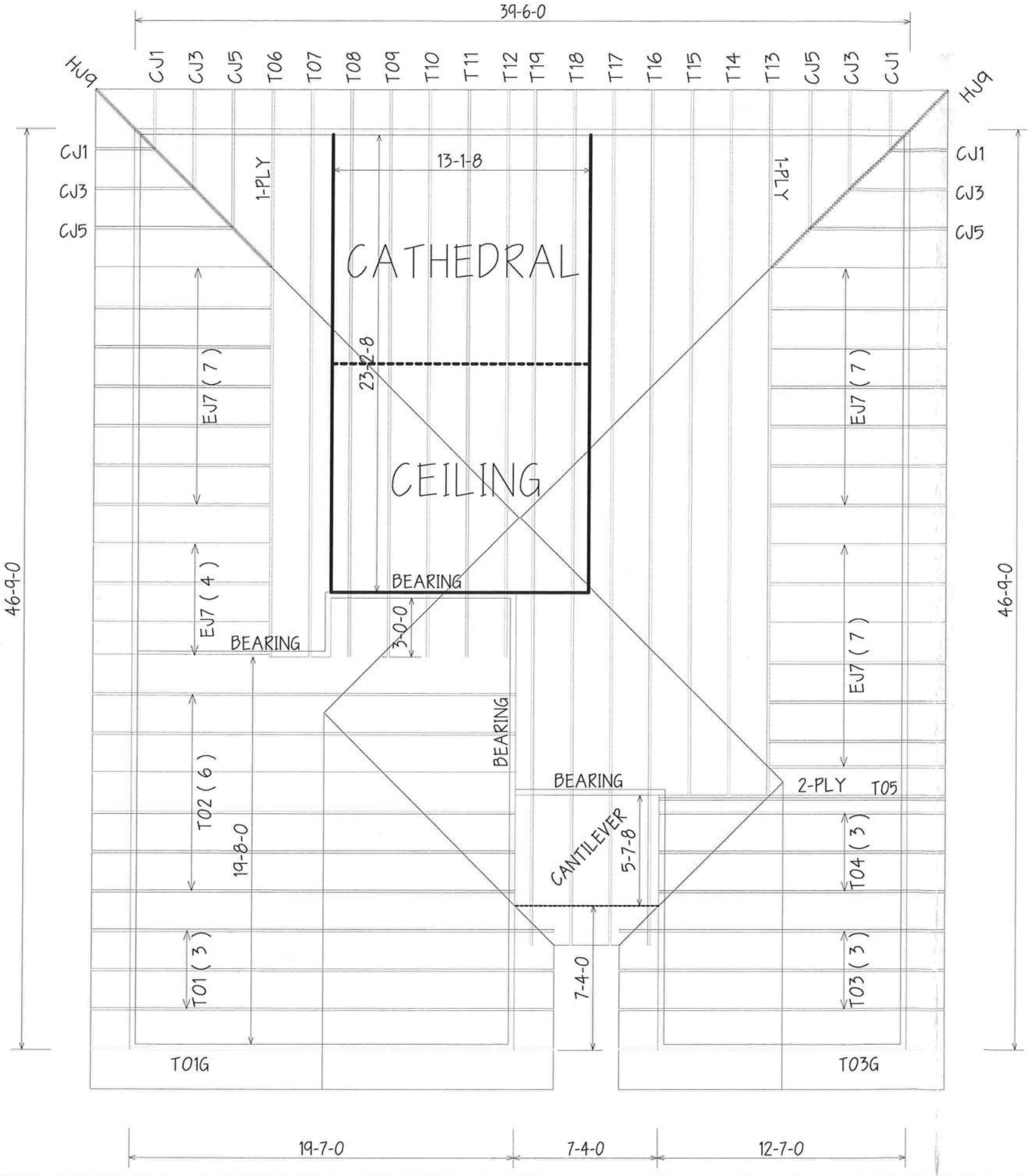
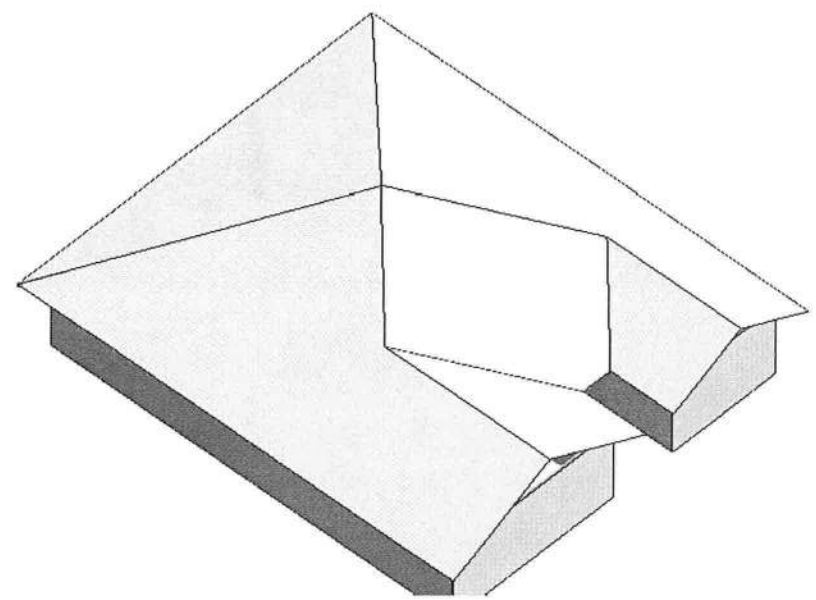
June 16,2008



6/12 PITCH
2'0" O/H



BEARING HEIGHT SCHEDULE

8'-1 1/8"

HANGER SCHEDULE
3 - HTU26

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 386-755-6894 FAX: 386-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: JOHN & PAM SMITH
LEGAL ADDRESS:

MODEL:	REVISION:
CUSTOM	SCALE: NTS
DATE: 6-6-08 / DRAWN BY: K.L.H.	JOB #: L279639

NOTICE OF INSPECTION AND/OR TREATMENT

Date of Inspection
7/7/08 TH

Date of Treatment
27207

Date of Spot Treatment
Premise Pro

Pesticide Used
subterranean Termites

Wood-Destroying Organisms Treated

Notice
It is a violation of Florida State Law (Chap. 482.226) for anyone other than the property owner to remove this notice.

Address:
Pestmaster Services of Lake City
879 S.W. Arlington Blvd., Suite 106 • Lake City, FL 32025